

PO Box 979 Level 1, 91 George Street PARRAMATTA NSW 2150

Office 02 9891 5033 Fax 02 9891 3898 admin@sparksandpartners.com.au

sparksandpartners.com.au

ABN 95 161 152 969

# WATER SENSITIVE URBAN DESIGN REPORT

## Lot 8- First Estate Orchard Hills Warehouse Facilities

Lot 8 585-649 Mamre Road Orchard Hills, NSW 2748

Date: 9 July 2018

Revision: 2
Issue: 1

Ref. No.: 18086

Prepared for: ALTIS Property

C/o Hansen Yuncken

Client Details: Email: CSims@hansenyuncken.com.au











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#### **Document Control**

Revision	Date	Description	Prepared	Reviewed	Approved
1	21.06.18	SSD Application Issue	SK	MW	MW
2	09.07.18	SSD Application Issue	SK	MW	MW

Prepared by	Simon Kapsis	Revision	1
Approved by	Morgan Walter	Revision	1



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## 1. EXECUTIVE SUMMARY

Sparks & Partners have been engaged by Hansen Yuncken on behalf of Altis Property to provide civil engineering services to support the SSD application for two (2) new warehouse facilities at Lot 8, 585-649 Mamre Rd, Orchard Hills, NSW. The engineering services include the design and documentation of the concept stormwater drainage infrastructure, Water Sensitive Urban Design (WUSD) strategy and finished pavement levels for the proposed development.

Penrith City Council being the consent authority for the proposed development, require a WSUD strategy be prepared that takes into consideration the objectives and controls under Penrith Development Control Plan 2014 (PDCP) Part C3-Water Management, and the Mamre West Land Investigation Area DCP (MWDCP), Section 5-Stormwater and Flooding, prepared by Urbis. In response to these requirements Sparks and Partners have undertaken a review of the proposed estate infrastructure plans prepared by CostinRoe Consulting as part of the State Significant Development application (refer SSD 15\_7173), completed modelling and prepared this report to demonstrate that the proposed development identifies and incorporates water conservation and stormwater management measures into its design and operation in accordance with the requirements of Part C3 of the PDCP





## 2. INTRODUCTION

## 2.1 Existing Site

The site is Lot 8 within the First Estate sub-division and is bounded by the future estate road on the western and southern boundaries, existing warehouse development on the northern boundary, and a future development on the eastern boundary. Lot 8 is approximately 32,780m² in area and benched to a bulk earthworks level of RL35.500 once the estate infrastructure works are complete. Reference is made to the SSD 15\_7173 and engineering plans prepared by CostinRoe Consulting, ref: CO12042.00 for details of the site prior to this SSD Application.

## 2.2 Proposed Development

The proposed development consists of two (2) separate warehouse buildings (Lot 8A1 and 8A2), with associated hardstand areas, car park pavement areas and landscaping. Warehouse 8A1 is to run in an east-west orientation with the main entrances located on their southern elevation, and warehouse 8A2 is to run in a north-south orientation with the main entrances located on their western elevation. Warehouse 8A1 is to be a single tenancy and is approximately 11,720m² in area with the office located on the south eastern corner and car parking located along the eastern side of the warehouse. Warehouse 8A2 is to be a single tenancy and is approximately 5,500m² in area with the office and car parking located along the southern side of the warehouse. The main truck entrance to the site will be from the southern boundary for Warehouse 8A1 and on the western side for 8A2, with the exit driveways located on the southern boundary. Detailed architectural plans have been prepared by Nettleton Tribe architects for the DA submission and are to be read in conjunction with this report.



### 3. INTEGRATED WATER MANAGEMENT

#### 3.1 General

The objective of WSUD is to provide a strategy that brings together the different aspects of the water cycle as a whole rather than an ad hoc approach to water management. This includes the management aspects of freshwater, wastewater and stormwater. The following WUSD strategies have been considered and addressed for the proposed development:

- Employ an integrated water collection and recycling system for capturing and recycling roofwater;
- Control the quality of stormwater that is disposed from the site;
- Control the quantity of stormwater that is discharged for the site.

The estate works as part of the SSD 15\_7173 take the above objectives into consideration, and as such have included estate wide treatment measures to address water quality and quantity. These treatment measures consist of the following:

- Estate gross pollutant traps that provide primary treatment of collected stormwater runoff from roads and developed lots;
- An estate bio retention basin that provides secondary and tertiary treatment of collected stormwater runoff from roads and developed lots;
- An estate on-site detention (OSD) basin that provides discharge control of stormwater runoff from the estate.

By providing the above measures at an estate level on lot measures are not required to be implemented for quality and quantity objectives, with rainwater reuse being required to be implemented on lot. Reference is made to appendix A for the detailed estate plans prepared by CostinRoe Consulting which detail the estate wide measures that will be provided.

