SITE 2A+2B
1 PARKVIEW DRIVE
SYDNEY OLYMPIC PARK

CIVIL DRAWING LIST

LOCALITY PLAN
NTS

REVISIONS:

No.	REVISION DESCRIPTION	DRAWN	DATE
A	FOR DEVELOPMENT APPLICATION	HB	14.08.19

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PROJECT TITLE
SITE 2A+2B
SYDNEY OLYMPIC PARK
AUSTRALIA AVE, SYDNEY OLYMPIC PARK, NSW 2127

DRAWING TITLE
COVER SHEET

DRAWING STATUS
APPROVAL ISSUE
NOT TO BE USED FOR CONSTRUCTION

PROJECT LEADER	DESIGNER	SIGNATURE	
RJB	PF		
DRAFTSPERSON	SCALE	DATE	SHEET SIZE
HB		DATE DRAWN	A1
JOB No.		DRAWING No.	REVISION
SY182-088		DA-C000	A

GENERAL NOTES

G1. ALL LEVELS SHALL BE OBTAINED FROM ESTABLISHED BMS OR SSM.

G2. CONTRACTOR MUST VERIFY ALL DIMENSIONS AND EXISTING LEVELS ON SITE PRIOR TO COMMENCEMENT OF WORK.

G3. ALL WORKS ARE TO BE UNDERTAKEN IN ACCORDANCE WITH COUNCIL'S SPECIFICATIONS AND THE DIRECTIONS OF THE SUPERINTENDENT.

G4. DIMENSIONS MUST NOT BE SCALED FROM DRAWINGS.

G5. CONTRACTOR TO ENSURE THAT ALL ROADWORKS ARE SMOOTHLY TRANSITIONED TO EXISTING LEVELS FREE FROM ABRUPT CHANGES.

G6. THE CONTRACTOR SHALL ARRANGE ALL SURVEY SETOUT TO BE CARRIED OUT BY A REGISTERED SURVEYOR. FURTHER, THE LOCATION OF RECOVERY MARKS SHOULD BE VERIFIED AND CONFIRMED BY THE CONTRACTOR AND ANY DISCREPANCIES SHOULD BE CLARIFIED IN WRITING WITH THE SUPERINTENDENT PRIOR TO THE COMMENCEMENT OF WORK.

G7. AT COMPLETION OF WORKS ALL ADJOINING DISTURBED AREAS ARE TO BE REINSTATED TO THE "AS FOUND" CONDITION.

G8. THE CONTRACTOR SHALL ENSURE ALL AREAS DRAIN WITH A MINIMUM FALL OF 1% (1:100) GRADE TO OUTLETS UNLESS INDICATED OTHERWISE. NO WORKS SHALL CAUSE PONDING OF STORMWATER ON UPSTREAM PROPERTIES OR CONCENTRATE RUNOFF ONTO DOWNSTREAM PROPERTIES.

G9. THESE PLANS SHALL BE READ IN CONJUNCTION WITH APPROVED LANDSCAPE, ARCHITECTURAL, ELECTRICAL, RETICULATION, WATER & SEWER DRAWINGS AND SPECIFICATIONS AND OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED.

G10. THE CONTRACTOR SHALL ENSURE THAT ALL PAVEMENTS GRADE EVENLY BETWEEN NOMINATED RL'S ON PLAN AND NO POND OF WATER OCCURS.

G11. ALL DIMENSIONS ARE IN METERS UNLESS STATED OTHERWISE. ALL LEVELS ARE EXPRESSED IN METERS.

G12. DURING CONSTRUCTION THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE STRUCTURE IN A STABLE CONDITION AND ENSURING NO PART SHALL BE OVERSTRESSED UNDER CONSTRUCTION ACTIVITIES.

G13. WORKMANSHIP AND MATERIALS ARE TO BE IN ACCORDANCE WITH THE RELEVANT CURRENT S.A.A. CODES INCLUDING ALL AMENDMENTS, AND THE LOCAL STATUTORY AUTHORITIES, EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.

G14 THE APPROVAL OF A SUBSTITUTION SHALL BE SOUGHT FROM THE ENGINEER BUT IS NOT AN AUTHORIZATION FOR AN EXTRA. ANY EXTRAS INVOLVED MUST BE TAKEN UP WITH THE SUPERINTENDENT BEFORE THE WORK COMMENCES.

G15 THE CONTRACTOR IS TO EMPLOY A QUALIFIED GEOTECHNICAL ENGINEER AS REQUIRED FOR ALL GEOTECHNICAL ASPECTS OF THE BUILDING WORKS. REFER TO FOUNDATION, GROUNDWORKS AND RETENTION/SHORING NOTES. REFER ALSO TO THE GEOTECHNICAL REPORT FOR THIS PROJECT.

SUBGRADE PREPARATION

RW1. REMOVE ALL VEGETATION, TOPSOIL AND DELETERIOUS MATERIAL FROM AREA OF PROPOSED BUILDING PLATFORM AND PAVEMENTS.

RW2. PROOF ROLL EXPOSED SUB GRADE TO ACHIEVE A MINIMUM COMPACTION OF 98% STANDARD MAXIMUM DRY DENSITY (SMDD), DETERMINED BY THE STANDARD COMPACTION TEST IN ACCORDANCE WITH CURRENT AUSTRALIAN STANDARD 1289.5.1.1.

RW3. REMOVE ANY SOFT, HEAVING, WET OR UNSTABLE AREAS IDENTIFIED DURING PROOF ROLLING AND REPLACE USING SELECT IMPORTED FILL COMPACTED IN LAYERS NOT EXCEEDING 200mm MEASURED LOOSE TO ACHIEVE A MINIMUM 98% STANDARD MAXIMUM DRY DENSITY.

RW4. NOTE THAT THE SITE IS UNDERLAIN BY EXISTING SERVICES AND COMPACTION UTILISING VIBRATION MAY NOT BE SUITABLE IN THE VICINITY OF UNDERGROUND SERVICES.

RW5. ANY FILL REQUIRED TO RAISE LEVELS TO BULK EARTHWORKS TO WITHIN 50mm OF NOMINATED LEVELS IS TO BE APPROVED GRANULAR MATERIAL COMPACTED IN LAYERS NOT EXCEEDING 300mm MEASURED LOOSE TO 98% STANDARD MAXIMUM DRY DENSITY WITHIN 2% OF STANDARD OPTIMUM MOISTURE CONTENT (SOMC).

RW6. THE CONTRACTOR IS TO PROVIDE CERTIFICATION TO THE EFFECT THAT EARTHWORKS COMPACTION TO 98% STANDARD MAXIMUM DRY DENSITY, (AS 1289 E1.1, E4.1) HAS BEEN ACHIEVED, UNLESS OTHERWISE AGREED IN WRITING BY SITE SUPERINTENDENT.

RW7. THE CONTRACTOR IS TO PROVIDE TO THE SITE SUPERINTENDENT A SURVEY CONFIRMATION FROM A REGISTERED SURVEYOR, CONFIRMING BULK EARTHWORKS LEVELS AS WITHIN +/-50mm OF LEVELS NOMINATED.

RW8. SUBGRADE REPLACEMENT MATERIAL IS TO CONSIST OF CLEAN, UNCONTAMINATED, WELL-GRADED MATERIAL WITH A MAXIMUM PARTICLE SIZE OF 75mm, WITH 80% LESS THAN 20mm, AND A SOAKED C.B.R. GREATER THAN 10% AND A PLASTICITY INDEX LESS THAN 12.

RW9. BACK FILLING FOR SERVICE TRENCHES AND REMOVED SERVICES OR PITS OR FOUNDATIONS IS TO USE APPROVED WELL-GRADED GRANULAR MATERIAL WITH MINIMUM VOIDS, (EITHER SELECT INSITU OR IMPORTED FILL), COMPACTION AS SPECIFIED ABOVE.

RW10. ALL EARTHWORKS TO BE UNDERTAKEN IN ACCORDANCE WITH AS3798-1996: GUIDELINES ON EARTHWORKS FOR COMMERCIAL & RESIDENTIAL DEVELOPMENTS.

GENERAL EARTHWORKS

E1. THE SITE OF THE WORKS SHALL BE PREPARED BY STRIPPING ALL EXISTING TOPSOIL, FILL AND VEGETATION.

E2. COMPACT SUBGRADE TO 98% OF THE STANDARD MAXIMUM DRY DENSITY WHEN TESTED IN ACCORDANCE WITH AUSTRALIAN STANDARD AS 1289 TESTS E.1.1. OR E.1.2. THE EXPOSED SUBGRADE SHOULD BE PROOF ROLLED TO DETECT ANY SOFT OR WET AREAS WHICH SHOULD BE LOCALLY EXCAVATED AND BACK FILLED WITH SELECTED MATERIAL. THE BACK FILLING MATERIAL SHALL BE IMPORTED GRANULAR FILL OF LOW PLASTICITY, PREFERABLY CRUSHED SANDSTONE, AND TO BE PLACED IN LAYERS NOT EXCEEDING 300mm LOOSE THICKNESS AND COMPACTED TO 98% OF STANDARD MAXIMUM DRY DENSITY WITHIN 2% OF STANDARD OPTIMUM MOISTURE CONTENT. SITE WORKS ARE TO BE BATTERED TO ADJACENT PROPERTY LEVELS.

E3. NO STORMWATER IS TO POND ON ADJOINING PROPERTIES. THE SITE SHALL BE GRADED AND DRAINED SO THAT STORMWATER WILL BE DIRECTED AWAY FROM THE BUILDING PLATFORM. STORMWATER DRAINAGE SHALL BE PROVIDED AND MAINTAINED THROUGHOUT THE COURSE OF CONSTRUCTION. ALL STORMWATER RUNOFF SHALL BE GRADED AWAY FROM THE DWELLING AND DISPOSED OF VIA SURFACE CATCHDRAINS AND STORMWATER COLLECTION PITS.

