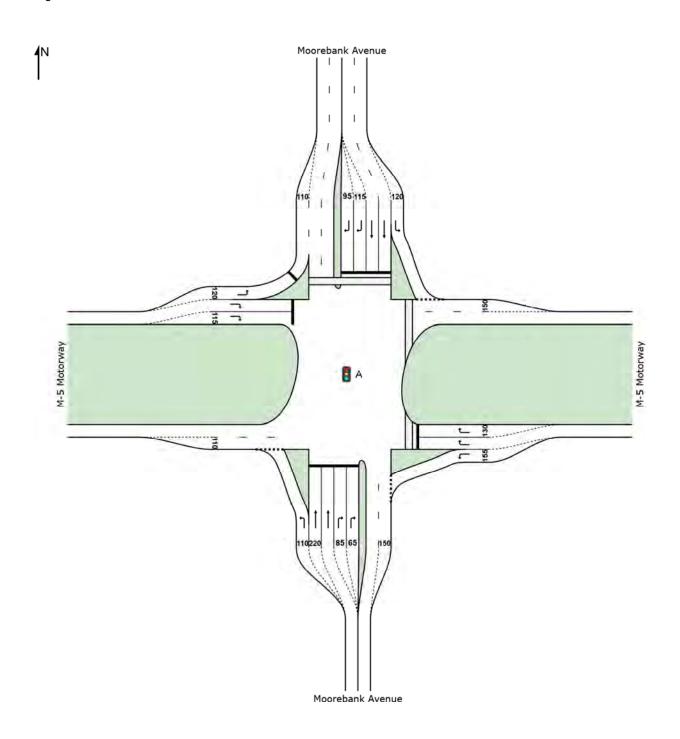
APPENDIX B – DETAILED SIDRA MOVEMENT SUMMARIES – SCENARIO 2

Stage 1

Site: A [M5/Moorebank Avenue_AM]

Intersection of Moorebank Avenue and M5 Motorway AM PEAK
Signals - Fixed Time Isolated



Site: A [M5/Moorebank Avenue_AM]

Intersection of Moorebank Avenue and M5 Motorway

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

| Move | | Performan | | | | | | | | | | | |
|-----------|-----------|-----------------|-------------|------------------|-------------|--------------|------------------|---------------------|----------------------|----------------------|-----------------|------------------------|------|
| Mov ID | OD Mov | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delav | Level of Service | 95% Back Vehicles | of Queue Distance | Prop. Queued | Effective Stop Rate | |
| טו | IVIOV | veh/h | % | veh/h | % | v/c | sec | Service | venicies | m | Queueu | per veh | km/h |
| South | : Moorel | bank Avenu | е | | | | | | | | | | |
| 1 | L2 | 419 | 12.8 | 419 | 12.8 | 0.377 | 14.2 | LOSA | 9.7 | 85.0 | 0.41 | 0.73 | 50.8 |
| 2 | T1 | 402 | 3.4 | 402 | 3.4 | 0.252 | 29.2 | LOS C | 9.3 | 69.3 | 0.68 | 0.58 | 34.6 |
| 3 | R2 | 261 | 17.3 | 261 | 17.3 | 0.410 | 57.4 | LOS E | 8.9 | 83.5 | 0.88 | 0.79 | 26.5 |
| Appro | ach | 1082 | 10.4 | 1082 | 10.4 | 0.410 | 30.2 | LOS C | 9.7 | 85.0 | 0.63 | 0.69 | 37.2 |
| East: | M-5 Mot | orway | | | | | | | | | | | |
| 4 | L2 | 329 | 15.3 | 329 | 15.3 | 0.272 | 6.2 | LOSA | 1.7 | 15.5 | 0.13 | 0.59 | 47.6 |
| 6 | R2 | 243 | 4.3 | 243 | 4.3 | 0.949 | 104.0 | LOS F | 10.7 | 81.6 | 1.00 | 1.05 | 17.1 |
| Appro | ach | 573 | 10.7 | 573 | 10.7 | 0.949 | 47.8 | LOS D | 10.7 | 81.6 | 0.50 | 0.78 | 23.7 |
| North | : Mooreb | ank Avenu | е | | | | | | | | | | |
| 7 | L2 | 48 | 19.6 | 48 | 19.6 | 0.042 | 7.2 | LOSA | 0.5 | 4.4 | 0.17 | 0.58 | 52.9 |
| 8 | T1 | 218 | 6.8 | 218 | 6.8 | 0.156 | 27.7 | LOS B | 5.2 | 41.5 | 0.65 | 0.53 | 24.6 |
| 9 | R2 | 506 | 20.2 | 506 | 20.2 | 0.967 | 87.5 | LOS F | 28.7 | 282.0 | 0.98 | 0.98 | 22.1 |
| Appro | ach | 773 | 16.3 | 773 | 16.3 | 0.967 | 65.6 | LOS E | 28.7 | 282.0 | 0.83 | 0.82 | 23.3 |
| West: | M-5 Mo | torway | | | | | | | | | | | |
| 10 | L2 | 1356 | 7.6 | 1356 | 7.6 | 0.887 | 7.1 | LOSA | 21.5 | 173.2 | 0.48 | 0.66 | 50.5 |
| 12 | R2 | 512 | 8.0 | 512 | 8.0 | 0.778 | 67.0 | LOS E | 19.8 | 160.8 | 0.99 | 0.86 | 19.2 |
| Appro | ach | 1867 | 7.7 | 1867 | 7.7 | 0.887 | 23.5 | LOS B | 21.5 | 173.2 | 0.62 | 0.72 | 38.5 |
| All Ve | hicles | 4295 | 10.3 | 4295 | 10.3 | 0.967 | 36.0 | LOS C | 28.7 | 282.0 | 0.64 | 0.74 | 32.5 |

+ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 %

Number of Iterations: 16 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | | |
|--------|------------------------------------|--------|---------|---------|--------------|----------|--------|-----------|--|--|--|
| Mov | Decembrish | Demand | Average | | Average Back | | Prop. | Effective | | | |
| ID | Description | Flow | Delay | Service | | Distance | Queued | Stop Rate | | | |
| | | ped/h | sec | | ped | m | | per ped | | | |
| P21 | East Stage 1 | 26 | 64.5 | LOS F | 0.1 | 0.1 | 0.93 | 0.93 | | | |
| P22 | East Stage 2 | 26 | 68.2 | LOS F | 0.1 | 0.1 | 0.95 | 0.95 | | | |
| P3 | North Full Crossing | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 | | | |
| All Pe | destrians | 79 | 67.3 | LOS F | | | 0.95 | 0.95 | | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: A [M5/Moorebank Avenue_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and M5 Motorway AM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

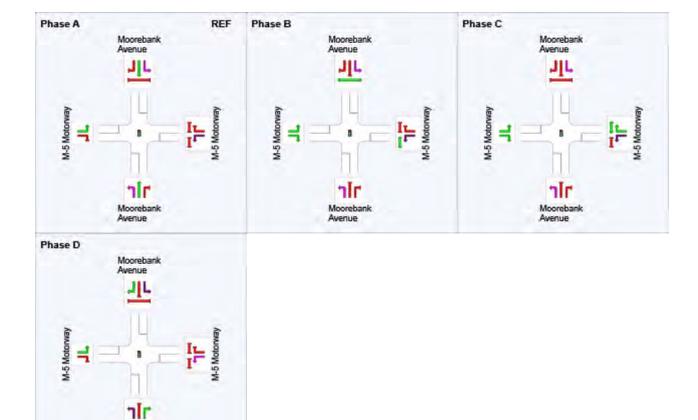
Phase Times determined by the program

Phase Sequence: 4-phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|-----|-----|
| Phase Change Time (sec) | 0 | 70 | 91 | 108 |
| Green Time (sec) | 64 | 15 | 11 | 36 |
| Phase Time (sec) | 70 | 21 | 17 | 42 |
| Phase Split | 47% | 14% | 11% | 28% |

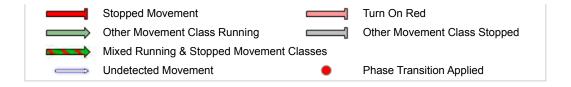
See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

Moorebank Avenue

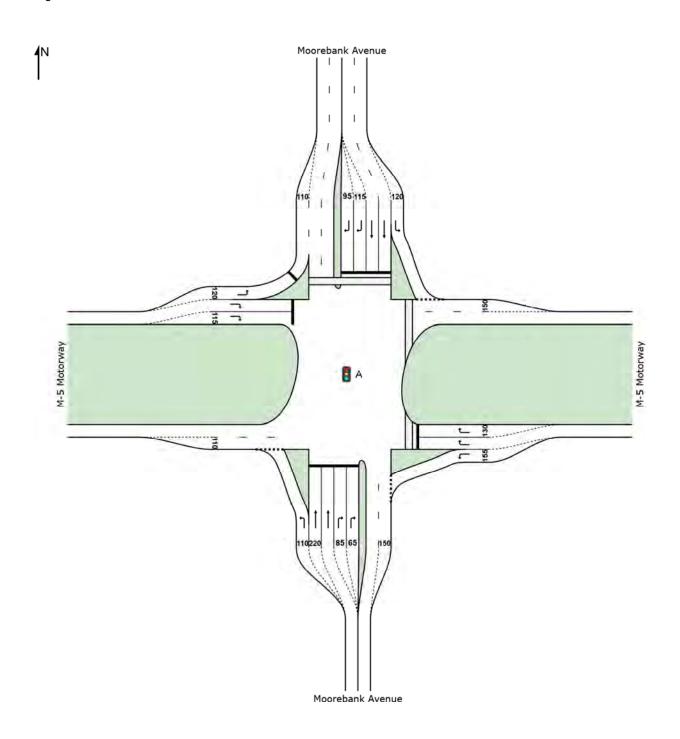




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Site: A [M5/Moorebank Avenue_PM]

Intersection of Moorebank Avenue and M5 Motorway PM PEAK
Signals - Fixed Time Isolated



Site: A [M5/Moorebank Avenue_PM]

Intersection of Moorebank Avenue and M5 Motorway PM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

| | | Performan | | | | | | | | | | | |
|--------|----------|----------------------|---------|---------|-----|-------|---------|----------|----------|----------|--------|-----------|------|
| Mov | OD | Demand | | Arrival | | Deg. | Average | Level of | 95% Back | | Prop. | Effective | |
| ID | Mov | Total veh/h | HV % | Total | HV | Satn | Delay | Service | Vehicles | Distance | Queued | Stop Rate | |
| South | · Moorel | ven/n pank Avenue | | veh/h | % | v/c | sec | | veh | m | | per veh | km/h |
| 1 | L2 | 457 | - | 457 | 6.7 | 0.636 | 38.9 | LOS C | 22.2 | 175.8 | 0.85 | 1.00 | 36.8 |
| | | | 6.7 | | | | | | | | | 1.00 | |
| 2 | T1 | 236 | 3.1 | 236 | 3.1 | 0.471 | 65.8 | LOS E | 8.1 | 60.3 | 0.97 | 0.78 | 22.6 |
| 3 | R2 | 320 | 8.2 | 320 | 8.2 | 0.187 | 22.1 | LOS B | 6.1 | 49.4 | 0.51 | 0.70 | 42.0 |
| Appro | ach | 1013 | 6.3 | 1013 | 6.3 | 0.636 | 39.8 | LOS C | 22.2 | 175.8 | 0.77 | 0.86 | 33.8 |
| East: | M-5 Mot | orway | | | | | | | | | | | |
| 4 | L2 | 272 | 9.7 | 272 | 9.7 | 0.227 | 7.1 | LOSA | 2.8 | 23.1 | 0.20 | 0.61 | 46.3 |
| 6 | R2 | 87 | 6.0 | 87 | 6.0 | 0.642 | 89.0 | LOS F | 3.4 | 26.9 | 1.00 | 0.78 | 19.0 |
| Appro | ach | 359 | 8.8 | 359 | 8.8 | 0.642 | 27.0 | LOS B | 3.4 | 26.9 | 0.39 | 0.65 | 30.5 |
| North | : Mooreb | ank Avenue |) | | | | | | | | | | |
| 7 | L2 | 74 | 5.7 | 74 | 5.7 | 0.061 | 6.4 | LOSA | 0.5 | 3.8 | 0.14 | 0.58 | 56.3 |
| 8 | T1 | 405 | 1.8 | 405 | 1.8 | 0.864 | 74.2 | LOS F | 17.4 | 126.4 | 1.00 | 0.92 | 12.4 |
| 9 | R2 | 1296 | 4.5 | 1296 | 4.5 | 0.884 | 35.2 | LOS C | 46.1 | 352.4 | 0.76 | 0.85 | 38.0 |
| Appro | ach | 1775 | 4.0 | 1775 | 4.0 | 0.884 | 42.9 | LOS D | 46.1 | 352.4 | 0.79 | 0.85 | 31.6 |
| West: | M-5 Mo | torway | | | | | | | | | | | |
| 10 | L2 | 595 | 7.3 | 595 | 7.3 | 0.387 | 6.1 | LOSA | 2.8 | 22.5 | 0.13 | 0.56 | 52.0 |
| 12 | R2 | 429 | 7.6 | 429 | 7.6 | 0.769 | 70.5 | LOS F | 16.8 | 135.6 | 0.99 | 0.86 | 18.6 |
| Appro | ach | 1024 | 7.4 | 1024 | 7.4 | 0.769 | 33.1 | LOS C | 16.8 | 135.6 | 0.49 | 0.69 | 33.2 |
| All Ve | hicles | 4171 | 5.8 | 4171 | 5.8 | 0.884 | 38.4 | LOS C | 46.1 | 352.4 | 0.68 | 0.80 | 32.5 |

+ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 %

Number of Iterations: 9 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | |
|--------|------------------------------------|--------|---------|---------|--------------|----------|--------|-----------|--|--|
| Mov | D | Demand | Average | | Average Back | | Prop. | Effective | | |
| ID | Description | Flow | Delay | Service | Pedestrian | Distance | Queued | Stop Rate | | |
| | | ped/h | sec | | ped | m | | per ped | | |
| P21 | East Stage 1 | 26 | 64.5 | LOS F | 0.1 | 0.1 | 0.93 | 0.93 | | |
| P22 | East Stage 2 | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 | | |
| P3 | North Full Crossing | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 | | |
| All Pe | destrians | 79 | 67.6 | LOS F | | | 0.95 | 0.95 | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: A [M5/Moorebank Avenue_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and M5 Motorway

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

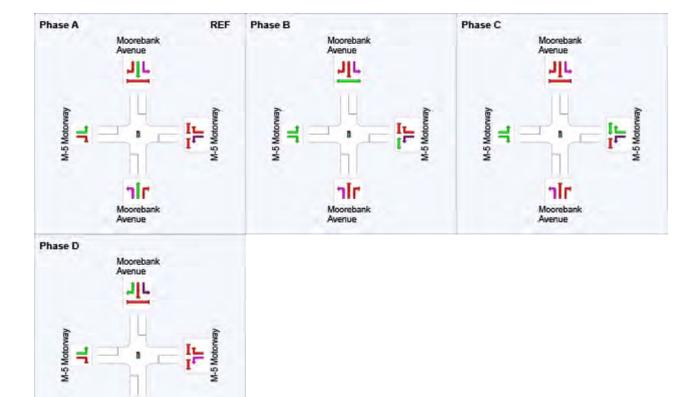
Phase Times determined by the program

Phase Sequence: 4-phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|----|-----|
| Phase Change Time (sec) | 0 | 26 | 47 | 59 |
| Green Time (sec) | 20 | 15 | 6 | 85 |
| Phase Time (sec) | 26 | 21 | 12 | 91 |
| Phase Split | 17% | 14% | 8% | 61% |

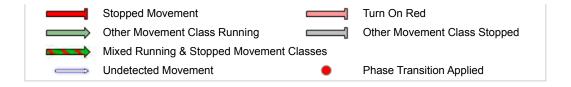
See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

Moorebank Avenue

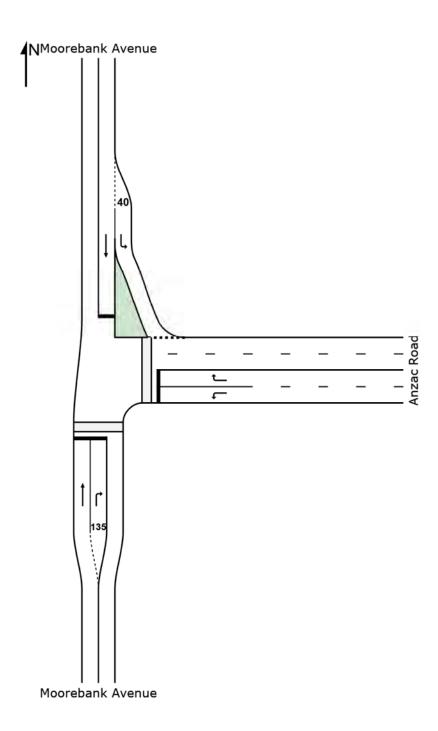




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Site: C [Moorebank Avenue_Anzac Road_AM]

Intersection of Moorebank Avenue and Anzac Road AM PEAK
Signals - Fixed Time Isolated



Site: C [Moorebank Avenue_Anzac Road_AM]

Intersection of Moorebank Avenue and Anzac Road AM PEAK

Signals - Fixed Time Isolated Cycle Time = 110 seconds (Practical Cycle Time)

| Move | ment F | Performan | ce - Ve | hicles | | | | | | | | | |
|-----------|-----------|--------------------------|---------|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | HV | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South | : Moore | bank Avenu | е | | | | | | | | | | |
| 2 | T1 | 738 | 9.3 | 738 | 9.3 | 0.598 | 10.9 | LOSA | 21.7 | 180.5 | 0.60 | 0.55 | 28.9 |
| 3 | R2 | 381 | 3.3 | 381 | 3.3 | 0.953 | 76.0 | LOS F | 26.7 | 199.4 | 1.00 | 1.12 | 14.6 |
| Appro | ach | 1119 | 7.2 | 1119 | 7.2 | 0.953 | 33.1 | LOS C | 26.7 | 199.4 | 0.74 | 0.75 | 20.3 |
| East: | Anzac F | Road | | | | | | | | | | | |
| 4 | L2 | 208 | 3.0 | 208 | 3.0 | 0.520 | 47.0 | LOS D | 10.0 | 74.3 | 0.93 | 0.81 | 10.7 |
| 6 | R2 | 363 | 11.9 | 363 | 11.9 | 0.924 | 70.1 | LOS E | 24.0 | 207.6 | 1.00 | 1.02 | 7.7 |
| Appro | ach | 572 | 8.7 | 572 | 8.7 | 0.924 | 61.6 | LOS E | 24.0 | 207.6 | 0.98 | 0.95 | 8.6 |
| North: | Moorel | oank Avenue | е | | | | | | | | | | |
| 7 | L2 | 403 | 7.8 | 403 | 7.8 | 0.320 | 9.4 | LOSA | 6.7 | 54.5 | 0.40 | 0.63 | 32.3 |
| 8 | T1 | 517 | 14.7 | 517 | 14.7 | 0.946 | 61.6 | LOS E | 19.8 | 179.5 | 0.92 | 1.13 | 6.3 |
| Appro | ach | 920 | 11.7 | 920 | 11.7 | 0.946 | 38.7 | LOS C | 19.8 | 179.5 | 0.69 | 0.91 | 12.9 |
| All Ve | hicles | 2611 | 9.1 | 2611 | 9.1 | 0.953 | 41.3 | LOS C | 26.7 | 207.6 | 0.77 | 0.85 | 15.1 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 %

Number of Iterations: 16 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | |
|-----------|------------------------------------|-------------------------|------------------|---------------------|------------|----------|-----------------|------------------------|--|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay | Level of Service | | Distance | Prop. Queued | Effective Stop Rate | | |
| P1 | South Full Crossing | 11 | sec 39.3 | LOS D | ped 0.0 | 0.0 | 0.85 | per ped 0.85 | | |
| P2 | East Full Crossing | 11 | 27.7 | LOS C | 0.0 | 0.0 | 0.71 | 0.71 | | |
| All Pe | destrians | 21 | 33.5 | LOS D | | | 0.78 | 0.78 | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: C [Moorebank Avenue_Anzac Road_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 110 seconds (Practical Cycle Time)

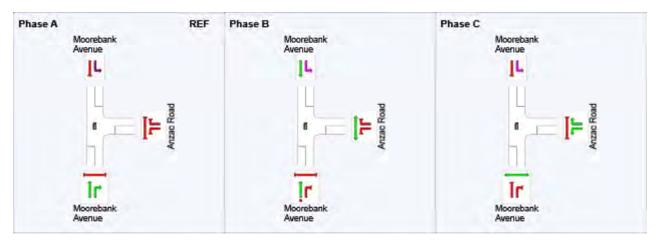
Phase Times determined by the program

Phase Sequence: 3 Phase Reference Phase: Phase A Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

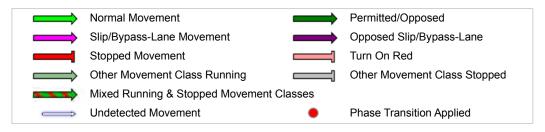
Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 0 | 30 | 79 |
| Green Time (sec) | 24 | 43 | 25 |
| Phase Time (sec) | 30 | 49 | 31 |
| Phase Split | 27% | 45% | 28% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase



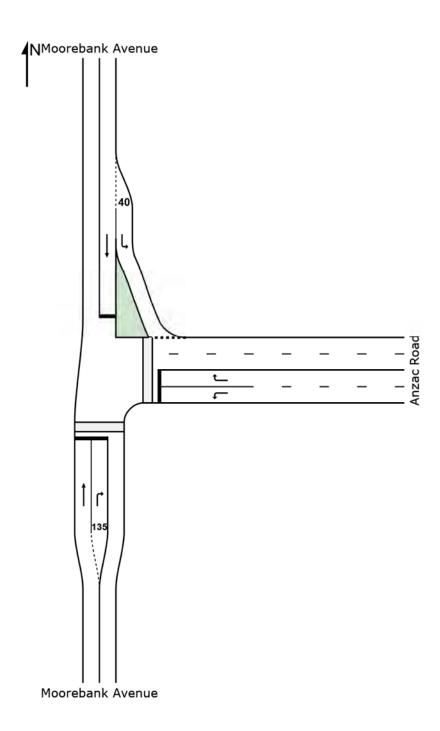
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Site: C [Moorebank Avenue_Anzac Road_PM]

Intersection of Moorebank Avenue and Anzac Road AM PEAK
Signals - Fixed Time Isolated



Site: C [Moorebank Avenue_Anzac Road_PM]

Intersection of Moorebank Avenue and Anzac Road AM PEAK

| Move | Movement Performance - Vehicles | | | | | | | | | | | | |
|-----------|---------------------------------|----------------------------|----------|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | HV | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South | : Moore | bank Avenue | 9 | | | | | | | | | | |
| 2 | T1 | 614 | 9.1 | 614 | 9.1 | 0.475 | 7.0 | LOSA | 12.8 | 105.7 | 0.49 | 0.44 | 32.1 |
| 3 | R2 | 188 | 0.6 | 188 | 0.6 | 0.871 | 57.8 | LOS E | 10.0 | 70.7 | 1.00 | 1.03 | 17.4 |
| Appro | ach | 802 | 7.1 | 802 | 7.1 | 0.871 | 18.9 | LOS B | 12.8 | 105.7 | 0.61 | 0.58 | 25.4 |
| East: | Anzac R | Road | | | | | | | | | | | |
| 4 | L2 | 280 | 1.5 | 280 | 1.5 | 0.895 | 60.0 | LOS E | 15.3 | 110.3 | 1.00 | 1.00 | 8.7 |
| 6 | R2 | 287 | 4.0 | 287 | 4.0 | 0.880 | 57.7 | LOS E | 15.4 | 116.2 | 1.00 | 0.98 | 9.1 |
| Appro | ach | 567 | 2.8 | 567 | 2.8 | 0.895 | 58.8 | LOS E | 15.4 | 116.2 | 1.00 | 0.99 | 8.9 |
| North: | Moorek | oank Avenue | : | | | | | | | | | | |
| 7 | L2 | 419 | 3.0 | 419 | 3.0 | 0.286 | 5.1 | LOSA | 4.0 | 29.5 | 0.28 | 0.51 | 37.6 |
| 8 | T1 | 697 | 7.6 | 697 | 7.6 | 0.893 | 33.6 | LOS C | 22.3 | 179.5 | 0.83 | 0.92 | 10.2 |
| Appro | ach | 1116 | 5.8 | 1116 | 5.8 | 0.893 | 22.9 | LOS B | 22.3 | 179.5 | 0.62 | 0.77 | 17.6 |
| All Ve | hicles | 2485 | 5.5 | 2485 | 5.5 | 0.895 | 29.8 | LOS C | 22.3 | 179.5 | 0.71 | 0.76 | 17.4 |

♦♦ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 %

Number of Iterations: 9 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | |
|-----------|------------------------------------|----------------|------------------|---------------------|----------------------------|----------------------|-----------------|------------------------|--|--|
| Mov ID | Description | Demand Flow | Average Delay | Level of Service | Average Back Pedestrian | of Queue Distance | Prop. Queued | Effective Stop Rate | | |
| | | ped/h | sec | OCIVICC | ped | m | Queucu | per ped | | |
| P1 | South Full Crossing | 11 | 38.9 | LOS D | 0.0 | 0.0 | 0.91 | 0.91 | | |
| P2 | East Full Crossing | 11 | 17.1 | LOS B | 0.0 | 0.0 | 0.60 | 0.60 | | |
| All Pe | destrians | 21 | 28.0 | LOS C | | | 0.75 | 0.75 | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: C [Moorebank Avenue_Anzac Road_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK Signals - Fixed Time Isolated Cycle Time = 95 seconds (Practical Cycle Time)

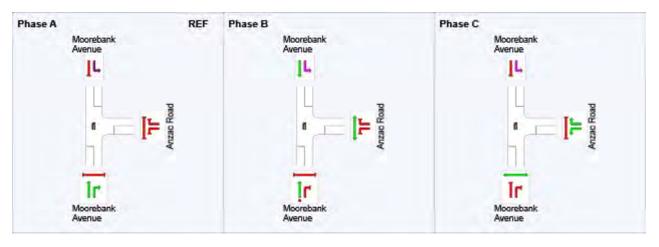
Phase Times determined by the program

Phase Sequence: 3 Phase Reference Phase: Phase A Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

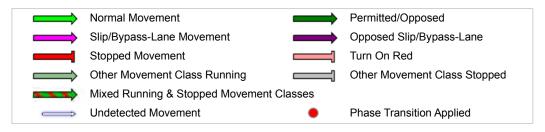
Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 0 | 17 | 72 |
| Green Time (sec) | 11 | 49 | 17 |
| Phase Time (sec) | 17 | 55 | 23 |
| Phase Split | 18% | 58% | 24% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase



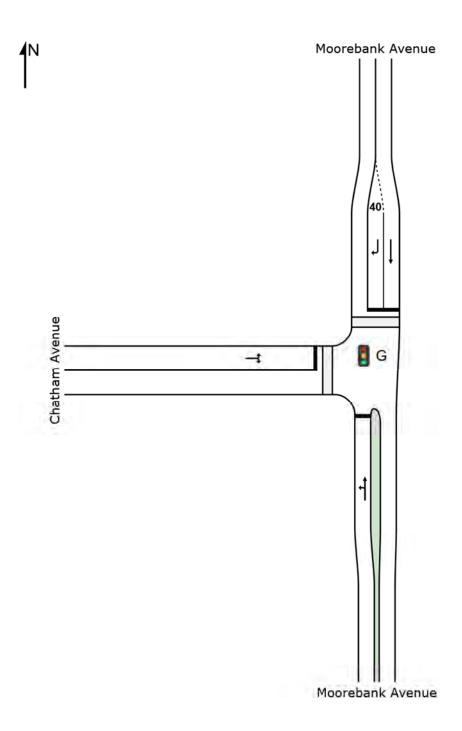
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Project: \\HC-AUS-NS-FS-01\jobs\AA008765\D-Calculations\Traffic\01 MPW Stage 2 Response\03 Anzac Rd Sensitivity Testing\SIDRA Model \Scenario 2\Scenario 2_Stage 1.sip7

Site: G [Moorebank Avenue/Chatham Avenue_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK
Signals - Fixed Time Isolated



Site: G [Moorebank Avenue/Chatham Avenue_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK

| Move | ement l | Performan | ice - Ve | hicles | | | | | | | | | |
|-----------|-------------------------|--------------------------|----------|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | HV | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South | South: Moorebank Avenue | | | | | | | | | | | | |
| 1 | L2 | 1 | 0.0 | 1 | 0.0 | 0.896 | 30.2 | LOS C | 48.0 | 361.3 | 0.91 | 0.99 | 35.2 |
| 2 | T1 | 1103 | 3.7 | 1103 | 3.7 | 0.896 | 27.0 | LOS B | 48.0 | 361.3 | 0.91 | 0.99 | 32.0 |
| Appro | oach | 1104 | 3.7 | 1104 | 3.7 | 0.896 | 27.0 | LOS B | 48.0 | 361.3 | 0.91 | 0.99 | 32.0 |
| North | : Moorel | bank Avenu | e | | | | | | | | | | |
| 8 | T1 | 457 | 9.2 | 457 | 9.2 | 0.315 | 2.7 | LOSA | 5.3 | 43.6 | 0.30 | 0.27 | 45.7 |
| 9 | R2 | 40 | 100.0 | 40 | 100. 0 | 0.523 | 50.3 | LOS D | 1.8 | 38.1 | 1.00 | 0.78 | 23.7 |
| Appro | ach | 497 | 16.5 | 497 | 16.5 | 0.523 | 6.5 | LOSA | 5.3 | 43.6 | 0.36 | 0.31 | 43.3 |
| West | : Chatha | ım Avenue | | | | | | | | | | | |
| 10 | L2 | 40 | 100.0 | 40 | 100. 0 | 0.488 | 50.8 | LOS D | 1.8 | 37.9 | 1.00 | 0.76 | 11.8 |
| 12 | R2 | 1 | 0.0 | 1 | 0.0 | 0.488 | 50.4 | LOS D | 1.8 | 37.9 | 1.00 | 0.76 | 26.3 |
| Appro | oach | 41 | 97.4 | 41 | 97.4 | 0.488 | 50.8 | LOS D | 1.8 | 37.9 | 1.00 | 0.76 | 12.3 |
| All Ve | hicles | 1642 | 9.9 | 1642 | 9.9 | 0.896 | 21.4 | LOS B | 48.0 | 361.3 | 0.75 | 0.78 | 36.0 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations; 0.9 %

Number of Iterations: 16 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | | |
|-----------|------------------------------------|----------------|------------------|-------|----------------------------|----------------------|-----------------|------------------------|--|--|--|
| Mov ID | Description | Demand Flow | Average Delay | | Average Back Pedestrian | of Queue Distance | Prop. Queued | Effective Stop Rate | | | |
| | | ped/h | sec | | ped | m | | per ped | | | |
| P3 | North Full Crossing | 11 | 36.7 | LOS D | 0.0 | 0.0 | 0.93 | 0.93 | | | |
| P4 | West Full Crossing | 11 | 8.1 | LOS A | 0.0 | 0.0 | 0.44 | 0.44 | | | |
| All Pe | destrians | 21 | 22.4 | LOS C | | | 0.68 | 0.68 | | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: G [Moorebank Avenue/Chatham Avenue_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK

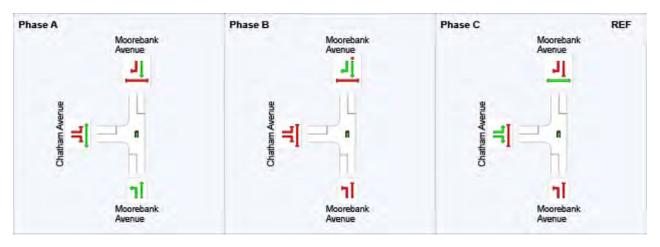
Signals - Fixed Time Isolated Cycle Time = 85 seconds (Practical Cycle Time)

Phase Times determined by the program Green Split Priority applies **Phase Sequence: Opposed Turns** Reference Phase: Phase C Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 12 | 73 | 0 |
| Green Time (sec) | 55 | 6 | 6 |
| Phase Time (sec) | 61 | 12 | 12 |
| Phase Split | 72% | 14% | 14% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

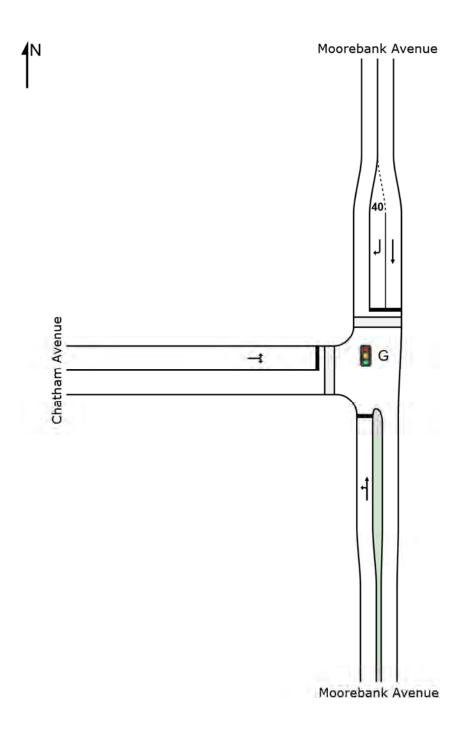


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Site: G [Moorebank Avenue/Chatham Avenue_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK
Signals - Fixed Time Isolated



Site: G [Moorebank Avenue/Chatham Avenue_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK

Signals - Fixed Time Isolated Cycle Time = 45 seconds (Practical Cycle Time)

| Move | ement F | Performar | ice - Ve | hicles | | | | | | | | | |
|-----------|-------------------------|--------------------------|------------------|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South | South: Moorebank Avenue | | | | | | | | | | | | |
| 1 | L2 | 1 | 0.0 | 1 | 0.0 | 0.784 | 23.9 | LOS B | 11.6 | 85.1 | 0.96 | 0.96 | 38.6 |
| 2 | T1 | 501 | 2.3 | 501 | 2.3 | 0.784 | 20.7 | LOS B | 11.6 | 85.1 | 0.96 | 0.96 | 35.9 |
| Appro | ach | 502 | 2.3 | 502 | 2.3 | 0.784 | 20.7 | LOS B | 11.6 | 85.1 | 0.96 | 0.96 | 35.9 |
| North | : Moorel | oank Avenu | ie | | | | | | | | | | |
| 8 | T1 | 955 | 1.2 | 955 | 1.2 | 0.859 | 15.8 | LOS B | 22.7 | 163.1 | 0.85 | 0.98 | 39.7 |
| 9 | R2 | 40 | 100.0 | 40 | 100. 0 | 0.277 | 25.1 | LOS B | 0.9 | 18.7 | 0.93 | 0.73 | 30.0 |
| Appro | ach | 995 | 5.2 | 995 | 5.2 | 0.859 | 16.2 | LOS B | 22.7 | 163.1 | 0.86 | 0.97 | 39.3 |
| West: | Chatha | m Avenue | | | | | | | | | | | |
| 10 | L2 | 105 | 38.0 | 105 | 38.0 | 0.500 | 26.4 | LOS B | 2.4 | 29.4 | 0.97 | 0.78 | 18.7 |
| 12 | R2 | 1 | 0.0 | 1 | 0.0 | 0.500 | 26.1 | LOS B | 2.4 | 29.4 | 0.97 | 0.78 | 35.0 |
| Appro | ach | 106 | 37.6 | 106 | 37.6 | 0.500 | 26.4 | LOS B | 2.4 | 29.4 | 0.97 | 0.78 | 18.9 |
| All Ve | hicles | 1603 | 6.4 | 1603 | 6.4 | 0.859 | 18.3 | LOS B | 22.7 | 163.1 | 0.90 | 0.95 | 37.9 |

