

LEGEND

	INDICATIVE DIRECTION OF OVERLAND FLOW PATH
	INTRAPAVEMENT DRAINAGE. REFER RMS DETAIL
	SUBSOIL DRAINAGE. REFER TWEED SHIRE COUNCIL ROADWORKS STANDARD DRAWING NO. S.D.012 FOR DETAIL
	NEW STORMWATER PIPE
	NEW STANDARD GULLY PIT. REFER TWEED SHIRE COUNCIL STORMWATER STANDARDS DRAWING NO. S.D.104 FOR DETAILS
	NEW GRATED INLET PIT. REFER TWEED SHIRE COUNCIL STORMWATER STANDARDS DRAWING NO. S.D.106 DETAILS
	NEW JUNCTION PIT. REFER TWEED SHIRE COUNCIL STORMWATER STANDARDS DRAWING NO. S.D.106 DETAILS

CONCRETE ENCASEMENT.
REFER DRAWING C460 FOR DETAILS

BARRIER KERB AND GUTTER
REFER TWEED SHIRE COUNCIL ROADWORKS
STANDARD DRAWING NO. S.D.007 FOR DETAIL

KERB ONLY
REFER TYPICAL DETAIL ON DRAWING NO. 460



KERB RAMP
REFER TWEED SHIRE COUNCIL ROADWORKS
STANDARD DRAWING NO. S.D.014 FOR DETAIL

PAVEMENT TYPE 1
HEAVY DUTY ASPHALT PAVEMENT.
REFER TO DRAWING C460 FOR DETAILS

CONCRETE FOOTPATH
REFER TWEED SHIRE COUNCIL ROAD WORKS
STANDARD DRAWING NO. S.D.013 FOR DETAIL

PAVEMENT REINSTATEMENT
REFER DRAWING C460 FOR DETAILS

RETAINING WALL.
REFER TO DRAWING C460 FOR DETAILS

----- NEATLY MATCH TO EXISTING SURFACE LEVELS

LIMIT OF WORKS

— Fy S — EXISTING SEWER

— Ex OH — EXISTING OVERHEAD LINES

EXISTING TELSTRA

—Ex SW— EXISTING STORMWATER

— Ex W — EXISTING WATER

— Ex OF — EXISTING OPTICAL FIBRE

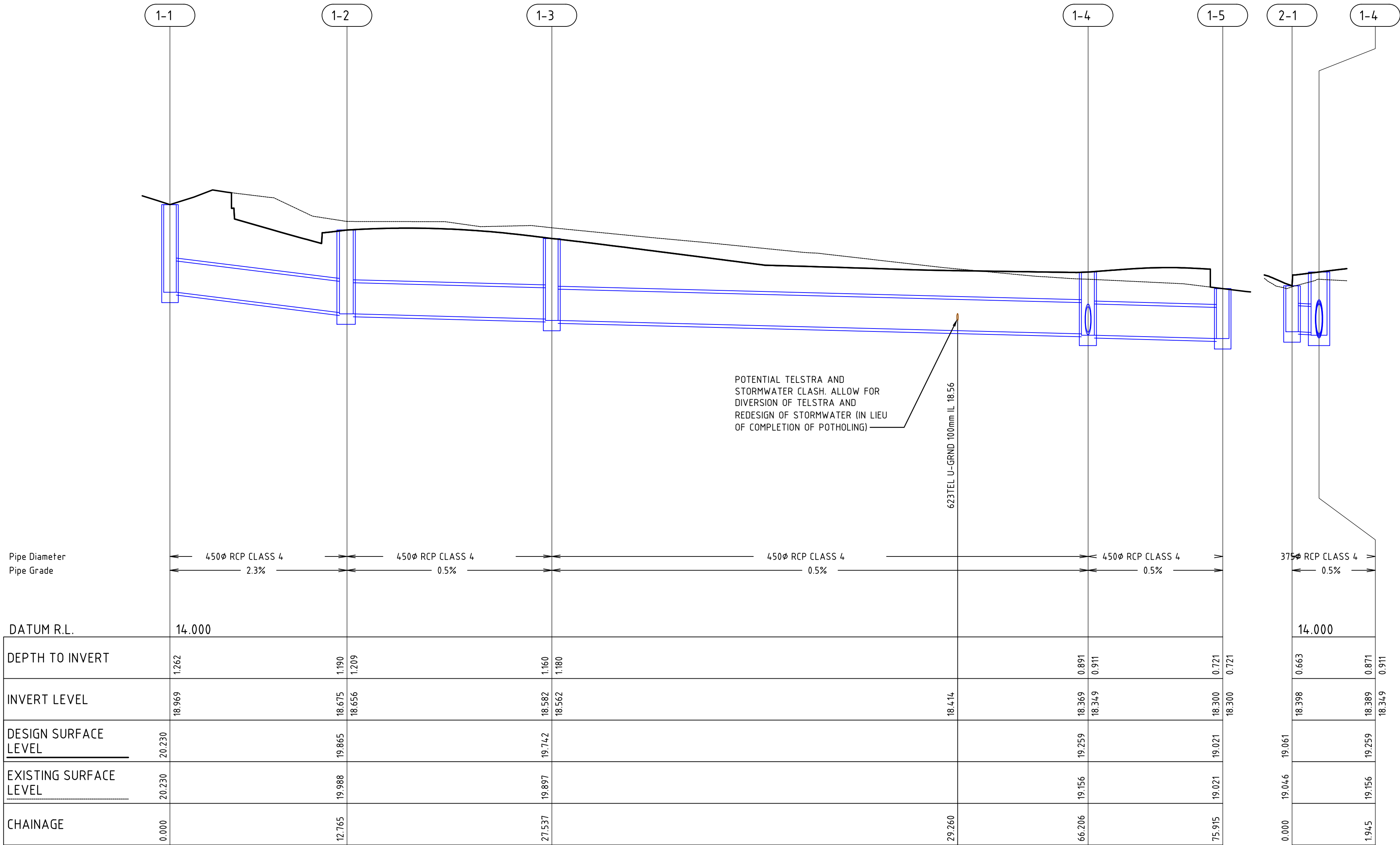


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Project Name	TWEED VALLEY HOSPITAL DEVELOPMENT, CUDGEN ROAD AUXILIARY LANE WORKS
Drawing Title	AUXILIARY LANE STORMWATER DRAINAGE PLAN

TENDER				
Designed	CS	Project Director Approved	Date	North 
Drawn	PA			
Scale	1:200			
Date	05.11.18	Project Ref	Drawing No	Rev
Sheet	A1	20 10748 01	C442	P1

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P1	ISSUED FOR TENDER	05.11.18	PA

Rev	Description	Date	By	App	

BONACCI

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**TWEED VALLEY HOSPITAL
CUDGEN ROAD AUXILIARY
LANE WORKS**

**AUXILIARY LANE
STORMWATER DRAINAGE
LONGITUDINAL SECTION**

TENDER

Designed CS

Drawn PA

Scale NOTED

Date 05.11.18

Sheet A1

Project Director Approved

Date

Project Ref

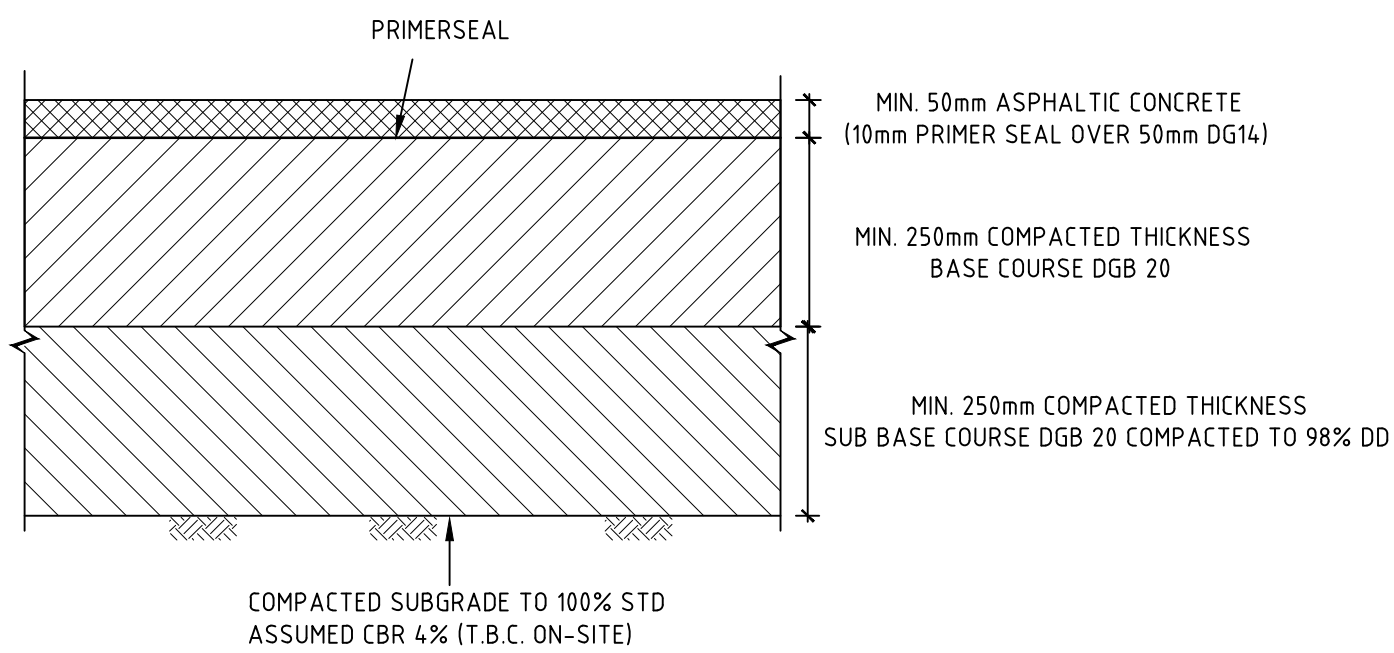
Drawing No

Rev

20 10748 01

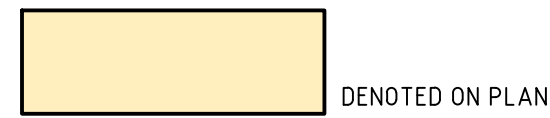
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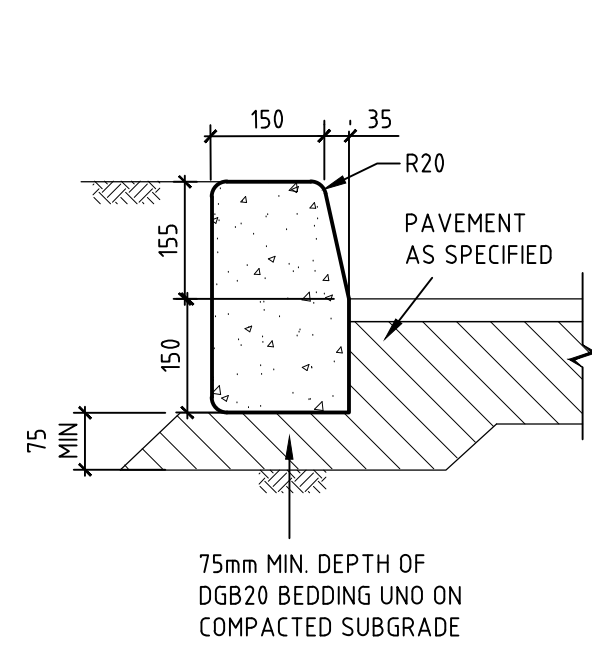


PAVEMENT TYPE P1
HEAVY DUTY ASPHALT PAVEMENT

SCALE 1:10

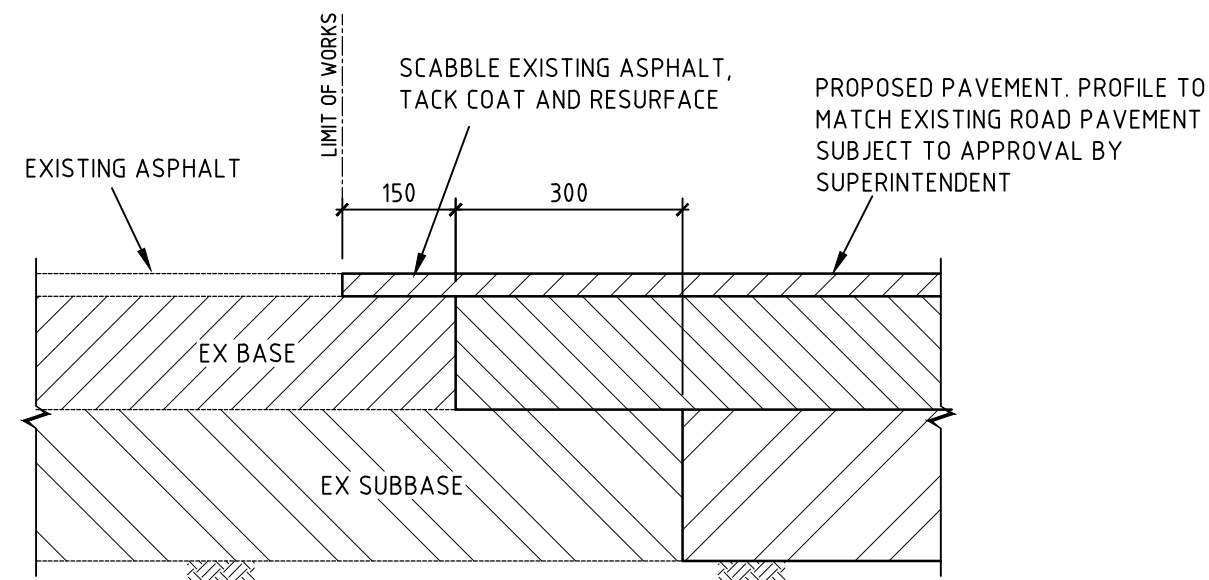


- ROAD DESIGN PARAMETERS (FLEXIBLE)**
DESIGN TRAFFIC 1.5 X 10⁶ (ESA's OR HVAG)
ASSUMPTIONS:
- TWEED SHIRE COUNCIL PAVEMENT DESIGN TABLE D2.2 ARTERIAL DISTRIBUTOR, SHOPPING STRIP ACCESS, INDUSTRIAL, DESIGN PERIOD 25 YEAR.
 - CBR 4.0%
 - PREPARE SUBGRADE AND SELECT FILL IN ACCORDANCE WITH (MORRISON GEOTECH, REFERENCE NO. GE18/144-REV 1)