E4. ENSURE ALL RETAINING WALLS ARE CONSTRUCTED WITH ADEQUATE SUBSOIL DRAINAGE.

GROUND WORKS & EXCAVATION

GW1. ALL GROUND WORKS & EXCAVATION SHALL BE IN ACCORDANCE WITH GEOTECHNICAL REPORT.

GW2. SEPARATE AND REMOVE ALL TOPSOIL, NON SOIL MATERIAL, CONCRETE, VEGETATION, BRICKBATS, TIMBER, ROOT AFFECTED SOIL AND EXISTING FILL. STORE TOPSOIL IF REQUIRED.

GW3. ALL EXCAVATIONS SHALL BE FINISHED CLEAN AND HORIZONTAL AND SHALL NOT UNDERMINE FOOTINGS, WALLS etc...

GW4. PROOF ROLL WITH AN 8 TONNE ROLLER, REPLACE ANY SOFT MATERIAL WITH APPROVED FILL AND RE-COMPACT. GEOTECHNICAL ENGINEER TO APPROVE.

GW5. THE FILL IS TO BE PLACED AND COMPACTED IN LAYERS OF MAXIMUM LOOSE THICKNESS 300mm.

GW6. TOP LAYER OF PAVED AREAS TO BE COMPACTED TO MINIMUM 98% STANDARD MAXIMUM DRY DENSITY. GEOTECHNICAL ENGINEER TO VERIFY.

GW7. ALL PERMANENT EMBANKMENTS TO BE COMPACTED IN 200 mm LAYERS AS PER NOTE GW6 AND AT A MAXIMUM SLOPE OF 1 VERTICAL TO 2.5 HORIZONTAL UNLESS NOTED OTHERWISE. SHOULD DRAINAGE BE REQUIRED THEN SUBMIT DETAILS TO THE ENGINEER.

GW8. ALL GROUND WORKS SHALL BE TESTED BY AN APPROVED GEOTECHNICAL ENGINEER TO A LEVEL 1 STANDARD IN ACCORDANCE WITH AS 3798 - 1996.

GW9. ALL EXCAVATIONS TO BE INSPECTED AT REGULAR INTERVALS BY A GEOTECHNICAL ENGINEER.

GW10. REFER TO ARCHITECTURAL DRAWINGS TO CONFIRM SETOUT OF BUILDINGS, CARPARKS ETC.

GW11. THE LEVELS SHOWN ARE ONLY RELEVANT TO THE PLAN UPON WHICH THEY ARE SHOWN.

GW12. ALL CONTOURS AND LEVELS USED TO PRODUCE EARTHWORK DETAILS HAVE BEEN BASED ON SURVEYOR AND ARCHITECTS SURVEY INFORMATION.

GW13. ALL FINISHED FLOOR LEVELS ARE TO BE CONFIRMED BY ARCHITECT.

GW14. ALL EXISTING SERVICES ARE TO BE CAPPED OFF PRIOR TO ANY WORKS.

GW15. A PRE-CONSTRUCTION MEETING SHALL BE HELD BETWEEN THE CONTRACTOR, THE GEOTECHNICAL ENGINEER, AND THE EARTHWORKS CONTRACTOR TO UNDERSTAND POTENTIAL DIFFICULTIES AND TO ORGANISE TESTING PROCEDURES. THE CONTRACTOR SHALL CONFIRM TO THE ENGINEER THAT THE MEETING HAS BEEN HELD.

DRAINAGE NOTES

D1. PIT LEVELS SHOWN ON STORMWATER DRAINAGE PLANS ARE FOR INFORMATION. EXACT PIT LEVELS TO BE ADJUSTED TO SUIT FALLS IN PAVEMENT/LANDSCAPED AREA.

D2. PITS GREATER THAN 1.2m DEEP TO BE FITTED WITH STEP IRONS.

D3. DRAINAGE PIPES SHALL BE BACKFILLED WITH COMPACTED CLEAN SHARP SAND TO 200 ABOVE PIPE OBVERT. ADDITIONAL BACKFILL UNDER ROADS SHALL CONSIST OF CLASS 2 F.C.R. MATERIAL COMPACTED IN 200mm LAYERS TO 98% SMDD. UNDER LANDSCAPED AREAS ADDITIONAL BACKFILL SHALL CONSIST OF GRANULAR MATERIAL COMPACTED IN 200mm LAYERS TO 95% SMDD.
A 3m LENGTH OF 100 Ø SLOTTED AGRICULTURAL LINE SURROUNDED BY GEOTECH STOCKING SHALL BE PROVIDED ON THE UPSTREAM SIDE OF ALL PITS.

D4. CONCRETE STORMWATER PIPES TO BE CLASS '3' UNDER ROADS AND CLASS '2' IN NON-TRAFFICED AREAS. ALL PIPES GREATER THAN 300Ø ARE TO BE RUBBER RING JOINTS U.N.O.

D5. CONCRETE PITS GREATER THAN 1.0m DEEP TO BE REINFORCED WITH N12-200 EACH WAY CENTRED, MIN. 300 LAP, CONCRETE - F'c 25MPa

D6. 150Ø, 225Ø & 300Ø uPVC PIPES TO BE SEWER GRADE PIPE UNDER TRAFFICABLE PAVEMENT. MIN. 400 COVER UNDER NON-TRAFFICABLE PAVEMENT.

D7. PIT COVERS & GRATED DRAINS IN TRAFFICABLE PAVEMENT TO BE AS 3996 CLASS D "HEAVY DUTY" & IN NON-TRAFFICABLE AREAS TO BE AS 3996 CLASS C "LIGHT DUTY".

D8. GPT UNITS TO BE INSTALLED STRICTLY IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

D9. MAINTENANCE OF GPT UNITS TO BE INCORPORATED IN THE BUILDING ENVIRONMENTAL MANAGEMENT PLAN & MUST BE SUPPLEMENTED WITH A REGULAR ROAD SWEEPING PROGRAM.

UTILITY SERVICES

S1. CONDUITS TO BE PROVIDED FOR WATER AND ENERGY AUTHORITIES, TELSTRA AND OTHER SERVICES AS REQUIRED.

S2. THE LOCATIONS OF UNDERGROUND SERVICES SHOWN ON THESE DRAWING'S HAVE BEEN PLOTTED FROM SURVEY AND AUTHORITY INFORMATION. THE SERVICE INFORMATION HAS BEEN PREPARED ONLY TO SHOW THE APPROXIMATE POSITIONS OF ANY KNOWN SERVICES AND MAY NOT BE AS CONSTRUCTED OR ACCURATE.

S3. VAN DER MEER CANNOT GUARANTEE THAT THE SERVICES INFORMATION SHOWN ON THESE DRAWINGS, ACCURATELY INDICATES THE PRESENCE OR ABSENCE OF SERVICES OR THEIR LOCATION AND WILL ACCEPT NO LIABILITY FOR INACCURACIES IN THE SERVICES INFORMATION SHOWN ARISING FROM ANY CAUSE WHATSOEVER.

S4. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ESTABLISH THE LOCATION AND LEVEL OF ALL EXISTING SERVICES PRIOR TO THE COMMENCEMENT OF ANY WORK. ANY DISCREPANCIES SHALL BE REPORTED TO THE SUPERINTENDENT. CLEARANCES SHALL BE OBTAINED FROM THE RELEVANT SERVICE AUTHORITY.

S5. CONTRACTORS SHALL TAKE DUE CARE WHEN EXCAVATING ON SITE INCLUDING HAND EXCAVATION WHERE NECESSARY.

S6. CONTRACTORS ARE TO CONTACT THE RELEVANT SERVICE AUTHORITY PRIOR TO COMMENCEMENT OF EXCAVATION OR FUTURE WORKS.

S7. CONTRACTORS ARE TO UNDERTAKE A SERVICES SEARCH PRIOR TO COMMENCEMENT OF WORKS ON SITE. SEARCH RESULTS ARE TO BE KEPT ON SITE AT ALL TIMES.

TELSTRA - DUTY OF CARE NOTE:
TELSTRA'S PLANS SHOW ONLY THE PRESENCE OF CABLES AND PLANT. THEY ONLY SHOW THEIR POSITION RELATIVE TO ROAD BOUNDARIES, PROPERTY FENCES ETC. AT THE TIME OF INSTALLATION AND TELSTRA DOES NOT WARRANT OR UPHOLD THAT SUCH PLANS ARE ACCURATE THEREAFTER DUE TO CHANGES THAT MAY OCCUR OVER TIME. DO NOT ASSUME DEPTH OR ALIGNMENT OF CABLES OR PLANT AS THESE VARY SIGNIFICANTLY.

THE CONTRACTOR HAS A DUTY OF CARE WHEN EXCAVATING NEAR TELSTRA CABLES AND PLANT. BEFORE USING MACHINE EXCAVATORS TELSTRA PLANT MUST FIRST BE PHYSICALLY EXPOSED BY SOFT DIG POT HOLING TO IDENTIFY IT'S LOCATION. TELSTRA WILL SEEK COMPENSATION FOR DAMAGES CAUSED TO IT'S PROPERTY AND LOSSES CAUSED TO TELSTRA AND IT'S CUSTOMERS.

ELECTRICAL & GAS NETWORK:
A MINIMUM OF 30 DAYS PRIOR TO COMMENCEMENT OF EXCAVATION WORKS THE SUBCONTRACTOR MUST CONTACT DIAL BEFORE YOU DIG.