♦♦ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 %

Number of Iterations: 9 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | | |
|-----------|------------------------------------|-------------------------|-------------------------|-------|-----------------------------------|---------------------------|-----------------|-----------------------------------|--|--|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | | Average Back Pedestrian ped | of Queue Distance m | Prop. Queued | Effective Stop Rate per ped | | | |
| P3 | North Full Crossing | 11 | 16.9 | LOS B | 0.0 | 0.0 | 0.87 | 0.87 | | | |
| P4 | West Full Crossing | 11 | 15.2 | LOS B | 0.0 | 0.0 | 0.82 | 0.82 | | | |
| All Pe | destrians | 21 | 16.1 | LOS B | | | 0.84 | 0.84 | | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: G [Moorebank Avenue/Chatham Avenue_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK

Signals - Fixed Time Isolated Cycle Time = 45 seconds (Practical Cycle Time)

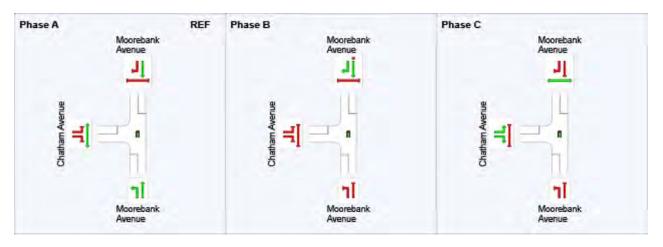
Phase Times determined by the program Green Split Priority applies **Phase Sequence: Opposed Turns** Reference Phase: Phase A Input Phase Sequence: A, B, C

Output Phase Sequence: A, B, C

Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 0 | 21 | 33 |
| Green Time (sec) | 15 | 6 | 6 |
| Phase Time (sec) | 21 | 12 | 12 |
| Phase Split | 47% | 27% | 27% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase



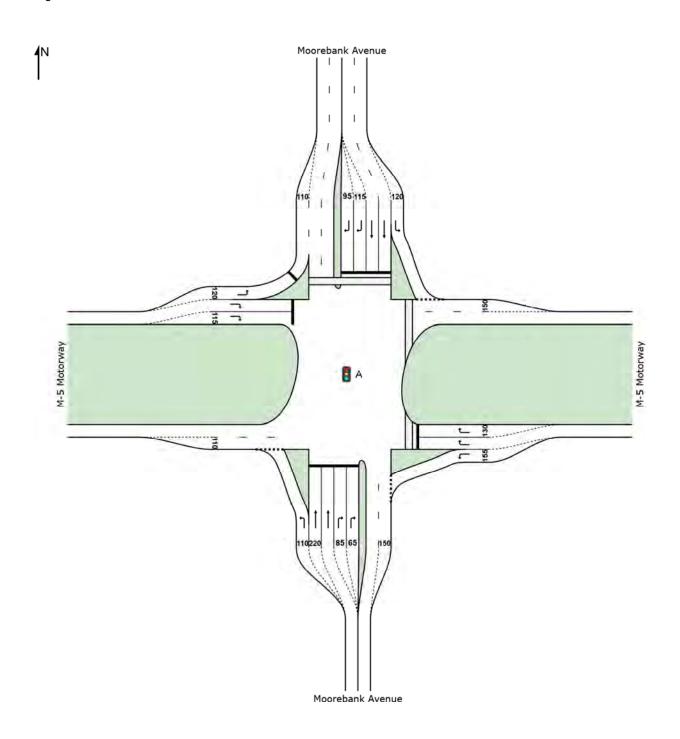
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Stage 2(i)

Site: A [M5/Moorebank Avenue_AM]

Intersection of Moorebank Avenue and M5 Motorway AM PEAK
Signals - Fixed Time Isolated



Site: A [M5/Moorebank Avenue_AM]

Intersection of Moorebank Avenue and M5 Motorway

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|-----------|-----------------|---------|------------------|---------|--------------|------------------|---------------------|----------------------|----------------------|--------|---------------------|---------|
| | | | | | | | | | | | | | |
| Mov ID | OD Mov | Demand Total | Flows | Arrival Total | Flows | Deg. Satn | Average Delav | Level of Service | 95% Back Vehicles | of Queue Distance | Prop. | Effective Stop Rate | |
| טו | IVIOV | veh/h | пv % | veh/h | пv % | v/c | sec | Service | venicies | Distance | Queued | per veh | km/h |
| South | : Moorel | oank Avenu | | VO11/11 | /0 | V/ O | 300 | | VOII | | | per veri | KIII/II |
| 1 | L2 | 428 | 14.7 | 428 | 14.7 | 0.396 | 14.4 | LOSA | 9.9 | 89.8 | 0.42 | 0.73 | 50.3 |
| 2 | T1 | 402 | 3.4 | 402 | 3.4 | 0.252 | 29.2 | LOS C | 9.3 | 69.3 | 0.68 | 0.58 | 34.6 |
| 3 | R2 | 271 | 20.2 | 271 | 20.2 | 0.441 | 57.9 | LOS E | 9.3 | 91.2 | 0.89 | 0.80 | 26.2 |
| Appro | ach | 1101 | 12.0 | 1101 | 12.0 | 0.441 | 30.5 | LOS C | 9.9 | 91.2 | 0.63 | 0.69 | 36.9 |
| East: | M-5 Mot | orway | | | | | | | | | | | |
| 4 | L2 | 339 | 17.7 | 339 | 17.7 | 0.285 | 6.4 | LOSA | 2.0 | 18.9 | 0.14 | 0.59 | 47.5 |
| 6 | R2 | 243 | 4.3 | 243 | 4.3 | 0.949 | 104.0 | LOS F | 10.7 | 81.6 | 1.00 | 1.05 | 17.1 |
| Appro | ach | 582 | 12.1 | 582 | 12.1 | 0.949 | 47.1 | LOS D | 10.7 | 81.6 | 0.50 | 0.78 | 23.9 |
| North | : Mooreb | ank Avenu | е | | | | | | | | | | |
| 7 | L2 | 48 | 19.6 | 48 | 19.6 | 0.042 | 7.3 | LOSA | 0.5 | 4.7 | 0.18 | 0.58 | 52.8 |
| 8 | T1 | 218 | 6.8 | 218 | 6.8 | 0.156 | 27.7 | LOS B | 5.2 | 41.5 | 0.65 | 0.53 | 24.6 |
| 9 | R2 | 506 | 20.2 | 506 | 20.2 | 0.967 | 87.5 | LOS F | 28.7 | 282.0 | 0.98 | 0.98 | 22.1 |
| Appro | ach | 773 | 16.3 | 773 | 16.3 | 0.967 | 65.6 | LOS E | 28.7 | 282.0 | 0.83 | 0.82 | 23.3 |
| West: | M-5 Mo | torway | | | | | | | | | | | |
| 10 | L2 | 1356 | 7.6 | 1356 | 7.6 | 0.887 | 7.1 | LOSA | 21.5 | 173.2 | 0.48 | 0.66 | 50.5 |
| 12 | R2 | 521 | 9.7 | 521 | 9.7 | 0.812 | 68.9 | LOS E | 20.8 | 173.8 | 0.99 | 0.88 | 18.9 |
| Appro | ach | 1877 | 8.2 | 1877 | 8.2 | 0.887 | 24.3 | LOS B | 21.5 | 173.8 | 0.62 | 0.72 | 38.0 |
| All Ve | hicles | 4333 | 11.1 | 4333 | 11.1 | 0.967 | 36.3 | LOS C | 28.7 | 282.0 | 0.64 | 0.74 | 32.4 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 1.0 %

Number of Iterations: 13 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | | |
|-----------|------------------------------------|----------------|------------------|---------------------|----------------------------|----------------------|-----------------|------------------------|--|--|--|
| Mov ID | Description | Demand Flow | Average Delay | Level of Service | Average Back Pedestrian | of Queue Distance | Prop. Queued | Effective Stop Rate | | | |
| טו | · · · · · · · · · · · · · · · | ped/h | sec | Service | ped | m | Queueu | per ped | | | |
| P21 | East Stage 1 | 26 | 64.5 | LOS F | 0.1 | 0.1 | 0.93 | 0.93 | | | |
| P22 | East Stage 2 | 26 | 68.2 | LOS F | 0.1 | 0.1 | 0.95 | 0.95 | | | |
| P3 | North Full Crossing | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 | | | |
| All Pe | destrians | 79 | 67.3 | LOS F | | | 0.95 | 0.95 | | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: A [M5/Moorebank Avenue_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and M5 Motorway AM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

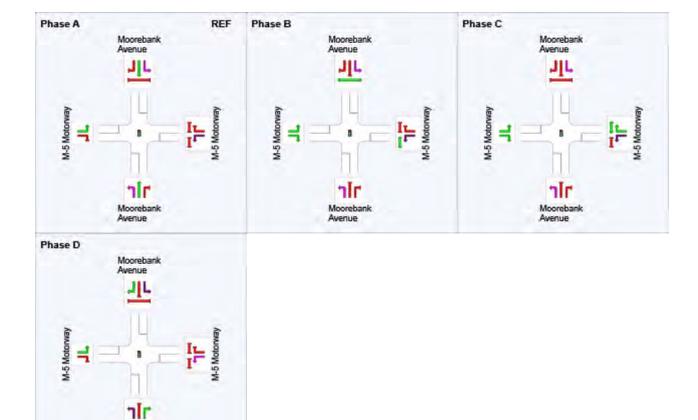
Phase Times determined by the program

Phase Sequence: 4-phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|-----|-----|
| Phase Change Time (sec) | 0 | 70 | 91 | 108 |
| Green Time (sec) | 64 | 15 | 11 | 36 |
| Phase Time (sec) | 70 | 21 | 17 | 42 |
| Phase Split | 47% | 14% | 11% | 28% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

Moorebank Avenue

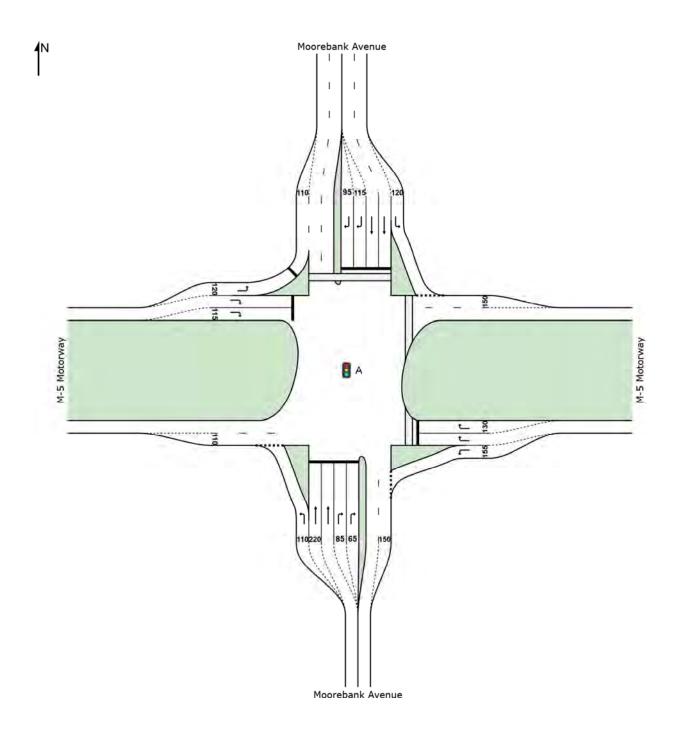




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Site: A [M5/Moorebank Avenue_PM]

Intersection of Moorebank Avenue and M5 Motorway PM PEAK
Signals - Fixed Time Isolated



Site: A [M5/Moorebank Avenue_PM]

Intersection of Moorebank Avenue and M5 Motorway

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|----------|----------------|---------|----------------|---------|-------------|--------------|----------|-----------------|---------------|--------|-------------------|---------------|
| | | | | | | | | | | | | | |
| Mov | OD | Demand | | Arrival | | Deg. | Average | Level of | 95% Back | | Prop. | Effective | |
| ID | Mov | Total veh/h | HV % | Total veh/h | HV % | Satn v/c | Delay sec | Service | Vehicles veh | Distance m | Queued | Stop Rate per veh | Speed km/h |
| South | : Moorel | bank Avenu | | VCII/II | /0 | V/C | 300 | | VEII | - ''' | | per veri | KIII/II |
| 1 | L2 | 541 | 7.4 | 541 | 7.4 | 0.761 | 42.8 | LOS D | 26.3 | 211.1 | 0.92 | 1.05 | 35.1 |
| 2 | T1 | 286 | 2.6 | 286 | 2.6 | 0.567 | 66.8 | LOSE | 10.0 | 73.6 | 0.99 | 0.80 | |
| 3 | R2 | 404 | 8.9 | 404 | 8.9 | 0.239 | 22.6 | LOS B | 7.9 | 65.3 | 0.53 | 0.72 | 41.6 |
| | | | | | | | | | | | | | |
| Appro | ach | 1232 | 6.8 | 1232 | 6.8 | 0.761 | 41.7 | LOS C | 26.3 | 211.1 | 0.81 | 0.88 | 33.0 |
| East: | M-5 Mot | orway | | | | | | | | | | | |
| 4 | L2 | 281 | 12.7 | 281 | 12.7 | 0.239 | 7.1 | LOSA | 2.9 | 25.6 | 0.20 | 0.61 | 46.3 |
| 6 | R2 | 87 | 6.0 | 87 | 6.0 | 0.642 | 89.0 | LOS F | 3.4 | 26.9 | 1.00 | 0.78 | 19.0 |
| Appro | ach | 368 | 11.1 | 368 | 11.1 | 0.642 | 26.5 | LOS B | 3.4 | 26.9 | 0.39 | 0.65 | 30.7 |
| | | | | | | | | | | | | | |
| | | ank Avenue | | | | | | | | | | | |
| 7 | L2 | 74 | 5.7 | 74 | 5.7 | 0.065 | 6.8 | LOSA | 0.6 | 4.8 | 0.16 | 0.59 | 55.9 |
| 8 | T1 | 405 | 1.8 | 405 | 1.8 | 0.864 | 74.2 | LOS F | 17.4 | 126.4 | 1.00 | 0.92 | 12.4 |
| 9 | R2 | 1296 | 4.5 | 1296 | 4.5 | 0.884 | 35.2 | LOS C | 46.1 | 352.4 | 0.76 | 0.85 | 38.0 |
| Appro | ach | 1775 | 4.0 | 1775 | 4.0 | 0.884 | 42.9 | LOS D | 46.1 | 352.4 | 0.79 | 0.85 | 31.6 |
| West: | M-5 Mo | torway | | | | | | | | | | | |
| 10 | L2 | 595 | 7.3 | 595 | 7.3 | 0.387 | 6.1 | LOSA | 2.8 | 22.5 | 0.13 | 0.56 | 52.0 |
| 12 | R2 | 439 | 9.6 | 439 | 9.6 | 0.810 | 72.7 | LOS F | 17.7 | 148.0 | 1.00 | 0.88 | 18.2 |
| Appro | ach | 1034 | 8.2 | 1034 | 8.2 | 0.810 | 34.4 | LOS C | 17.7 | 148.0 | 0.50 | 0.70 | 32.7 |
| All Ve | hicles | 4408 | 6.4 | 4408 | 6.4 | 0.884 | 39.2 | LOS C | 46.1 | 352.4 | 0.69 | 0.81 | 32.2 |

+ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.1 %

Number of Iterations: 9 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | | |
|-----------|---|----------------|------------------|---------------------|----------------------------|----------------------|-----------------|------------------------|--|--|--|
| Mov ID | Description | Demand Flow | Average Delay | Level of Service | Average Back Pedestrian | of Queue Distance | Prop. Queued | Effective Stop Rate | | | |
| טו | 2 3 3 3 7 1 3 1 3 1 3 1 3 1 3 1 3 1 3 1 3 | ped/h | sec | Service | ped | Distance m | Queueu | per ped | | | |
| P21 | East Stage 1 | 26 | 64.5 | LOS F | 0.1 | 0.1 | 0.93 | 0.93 | | | |
| P22 | East Stage 2 | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 | | | |
| P3 | North Full Crossing | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 | | | |
| All Pe | destrians | 79 | 67.6 | LOS F | | | 0.95 | 0.95 | | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: A [M5/Moorebank Avenue_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and M5 Motorway

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

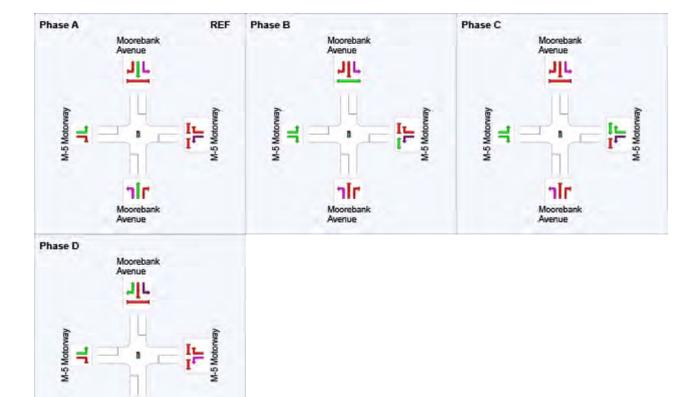
Phase Times determined by the program

Phase Sequence: 4-phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|----|-----|
| Phase Change Time (sec) | 0 | 26 | 47 | 59 |
| Green Time (sec) | 20 | 15 | 6 | 85 |
| Phase Time (sec) | 26 | 21 | 12 | 91 |
| Phase Split | 17% | 14% | 8% | 61% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

Moorebank Avenue

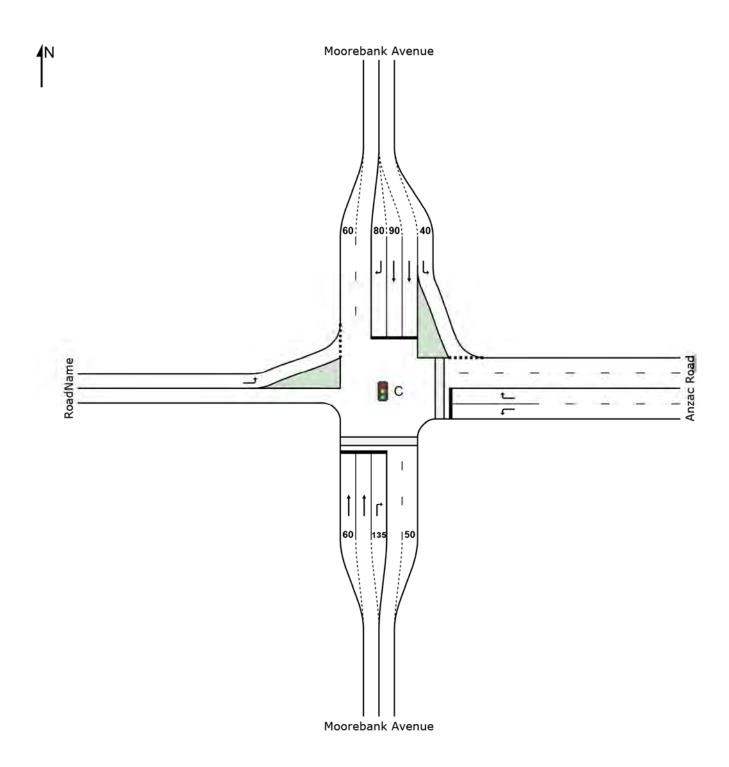




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Site: C [Moorebank Avenue_Anzac Road_AM]

Intersection of Moorebank Avenue and Anzac Road AM PEAK Signals - Fixed Time Isolated



Site: C [Moorebank Avenue_Anzac Road_AM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|-----------|------------|-------|--------|-----------|-------|---------|----------|----------|----------|--------|-----------|---------|
| Mov | OD | Demand | Flows | Arriva | l Flows | Deg. | Average | Level of | 95% Back | of Queue | Prop. | Effective | Average |
| ID | Mov | Total | HV | Total | HV | Satn | Delay | Service | Vehicles | Distance | Queued | Stop Rate | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | per veh | km/h |
| South: Moorebank Avenue | | | | | | | | | | | | | |
| 2 | T1 | 727 | 8.0 | 727 | 8.0 | 0.897 | 41.6 | LOS C | 28.9 | 234.6 | 0.88 | 0.90 | 16.3 |
| 3 | R2 | 381 | 3.3 | 381 | 3.3 | 0.572 | 33.7 | LOS C | 16.4 | 122.6 | 0.86 | 0.81 | 23.2 |
| Appr | oach | 1108 | 6.4 | 1108 | 6.4 | 0.897 | 38.8 | LOS C | 28.9 | 234.6 | 0.87 | 0.87 | 18.7 |
| East | : Anzac I | Road | | | | | | | | | | | |
| 4 | L2 | 208 | 3.0 | 208 | 3.0 | 0.499 | 46.0 | LOS D | 9.9 | 73.3 | 0.92 | 0.81 | 10.9 |
| 6 | R2 | 363 | 11.9 | 363 | 11.9 | 0.888 | 62.4 | LOS E | 22.3 | 193.6 | 1.00 | 0.99 | 8.8 |
| Appr | oach | 572 | 8.7 | 572 | 8.7 | 0.888 | 56.4 | LOS D | 22.3 | 193.6 | 0.97 | 0.93 | 9.4 |
| North | n: Moore | bank Avenu | ıe | | | | | | | | | | |
| 7 | L2 | 403 | 7.8 | 403 | 7.8 | 0.340 | 7.8 | LOSA | 7.3 | 59.4 | 0.42 | 0.60 | 33.8 |
| 8 | T1 | 506 | 12.9 | 506 | 12.9 | 0.906 | 53.7 | LOS D | 20.4 | 179.5 | 0.97 | 1.01 | 7.0 |
| 9 | R2 | 29 | 100.0 | 29 | 100. 0 | 0.114 | 42.1 | LOS C | 1.3 | 16.5 | 0.82 | 0.72 | 26.8 |
| Appr | oach | 939 | 13.5 | 939 | 13.5 | 0.906 | 33.6 | LOS C | 20.4 | 179.5 | 0.73 | 0.82 | 15.0 |
| West | t: RoadN | lame | | | | | | | | | | | |
| 10 | L2 | 29 | 100.0 | 29 | 100. 0 | 0.067 | 15.3 | LOS B | 0.5 | 6.0 | 0.57 | 0.65 | 40.8 |
| Appr | oach | 29 | 100.0 | 29 | 100. 0 | 0.067 | 15.3 | LOS B | 0.5 | 6.0 | 0.57 | 0.65 | 40.8 |
| All V | ehicles | 2648 | 10.4 | 2648 | 10.4 | 0.906 | 40.5 | LOS C | 28.9 | 234.6 | 0.84 | 0.86 | 15.7 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 1.0 %

Number of Iterations: 13 (maximum specified: 20)

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------------|-------------------------|-------|-----------------------------------|---------------------------|-----------------|-----------------------------------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | | Average Back Pedestrian ped | of Queue Distance m | Prop. Queued | Effective Stop Rate per ped | |
| P1 | South Full Crossing | 11 | 43.7 | LOS E | 0.0 | 0.1 | 0.89 | 0.89 | |
| P2 | East Full Crossing | 11 | 41.0 | LOS E | 0.0 | 0.1 | 0.86 | 0.86 | |
| All Pe | destrians | 21 | 42.4 | LOS E | | | 0.88 | 0.88 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: C [Moorebank Avenue_Anzac Road_AM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 110 seconds (Practical Cycle Time)

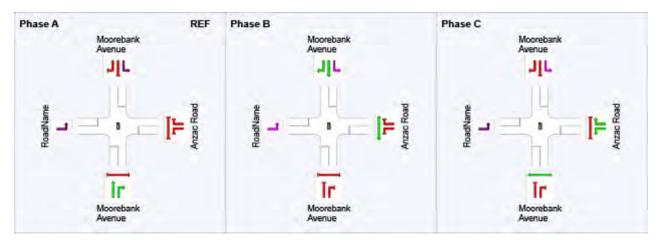
Phase Times determined by the program

Phase Sequence: 4 Phase Reference Phase: Phase A Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

Phase Timing Results

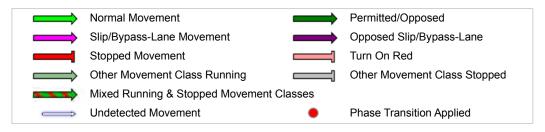
| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 0 | 46 | 78 |
| Green Time (sec) | 40 | 26 | 26 |
| Phase Time (sec) | 46 | 32 | 32 |
| Phase Split | 42% | 29% | 29% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



♦♦ Network: 1 [Scenario 2_AM]

REF: Reference Phase VAR: Variable Phase

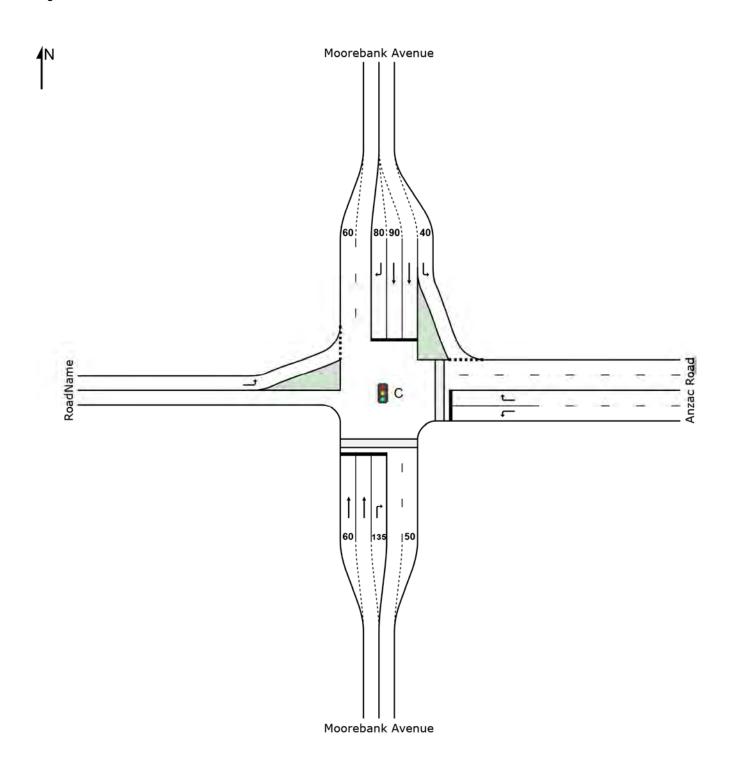


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Site: C [Moorebank Avenue_Anzac Road_PM]

Intersection of Moorebank Avenue and Anzac Road AM PEAK Signals - Fixed Time Isolated



Site: C [Moorebank Avenue_Anzac Road_PM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 55 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|---------|----------------------|-------|-------|-------|-------|---------|----------|----------|----------|--------|-----------|------|
| Mov | OD | Demand | | | Flows | Deg. | Average | Level of | 95% Back | | Prop. | Effective | |
| ID | Mov | Total | HV | Total | HV | Satn | Delay | Service | Vehicles | Distance | Queued | Stop Rate | |
| Courth | · Moore | veh/h ebank Aveni | % | veh/h | % | v/c | sec | | veh | m | | per veh | km/h |
| | | | - | | | | | | | | | | |
| 2 | T1 | 803 | 5.6 | 803 | 5.6 | 0.862 | 22.5 | LOS B | 18.2 | 141.4 | 0.92 | 0.98 | 22.4 |
| 3 | R2 | 211 | 0.5 | 211 | 0.5 | 0.885 | 38.3 | LOS C | 7.0 | 49.3 | 1.00 | 1.16 | 21.8 |
| Appro | ach | 1014 | 4.6 | 1014 | 4.6 | 0.885 | 25.8 | LOS B | 18.2 | 141.4 | 0.94 | 1.02 | 22.2 |
| Coot | Anzac I | Dood | | | | | | | | | | | |
| | | | 4 - | 000 | 4.5 | 0.000 | 05.4 | 1000 | 0.0 | 00.0 | 4.00 | 0.00 | 40.0 |
| 4 | L2 | 280 | 1.5 | 280 | 1.5 | 0.829 | 35.1 | LOS C | 8.6 | 62.2 | 1.00 | 0.98 | 13.6 |
| 6 | R2 | 287 | 4.0 | 287 | 4.0 | 0.866 | 37.8 | LOS C | 9.3 | 70.5 | 1.00 | 1.06 | 13.2 |
| Appro | ach | 567 | 2.8 | 567 | 2.8 | 0.866 | 36.5 | LOS C | 9.3 | 70.5 | 1.00 | 1.02 | 13.4 |
| North | · Moore | ebank Avenu | ۵۱ | | | | | | | | | | |
| 7 | L2 | 419 | 3.0 | 419 | 3.0 | 0.349 | 5.6 | LOSA | 3.4 | 25.1 | 0.46 | 0.61 | 36.8 |
| | | | | | | | | | | | | | |
| 8 | T1 | 686 | 6.1 | 686 | 6.1 | 0.749 | 17.6 | LOS B | 13.3 | 104.9 | 0.89 | 0.82 | 15.8 |
| 9 | R2 | 29 | 100.0 | 29 | 100. | 0.211 | 31.5 | LOS C | 8.0 | 10.2 | 0.93 | 0.73 | 30.6 |
| | | | | | 0 | | | | | | | | |
| Appro | ach | 1135 | 7.4 | 1135 | 7.4 | 0.749 | 13.5 | LOSA | 13.3 | 104.9 | 0.73 | 0.74 | 24.1 |
| West | RoadN | Jame | | | | | | | | | | | |
| 10 | L2 | 29 | 100.0 | 29 | 100. | 0.066 | 12.9 | LOSA | 0.3 | 4.2 | 0.67 | 0.66 | 43.0 |
| 10 | LZ | 29 | 100.0 | 29 | 0 | 0.000 | 12.9 | LUSA | 0.3 | 4.2 | 0.07 | 0.00 | 43.0 |
| Appro | ach | 29 | 100.0 | 29 | 100. | 0.066 | 12.9 | LOSA | 0.3 | 4.2 | 0.67 | 0.66 | 43.0 |
| Appro | Jacii | 29 | 100.0 | 29 | 0 | 0.000 | 12.5 | LOSA | 0.5 | 4.2 | 0.07 | 0.00 | 45.0 |
| | | | | | 3 | | | | | | | | |
| ΔΙΙ \/e | hicles | 2745 | 6.4 | 2745 | 6.4 | 0.885 | 22.8 | LOS B | 18.2 | 141.4 | 0.86 | 0.90 | 21.0 |
| , ui VC | | 2140 | 0.4 | 2,40 | 5.4 | 0.000 | 22.0 | L00 B | 10.2 | 171.7 | 0.00 | 0.50 | 21.0 |

♦♦ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.1 %

Number of Iterations: 9 (maximum specified: 20)

| Movement Performance - Pedestrians | | | | | | | | | | |
|------------------------------------|---------------------|-------------------------|-------------------------|-------|-----------------------------------|---------------------------|-----------------|-----------------------------------|--|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | | Average Back Pedestrian ped | of Queue Distance m | Prop. Queued | Effective Stop Rate per ped | | |
| P1 | South Full Crossing | 11 | 21.8 | LOS C | 0.0 | 0.0 | 0.89 | 0.89 | | |
| P2 | East Full Crossing | 11 | 19.2 | LOS B | 0.0 | 0.0 | 0.84 | 0.84 | | |
| All Pe | destrians | 21 | 20.5 | LOS C | | | 0.86 | 0.86 | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: C [Moorebank Avenue_Anzac Road_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 55 seconds (Practical Cycle Time)

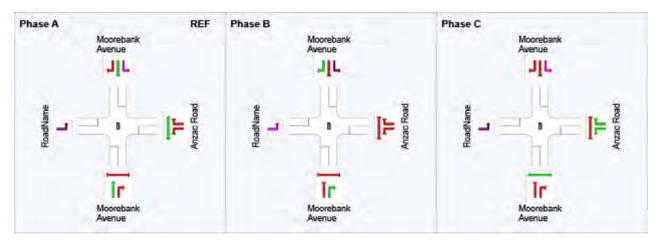
Phase Times determined by the program

Phase Sequence: 4 Phase Reference Phase: Phase A Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

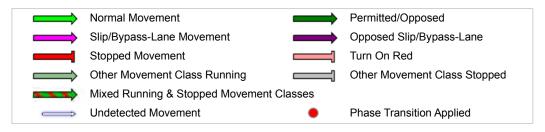
Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 0 | 26 | 39 |
| Green Time (sec) | 20 | 7 | 10 |
| Phase Time (sec) | 26 | 13 | 16 |
| Phase Split | 47% | 24% | 29% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase



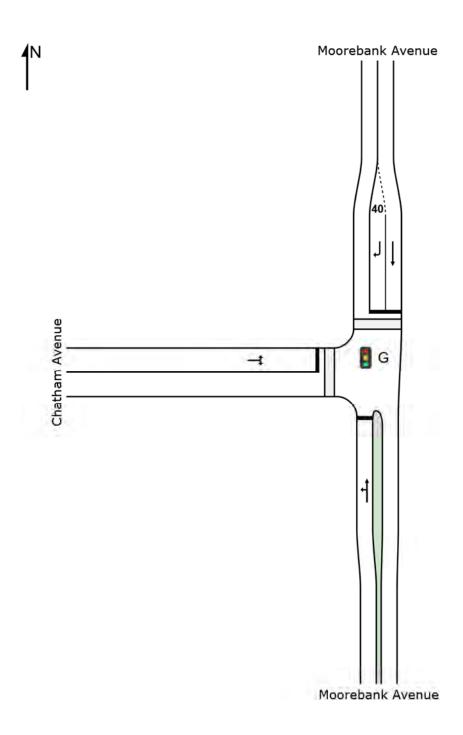
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\Scenario 2\Scenario 2_Stage 2_50%.sip7

Site: G [Moorebank Avenue/Chatham Avenue_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK
Signals - Fixed Time Isolated



Site: G [Moorebank Avenue/Chatham Avenue_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK

| Move | ement I | Performar | ice - Ve | hicles | | | | | | | | | |
|-----------|-------------------------|--------------------------|----------|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | HV | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South | South: Moorebank Avenue | | | | | | | | | | | | |
| 1 | L2 | 1 | 0.0 | 1 | 0.0 | 0.896 | 30.2 | LOS C | 48.0 | 361.3 | 0.91 | 0.99 | 35.2 |
| 2 | T1 | 1103 | 3.7 | 1103 | 3.7 | 0.896 | 27.0 | LOS B | 48.0 | 361.3 | 0.91 | 0.99 | 32.0 |
| Appro | oach | 1104 | 3.7 | 1104 | 3.7 | 0.896 | 27.0 | LOS B | 48.0 | 361.3 | 0.91 | 0.99 | 32.0 |
| North | : Moore | bank Avenu | e | | | | | | | | | | |
| 8 | T1 | 457 | 9.2 | 457 | 9.2 | 0.315 | 2.7 | LOSA | 5.3 | 43.6 | 0.30 | 0.27 | 45.7 |
| 9 | R2 | 29 | 100.0 | 29 | 100. 0 | 0.385 | 49.3 | LOS D | 1.3 | 27.5 | 0.99 | 0.73 | 23.9 |
| Appro | oach | 486 | 14.7 | 486 | 14.7 | 0.385 | 5.5 | LOSA | 5.3 | 43.6 | 0.34 | 0.30 | 43.9 |
| West | : Chatha | m Avenue | | | | | | | | | | | |
| 10 | L2 | 29 | 100.0 | 29 | 100. 0 | 0.361 | 50.1 | LOS D | 1.3 | 27.5 | 0.99 | 0.73 | 11.9 |
| 12 | R2 | 1 | 0.0 | 1 | 0.0 | 0.361 | 49.6 | LOS D | 1.3 | 27.5 | 0.99 | 0.73 | 26.5 |
| Appro | oach | 31 | 96.6 | 31 | 96.6 | 0.361 | 50.1 | LOS D | 1.3 | 27.5 | 0.99 | 0.73 | 12.7 |
| All Ve | hicles | 1621 | 8.8 | 1621 | 8.8 | 0.896 | 21.0 | LOS B | 48.0 | 361.3 | 0.74 | 0.78 | 36.3 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 1.0 %