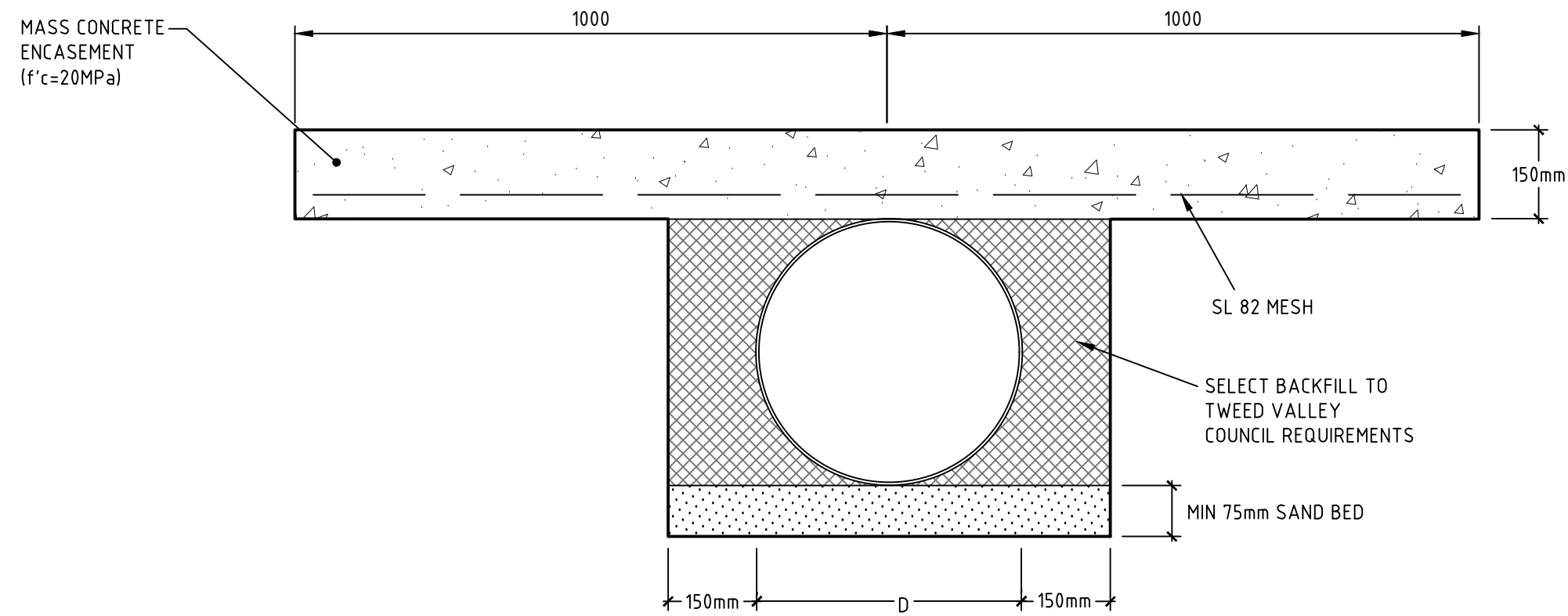
KERB ONLY (KO)

SCALE 1:10
(TO BE APPROVED BY COUNCIL)



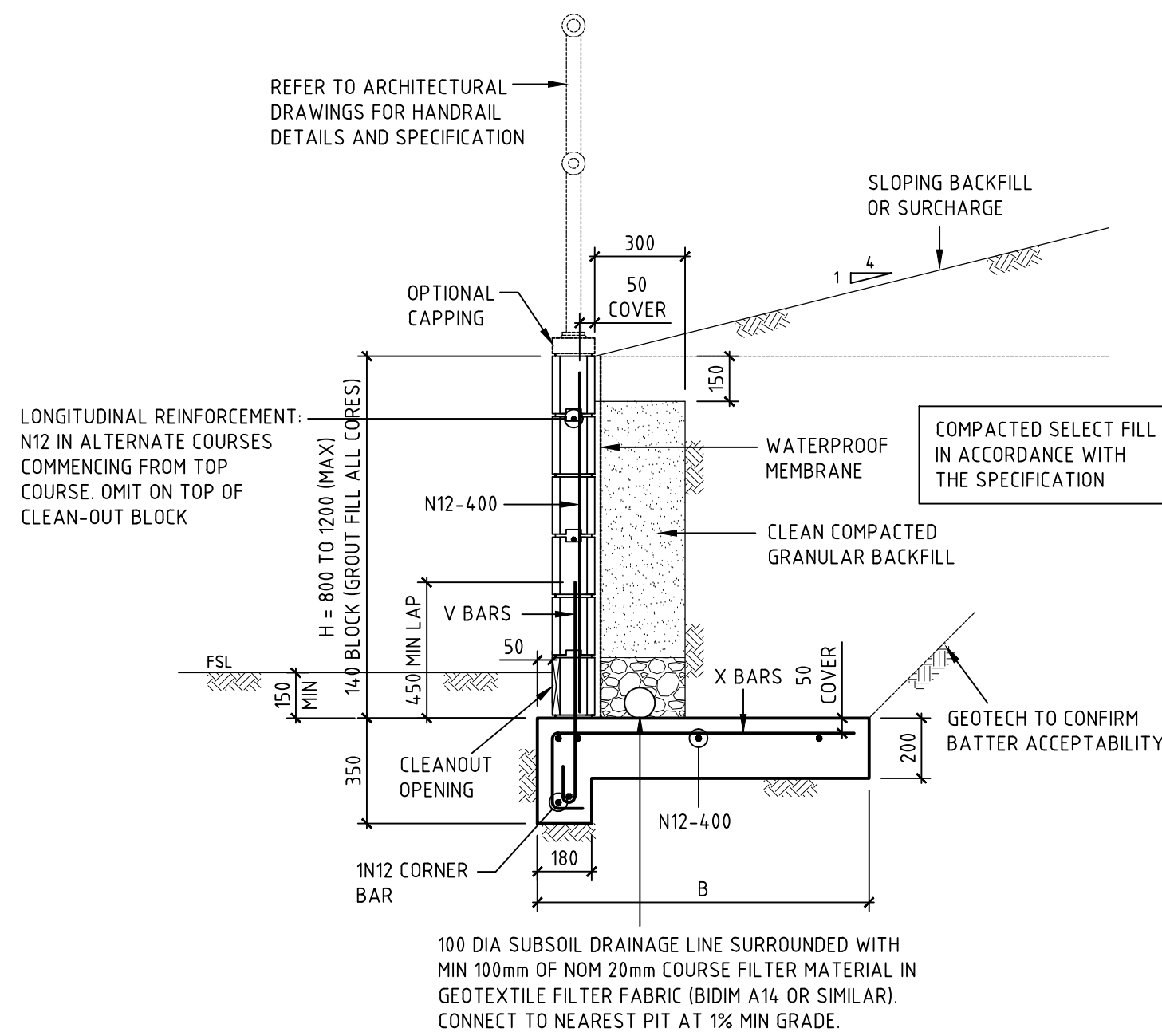
AC CONNECTION TO EXISTING PAVEMENT

SCALE 1:10



CONCRETE ENCASEMENT DETAIL

SCALE 1:10



BLOCK RETAINING WALL (MAX 1200 HIGH)

SCALE 1:20

NOTE: DESIGNER TO CHECK THE NEED FOR SHEAR KEY

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Rev Description Date By App

P2	ISSUED FOR TENDER	05.11.18	PA	-
P1	ISSUED FOR DRAFT TENDER	02.11.18	PA	-

Rev Description Date By App

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Project Name
**TWEED VALLEY HOSPITAL
CUDGEN ROAD AUXILIARY
LANE WORKS**

Drawing Title
**CIVIL WORKS AND
STORMWATER DRAINAGE DETAILS**

TENDER

Designed	CS	Project Director Approved	Date	North
Drawn	PA			
Scale	NOTED	Project Ref	Drawing No	Rev
Date	05.11.18	20 10748 01	C460	P2
Sheet	A1			



SLIP LANE KR01 CL LONGITUDINAL SECTION

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Project Name	TWEED VALLEY HOSPITAL CUDGEN ROAD AUXILIARY LANE WORKS		TENDER		
Drawing Title	Designed	CS	Project Director Approved	Date	North
	Drawn	PA			
	Scale	NOTED			
	Date	05.11.18			
	Sheet	A1			
	Project Ref	Drawing No	Rev		
		20 10748 01	C485	P1	

TWEED VALLEY HOSPITAL DEVELOPMENT TURNOCK STREET ROUNDABOUT WORKS

<u>DRAWING No.</u>	<u>DESCRIPTION</u>
20 10748 C500	DRAWING REGISTER AND CONSTRUCTION NOTES
20 10748 C505	SEDIMENT AND EROSION CONTROL PLAN
20 10748 C540	EXTERNAL WORKS INTERSECTION PLAN
20 10748 C545	DEMOLITION PLAN
20 10748 C560	CIVIL WORKS DETAILS

GENERAL NOTES

- 01 THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ARCHITECTURAL AND OTHER CONSULTANTS
DRAWINGS AND SPECIFICATIONS AND WITH SUCH OTHER WRITTEN INSTRUCTIONS OR SKETCHES AS
MAY BE ISSUED DURING THE COURSE OF THE CONTRACT. ANY DISCREPANCY SHALL BE REFERRED TO
THE SUPERINTENDENT BEFORE PROCEEDING WITH WORK.
- 02 MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE SPECIFICATION, CURRENT SAA
CODES, BUILDING REGULATIONS AND THE REQUIREMENTS OF ANY OTHER RELEVANT STATUTORY
AUTHORITIES.
- 03 THESE DRAWINGS MUST NOT BE SCALED. ALL DIMENSIONS ARE IN METERS. ALL SET OUT DIMENSIONS
AND LEVELS, INCLUDING THOSE SHOWN ON THESE DRAWINGS SHALL BE IN ACCORDANCE WITH THE
ARCHITECT'S DRAWINGS AND VERIFIED ON SITE.
- 04 ALL SETOUT AND DIMENSIONS OF THE STRUCTURE INCLUDING KERBS AND RETAINING WALLS, AND
BULK EARTHWORKS MUST BE TAKEN FROM THE ARCHITECT'S DRAWINGS. SETOUT OF THE
STORMWATER PITS BY OTHERS. CONTRACTOR TO CONFIRM SETOUT OF SERVICE TRENCHING
INCLUDING SUBSOIL ON SITE.
- 05 THE CONTRACTOR SHALL COMPLY WITH ALL REGULATIONS OF AUTHORITIES HAVING JURISDICTION
OVER THE WORKS. REFER TO GEOTECHNICAL REPORT BY MORRISON GEOTECHNICAL PTY LTD,
REFERENCE: GE18/144 REV 2, DATED 28th SEPTEMBER 2018.
- 06 ALL DIMENSIONS AND REDUCED LEVELS MUST BE VERIFIED ON SITE BEFORE THE COMMENCEMENT OF
ANY WORK.
- 07 THE APPROVAL OF A SUBSTITUTION SHALL BE SOUGHT FROM THE SUPERINTENDENT BUT IS NOT AN
AUTHORISATION OF A COST VARIATION. THE SUPERINTENDENT MUST APPROVE ANY COST
VARIATION INVOLVED BEFORE ANY WORK STARTS.
- 08 ALL LEVELS SHOWN ARE TO THE AUSTRALIAN HEIGHT DATUM.
- 09 SERVICE INFORMATION SHOWN IS APPROXIMATE ONLY. PRIOR TO COMMENCEMENT OF ANY WORKS,
THE CONTRACTOR SHALL LOCATE ALL UNDERGROUND SERVICES AND COMPLY WITH ALL
REQUIREMENTS OF THOSE AUTHORITIES.
- 10 EXISTING SURFACE CONTOURS, WHERE SHOWN, ARE INTERPOLATED AND MAY NOT BE ACCURATE.
- 11 UNLESS NOTED OTHERWISE, ALL VEGETATION SHALL BE STRIPPED TO A MINIMUM DEPTH OF 150mm
UNDER ALL PROPOSED PAVEMENT AND BUILDING AREAS.
- 12 MAKE SMOOTH CONNECTION WITH ALL EXISTING WORKS.
- 13 THESE DWGS SHOULD BE READ IN CONJUNCTION WITH COUNCIL'S STANDARD DETAILS.

SITEWORKS NOTES

- PRIOR TO THE PLACEMENT OF ANY PAVEMENTS, BUILDINGS OR DRAINS THE EXPOSED SUBGRADE SHALL BE COMPACTED TO A MINIMUM OF 98% STANDARD COMPACTION IN ACCORDANCE WITH TEST 'E1' OF A.S. 1289 FOR THE TOP 300mm. ANY SOFT SPOTS SHALL BE REMOVED AND REPLACED WITH GRANULAR FILL TO THE ENGINEERS APPROVAL AND COMPACTED IN ACCORDANCE WITH THE COMPACTION REQUIREMENTS SET OUT BELOW. ON HIGHLY REACTIVE CLAY AREAS SITE EXCAVATED MATERIAL MAY BE USED WITH THE PRIOR AUTHORISATION OF THE ENGINEER.
- S2 ALL FILL AND PAVEMENT MATERIALS SHALL BE COMPACTED IN ACCORDANCE WITH GEOTECHNICAL REPORT BY MORRISON GEOTECHNIC PTY LTD REFERENCE: GE18/144 REV 2 DATED 28th SEPTEMBER 2018 MOISTURE CONTENT TO BE MAINTAINED AT +/- 2% OMC. MINIMUM COMPACTION REQUIREMENTS ARE DETAILLED BELOW FOR (ALL REQUIREMENTS ARE TO BE VERIFIED BY A SUITABLY QUALIFIED GEOTECHNICAL ENGINEER):
- LANDSCAPED AREAS 95% STD.
 - FILL UNDER ANY FOOTINGS AND FLOOR SLABS FOR ANY STRUCTURE TO SUBGRADE LEVEL;
 - FINE CRUSHED ROCK 98% STD.
 - SELECTED FILL WITHOUT CONSPICUOUS CLAY CONTENT 98% STD.
 - BUILDING BASECOURSE 98% MOD
 - FILL UNDER ROAD PAVEMENTS;
 - TO WITHIN 500mm OF FINISHED SUBGRADE LEVEL 98% STD.
 - UP TO FINISHED SUBGRADE LEVEL 98% STD.
 - ROAD PAVEMENT MATERIALS;
 - SUB BASE 98% MOD.
 - BASE COURSE 98% MOD.
- THE MAXIMUM COMPACTION IS TO BE NO GREAT THAN 4% ON TOP OF THE ABOVE MENTION VALUES.
- S3 GRADE EVENLY BETWEEN FINISHED SURFACE SPOT LEVELS. FINISHED SURFACE CONTOURS ARE SHOWN FOR CLARITY. WHERE FINISHED SURFACE LEVELS ARE NOT SHOWN, THE SURFACE SHALL BE GRADED SMOOTHLY SO THAT IT WILL DRAIN AND MATCH ADJACENT SURFACES OR STRUCTURES.
- S4 ALL DIMENSIONS GIVEN ARE TO FACE OF KERB, CENTER OF PIPE OR EXTERIOR FACE OF BUILDING UNLESS NOTED OTHERWISE.
- S5 ANY STRUCTURES, PAVEMENTS OR SURFACES DAMAGED, DIRTIED OR MADE UNSERVICABLE DUE TO CONSTRUCTION WORK SHALL BE REINSTATED TO THE SATISFACTION OF THE ENGINEER.
- S6 ANY FILL REQUIRED SHALL BE APPROVED BY THE ENGINEER / GEOTECHNICAL CONSULTANT
- S7 CONTRACTOR IS TO ENSURE THAT ALL EXCAVATIONS ARE MAINTAINED IN A DRY CONDITION WITH NO WATER ALLOWED TO REMAIN IN THE EXCAVATIONS.
- S8 ALL FINISHES AND COLOURS TO BE IN ACCORDANCE WITH ARCHITECTURAL SPECIFICATIONS.
- S9 REFER TO STRUCTURAL DRAWINGS FOR CONCRETE, REINFORCEMENT AND RETAINING WALL DETAILS.
- S10 GENERALLY FOR TRENCHING WORKS THE CONTRACTOR MUST:
- A) COMPLY WITH THE GENERAL PROVISIONS OF PART 31 "MANAGING RISKS TO HEALTH AND SAFETY" OF NSW WORK HEALTH AND SAFETY REGULATION 2011
 - B) COMPLY PART 6.3 DIVISION 3 "EXCAVATION WORK" OF NSW WORK HEALTH AND SAFETY REGULATION NSW 2011
- S11 PRIOR TO THE EXCAVATION OF ANY TRENCH DEEPER THAN 1.5 METRES THE CONTRACTOR MUST:
- A) NOTIFY THE OCCUPATIONAL HEALTH AND SAFETY AUTHORITY ON THE APPROPRIATE FORM.