RETAINING WALL GENERAL

GR1. BASE MATERIAL SHALL BE COMPACTED TO MINIMUM 98% STANDARD MAXIMUM DRY DENSITY (SMDD) WITHIN 2% OF STANDARD OPTIMUM MOISTURE CONTENT (SMOC) DETERMINED BY THE STANDARD COMPACTION TEST IN ACCORDANCE WITH CURRENT AUSTRALIAN STANDARD 1289.5.1.1. MINIMUM ALLOWABLE BEARING PRESSURE OF 150 kPa. GEOTECHNICAL ENGINEER EMPLOYED BY CONTRACTOR TO INSPECT AND CONFIRM.

GR2. DRAINAGE MATERIAL WITHIN AND IMMEDIATELY BEHIND THE WALL SHALL BE 12-20mm CLEAN AGGREGATE. DRAINAGE MATERIAL TO EXTEND A MINIMUM 300 mm BEHIND WALL. COMPACT DRAINAGE MATERIAL.
ALTERNATIVELY, USE NO FINES CONCRETE, AS FOLLOWS:-
- CONCRETE STRENGTH N15.
- 210kg/m3 PORTLAND CEMENT
- MAXIMUM AGGREGATE SIZE 20 mm.
- W/C RATIO 0.45 TO 0.55.
- DENSITY 1600 TO 2000 kg/m3.

GR3. INFILL SOIL SHALL BE CLASS 1 CONTROLLED FILL TO AS4678, OR AS SPECIFIED ON THE DRAWINGS. UNSUITABLE SOILS, SUCH AS HEAVY CLAYS OR ORGANIC SOILS WITH HIGH PLASTICITY, SHALL NOT BE USED IN THE REINFORCED SOIL MASS.

GR4. SPREAD BACKFILL IN UNIFORM LIFTS OF 200 mm UNCOMPACTED THICKNESS. COMPACT TO MINIMUM 95% OF SMDD.
COMPACTION WITHIN 1.0 m BEHIND THE WALL SHALL BE ACCOMPLISHED BY USING A HAND-OPERATED PLATE COMPACTOR AND SHALL BEGIN BY RUNNING THE PLATE DIRECTLY ON THE BLOCK, THEN COMPACTING IN PARALLEL PATHS, PROGRESSIVELY AWAY FROM THE WALL FACE.

GR5. WHERE ROADWAYS OR BUILDING STRUCTURES ARE LOCATED ABOVE THE REINFORCED ZONE, COMPACT TO 98% SMDD WITHIN 2% OF SOMC DETERMINED BY THE STANDARD COMPACTION TEST IN ACCORDANCE WITH CURRENT AUSTRALIAN STANDARD 1289.5.1.1. COMPACTION TESTING SHALL BE TAKEN AT 1.2 m BEHIND THE WALL.

PAVEMENT

F1. SUBGRADE SHALL BE PREPARED AS OUTLINED IN EARTHWORKS.

F2. PAVEMENT MATERIAL SHALL CONSIST OF APPROVED OR RIPPED SANDSTONE, NATURAL GRAVEL OR FINE CRUSH ROCK AS PER COUNCIL SPECIFICATION.

F3. PAVEMENT MATERIALS SHALL BE SPREAD IN LAYERS NOT EXCEEDING 150mm AND NOT LESS THEN 75mm COMPACTED THICKNESS. PAVEMENT MATERIALS SHALL BE SIZED AND OF A STANDARD OUTLINED IN AS1141.

F4. CRUSHED OR RIPPED SANDSTONE SHALL BE MINUS 75mm NOMINAL SIZE DERIVED FROM SOUND, CLEAN SANDSTONE FREE FROM OVERBURDEN, CLAY SEAMS, SHALE AND OTHER DELETERIOUS MATERIAL.

F5. PAVEMENT MATERIALS SHALL BE COMPACTED BY SUITABLE MEANS TO SATISFY THE FOLLOWING MINIMUM SPECIFICATIONS (AS PER AS1289.52)

DESCRIPTION	MODIFIED DENSITY RATIO
SUB-BASE	98% MDD
BASE COURSE	98% MDD
ASPHALTIC CONCRETE	97% MDD

AND SUBJECT TO COUNCIL'S CONSTRUCTION SPECIFICATION.

F6. TESTING FOR EACH LAYER SHALL BE UNDERTAKEN BY A N.A.T.A. REGISTERED LABORATORY IN ACCORDANCE WITH AS1289, AT NOT MORE THAN 50m INTERVALS AND A MINIMUM OF TWO PER LAYER. FURTHER FREQUENCY OF TESTING SHALL BE NO LESS THAN THAT REQUIRED BY AS3978-1996.

REINFORCED CONCRETE BLOCKWORK

M1. CONCRETE BLOCKS SHALL BE BORAL 'CORE FILL BLOCKS', DOUBLE-U TYPE, OR SIMILAR APPROVED.

M2. MINIMUM DURABILITY REQUIREMENTS:

LOCATION	SALT ATTACK RESISTANCE GRADE OF MASONRY UNIT	MORTAR CLASS	DURABILITY CLASS OF WALL TIES AND BUILT-IN COMPONENTS
INTERIOR MASONRY	GENERAL PURPOSE	M3	R3
EXTERIOR MASONRY ABOVE DAMP PROOF COURSE	GENERAL PURPOSE	M3	R3
BELOW DAMP PROOF COURSE OR IN CONTACT WITH GROUND	EXPOSURE	M4	R4

M3. MINIMUM STRENGTH REQUIREMENTS:

ELEMENT	STRENGTH OF MASONRY UNIT	MORTAR CLASS #
CONCRETE BLOCKWORK (REINF)	f'uc = 15 MPa	M3

UNLESS A HIGHER CLASSIFICATION IS REQUIRED FOR DURABILITY (REFER NOTE M2).

M4. LAY BOTTOM COURSE OF BLOCKS ON FULL MORTAR BED.
ALL PERPENDS SHALL BE FILLED WITH MORTAR, EXCEPT WEEPHOLES.

M5. ALL CORES SHALL BE GROUTED UNLESS NOTED OTHERWISE.

M6. GROUT FOR CORE FILLING SHALL BE IN ACCORDANCE WITH AS3600, WITH THE FOLLOWING PROPERTIES:
- STRENGTH GRADE S20
- MAX. AGGREGATE SIZE 10mm
- SLUMP 230mm ± 25mm
- MIN. CEMENT CONTENT 300 kg/m³

M7. PROVIDE VERTICAL CONTROL JOINTS IN MASONRY WALLS AS FOLLOWS :

WALL TYPE	JOINT WIDTH	MAX JOINT SPACING
CONCRETE BLOCKWORK (REINF)	15mm	12m

AT CORNERS, CONTROL JOINTS SHALL BE WITHIN HALF THE SPECIFIED JOINTS SPACING FROM THE CORNER. JOINTS SHALL BE SEALED WITH AN APPROVED FLEXIBLE SEALANT. PROVIDE JOINTS TO MATCH JOINTS IN SUPPORTING SLABS.

M8. PROVIDE CLEANOUT OPENINGS AT THE BASE OF ALL REINFORCED CORES AND REMOVE ALL MORTAR PROTRUSIONS BEFORE GROUTING.
ADDITIONAL CLEANOUT OPENINGS SHALL BE PROVIDED ABOVE EACH HORIZONTAL POUR BREAK.

M9. MAXIMUM HEIGHT OF POUR FOR GROUTING SHALL NOT EXCEED 3.6m FOR 190 BLOCKWORK, AND 0.8m FOR 140 BLOCKWORK.
STOP POUR 50mm BELOW TOP OF BLOCK TO PROVIDE KEY FOR SUBSEQUENT POUR.

M10. GROUT SHALL BE THOROUGHLY COMPACTED IN THE CORES BY RODDING OR MECHANICAL VIBRATION.

CONCRETE

C1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS 3600 AND OTHER RELEVANT AUSTRALIAN STANDARDS.

C2. CONCRETE SHALL BE SUPPLIED BY AN APPROVED MANUFACTURER IN ACCORDANCE WITH AS1379.

C3. CONCRETE SHALL HAVE THE FOLLOWING PARAMETERS:

ELEMENT	SLUMP (mm)	AGGREGATE	f'c (MPa)	OTHER REQ
EXTERNAL VEHICLE SLAB	+ 80	20	N32	(1)

+ DENOTES SLUMP AT PLANT
(1) DENOTES MAXIMUM BASE SHRINKAGE STRAIN 600 x 10^{-6} AT 56 DAYS (TO AS 1012 PART 13)

C4. SIZES OF CONCRETE ELEMENTS DO NOT INCLUDE THICKNESS OF APPLIED FINISHES.

C5. BEAM DEPTHS ARE WRITTEN FIRST AND INCLUDE SLAB THICKNESS, IF ANY.

C6. HOLES, CHASES OR EMBEDMENT ITEMS, INCLUDING PIPES AND CONDUITS SHALL NOT BE PLACED IN CONCRETE MEMBERS WITHOUT PRIOR APPROVAL OF THE ENGINEER.

C7. CONDUITS, PIPES AND LIKE SHALL NOT BE PLACED WITHIN THE CONCRETE COVER, NOR DISPLACE THE REINFORCEMENT LAYERS.

C8. CONSTRUCTION JOINTS (CJ) SHALL BE PROPERLY FORMED AND USED ONLY WHERE SHOWN OR SPECIFICALLY APPROVED BY THE ENGINEER. ALL CONSTRUCTION JOINTS SHALL BE THOROUGHLY SCABBLED.

C9. THE MAXIMUM HEIGHT OF POUR FOR CONCRETE ELEMENTS SHALL BE 3.0m UNLESS METHOD OF PLACEMENT HAS BEEN APPROVED BY THE ENGINEER. COLUMNS SHALL NOT BE POURED WITH THE SLAB OVER.

C10. CONCRETE SHALL BE THOROUGHLY COMPACTED IN THE FORMS BY MEANS OF MECHANICAL VIBRATION.