Number of Iterations: 13 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | | |
|-----------|------------------------------------|----------------|------------------|-------|----------------------------|----------------------|-----------------|------------------------|--|--|--|
| Mov ID | Description | Demand Flow | Average Delay | | Average Back Pedestrian | of Queue Distance | Prop. Queued | Effective Stop Rate | | | |
| | | ped/h | sec | | ped | m | | per ped | | | |
| P3 | North Full Crossing | 11 | 36.7 | LOS D | 0.0 | 0.0 | 0.93 | 0.93 | | | |
| P4 | West Full Crossing | 11 | 8.1 | LOS A | 0.0 | 0.0 | 0.44 | 0.44 | | | |
| All Pe | destrians | 21 | 22.4 | LOS C | | | 0.68 | 0.68 | | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: G [Moorebank Avenue/Chatham Avenue_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK

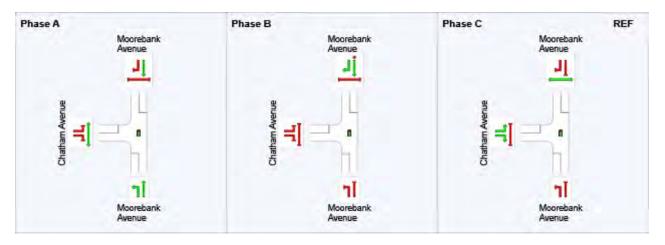
Signals - Fixed Time Isolated Cycle Time = 85 seconds (Practical Cycle Time)

Phase Times determined by the program Green Split Priority applies **Phase Sequence: Opposed Turns** Reference Phase: Phase C Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

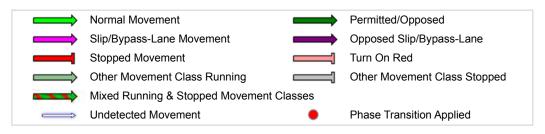
Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 12 | 73 | 0 |
| Green Time (sec) | 55 | 6 | 6 |
| Phase Time (sec) | 61 | 12 | 12 |
| Phase Split | 72% | 14% | 14% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



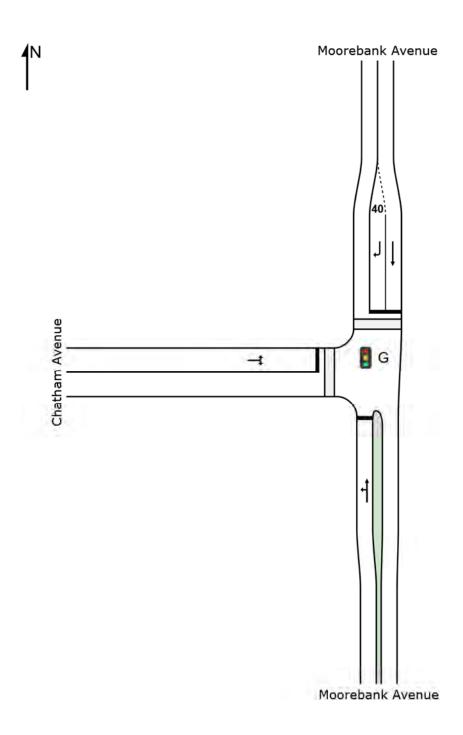
REF: Reference Phase VAR: Variable Phase



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Site: G [Moorebank Avenue/Chatham Avenue_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK
Signals - Fixed Time Isolated



Site: G [Moorebank Avenue/Chatham Avenue_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK

Signals - Fixed Time Isolated Cycle Time = 50 seconds (Practical Cycle Time)

| Move | Movement Performance - Vehicles | | | | | | | | | | | | |
|-----------|---------------------------------|--------------------------|------------------|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South | South: Moorebank Avenue | | | | | | | | | | | | |
| 1 | L2 | 1 | 0.0 | 1 | 0.0 | 0.817 | 27.6 | LOS B | 13.5 | 98.5 | 0.98 | 1.00 | 36.5 |
| 2 | T1 | 501 | 2.3 | 501 | 2.3 | 0.817 | 24.4 | LOS B | 13.5 | 98.5 | 0.98 | 1.00 | 33.5 |
| Appro | ach | 502 | 2.3 | 502 | 2.3 | 0.817 | 24.4 | LOS B | 13.5 | 98.5 | 0.98 | 1.00 | 33.5 |
| North | : Moorel | bank Avenu | ie | | | | | | | | | | |
| 8 | T1 | 955 | 1.2 | 955 | 1.2 | 0.911 | 25.9 | LOS B | 30.9 | 221.5 | 0.94 | 1.18 | 36.0 |
| 9 | R2 | 29 | 100.0 | 29 | 100. 0 | 0.227 | 27.8 | LOS B | 0.7 | 15.3 | 0.94 | 0.72 | 29.2 |
| Appro | ach | 984 | 4.2 | 984 | 4.2 | 0.911 | 26.0 | LOS B | 30.9 | 221.5 | 0.94 | 1.16 | 35.8 |
| West: | Chatha | ım Avenue | | | | | | | | | | | |
| 10 | L2 | 318 | 9.3 | 318 | 9.3 | 0.841 | 31.9 | LOS C | 9.1 | 75.6 | 1.00 | 1.03 | 16.4 |
| 12 | R2 | 1 | 0.0 | 1 | 0.0 | 0.841 | 31.9 | LOS C | 9.1 | 75.6 | 1.00 | 1.03 | 32.5 |
| Appro | ach | 319 | 9.2 | 319 | 9.2 | 0.841 | 31.9 | LOS C | 9.1 | 75.6 | 1.00 | 1.03 | 16.5 |
| All Ve | hicles | 1805 | 4.5 | 1805 | 4.5 | 0.911 | 26.6 | LOS B | 30.9 | 221.5 | 0.96 | 1.09 | 33.4 |

♦♦ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.1 %

Number of Iterations: 9 (maximum specified: 20)

| Movement Performance - Pedestrians | | | | | | | | | | |
|------------------------------------|---------------------|-------------------------|-------------------------|-------|-----------------------------------|---------------------------|-----------------|-----------------------------------|--|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | | Average Back Pedestrian ped | of Queue Distance m | Prop. Queued | Effective Stop Rate per ped | | |
| P3 | North Full Crossing | 11 | 19.4 | LOS B | 0.0 | 0.0 | 0.88 | 0.88 | | |
| P4 | West Full Crossing | 11 | 16.8 | LOS B | 0.0 | 0.0 | 0.82 | 0.82 | | |
| All Pe | destrians | 21 | 18.1 | LOS B | | | 0.85 | 0.85 | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: G [Moorebank Avenue/Chatham Avenue_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK

Signals - Fixed Time Isolated Cycle Time = 50 seconds (Practical Cycle Time)

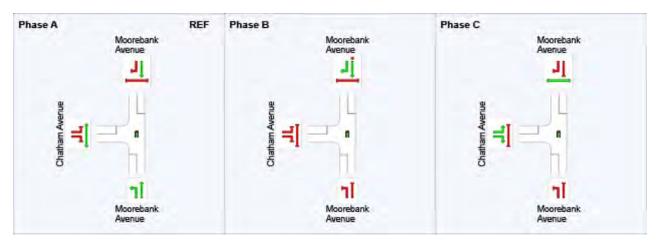
Phase Times determined by the program Green Split Priority applies **Phase Sequence: Opposed Turns** Reference Phase: Phase A Input Phase Sequence: A, B, C

Output Phase Sequence: A, B, C

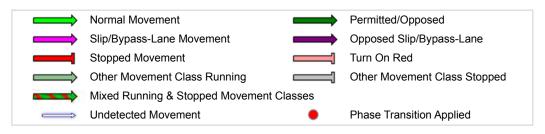
Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 0 | 22 | 34 |
| Green Time (sec) | 16 | 6 | 10 |
| Phase Time (sec) | 22 | 12 | 16 |
| Phase Split | 44% | 24% | 32% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

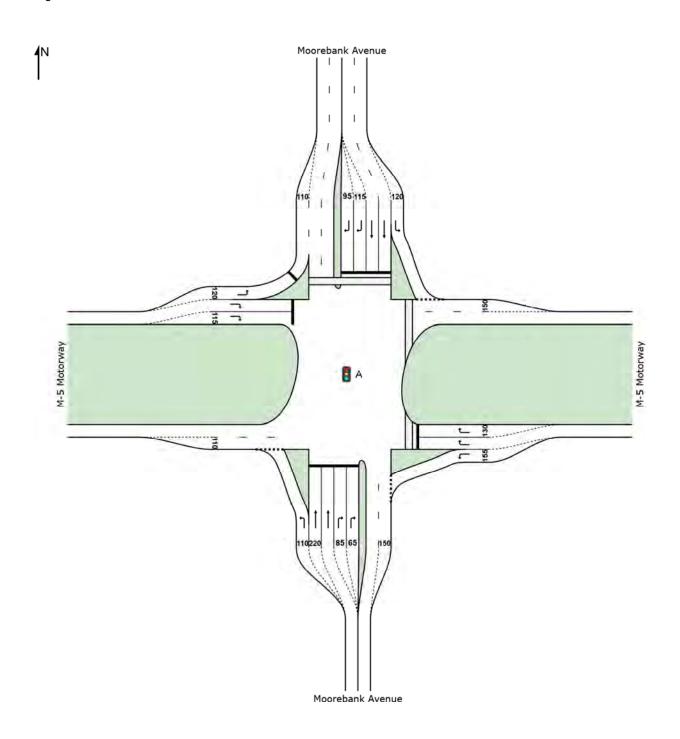


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Stage 2(ii)

Site: A [M5/Moorebank Avenue_AM]

Intersection of Moorebank Avenue and M5 Motorway AM PEAK
Signals - Fixed Time Isolated



Site: A [M5/Moorebank Avenue_AM]

Intersection of Moorebank Avenue and M5 Motorway AM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|-----------|-----------------|---------|------------------|---------|--------------|------------------|---------------------|----------------------|----------------------|--------|---------------------|---------|
| | | | | | | | | | | | | | |
| Mov ID | OD Mov | Demand Total | Flows | Arrival Total | Flows | Deg. Satn | Average Delav | Level of Service | 95% Back Vehicles | of Queue Distance | Prop. | Effective Stop Rate | |
| טו | IVIOV | veh/h | пv % | veh/h | пv % | v/c | sec | Service | venicies | Distance | Queued | per veh | km/h |
| South | : Moorel | oank Avenu | | VO11/11 | /0 | V/ O | 300 | | VOII | | | per veri | KIII/II |
| 1 | L2 | 428 | 14.7 | 428 | 14.7 | 0.396 | 14.4 | LOSA | 9.9 | 89.8 | 0.42 | 0.73 | 50.3 |
| 2 | T1 | 402 | 3.4 | 402 | 3.4 | 0.252 | 29.2 | LOS C | 9.3 | 69.3 | 0.68 | 0.58 | 34.6 |
| 3 | R2 | 271 | 20.2 | 271 | 20.2 | 0.441 | 57.9 | LOS E | 9.3 | 91.2 | 0.89 | 0.80 | 26.2 |
| Appro | ach | 1101 | 12.0 | 1101 | 12.0 | 0.441 | 30.5 | LOS C | 9.9 | 91.2 | 0.63 | 0.69 | 36.9 |
| East: | M-5 Mot | orway | | | | | | | | | | | |
| 4 | L2 | 339 | 17.7 | 339 | 17.7 | 0.285 | 6.4 | LOSA | 2.0 | 18.9 | 0.14 | 0.59 | 47.5 |
| 6 | R2 | 243 | 4.3 | 243 | 4.3 | 0.949 | 104.0 | LOS F | 10.7 | 81.6 | 1.00 | 1.05 | 17.1 |
| Appro | ach | 582 | 12.1 | 582 | 12.1 | 0.949 | 47.1 | LOS D | 10.7 | 81.6 | 0.50 | 0.78 | 23.9 |
| North | : Mooreb | ank Avenu | е | | | | | | | | | | |
| 7 | L2 | 48 | 19.6 | 48 | 19.6 | 0.042 | 7.3 | LOSA | 0.5 | 4.7 | 0.18 | 0.58 | 52.8 |
| 8 | T1 | 218 | 6.8 | 218 | 6.8 | 0.156 | 27.7 | LOS B | 5.2 | 41.5 | 0.65 | 0.53 | 24.6 |
| 9 | R2 | 506 | 20.2 | 506 | 20.2 | 0.967 | 87.5 | LOS F | 28.7 | 282.0 | 0.98 | 0.98 | 22.1 |
| Appro | ach | 773 | 16.3 | 773 | 16.3 | 0.967 | 65.6 | LOS E | 28.7 | 282.0 | 0.83 | 0.82 | 23.3 |
| West: | M-5 Mo | torway | | | | | | | | | | | |
| 10 | L2 | 1356 | 7.6 | 1356 | 7.6 | 0.887 | 7.1 | LOSA | 21.5 | 173.2 | 0.48 | 0.66 | 50.5 |
| 12 | R2 | 521 | 9.7 | 521 | 9.7 | 0.812 | 68.9 | LOS E | 20.8 | 173.8 | 0.99 | 0.88 | 18.9 |
| Appro | ach | 1877 | 8.2 | 1877 | 8.2 | 0.887 | 24.3 | LOS B | 21.5 | 173.8 | 0.62 | 0.72 | 38.0 |
| All Ve | hicles | 4333 | 11.1 | 4333 | 11.1 | 0.967 | 36.3 | LOS C | 28.7 | 282.0 | 0.64 | 0.74 | 32.4 |

♦♦ Network: 1 [Scenario 1_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 1.0 %

Number of Iterations: 20 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | | |
|--------|------------------------------------|--------|---------|---------|--------------|----------|--------|-----------|--|--|--|
| Mov | | Demand | Average | | Average Back | | Prop. | Effective | | | |
| ID | Description | Flow | Delay | Service | | Distance | Queued | Stop Rate | | | |
| | | ped/h | sec | | ped | m | | per ped | | | |
| P21 | East Stage 1 | 26 | 64.5 | LOS F | 0.1 | 0.1 | 0.93 | 0.93 | | | |
| P22 | East Stage 2 | 26 | 68.2 | LOS F | 0.1 | 0.1 | 0.95 | 0.95 | | | |
| P3 | North Full Crossing | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 | | | |
| All Pe | destrians | 79 | 67.3 | LOS F | | | 0.95 | 0.95 | | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

AM PEAK

Site: A [M5/Moorebank Avenue_AM]

♦♦ Network: 1 [Scenario 1_AM]

Intersection of Moorebank Avenue and M5 Motorway

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

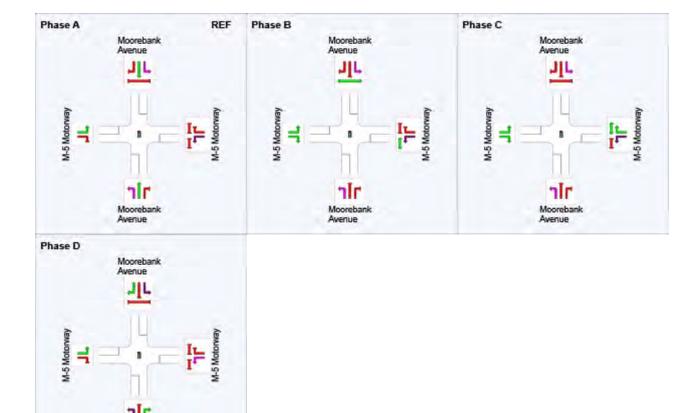
Phase Times determined by the program

Phase Sequence: 4-phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|-----|-----|
| Phase Change Time (sec) | 0 | 70 | 91 | 108 |
| Green Time (sec) | 64 | 15 | 11 | 36 |
| Phase Time (sec) | 70 | 21 | 17 | 42 |
| Phase Split | 47% | 14% | 11% | 28% |

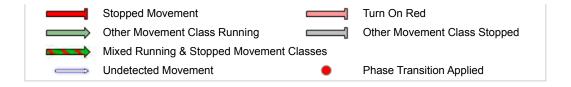
See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

Moorebank Avenue

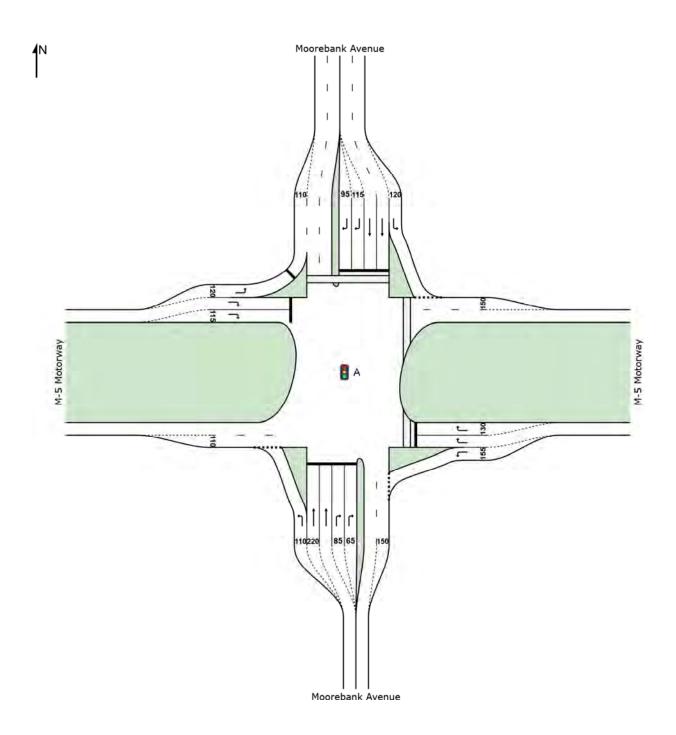




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Site: A [M5/Moorebank Avenue_PM]

Intersection of Moorebank Avenue and M5 Motorway PM PEAK
Signals - Fixed Time Isolated



Site: A [M5/Moorebank Avenue_PM]

Intersection of Moorebank Avenue and M5 Motorway

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

| Move | Movement Performance - Vehicles | | | | | | | | | | | | |
|-------------------------|---------------------------------|--------------------------|------------------|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South: Moorebank Avenue | | | | | | | | | | | | | |
| 1 | L2 | 541 | 7.4 | 541 | 7.4 | 0.761 | 42.8 | LOS D | 26.3 | 211.1 | 0.92 | 1.05 | 35.1 |
| 2 | T1 | 286 | 2.6 | 286 | 2.6 | 0.567 | 66.8 | LOS E | 10.0 | 73.6 | 0.99 | 0.80 | 22.4 |
| 3 | R2 | 404 | 8.9 | 404 | 8.9 | 0.239 | 22.6 | LOS B | 7.9 | 65.3 | 0.53 | 0.72 | 41.6 |
| Appro | ach | 1232 | 6.8 | 1232 | 6.8 | 0.761 | 41.7 | LOS C | 26.3 | 211.1 | 0.81 | 0.88 | 33.0 |
| East: | M-5 Mot | orway | | | | | | | | | | | |
| 4 | L2 | 281 | 12.7 | 281 | 12.7 | 0.239 | 7.1 | LOSA | 2.9 | 25.6 | 0.20 | 0.61 | 46.3 |
| 6 | R2 | 87 | 6.0 | 87 | 6.0 | 0.642 | 89.0 | LOS F | 3.4 | 26.9 | 1.00 | 0.78 | 19.0 |
| Appro | ach | 368 | 11.1 | 368 | 11.1 | 0.642 | 26.5 | LOS B | 3.4 | 26.9 | 0.39 | 0.65 | 30.7 |
| North | : Mooreb | ank Avenue | Э | | | | | | | | | | |
| 7 | L2 | 74 | 5.7 | 74 | 5.7 | 0.065 | 6.8 | LOSA | 0.6 | 4.8 | 0.16 | 0.59 | 55.9 |
| 8 | T1 | 405 | 1.8 | 405 | 1.8 | 0.864 | 74.2 | LOS F | 17.4 | 126.4 | 1.00 | 0.92 | 12.4 |
| 9 | R2 | 1296 | 4.5 | 1296 | 4.5 | 0.884 | 35.2 | LOS C | 46.1 | 352.4 | 0.76 | 0.85 | 38.0 |
| Appro | ach | 1775 | 4.0 | 1775 | 4.0 | 0.884 | 42.9 | LOS D | 46.1 | 352.4 | 0.79 | 0.85 | 31.6 |
| West: | M-5 Mo | torway | | | | | | | | | | | |
| 10 | L2 | 595 | 7.3 | 595 | 7.3 | 0.387 | 6.1 | LOSA | 2.8 | 22.5 | 0.13 | 0.56 | 52.0 |
| 12 | R2 | 439 | 9.6 | 439 | 9.6 | 0.810 | 72.7 | LOS F | 17.7 | 148.0 | 1.00 | 0.88 | 18.2 |
| Appro | ach | 1034 | 8.2 | 1034 | 8.2 | 0.810 | 34.4 | LOS C | 17.7 | 148.0 | 0.50 | 0.70 | 32.7 |
| All Ve | hicles | 4408 | 6.4 | 4408 | 6.4 | 0.884 | 39.2 | LOS C | 46.1 | 352.4 | 0.69 | 0.81 | 32.2 |

+ Network: 1 [Scenario 1_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.1 %

Number of Iterations: 9 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | | |
|--------|------------------------------------|--------|---------|---------|--------------|----------|--------|-----------|--|--|--|
| Mov | D | Demand | Average | | Average Back | | Prop. | Effective | | | |
| ID | Description | Flow | Delay | Service | Pedestrian | Distance | Queued | Stop Rate | | | |
| | | ped/h | sec | | ped | m | | per ped | | | |
| P21 | East Stage 1 | 26 | 64.5 | LOS F | 0.1 | 0.1 | 0.93 | 0.93 | | | |
| P22 | East Stage 2 | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 | | | |
| P3 | North Full Crossing | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 | | | |
| All Pe | destrians | 79 | 67.6 | LOS F | | | 0.95 | 0.95 | | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

PM PEAK

Site: A [M5/Moorebank Avenue_PM]

♦♦ Network: 1 [Scenario 1_PM]

Intersection of Moorebank Avenue and M5 Motorway

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

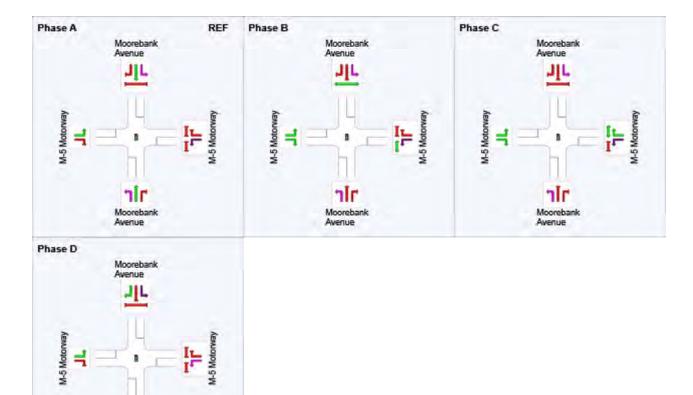
Phase Times determined by the program

Phase Sequence: 4-phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|----|-----|
| Phase Change Time (sec) | 0 | 26 | 47 | 59 |
| Green Time (sec) | 20 | 15 | 6 | 85 |
| Phase Time (sec) | 26 | 21 | 12 | 91 |
| Phase Split | 17% | 14% | 8% | 61% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

Moorebank Avenue

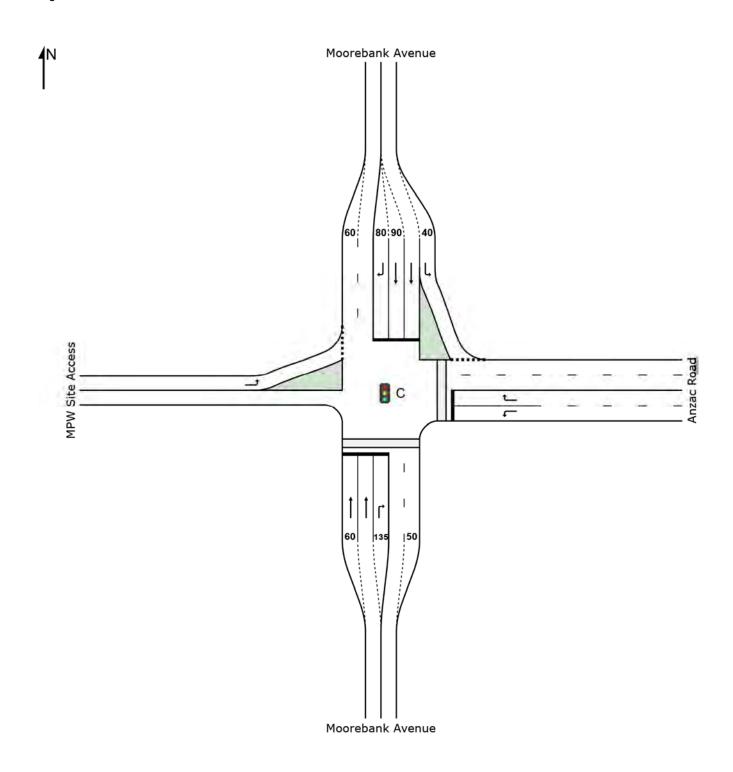




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Site: C [Moorebank Avenue_Anzac Road_AM]

Intersection of Moorebank Avenue and Anzac Road AM PEAK
Signals - Fixed Time Isolated



Site: C [Moorebank Avenue_Anzac Road_AM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 90 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|---------|----------------|---------|----------------|-----------|-------------|--------------|----------|-----------------|---------------|--------|-------------------|---------------|
| Mov | OD | Demand | l Flows | Arriva | l Flows | Deg. | Average | Level of | 95% Back | of Queue | Prop. | Effective | |
| ID | Mov | Total veh/h | HV % | Total veh/h | HV % | Satn v/c | Delay sec | Service | Vehicles veh | Distance m | Queued | Stop Rate per veh | Speed km/h |
| South | : Moore | bank Avenu | | | | | | | | | | | |
| 2 | T1 | 713 | 6.1 | 713 | 6.1 | 0.878 | 35.0 | LOS C | 23.9 | 187.4 | 0.90 | 0.92 | 18.0 |
| 3 | R2 | 381 | 3.3 | 381 | 3.3 | 0.603 | 29.9 | LOS C | 14.0 | 104.3 | 0.88 | 0.82 | 24.4 |
| Appro | ach | 1094 | 5.1 | 1094 | 5.1 | 0.878 | 33.2 | LOS C | 23.9 | 187.4 | 0.90 | 0.89 | 20.3 |
| East: | Anzac F | Road | | | | | | | | | | | |
| 4 | L2 | 208 | 3.0 | 208 | 3.0 | 0.486 | 38.3 | LOS C | 8.1 | 60.0 | 0.91 | 0.81 | 12.7 |
| 6 | R2 | 363 | 11.9 | 363 | 11.9 | 0.900 | 56.2 | LOS D | 19.2 | 166.7 | 1.00 | 1.03 | 9.6 |
| Appro | ach | 572 | 8.7 | 572 | 8.7 | 0.900 | 49.7 | LOS D | 19.2 | 166.7 | 0.97 | 0.95 | 10.5 |
| North | : Moore | bank Avenu | ie | | | | | | | | | | |
| 7 | L2 | 403 | 7.8 | 403 | 7.8 | 0.346 | 7.5 | LOSA | 6.3 | 51.1 | 0.45 | 0.61 | 34.1 |
| 8 | T1 | 492 | 10.3 | 492 | 10.3 | 0.901 | 45.9 | LOS D | 19.5 | 164.4 | 0.97 | 1.04 | 8.0 |
| 9 | R2 | 44 | 100.0 | 44 | 100. 0 | 0.182 | 37.6 | LOS C | 1.6 | 21.2 | 0.85 | 0.74 | 28.3 |
| Appro | ach | 939 | 13.5 | 939 | 13.5 | 0.901 | 29.0 | LOS C | 19.5 | 164.4 | 0.74 | 0.84 | 16.9 |
| West | MPW S | Site Access | | | | | | | | | | | |
| 10 | L2 | 44 | 100.0 | 44 | 100. 0 | 0.099 | 14.7 | LOS B | 0.6 | 8.1 | 0.61 | 0.67 | 41.3 |
| Appro | ach | 44 | 100.0 | 44 | 100. 0 | 0.099 | 14.7 | LOS B | 0.6 | 8.1 | 0.61 | 0.67 | 41.3 |
| All Ve | hicles | 2648 | 10.4 | 2648 | 10.4 | 0.901 | 35.0 | LOSC | 23.9 | 187.4 | 0.85 | 0.88 | 17.4 |
| | | | | | | | | | | | | | |

♦♦ Network: 1 [Scenario 1_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 1.0 %

Number of Iterations: 20 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | |
|--------|------------------------------------|--------|---------|---------|--------------|----------|--------|-----------|--|--|
| Mov | | Demand | Average | | Average Back | | Prop. | Effective | | |
| ID | Description | Flow | Delay | Service | Pedestrian | Distance | Queued | Stop Rate | | |
| | | ped/h | sec | | ped | m | | per ped | | |
| P1 | South Full Crossing | 11 | 38.3 | LOS D | 0.0 | 0.0 | 0.92 | 0.92 | | |
| P2 | East Full Crossing | 11 | 36.5 | LOS D | 0.0 | 0.0 | 0.90 | 0.90 | | |
| All Pe | destrians | 21 | 37.4 | LOS D | | | 0.91 | 0.91 | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: C [Moorebank Avenue_Anzac Road_AM]

♦♦ Network: 1 [Scenario 1_AM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 90 seconds (Practical Cycle Time)

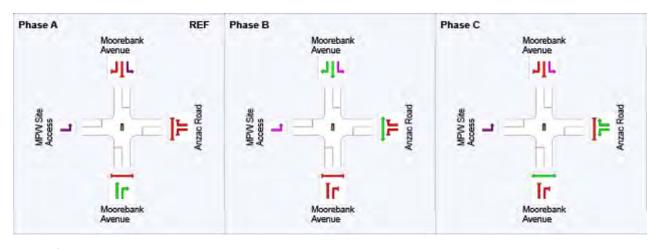
Phase Times determined by the program

Phase Sequence: 4 Phase Reference Phase: Phase A Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 0 | 37 | 63 |
| Green Time (sec) | 31 | 20 | 21 |
| Phase Time (sec) | 37 | 26 | 27 |
| Phase Split | 41% | 29% | 30% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase



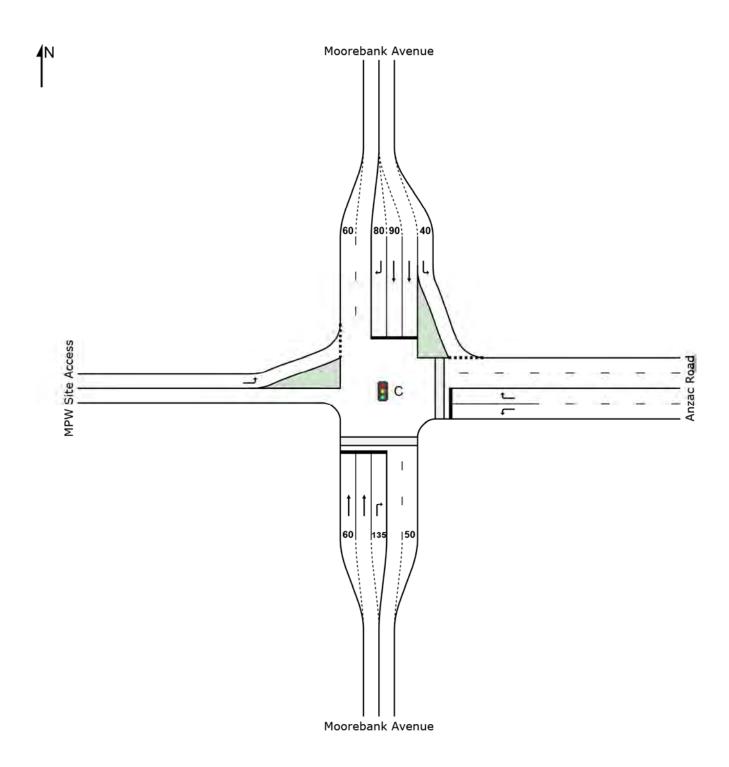
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\Scenario 2\Scenario 2_Stage 2_75%.sip7

Site: C [Moorebank Avenue_Anzac Road_PM]

Intersection of Moorebank Avenue and Anzac Road AM PEAK
Signals - Fixed Time Isolated



Site: C [Moorebank Avenue_Anzac Road_PM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 50 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|----------|-------------|-------|--------|-----------|-------|---------|----------|----------|----------|--------|-----------|---------|
| Mov | OD | Demand | Flows | Arriva | l Flows | Deg. | Average | Level of | 95% Back | of Queue | Prop. | Effective | Average |
| ID | Mov | Total | HV | Total | HV | Satn | Delay | Service | Vehicles | Distance | Queued | Stop Rate | |
| | | veh/h | | veh/h | % | v/c | sec | | veh | m | | per veh | km/h |
| South | n: Moore | ebank Avenu | ıe | | | | | | | | | | |
| 2 | T1 | 789 | 4.0 | 789 | 4.0 | 0.936 | 30.8 | LOS C | 20.8 | 157.3 | 0.95 | 1.21 | 19.2 |
| 3 | R2 | 211 | 0.5 | 211 | 0.5 | 0.804 | 31.0 | LOS C | 5.9 | 41.5 | 1.00 | 1.02 | 24.1 |
| Appro | oach | 1000 | 3.3 | 1000 | 3.3 | 0.936 | 30.8 | LOS C | 20.8 | 157.3 | 0.96 | 1.17 | 20.4 |
| East: | Anzac F | Road | | | | | | | | | | | |
| 4 | L2 | 280 | 1.5 | 280 | 1.5 | 0.838 | 33.4 | LOS C | 8.0 | 57.7 | 1.00 | 1.00 | 14.1 |
| 6 | R2 | 287 | 4.0 | 287 | 4.0 | 0.875 | 36.1 | LOS C | 8.7 | 65.8 | 1.00 | 1.09 | 13.7 |
| Appro | oach | 567 | 2.8 | 567 | 2.8 | 0.875 | 34.8 | LOS C | 8.7 | 65.8 | 1.00 | 1.05 | 13.9 |
| North | n: Moore | bank Avenu | ie | | | | | | | | | | |
| 7 | L2 | 419 | 3.0 | 419 | 3.0 | 0.361 | 5.5 | LOSA | 3.1 | 23.2 | 0.48 | 0.62 | 36.9 |
| 8 | T1 | 673 | 4.2 | 673 | 4.2 | 0.824 | 20.4 | LOS B | 13.7 | 104.3 | 0.94 | 0.96 | 14.4 |
| 9 | R2 | 44 | 100.0 | 44 | 100. 0 | 0.288 | 29.0 | LOS C | 1.1 | 14.0 | 0.93 | 0.74 | 31.7 |
| Appro | oach | 1136 | 7.5 | 1136 | 7.5 | 0.824 | 15.2 | LOS B | 13.7 | 104.3 | 0.77 | 0.82 | 23.2 |
| West | : MPW S | Site Access | | | | | | | | | | | |
| 10 | L2 | 44 | 100.0 | 44 | 100. 0 | 0.095 | 12.9 | LOSA | 0.5 | 6.1 | 0.71 | 0.68 | 43.0 |
| Appro | oach | 44 | 100.0 | 44 | 100. | 0.095 | 12.9 | LOSA | 0.5 | 6.1 | 0.71 | 0.68 | 43.0 |
| All Ve | ehicles | 2747 | 6.5 | 2747 | 6.5 | 0.936 | 24.9 | LOS B | 20.8 | 157.3 | 0.88 | 0.99 | 20.2 |

♦♦ Network: 1 [Scenario 1_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.1 %

Number of Iterations: 9 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | |
|--------|------------------------------------|--------|---------|---------|--------------|----------|--------|-----------|--|--|
| Mov | Description | Demand | Average | | Average Back | | Prop. | Effective | | |
| ID | Description | Flow | Delay | Service | Pedestrian | Distance | Queued | Stop Rate | | |
| | | ped/h | sec | | ped | m | | per ped | | |
| P1 | South Full Crossing | 11 | 19.4 | LOS B | 0.0 | 0.0 | 0.88 | 0.88 | | |
| P2 | East Full Crossing | 11 | 19.4 | LOS B | 0.0 | 0.0 | 0.88 | 0.88 | | |
| All Pe | destrians | 21 | 19.4 | LOS B | | | 0.88 | 0.88 | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: C [Moorebank Avenue_Anzac Road_PM]

♦♦ Network: 1 [Scenario 1_PM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 50 seconds (Practical Cycle Time)