STORMWATER DRAINAGE NOTES

- | | |
|------|---|
| SW1 | UNLESS NOTED OTHERWISE BY HYDRAULIC ENGINEERS DRAWINGS, ALL DOWNPIPES & GRATED INLETS SHALL BE CONNECTED TO PITS OR MAIN STORMWATER DRAINS WITH 150 DIA. UPVC PIPES LAID AT A MINIMUM GRADE OF 1 IN 100. FOR SYMPHONIC ROOF DRAINAGE SYSTEMS ALL DOWNPIPES CONNECTION DRAIN SIZES TO BE CONNECTED INTO MAIN STORMWATER DRAINS SHALL BE IN ACCORDANCE WITH HYDRAULIC ENGINEERS DRAWINGS. |
| SW2 | ALL MAIN STORMWATER DRAINS SHALL BE CONSTRUCTED USING MATERIALS AS SPECIFIED ON THE DRAWINGS IN ACCORDANCE WITH THE APPROPRIATE A.S. IF NOT SPECIFIED THEN CLASS 4 RRP RCP SHALL BE USED FOR DIAMETERS > 225mm. SEWER CLASS SEH UPVC IN ACCORDANCE WITH AS1260 SHALL BE USED FOR Ø225mm OR SMALLER. |
| SW3 | ALL PIPEWORK TO BE INSTALLED IN ACCORDANCE WITH AS3725 FOR RCP AND AS2032 FOR PVC. ALL BEDDING TO BE TYPE H2 UNLESS NOTED OTHERWISE. |
| SW4 | FOR ALL PITS > 1.2m DEEP, STEP IRONS SHALL BE INSTALLED. |
| SW5 | PRECAST PITS MAY BE USED EXTERNAL TO THE BUILDING SUBJECT TO APPROVAL BY BONACCI GROUP. |
| SW6 | ENLARGERS, CONNECTIONS AND JUNCTIONS TO BE PREFABRICATED FITTINGS WHERE PIPES ARE LESS THAN 300 DIA. |
| SW7 | WHERE SUBSOIL DRAINS PASS UNDER FLOOR SLABS AND VEHICULAR PAVEMENTS, UNSLOTTED UPVC SEWER GRADE PIPE IS TO BE USED. |
| SW8 | GRATES AND COVERS SHALL CONFORM WITH AS 3996 AND AS 1428.1 FOR ACCESS REQUIREMENTS. |
| SW9 | CARE IS TO BE TAKEN WITH LEVELS OF STORMWATER LINES. GRADES ARE NOT TO BE REDUCED WITHOUT APPROVAL. |
| SW10 | AT ALL TIMES DURING CONSTRUCTION OF STORMWATER PITS, ADEQUATE SAFETY PROCEDURES SHALL BE TAKEN TO ENSURE AGAINST THE POSSIBILITY OF PERSONNEL FALLING DOWN PITS. |
| SW11 | ALL EXISTING STORMWATER DRAINAGE LINES AND PITS THAT ARE TO REMAIN ARE TO BE INSPECTED AND CLEANED. DURING THIS PROCESS ANY PART OF THE STORMWATER DRAINAGE SYSTEM THAT WARRANTS REPAIR SHALL BE REPORTED TO THE SUPERINTENDENT/ENGINEER FOR FURTHER DIRECTIONS. |

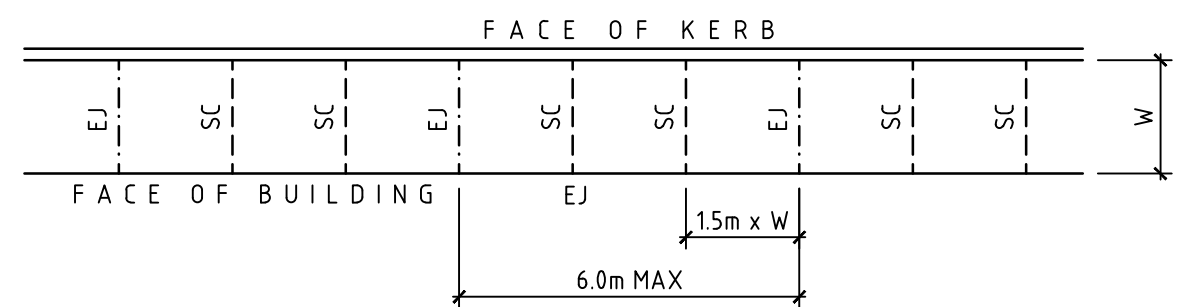
KERBING NOTES

- | | |
|----|--|
| K1 | ALL CONCRETE TO HAVE A MINIMUM COMPRESSIVE STRENGTH OF 32 MPa U.N.O. |
| K2 | ALL KERBS, GUTTERS, DISH DRAINS AND CROSSINGS TO BE CONSTRUCTED ON 75mm GRANULAR BASECOURSE COMPACTED TO A MINIMUM 98% MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS1289 5.2.1. |
| K3 | EXPANSION JOINTS (EJ) TO BE FORMED FROM 10mm COMPRESSIBLE CORK FILLER BOARD FOR THE FULL DEPTH OF THE SECTION AND CUT TO PROFILE. EXPANSION JOINTS TO BE LOCATED AT DRAINAGE FITS ON TANGENT PORTION OF CURVES AND ELSEWHERE AT MAX 12m CENTRES EXCEPT FOR INTEGRAL KERBS WHERE THE EXPANSION JOINTS ARE TO MATCH THE JOINT LOCATIONS IN THE SLAB. |
| K4 | WEAKENED PLANE JOINTS TO BE MIN 3mm WIDE AND LOCATED AT 3m CENTRES EXCEPT FOR INTEGRAL KERBS WHERE THE WEAKENED PLANE JOINTS ARE TO MATCH THE JOINT LOCATIONS IN THE SLAB. |
| K5 | BROOMED FINISH TO ALL RAMPED AND VEHICULAR CROSSINGS. ALL OTHER KERBING OR DISH DRAINS TO BE STEEL FLOAT FINISHED. |
| K6 | IN THE REPLACEMENT OF KERBS:-
- EXISTING ROAD PAVEMENT IS TO BE SAWCUT 900mm U.N.O. FROM THE LIP OF GUTTER, UPON COMPLETION OF THE NEW KERB AND GUTTER, NEW BASECOURSE AND SURFACE TO BE LAID 600mm WIDE U.N.O.
- EXISTING KERBS ARE TO BE COMPLETELY REMOVED WHERE NEW KERBS ARE SHOWN. |

JOINTING NOTES

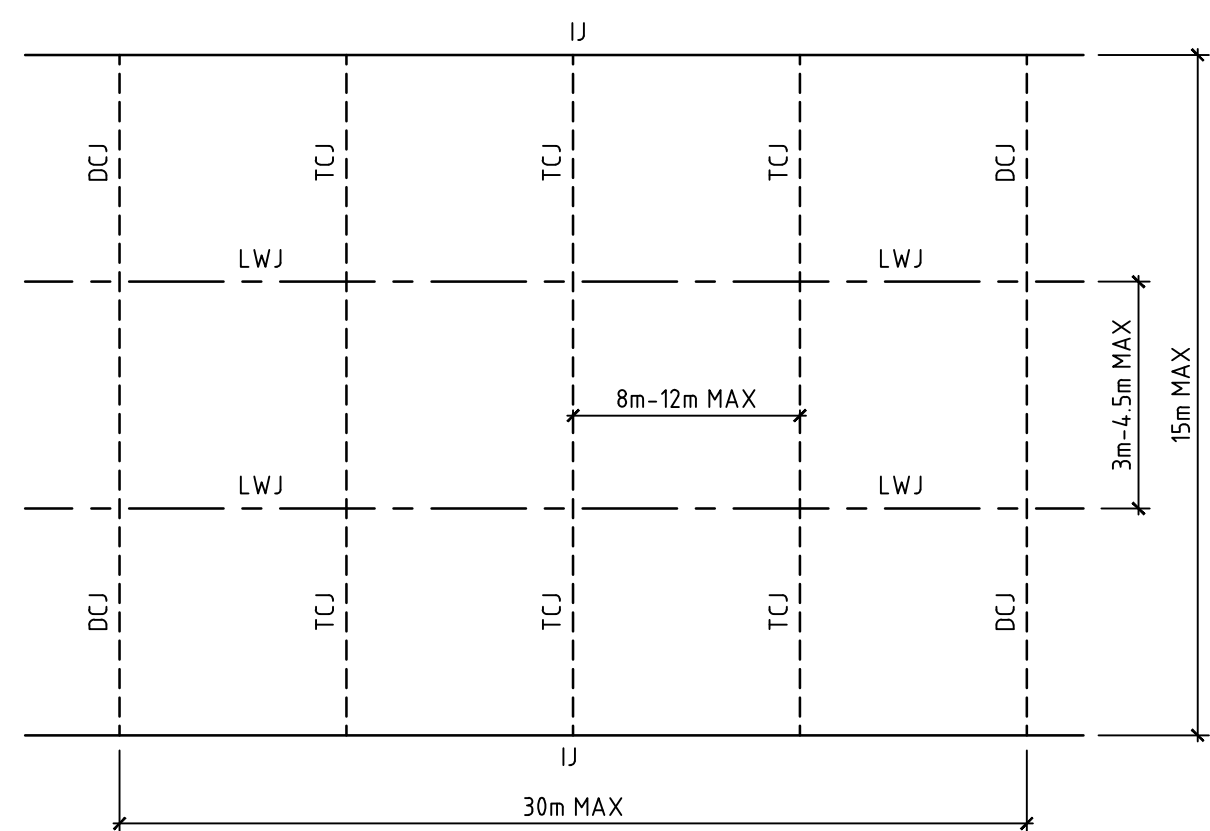
PEDESTRIAN FOOTPATH JOINTS

- | | |
|----|--|
| J1 | EXPANSION JOINTS (EJ) ARE TO BE LOCATED WHERE POSSIBLE AT TANGENT POINTS OF CURVES AND ELSEWHERE AT 6m CENTRES. |
| J2 | SAWCUT JOINTS (SC) ARE TO BE LOCATED AT A MAX 15m x WIDTH OF PAVEMENT. THE TIMING OF THE SAWCUT IS TO BE CONFIRMED BY THE CONTRACTOR ON SITE. SITE CONDITIONS WILL DETERMINE HOW MANY HOURS AFTER THE CONCRETE POUR BEFORE THE SAW CUTS ARE COMMENCED. |
| J3 | WHERE POSSIBLE JOINTS SHOULD BE LOCATED TO MATCH KERBING AND / OR ADJACENT PAVEMENT JOINTS. |
| J4 | PROVIDE 10mm WIDE FULL DEPTH EXPANSION JOINTS (EJ) BETWEEN BUILDINGS AND ALL CONCRETE OR UNIT PAVERS |
| J5 | ALL PEDESTRIAN FOOTPATH JOINTINGS AS FOLLOWS (U.N.D.). |

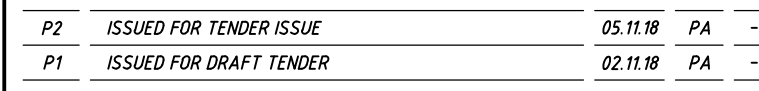


VEHICULAR PAVEMENT JOINTS

- | | |
|-----|--|
| J6 | ALL VEHICULAR PAVEMENTS TO BE JOINED AS SHOWN ON DRAWINGS. |
| J7 | LONGITUDINAL WARPING JOINTS (LWJ) SHOULD GENERALLY BE LOCATED AT A MAXIMUM OF 3m TO 4.5m MAX CENTERS. ALL LWJ's SHOULD BE TIED UP TO A MAXIMUM TOTAL WIDTH OF 30m. |
| J8 | TRANSVERSE CONTRACTION JOINTS (TCJ) SHOULD GENERALLY BE LOCATED AT A MAXIMUM OF 8m TO 12m MAX CENTERS. TCJ's CAN BE SPACED AT SUITABLE INTERVALS UP TO A RECOMMENDED MAXIMUM LENGTH OF 15m. |
| J9 | TRANSVERSE DOWELLED CONSTRUCTION JOINTS (DCJ) TO BE PROVIDED FOR PLANNED INTERRUPTIONS SUCH AS AT THE END OF EACH DAY'S OPERATIONS (POUR BREAK), AT BLOCK OUTS FOR BRIDGES AND INTERSECTIONS OR FOR UNEXPECTED DELAYS WHEN THE SUSPENSION OF OPERATIONS IS LIKELY TO CREATE A JOINT. |
| J10 | ISOLATION JOINTS WITH SUB-GRADE BEAM (IJ) TO BE PROVIDED AT INTERSECTIONS OR AT THE JUNCTION OF A POUR BREAK. |
| J11 | ALL VEHICULAR PAVEMENTS TO BE JOINED IN ACCORDANCE WITH AUSTRADO'S AGPT02-12 GUIDE TO PAVEMENT TECHNOLOGY PART 2 STRUCTURAL PAVEMENT DESIGN AND SUPPLEMENT AP-336-06 PAVEMENT DESIGN FOR LIGHT TRAFFIC |
| J12 | VEHICULAR PAVEMENT JOINING AS FOLLOWS (U.N.O.) |