C11. WHEN THE SHADE TEMPERATURE EXCEEDS 35°C, THE EXPOSED SURFACE OF CONCRETE SHALL BE SPRAYED WITH A FINE FILM OF APPROVED ALIPHATIC ALCOHOL DURING CONCRETE PLACEMENT AND FINISHING IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. ENSURING ADEQUATE SUPPLY OF ALIPHATIC ALCOHOL ON SITE BEFORE COMMENCING CONCRETE WORK.

C12. CURING OF CONCRETE SHALL COMMENCE WITHIN 2 HOURS OF FINISHING OPERATIONS AND SHALL BE MAINTAINED FOR A MINIMUM OF 7 DAYS USING AN APPROVED PROPRIETARY CURING COMPOUND IN ACCORDANCE WITH AS 3799 AND COMPATIBLE WITH THE PROPOSED FINISH OR CONTINUOUS PONDING WITH POTABLE WATER.
THE CONTRACTOR TO SUBMIT PROPOSED CURING PROCEDURE FOR APPROVAL OF THE ENGINEER.

C13. ALL CONCRETE DELIVERED TO SITE SHALL BE SUBJECT TO PROJECT ASSESSMENT IN ACCORDANCE WITH AS 1379.

C14. THE CONTRACTOR SHALL NOMINATE A CONCRETE DELIVERY SUPERVISOR WHO SHALL BE A SUITABLE EXPERIENCED PERSON FOR THE APPROVAL OF THE ENGINEER. TO MONITOR THE DELIVERY AND PLACING OF THE CONCRETE FOR EACH POUR ON THE PROJECT. IN ADDITION, THE MANUFACTURER SHALL SAMPLE AND TEST FOR DRYING SHRINKAGE EACH TYPE OF CONCRETE SUPPLIED, AT LEAST EVERY MONTH DURING THE COURSE OF THE PROJECT OR FOR EVERY 1000 CUBIC METRES PLACED. NATA TEST CERTIFICATES SHALL BE FORWARDED TO THE ENGINEER. THE RESULTS OF THESE TESTS SHALL ALSO BE KEPT ON SITE.

C15. ## CONCRETE SAMPLES AND TESTS

ARRANGE FOR A NATA REGISTERED TESTING LABORATORY TO TAKE SAMPLES OF AND TEST CONCRETE FOR COMPRESSION, FLEXURAL TENSILE STRENGTH (SLABS ON GROUND ONLY) AND SLUMP.

COMPRESSION TEST SAMPLES SHALL CONSIST OF 3 STANDARD CYLINDERS (4 STANDARD CYLINDERS FOR POST-TENSIONED CONCRETE), TESTED FOR COMPRESSIVE STRENGTH AS FOLLOWS:

ONE (1) CYLINDER AT 3 DAYS FOR POST-TENSIONED CONCRETE ONLY.
ONE (1) CYLINDER AT 7 DAYS.
TWO (2) CYLINDERS AT 28 DAYS.

THE MINIMUM NUMBER OF DAILY SAMPLES SHALL BE AS FOLLOWS:

IN COLUMNS/WALLS: 1 SAMPLE PER TRUCK

ALL OTHER CONCRETE OF ANY ONE TYPE AS FOLLOWS:
1 TRUCK PER DAY - 1 SAMPLE
2 TO 5 TRUCKS PER DAY - 2 SAMPLES
6 TO 10 TRUCKS PER DAY - 3 SAMPLES
10 TO 20 TRUCKS PER DAY - 4 SAMPLES
FOR EACH ADDITIONAL 10 TRUCKS PER DAY, 1 SAMPLE.

SLUMP: 1 SAMPLE PER TRUCK AT TIME OF POURING.

C16. REFER TO TYPICAL STRIPPING AND PROPPING DETAIL.

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AUSTRALIA AVENUE

PARKVIEW DRIVE

FRASER AVENUE

DAWN AVENUE

ROSE AVENUE

MURRAY AVENUE

BOUNDARY

BOUNDARY

BOUNDARY

BOUNDARY

CONSTRUCT SEDIMENT CONTROL FENCE AROUND PERIMETER OF WORKS

PROVIDE SEDIMENT TRAPS AROUND EXISTING INLET PITS

NOMINAL STOCKPILE LOCATION

LEGEND

- SITE BOUNDARY
- TUNNEL BELOW
- -STORMWATER SEDIMENT TRAPS AT ALL PROPOSED PIT INLETS
- -TEMPORARY CONSTRUCTION VEHICLE EXIT
- SEDIMENT CONTROL FENCE
- -STOCKPILE AREA
- -SANDBAG KERB INLET SEDIMENT TRAP

WARNING

BEWARE OF UNDERGROUND SERVICES

THE LOCATIONS OF UNDERGROUND SERVICES ARE APPROXIMATE ONLY AND THEIR EXACT POSITION SHOULD BE PROVEN ON SITE. NO GUARANTEE IS GIVEN THAT ALL EXISTING SERVICES ARE SHOWN.

PLAN
SCALE 1:200

REVISIONS:

No.	REVISION DESCRIPTION	DRAWN	DATE
A	FOR DEVELOPMENT APPLICATION	HB	14.08.19

SCALE BAR
0 1 2 3 4 5 10m 15 20m
SCALE 1:200

van der Meer Consulting
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LEVEL 6, 39 CHANDOS STREET
ST LEONARDS NSW 2065
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www.vandermeer.com.au
van der Meer (NSW) Pty Ltd
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1 AUSTRALIA AVE, SYDNEY OLYMPIC PARK, NSW 2127

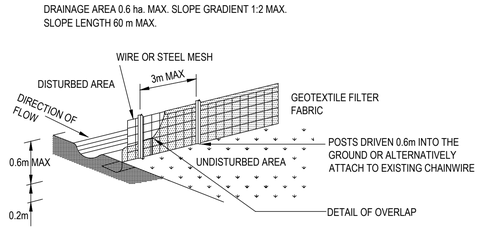
ARCHITECT
FITZPATRICK+PARTNERS
LEVEL 6, 156 CLARENCE STREET, SYDNEY 2000

PROJECT TITLE
SITE 2A+2B
SYDNEY OLYMPIC PARK
AUSTRALIA AVE, SYDNEY OLYMPIC PARK, NSW 2127

DRAWING TITLE
EROSION & SEDIMENTS PLAN

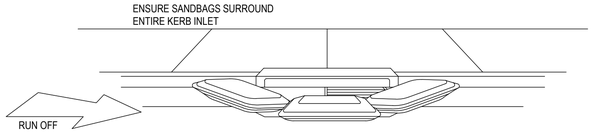
DRAWING STATUS
APPROVAL ISSUE
NOT TO BE USED FOR CONSTRUCTION

PROJECT LEADER	DESIGNER	SIGNATURE	
RJB	PF		
DRAFTSPERSON	SCALE	DATE	SHEET SIZE
HB	AS SHOWN	DATE DRAWN	A1
JOB No.		DRAWING No.	REVISION
SY182-088		DA-C210	A



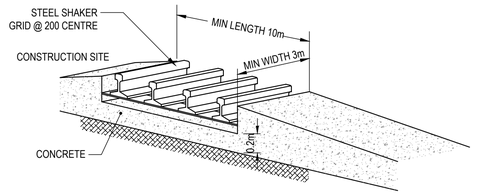
SEDIMENT CONTROL FENCE

N.T.S.



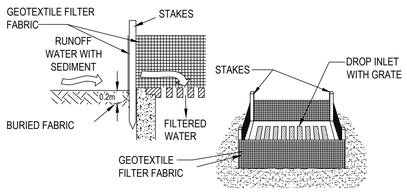
SANDBAG KERB INLET SEDIMENT TRAP

NTS



TEMPORARY CONSTRUCTION VEHICLE EXIT

N.T.S.



GEOTEXTILE FILTER FABRIC INLET SEDIMENT TRAP

N.T.S.

EROSION AND SEDIMENT NOTES

B1. THIS PLAN IS TO BE READ IN CONJUNCTION WITH EROSION AND SEDIMENT CONTROL DETAILS AS SHOWN

B2. THE CONTRACTOR SHALL IMPLEMENT ALL SOIL EROSION AND SEDIMENT CONTROL MEASURES AS NECESSARY AND TO THE SATISFACTION OF COUNCIL PRIOR TO THE COMMENCEMENT OF AND DURING CONSTRUCTION. NO DISTURBANCE TO THE SITE SHALL BE PERMITTED OTHER THAN IN THE IMMEDIATE AREA OF THE WORKS AND NO MATERIAL SHALL BE REMOVED FROM THE SITE WITHOUT COUNCIL'S APPROVAL. ALL EROSION AND SEDIMENT CONTROL DEVICES TO BE INSTALLED AND MAINTAINED IN ACCORDANCE WITH STANDARDS OUTLINED IN NSW DEPARTMENT OF HOUSING'S "MANAGING URBAN STORMWATER - SOILS AND CONSTRUCTIONS".

B3. TOPSOIL SHALL BE STRIPPED AND STOCKPILED OUTSIDE HAZARD AREAS SUCH AS DRAINAGE LINES. THIS TOPSOIL IS TO BE RESPREAD LATER ON AREAS TO BE REVEGETATED AND STABILISED ONLY, (I.E. ALL FOOTPATHS, BATTERS, SITE REGARDING AREAS, BASINS AND CATCHDRAINS). TOPSOIL SHALL NOT BE RESPREAD ON ANY OTHER AREAS UNLESS SPECIFICALLY INSTRUCTED BY THE SUPERINTENDENT. IF THEY ARE TO REMAIN FOR LONGER THAN ONE MONTH STOCKPILES SHALL BE PROTECTED FROM EROSION BY COVERING THEM WITH A MULCH AND HYDROSEEDING AND, IF NECESSARY, BY LOCATING BANKS OR DRAINS DOWNSTREAM OF A STOCKPILE TO RETARD SILT LADEN RUNOFF.