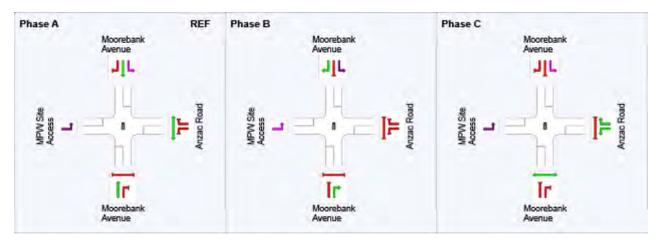
Phase Times determined by the program

Phase Sequence: 4 Phase Reference Phase: Phase A Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 0 | 22 | 35 |
| Green Time (sec) | 16 | 7 | 9 |
| Phase Time (sec) | 22 | 13 | 15 |
| Phase Split | 44% | 26% | 30% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

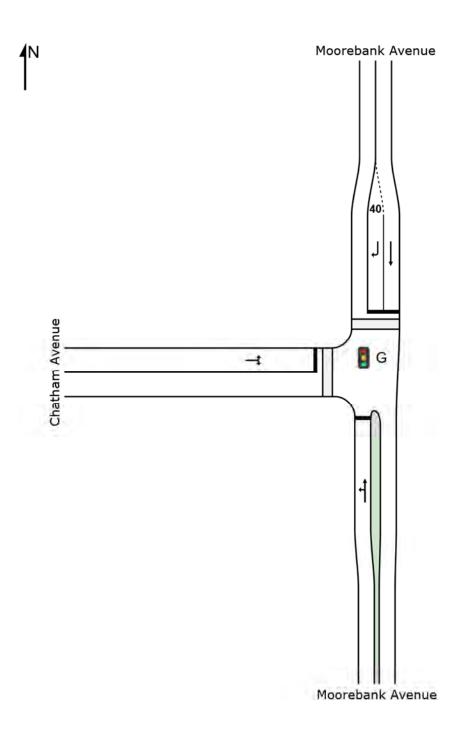


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Site: G [Moorebank Avenue/Chatham Avenue_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK
Signals - Fixed Time Isolated



Site: G [Moorebank Avenue/Chatham Avenue_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|-----------|--------------------------|---------|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | HV % | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South: Moorebank Avenue | | | | | | | | | | | | | |
| 1 | L2 | 1 | 0.0 | 1 | 0.0 | 0.896 | 30.2 | LOS C | 48.0 | 361.3 | 0.91 | 0.99 | 35.2 |
| 2 | T1 | 1103 | 3.7 | 1103 | 3.7 | 0.896 | 27.0 | LOS B | 48.0 | 361.3 | 0.91 | 0.99 | 32.0 |
| Appro | ach | 1104 | 3.7 | 1104 | 3.7 | 0.896 | 27.0 | LOS B | 48.0 | 361.3 | 0.91 | 0.99 | 32.0 |
| North | : Moorel | bank Avenu | e | | | | | | | | | | |
| 8 | T1 | 457 | 9.2 | 457 | 9.2 | 0.315 | 2.7 | LOSA | 5.3 | 43.6 | 0.30 | 0.27 | 45.7 |
| 9 | R2 | 15 | 100.0 | 15 | 100. 0 | 0.193 | 48.2 | LOS D | 0.6 | 13.4 | 0.97 | 0.70 | 24.1 |
| Appro | ach | 472 | 12.1 | 472 | 12.1 | 0.315 | 4.1 | LOSA | 5.3 | 43.6 | 0.32 | 0.28 | 44.8 |
| West: | Chatha | m Avenue | | | | | | | | | | | |
| 10 | L2 | 15 | 100.0 | 15 | 100. 0 | 0.184 | 49.0 | LOS D | 0.7 | 13.6 | 0.97 | 0.70 | 12.1 |
| 12 | R2 | 1 | 0.0 | 1 | 0.0 | 0.184 | 48.5 | LOS D | 0.7 | 13.6 | 0.97 | 0.70 | 26.8 |
| Appro | ach | 16 | 93.3 | 16 | 93.3 | 0.184 | 49.0 | LOS D | 0.7 | 13.6 | 0.97 | 0.70 | 13.5 |
| All Ve | hicles | 1592 | 7.1 | 1592 | 7.1 | 0.896 | 20.5 | LOS B | 48.0 | 361.3 | 0.74 | 0.78 | 36.7 |

♦♦ Network: 1 [Scenario 1_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 1.0 %

Number of Iterations: 20 (maximum specified: 20)

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|----------------|------------------|-------|----------------------------|----------------------|-----------------|------------------------|--|
| Mov ID | Description | Demand Flow | Average Delay | | Average Back Pedestrian | of Queue Distance | Prop. Queued | Effective Stop Rate | |
| | | ped/h | sec | | ped | m | | per ped | |
| P3 | North Full Crossing | 11 | 36.7 | LOS D | 0.0 | 0.0 | 0.93 | 0.93 | |
| P4 | West Full Crossing | 11 | 8.1 | LOS A | 0.0 | 0.0 | 0.44 | 0.44 | |
| All Pe | destrians | 21 | 22.4 | LOS C | | | 0.68 | 0.68 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: G [Moorebank Avenue/Chatham Avenue_AM]

♦♦ Network: 1 [Scenario 1_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK

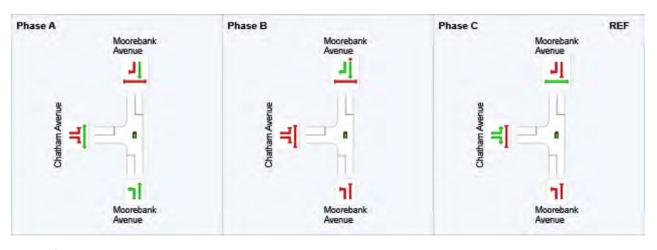
Signals - Fixed Time Isolated Cycle Time = 85 seconds (Practical Cycle Time)

Phase Times determined by the program Green Split Priority applies **Phase Sequence: Opposed Turns** Reference Phase: Phase C Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

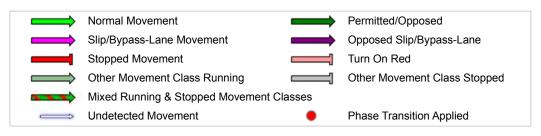
Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 12 | 73 | 0 |
| Green Time (sec) | 55 | 6 | 6 |
| Phase Time (sec) | 61 | 12 | 12 |
| Phase Split | 72% | 14% | 14% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



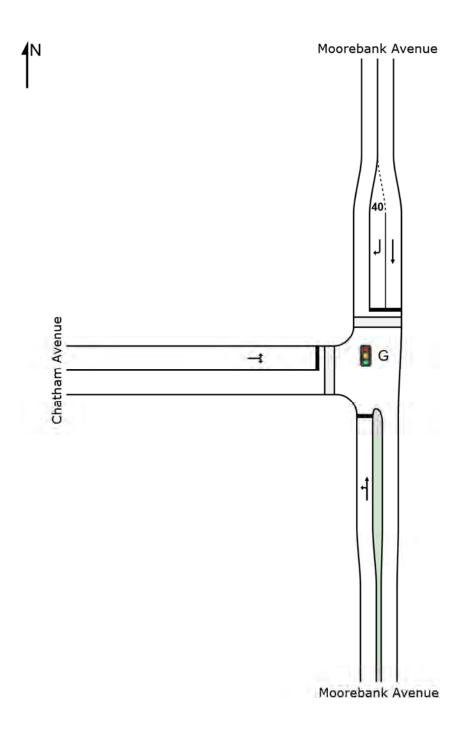
REF: Reference Phase VAR: Variable Phase



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Site: G [Moorebank Avenue/Chatham Avenue_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK
Signals - Fixed Time Isolated



Site: G [Moorebank Avenue/Chatham Avenue_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK

Signals - Fixed Time Isolated Cycle Time = 50 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|-----------|--------------------------|------------------|-------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Total | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South: Moorebank Avenue | | | | | | | | | | | | | |
| 1 | L2 | 1 | 0.0 | 1 | 0.0 | 0.769 | 24.6 | LOS B | 12.4 | 90.9 | 0.95 | 0.93 | 38.2 |
| 2 | T1 | 501 | 2.3 | 501 | 2.3 | 0.769 | 21.4 | LOS B | 12.4 | 90.9 | 0.95 | 0.93 | 35.4 |
| Appro | ach | 502 | 2.3 | 502 | 2.3 | 0.769 | 21.4 | LOS B | 12.4 | 90.9 | 0.95 | 0.93 | 35.4 |
| North | : Moore | bank Avenu | ie | | | | | | | | | | |
| 8 | T1 | 955 | 1.2 | 955 | 1.2 | 0.866 | 18.0 | LOS B | 25.9 | 185.4 | 0.90 | 1.01 | 38.8 |
| 9 | R2 | 15 | 100.0 | 15 | 100. 0 | 0.113 | 27.2 | LOS B | 0.4 | 7.5 | 0.92 | 0.68 | 29.4 |
| Appro | ach | 969 | 2.7 | 969 | 2.7 | 0.866 | 18.2 | LOS B | 25.9 | 185.4 | 0.90 | 1.00 | 38.7 |
| West: | Chatha | m Avenue | | | | | | | | | | | |
| 10 | L2 | 303 | 4.9 | 303 | 4.9 | 0.865 | 33.9 | LOS C | 9.0 | 69.0 | 1.00 | 1.07 | 15.7 |
| 12 | R2 | 1 | 0.0 | 1 | 0.0 | 0.865 | 33.9 | LOS C | 9.0 | 69.0 | 1.00 | 1.07 | 31.7 |
| Appro | ach | 304 | 4.8 | 304 | 4.8 | 0.865 | 33.9 | LOS C | 9.0 | 69.0 | 1.00 | 1.07 | 15.8 |
| All Ve | hicles | 1776 | 3.0 | 1776 | 3.0 | 0.866 | 21.8 | LOS B | 25.9 | 185.4 | 0.93 | 0.99 | 35.6 |

♦♦ Network: 1 [Scenario 1_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.1 %

Number of Iterations: 9 (maximum specified: 20)

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------------|-------------------------|-------|-----------------------------------|---------------------------|-----------------|-----------------------------------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | | Average Back Pedestrian ped | of Queue Distance m | Prop. Queued | Effective Stop Rate per ped | |
| P3 | North Full Crossing | 11 | 19.4 | LOS B | 0.0 | 0.0 | 0.88 | 0.88 | |
| P4 | West Full Crossing | 11 | 16.0 | LOS B | 0.0 | 0.0 | 0.80 | 0.80 | |
| All Pe | destrians | 21 | 17.7 | LOS B | | | 0.84 | 0.84 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: G [Moorebank Avenue/Chatham Avenue_PM]

♦♦ Network: 1 [Scenario 1_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK

Signals - Fixed Time Isolated Cycle Time = 50 seconds (Practical Cycle Time)

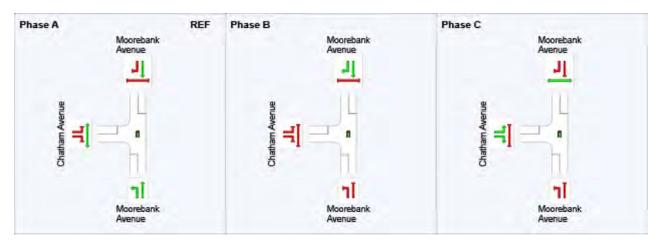
Phase Times determined by the program Green Split Priority applies **Phase Sequence: Opposed Turns** Reference Phase: Phase A Input Phase Sequence: A, B, C

Output Phase Sequence: A, B, C

Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 0 | 23 | 35 |
| Green Time (sec) | 17 | 6 | 9 |
| Phase Time (sec) | 23 | 12 | 15 |
| Phase Split | 46% | 24% | 30% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase



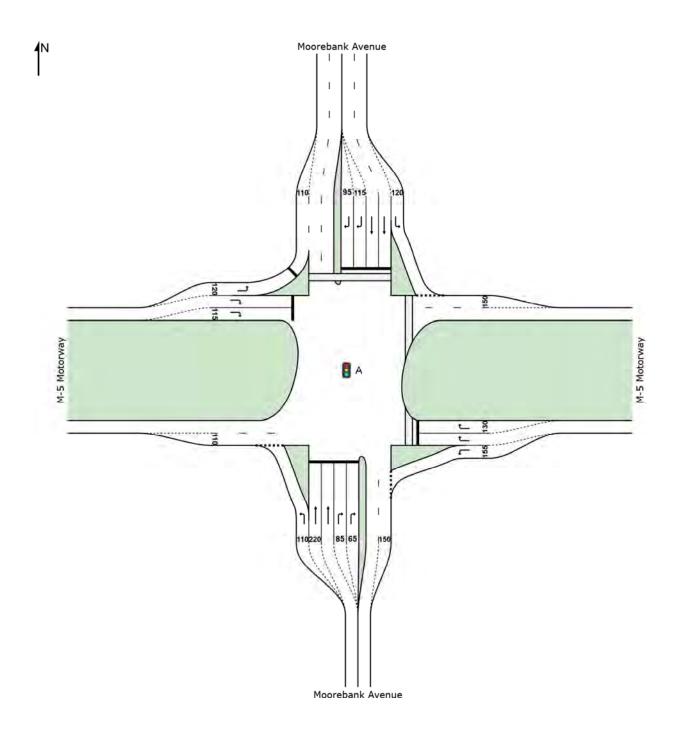
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Stage 2(iii)

Site: A [M5/Moorebank Avenue_AM]

Intersection of Moorebank Avenue and M5 Motorway AM PEAK
Signals - Fixed Time Isolated



Site: A [M5/Moorebank Avenue_AM]

Intersection of Moorebank Avenue and M5 Motorway

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|-----------|-----------------|---------|------------------|---------|--------------|------------------|---------------------|----------------------|----------------------|--------|---------------------|---------|
| Mov ID | OD Mov | Demand Total | Flows | Arrival Total | Flows | Deg. Satn | Average Delav | Level of Service | 95% Back Vehicles | of Queue Distance | Prop. | Effective Stop Rate | |
| טו | IVIOV | veh/h | пv % | veh/h | пv % | v/c | sec | Service | venicies | Distance | Queued | per veh | km/h |
| South | : Moorel | oank Avenu | | VO11/11 | /0 | V/ O | 500 | | VOII | | | per veri | KITI/TI |
| 1 | L2 | 428 | 14.7 | 428 | 14.7 | 0.396 | 14.4 | LOSA | 9.9 | 89.8 | 0.42 | 0.73 | 50.3 |
| 2 | T1 | 402 | 3.4 | 402 | 3.4 | 0.252 | 29.2 | LOS C | 9.3 | 69.3 | 0.68 | 0.58 | 34.6 |
| 3 | R2 | 271 | 20.2 | 271 | 20.2 | 0.441 | 57.9 | LOS E | 9.3 | 91.2 | 0.89 | 0.80 | 26.2 |
| Appro | ach | 1101 | 12.0 | 1101 | 12.0 | 0.441 | 30.5 | LOS C | 9.9 | 91.2 | 0.63 | 0.69 | 36.9 |
| East: | M-5 Mot | orway | | | | | | | | | | | |
| 4 | L2 | 339 | 17.7 | 339 | 17.7 | 0.285 | 6.4 | LOSA | 2.0 | 18.9 | 0.14 | 0.59 | 47.5 |
| 6 | R2 | 243 | 4.3 | 243 | 4.3 | 0.949 | 104.0 | LOS F | 10.7 | 81.6 | 1.00 | 1.05 | 17.1 |
| Appro | ach | 582 | 12.1 | 582 | 12.1 | 0.949 | 47.1 | LOS D | 10.7 | 81.6 | 0.50 | 0.78 | 23.9 |
| North | : Mooreb | ank Avenu | е | | | | | | | | | | |
| 7 | L2 | 48 | 19.6 | 48 | 19.6 | 0.042 | 7.3 | LOSA | 0.5 | 4.7 | 0.18 | 0.58 | 52.8 |
| 8 | T1 | 218 | 6.8 | 218 | 6.8 | 0.156 | 27.7 | LOS B | 5.2 | 41.5 | 0.65 | 0.53 | 24.6 |
| 9 | R2 | 506 | 20.2 | 506 | 20.2 | 0.967 | 87.5 | LOS F | 28.7 | 282.0 | 0.98 | 0.98 | 22.1 |
| Appro | ach | 773 | 16.3 | 773 | 16.3 | 0.967 | 65.6 | LOS E | 28.7 | 282.0 | 0.83 | 0.82 | 23.3 |
| West: | M-5 Mo | torway | | | | | | | | | | | |
| 10 | L2 | 1356 | 7.6 | 1356 | 7.6 | 0.887 | 7.1 | LOSA | 21.5 | 173.2 | 0.48 | 0.66 | 50.5 |
| 12 | R2 | 521 | 9.7 | 521 | 9.7 | 0.812 | 68.9 | LOS E | 20.8 | 173.8 | 0.99 | 0.88 | 18.9 |
| Appro | ach | 1877 | 8.2 | 1877 | 8.2 | 0.887 | 24.3 | LOS B | 21.5 | 173.8 | 0.62 | 0.72 | 38.0 |
| All Ve | hicles | 4333 | 11.1 | 4333 | 11.1 | 0.967 | 36.3 | LOS C | 28.7 | 282.0 | 0.64 | 0.74 | 32.4 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 %

Number of Iterations: 9 (maximum specified: 20)

| Move | ment Performance - Pedestrians | S | | | | | | |
|--------|--------------------------------|--------|---------|---------|--------------|----------|--------|-----------|
| Mov | | Demand | Average | | Average Back | | Prop. | Effective |
| ID | Description | Flow | Delay | Service | | Distance | Queued | Stop Rate |
| | | ped/h | sec | | ped | m | | per ped |
| P21 | East Stage 1 | 26 | 64.5 | LOS F | 0.1 | 0.1 | 0.93 | 0.93 |
| P22 | East Stage 2 | 26 | 68.2 | LOS F | 0.1 | 0.1 | 0.95 | 0.95 |
| P3 | North Full Crossing | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 |
| All Pe | destrians | 79 | 67.3 | LOS F | | | 0.95 | 0.95 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: A [M5/Moorebank Avenue_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and M5 Motorway AM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

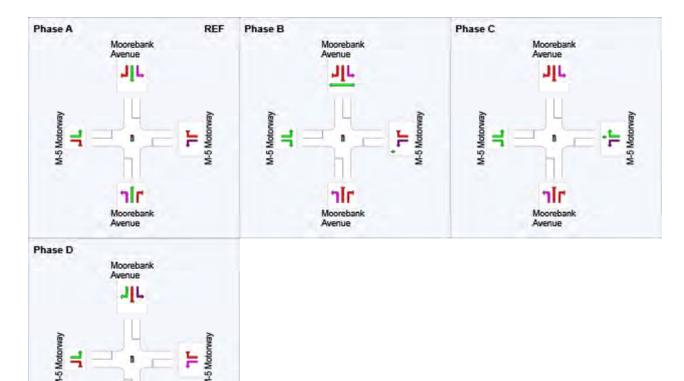
Phase Times determined by the program

Phase Sequence: 4-phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|-----|-----|
| Phase Change Time (sec) | 0 | 70 | 91 | 108 |
| Green Time (sec) | 64 | 15 | 11 | 36 |
| Phase Time (sec) | 70 | 21 | 17 | 42 |
| Phase Split | 47% | 14% | 11% | 28% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

Moorebank Avenue

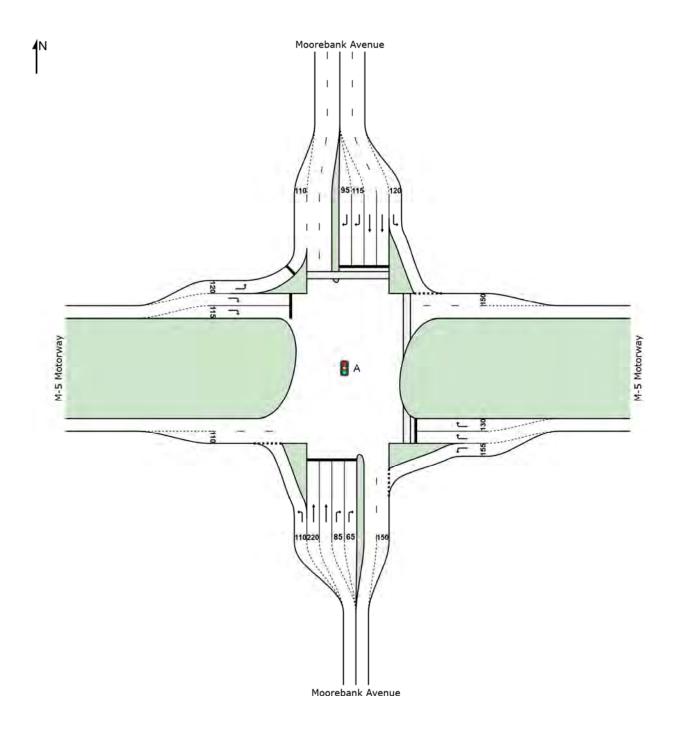




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Site: A [M5/Moorebank Avenue_PM]

Intersection of Moorebank Avenue and M5 Motorway PM PEAK
Signals - Fixed Time Isolated



Site: A [M5/Moorebank Avenue_PM]

Intersection of Moorebank Avenue and M5 Motorway

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|-----------|--------------------------|------------------|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| Sout | h: Moorel | oank Avenu | | | | | | | | | | | |
| 1 | L2 | 541 | 7.4 | 541 | 7.4 | 0.761 | 42.8 | LOS D | 26.3 | 211.1 | 0.92 | 1.05 | 35.1 |
| 2 | T1 | 286 | 2.6 | 286 | 2.6 | 0.567 | 66.8 | LOS E | 10.0 | 73.6 | 0.99 | 0.80 | 22.4 |
| 3 | R2 | 404 | 8.9 | 404 | 8.9 | 0.239 | 22.6 | LOS B | 7.9 | 65.3 | 0.53 | 0.72 | 41.6 |
| Appr | oach | 1232 | 6.8 | 1232 | 6.8 | 0.761 | 41.7 | LOS C | 26.3 | 211.1 | 0.81 | 0.88 | 33.0 |
| East: | M-5 Mot | orway | | | | | | | | | | | |
| 4 | L2 | 281 | 12.7 | 281 | 12.7 | 0.239 | 7.1 | LOSA | 2.9 | 25.6 | 0.20 | 0.61 | 46.3 |
| 6 | R2 | 87 | 6.0 | 87 | 6.0 | 0.642 | 89.0 | LOS F | 3.4 | 26.9 | 1.00 | 0.78 | 19.0 |
| Appr | oach | 368 | 11.1 | 368 | 11.1 | 0.642 | 26.5 | LOS B | 3.4 | 26.9 | 0.39 | 0.65 | 30.7 |
| North | n: Mooreb | ank Avenue | е | | | | | | | | | | |
| 7 | L2 | 74 | 5.7 | 74 | 5.7 | 0.065 | 6.8 | LOSA | 0.6 | 4.8 | 0.16 | 0.59 | 55.9 |
| 8 | T1 | 405 | 1.8 | 405 | 1.8 | 0.864 | 74.2 | LOS F | 17.4 | 126.4 | 1.00 | 0.92 | 12.4 |
| 9 | R2 | 1296 | 4.5 | 1296 | 4.5 | 0.884 | 35.2 | LOS C | 46.1 | 352.4 | 0.76 | 0.85 | 38.0 |
| Appr | oach | 1775 | 4.0 | 1775 | 4.0 | 0.884 | 42.9 | LOS D | 46.1 | 352.4 | 0.79 | 0.85 | 31.6 |
| West | : M-5 Mo | torway | | | | | | | | | | | |
| 10 | L2 | 595 | 7.3 | 595 | 7.3 | 0.387 | 6.1 | LOSA | 2.8 | 22.5 | 0.13 | 0.56 | 52.0 |
| 12 | R2 | 439 | 9.6 | 439 | 9.6 | 0.810 | 72.7 | LOS F | 17.7 | 148.0 | 1.00 | 0.88 | 18.2 |
| Appr | oach | 1034 | 8.2 | 1034 | 8.2 | 0.810 | 34.4 | LOS C | 17.7 | 148.0 | 0.50 | 0.70 | 32.7 |
| All Ve | ehicles | 4408 | 6.4 | 4408 | 6.4 | 0.884 | 39.2 | LOS C | 46.1 | 352.4 | 0.69 | 0.81 | 32.2 |

+ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.0 %

Number of Iterations: 9 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | | | |
|--------|------------------------------------|--------|---------|---------|--------------|----------|--------|-----------|--|--|--|--|
| Mov | D | Demand | Average | | Average Back | | Prop. | Effective | | | | |
| ID | Description | Flow | Delay | Service | Pedestrian | Distance | Queued | Stop Rate | | | | |
| | | ped/h | sec | | ped | m | | per ped | | | | |
| P21 | East Stage 1 | 26 | 64.5 | LOS F | 0.1 | 0.1 | 0.93 | 0.93 | | | | |
| P22 | East Stage 2 | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 | | | | |
| P3 | North Full Crossing | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 | | | | |
| All Pe | destrians | 79 | 67.6 | LOS F | | | 0.95 | 0.95 | | | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: A [M5/Moorebank Avenue_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and M5 Motorway

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

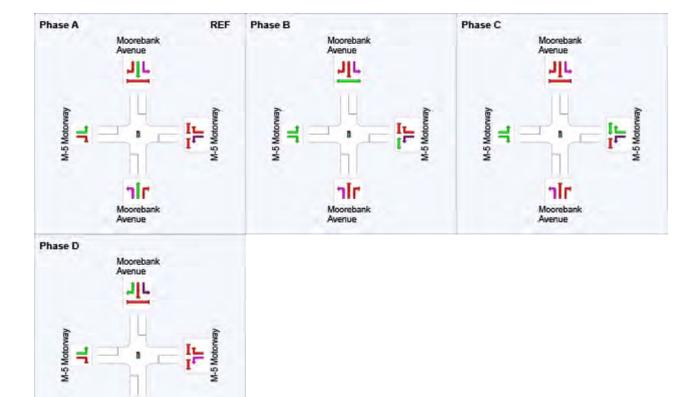
Phase Times determined by the program

Phase Sequence: 4-phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|----|-----|
| Phase Change Time (sec) | 0 | 26 | 47 | 59 |
| Green Time (sec) | 20 | 15 | 6 | 85 |
| Phase Time (sec) | 26 | 21 | 12 | 91 |
| Phase Split | 17% | 14% | 8% | 61% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

Moorebank Avenue

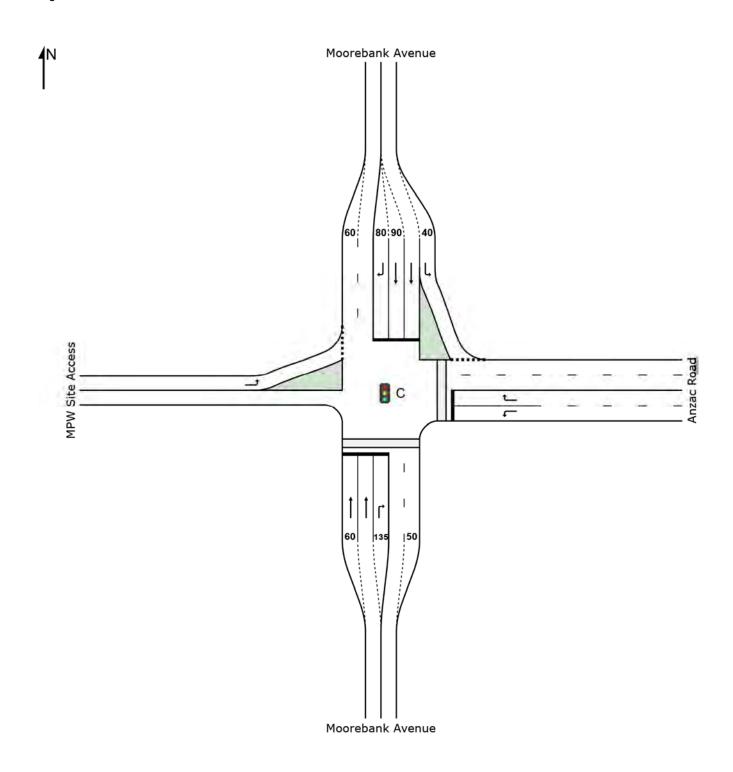




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Site: C [Moorebank Avenue_Anzac Road_AM]

Intersection of Moorebank Avenue and Anzac Road AM PEAK
Signals - Fixed Time Isolated



Site: C [Moorebank Avenue_Anzac Road_AM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 75 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|-----------|-------------|---------|--------|-----------|-------|---------|----------|----------|----------|--------|-----------|---------|
| Mov | OD | Demand | l Flows | Arriva | l Flows | Deg. | Average | Level of | 95% Back | of Queue | Prop. | Effective | Average |
| ID | Mov | Total | HV | Total | HV | Satn | Delay | Service | Vehicles | Distance | Queued | Stop Rate | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | per veh | km/h |
| Sout | h: Moore | ebank Avenu | ne | | | | | | | | | | |
| 2 | T1 | 698 | 4.1 | 698 | 4.1 | 0.915 | 37.0 | LOS C | 22.8 | 172.8 | 0.95 | 1.06 | 17.4 |
| 3 | R2 | 381 | 3.3 | 381 | 3.3 | 0.678 | 28.8 | LOS C | 12.6 | 94.0 | 0.93 | 0.84 | 24.8 |
| Appr | oach | 1079 | 3.8 | 1079 | 3.8 | 0.915 | 34.1 | LOS C | 22.8 | 172.8 | 0.94 | 0.98 | 20.1 |
| East | : Anzac I | Road | | | | | | | | | | | |
| 4 | L2 | 208 | 3.0 | 208 | 3.0 | 0.473 | 32.6 | LOS C | 6.7 | 49.9 | 0.91 | 0.80 | 14.4 |
| 6 | R2 | 363 | 11.9 | 363 | 11.9 | 0.875 | 45.9 | LOS D | 15.7 | 136.0 | 1.00 | 1.02 | 11.3 |
| Appr | oach | 572 | 8.7 | 572 | 8.7 | 0.875 | 41.1 | LOS C | 15.7 | 136.0 | 0.97 | 0.94 | 12.2 |
| North | n: Moore | bank Avenu | ie | | | | | | | | | | |
| 7 | L2 | 403 | 7.8 | 403 | 7.8 | 0.351 | 7.4 | LOSA | 5.6 | 44.9 | 0.48 | 0.61 | 34.2 |
| 8 | T1 | 477 | 7.5 | 477 | 7.5 | 0.895 | 39.3 | LOS C | 15.9 | 128.3 | 0.97 | 1.07 | 9.0 |
| 9 | R2 | 59 | 100.0 | 59 | 100. 0 | 0.252 | 33.7 | LOS C | 1.9 | 24.6 | 0.87 | 0.75 | 29.7 |
| Appr | oach | 939 | 13.5 | 939 | 13.5 | 0.895 | 25.2 | LOS B | 15.9 | 128.3 | 0.75 | 0.85 | 18.8 |
| West | t: MPW S | Site Access | | | | | | | | | | | |
| 10 | L2 | 59 | 100.0 | 59 | 100. 0 | 0.128 | 14.3 | LOSA | 0.8 | 9.8 | 0.65 | 0.68 | 41.7 |
| Appr | oach | 59 | 100.0 | 59 | 100. 0 | 0.128 | 14.3 | LOSA | 0.8 | 9.8 | 0.65 | 0.68 | 41.7 |
| All V | ehicles | 2648 | 10.4 | 2648 | 10.4 | 0.915 | 32.0 | LOS C | 22.8 | 172.8 | 0.87 | 0.92 | 18.5 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: $0.9\,\%$

Number of Iterations: 9 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | |
|--------|------------------------------------|--------|---------|---------|--------------|----------|--------|-----------|--|--|
| Mov | Decembring | Demand | Average | | Average Back | | Prop. | Effective | | |
| ID | Description | Flow | Delay | Service | Pedestrian | Distance | Queued | Stop Rate | | |
| | | ped/h | sec | | ped | m | | per ped | | |
| P1 | South Full Crossing | 11 | 31.8 | LOS D | 0.0 | 0.0 | 0.92 | 0.92 | | |
| P2 | East Full Crossing | 11 | 31.8 | LOS D | 0.0 | 0.0 | 0.92 | 0.92 | | |
| All Pe | destrians | 21 | 31.8 | LOS D | | | 0.92 | 0.92 | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: C [Moorebank Avenue_Anzac Road_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 75 seconds (Practical Cycle Time)

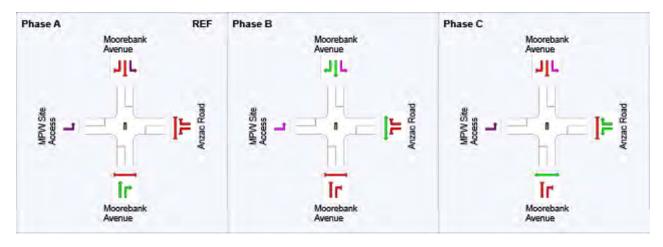
Phase Times determined by the program

Phase Sequence: 4 Phase Reference Phase: Phase A Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

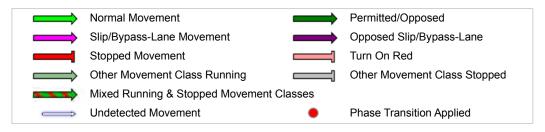
Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 0 | 29 | 51 |
| Green Time (sec) | 23 | 16 | 18 |
| Phase Time (sec) | 29 | 22 | 24 |
| Phase Split | 39% | 29% | 32% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase



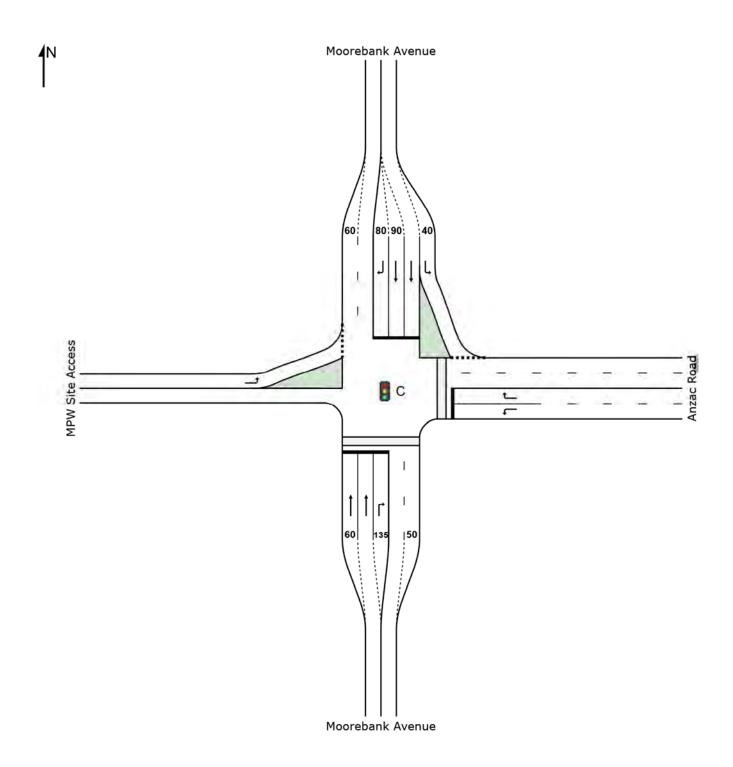
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Site: C [Moorebank Avenue_Anzac Road_PM]

Intersection of Moorebank Avenue and Anzac Road AM PEAK
Signals - Fixed Time Isolated