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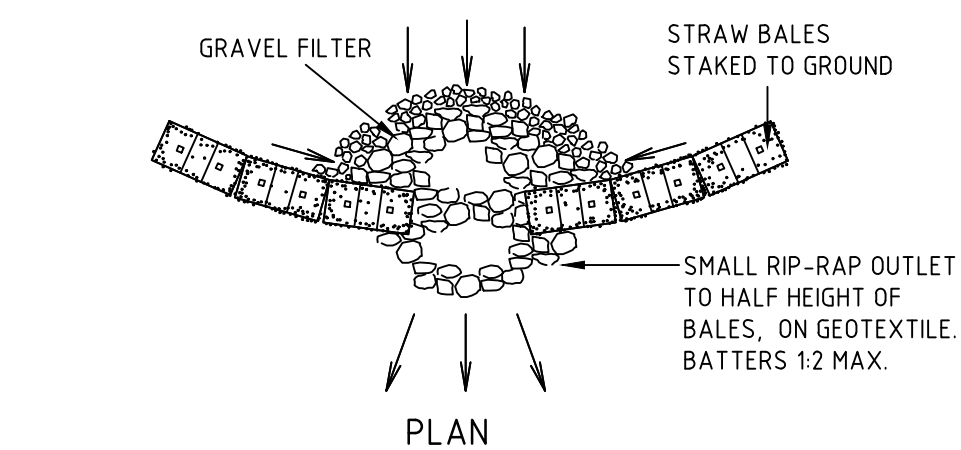
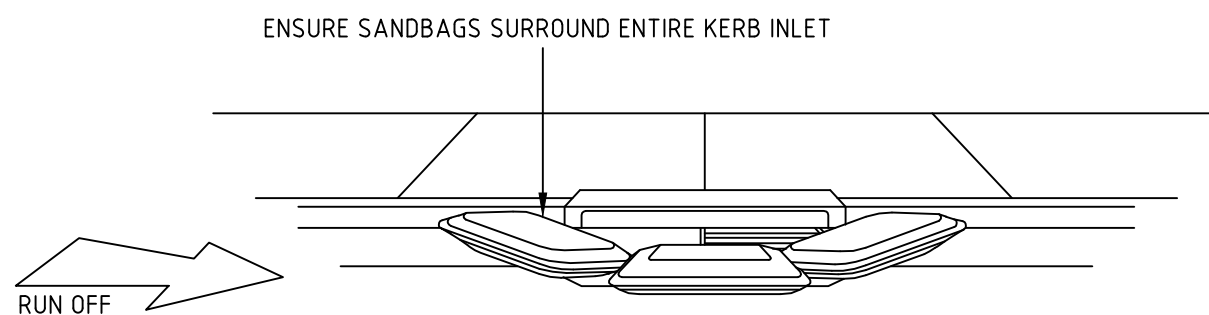
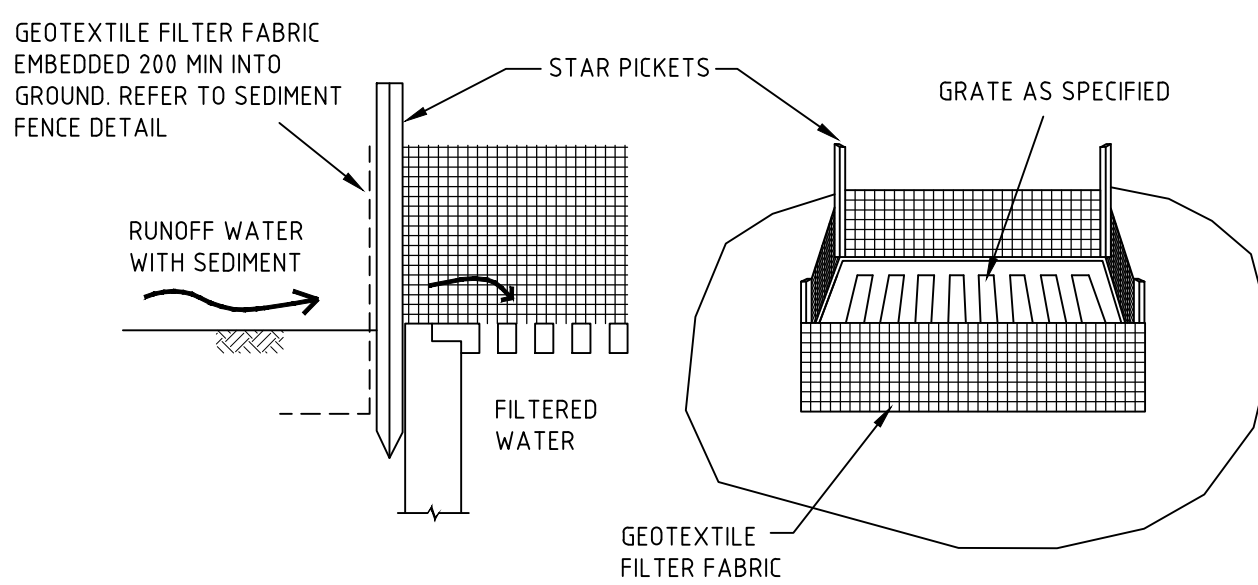
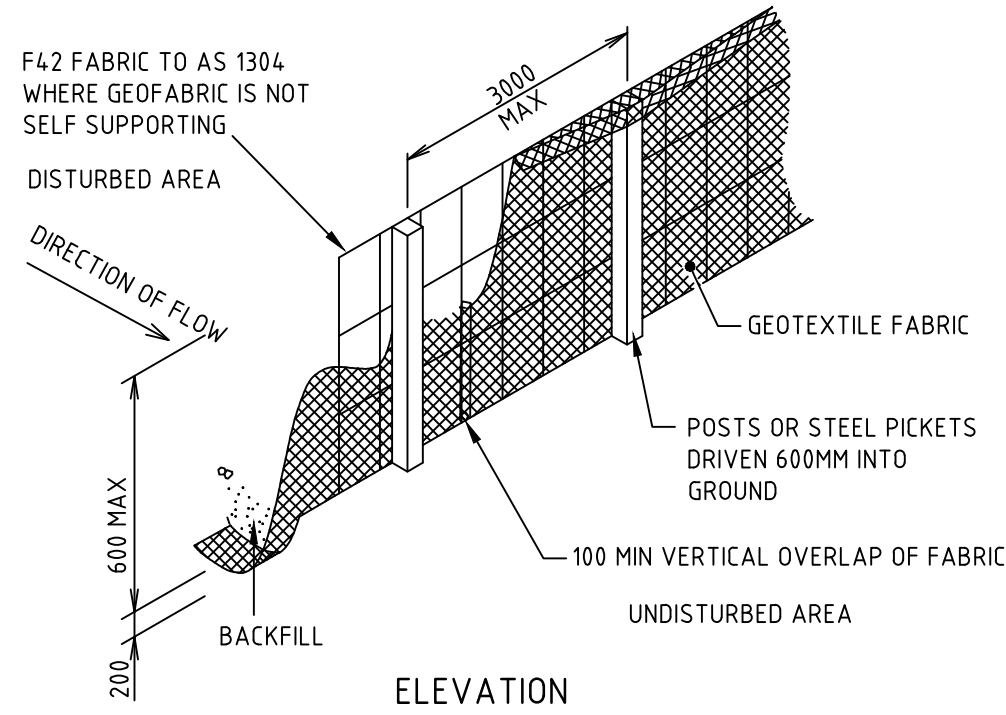
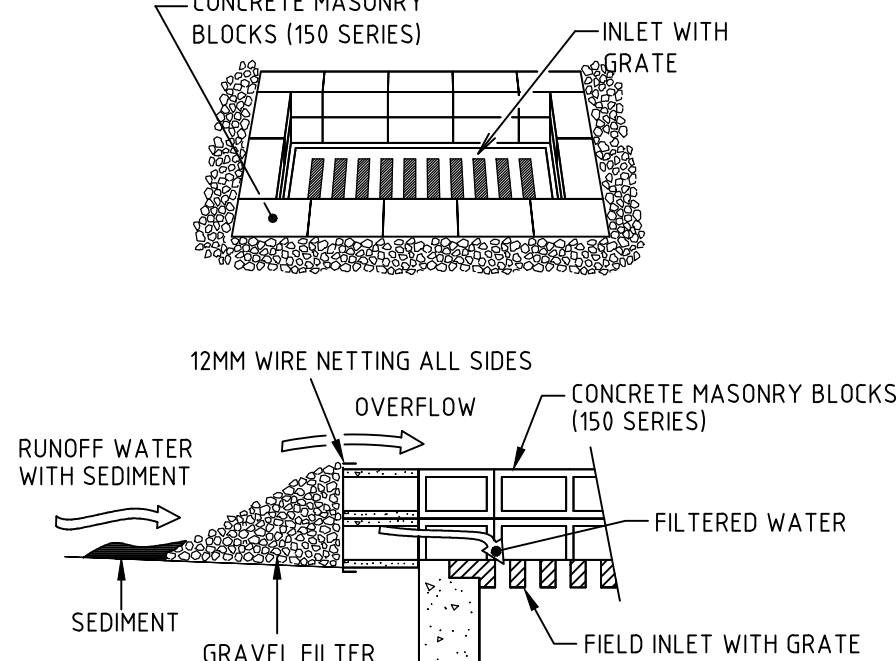
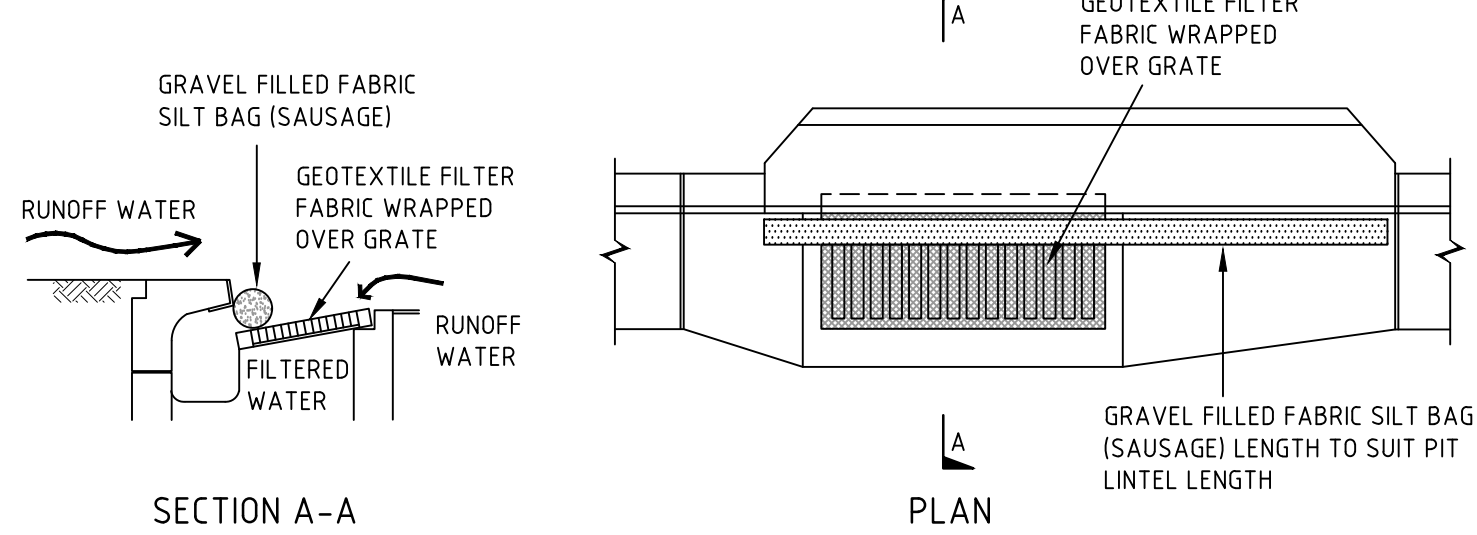
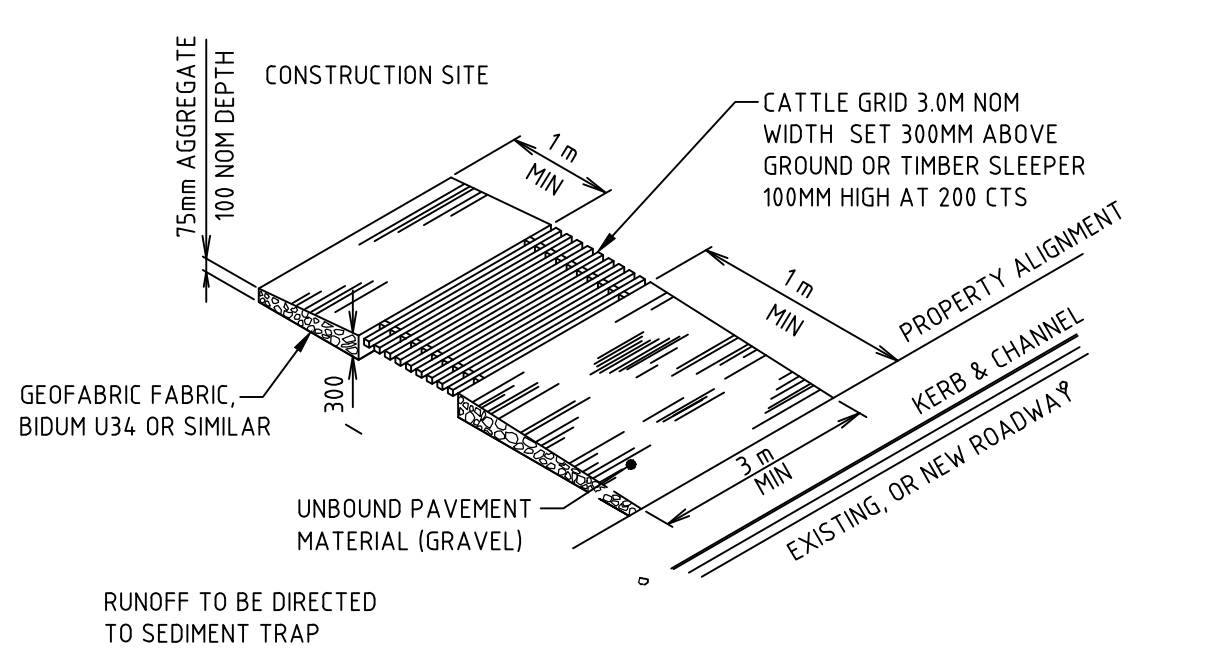
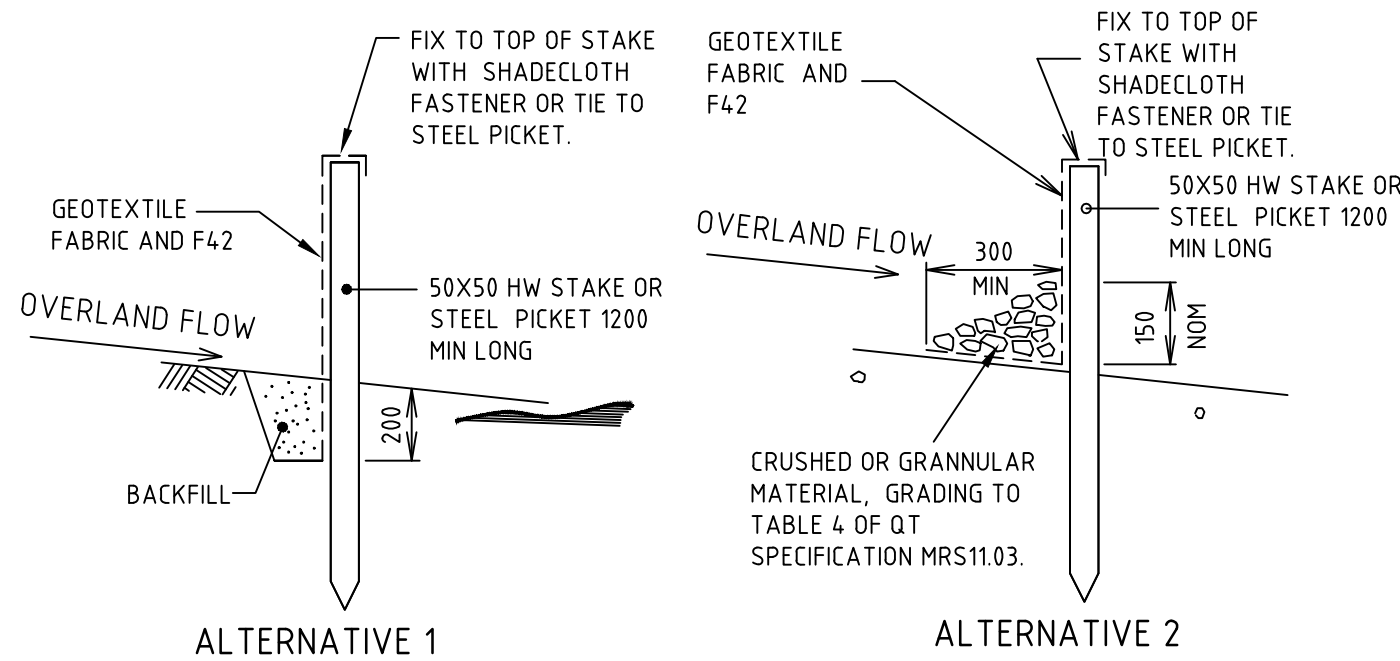
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			Designed	CS	Project Director Approved		Date	North
Drawing Title	DRAWING REGISTER AND CONSTRUCTION NOTES		Drawn	PA				
			Scale	NTS				
			Date	3.10.2018				
			Sheet	A1				
					Project Ref		Drawing No	
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Designed	CS	Project Director Approved	Date	
Drawn	PA			
Scale	NTS			
Date	3.10.2018	Project Ref	Drawing No	Rev
Sheet	A1	20 10748 01	C505	P2