B4. THE CONTRACTOR SHALL REGULARLY MAINTAIN ALL EROSION AND SEDIMENT CONTROL DEVICES AND REMOVE ACCUMULATED SILT FROM DEVICES SUCH THAT NO MORE THAN 60% OF THEIR CAPACITY IS LOST. ALL THE SILT IS TO BE PLACED OUTSIDE THE LIMIT OF WORKS. THE PERIOD FOR MAINTAINING THESE DEVICES SHALL BE AT LEAST UNTIL ALL DISTURBED AREAS ARE REVEGETATED OR AS DIRECTED BY THE SUPERINTENDENT OR COUNCIL.

B5. VEHICULAR TRAFFIC SHALL BE CONTROLLED DURING CONSTRUCTION CONFINING ACCESS WHERE POSSIBLE TO NOMINATED STABILISED ACCESS POINTS.

B6. THE CONTRACTOR SHALL IMPLEMENT DUST CONTROL BY REGULAR WETTING DOWN (BUT NOT SATURATING) DISTURBED AREA.

B7. PROVIDE AND MAINTAIN SILT TRAPS AROUND ALL SURFACE INLET PITS UNTIL CATCHMENTS ARE REVEGETATED OR PAVED.

B8. REVEGETATE ALL TRENCHES IMMEDIATELY UPON COMPLETION OF BACKFILLING.

B9. ALL DRAINAGE PIPE INLETS TO BE CAPPED UNTIL :
A) DOWNPIPES CONNECTED
B) PITS CONSTRUCTED AND PROTECTED WITH SILT BARRIER

B10. SILT FENCE MAINTENANCE INSPECTION TO BE CARRIED OUT EVERY 3 MONTHS AND AFTER EACH RAINFALL EVENT.

B11. EROSION & SEDIMENT CONTROL SIGNAGE AVAILABLE FROM COUNCIL MUST BE ATTACHED TO THE MOST PROMINENT AVAILABLE STRUCTURE AND BE VISIBLE AT ALL TIMES WHEN ENTERING THE SITE FOR THE DURATION OF CONSTRUCTION.

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SY182-088		DA-C211	A

LEGEND

- STORMWATER PIT WITH "OCEAN GUARD" OR EQUIVALENT APPROVED PRODUCT REFER DWG DA-C402 FOR DETAILS
- STORMWATER PIPE REFER DWG DA-C402 FOR DETAILS
- 100YR SURCHARGE PATH — 100 YEAR OVERLAND FLOW PATH

AUSTRALIA AVENUE

PARKVIEW DRIVE

FRASER AVENUE

DAWN AVENUE

ROSE MURRAY AVENUE

BOUNDARY

100YR SURCHARGE PATH

BENCHMARK SSM 12536 RL 16.58 (AHD)

BASEMENT AND SHORING OUTLINE, REFER TO ARCHITECT AND STRUCTURAL PLANS FOR DETAILS

TREE FOOT PRINT & ROOT BOWL TBC

WATER QUALITY TREATMENT CHAMBERS WITHIN OSD TANK.

ON SITE DETENTION TANK
VOLUME REQUIRED: 179 m3
VOLUME PROVIDED: 185 m3

OSD TANK

PIT A-6
SL 15.90
IL 14.21

PIT A-5
SL 15.05
IL 14.10

PIT A-4
SL 15.05
IL 13.85

PIT B-1
SL 15.05
IL 13.76

PIT F-1
SL 13.00
IL 12.61

PIT A-3
SL 14.30
IL 12.56

PIT A-2
SL 14.00
IL 12.32

PIT A-1
SL 13.15
IL 12.18

PIT E-2
SL 14.87
IL 13.90

PIT D-2
SL 14.87
IL 13.90

PIT E-1
SL 14.35
IL 13.37

PIT D-1
SL 14.35
IL 13.33

PIT C-4
SL 13.83
IL 12.85

PIT C-3
SL 14.25
IL 12.72

PIT C-2
SL 13.93
IL 12.59

PIT C-1
SL 13.18
IL 12.39

SL 15.45

SL 14.50

3750 RCP

3750Ø RCP

4500Ø RCP

450Ø RCP

IL 13.88

2250Ø UPVC

IL 13.70

300Ø UPVC WITH NON RETURN FLAP VALVE.

IL 12.37

IL 12.33

375Ø RCP

PROPOSED KERB INLET PIT WITH EXTENDED PIT CHAMBER TO MATCH EXISTING.

EXISTING PIT COVER TO BE CONVERTED INTO SEAL PIT LID. EXISTING STORMWATER OUTLET TO BE CONFIRMED BY SURVEYOR AT LATER STAGE.

STUD FOR ALL DOWNPIPES CONNECTION. INDICATIVE ONLY FINAL DP LOCATIONS REFER TO HYDRAULIC CONSULTANTS DRAWING.

STUD FOR ALL DOWNPIPES CONNECTION. INDICATIVE ONLY FINAL DP LOCATIONS REFER TO HYDRAULIC CONSULTANTS DRAWING.

ROOF 19.14

PLAN
SCALE 1:200

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DRAINAGE LAYOUT

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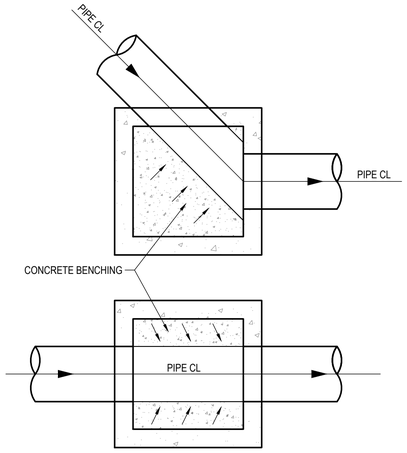
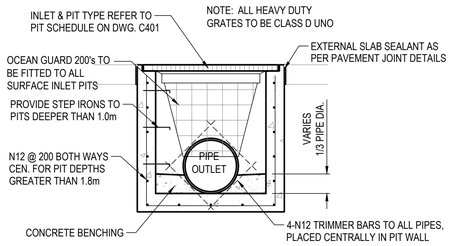


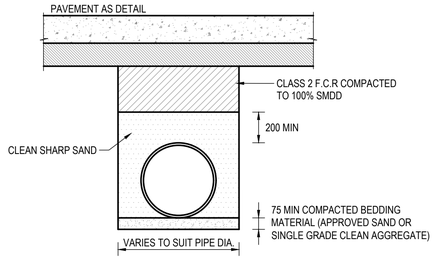
TYPICAL CONCRETE BENCHING

SCALE 1:25



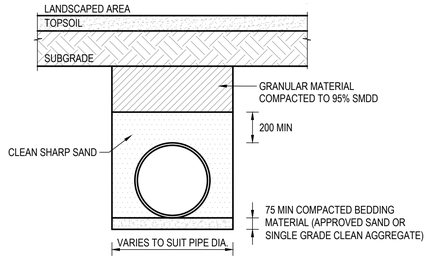
TYPICAL INLET PIT & JUNCTION PIT

SCALE 1:25



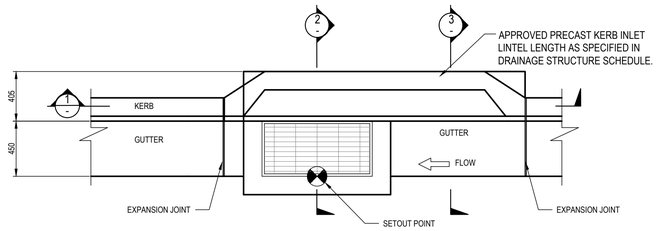
UNDER PAVEMENT PIPE BEDDING

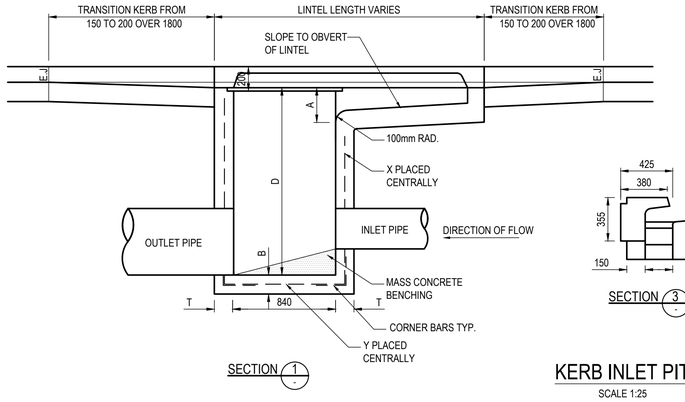
NTS



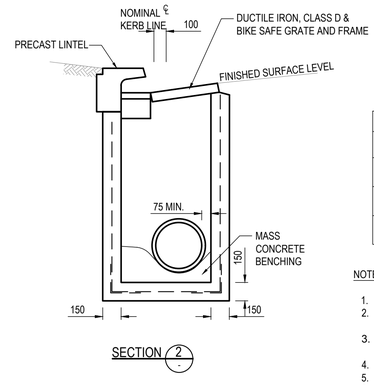
UNDER LANDSCAPING PIPE BEDDING

NTS





SECTION 1



SECTION 2

SECTION 3

KERB INLET PIT

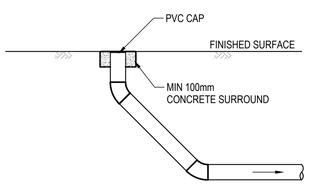
SCALE 1:25

MINIMUM DIMENSION A (mm)	INLET LENGTH EKI (m)
250	1.8
300	2.4
400	3.0
450	3.6
500	4.2

PIT SPECIFICATIONS					
D	B	T	X	Y	CORNER BARS
750 - 1200	150	150	SL82	SL81	N12-300 L 500
1200 - 2100	200	180	SL81	SL81	N12-200 L 600
> 2100	TO STRUCTURAL ENGINEERS SPECIFICATION				