Site: C [Moorebank Avenue_Anzac Road_PM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 50 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|-----------|--------------------------|------------------|--------------------------|--------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Arriva Total veh/h | I Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South: Moorebank Avenue | | | | | | | | | | 1011/11 | | | |
| 2 | T1 | 775 | 2.2 | 775 | 2.2 | 0.908 | 26.6 | LOS B | 18.5 | 135.4 | 0.95 | 1.12 | 20.7 |
| 3 | R2 | 211 | 0.5 | 211 | 0.5 | 0.804 | 31.0 | LOS C | 5.9 | 41.5 | 1.00 | 1.02 | 24.1 |
| Appr | oach | 985 | 1.8 | 985 | 1.8 | 0.908 | 27.5 | LOS B | 18.5 | 135.4 | 0.96 | 1.10 | 21.6 |
| East | : Anzac F | Road | | | | | | | | | | | |
| 4 | L2 | 280 | 1.5 | 280 | 1.5 | 0.838 | 33.4 | LOS C | 8.0 | 57.7 | 1.00 | 1.00 | 14.1 |
| 6 | R2 | 287 | 4.0 | 287 | 4.0 | 0.875 | 36.1 | LOS C | 8.7 | 65.8 | 1.00 | 1.09 | 13.7 |
| Appr | oach | 567 | 2.8 | 567 | 2.8 | 0.875 | 34.8 | LOS C | 8.7 | 65.8 | 1.00 | 1.05 | 13.9 |
| North | n: Moore | bank Avenu | e | | | | | | | | | | |
| 7 | L2 | 419 | 3.0 | 419 | 3.0 | 0.361 | 5.5 | LOSA | 3.1 | 23.2 | 0.48 | 0.62 | 36.9 |
| 8 | T1 | 658 | 2.1 | 658 | 2.1 | 0.795 | 19.2 | LOS B | 12.8 | 93.4 | 0.93 | 0.91 | 15.0 |
| 9 | R2 | 59 | 100.0 | 59 | 100. 0 | 0.384 | 29.4 | LOS C | 1.5 | 19.0 | 0.95 | 0.76 | 31.5 |
| Appr | oach | 1136 | 7.5 | 1136 | 7.5 | 0.795 | 14.7 | LOS B | 12.8 | 93.4 | 0.76 | 0.79 | 24.1 |
| Wes | t: MPW S | Site Access | | | | | | | | | | | |
| 10 | L2 | 59 | 100.0 | 59 | 100. 0 | 0.125 | 12.6 | LOSA | 0.6 | 7.9 | 0.69 | 0.69 | 43.4 |
| Appr | oach | 59 | 100.0 | 59 | 100. 0 | 0.125 | 12.6 | LOSA | 0.6 | 7.9 | 0.69 | 0.69 | 43.4 |
| All V | ehicles | 2747 | 6.5 | 2747 | 6.5 | 0.908 | 23.4 | LOS B | 18.5 | 135.4 | 0.88 | 0.95 | 21.2 |

♦♦ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.0 %

Number of Iterations: 9 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | |
|--------|------------------------------------|--------|---------|---------|--------------|----------|--------|-----------|--|--|
| Mov | Description | Demand | Average | | Average Back | | Prop. | Effective | | |
| ID | Description | Flow | Delay | Service | Pedestrian | Distance | Queued | Stop Rate | | |
| | | ped/h | sec | | ped | m | | per ped | | |
| P1 | South Full Crossing | 11 | 19.4 | LOS B | 0.0 | 0.0 | 0.88 | 0.88 | | |
| P2 | East Full Crossing | 11 | 19.4 | LOS B | 0.0 | 0.0 | 0.88 | 0.88 | | |
| All Pe | destrians | 21 | 19.4 | LOS B | | | 0.88 | 0.88 | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: C [Moorebank Avenue_Anzac Road_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and Anzac Road AM PEAK

Signals - Fixed Time Isolated Cycle Time = 50 seconds (Practical Cycle Time)

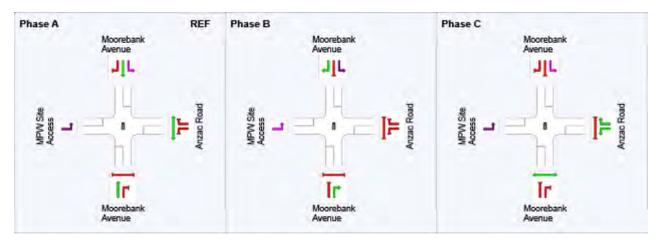
Phase Times determined by the program

Phase Sequence: 4 Phase Reference Phase: Phase A Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 0 | 22 | 35 |
| Green Time (sec) | 16 | 7 | 9 |
| Phase Time (sec) | 22 | 13 | 15 |
| Phase Split | 44% | 26% | 30% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase



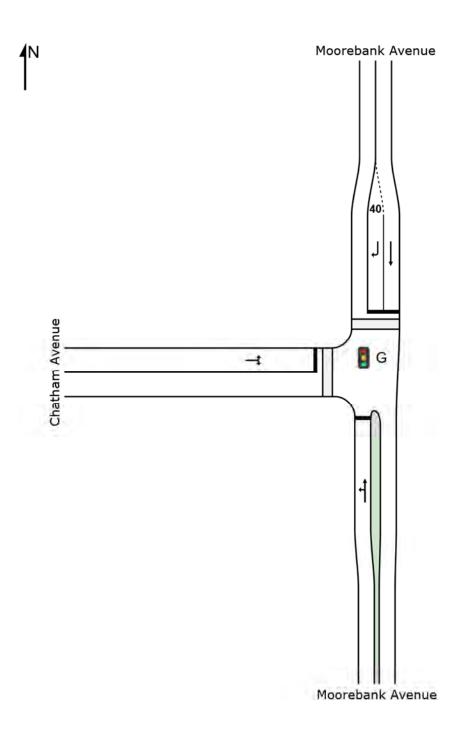
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Site: G [Moorebank Avenue/Chatham Avenue_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK
Signals - Fixed Time Isolated



Site: G [Moorebank Avenue/Chatham Avenue_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK

Signals - Fixed Time Isolated Cycle Time = 85 seconds (Practical Cycle Time)

| Move | Movement Performance - Vehicles | | | | | | | | | | | | |
|-----------|---------------------------------|--------------------------|-----|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | HV | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South | South: Moorebank Avenue | | | | | | | | | | | | |
| 1 | L2 | 1 | 0.0 | 1 | 0.0 | 0.896 | 30.2 | LOS C | 48.0 | 361.3 | 0.91 | 0.99 | 35.2 |
| 2 | T1 | 1103 | 3.7 | 1103 | 3.7 | 0.896 | 27.0 | LOS B | 48.0 | 361.3 | 0.91 | 0.99 | 32.0 |
| Appro | ach | 1104 | 3.7 | 1104 | 3.7 | 0.896 | 27.0 | LOS B | 48.0 | 361.3 | 0.91 | 0.99 | 32.0 |
| North | : Moorel | bank Avenue | Э | | | | | | | | | | |
| 8 | T1 | 457 | 9.2 | 457 | 9.2 | 0.315 | 2.7 | LOS A | 5.3 | 43.6 | 0.30 | 0.27 | 45.7 |
| 9 | R2 | 1 | 0.0 | 1 | 0.0 | 0.008 | 44.2 | LOS D | 0.0 | 0.3 | 0.95 | 0.58 | 25.2 |
| Appro | ach | 458 | 9.2 | 458 | 9.2 | 0.315 | 2.8 | LOSA | 5.3 | 43.6 | 0.30 | 0.27 | 45.6 |
| West: | Chatha | m Avenue | | | | | | | | | | | |
| 10 | L2 | 1 | 0.0 | 1 | 0.0 | 0.015 | 45.3 | LOS D | 0.1 | 0.6 | 0.95 | 0.61 | 12.7 |
| 12 | R2 | 1 | 0.0 | 1 | 0.0 | 0.015 | 45.3 | LOS D | 0.1 | 0.6 | 0.95 | 0.61 | 27.7 |
| Appro | ach | 2 | 0.0 | 2 | 0.0 | 0.015 | 45.3 | LOS D | 0.1 | 0.6 | 0.95 | 0.61 | 21.8 |
| All Ve | hicles | 1564 | 5.3 | 1564 | 5.3 | 0.896 | 20.0 | LOS B | 48.0 | 361.3 | 0.74 | 0.78 | 37.1 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 %

Number of Iterations: 9 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | |
|-----------|------------------------------------|-------------------------|-------------------------|---------------------|-----------------------------------|---------------------------|-----------------|-----------------------------------|--|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back Pedestrian ped | of Queue Distance m | Prop. Queued | Effective Stop Rate per ped | | |
| P3 | North Full Crossing | 11 | 36.7 | LOS D | 0.0 | 0.0 | 0.93 | 0.93 | | |
| P4 | West Full Crossing | 11 | 8.1 | LOS A | 0.0 | 0.0 | 0.44 | 0.44 | | |
| All Pe | destrians | 21 | 22.4 | LOS C | | | 0.68 | 0.68 | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: G [Moorebank Avenue/Chatham Avenue_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK

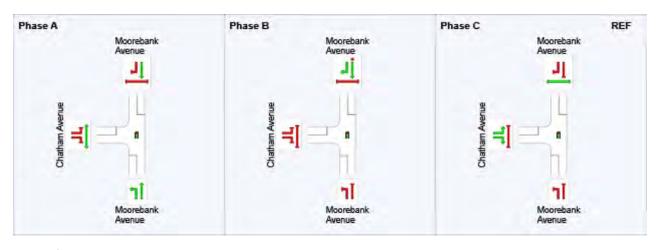
Signals - Fixed Time Isolated Cycle Time = 85 seconds (Practical Cycle Time)

Phase Times determined by the program Green Split Priority applies **Phase Sequence: Opposed Turns** Reference Phase: Phase C Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

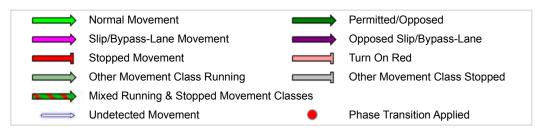
Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 12 | 73 | 0 |
| Green Time (sec) | 55 | 6 | 6 |
| Phase Time (sec) | 61 | 12 | 12 |
| Phase Split | 72% | 14% | 14% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

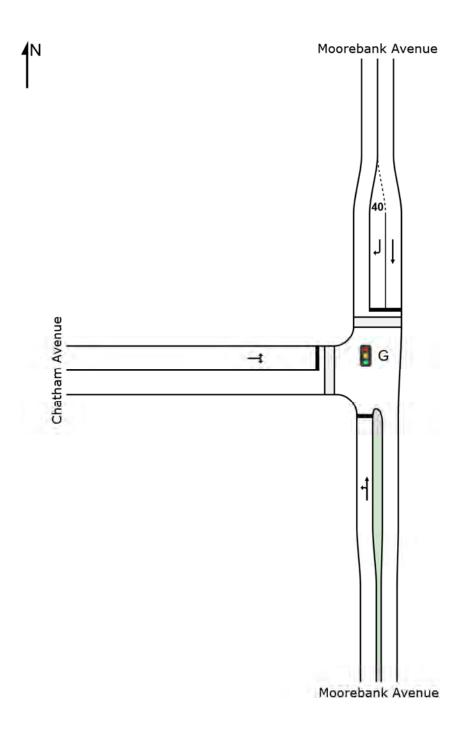


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Site: G [Moorebank Avenue/Chatham Avenue_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK
Signals - Fixed Time Isolated



Site: G [Moorebank Avenue/Chatham Avenue_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK

| Move | Movement Performance - Vehicles | | | | | | | | | | | | |
|-----------|---------------------------------|----------------------------|----------|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | HV | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South | South: Moorebank Avenue | | | | | | | | | | | | |
| 1 | L2 | 1 | 0.0 | 1 | 0.0 | 0.905 | 34.6 | LOS C | 15.0 | 109.5 | 1.00 | 1.18 | 33.2 |
| 2 | T1 | 501 | 2.3 | 501 | 2.3 | 0.905 | 31.4 | LOS C | 15.0 | 109.5 | 1.00 | 1.18 | 29.7 |
| Appro | ach | 502 | 2.3 | 502 | 2.3 | 0.905 | 31.4 | LOS C | 15.0 | 109.5 | 1.00 | 1.18 | 29.7 |
| North: | Moorel | bank Avenue | ; | | | | | | | | | | |
| 8 | T1 | 955 | 1.2 | 955 | 1.2 | 0.889 | 20.8 | LOS B | 26.5 | 190.3 | 0.95 | 1.14 | 37.8 |
| 9 | R2 | 1 | 0.0 | 1 | 0.0 | 0.004 | 22.2 | LOS B | 0.0 | 0.1 | 0.88 | 0.57 | 31.3 |
| Appro | ach | 956 | 1.2 | 956 | 1.2 | 0.889 | 20.8 | LOS B | 26.5 | 190.3 | 0.95 | 1.14 | 37.7 |
| West: | Chatha | m Avenue | | | | | | | | | | | |
| 10 | L2 | 288 | 0.0 | 288 | 0.0 | 0.806 | 28.3 | LOS B | 7.2 | 50.5 | 1.00 | 0.99 | 17.7 |
| 12 | R2 | 1 | 0.0 | 1 | 0.0 | 0.806 | 28.3 | LOS B | 7.2 | 50.5 | 1.00 | 0.99 | 34.0 |
| Appro | ach | 289 | 0.0 | 289 | 0.0 | 0.806 | 28.3 | LOS B | 7.2 | 50.5 | 1.00 | 0.99 | 17.8 |
| All Ve | hicles | 1747 | 1.3 | 1747 | 1.3 | 0.905 | 25.1 | LOS B | 26.5 | 190.3 | 0.97 | 1.13 | 34.2 |

♦♦ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.0 %

Number of Iterations: 9 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | |
|-----------|------------------------------------|-------------------------|-------------------------|-------|-----------------------------------|---------------------------|-----------------|-----------------------------------|--|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | | Average Back Pedestrian ped | of Queue Distance m | Prop. Queued | Effective Stop Rate per ped | | |
| P3 | North Full Crossing | 11 | 16.9 | LOS B | 0.0 | 0.0 | 0.87 | 0.87 | | |
| P4 | West Full Crossing | 11 | 16.9 | LOS B | 0.0 | 0.0 | 0.87 | 0.87 | | |
| All Pe | destrians | 21 | 16.9 | LOS B | | | 0.87 | 0.87 | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: G [Moorebank Avenue/Chatham Avenue_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK

Signals - Fixed Time Isolated Cycle Time = 45 seconds (Practical Cycle Time)

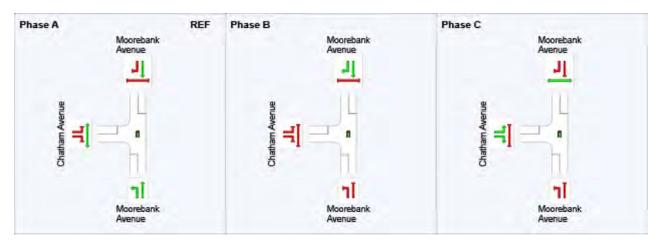
Phase Times determined by the program Green Split Priority applies **Phase Sequence: Opposed Turns** Reference Phase: Phase A Input Phase Sequence: A, B, C

Output Phase Sequence: A, B, C

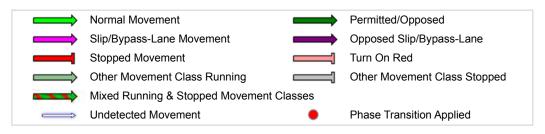
Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 0 | 19 | 31 |
| Green Time (sec) | 13 | 6 | 8 |
| Phase Time (sec) | 19 | 12 | 14 |
| Phase Split | 42% | 27% | 31% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase



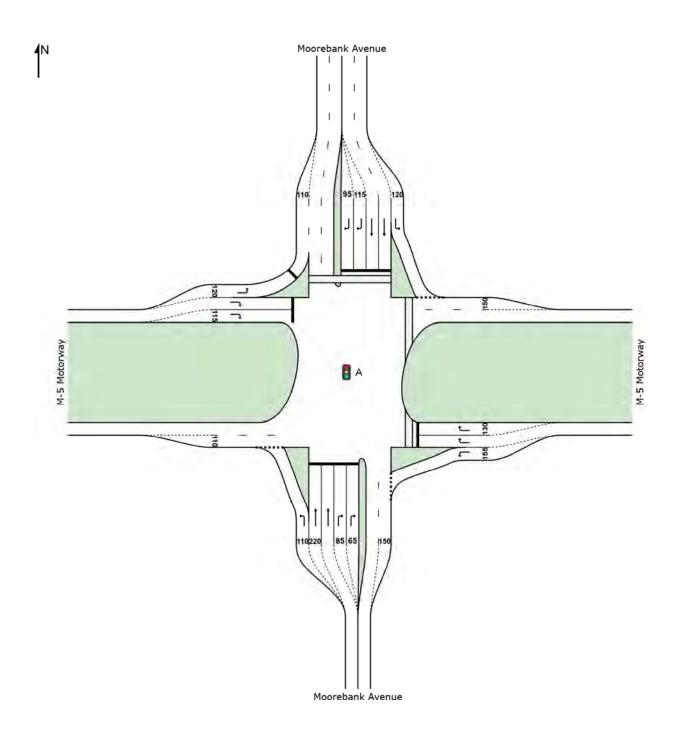
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\Scenario 2\Scenario 2_Stage 2_100%.sip7

Stage 3(i)

Site: A [M5/Moorebank Avenue_AM]

Intersection of Moorebank Avenue and M5 Motorway AM PEAK
Signals - Fixed Time Isolated



Site: A [M5/Moorebank Avenue_AM]

Intersection of Moorebank Avenue and M5 Motorway

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|-----------|-----------------|---------|------------------|---------|--------------|------------------|---------------------|----------------------|----------------------|--------|---------------------|---------|
| | | | | | | | | | | | | | |
| Mov ID | OD Mov | Demand Total | Flows | Arrival Total | Flows | Deg. Satn | Average Delav | Level of Service | 95% Back Vehicles | of Queue Distance | Prop. | Effective Stop Rate | |
| טו | IVIOV | veh/h | пv % | veh/h | пv % | v/c | sec | Service | venicies | Distance | Queued | per veh | km/h |
| South | : Moorel | oank Avenu | | VO11/11 | /0 | V/ O | 300 | | VOII | | | per veri | KIII/II |
| 1 | L2 | 428 | 14.7 | 428 | 14.7 | 0.396 | 14.4 | LOSA | 9.9 | 89.8 | 0.42 | 0.73 | 50.3 |
| 2 | T1 | 402 | 3.4 | 402 | 3.4 | 0.252 | 29.2 | LOS C | 9.3 | 69.3 | 0.68 | 0.58 | 34.6 |
| 3 | R2 | 271 | 20.2 | 271 | 20.2 | 0.441 | 57.9 | LOS E | 9.3 | 91.2 | 0.89 | 0.80 | 26.2 |
| Appro | ach | 1101 | 12.0 | 1101 | 12.0 | 0.441 | 30.5 | LOS C | 9.9 | 91.2 | 0.63 | 0.69 | 36.9 |
| East: | M-5 Mot | orway | | | | | | | | | | | |
| 4 | L2 | 339 | 17.7 | 339 | 17.7 | 0.285 | 6.4 | LOSA | 2.0 | 18.9 | 0.14 | 0.59 | 47.5 |
| 6 | R2 | 243 | 4.3 | 243 | 4.3 | 0.949 | 104.0 | LOS F | 10.7 | 81.6 | 1.00 | 1.05 | 17.1 |
| Appro | ach | 582 | 12.1 | 582 | 12.1 | 0.949 | 47.1 | LOS D | 10.7 | 81.6 | 0.50 | 0.78 | 23.9 |
| North | : Mooreb | ank Avenu | е | | | | | | | | | | |
| 7 | L2 | 48 | 19.6 | 48 | 19.6 | 0.042 | 7.3 | LOSA | 0.5 | 4.7 | 0.18 | 0.58 | 52.8 |
| 8 | T1 | 218 | 6.8 | 218 | 6.8 | 0.156 | 27.7 | LOS B | 5.2 | 41.5 | 0.65 | 0.53 | 24.6 |
| 9 | R2 | 506 | 20.2 | 506 | 20.2 | 0.967 | 87.5 | LOS F | 28.7 | 282.0 | 0.98 | 0.98 | 22.1 |
| Appro | ach | 773 | 16.3 | 773 | 16.3 | 0.967 | 65.6 | LOS E | 28.7 | 282.0 | 0.83 | 0.82 | 23.3 |
| West: | M-5 Mo | torway | | | | | | | | | | | |
| 10 | L2 | 1356 | 7.6 | 1356 | 7.6 | 0.887 | 7.1 | LOSA | 21.5 | 173.2 | 0.48 | 0.66 | 50.5 |
| 12 | R2 | 521 | 9.7 | 521 | 9.7 | 0.812 | 68.9 | LOS E | 20.8 | 173.8 | 0.99 | 0.88 | 18.9 |
| Appro | ach | 1877 | 8.2 | 1877 | 8.2 | 0.887 | 24.3 | LOS B | 21.5 | 173.8 | 0.62 | 0.72 | 38.0 |
| All Ve | hicles | 4333 | 11.1 | 4333 | 11.1 | 0.967 | 36.3 | LOS C | 28.7 | 282.0 | 0.64 | 0.74 | 32.4 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 1.0 %

Number of Iterations: 13 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | | |
|-----------|------------------------------------|----------------|------------------|---------------------|----------------------------|----------------------|-----------------|------------------------|--|--|--|
| Mov ID | Description | Demand Flow | Average Delay | Level of Service | Average Back Pedestrian | of Queue Distance | Prop. Queued | Effective Stop Rate | | | |
| טו | · · · · · · · · · · · · · · · | ped/h | sec | Service | ped | m | Queueu | per ped | | | |
| P21 | East Stage 1 | 26 | 64.5 | LOS F | 0.1 | 0.1 | 0.93 | 0.93 | | | |
| P22 | East Stage 2 | 26 | 68.2 | LOS F | 0.1 | 0.1 | 0.95 | 0.95 | | | |
| P3 | North Full Crossing | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 | | | |
| All Pe | destrians | 79 | 67.3 | LOS F | | | 0.95 | 0.95 | | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: A [M5/Moorebank Avenue_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and M5 Motorway AM PEAK

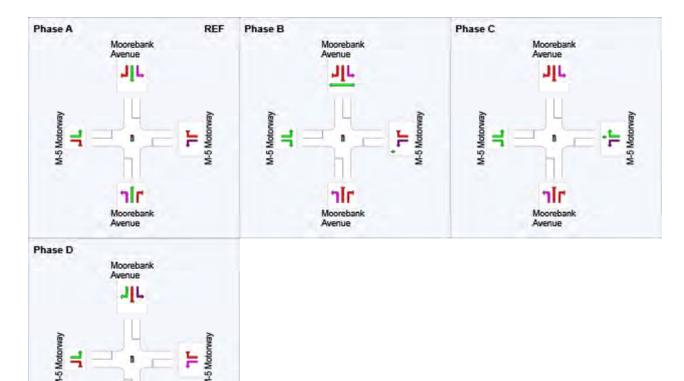
Phase Times determined by the program

Phase Sequence: 4-phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|-----|-----|
| Phase Change Time (sec) | 0 | 70 | 91 | 108 |
| Green Time (sec) | 64 | 15 | 11 | 36 |
| Phase Time (sec) | 70 | 21 | 17 | 42 |
| Phase Split | 47% | 14% | 11% | 28% |

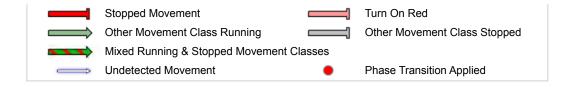
See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

Moorebank Avenue

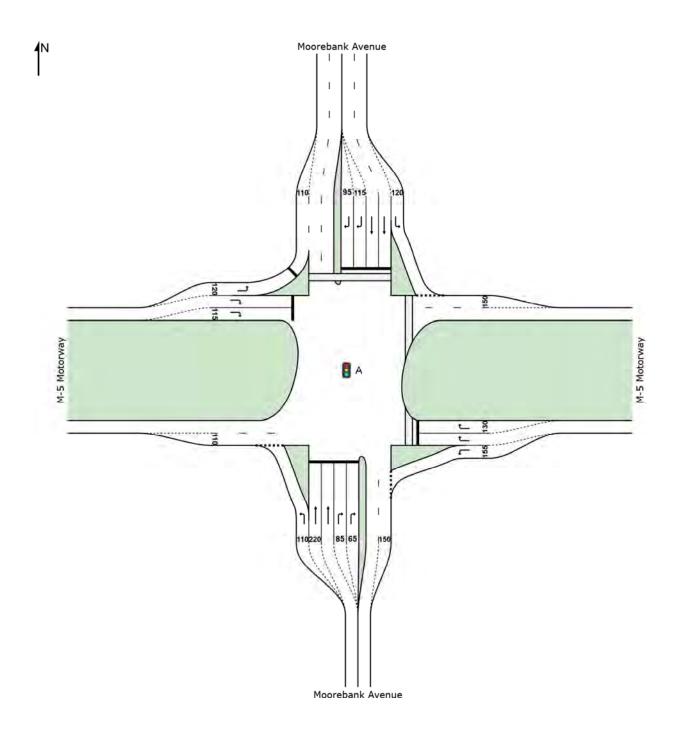




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Site: A [M5/Moorebank Avenue_PM]

Intersection of Moorebank Avenue and M5 Motorway PM PEAK
Signals - Fixed Time Isolated



Site: A [M5/Moorebank Avenue_PM]

Intersection of Moorebank Avenue and M5 Motorway

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|----------|----------------|---------|----------------|---------|-------------|--------------|----------|-----------------|---------------|--------|-------------------|---------------|
| | | | | | | | | | | | | | |
| Mov | OD | Demand | | Arrival | | Deg. | Average | Level of | 95% Back | | Prop. | Effective | |
| ID | Mov | Total veh/h | HV % | Total veh/h | HV % | Satn v/c | Delay sec | Service | Vehicles veh | Distance m | Queued | Stop Rate per veh | Speed km/h |
| South | : Moorel | bank Avenu | | VCII/II | /0 | V/C | 300 | | VEII | - ''' | | per veri | KIII/II |
| 1 | L2 | 541 | 7.4 | 541 | 7.4 | 0.761 | 42.8 | LOS D | 26.3 | 211.1 | 0.92 | 1.05 | 35.1 |
| 2 | T1 | 286 | 2.6 | 286 | 2.6 | 0.567 | 66.8 | LOSE | 10.0 | 73.6 | 0.99 | 0.80 | |
| 3 | R2 | 404 | 8.9 | 404 | 8.9 | 0.239 | 22.6 | LOS B | 7.9 | 65.3 | 0.53 | 0.72 | 41.6 |
| | | | | | | | | | | | | | |
| Appro | ach | 1232 | 6.8 | 1232 | 6.8 | 0.761 | 41.7 | LOS C | 26.3 | 211.1 | 0.81 | 0.88 | 33.0 |
| East: | M-5 Mot | orway | | | | | | | | | | | |
| 4 | L2 | 281 | 12.7 | 281 | 12.7 | 0.239 | 7.1 | LOSA | 2.9 | 25.6 | 0.20 | 0.61 | 46.3 |
| 6 | R2 | 87 | 6.0 | 87 | 6.0 | 0.642 | 89.0 | LOS F | 3.4 | 26.9 | 1.00 | 0.78 | 19.0 |
| Appro | ach | 368 | 11.1 | 368 | 11.1 | 0.642 | 26.5 | LOS B | 3.4 | 26.9 | 0.39 | 0.65 | 30.7 |
| | | | | | | | | | | | | | |
| | | ank Avenue | | | | | | | | | | | |
| 7 | L2 | 74 | 5.7 | 74 | 5.7 | 0.065 | 6.8 | LOSA | 0.6 | 4.8 | 0.16 | 0.59 | 55.9 |
| 8 | T1 | 405 | 1.8 | 405 | 1.8 | 0.864 | 74.2 | LOS F | 17.4 | 126.4 | 1.00 | 0.92 | 12.4 |
| 9 | R2 | 1296 | 4.5 | 1296 | 4.5 | 0.884 | 35.2 | LOS C | 46.1 | 352.4 | 0.76 | 0.85 | 38.0 |
| Appro | ach | 1775 | 4.0 | 1775 | 4.0 | 0.884 | 42.9 | LOS D | 46.1 | 352.4 | 0.79 | 0.85 | 31.6 |
| West: | M-5 Mo | torway | | | | | | | | | | | |
| 10 | L2 | 595 | 7.3 | 595 | 7.3 | 0.387 | 6.1 | LOSA | 2.8 | 22.5 | 0.13 | 0.56 | 52.0 |
| 12 | R2 | 439 | 9.6 | 439 | 9.6 | 0.810 | 72.7 | LOS F | 17.7 | 148.0 | 1.00 | 0.88 | 18.2 |
| Appro | ach | 1034 | 8.2 | 1034 | 8.2 | 0.810 | 34.4 | LOS C | 17.7 | 148.0 | 0.50 | 0.70 | 32.7 |
| All Ve | hicles | 4408 | 6.4 | 4408 | 6.4 | 0.884 | 39.2 | LOS C | 46.1 | 352.4 | 0.69 | 0.81 | 32.2 |

+ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.5 %

Number of Iterations: 5 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | | |
|--------|------------------------------------|--------|---------|---------|--------------|----------|--------|-----------|--|--|--|
| Mov | D | Demand | Average | | Average Back | | Prop. | Effective | | | |
| ID | Description | Flow | Delay | Service | Pedestrian | Distance | Queued | Stop Rate | | | |
| | | ped/h | sec | | ped | m | | per ped | | | |
| P21 | East Stage 1 | 26 | 64.5 | LOS F | 0.1 | 0.1 | 0.93 | 0.93 | | | |
| P22 | East Stage 2 | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 | | | |
| P3 | North Full Crossing | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 | | | |
| All Pe | destrians | 79 | 67.6 | LOS F | | | 0.95 | 0.95 | | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: A [M5/Moorebank Avenue_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and M5 Motorway

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

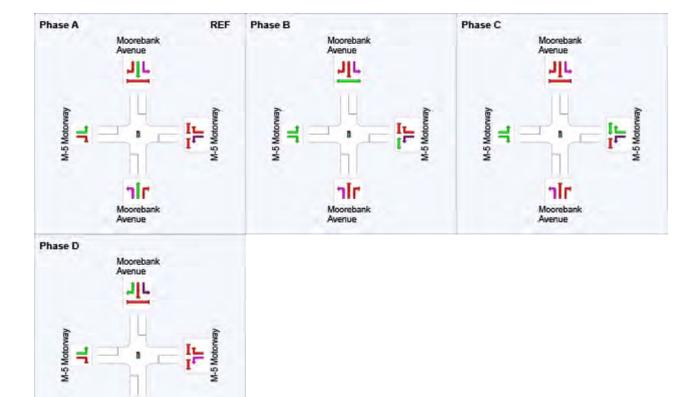
Phase Times determined by the program

Phase Sequence: 4-phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|----|-----|
| Phase Change Time (sec) | 0 | 26 | 47 | 59 |
| Green Time (sec) | 20 | 15 | 6 | 85 |
| Phase Time (sec) | 26 | 21 | 12 | 91 |
| Phase Split | 17% | 14% | 8% | 61% |

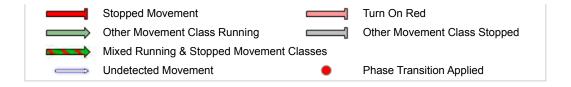
See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

Moorebank Avenue

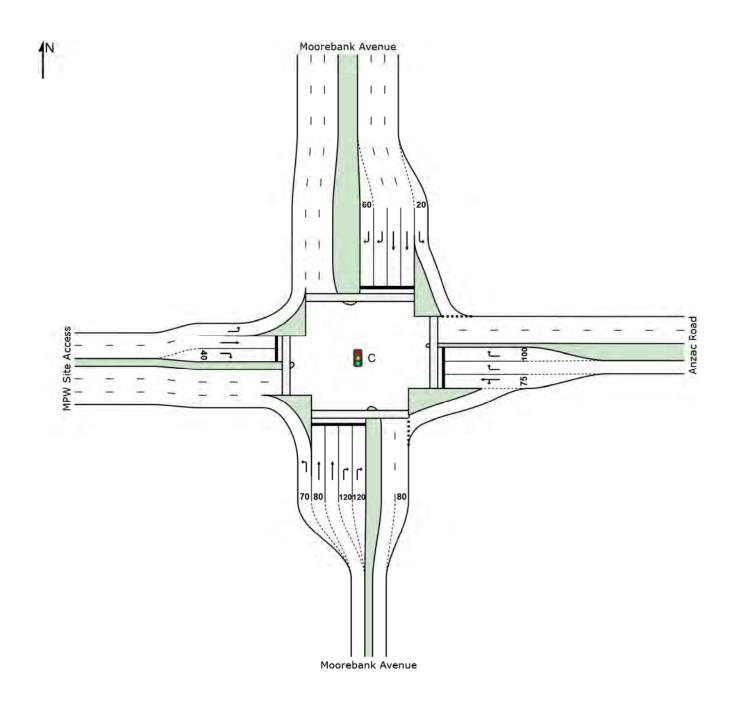




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Site: C [Moorebank Avenue_Anzac Road_AM]

Intersection of Moorebank Avenue and Anzac Road AM PEAK
Signals - Fixed Time Isolated



Site: C [Moorebank Avenue_Anzac Road_AM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 60 seconds (Practical Cycle Time)

| Mov | ement l | Performar | nce - Ve | hicles | ; | | | | | | | | |
|-----------|-----------|--------------------------|----------|--------------------------|--------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | HV | Arriva Total veh/h | I Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South | n: Moore | bank Aveni | ıe | | | | | | | | | | |
| 1 | L2 | 37 | 0.0 | 37 | 0.0 | 0.020 | 5.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.53 | 53.9 |
| 2 | T1 | 727 | 8.0 | 727 | 8.0 | 0.832 | 29.9 | LOS C | 12.0 | 97.3 | 1.00 | 1.09 | 19.7 |
| 3 | R2 | 381 | 3.3 | 381 | 3.3 | 0.780 | 35.1 | LOS C | 6.2 | 46.1 | 1.00 | 0.98 | 22.9 |
| Appro | oach | 1145 | 6.2 | 1145 | 6.2 | 0.832 | 30.9 | LOS C | 12.0 | 97.3 | 0.97 | 1.03 | 21.9 |
| East: | Anzac F | Road | | | | | | | | | | | |
| 4 | L2 | 208 | 3.0 | 208 | 3.0 | 0.209 | 8.7 | LOSA | 1.9 | 14.3 | 0.45 | 0.67 | 32.9 |
| 5 | T1 | 1 | 0.0 | 1 | 0.0 | 0.209 | 3.1 | LOSA | 1.9 | 14.3 | 0.45 | 0.67 | 49.3 |
| 6 | R2 | 363 | 11.9 | 363 | 11.9 | 0.787 | 38.0 | LOS C | 5.9 | 51.5 | 1.00 | 0.95 | 13.4 |
| Appro | oach | 573 | 8.6 | 573 | 8.6 | 0.787 | 27.3 | LOS B | 5.9 | 51.5 | 0.80 | 0.85 | 17.1 |
| North | : Moore | bank Avenu | ie | | | | | | | | | | |
| 7 | L2 | 403 | 7.8 | 403 | 7.8 | 0.320 | 5.3 | LOSA | 3.1 | 25.0 | 0.41 | 0.59 | 36.7 |
| 8 | T1 | 506 | 12.9 | 506 | 12.9 | 0.801 | 26.2 | LOS B | 10.8 | 95.2 | 0.96 | 0.93 | 12.6 |
| 9 | R2 | 172 | 17.2 | 172 | 17.2 | 0.385 | 33.1 | LOS C | 2.5 | 19.8 | 0.95 | 0.76 | 31.2 |
| Appro | oach | 1081 | 11.7 | 1081 | 11.7 | 0.801 | 19.5 | LOS B | 10.8 | 95.2 | 0.76 | 0.78 | 24.3 |
| West | : MPW S | Site Access | | | | | | | | | | | |
| 10 | L2 | 29 | 100.0 | 29 | 100. 0 | 0.027 | 6.1 | LOSA | 0.0 | 0.0 | 0.00 | 0.50 | 51.0 |
| 11 | T1 | 1 | 0.0 | 1 | 0.0 | 0.005 | 26.9 | LOS B | 0.0 | 0.2 | 0.92 | 0.54 | 36.3 |
| 12 | R2 | 3 | 33.3 | 3 | 33.3 | 0.021 | 33.6 | LOS C | 0.1 | 0.8 | 0.92 | 0.62 | 29.2 |
| Appro | oach | 34 | 90.6 | 34 | 90.6 | 0.027 | 9.3 | LOSA | 0.1 | 8.0 | 0.12 | 0.51 | 47.0 |
| All Ve | ehicles | 2833 | 9.8 | 2833 | 9.8 | 0.832 | 25.6 | LOS B | 12.0 | 97.3 | 0.84 | 0.89 | 22.1 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 1.0 %

Number of Iterations: 13 (maximum specified: 20)

| Move | ment Performance - Pedes | trians | | | | | | |
|------|--------------------------|--------|---------|---------|--------------|----------|--------|-----------|
| Mov | Description | Demand | Average | | Average Back | | Prop. | Effective |
| ID | Description | Flow | Delay | Service | | Distance | Queued | Stop Rate |
| | | ped/h | sec | | ped | m | | per ped |
| P1 | South Full Crossing | 11 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| P2 | East Full Crossing | 11 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| P3 | North Full Crossing | 11 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| P4 | West Full Crossing | 53 | 24.4 | LOS C | 0.1 | 0.1 | 0.90 | 0.90 |

All Pedestrians 84 24.3 LOS C 0.90 0.90

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: C [Moorebank Avenue_Anzac Road_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 60 seconds (Practical Cycle Time)

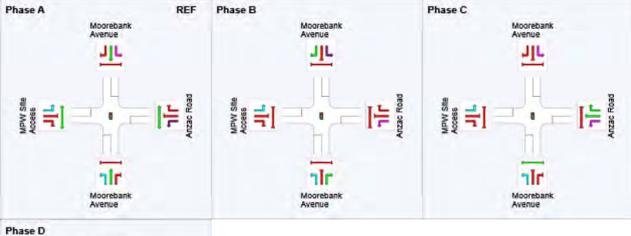
Phase Times determined by the program

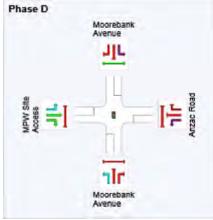
Phase Sequence: 4 Phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|-----|-----|
| Phase Change Time (sec) | 0 | 20 | 34 | 48 |
| Green Time (sec) | 14 | 8 | 8 | 6 |
| Phase Time (sec) | 20 | 14 | 14 | 12 |
| Phase Split | 33% | 23% | 23% | 20% |

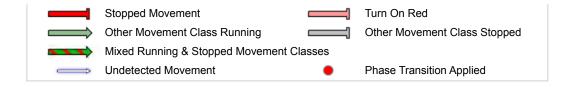
See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.