5.0 0 2.5 5.0 7.5 10.0m
PLAN SCALE 1:250 A1 SHEET

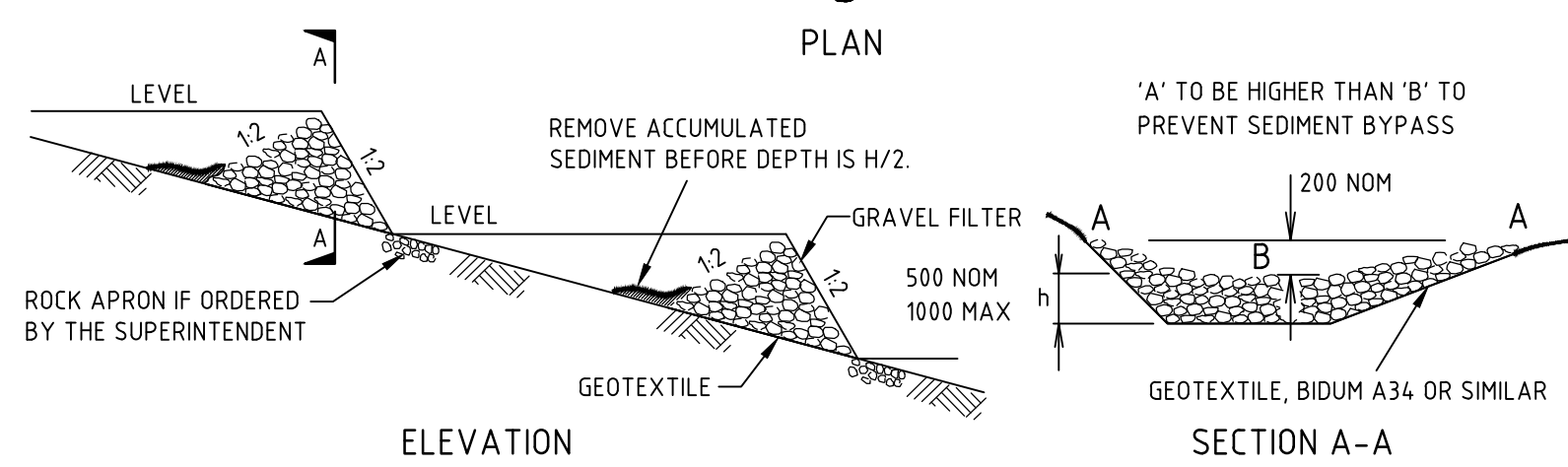
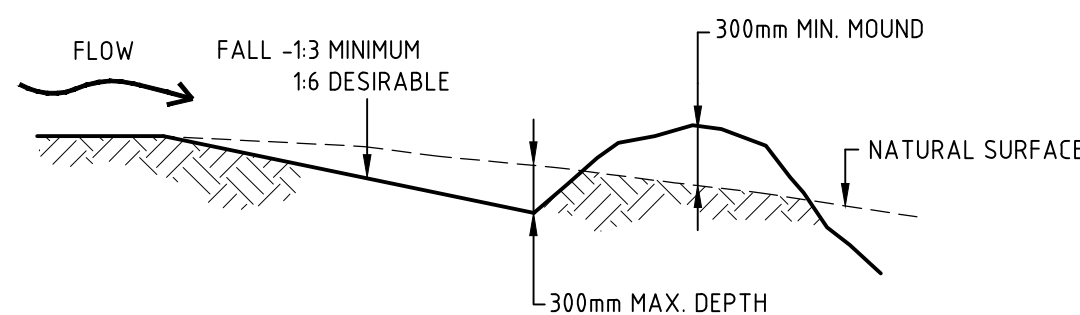
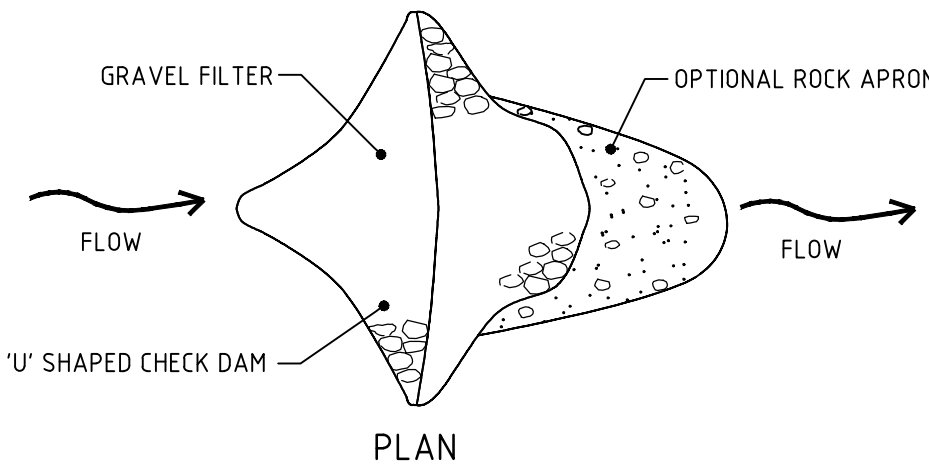
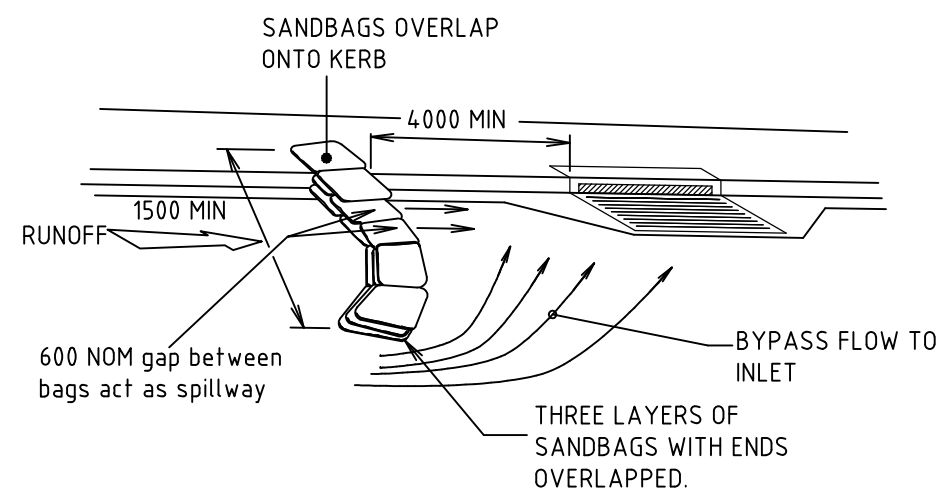
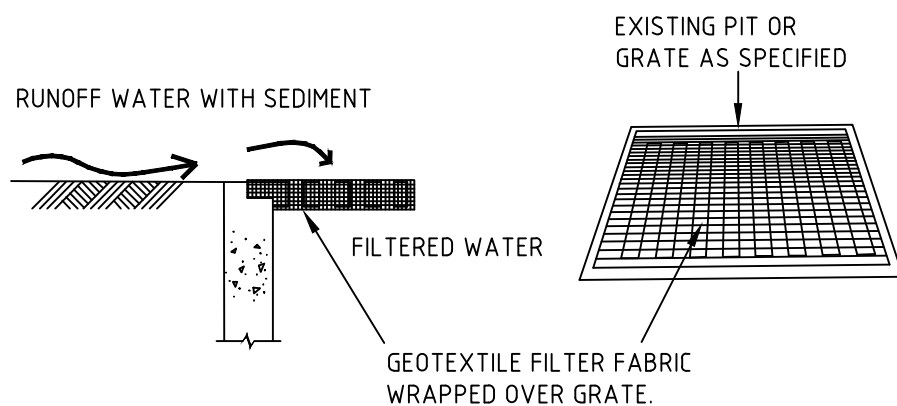
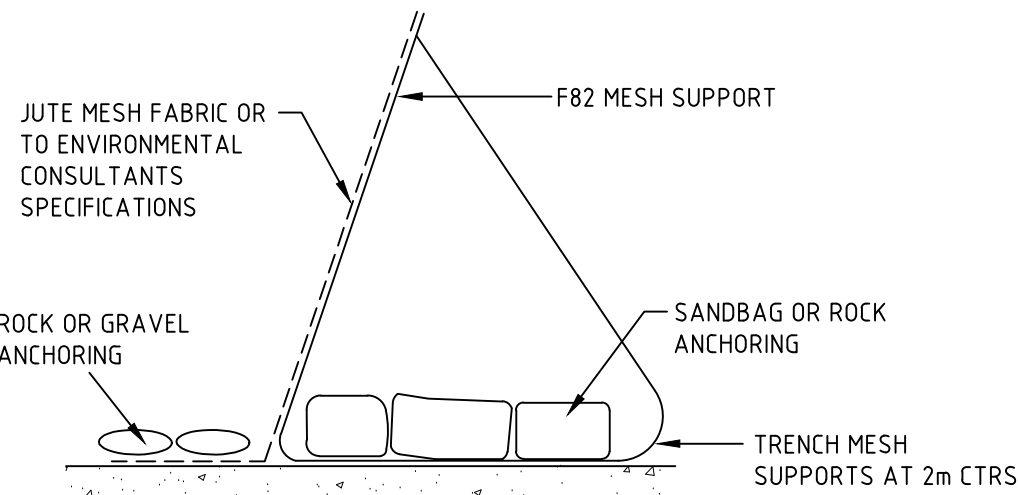


SEDIMENT FENCE
NOT TO SCALE

GEOTEXTILE PIT FILTER 1
NOT TO SCALE

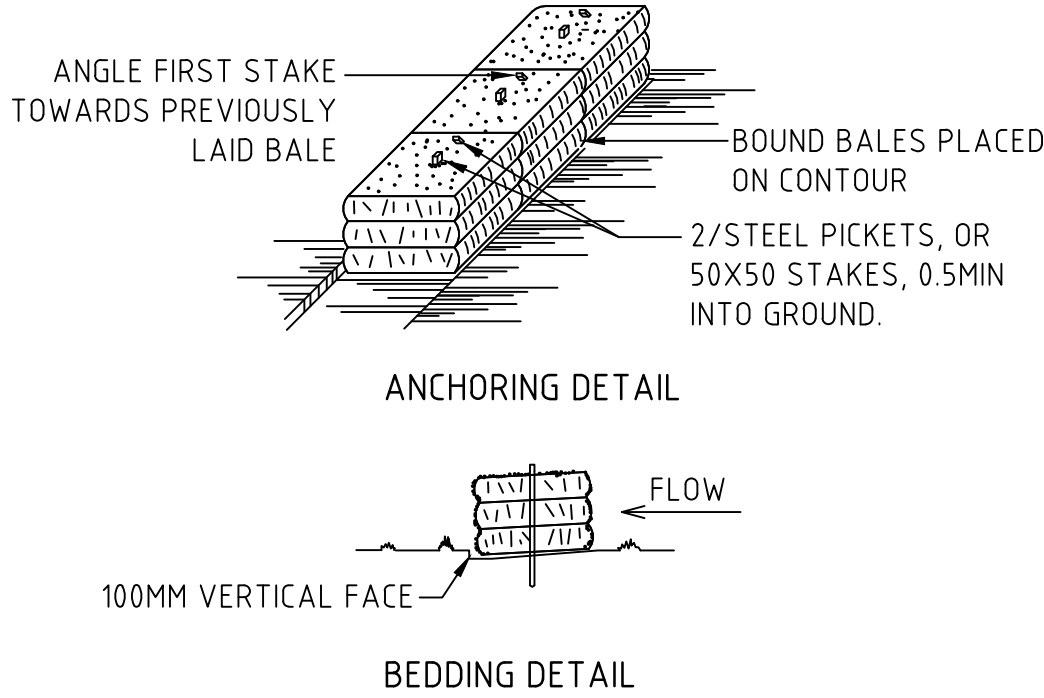
SANDBAG KERB INLET SEDIMENT TRAP
NOT TO SCALE

STRAW BALE AND STONE TRAP SEDIMENT CONTROL (CONCENTRATE FLOW)
NOT TO SCALE



CATCH DRAIN
NOT TO SCALE

CHECK DAMS FLOW CONTROL
NOT TO SCALE



STRAW BALE BANK SEDIMENT CONTROL
NOT TO SCALE

ALTERNATIVE SEDIMENT FENCE NOTES

1. INSTALL THIS TYPE OF SEDIMENT FENCE WHEN USE OF SUPPORT POSTS IS NOT DESIRABLE OR NOT POSSIBLE. SUCH CONDITIONS MIGHT APPLY, FOR EXAMPLE, WHERE APPROVAL IS GRANTED FROM THE APPROPRIATE AUTHORITIES TO PLACE THESE FENCES IN HIGHLY SENSITIVE ESTUARINE AREAS.
2. USE BENT TRENCH MESH TO SUPPORT THE F82 WELDED MESH FACING AS SHOWN ON THE DRAWING ABOVE. ATTACH THE JUTE MESH TO THE WELDED MESH FACING USING UV-RESISTANT CABLE TIES.
3. STABILISE THE WHOLE STRUCTURE WITH SANDBAG OR ROCK ANCHORING OVER THE TRENCH MESH AND THE LEADING EDGE OF THE JUTE MESH. THE ANCHORING SHOULD BE SUFFICIENTLY LARGE TO ENSURE STABILITY OF THE STRUCTURE IN THE DESIGN STORM EVENT, USUALLY THE 10 YEAR EVENT.