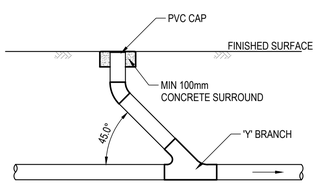
NOTES:

1. COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS TO BE 32MPa.
2. 75mm MINIMUM BENCHING TO HALF PIPE HEIGHT TOTAL BENCHING TO OBVERT OF PIPE.
3. 100mmØ SUBSOIL DRAINAGE PIPE 3.0m LONG WRAPPED IN FABRIC SOCK TO BE PROVIDED IN PIPE TRENCHES ADJACENT TO INLET PIPES.
4. PROVIDE STEP IRONS WHERE PIT IS DEEPER THAN 1.0m AT 300mm CENTRES.
5. PITS OVER 2.1m IN DEPTH TO BE DESIGNED BY STRUCTURAL ENGINEER.
6. GRATES SHALL BE BICYCLE SAFE AND HAVE MAXIMUM INLET CAPACITY. ALL GRATES MUST BE APPROVED BY THE CITY'S REPRESENTATIVE.
7. REINFORCEMENT TO COMPLY WITH AS 1302, 1303 & 1304.
8. DRAINAGE PIPE TO BE MINIMUM 375Ø CLASS 4 REINFORCED CONCRETE PIPE



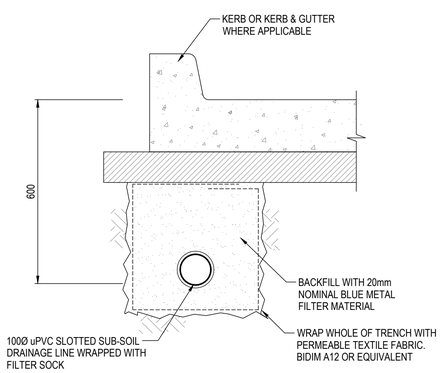
FLUSH POINT DETAIL

SCALE 1:20



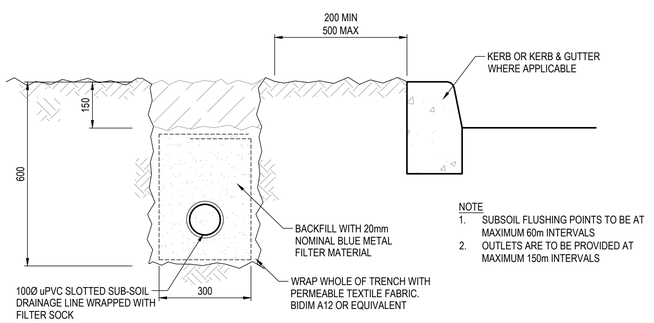
FLUSH POINT DETAIL (INLINE)

SCALE 1:20



SUB-SOIL DRAIN DETAIL

SCALE 1:10



NOTE
1. SUBSOIL FLUSHING POINTS TO BE AT MAXIMUM 60m INTERVALS
2. OUTLETS ARE TO BE PROVIDED AT MAXIMUM 150m INTERVALS

SUB-SOIL DRAIN DETAIL

SCALE 1:10

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SCALE BAR 0 .1 .2 .3 .4 .5 1m 1.5 2m

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SCALE 1:25

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DRAINAGE DETAILS

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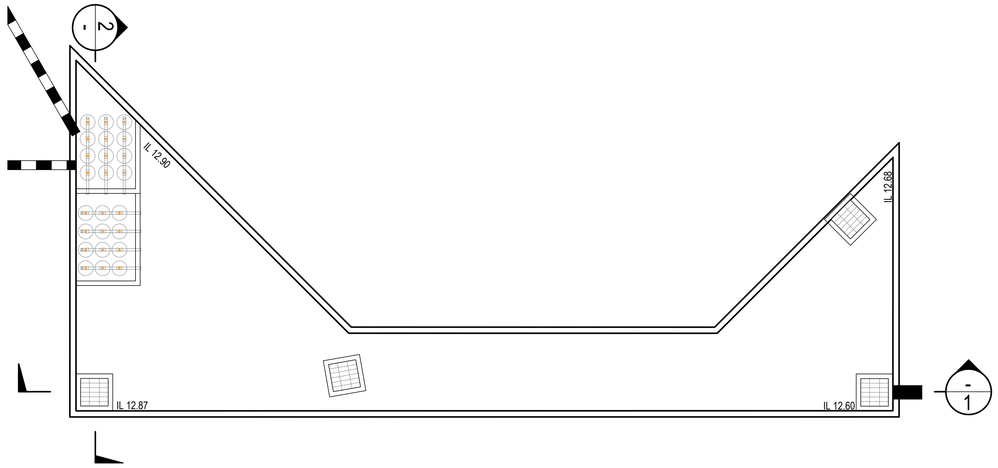
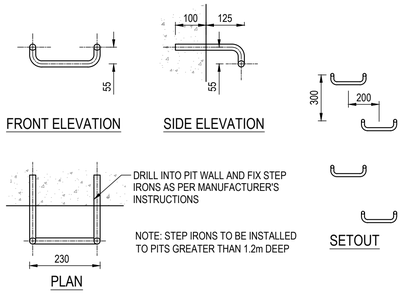
SY182-088		DA-C402	A



OSD TANK

SCALE 1:100

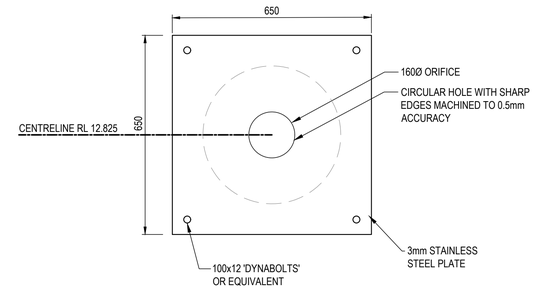


TYPICAL STEP IRON DETAIL

NTS

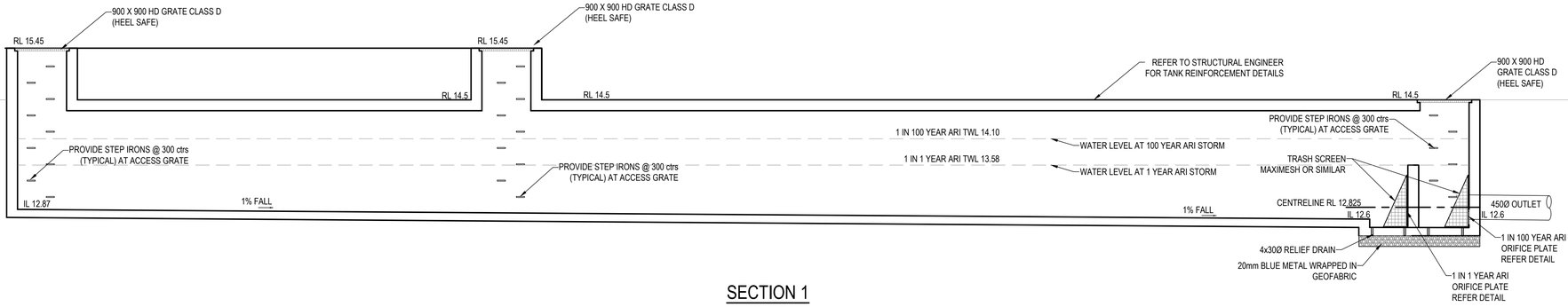
SITE PEAK DISCHARGES				
REDUCE PEAK FLOWS FOR ALL STORMS BACK TO UNDEVELOPED STATEAS PER COUNCILS REQUIREMENT AND SITE AREA = 7,711m²				
PRE DEVELOPMENT 100% PERVIOUS		PROPOSED DEV. WITH OSD		
STORM (ARI)	PEAK FLOW (m³/s)	STORM (ARI)	PEAK FLOW (m³/s)	PEAK STORAGE (m³)
1 YEAR	0.062	1 YEAR	0.060	120.5
100 YEAR	0.314	100 YEAR	0.194	184.5