REF: Reference Phase VAR: Variable Phase

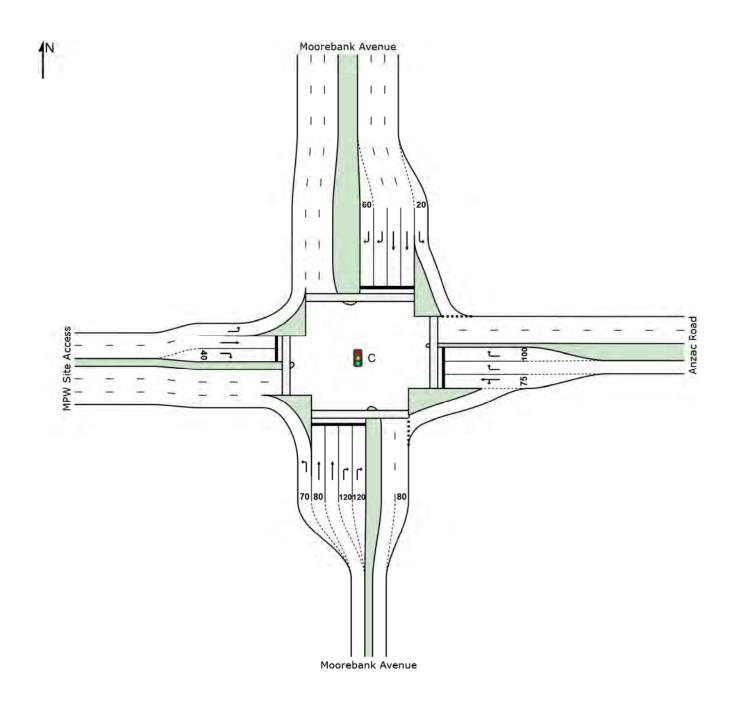




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Site: C [Moorebank Avenue_Anzac Road_PM]

Intersection of Moorebank Avenue and Anzac Road PM PEAK
Signals - Fixed Time Isolated



Site: C [Moorebank Avenue_Anzac Road_PM]

Intersection of Moorebank Avenue and Anzac Road

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 70 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|-----------|----------------|---------|----------------|---------|-------------|--------------|----------|-----------------|---------------|--------|-------------------|---------------|
| Mov | OD | Demand | | | Flows | Deg. | Average | Level of | | of Queue | Prop. | Effective | |
| ID | Mov | Total veh/h | HV % | Total veh/h | HV % | Satn v/c | Delay sec | Service | Vehicles veh | Distance m | Queued | Stop Rate per veh | Speed km/h |
| South | n: Moorel | bank Avenu | | VO11//11 | ,, | V/ O | 555 | | 7011 | | | por vori | 1011/11 |
| 1 | L2 | 6 | 83.3 | 6 | 83.3 | 0.005 | 6.5 | LOSA | 0.0 | 0.0 | 0.00 | 0.49 | 50.5 |
| 2 | T1 | 667 | 6.0 | 667 | 6.0 | 0.560 | 21.9 | LOS B | 9.7 | 75.9 | 0.88 | 0.75 | 22.8 |
| 3 | R2 | 196 | 0.5 | 196 | 0.5 | 0.611 | 39.8 | LOS C | 3.5 | 25.0 | 1.00 | 0.82 | 21.6 |
| Appro | oach | 869 | 5.3 | 869 | 5.3 | 0.611 | 25.9 | LOS B | 9.7 | 75.9 | 0.90 | 0.77 | 22.6 |
| East: | Anzac R | load | | | | | | | | | | | |
| 4 | L2 | 280 | 1.5 | 280 | 1.5 | 0.301 | 11.0 | LOSA | 4.0 | 28.9 | 0.52 | 0.71 | 29.4 |
| 5 | T1 | 1 | 0.0 | 1 | 0.0 | 0.301 | 5.4 | LOSA | 4.0 | 28.9 | 0.52 | 0.71 | 47.1 |
| 6 | R2 | 287 | 4.0 | 287 | 4.0 | 0.460 | 35.0 | LOS C | 4.6 | 34.9 | 0.94 | 0.79 | 14.2 |
| Appro | oach | 568 | 2.8 | 568 | 2.8 | 0.460 | 23.1 | LOS B | 4.6 | 34.9 | 0.74 | 0.75 | 19.1 |
| North | : Mooreb | ank Avenu | е | | | | | | | | | | |
| 7 | L2 | 419 | 3.0 | 419 | 3.0 | 0.294 | 4.5 | LOSA | 2.7 | 19.8 | 0.32 | 0.55 | 38.3 |
| 8 | T1 | 664 | 6.3 | 664 | 6.3 | 0.749 | 23.9 | LOS B | 14.7 | 116.0 | 0.91 | 0.83 | 13.4 |
| 9 | R2 | 41 | 71.8 | 41 | 71.8 | 0.193 | 41.3 | LOS C | 0.7 | 8.1 | 0.96 | 0.71 | 27.2 |
| Appro | oach | 1124 | 7.5 | 1124 | 7.5 | 0.749 | 17.3 | LOS B | 14.7 | 116.0 | 0.69 | 0.72 | 21.9 |
| West | : MPW S | ite Access | | | | | | | | | | | |
| 10 | L2 | 159 | 18.5 | 159 | 18.5 | 0.097 | 5.8 | LOS A | 0.0 | 0.0 | 0.00 | 0.52 | 51.0 |
| 11 | T1 | 15 | 0.0 | 15 | 0.0 | 0.088 | 33.5 | LOS C | 0.5 | 3.4 | 0.95 | 0.65 | 33.1 |
| 12 | R2 | 22 | 0.0 | 22 | 0.0 | 0.139 | 39.5 | LOS C | 0.7 | 5.2 | 0.96 | 0.70 | 26.7 |
| Appro | oach | 196 | 15.1 | 196 | 15.1 | 0.139 | 11.7 | LOSA | 0.7 | 5.2 | 0.18 | 0.55 | 44.3 |
| All Ve | ehicles | 2758 | 6.4 | 2758 | 6.4 | 0.749 | 20.8 | LOS B | 14.7 | 116.0 | 0.73 | 0.73 | 23.4 |

♦♦ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.5 %

Number of Iterations: 5 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | | |
|-----------|------------------------------------|-------------------------|-------------------------|---------------------|-----------------------------------|---------------------------|-----------------|-----------------------------------|--|--|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back Pedestrian ped | of Queue Distance m | Prop. Queued | Effective Stop Rate per ped | | | |
| P1 | South Full Crossing | 11 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | | | |
| P2 | East Full Crossing | 11 | 28.4 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 | | | |
| P3 | North Full Crossing | 11 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | | | |
| P4 | West Full Crossing | 53 | 28.4 | LOS C | 0.1 | 0.1 | 0.90 | 0.90 | | | |
| All Pe | destrians | 84 | 28.6 | LOS C | | | 0.90 | 0.90 | | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: C [Moorebank Avenue_Anzac Road_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and Anzac Road

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 70 seconds (Practical Cycle Time)

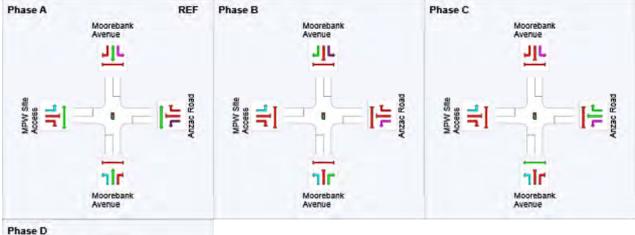
Phase Times determined by the program

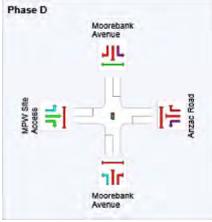
Phase Sequence: 4 Phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|-----|-----|
| Phase Change Time (sec) | 0 | 28 | 40 | 58 |
| Green Time (sec) | 22 | 6 | 12 | 6 |
| Phase Time (sec) | 28 | 12 | 18 | 12 |
| Phase Split | 40% | 17% | 26% | 17% |

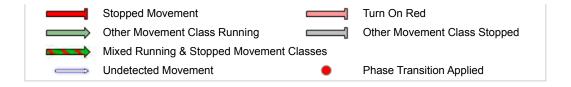
See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.





REF: Reference Phase VAR: Variable Phase

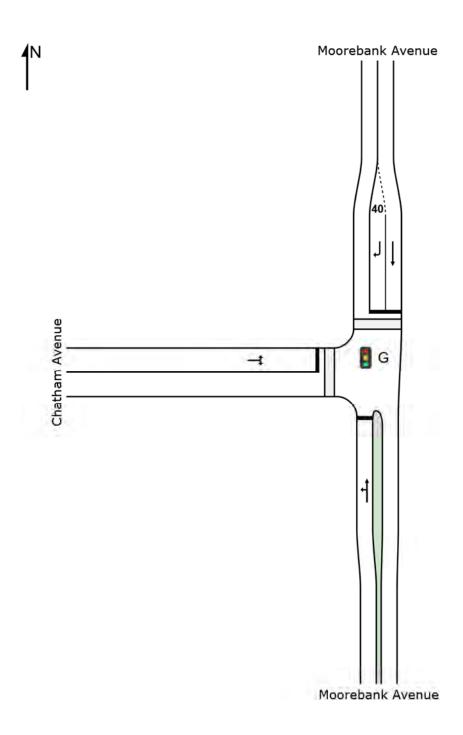




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Site: G [Moorebank Avenue/Chatham Avenue_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK
Signals - Fixed Time Isolated



Site: G [Moorebank Avenue/Chatham Avenue_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|-------------------------|--------------------------|-------|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | HV | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South | South: Moorebank Avenue | | | | | | | | | | | | |
| 1 | L2 | 1 | 0.0 | 1 | 0.0 | 0.896 | 30.2 | LOS C | 48.0 | 361.3 | 0.91 | 0.99 | 35.2 |
| 2 | T1 | 1103 | 3.7 | 1103 | 3.7 | 0.896 | 27.0 | LOS B | 48.0 | 361.3 | 0.91 | 0.99 | 32.0 |
| Appro | oach | 1104 | 3.7 | 1104 | 3.7 | 0.896 | 27.0 | LOS B | 48.0 | 361.3 | 0.91 | 0.99 | 32.0 |
| North | : Moore | bank Avenu | e | | | | | | | | | | |
| 8 | T1 | 457 | 9.2 | 457 | 9.2 | 0.315 | 2.7 | LOSA | 5.3 | 43.6 | 0.30 | 0.27 | 45.7 |
| 9 | R2 | 29 | 100.0 | 29 | 100. 0 | 0.385 | 49.3 | LOS D | 1.3 | 27.5 | 0.99 | 0.73 | 23.9 |
| Appro | oach | 486 | 14.7 | 486 | 14.7 | 0.385 | 5.5 | LOSA | 5.3 | 43.6 | 0.34 | 0.30 | 43.9 |
| West | : Chatha | m Avenue | | | | | | | | | | | |
| 10 | L2 | 29 | 100.0 | 29 | 100. 0 | 0.361 | 50.1 | LOS D | 1.3 | 27.5 | 0.99 | 0.73 | 11.9 |
| 12 | R2 | 1 | 0.0 | 1 | 0.0 | 0.361 | 49.6 | LOS D | 1.3 | 27.5 | 0.99 | 0.73 | 26.5 |
| Appro | oach | 31 | 96.6 | 31 | 96.6 | 0.361 | 50.1 | LOS D | 1.3 | 27.5 | 0.99 | 0.73 | 12.7 |
| All Ve | hicles | 1621 | 8.8 | 1621 | 8.8 | 0.896 | 21.0 | LOS B | 48.0 | 361.3 | 0.74 | 0.78 | 36.3 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 1.0 %

Number of Iterations: 13 (maximum specified: 20)

| Movement Performance - Pedestrians | | | | | | | | | | |
|------------------------------------|---------------------|----------------|------------------|-------|----------------------------|----------------------|-----------------|------------------------|--|--|
| Mov ID | Description | Demand Flow | Average Delay | | Average Back Pedestrian | of Queue Distance | Prop. Queued | Effective Stop Rate | | |
| | | ped/h | sec | | ped | m | | per ped | | |
| P3 | North Full Crossing | 11 | 36.7 | LOS D | 0.0 | 0.0 | 0.93 | 0.93 | | |
| P4 | West Full Crossing | 11 | 8.1 | LOS A | 0.0 | 0.0 | 0.44 | 0.44 | | |
| All Pe | destrians | 21 | 22.4 | LOS C | | | 0.68 | 0.68 | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: G [Moorebank Avenue/Chatham Avenue_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK

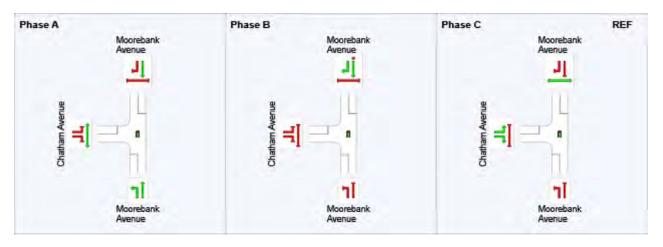
Signals - Fixed Time Isolated Cycle Time = 85 seconds (Practical Cycle Time)

Phase Times determined by the program Green Split Priority applies **Phase Sequence: Opposed Turns** Reference Phase: Phase C Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 12 | 73 | 0 |
| Green Time (sec) | 55 | 6 | 6 |
| Phase Time (sec) | 61 | 12 | 12 |
| Phase Split | 72% | 14% | 14% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



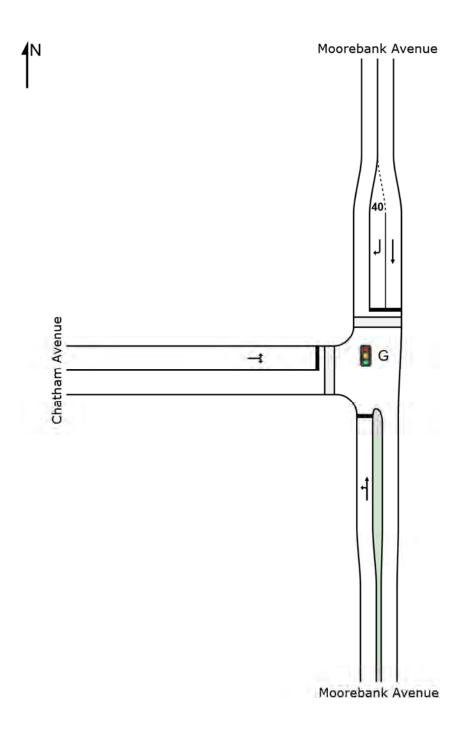
REF: Reference Phase VAR: Variable Phase



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Site: G [Moorebank Avenue/Chatham Avenue_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK Signals - Fixed Time Isolated



Site: G [Moorebank Avenue/Chatham Avenue_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK

Signals - Fixed Time Isolated Cycle Time = 45 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|-------------------------|--------------------------|-------|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | HV | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South | South: Moorebank Avenue | | | | | | | | | | | | |
| 1 | L2 | 1 | 0.0 | 1 | 0.0 | 0.784 | 23.9 | LOS B | 11.6 | 85.1 | 0.96 | 0.96 | 38.6 |
| 2 | T1 | 501 | 2.3 | 501 | 2.3 | 0.784 | 20.7 | LOS B | 11.6 | 85.1 | 0.96 | 0.96 | 35.9 |
| Appro | Approach | | 2.3 | 502 | 2.3 | 0.784 | 20.7 | LOS B | 11.6 | 85.1 | 0.96 | 0.96 | 35.9 |
| North | : Moore | bank Avenu | ie | | | | | | | | | | |
| 8 | T1 | 955 | 1.2 | 955 | 1.2 | 0.850 | 14.9 | LOS B | 22.1 | 158.5 | 0.85 | 0.96 | 40.1 |
| 9 | R2 | 29 | 100.0 | 29 | 100. 0 | 0.204 | 24.8 | LOS B | 0.6 | 13.6 | 0.92 | 0.71 | 30.1 |
| Appro | ach | 984 | 4.2 | 984 | 4.2 | 0.850 | 15.2 | LOS B | 22.1 | 158.5 | 0.85 | 0.95 | 39.8 |
| West: | Chatha | m Avenue | | | | | | | | | | | |
| 10 | L2 | 174 | 17.0 | 174 | 17.0 | 0.726 | 28.3 | LOS B | 4.2 | 39.7 | 1.00 | 0.93 | 17.8 |
| 12 | R2 | 1 | 0.0 | 1 | 0.0 | 0.726 | 28.2 | LOS B | 4.2 | 39.7 | 1.00 | 0.93 | 34.0 |
| Appro | ach | 175 | 16.9 | 175 | 16.9 | 0.726 | 28.3 | LOS B | 4.2 | 39.7 | 1.00 | 0.93 | 17.9 |
| All Ve | hicles | 1661 | 4.9 | 1661 | 4.9 | 0.850 | 18.2 | LOS B | 22.1 | 158.5 | 0.90 | 0.95 | 37.7 |

♦♦ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.5 %

Number of Iterations: 5 (maximum specified: 20)

| Movement Performance - Pedestrians | | | | | | | | | | | |
|------------------------------------|---------------------|-------------------------|-------------------------|-------|-----------------------------------|---------------------------|-----------------|-----------------------------------|--|--|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | | Average Back Pedestrian ped | of Queue Distance m | Prop. Queued | Effective Stop Rate per ped | | | |
| P3 | North Full Crossing | 11 | 16.9 | LOS B | 0.0 | 0.0 | 0.87 | 0.87 | | | |
| P4 | West Full Crossing | 11 | 15.2 | LOS B | 0.0 | 0.0 | 0.82 | 0.82 | | | |
| All Pe | destrians | 21 | 16.1 | LOS B | | | 0.84 | 0.84 | | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: G [Moorebank Avenue/Chatham Avenue_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK

Signals - Fixed Time Isolated Cycle Time = 45 seconds (Practical Cycle Time)

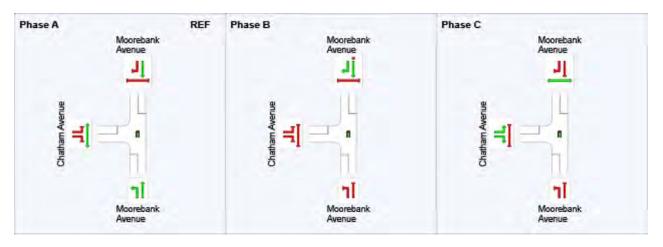
Phase Times determined by the program Green Split Priority applies **Phase Sequence: Opposed Turns** Reference Phase: Phase A Input Phase Sequence: A, B, C

Output Phase Sequence: A, B, C

Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 0 | 21 | 33 |
| Green Time (sec) | 15 | 6 | 6 |
| Phase Time (sec) | 21 | 12 | 12 |
| Phase Split | 47% | 27% | 27% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

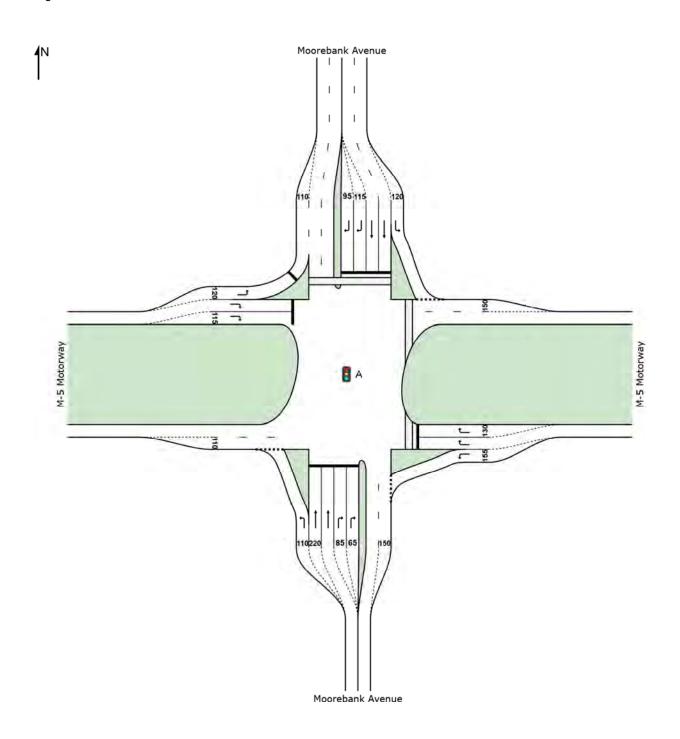


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Stage 3(ii)

Site: A [M5/Moorebank Avenue_AM]

Intersection of Moorebank Avenue and M5 Motorway AM PEAK
Signals - Fixed Time Isolated



Site: A [M5/Moorebank Avenue_AM]

Intersection of Moorebank Avenue and M5 Motorway AM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|-----------|-----------------|---------|------------------|---------|--------------|------------------|---------------------|----------------------|----------------------|--------|---------------------|---------|
| | | | | | | | | | | | | | |
| Mov ID | OD Mov | Demand Total | Flows | Arrival Total | Flows | Deg. Satn | Average Delav | Level of Service | 95% Back Vehicles | of Queue Distance | Prop. | Effective Stop Rate | |
| טו | IVIOV | veh/h | пv % | veh/h | пv % | v/c | sec | Service | venicies | Distance | Queued | per veh | km/h |
| South | : Moorel | oank Avenu | | VO11/11 | /0 | V/ O | 300 | | VOII | | | per veri | KIII/II |
| 1 | L2 | 428 | 14.7 | 428 | 14.7 | 0.396 | 14.4 | LOSA | 9.9 | 89.8 | 0.42 | 0.73 | 50.3 |
| 2 | T1 | 402 | 3.4 | 402 | 3.4 | 0.252 | 29.2 | LOS C | 9.3 | 69.3 | 0.68 | 0.58 | 34.6 |
| 3 | R2 | 271 | 20.2 | 271 | 20.2 | 0.441 | 57.9 | LOS E | 9.3 | 91.2 | 0.89 | 0.80 | 26.2 |
| Appro | ach | 1101 | 12.0 | 1101 | 12.0 | 0.441 | 30.5 | LOS C | 9.9 | 91.2 | 0.63 | 0.69 | 36.9 |
| East: | M-5 Mot | orway | | | | | | | | | | | |
| 4 | L2 | 339 | 17.7 | 339 | 17.7 | 0.285 | 6.4 | LOSA | 2.0 | 18.9 | 0.14 | 0.59 | 47.5 |
| 6 | R2 | 243 | 4.3 | 243 | 4.3 | 0.949 | 104.0 | LOS F | 10.7 | 81.6 | 1.00 | 1.05 | 17.1 |
| Appro | ach | 582 | 12.1 | 582 | 12.1 | 0.949 | 47.1 | LOS D | 10.7 | 81.6 | 0.50 | 0.78 | 23.9 |
| North | : Mooreb | ank Avenu | е | | | | | | | | | | |
| 7 | L2 | 48 | 19.6 | 48 | 19.6 | 0.042 | 7.3 | LOSA | 0.5 | 4.7 | 0.18 | 0.58 | 52.8 |
| 8 | T1 | 218 | 6.8 | 218 | 6.8 | 0.156 | 27.7 | LOS B | 5.2 | 41.5 | 0.65 | 0.53 | 24.6 |
| 9 | R2 | 506 | 20.2 | 506 | 20.2 | 0.967 | 87.5 | LOS F | 28.7 | 282.0 | 0.98 | 0.98 | 22.1 |
| Appro | ach | 773 | 16.3 | 773 | 16.3 | 0.967 | 65.6 | LOS E | 28.7 | 282.0 | 0.83 | 0.82 | 23.3 |
| West: | M-5 Mo | torway | | | | | | | | | | | |
| 10 | L2 | 1356 | 7.6 | 1356 | 7.6 | 0.887 | 7.1 | LOSA | 21.5 | 173.2 | 0.48 | 0.66 | 50.5 |
| 12 | R2 | 521 | 9.7 | 521 | 9.7 | 0.812 | 68.9 | LOS E | 20.8 | 173.8 | 0.99 | 0.88 | 18.9 |
| Appro | ach | 1877 | 8.2 | 1877 | 8.2 | 0.887 | 24.3 | LOS B | 21.5 | 173.8 | 0.62 | 0.72 | 38.0 |
| All Ve | hicles | 4333 | 11.1 | 4333 | 11.1 | 0.967 | 36.3 | LOS C | 28.7 | 282.0 | 0.64 | 0.74 | 32.4 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 %

Number of Iterations: 14 (maximum specified: 20)

| Move | ment Performance - Pedestrian | S | | | | | | |
|-----------|-------------------------------|----------------|------------------|---------------------|----------------------------|----------------------|-----------------|------------------------|
| Mov ID | Description | Demand Flow | Average Delay | Level of Service | Average Back Pedestrian | of Queue Distance | Prop. Queued | Effective Stop Rate |
| טו | · · · · · · · · · · · · · · · | ped/h | sec | Service | ped | m | Queueu | per ped |
| P21 | East Stage 1 | 26 | 64.5 | LOS F | 0.1 | 0.1 | 0.93 | 0.93 |
| P22 | East Stage 2 | 26 | 68.2 | LOS F | 0.1 | 0.1 | 0.95 | 0.95 |
| P3 | North Full Crossing | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 |
| All Pe | destrians | 79 | 67.3 | LOS F | | | 0.95 | 0.95 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: A [M5/Moorebank Avenue_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and M5 Motorway AM PEAK

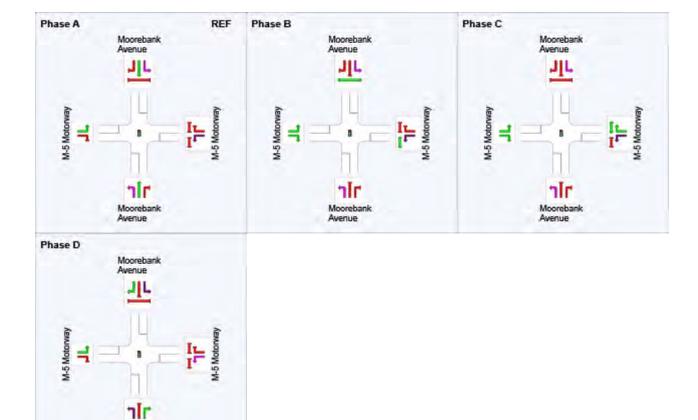
Phase Times determined by the program

Phase Sequence: 4-phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|-----|-----|
| Phase Change Time (sec) | 0 | 70 | 91 | 108 |
| Green Time (sec) | 64 | 15 | 11 | 36 |
| Phase Time (sec) | 70 | 21 | 17 | 42 |
| Phase Split | 47% | 14% | 11% | 28% |

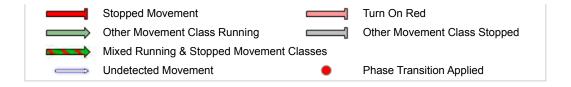
See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

Moorebank Avenue

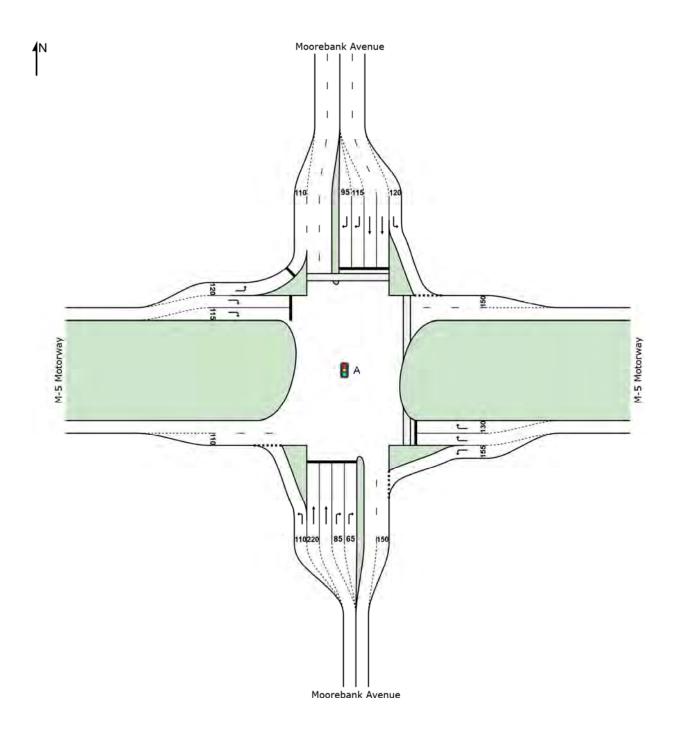




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Site: A [M5/Moorebank Avenue_PM]

Intersection of Moorebank Avenue and M5 Motorway PM PEAK
Signals - Fixed Time Isolated



Site: A [M5/Moorebank Avenue_PM]

Intersection of Moorebank Avenue and M5 Motorway

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

| | | Performan | | | | | | | | | | | |
|--------|----------|----------------|---------|----------------|---------|-------------|--------------|----------|-----------------|---------------|--------|-------------------|---------------|
| Mov | OD | Demand | | Arrival | | Deg. | Average | Level of | 95% Back | | Prop. | Effective | |
| ID | Mov | Total veh/h | HV % | Total veh/h | HV % | Satn v/c | Delay sec | Service | Vehicles veh | Distance m | Queued | Stop Rate per veh | Speed km/h |
| South | : Moorel | bank Avenu | | VCII/II | /0 | V/C | 300 | | VEII | - ''' | | per veri | KIII/II |
| 1 | L2 | 541 | 7.4 | 541 | 7.4 | 0.761 | 42.8 | LOS D | 26.3 | 211.1 | 0.92 | 1.05 | 35.1 |
| 2 | T1 | 286 | 2.6 | 286 | 2.6 | 0.567 | 66.8 | LOSE | 10.0 | 73.6 | 0.99 | 0.80 | |
| 3 | R2 | 404 | 8.9 | 404 | 8.9 | 0.239 | 22.6 | LOS B | 7.9 | 65.3 | 0.53 | 0.72 | 41.6 |
| | | | | | | | | | | | | | |
| Appro | ach | 1232 | 6.8 | 1232 | 6.8 | 0.761 | 41.7 | LOS C | 26.3 | 211.1 | 0.81 | 0.88 | 33.0 |
| East: | M-5 Mot | orway | | | | | | | | | | | |
| 4 | L2 | 281 | 12.7 | 281 | 12.7 | 0.239 | 7.1 | LOSA | 2.9 | 25.6 | 0.20 | 0.61 | 46.3 |
| 6 | R2 | 87 | 6.0 | 87 | 6.0 | 0.642 | 89.0 | LOS F | 3.4 | 26.9 | 1.00 | 0.78 | 19.0 |
| Appro | ach | 368 | 11.1 | 368 | 11.1 | 0.642 | 26.5 | LOS B | 3.4 | 26.9 | 0.39 | 0.65 | 30.7 |
| | | | | | | | | | | | | | |
| | | ank Avenue | | | | | | | | | | | |
| 7 | L2 | 74 | 5.7 | 74 | 5.7 | 0.065 | 6.8 | LOSA | 0.6 | 4.8 | 0.16 | 0.59 | 55.9 |
| 8 | T1 | 405 | 1.8 | 405 | 1.8 | 0.864 | 74.2 | LOS F | 17.4 | 126.4 | 1.00 | 0.92 | 12.4 |
| 9 | R2 | 1296 | 4.5 | 1296 | 4.5 | 0.884 | 35.2 | LOS C | 46.1 | 352.4 | 0.76 | 0.85 | 38.0 |
| Appro | ach | 1775 | 4.0 | 1775 | 4.0 | 0.884 | 42.9 | LOS D | 46.1 | 352.4 | 0.79 | 0.85 | 31.6 |
| West: | M-5 Mo | torway | | | | | | | | | | | |
| 10 | L2 | 595 | 7.3 | 595 | 7.3 | 0.387 | 6.1 | LOSA | 2.8 | 22.5 | 0.13 | 0.56 | 52.0 |
| 12 | R2 | 439 | 9.6 | 439 | 9.6 | 0.810 | 72.7 | LOS F | 17.7 | 148.0 | 1.00 | 0.88 | 18.2 |
| Appro | ach | 1034 | 8.2 | 1034 | 8.2 | 0.810 | 34.4 | LOS C | 17.7 | 148.0 | 0.50 | 0.70 | 32.7 |
| All Ve | hicles | 4408 | 6.4 | 4408 | 6.4 | 0.884 | 39.2 | LOS C | 46.1 | 352.4 | 0.69 | 0.81 | 32.2 |

+ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.3 %

Number of Iterations: 5 (maximum specified: 20)

| Move | ment Performance - Pedestrian | s | | | | | | |
|--------|-------------------------------|--------|---------|---------|--------------|----------|--------|-----------|
| Mov | D | Demand | Average | | Average Back | | Prop. | Effective |
| ID | Description | Flow | Delay | Service | Pedestrian | Distance | Queued | Stop Rate |
| | | ped/h | sec | | ped | m | | per ped |
| P21 | East Stage 1 | 26 | 64.5 | LOS F | 0.1 | 0.1 | 0.93 | 0.93 |
| P22 | East Stage 2 | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 |
| P3 | North Full Crossing | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 |
| All Pe | destrians | 79 | 67.6 | LOS F | | | 0.95 | 0.95 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: A [M5/Moorebank Avenue_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and M5 Motorway