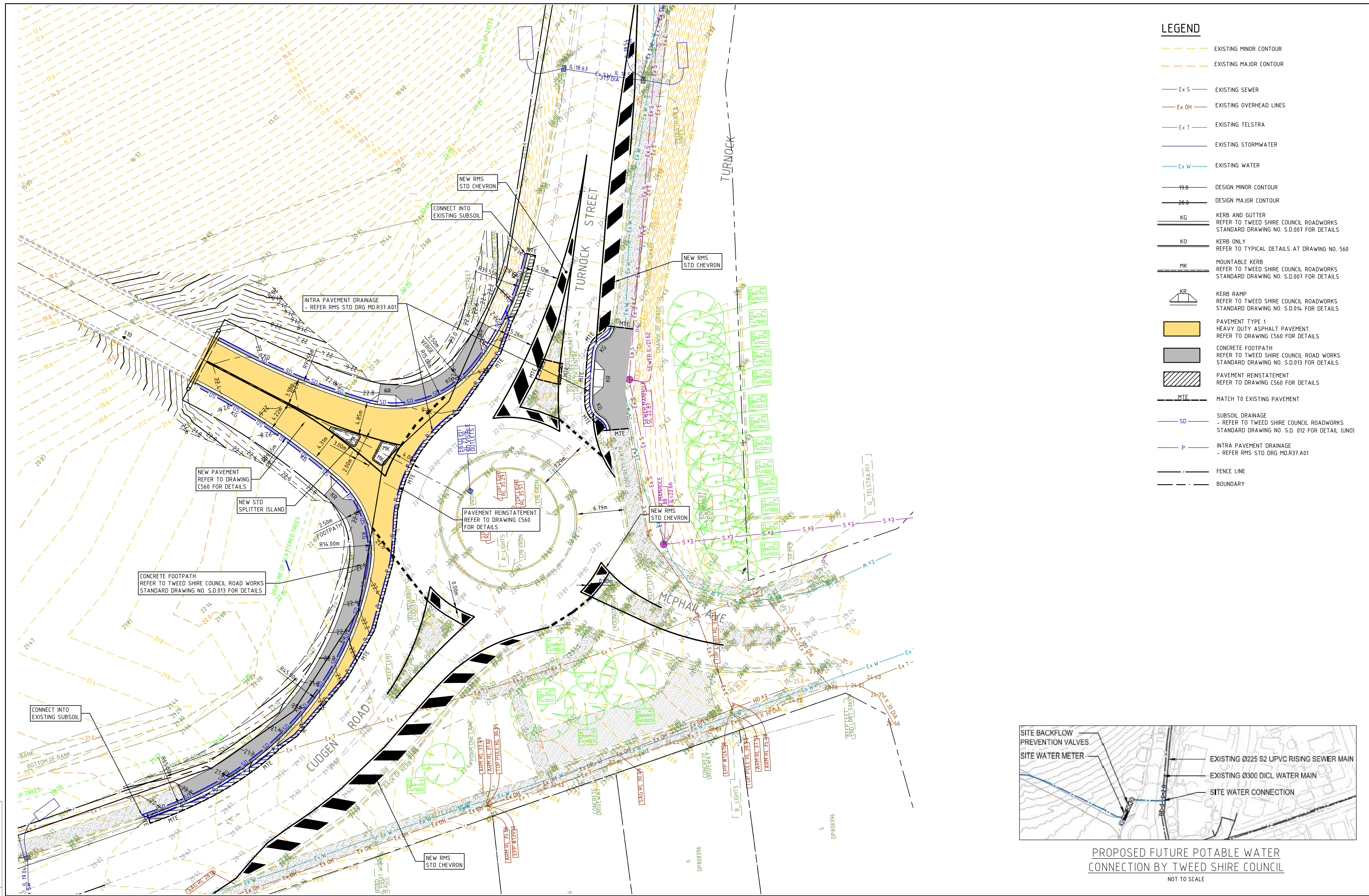
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Rev	Description	Date	By	App	Rev	Description	Date
P1	ISSUED FOR TENDER ISSUE	05.11.18	PA				

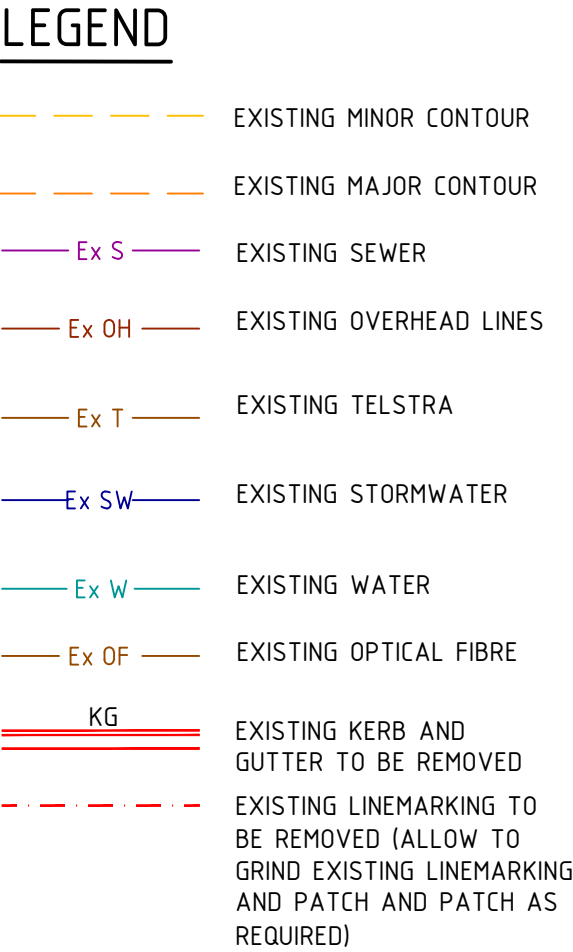
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Project Name	TWEED VALLEY HOSPITAL DEVELOPMENT, TURNOCK STREET ROUNDABOUT WORKS		
Drawing Title	SOIL AND WATER MANAGEMENT DETAILS		

TENDER			
Designed	PA	Project Director Approved	Date
Drawn	PA		
Scale	-	Project Ref	Drawing No
Date	02.10.18	20 10748 01	C507
Sheet	A1		P1





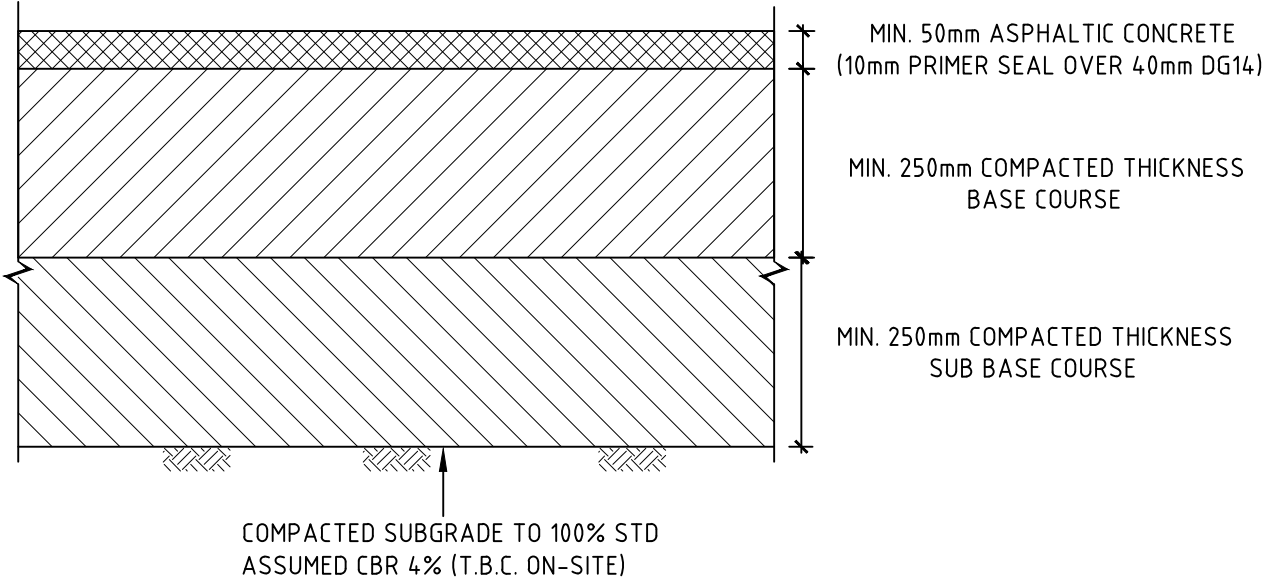
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P2	ISSUED FOR TENDER ISSUE	05.11.18	PA	-					
P1	ISSUED FOR DRAFT TENDER	02.11.18	PA	-					
Rev	Description	Date	By	App	Rev	Description	Date	By	App

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Project Name	TWEED VALLEY HOSPITAL DEVELOPMENT, TURNOCK STREET ROUNDABOUT WORKS
Drawing Title	DEMOLITION PLAN

TENDER				
Designed	CS	Project Director Approved	Date	North 
Drawn	PA	Project Ref	Drawing No	Rev
Scale	1:250			
Date	3.10.2018			
Sheet	A1	20 10748 01	C545	P2



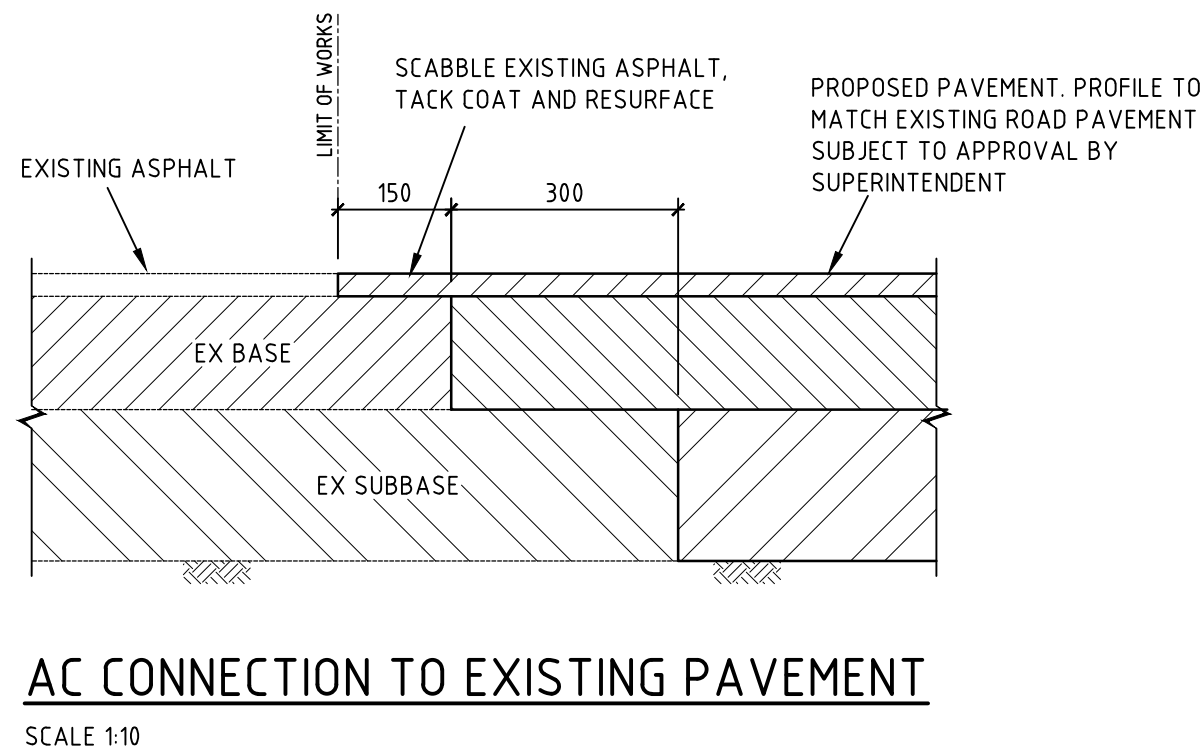
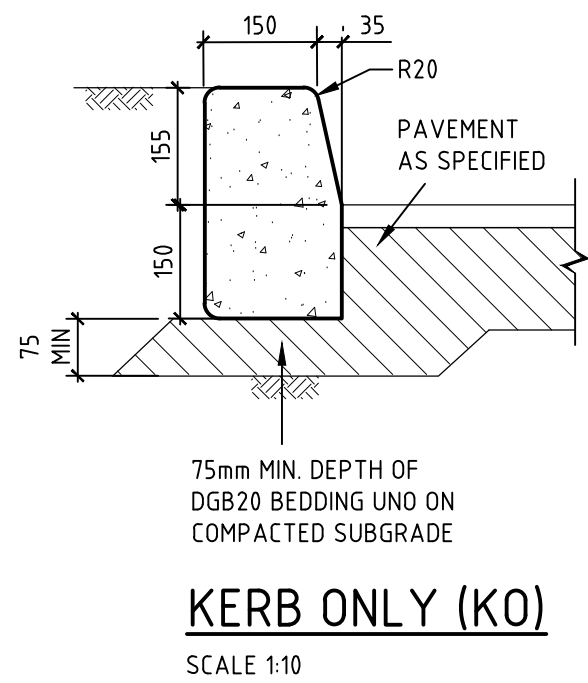
PAVEMENT TYPE P1
HEAVY DUTY ASPHALT PAVEMENT

SCALE 1:10



DENOTED ON PLAN

ROAD DESIGN PARAMETERS (FLEXIBLE)
DESIGN TRAFFIC 1.5 X 10⁶ (ESA's OR HVAG)
ASSUMPTIONS:
• TWEED SHIRE COUNCIL PAVEMENT DESIGN TABLE D2.2 ARTERIAL DISTRIBUTOR, SHOPPING STRIP ACCESS, INDUSTRIAL, DESIGN PERIOD 25 YEAR.
• CBR 4.0%
• PREPARE SUBGRADE AND SELECT FILL IN ACCORDANCE WITH (MORRISON GEOTECH, REFERENCE NO. GE18/144-REV 1)