NOTE:
1. RESULTS AS PER DRAINS OUTPUT
2. CALCULATIONS DO NOT CONSIDER RAINWATER REUSE



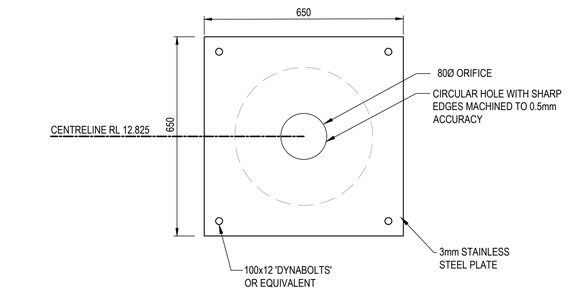
1 IN 100 YEAR ARI ORIFICE PLATE DETAIL

SCALE 1:10



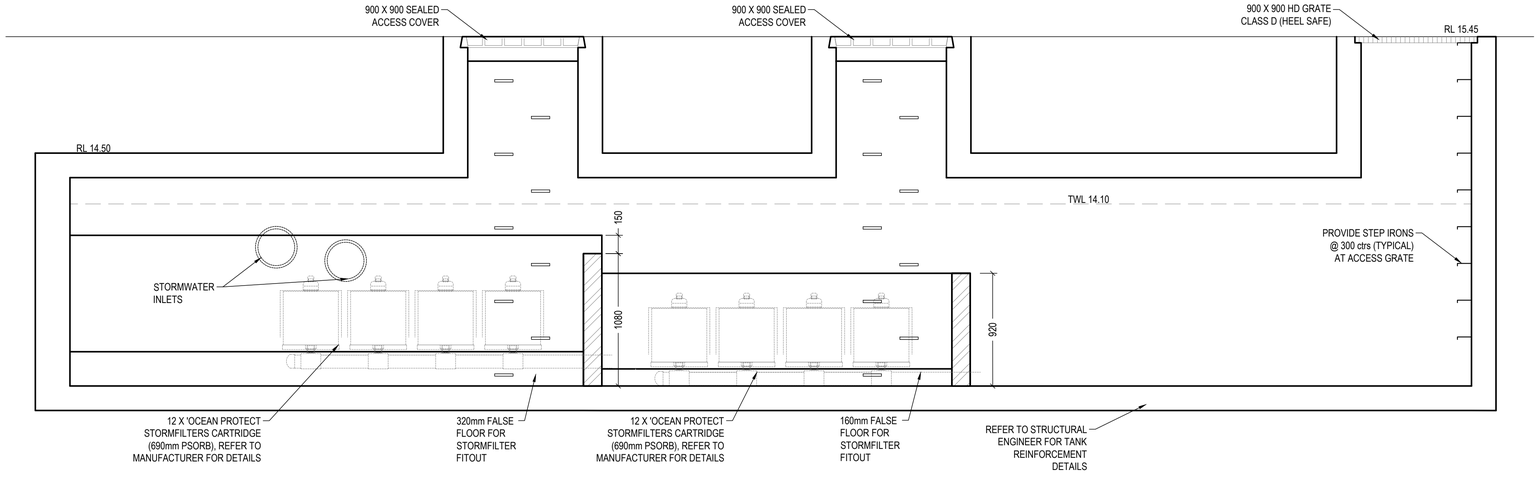
SECTION 1

SCALE 1:50



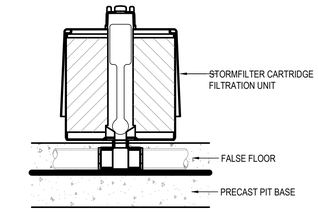
1 IN 1 YEAR ARI ORIFICE PLATE DETAIL

SCALE 1:10



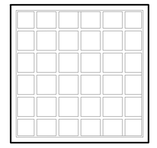
SECTION 2

SCALE 1:25



STROMFILTER DETAIL

NTS



900 x 900 ACCESS COVER

900X900 ACCESS COVER

NTS

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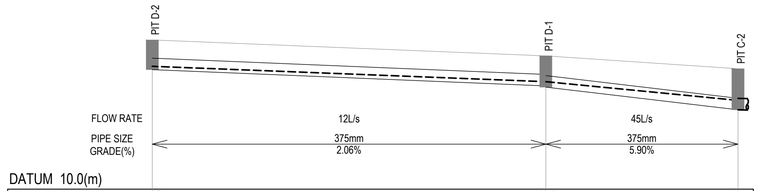
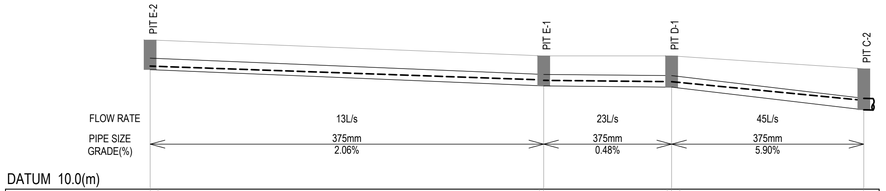


DATUM 10.0(m)

HGL 20YR ARI	14.010	13.508 / 13.508	12.897
SURFACE LEVEL	14.87	14.35	13.93
INVERT LEVEL	13.900	13.370 / 13.330	12.590
CHAINAGE (m)	0	25.70	38.25

LINE D

SCALE 1:200 (H)
SCALE 1:100 (V)

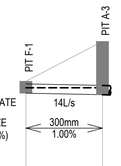


DATUM 10.0(m)

HGL 20YR ARI	14.016	13.563 / 13.551	13.508 / 13.508	12.897
SURFACE LEVEL	14.87	14.35	14.35	13.93
INVERT LEVEL	13.900	13.370 / 13.370	13.330 / 13.330	12.590
CHAINAGE (m)	0	25.70	34.05	46.60

LINE E

SCALE 1:200 (H)
SCALE 1:100 (V)

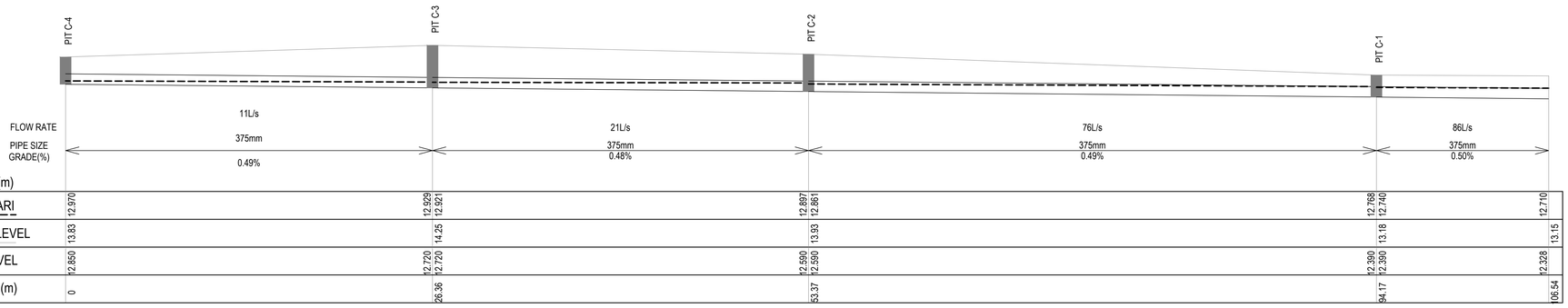


DATUM 10.0(m)

HGL 20YR ARI	12.787	12.787
SURFACE LEVEL	13	14.30
INVERT LEVEL	12.610	12.560
CHAINAGE (m)	0	5.00

LINE F

SCALE 1:200 (H)
SCALE 1:100 (V)

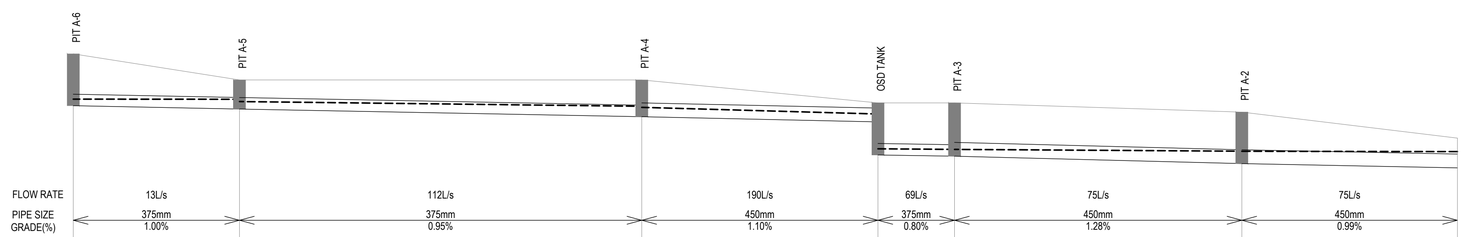


DATUM 9.0(m)

HGL 20YR ARI	12.970	12.929 / 12.921	12.897 / 12.861	12.768 / 12.740	12.710
SURFACE LEVEL	13.83	14.25	13.93	13.18	13.15
INVERT LEVEL	12.850	12.720 / 12.720	12.590 / 12.590	12.390 / 12.390	12.328
CHAINAGE (m)	0	26.36	53.37	94.17	106.54

LINE C

SCALE 1:200 (H)
SCALE 1:100 (V)

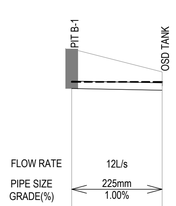


DATUM 9.0(m)

HGL 20YR ARI	14.423	14.419 / 14.344	14.169 / 14.155	13.937 / 12.802	12.782 / 12.782	12.724 / 12.721 / 12.710
SURFACE LEVEL	15.9	15.05	15.05	14.300	14.30	14.06 / 13.15
INVERT LEVEL	14.209	14.100 / 14.100	13.850 / 13.850	13.680 / 12.600	12.560 / 12.560	12.320 / 12.320 / 12.180
CHAINAGE (m)	0	10.85	37.12	52.54	57.54	76.30 / 90.38

LINE A

SCALE 1:200 (H)
SCALE 1:100 (V)



DATUM 9.0(m)

HGL 20YR ARI	13.974	13.974
SURFACE LEVEL	15.05	14.300
INVERT LEVEL	13.759	13.700
CHAINAGE (m)	0	5.91

LINE B

SCALE 1:200 (H)
SCALE 1:100 (V)

REVISIONS:

No.	REVISION DESCRIPTION	DRAWN	DATE
A	FOR DEVELOPMENT APPLICATION	HB	14.08.19

SCALE BAR
0 1 2 3 4 5m 10m
SCALE 1:100
0 1 2 3 4 5 10m 15 20m
SCALE 1:200

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ARCHITECT
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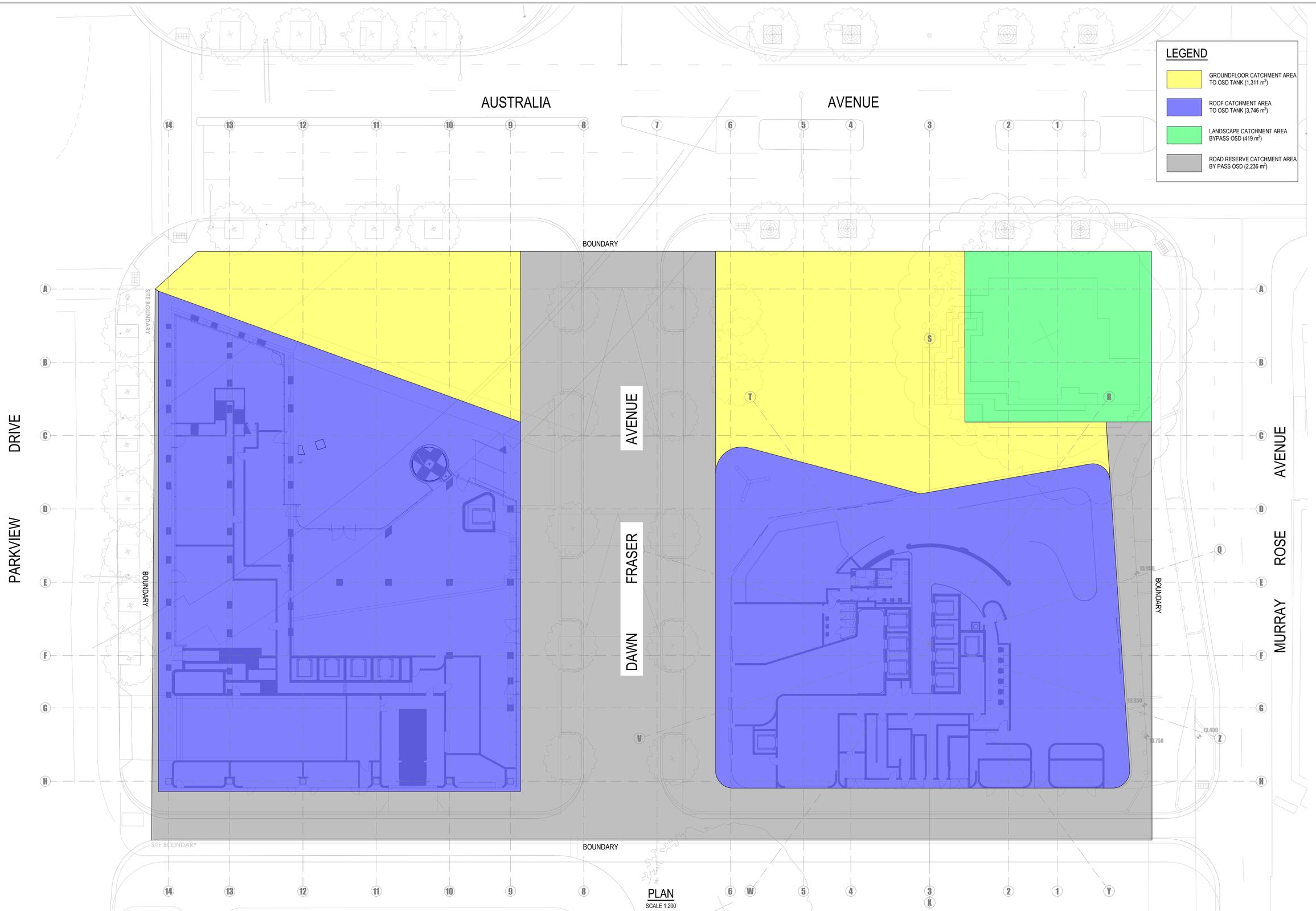
LEVEL 6, 156 CLARENCE STREET, SYDNEY 2000

PROJECT TITLE
SITE 2A+2B
SYDNEY OLYMPIC PARK
AUSTRALIA AVE, SYDNEY OLYMPIC PARK, NSW 2127

DRAWING TITLE
DRAINAGE LONG. SECTIONS

DRAWING STATUS
APPROVAL ISSUE
NOT TO BE USED FOR CONSTRUCTION

PROJECT LEADER	DESIGNER	SIGNATURE	
RJB	PF		
DRAFTSPERSON	SCALE	DATE	SHEET SIZE
HB	AS SHOWN	DATE DRAWN	A1
JOB No.		DRAWING No.	REVISION
SY182-088		DA-C404	A



PLAN
SCALE 1:200

REVISIONS:			
A	FOR DEVELOPMENT APPLICATION	HB	14.08.19
No.	REVISION DESCRIPTION	DRAWN	DATE

SCALE BAR
0 1 2 3 4 5 10m 15 20m
SCALE 1:200

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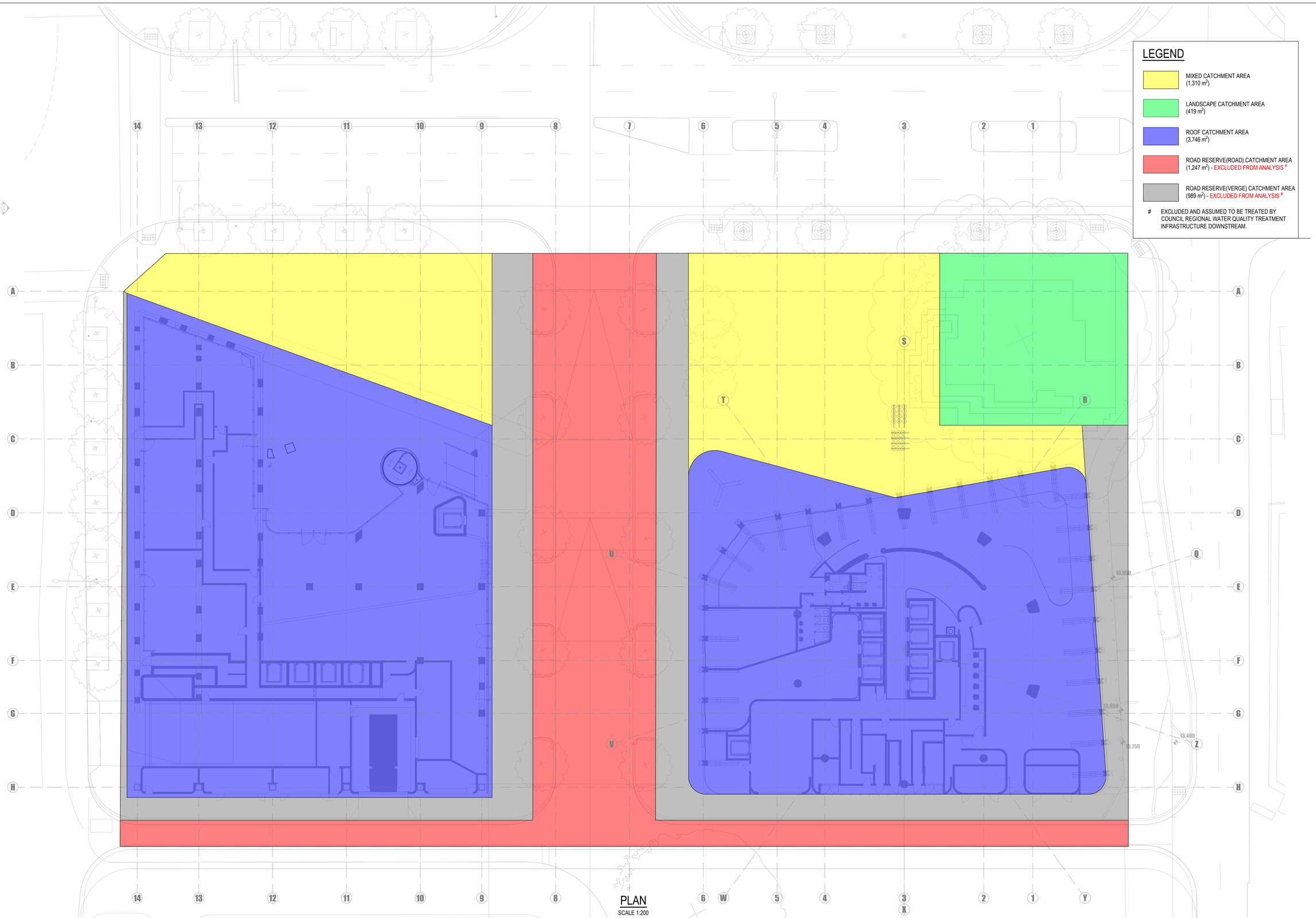
PROJECT TITLE
SITE 2A+2B
SYDNEY OLYMPIC PARK
AUSTRALIA AVE, SYDNEY OLYMPIC PARK, NSW 2127

DRAWING TITLE
DRAINS CATCHMENT PLAN

DRAWING STATUS
APPROVAL ISSUE
NOT TO BE USED FOR CONSTRUCTION

PROJECT LEADER	DESIGNER	SIGNATURE	
RJB	PF		
DRAFTSPERSON	SCALE	DATE	SHEET SIZE
HB	AS SHOWN	DATE DRAWN	A1
JOB No.		DRAWING No.	REVISION
SY182-088		DA-C410	A



PLAN
SCALE 1:200

REVISIONS:			
A	FOR DEVELOPMENT APPLICATION	HB	14.08.19
No.	REVISION DESCRIPTION	DRAWN	DATE

SCALE BAR
0 1 2 3 4 5 10m 15 20m
SCALE 1:200

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PROJECT TITLE
SITE 2A+2B
SYDNEY OLYMPIC PARK
AUSTRALIA AVE, SYDNEY OLYMPIC PARK, NSW 2127

DRAWING TITLE
MUSIC CATCHMENT PLAN

DRAWING STATUS
APPROVAL ISSUE
NOT TO BE USED FOR CONSTRUCTION

PROJECT LEADER	DESIGNER	SIGNATURE	
RJB	PF		
DRAFTSPERSON	SCALE	DATE	SHEET SIZE
HB	AS SHOWN	DATE DRAWN	A1
JOB No.		DRAWING No.	REVISION
SY182-088		DA-C411	A