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

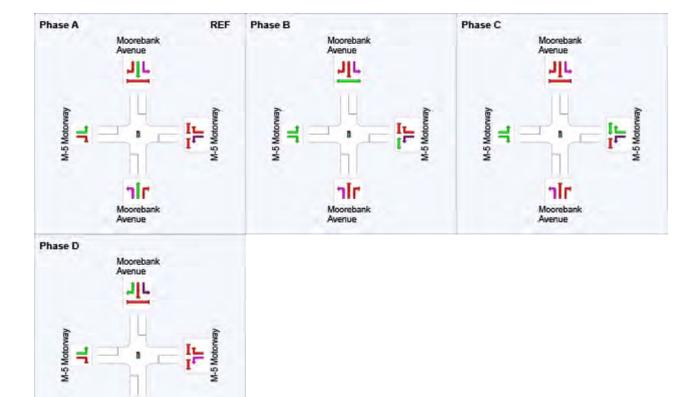
Phase Times determined by the program

Phase Sequence: 4-phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|----|-----|
| Phase Change Time (sec) | 0 | 26 | 47 | 59 |
| Green Time (sec) | 20 | 15 | 6 | 85 |
| Phase Time (sec) | 26 | 21 | 12 | 91 |
| Phase Split | 17% | 14% | 8% | 61% |

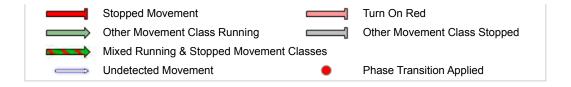
See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

Moorebank Avenue

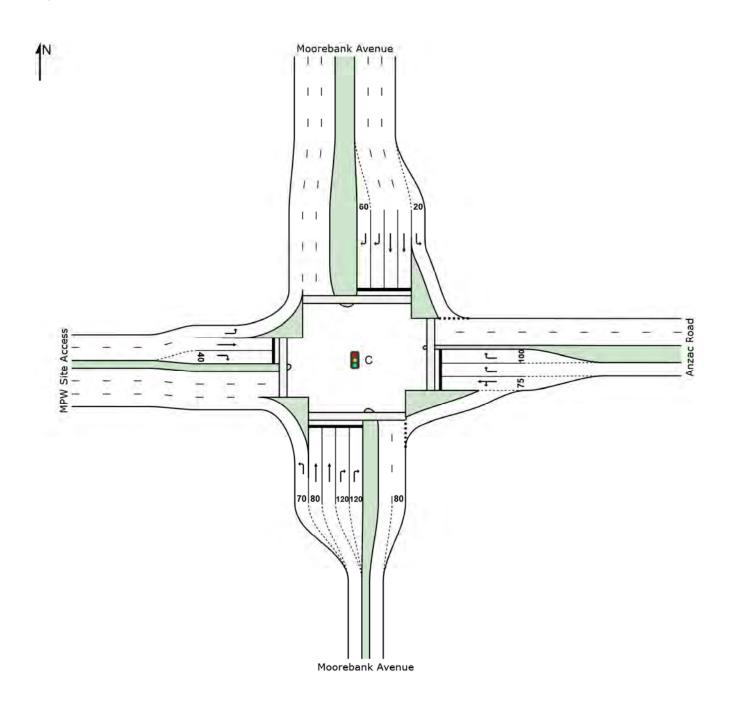




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Site: C [Moorebank Avenue_Anzac Road_AM]

Intersection of Moorebank Avenue and Anzac Road AM PEAK
Signals - Fixed Time Isolated



Site: C [Moorebank Avenue_Anzac Road_AM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 60 seconds (Practical Cycle Time)

| Move | Movement Performance - Vehicles | | | | | | | | | | | | |
|--------|---------------------------------|----------------|---------|-------------|-----------|-------------|--------------|----------|--------------|---------------|--------|-------------------|---------------|
| Mov | OD | Demand | | | Flows | Deg. | Average | Level of | | of Queue | Prop. | Effective | |
| ID | Mov | Total veh/h | HV % | Total veh/h | HV % | Satn v/c | Delay sec | Service | Vehicles veh | Distance m | Queued | Stop Rate per veh | Speed km/h |
| South | n: Moore | bank Avenu | | VGH/H | /0 | V/C | 366 | | Ven | - ''' | | per veri | KIII/II |
| 1 | L2 | 37 | 0.0 | 37 | 0.0 | 0.020 | 5.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.53 | 53.9 |
| 2 | T1 | 713 | 6.1 | 713 | 6.1 | 0.806 | 28.4 | LOS B | 11.4 | 89.1 | 1.00 | 1.04 | 20.2 |
| 3 | R2 | 381 | 3.3 | 381 | 3.3 | 0.780 | 35.1 | LOS C | 6.2 | 46.1 | 1.00 | 0.98 | 22.9 |
| Appro | oach | 1131 | 4.9 | 1131 | 4.9 | 0.806 | 29.9 | LOS C | 11.4 | 89.1 | 0.97 | 1.00 | 22.2 |
| East: | Anzac F | Road | | | | | | | | | | | |
| 4 | L2 | 208 | 3.0 | 208 | 3.0 | 0.206 | 8.4 | LOSA | 1.8 | 13.4 | 0.43 | 0.67 | 33.5 |
| 5 | T1 | 1 | 0.0 | 1 | 0.0 | 0.206 | 2.8 | LOSA | 1.8 | 13.4 | 0.43 | 0.67 | 49.6 |
| 6 | R2 | 363 | 11.9 | 363 | 11.9 | 0.787 | 38.0 | LOS C | 5.9 | 51.5 | 1.00 | 0.95 | 13.4 |
| Appro | oach | 573 | 8.6 | 573 | 8.6 | 0.787 | 27.2 | LOS B | 5.9 | 51.5 | 0.79 | 0.85 | 17.1 |
| North | : Moore | bank Avenu | ie | | | | | | | | | | |
| 7 | L2 | 403 | 7.8 | 403 | 7.8 | 0.320 | 5.3 | LOSA | 3.1 | 25.0 | 0.41 | 0.59 | 36.7 |
| 8 | T1 | 492 | 10.3 | 492 | 10.3 | 0.765 | 25.1 | LOS B | 10.1 | 85.3 | 0.96 | 0.89 | 12.9 |
| 9 | R2 | 186 | 23.7 | 186 | 23.7 | 0.435 | 33.5 | LOS C | 2.7 | 22.8 | 0.96 | 0.77 | 30.9 |
| Appro | oach | 1081 | 11.7 | 1081 | 11.7 | 0.765 | 19.2 | LOS B | 10.1 | 85.3 | 0.75 | 0.76 | 24.9 |
| West | MPW S | Site Access | | | | | | | | | | | |
| 10 | L2 | 44 | 100.0 | 44 | 100. 0 | 0.041 | 6.1 | LOSA | 0.0 | 0.0 | 0.00 | 0.50 | 51.0 |
| 11 | T1 | 1 | 0.0 | 1 | 0.0 | 0.005 | 26.9 | LOS B | 0.0 | 0.2 | 0.92 | 0.54 | 36.3 |
| 12 | R2 | 3 | 33.3 | 3 | 33.3 | 0.021 | 33.6 | LOS C | 0.1 | 0.8 | 0.92 | 0.62 | 29.2 |
| Appro | oach | 48 | 93.5 | 48 | 93.5 | 0.041 | 8.4 | LOSA | 0.1 | 0.8 | 0.08 | 0.51 | 48.2 |
| All Ve | hicles | 2833 | 9.8 | 2833 | 9.8 | 0.806 | 24.9 | LOS B | 11.4 | 89.1 | 0.83 | 0.87 | 22.6 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 %

Number of Iterations: 14 (maximum specified: 20)

| Move | ement Performance - Pedes | strians | | | | | | |
|------|---------------------------|---------|---------|---------|--------------|----------|--------|-----------|
| Mov | 5 | Demand | Average | | Average Back | | Prop. | Effective |
| ID | Description | Flow | Delay | Service | Pedestrian | Distance | Queued | Stop Rate |
| | | ped/h | sec | | ped | m | | per ped |
| P1 | South Full Crossing | 11 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| P2 | East Full Crossing | 11 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| P3 | North Full Crossing | 11 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| P4 | West Full Crossing | 53 | 24.4 | LOS C | 0.1 | 0.1 | 0.90 | 0.90 |

All Pedestrians 84 24.3 LOS C 0.90 0.90

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: C [Moorebank Avenue_Anzac Road_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 60 seconds (Practical Cycle Time)

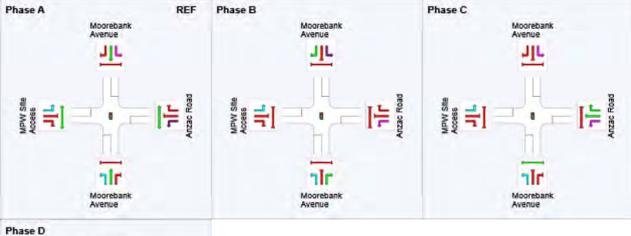
Phase Times determined by the program

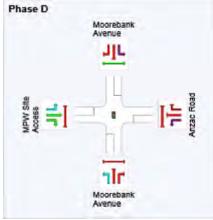
Phase Sequence: 4 Phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|-----|-----|
| Phase Change Time (sec) | 0 | 20 | 34 | 48 |
| Green Time (sec) | 14 | 8 | 8 | 6 |
| Phase Time (sec) | 20 | 14 | 14 | 12 |
| Phase Split | 33% | 23% | 23% | 20% |

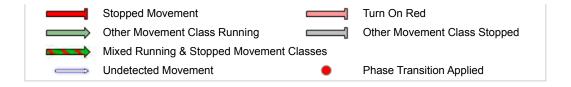
See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.





REF: Reference Phase VAR: Variable Phase

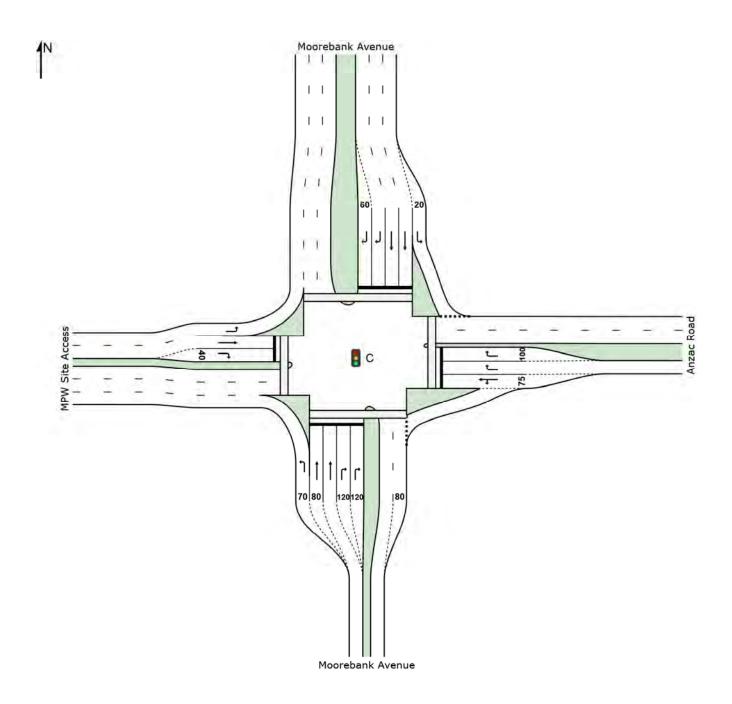




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Site: C [Moorebank Avenue_Anzac Road_PM]

Intersection of Moorebank Avenue and Anzac Road PM PEAK
Signals - Fixed Time Isolated



Site: C [Moorebank Avenue_Anzac Road_PM]

Intersection of Moorebank Avenue and Anzac Road

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 70 seconds (Practical Cycle Time)

| Mov | ement F | Performan | ce - Ve | hicles | | | | | | | | | |
|--------|----------|----------------|---------|-------------|---------|-------------|---------|----------|--------------|----------|--------|-------------------|------|
| Mov | OD | Demand | | | l Flows | Deg. | Average | Level of | | of Queue | Prop. | Effective | |
| ID | Mov | Total veh/h | HV | Total veh/h | HV % | Satn v/c | Delay | Service | Vehicles veh | Distance | Queued | Stop Rate per veh | |
| South | n: Moore | bank Avenu | | ven/n | 70 | V/C | sec | | ven | m | | per veri | km/h |
| 1 | L2 | 6 | 83.3 | 6 | 83.3 | 0.005 | 6.5 | LOSA | 0.0 | 0.0 | 0.00 | 0.49 | 50.5 |
| 2 | T1 | 589 | 4.5 | 589 | 4.5 | 0.513 | 22.3 | LOS B | 8.5 | 64.9 | 0.88 | 0.74 | 22.6 |
| 3 | R2 | 188 | 0.6 | 188 | 0.6 | 0.588 | 39.6 | LOS C | 3.4 | 24.0 | 1.00 | 0.81 | 21.6 |
| Appro | | 784 | 4.2 | 784 | 4.2 | 0.588 | 26.3 | LOS B | 8.5 | 64.9 | 0.90 | 0.76 | 22.5 |
| | | | 7.2 | 704 | 7.2 | 0.500 | 20.5 | LOGB | 0.5 | 04.3 | 0.30 | 0.70 | 22.5 |
| East: | Anzac F | Road | | | | | | | | | | | |
| 4 | L2 | 280 | 1.5 | 280 | 1.5 | 0.293 | 10.6 | LOSA | 3.8 | 27.6 | 0.51 | 0.70 | 30.0 |
| 5 | T1 | 1 | 0.0 | 1 | 0.0 | 0.293 | 5.0 | LOS A | 3.8 | 27.6 | 0.51 | 0.70 | 47.5 |
| 6 | R2 | 287 | 4.0 | 287 | 4.0 | 0.424 | 33.9 | LOS C | 4.5 | 34.2 | 0.93 | 0.78 | 14.6 |
| Appro | oach | 568 | 2.8 | 568 | 2.8 | 0.424 | 22.4 | LOS B | 4.5 | 34.2 | 0.72 | 0.74 | 19.6 |
| North | : Moorel | bank Avenu | е | | | | | | | | | | |
| 7 | L2 | 419 | 3.0 | 419 | 3.0 | 0.295 | 4.5 | LOSA | 2.7 | 19.8 | 0.32 | 0.55 | 38.3 |
| 8 | T1 | 651 | 4.4 | 651 | 4.4 | 0.759 | 24.9 | LOS B | 14.7 | 111.6 | 0.92 | 0.84 | 13.0 |
| 9 | R2 | 56 | 79.2 | 56 | 79.2 | 0.271 | 41.9 | LOS C | 1.0 | 11.6 | 0.97 | 0.72 | 26.9 |
| Appro | oach | 1125 | 7.6 | 1125 | 7.6 | 0.759 | 18.1 | LOS B | 14.7 | 111.6 | 0.70 | 0.73 | 21.8 |
| West | : MPW S | Site Access | | | | | | | | | | | |
| 10 | L2 | 239 | 18.5 | 239 | 18.5 | 0.146 | 5.8 | LOS A | 0.0 | 0.0 | 0.00 | 0.52 | 51.0 |
| 11 | T1 | 22 | 0.0 | 22 | 0.0 | 0.132 | 33.8 | LOS C | 0.7 | 5.2 | 0.95 | 0.67 | 33.0 |
| 12 | R2 | 22 | 0.0 | 22 | 0.0 | 0.139 | 39.5 | LOS C | 0.7 | 5.2 | 0.96 | 0.70 | 26.7 |
| Appro | oach | 283 | 15.6 | 283 | 15.6 | 0.146 | 10.6 | LOSA | 0.7 | 5.2 | 0.15 | 0.54 | 45.4 |
| All Ve | hicles | 2761 | 6.4 | 2761 | 6.4 | 0.759 | 20.6 | LOS B | 14.7 | 111.6 | 0.70 | 0.72 | 24.2 |

♦♦ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.3 %

Number of Iterations: 5 (maximum specified: 20)

| Move | ement Performance - Pedes | strians | | | | | | |
|-----------|---------------------------|-------------------------|-------------------------|---------------------|-----------------------------------|---------------------------|-----------------|-----------------------------------|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back Pedestrian ped | of Queue Distance m | Prop. Queued | Effective Stop Rate per ped |
| P1 | South Full Crossing | 11 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 |
| P2 | East Full Crossing | 11 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 |
| P3 | North Full Crossing | 11 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 |
| P4 | West Full Crossing | 53 | 29.3 | LOS C | 0.1 | 0.1 | 0.92 | 0.92 |
| All Pe | destrians | 84 | 29.3 | LOS C | | | 0.92 | 0.92 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: C [Moorebank Avenue_Anzac Road_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and Anzac Road

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 70 seconds (Practical Cycle Time)

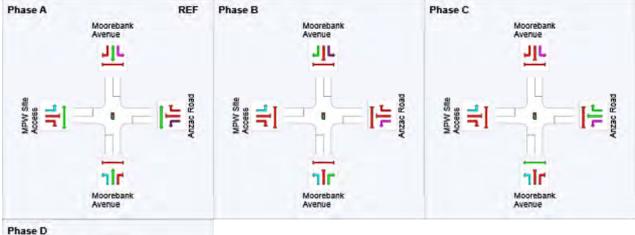
Phase Times determined by the program

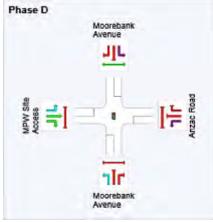
Phase Sequence: 4 Phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|-----|-----|
| Phase Change Time (sec) | 0 | 27 | 39 | 58 |
| Green Time (sec) | 21 | 6 | 13 | 6 |
| Phase Time (sec) | 27 | 12 | 19 | 12 |
| Phase Split | 39% | 17% | 27% | 17% |

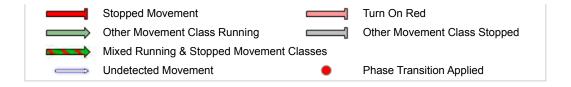
See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.





REF: Reference Phase VAR: Variable Phase

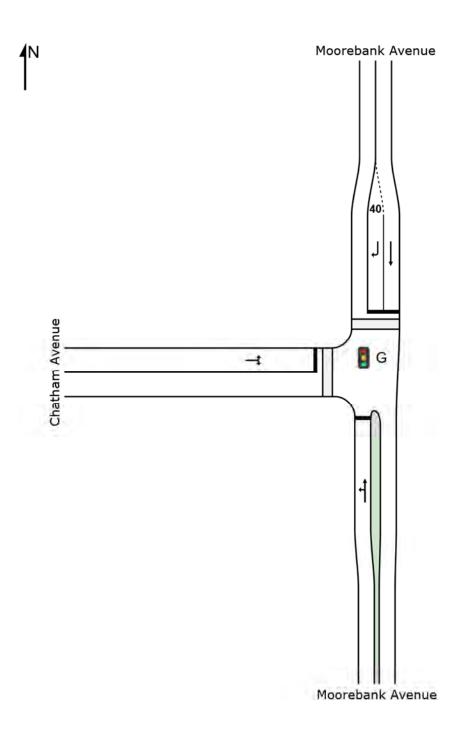




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Site: G [Moorebank Avenue/Chatham Avenue_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK
Signals - Fixed Time Isolated



Site: G [Moorebank Avenue/Chatham Avenue_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|-----------|--------------------------|------------------|--------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Arriva Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South | : Moore | bank Avenu | ıe | | | | | | | | | | |
| 1 | L2 | 1 | 0.0 | 1 | 0.0 | 0.896 | 30.2 | LOS C | 48.0 | 361.3 | 0.91 | 0.99 | 35.2 |
| 2 | T1 | 1103 | 3.7 | 1103 | 3.7 | 0.896 | 27.0 | LOS B | 48.0 | 361.3 | 0.91 | 0.99 | 32.0 |
| Appro | ach | 1104 | 3.7 | 1104 | 3.7 | 0.896 | 27.0 | LOS B | 48.0 | 361.3 | 0.91 | 0.99 | 32.0 |
| North: | Moorel | oank Avenu | e | | | | | | | | | | |
| 8 | T1 | 457 | 9.2 | 457 | 9.2 | 0.315 | 2.7 | LOS A | 5.3 | 43.6 | 0.30 | 0.27 | 45.7 |
| 9 | R2 | 15 | 100.0 | 15 | 100. 0 | 0.193 | 48.2 | LOS D | 0.6 | 13.4 | 0.97 | 0.70 | 24.1 |
| Appro | ach | 472 | 12.1 | 472 | 12.1 | 0.315 | 4.1 | LOSA | 5.3 | 43.6 | 0.32 | 0.28 | 44.8 |
| West: | Chatha | m Avenue | | | | | | | | | | | |
| 10 | L2 | 15 | 100.0 | 15 | 100. 0 | 0.184 | 49.0 | LOS D | 0.7 | 13.6 | 0.97 | 0.70 | 12.1 |
| 12 | R2 | 1 | 0.0 | 1 | 0.0 | 0.184 | 48.5 | LOS D | 0.7 | 13.6 | 0.97 | 0.70 | 26.8 |
| Appro | ach | 16 | 93.3 | 16 | 93.3 | 0.184 | 49.0 | LOS D | 0.7 | 13.6 | 0.97 | 0.70 | 13.5 |
| All Ve | hicles | 1592 | 7.1 | 1592 | 7.1 | 0.896 | 20.5 | LOS B | 48.0 | 361.3 | 0.74 | 0.78 | 36.7 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations; 0.9 %

Number of Iterations: 14 (maximum specified: 20)

| Move | ment Performance - Pedestri | ans | | | | | | |
|-----------|-----------------------------|----------------|------------------|-------|----------------------------|----------------------|-----------------|------------------------|
| Mov ID | Description | Demand Flow | Average Delay | | Average Back Pedestrian | of Queue Distance | Prop. Queued | Effective Stop Rate |
| | | ped/h | sec | | ped | m | | per ped |
| P3 | North Full Crossing | 11 | 36.7 | LOS D | 0.0 | 0.0 | 0.93 | 0.93 |
| P4 | West Full Crossing | 11 | 8.1 | LOS A | 0.0 | 0.0 | 0.44 | 0.44 |
| All Pe | destrians | 21 | 22.4 | LOS C | | | 0.68 | 0.68 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: G [Moorebank Avenue/Chatham Avenue_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK

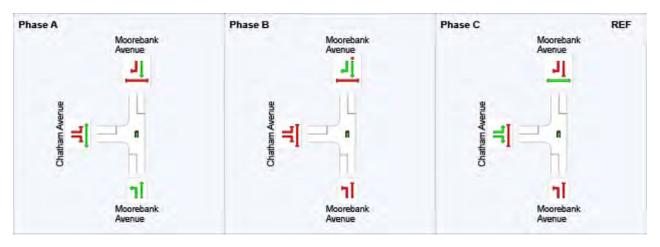
Signals - Fixed Time Isolated Cycle Time = 85 seconds (Practical Cycle Time)

Phase Times determined by the program Green Split Priority applies **Phase Sequence: Opposed Turns** Reference Phase: Phase C Input Phase Sequence: A, B, C Output Phase Sequence: A, B, C

Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 12 | 73 | 0 |
| Green Time (sec) | 55 | 6 | 6 |
| Phase Time (sec) | 61 | 12 | 12 |
| Phase Split | 72% | 14% | 14% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



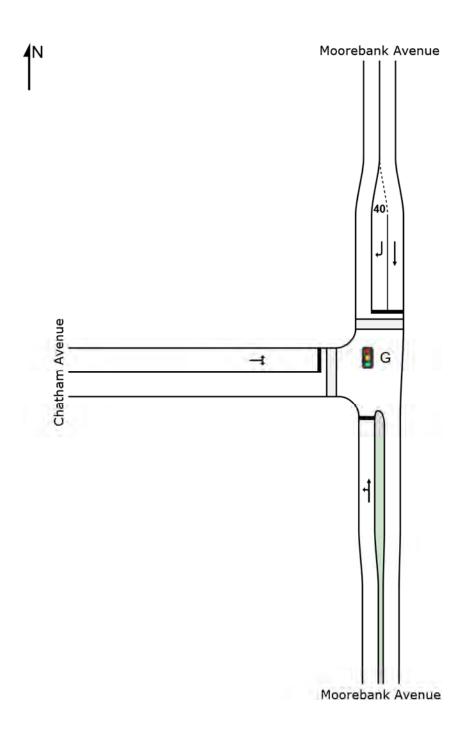
REF: Reference Phase VAR: Variable Phase



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Site: G [Moorebank Avenue/Chatham Avenue_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK Signals - Fixed Time Isolated



Site: G [Moorebank Avenue/Chatham Avenue_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK

Signals - Fixed Time Isolated Cycle Time = 45 seconds (Practical Cycle Time)

| Move | ement l | Performar | ice - Ve | hicles | | | | | | | | | |
|-----------|-----------|--------------------------|----------|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | HV | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South | : Moore | bank Avenu | ıe | | | | | | | | | | |
| 1 | L2 | 1 | 0.0 | 1 | 0.0 | 0.784 | 23.9 | LOS B | 11.6 | 85.1 | 0.96 | 0.96 | 38.6 |
| 2 | T1 | 501 | 2.3 | 501 | 2.3 | 0.784 | 20.7 | LOS B | 11.6 | 85.1 | 0.96 | 0.96 | 35.9 |
| Appro | ach | 502 | 2.3 | 502 | 2.3 | 0.784 | 20.7 | LOS B | 11.6 | 85.1 | 0.96 | 0.96 | 35.9 |
| North | : Moorel | bank Avenu | e | | | | | | | | | | |
| 8 | T1 | 955 | 1.2 | 955 | 1.2 | 0.836 | 13.6 | LOS A | 21.3 | 152.8 | 0.85 | 0.93 | 40.6 |
| 9 | R2 | 15 | 100.0 | 15 | 100. 0 | 0.102 | 24.2 | LOS B | 0.3 | 6.6 | 0.91 | 0.68 | 30.3 |
| Appro | ach | 969 | 2.7 | 969 | 2.7 | 0.836 | 13.8 | LOSA | 21.3 | 152.8 | 0.85 | 0.93 | 40.4 |
| West: | Chatha | ım Avenue | | | | | | | | | | | |
| 10 | L2 | 86 | 17.1 | 86 | 17.1 | 0.363 | 25.5 | LOS B | 1.9 | 17.8 | 0.95 | 0.75 | 19.0 |
| 12 | R2 | 1 | 0.0 | 1 | 0.0 | 0.363 | 25.4 | LOS B | 1.9 | 17.8 | 0.95 | 0.75 | 35.4 |
| Appro | ach | 87 | 16.9 | 87 | 16.9 | 0.363 | 25.5 | LOS B | 1.9 | 17.8 | 0.95 | 0.75 | 19.3 |
| All Ve | hicles | 1559 | 3.4 | 1559 | 3.4 | 0.836 | 16.7 | LOS B | 21.3 | 152.8 | 0.89 | 0.93 | 38.9 |

♦♦ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.3 %

Number of Iterations: 5 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | | | |
|-----------|------------------------------------|-------------------------|-------------------------|-------|-----------------------------------|---------------------------|-----------------|-----------------------------------|--|--|--|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | | Average Back Pedestrian ped | of Queue Distance m | Prop. Queued | Effective Stop Rate per ped | | | | |
| P3 | North Full Crossing | 11 | 16.9 | LOS B | 0.0 | 0.0 | 0.87 | 0.87 | | | | |
| P4 | West Full Crossing | 11 | 15.2 | LOS B | 0.0 | 0.0 | 0.82 | 0.82 | | | | |
| All Pe | destrians | 21 | 16.1 | LOS B | | | 0.84 | 0.84 | | | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: G [Moorebank Avenue/Chatham Avenue_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK

Signals - Fixed Time Isolated Cycle Time = 45 seconds (Practical Cycle Time)

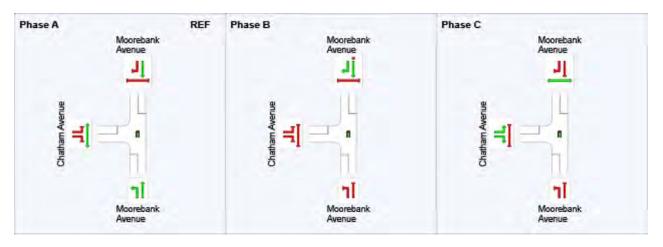
Phase Times determined by the program Green Split Priority applies **Phase Sequence: Opposed Turns** Reference Phase: Phase A Input Phase Sequence: A, B, C

Output Phase Sequence: A, B, C

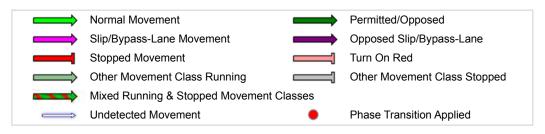
Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 0 | 21 | 33 |
| Green Time (sec) | 15 | 6 | 6 |
| Phase Time (sec) | 21 | 12 | 12 |
| Phase Split | 47% | 27% | 27% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

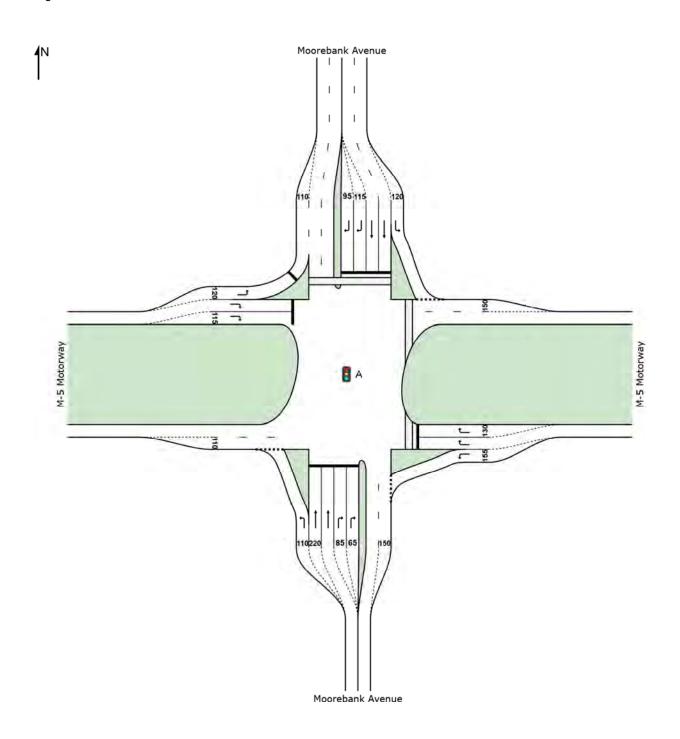


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Stage 3(iii)

Site: A [M5/Moorebank Avenue_AM]

Intersection of Moorebank Avenue and M5 Motorway AM PEAK
Signals - Fixed Time Isolated



Site: A [M5/Moorebank Avenue_AM]

Intersection of Moorebank Avenue and M5 Motorway

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|-----------|-----------------|---------|------------------|---------|--------------|------------------|---------------------|----------------------|----------------------|--------|---------------------|---------|
| | | | | | | | | | | | | | |
| Mov ID | OD Mov | Demand Total | Flows | Arrival Total | Flows | Deg. Satn | Average Delav | Level of Service | 95% Back Vehicles | of Queue Distance | Prop. | Effective Stop Rate | |
| טו | IVIOV | veh/h | пv % | veh/h | пv % | v/c | sec | Service | venicies | Distance | Queued | per veh | km/h |
| South | : Moorel | oank Avenu | | VO11/11 | /0 | V/ O | 300 | | VOII | | | per veri | KIII/II |
| 1 | L2 | 428 | 14.7 | 428 | 14.7 | 0.396 | 14.4 | LOSA | 9.9 | 89.8 | 0.42 | 0.73 | 50.3 |
| 2 | T1 | 402 | 3.4 | 402 | 3.4 | 0.252 | 29.2 | LOS C | 9.3 | 69.3 | 0.68 | 0.58 | 34.6 |
| 3 | R2 | 271 | 20.2 | 271 | 20.2 | 0.441 | 57.9 | LOS E | 9.3 | 91.2 | 0.89 | 0.80 | 26.2 |
| Appro | ach | 1101 | 12.0 | 1101 | 12.0 | 0.441 | 30.5 | LOS C | 9.9 | 91.2 | 0.63 | 0.69 | 36.9 |
| East: | M-5 Mot | orway | | | | | | | | | | | |
| 4 | L2 | 339 | 17.7 | 339 | 17.7 | 0.285 | 6.4 | LOSA | 2.0 | 18.9 | 0.14 | 0.59 | 47.5 |
| 6 | R2 | 243 | 4.3 | 243 | 4.3 | 0.949 | 104.0 | LOS F | 10.7 | 81.6 | 1.00 | 1.05 | 17.1 |
| Appro | ach | 582 | 12.1 | 582 | 12.1 | 0.949 | 47.1 | LOS D | 10.7 | 81.6 | 0.50 | 0.78 | 23.9 |
| North | : Mooreb | ank Avenu | е | | | | | | | | | | |
| 7 | L2 | 48 | 19.6 | 48 | 19.6 | 0.042 | 7.3 | LOSA | 0.5 | 4.7 | 0.18 | 0.58 | 52.8 |
| 8 | T1 | 218 | 6.8 | 218 | 6.8 | 0.156 | 27.7 | LOS B | 5.2 | 41.5 | 0.65 | 0.53 | 24.6 |
| 9 | R2 | 506 | 20.2 | 506 | 20.2 | 0.967 | 87.5 | LOS F | 28.7 | 282.0 | 0.98 | 0.98 | 22.1 |
| Appro | ach | 773 | 16.3 | 773 | 16.3 | 0.967 | 65.6 | LOS E | 28.7 | 282.0 | 0.83 | 0.82 | 23.3 |
| West: | M-5 Mo | torway | | | | | | | | | | | |
| 10 | L2 | 1356 | 7.6 | 1356 | 7.6 | 0.887 | 7.1 | LOSA | 21.5 | 173.2 | 0.48 | 0.66 | 50.5 |
| 12 | R2 | 521 | 9.7 | 521 | 9.7 | 0.812 | 68.9 | LOS E | 20.8 | 173.8 | 0.99 | 0.88 | 18.9 |
| Appro | ach | 1877 | 8.2 | 1877 | 8.2 | 0.887 | 24.3 | LOS B | 21.5 | 173.8 | 0.62 | 0.72 | 38.0 |
| All Ve | hicles | 4333 | 11.1 | 4333 | 11.1 | 0.967 | 36.3 | LOS C | 28.7 | 282.0 | 0.64 | 0.74 | 32.4 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 %

Number of Iterations: 9 (maximum specified: 20)

| Move | ment Performance - Pedestrians | S | | | | | | |
|--------|--------------------------------|--------|---------|---------|--------------|----------|--------|-----------|
| Mov | | Demand | Average | | Average Back | | Prop. | Effective |
| ID | Description | Flow | Delay | Service | | Distance | Queued | Stop Rate |
| | | ped/h | sec | | ped | m | | per ped |
| P21 | East Stage 1 | 26 | 64.5 | LOS F | 0.1 | 0.1 | 0.93 | 0.93 |
| P22 | East Stage 2 | 26 | 68.2 | LOS F | 0.1 | 0.1 | 0.95 | 0.95 |
| P3 | North Full Crossing | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 |
| All Pe | destrians | 79 | 67.3 | LOS F | | | 0.95 | 0.95 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: A [M5/Moorebank Avenue_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and M5 Motorway AM PEAK

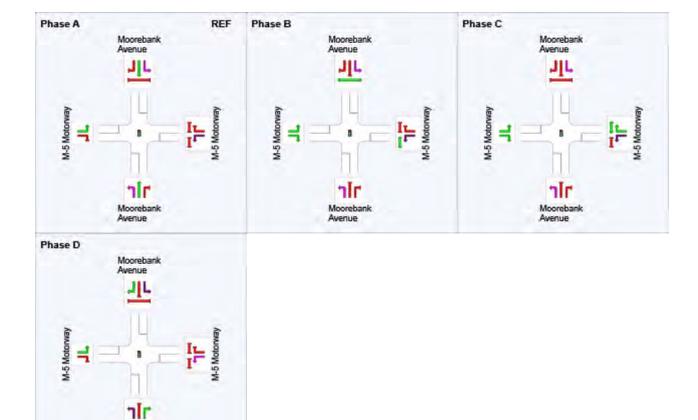
Phase Times determined by the program

Phase Sequence: 4-phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|-----|-----|
| Phase Change Time (sec) | 0 | 70 | 91 | 108 |
| Green Time (sec) | 64 | 15 | 11 | 36 |
| Phase Time (sec) | 70 | 21 | 17 | 42 |
| Phase Split | 47% | 14% | 11% | 28% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