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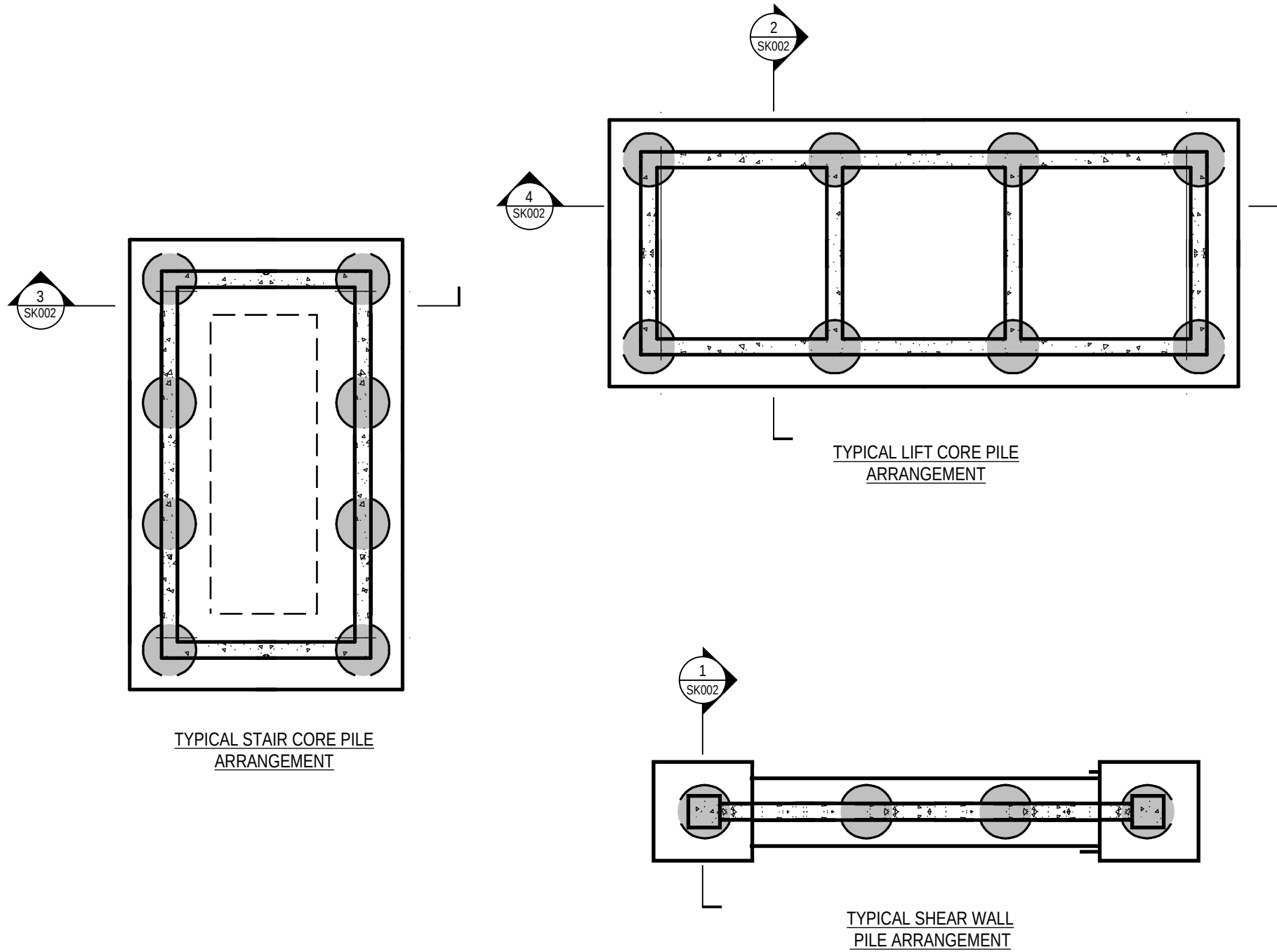
P2	ISSUED FOR TENDER ISSUE	05.11.18	PA	-	
P1	ISSUED FOR DRAFT TENDER	02.11.18	PA	-	

Rev	Description	Date	By	App	Rev	Description	Date	By	App

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Project Name	TWEED VALLEY HOSPITAL DEVELOPMENT, TURNOCK STREET ROUNDABOUT WORKS			TENDER		
Drawing Title	CIVIL WORKS DETAILS			Designed	CS	Project Director Approved
				Drawn	PA	Date
				Scale	NOTED	North
				Date	3.10.2018	
				Sheet	A1	
Project Ref	20 10748 01	Drawing No	C560	Rev	P2	



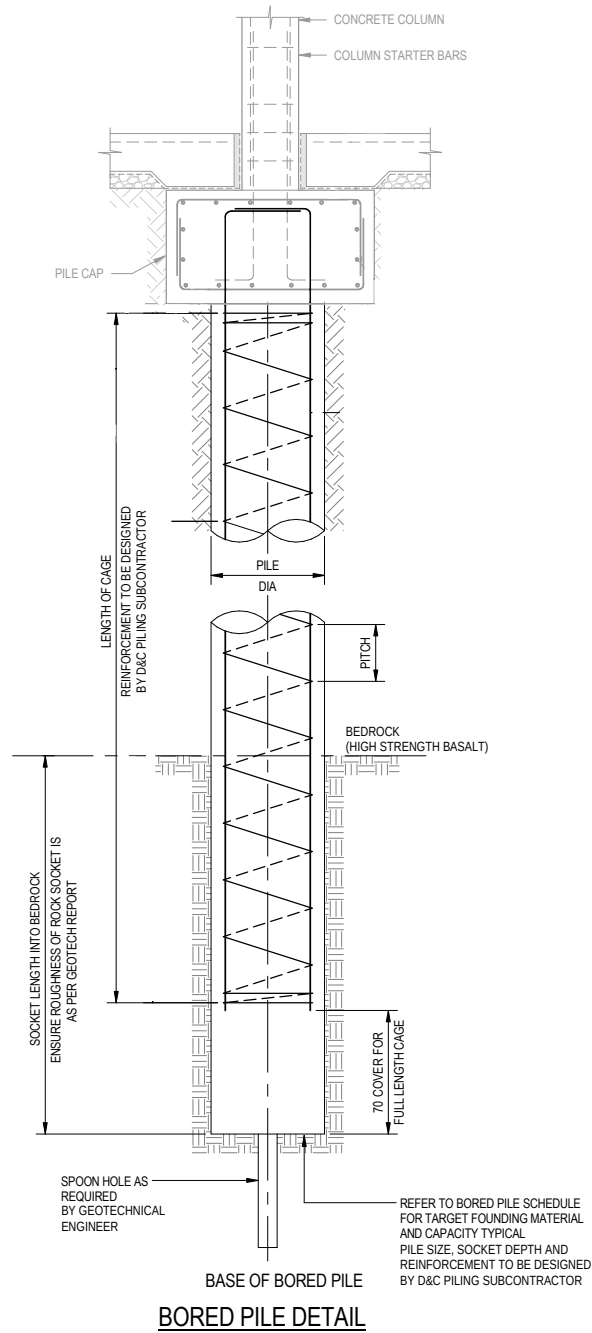
BORED PILE SCHEDULE			
PILE	NO OF SUSPENDED SLABS SUPPORTED	ULTIMATE LIMIT STATE VERTICAL LOAD (kN)	NOMINAL DIAMETER (mm)
P1	3	3600	1000
P2	4	4800	1000
P3	5	6000	1000
P4	6	7200	1000
P5	7	8400	1000
P6	8	9600	1000
P7	9	10800	1000

NOTES

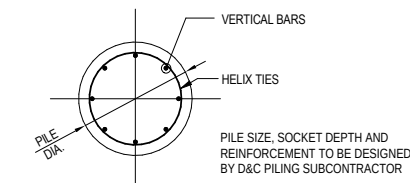
- PILES TO BE DESIGNED BY D&C PILING SUBCONTRACTOR IN ACCORDANCE WITH AS2159
- SUBJECT TO FINAL DESIGN, PILE DIAMETERS MAY VARY FROM 600MM TO 1200MM
- PILES UNDER CORES, LIFT SHAFTS AND SHEAR WALLS TO BE DESIGNED TO RESIST THE LATERAL LOADS NOMINATED ON THE STRUCTURAL DRAWINGS

BORED PILES

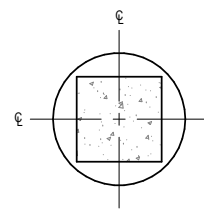
- BP1 REFER TO THE GEOTECHNICAL REPORT FOR A DESCRIPTION OF THE ANTICIPATED SITE CONDITIONS. THE PILING CONTRACTOR IS TO STUDY THE REPORT AND MAKE HIS OWN EVALUATION OF THE SITE CONDITIONS. ANY ADDITIONAL COSTS INCURRED SHALL BE BORNE BY THE PILING CONTRACTOR.
- BP2 THE BORED PILES ARE PROPORTIONED FOR THE SCHEDULED LOADS WITH ALLOWABLE SOCKET SKIN FRICTION AND END BEARING CAPACITY AS INDICATED IN THE REPORT. THE DEPTHS AND LENGTHS NOMINATED IN THE SCHEDULE ARE INDICATIVE ONLY. THEY MAY NEED TO BE VARIED DEPENDING ON THE SITE CONDITIONS ENCOUNTERED. THE PILING CONTRACTOR NEEDS TO INCORPORATE ANY DESIGN CHANGES REQUIRED.
- BP3 THE BORED PILES SHALL BE INSTALLED TO A MAXIMUM TOLERANCE OF $\pm 75\text{mm}$ FROM THAT REQUIRED IN PLAN AND INCLINED AT NOT MORE THAN 1 IN 75 FROM THE VERTICAL OR SPECIFIED RAKE.
- BP4 ALL WORKMANSHIP AND MATERIAL SHALL BE IN ACCORDANCE WITH AS 2159.
- BP5 THE BORED PILES SHALL BE LOCATED CONCENTRIC WITH THE COLUMNS AND WALLS UNLESS NOTED OTHERWISE.
- BP6 DRILL AND INSTALL THE BORED PILES IN THE LOCATIONS SHOWN ON THE DRAWINGS AND THE ABOVE REQUIREMENTS.
- BP7 BEFORE ANY CONCRETE IS POURED, ALL ROCK SOCKETS SHALL BE DEWATERED AND INSPECTED BY THE GEOTECHNICAL ENGINEER, WHO SHALL BE EMPLOYED BY THE BUILDER, TO VERIFY THE SOIL PARAMETERS. THE SOCKET BASE AND WALLS MUST BE CLEAN AND FREE FROM CLAY.
- BP8 IF THE CONCRETE NEEDS TO BE TREMIED, SUPER PLASTICIZER MUST BE ADDED TO THE MIX AND THE CONCRETE GRADE INCREASED BY 30% . REFER TO THE SPECIFICATIONS FOR THE INSPECTION OF THE HOLE PRIOR TO CONCRETING.
- BP9 THE PILING CONTRACTOR SHALL ALLOW FOR THE COST INTEGRITY TESTING OF ALL BORED PILES.
- BP10 ANY ALTERNATIVE DESIGN SHALL MEET THE ABOVE REQUIREMENTS AND THE SCHEDULED LOADS. THE PILING CONTRACTOR SHALL OBTAIN CERTIFICATION FOR THE CALCULATIONS OF THE ALTERNATIVE SYSTEM. THE DETAILS AND CALCULATIONS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW. THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR THE PERFORMANCE OF THE ALTERNATIVE BORED PILES.



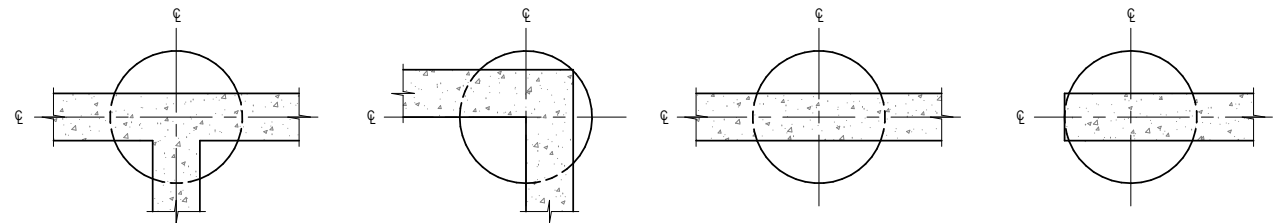
SECTION 1 SK001



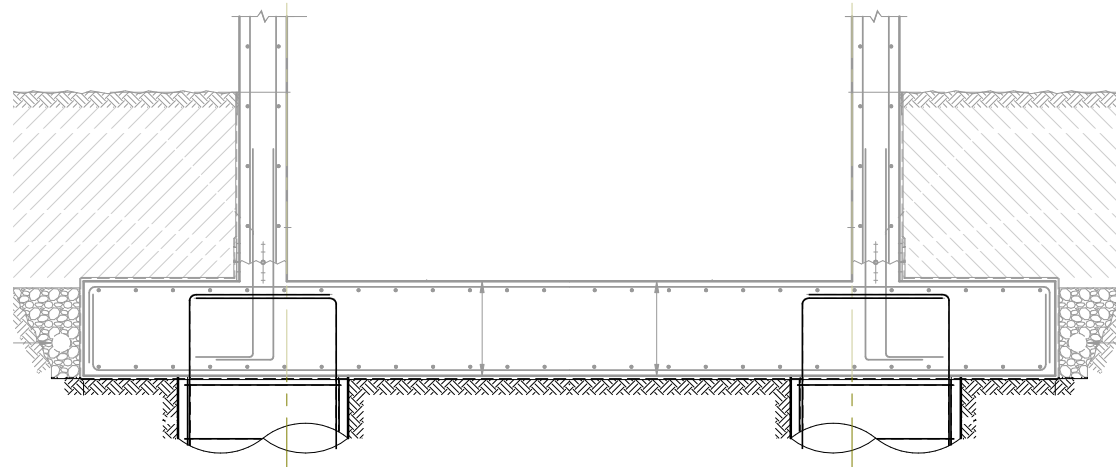
BORED PILE SECTION



TYPICAL PILE LOCATIONS AT COLUMNS
ALL PILES TO BE CENTRED ON COLUMN/WALLS OVER UNLESS OTHERWISE DENOTED ON PLAN

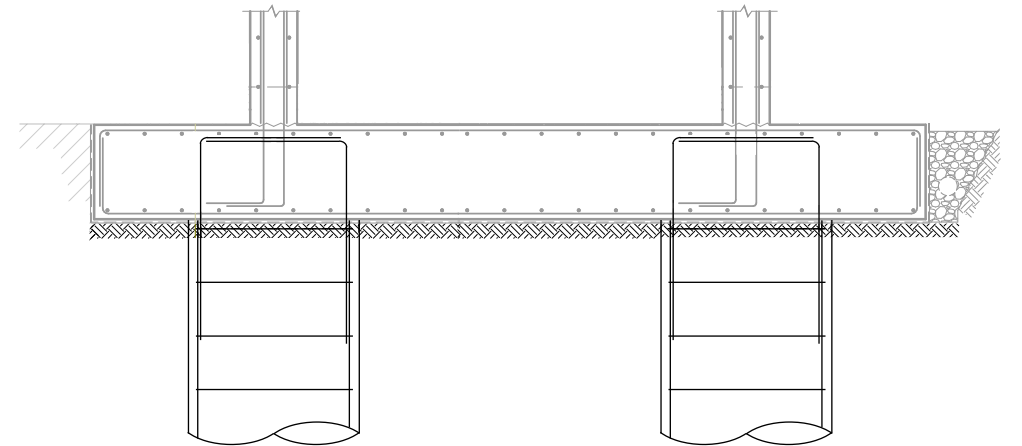


TYPICAL PILE LOCATIONS AT COLUMNS/WALLS
ALL PILES TO BE CENTRED ON COLUMN/WALLS OVER UNLESS OTHERWISE DENOTED ON PLAN



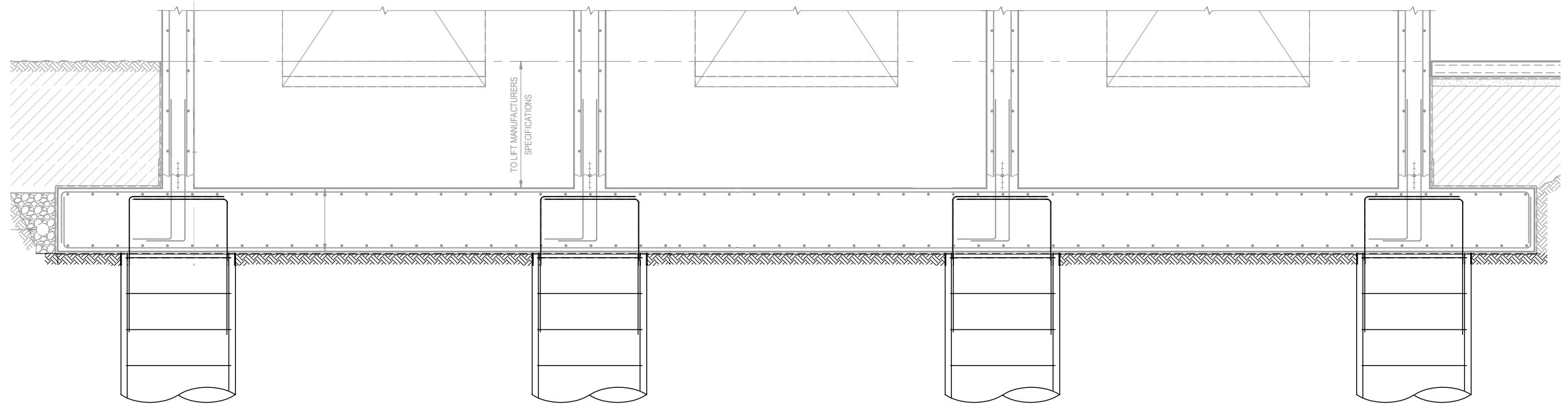
TYPICAL SECTION THROUGH
LIFT PIT BASE

SECTION 2 SK001



TYPICAL SECTION THROUGH
STAIR CORE BASE

SECTION 3 SK001



TYPICAL LONG SECTION THROUGH
LIFT PIT BASE

SECTION 4 SK001



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Project Name	TWEED VALLEY HOSPITAL	Drawn	TU	Date	08/08/2018
Drawing Title	PILING DETAILS SHEET 2	Project Ref	10748	Sketch No	SK002
				REV	1

4. Appendix B – Updated DRAINS Model

PIT / NODE DETAILS

Name	Type	Family	Size	Ponding Volume (cu.m)	Pressure Change Coeff. Ku	Surface Elev (m)	Max Pond Depth (m)	Base Inflow (cu.m/s)	Blocking Factor	x	y	Bolt-down lid	Part Full Shock	Inflow Los: Hydrograph	Pit is
N1	Node								0	243.056	-120.139		2	No	
N2	Node								0	635.417	-140.972		8	No	
N11467	Node								0	372.222	-114.583		27369	No	
N11471	Node								0	490.278	-125		27378	No	
N11478	Node								0	743.75	-155.556		27388	No	
N11479	Node								0	823.611	-159.028		27389	No	
N42218	Node					8.5			0	1304	-528		202768	No	
N57288	Node					9			0	1250.476	-516.647		241366	No	
N57291	Node					9			0	1188.989	-509.815		241373	No	
N57293	Node					5			0	1136.744	-508.61		241378	No	
N95390	Node								0	1064.969	-451.061		317619	No	

DETENTION BASIN DETAILS

Name	Elev	Volume	Not Used	Outlet Type	K	Dia(mm)	Centre RL	Pit Family	Pit Type	x	y	HED	Crest RL	Crest Leng id
Basin 1	7.9	0		Orifice		600	8.15				1311	-484 No		202766
	9.3	6050												
	9.6	7100												
Basin 2	8.5	0		Orifice		300	8.625			1258.513	-477.263 No			241365
	9.9	473												
	10.2	634												
Basin 3	9.5	0		Orifice		300	9.65			1202.251	-469.226 No			241372
	10.9	1339												
	11.2	1653												
Basin 4	6.4	0		Orifice		525	6.65			1148.399	-467.618 No			241377
	7.8	2534												
	8.1	3040												

SUB-CATCHMENT DETAILS

Name	Pit or Node	Total Area (ha)	Paved Area %	Grass Area %	Supp Area %	Paved Time (min)	Grass Time (min)	Supp Time (min)	Paved Length (m)	Grass Length (m)	Supp Length (m)	Paved Slope(%)	Grass Slope %	Supp Slope %	Paved Rough	Grass Rough	Supp Rough	Lag Time or Factor	Gutter Length (m)	Gutter Slope %	Gutter FlowFactor	Rainfall Multiplier
Un-Dev 5 \ N1		19.38	0	100	0	0	0	30	0										0			1
Dev 5 Year N2		19.38	27.1	72.9	0	5	28.5	0	0										0			1
Un-Dev 20 N11467		19.38	0	100	0	0	0	26.5	0										0			1
Un-Dev 10 N11471		19.38	0	100	0	0	0	23.5	0										0			1
Dev 20 Year N11478		19.38	27.1	72.9	0	5	25.8	0	0										0			1
Dev 100 Year N11479		19.38	27.1	72.9	0	5	15.1	0	0										0			1
Flows to Basin 1		5.6212	82.4	17.6	0	15.2	15.2	0	0										0			1
Cat Basin : Basin 2		0.639	68.8	31.1	0	5	5	0	0										0			1
Cat Basin : Basin 3		1.3112	45.7	54.3	0	15.2	15.2	0	0										0			1
Cat Basin : Basin 4		1.9986	38.7	61.3	0	15.2	15.2	0	0										0			1
ByPass N95390		9.81	0	100	0	5	19	0	0										0			1

PIPE DETAILS

Name	From	To	Length (m)	U/S IL (m)	D/S IL (m)	Slope (%)	Type	Dia (mm)	I.D. (mm)	Rough	Pipe Is	No. Pipes	Chg From	At Chg	Chg (m)	RI (m)	Chg (m)	RL (m)	etc (m)
P B1	Basin 1	N42218	15	7.9	7.825	0.5	Concrete, i	600	600	0.3	NewFixed	1	Basin 1		0				
P B2	Basin 2	N57288	10	8.5	8.45	0.5	Concrete, i	300	300	0.3	NewFixed	1	Basin 2		0				
P B3	Basin 3	N57291	10	9.5	9.45	0.5	Concrete, i	300	300	0.3	NewFixed	1	Basin 3		0				
P B4	Basin 4	N57293	10	6.4	6.35	0.5	Concrete, i	525	525	0.3	NewFixed	1	Basin 4		0				

DETAILS of SERVICES CROSSING PIPES

Pipe	Chg (m)	Bottom Elev (m)	Height of S Chg (m)	Bottom Elev (m)	Height of S Chg (m)	Bottom Elev (m)	Height of S etc (m)
------	---------	-----------------	---------------------	-----------------	---------------------	-----------------	---------------------

CHANNEL DETAILS

Name	From	To	Type	Length (m)	U/S IL (m)	D/S IL (m)	Slope (%)	Base Width (m)	L.B. Slope (1:?)	R.B. Slope (1:?)	Manning n	Depth (m)	Roofed
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OVERFLOW ROUTE DETAILS

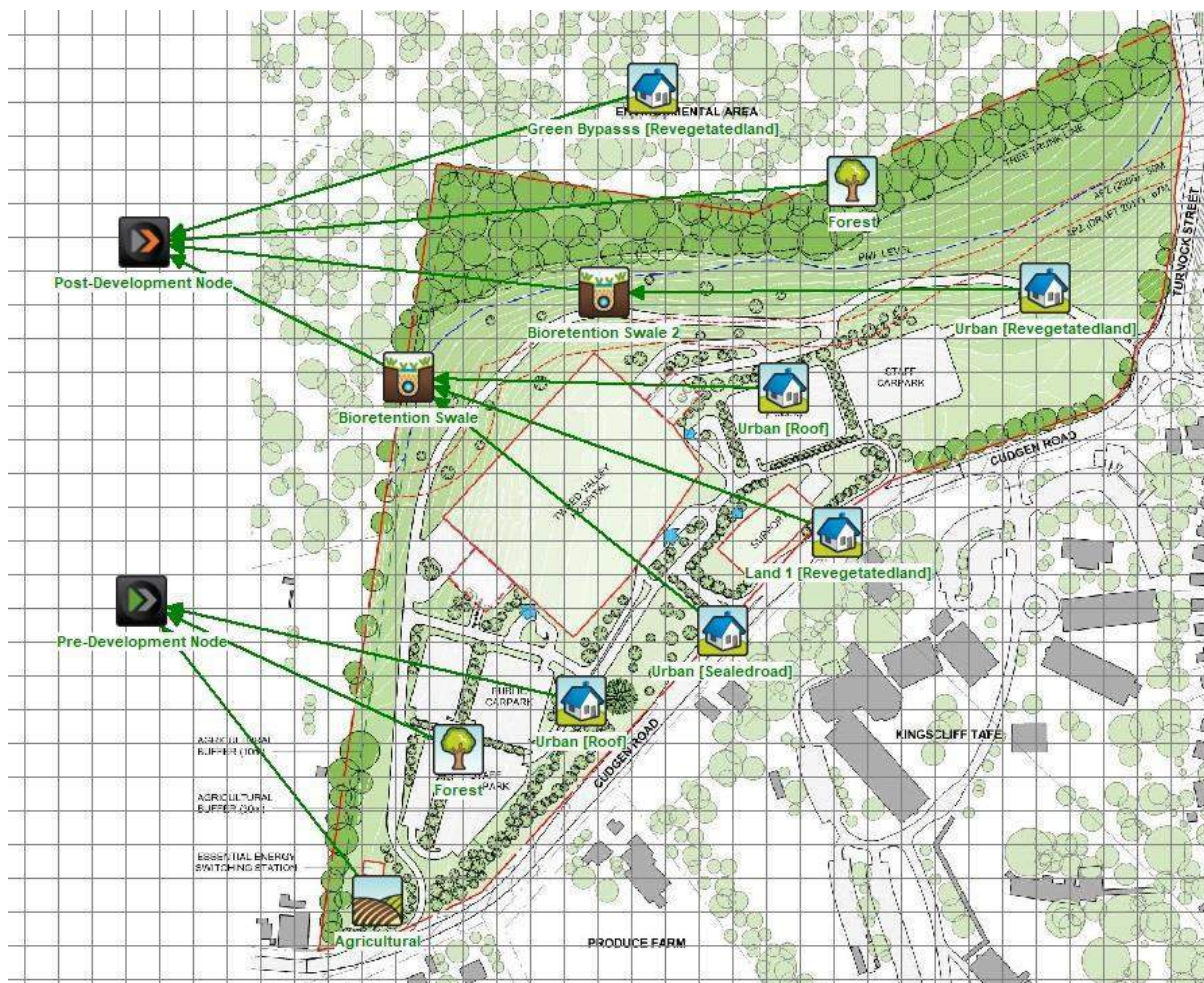
Name	From	To	Travel Time (min)	Spill Level (m)	Crest Length (m)	Weir Coeff. C	Cross Section	Safe Depth Major Stori (m)	SafeDepth Minor Stori (m)	Safe DxV (sq.m/sec)	Bed Slope (%)	D/S Area Contributing %	id	
Weir 1	Basin 1	N42218	0.1	9.3	1.5	1.7	Trapezoid	0.3	0.3	0.6	33	0	405052	8
Weir 2	Basin 2	N57288	0.1	9.9	1.5	1.7	Trapezoid	0.3	0.3	0.6	33.3	0	241367	10
Weir 3	Basin 3	N57291	0.1	10.9	1.5	1.7	Trapezoid	0.3	0.3	0.6	33.3	0	405066	10
Weir 4	Basin 4	N57293	0.1	7.8	1.5	1.7	Trapezoid	0.3	0.3	0.6	33.3	0	241379	10

PIPE COVER DETAILS

Name	Type	Dia (mm)	Safe Cove	Cover (m)	
P B1	Concrete, i	600	0.6	-0.65	Unsafe
P B2	Concrete, i	300	0.6	-0.33	Unsafe
P B3	Concrete, i	300	0.6	-0.78	Unsafe
P B4	Concrete, i	525	0.6	-1.92	Unsafe

This model has no pipes with non-return valves

5. Appendix C – Updated MUSIC Model



Treatment Train Effectiveness - Post-Development Node

	Sources	Residual Load	% Reduction
Flow (ML/yr)	149	147	1.5
Total Suspended Solids (kg/yr)	24400	4600	81.1
Total Phosphorus (kg/yr)	67.2	19.6	70.8
Total Nitrogen (kg/yr)	392	145	63.1
Gross Pollutants (kg/yr)	1940	0	100

☐ Include Pre-Development

Properties of Bioretention Swale Post-Development Node

Location: Bioretention Swale  Products >>

Inlet Properties

Low Flow By-pass (cubic metres per sec)

High Flow By-pass (cubic metres per sec)

Storage Properties

Extended Detention Depth (metres)

Surface Area (square metres)

Filter and Media Properties

Filter Area (square metres)

Unlined Filter Media Perimeter (metres)

Saturated Hydraulic Conductivity (mm/hour)

Filter Depth (metres)

TN Content of Filter Media (mg/kg)

Orthophosphate Content of Filter Media (mg/kg)

Infiltration Properties

Exfiltration Rate (mm/hr)

Lining Properties

Is Base Lined? ☒ Yes ☐ No

Vegetation Properties

☒ Vegetated with Effective Nutrient Removal Plants

☐ Vegetated with Ineffective Nutrient Removal Plants

☐ Unvegetated

Outlet Properties

Overflow Weir Width (metres)

Underdrain Present? ☒ Yes ☐ No

Submerged Zone With Carbon Present? ☐ Yes ☒ No

Depth (metres)