Moorebank Avenue

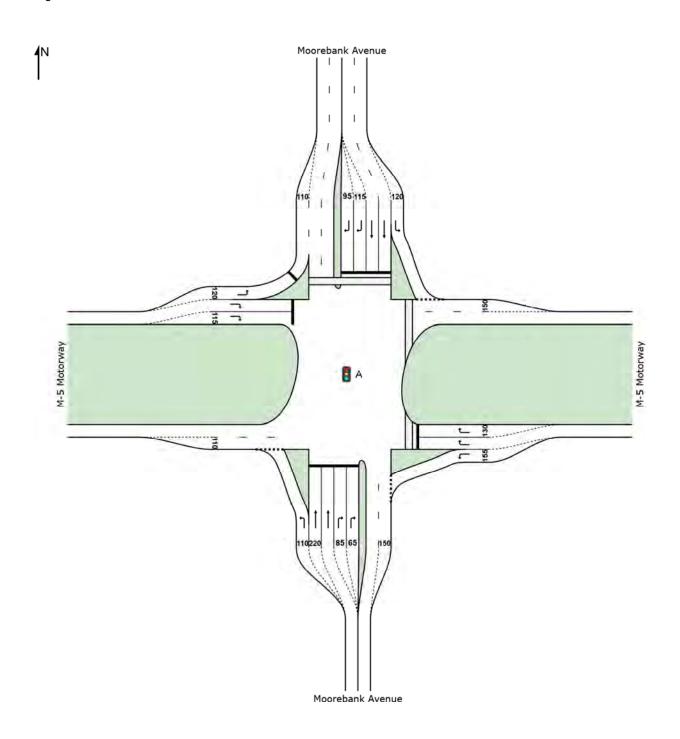




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Site: A [M5/Moorebank Avenue_PM]

Intersection of Moorebank Avenue and M5 Motorway PM PEAK
Signals - Fixed Time Isolated



Site: A [M5/Moorebank Avenue_PM]

Intersection of Moorebank Avenue and M5 Motorway

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

| Move | Movement Performance - Vehicles | | | | | | | | | | | | |
|-----------|---------------------------------|--------------------------|------------------|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South | : Moorel | oank Avenu | е | | | | | | | | | | |
| 1 | L2 | 541 | 7.4 | 541 | 7.4 | 0.761 | 42.8 | LOS D | 26.3 | 211.1 | 0.92 | 1.05 | 35.1 |
| 2 | T1 | 286 | 2.6 | 286 | 2.6 | 0.567 | 66.8 | LOS E | 10.0 | 73.6 | 0.99 | 0.80 | 22.4 |
| 3 | R2 | 404 | 8.9 | 404 | 8.9 | 0.239 | 22.6 | LOS B | 7.9 | 65.3 | 0.53 | 0.72 | 41.6 |
| Appro | ach | 1232 | 6.8 | 1232 | 6.8 | 0.761 | 41.7 | LOS C | 26.3 | 211.1 | 0.81 | 0.88 | 33.0 |
| East: | M-5 Mot | orway | | | | | | | | | | | |
| 4 | L2 | 281 | 12.7 | 281 | 12.7 | 0.239 | 7.1 | LOSA | 2.9 | 25.6 | 0.20 | 0.61 | 46.3 |
| 6 | R2 | 87 | 6.0 | 87 | 6.0 | 0.642 | 89.0 | LOS F | 3.4 | 26.9 | 1.00 | 0.78 | 19.0 |
| Appro | ach | 368 | 11.1 | 368 | 11.1 | 0.642 | 26.5 | LOS B | 3.4 | 26.9 | 0.39 | 0.65 | 30.7 |
| North | : Mooreb | ank Avenue | Э | | | | | | | | | | |
| 7 | L2 | 74 | 5.7 | 74 | 5.7 | 0.065 | 6.8 | LOSA | 0.6 | 4.8 | 0.16 | 0.59 | 55.9 |
| 8 | T1 | 405 | 1.8 | 405 | 1.8 | 0.864 | 74.2 | LOS F | 17.4 | 126.4 | 1.00 | 0.92 | 12.4 |
| 9 | R2 | 1296 | 4.5 | 1296 | 4.5 | 0.884 | 35.2 | LOS C | 46.1 | 352.4 | 0.76 | 0.85 | 38.0 |
| Appro | ach | 1775 | 4.0 | 1775 | 4.0 | 0.884 | 42.9 | LOS D | 46.1 | 352.4 | 0.79 | 0.85 | 31.6 |
| West: | M-5 Mo | torway | | | | | | | | | | | |
| 10 | L2 | 595 | 7.3 | 595 | 7.3 | 0.387 | 6.1 | LOSA | 2.8 | 22.5 | 0.13 | 0.56 | 52.0 |
| 12 | R2 | 439 | 9.6 | 439 | 9.6 | 0.810 | 72.7 | LOS F | 17.7 | 148.0 | 1.00 | 0.88 | 18.2 |
| Appro | ach | 1034 | 8.2 | 1034 | 8.2 | 0.810 | 34.4 | LOS C | 17.7 | 148.0 | 0.50 | 0.70 | 32.7 |
| All Ve | hicles | 4408 | 6.4 | 4408 | 6.4 | 0.884 | 39.2 | LOS C | 46.1 | 352.4 | 0.69 | 0.81 | 32.2 |

+ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 %

Number of Iterations: 7 (maximum specified: 20)

| Move | ment Performance - Pedestrian | s | | | | | | |
|--------|-------------------------------|--------|---------|---------|--------------|----------|--------|-----------|
| Mov | D | Demand | Average | | Average Back | | Prop. | Effective |
| ID | Description | Flow | Delay | Service | Pedestrian | Distance | Queued | Stop Rate |
| | | ped/h | sec | | ped | m | | per ped |
| P21 | East Stage 1 | 26 | 64.5 | LOS F | 0.1 | 0.1 | 0.93 | 0.93 |
| P22 | East Stage 2 | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 |
| P3 | North Full Crossing | 26 | 69.2 | LOS F | 0.1 | 0.1 | 0.96 | 0.96 |
| All Pe | destrians | 79 | 67.6 | LOS F | | | 0.95 | 0.95 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: A [M5/Moorebank Avenue_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and M5 Motorway

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Practical Cycle Time)

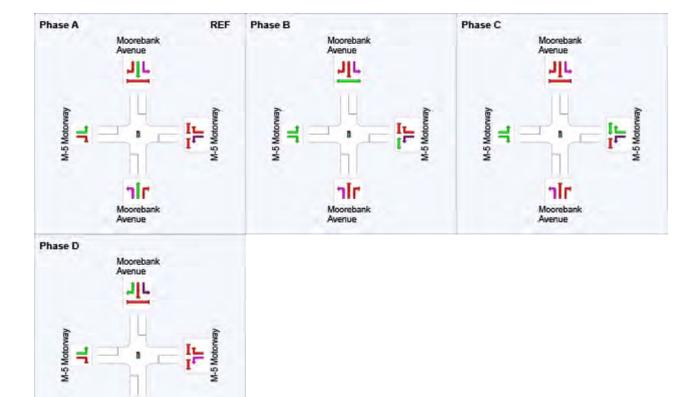
Phase Times determined by the program

Phase Sequence: 4-phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|----|-----|
| Phase Change Time (sec) | 0 | 26 | 47 | 59 |
| Green Time (sec) | 20 | 15 | 6 | 85 |
| Phase Time (sec) | 26 | 21 | 12 | 91 |
| Phase Split | 17% | 14% | 8% | 61% |

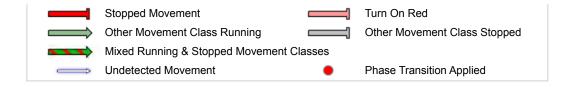
See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

Moorebank Avenue

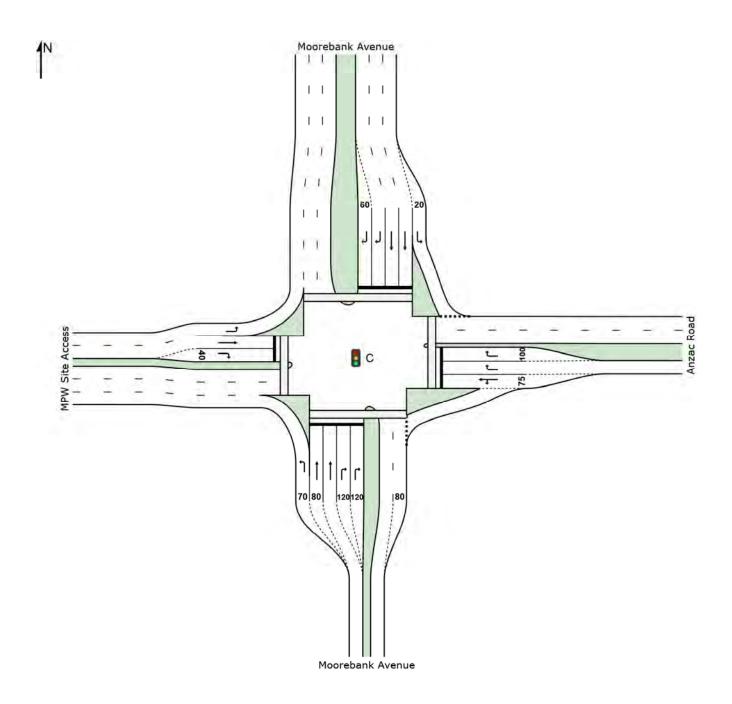




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Site: C [Moorebank Avenue_Anzac Road_AM]

Intersection of Moorebank Avenue and Anzac Road AM PEAK
Signals - Fixed Time Isolated



Site: C [Moorebank Avenue_Anzac Road_AM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 55 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|----------|----------------|---------|-------------|-----------|-------------|--------------|----------|--------------|---------------|--------|-------------------|---------------|
| Mov | OD | Demand | | | Flows | Deg. | Average | Level of | | of Queue | Prop. | Effective | |
| ID | Mov | Total veh/h | HV % | Total veh/h | HV % | Satn v/c | Delay sec | Service | Vehicles veh | Distance m | Queued | Stop Rate per veh | Speed km/h |
| South | n: Moore | bank Avenu | | VCII/II | /0 | V/C | 366 | | Ven | - ''' | | per verr | KIII/II |
| 1 | L2 | 37 | 0.0 | 37 | 0.0 | 0.020 | 5.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.53 | 53.9 |
| 2 | T1 | 698 | 4.1 | 698 | 4.1 | 0.909 | 36.0 | LOS C | 12.2 | 92.6 | 1.00 | 1.28 | 17.9 |
| 3 | R2 | 381 | 3.3 | 381 | 3.3 | 0.817 | 34.3 | LOS C | 5.9 | 43.9 | 1.00 | 1.04 | 23.1 |
| Appro | oach | 1116 | 3.7 | 1116 | 3.7 | 0.909 | 34.4 | LOS C | 12.2 | 92.6 | 0.97 | 1.17 | 20.8 |
| East: | Anzac F | Road | | | | | | | | | | | |
| 4 | L2 | 208 | 3.0 | 208 | 3.0 | 0.205 | 8.5 | LOSA | 1.8 | 13.4 | 0.44 | 0.67 | 33.2 |
| 5 | T1 | 1 | 0.0 | 1 | 0.0 | 0.205 | 2.9 | LOSA | 1.8 | 13.4 | 0.44 | 0.67 | 49.5 |
| 6 | R2 | 363 | 11.9 | 363 | 11.9 | 0.825 | 37.3 | LOS C | 5.7 | 49.1 | 1.00 | 1.00 | 13.6 |
| Appro | oach | 573 | 8.6 | 573 | 8.6 | 0.825 | 26.8 | LOS B | 5.7 | 49.1 | 0.80 | 0.88 | 17.3 |
| North | : Moore | bank Avenu | ie | | | | | | | | | | |
| 7 | L2 | 403 | 7.8 | 403 | 7.8 | 0.327 | 5.2 | LOSA | 2.8 | 23.0 | 0.43 | 0.59 | 36.8 |
| 8 | T1 | 477 | 7.5 | 477 | 7.5 | 0.851 | 27.4 | LOS B | 10.1 | 81.3 | 0.97 | 1.00 | 12.2 |
| 9 | R2 | 201 | 29.3 | 201 | 29.3 | 0.509 | 32.1 | LOS C | 2.8 | 24.1 | 0.98 | 0.78 | 31.4 |
| Appro | oach | 1081 | 11.7 | 1081 | 11.7 | 0.851 | 20.0 | LOS B | 10.1 | 81.3 | 0.77 | 0.81 | 24.7 |
| West | : MPW S | Site Access | | | | | | | | | | | |
| 10 | L2 | 59 | 100.0 | 59 | 100. 0 | 0.054 | 6.1 | LOSA | 0.0 | 0.0 | 0.00 | 0.50 | 51.0 |
| 11 | T1 | 1 | 0.0 | 1 | 0.0 | 0.005 | 24.1 | LOS B | 0.0 | 0.2 | 0.91 | 0.53 | 37.8 |
| 12 | R2 | 3 | 33.3 | 3 | 33.3 | 0.019 | 30.8 | LOS C | 0.1 | 0.7 | 0.91 | 0.62 | 30.6 |
| Appro | oach | 63 | 95.0 | 63 | 95.0 | 0.054 | 7.7 | LOSA | 0.1 | 0.7 | 0.06 | 0.51 | 49.0 |
| All Ve | hicles | 2833 | 9.8 | 2833 | 9.8 | 0.909 | 26.8 | LOS B | 12.2 | 92.6 | 0.84 | 0.96 | 22.0 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 %

Number of Iterations: 9 (maximum specified: 20)

| Move | ment Performance - Pede | strians | | | | | | |
|------|-------------------------|---------|---------|----------|--------------|----------|--------|-----------|
| Mov | | Demand | Average | Level of | Average Back | of Queue | Prop. | Effective |
| ID | Description | Flow | Delay | Service | Pedestrian | Distance | Queued | Stop Rate |
| | | ped/h | sec | | ped | m | | per ped |
| P1 | South Full Crossing | 11 | 21.8 | LOS C | 0.0 | 0.0 | 0.89 | 0.89 |
| P2 | East Full Crossing | 11 | 21.8 | LOS C | 0.0 | 0.0 | 0.89 | 0.89 |
| P3 | North Full Crossing | 11 | 21.8 | LOS C | 0.0 | 0.0 | 0.89 | 0.89 |
| P4 | West Full Crossing | 53 | 21.9 | LOS C | 0.1 | 0.1 | 0.89 | 0.89 |

All Pedestrians 84 21.9 LOS C 0.89 0.89

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: C [Moorebank Avenue_Anzac Road_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and Anzac Road

AM PEAK

Signals - Fixed Time Isolated Cycle Time = 55 seconds (Practical Cycle Time)

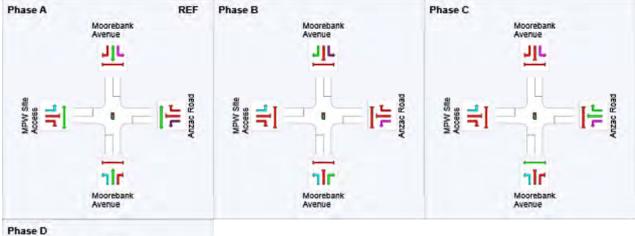
Phase Times determined by the program

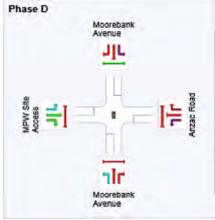
Phase Sequence: 4 Phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|-----|-----|
| Phase Change Time (sec) | 0 | 17 | 30 | 43 |
| Green Time (sec) | 11 | 7 | 7 | 6 |
| Phase Time (sec) | 17 | 13 | 13 | 12 |
| Phase Split | 31% | 24% | 24% | 22% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.





REF: Reference Phase VAR: Variable Phase

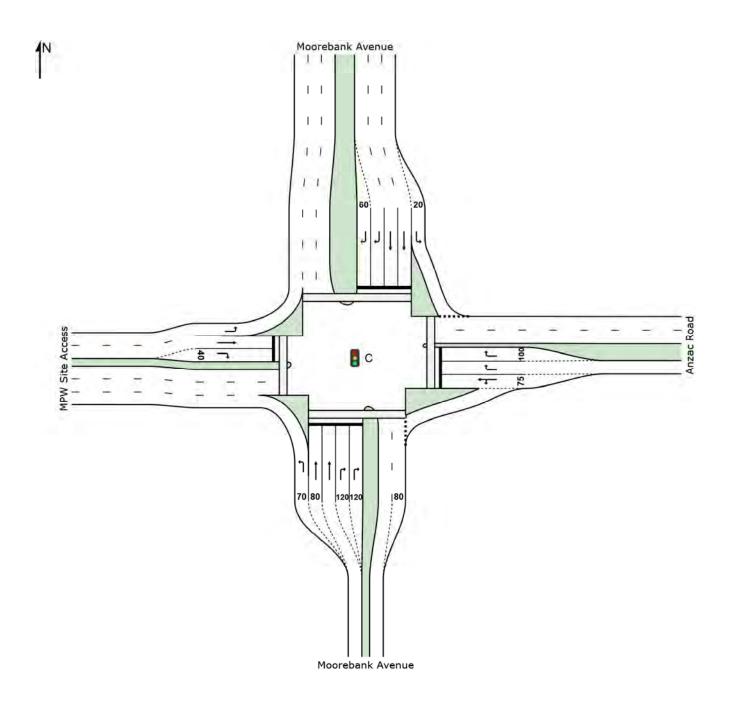




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Site: C [Moorebank Avenue_Anzac Road_PM]

Intersection of Moorebank Avenue and Anzac Road PM PEAK
Signals - Fixed Time Isolated



Site: C [Moorebank Avenue_Anzac Road_PM]

Intersection of Moorebank Avenue and Anzac Road

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 65 seconds (Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|----------|-------------|------|-------|---------|-------|---------|----------|----------|----------|-------|-----------|---------|
| Mov | OD | Demand | | | l Flows | Deg. | Average | Level of | 95% Back | of Queue | Prop. | Effective | Average |
| ID | Mov | Total | HV | Total | HV | Satn | Delay | Service | Vehicles | Distance | | Stop Rate | |
| | | veh/h | | veh/h | % | v/c | sec | | veh | m | | per veh | km/h |
| South | n: Moore | bank Avenu | е | | | | | | | | | | |
| 1 | L2 | 6 | 83.3 | 6 | 83.3 | 0.005 | 6.5 | LOSA | 0.0 | 0.0 | 0.00 | 0.49 | 50.5 |
| 2 | T1 | 509 | 2.3 | 509 | 2.3 | 0.502 | 22.6 | LOS B | 7.1 | 51.7 | 0.90 | 0.75 | 22.5 |
| 3 | R2 | 182 | 0.6 | 182 | 0.6 | 0.528 | 36.2 | LOS C | 3.0 | 21.2 | 1.00 | 0.78 | 22.6 |
| Appro | oach | 698 | 2.6 | 698 | 2.6 | 0.528 | 26.0 | LOS B | 7.1 | 51.7 | 0.92 | 0.75 | 22.8 |
| East: | Anzac F | Road | | | | | | | | | | | |
| 4 | L2 | 280 | 1.5 | 280 | 1.5 | 0.280 | 9.6 | LOSA | 3.3 | 23.7 | 0.48 | 0.69 | 31.4 |
| 5 | T1 | 1 | 0.0 | 1 | 0.0 | 0.280 | 4.0 | LOSA | 3.3 | 23.7 | 0.48 | 0.69 | 48.4 |
| 6 | R2 | 287 | 4.0 | 287 | 4.0 | 0.427 | 32.1 | LOS C | 4.2 | 31.9 | 0.93 | 0.78 | 15.2 |
| Appro | oach | 568 | 2.8 | 568 | 2.8 | 0.427 | 21.0 | LOS B | 4.2 | 31.9 | 0.71 | 0.74 | 20.4 |
| North | : Moorel | bank Avenu | е | | | | | | | | | | |
| 7 | L2 | 419 | 3.0 | 419 | 3.0 | 0.300 | 4.5 | LOSA | 2.4 | 17.9 | 0.32 | 0.55 | 38.4 |
| 8 | T1 | 636 | 2.2 | 636 | 2.2 | 0.915 | 35.2 | LOS C | 19.2 | 140.2 | 0.96 | 1.11 | 10.2 |
| 9 | R2 | 71 | 83.6 | 71 | 83.6 | 0.325 | 39.3 | LOS C | 1.2 | 14.0 | 0.97 | 0.73 | 27.7 |
| Appro | oach | 1125 | 7.6 | 1125 | 7.6 | 0.915 | 24.0 | LOS B | 19.2 | 140.2 | 0.72 | 0.88 | 19.1 |
| West | : MPW S | Site Access | | | | | | | | | | | |
| 10 | L2 | 318 | 18.5 | 318 | 18.5 | 0.194 | 5.8 | LOS A | 0.0 | 0.0 | 0.00 | 0.52 | 51.0 |
| 11 | T1 | 28 | 0.0 | 28 | 0.0 | 0.158 | 31.1 | LOS C | 0.9 | 6.2 | 0.95 | 0.68 | 34.2 |
| 12 | R2 | 22 | 0.0 | 22 | 0.0 | 0.129 | 36.7 | LOS C | 0.7 | 4.8 | 0.95 | 0.70 | 27.8 |
| Appro | oach | 368 | 16.0 | 368 | 16.0 | 0.194 | 9.6 | LOSA | 0.9 | 6.2 | 0.13 | 0.54 | 46.5 |
| All Ve | ehicles | 2760 | 6.4 | 2760 | 6.4 | 0.915 | 22.0 | LOS B | 19.2 | 140.2 | 0.69 | 0.77 | 24.0 |

♦♦ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 %

Number of Iterations: 7 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | |
|-----------|------------------------------------|-------------------------|-------------------------|---------------------|-----------------------------------|---------------------------|-----------------|-----------------------------------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back Pedestrian ped | of Queue Distance m | Prop. Queued | Effective Stop Rate per ped | |
| P1 | South Full Crossing | 11 | 26.8 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P2 | East Full Crossing | 11 | 26.8 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P3 | North Full Crossing | 11 | 26.8 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P4 | West Full Crossing | 53 | 26.8 | LOS C | 0.1 | 0.1 | 0.91 | 0.91 | |
| All Pe | destrians | 84 | 26.8 | LOS C | | | 0.91 | 0.91 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: C [Moorebank Avenue_Anzac Road_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and Anzac Road

PM PEAK

Signals - Fixed Time Isolated Cycle Time = 65 seconds (Practical Cycle Time)

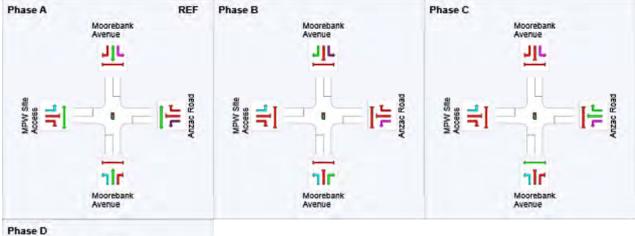
Phase Times determined by the program

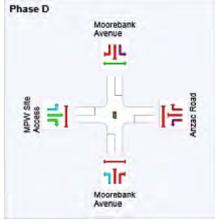
Phase Sequence: 4 Phase Reference Phase: Phase A Input Phase Sequence: A, B, C, D Output Phase Sequence: A, B, C, D

Phase Timing Results

| Phase | Α | В | С | D |
|-------------------------|-----|-----|-----|-----|
| Phase Change Time (sec) | 0 | 23 | 35 | 53 |
| Green Time (sec) | 17 | 6 | 12 | 6 |
| Phase Time (sec) | 23 | 12 | 18 | 12 |
| Phase Split | 35% | 18% | 28% | 18% |

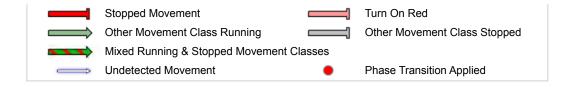
See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.





REF: Reference Phase VAR: Variable Phase

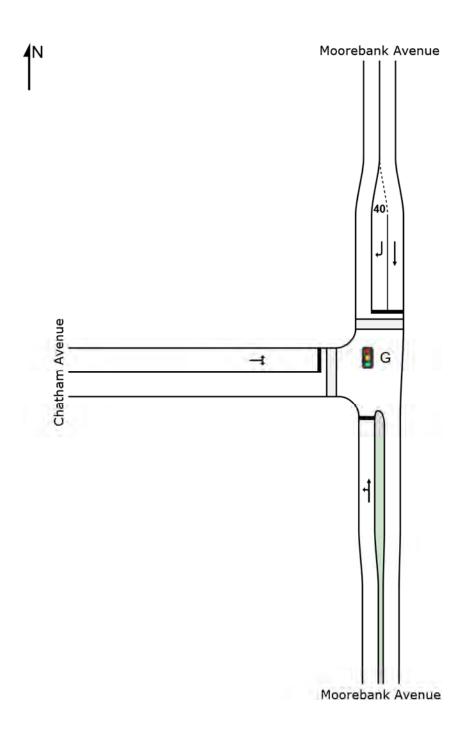




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Site: G [Moorebank Avenue/Chatham Avenue_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK
Signals - Fixed Time Isolated



Site: G [Moorebank Avenue/Chatham Avenue_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK

Signals - Fixed Time Isolated Cycle Time = 85 seconds (Practical Cycle Time)

| Move | ement F | Performan | ce - Ve | hicles | | | | | | | | | |
|-----------|-----------|----------------------------|------------------|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South | : Moore | bank Avenue | Э | | | | | | | | | | |
| 1 | L2 | 1 | 0.0 | 1 | 0.0 | 0.896 | 30.2 | LOS C | 48.0 | 361.3 | 0.91 | 0.99 | 35.2 |
| 2 | T1 | 1103 | 3.7 | 1103 | 3.7 | 0.896 | 27.0 | LOS B | 48.0 | 361.3 | 0.91 | 0.99 | 32.0 |
| Appro | ach | 1104 | 3.7 | 1104 | 3.7 | 0.896 | 27.0 | LOS B | 48.0 | 361.3 | 0.91 | 0.99 | 32.0 |
| North: | : Mooreb | oank Avenue |) | | | | | | | | | | |
| 8 | T1 | 457 | 9.2 | 457 | 9.2 | 0.315 | 2.7 | LOS A | 5.3 | 43.6 | 0.30 | 0.27 | 45.7 |
| 9 | R2 | 1 | 0.0 | 1 | 0.0 | 0.008 | 44.2 | LOS D | 0.0 | 0.3 | 0.95 | 0.58 | 25.2 |
| Appro | ach | 458 | 9.2 | 458 | 9.2 | 0.315 | 2.8 | LOSA | 5.3 | 43.6 | 0.30 | 0.27 | 45.6 |
| West: | Chatha | m Avenue | | | | | | | | | | | |
| 10 | L2 | 1 | 0.0 | 1 | 0.0 | 0.015 | 45.3 | LOS D | 0.1 | 0.6 | 0.95 | 0.61 | 12.7 |
| 12 | R2 | 1 | 0.0 | 1 | 0.0 | 0.015 | 45.3 | LOS D | 0.1 | 0.6 | 0.95 | 0.61 | 27.7 |
| Appro | ach | 2 | 0.0 | 2 | 0.0 | 0.015 | 45.3 | LOS D | 0.1 | 0.6 | 0.95 | 0.61 | 21.8 |
| All Ve | hicles | 1564 | 5.3 | 1564 | 5.3 | 0.896 | 20.0 | LOS B | 48.0 | 361.3 | 0.74 | 0.78 | 37.1 |

♦♦ Network: 1 [Scenario 2_AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 %

Number of Iterations: 9 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | |
|-----------|------------------------------------|-------------------------|-------------------------|---------------------|-----------------------------------|---------------------------|-----------------|-----------------------------------|--|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back Pedestrian ped | of Queue Distance m | Prop. Queued | Effective Stop Rate per ped | | |
| P3 | North Full Crossing | 11 | 36.7 | LOS D | 0.0 | 0.0 | 0.93 | 0.93 | | |
| P4 | West Full Crossing | 11 | 8.1 | LOS A | 0.0 | 0.0 | 0.44 | 0.44 | | |
| All Pe | destrians | 21 | 22.4 | LOS C | | | 0.68 | 0.68 | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: G [Moorebank Avenue/Chatham Avenue_AM]

♦♦ Network: 1 [Scenario 2_AM]

Intersection of Moorebank Avenue and Chatham Avenue AM PEAK

Signals - Fixed Time Isolated Cycle Time = 85 seconds (Practical Cycle Time)

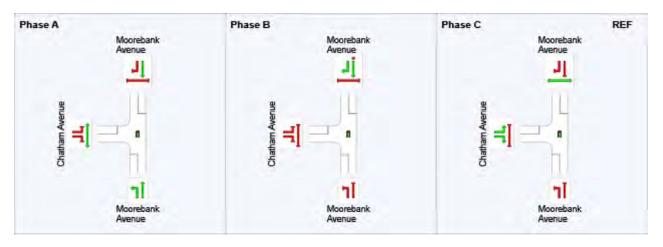
Phase Times determined by the program Green Split Priority applies **Phase Sequence: Opposed Turns** Reference Phase: Phase C Input Phase Sequence: A, B, C

Output Phase Sequence: A, B, C

Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 12 | 73 | 0 |
| Green Time (sec) | 55 | 6 | 6 |
| Phase Time (sec) | 61 | 12 | 12 |
| Phase Split | 72% | 14% | 14% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase

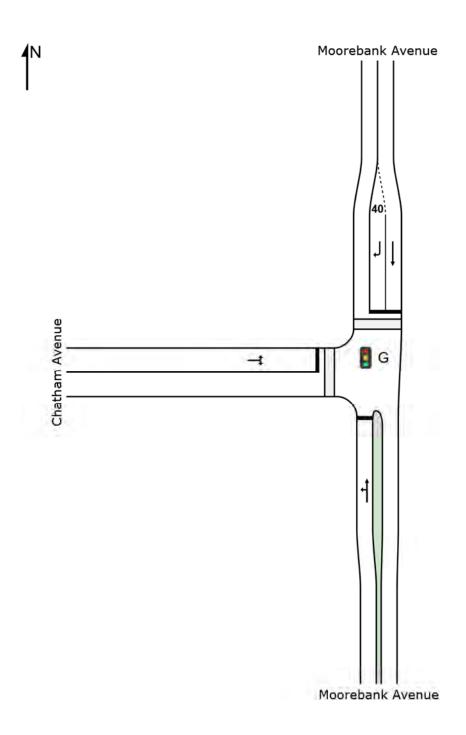


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Site: G [Moorebank Avenue/Chatham Avenue_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK Signals - Fixed Time Isolated



Site: G [Moorebank Avenue/Chatham Avenue_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK

Signals - Fixed Time Isolated Cycle Time = 45 seconds (Practical Cycle Time)

| Move | Movement Performance - Vehicles | | | | | | | | | | | | |
|-----------|---------------------------------|--------------------------|------------------|---------------------------|------------------|---------------------|-------------------------|---------------------|-----------------------------|---------------------------|-----------------|-----------------------------------|------|
| Mov ID | OD Mov | Demand Total veh/h | Flows HV % | Arrival Total veh/h | Flows HV % | Deg. Satn v/c | Average Delay sec | Level of Service | 95% Back Vehicles veh | of Queue Distance m | Prop. Queued | Effective Stop Rate per veh | |
| South | : Moorel | bank Avenu | е | | | | | | | | | | |
| 1 | L2 | 1 | 0.0 | 1 | 0.0 | 0.784 | 23.9 | LOS B | 11.6 | 85.1 | 0.96 | 0.96 | 38.6 |
| 2 | T1 | 501 | 2.3 | 501 | 2.3 | 0.784 | 20.7 | LOS B | 11.6 | 85.1 | 0.96 | 0.96 | 35.9 |
| Appro | ach | 502 | 2.3 | 502 | 2.3 | 0.784 | 20.7 | LOS B | 11.6 | 85.1 | 0.96 | 0.96 | 35.9 |
| North: | Mooreb | oank Avenue | Э | | | | | | | | | | |
| 8 | T1 | 955 | 1.2 | 955 | 1.2 | 0.823 | 12.6 | LOSA | 20.6 | 147.8 | 0.85 | 0.91 | 41.0 |
| 9 | R2 | 1 | 0.0 | 1 | 0.0 | 0.004 | 22.2 | LOS B | 0.0 | 0.1 | 0.88 | 0.57 | 31.3 |
| Appro | ach | 956 | 1.2 | 956 | 1.2 | 0.823 | 12.6 | LOSA | 20.6 | 147.8 | 0.85 | 0.91 | 41.0 |
| West: | Chatha | m Avenue | | | | | | | | | | | |
| 10 | L2 | 1 | 0.0 | 1 | 0.0 | 0.008 | 23.3 | LOS B | 0.0 | 0.3 | 0.88 | 0.60 | 19.9 |
| 12 | R2 | 1 | 0.0 | 1 | 0.0 | 0.008 | 23.3 | LOS B | 0.0 | 0.3 | 0.88 | 0.60 | 36.3 |
| Appro | ach | 2 | 0.0 | 2 | 0.0 | 0.008 | 23.3 | LOS B | 0.0 | 0.3 | 0.88 | 0.60 | 30.5 |
| All Ve | hicles | 1460 | 1.6 | 1460 | 1.6 | 0.823 | 15.4 | LOS B | 20.6 | 147.8 | 0.89 | 0.92 | 39.9 |

♦♦ Network: 1 [Scenario 2_PM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Largest change in Average Back of Queue or Degree of Saturation for any lane during the last three iterations: 0.9 %

Number of Iterations: 7 (maximum specified: 20)

| Move | Movement Performance - Pedestrians | | | | | | | | | |
|-----------|------------------------------------|-------------------------|-------------------------|---------------------|-----------------------------------|---------------------------|-----------------|-----------------------------------|--|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back Pedestrian ped | of Queue Distance m | Prop. Queued | Effective Stop Rate per ped | | |
| P3 | North Full Crossing | 11 | 16.9 | LOS B | 0.0 | 0.0 | 0.87 | 0.87 | | |
| P4 | West Full Crossing | 11 | 15.2 | LOS B | 0.0 | 0.0 | 0.82 | 0.82 | | |
| All Pe | destrians | 21 | 16.1 | LOS B | | | 0.84 | 0.84 | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: G [Moorebank Avenue/Chatham Avenue_PM]

♦♦ Network: 1 [Scenario 2_PM]

Intersection of Moorebank Avenue and Chatham Avenue PM PEAK

Signals - Fixed Time Isolated Cycle Time = 45 seconds (Practical Cycle Time)

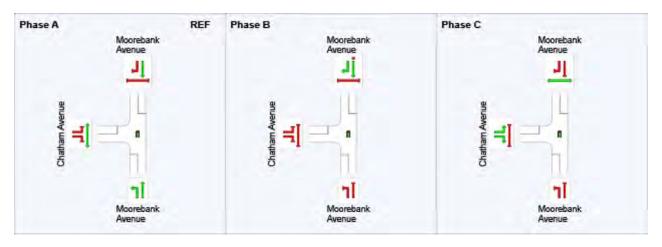
Phase Times determined by the program Green Split Priority applies **Phase Sequence: Opposed Turns** Reference Phase: Phase A Input Phase Sequence: A, B, C

Output Phase Sequence: A, B, C

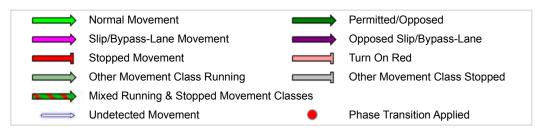
Phase Timing Results

| Phase | Α | В | С |
|-------------------------|-----|-----|-----|
| Phase Change Time (sec) | 0 | 21 | 33 |
| Green Time (sec) | 15 | 6 | 6 |
| Phase Time (sec) | 21 | 12 | 12 |
| Phase Split | 47% | 27% | 27% |

See the Phase Information section in the Detailed Output report for more detailed information including input values of Yellow Time and All-Red Time, and information on any adjustments to Intergreen Time, Phase Time and Green Time values in cases of Pedestrian Actuation, Phase Actuation and Phase Frequency values (user-specified or implied) less than 100%.



REF: Reference Phase VAR: Variable Phase



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