The following sections detail the on-lot measures that the development will implement to meet the requirements for rainwater collection and reuse. Concept Stormwater Management Plans for the subject development are provided in Appendix B for reference.

## 3.2 Water Conservation

Water usage reduction is to be achieved throughout the development through the use of a minimum of 4 Star WELS rated water fixtures and rainwater reuse in accordance with the performance criteria under section 3.1 Water Conservation of the Penrith City Council WSUD Policy, December 2013 and section 5.4 of the MWDCP.

#### 3.3 Rainwater Reuse

Through the reuse of collected roofwater for non-potable reuse the proposed demand on potable water resources is reduced. The proposed development will capture roof water portions of roof area providing catchment areas of 1,880m² for Warehouse 8A1 and 1,970



m² for Warehouse 8A2. This collected roofwater will be conveyed to one (1) 50kL Rainwater Tanks for the office and Warehouse 8A1 and two (2) 30KL rainwater tanks for the office and Warehouse 8A2 and will be used for storage and reuse throughout the development. Re-use purposes will primarily include toilet flushing and irrigation. A water balance of the proposed reuse system has been completed to model its effectiveness and efficiency. The water balance model was constructed using the MUSIC software package with the following inputs:

- Penrith City Council MUSIClink files;
- Warehouse 8A1:
  - o Total approximate non-potable reuse of 1.25KL per day based on:
    - Ten (10) toilets & urinals with each using 0.1KL/day for five (5) days of the week [0.72kL/day]
    - 500m² of irrigated area at 0.4KL/year/m².
       0.55kL/day.
- Warehouse 8A2:
  - Total approximate non-potable reuse of 1.43KL per day based on:
    - Ten (10) toilets & urinals with each using 0.1KL/day for five (5) days of the week [0.72kL per day].
    - 650m² of irrigated area at 0.4KL/year/m².
       0.71kL/day

Using the above determined non-potable demand the MUSIC model determines the rainwater tanks have an approximate efficiency of between 85%-86% resulting in an approximate reduction in the proposed demand on potable water supplies of 840,000L per year. The results of the MUSIC Model are presented in Table 1 for reference, with a MUSICLink report provided in Appendix C. This demonstrates that the reuse efficiency meets the minimum 80% requirement in accordance with the performance criteria under section 3.1 Water Conservation of the Penrith City Council WSUD Policy, December 2013 and section 5.4 of the MWDCP.



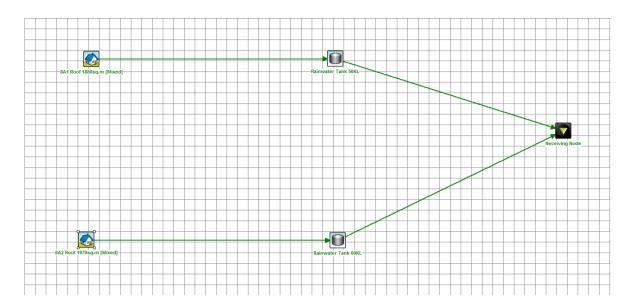


Figure 1 - MUSIC Model

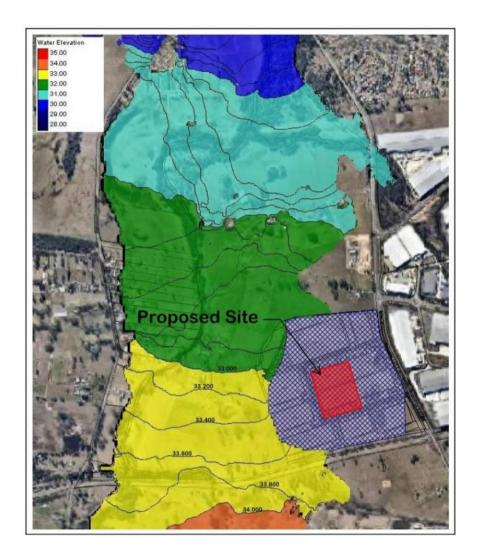
	8A1	8A2
Rainwater Tank Size	50kL	60kL
Flow In (ML/yr)	1.11	1.16
Pipe Out (ML/yr)	0.72	0.72
Weir Out (ML/yr)	0.00	0.00
Reuse Supplied (ML/yr)	0.39	0.45
Reuse Requested (ML/yr)	0.46	0.52
% Reuse Demand Met	85.9	86.03
% Load Reduction	35.37	38.52

Table 1 - MUSIC Model Water Balance Results

## 3.4 Flooding

Detailed flood modelling was undertaken as part of the SSD application for the proposed estate subdivision. The figure below has been extracted from the ConstinRoe Consulting Flood Report and demonstrates that the subject site is located outside the flood zone. The flood level for the 1% AEP flood event adjacent to the site has been determined to be approximately 33.000-33.500 AHD. This flood level is below the proposed floor level of 35.500 AHD, with approximately 1.6m of freeboard being provided to the site.



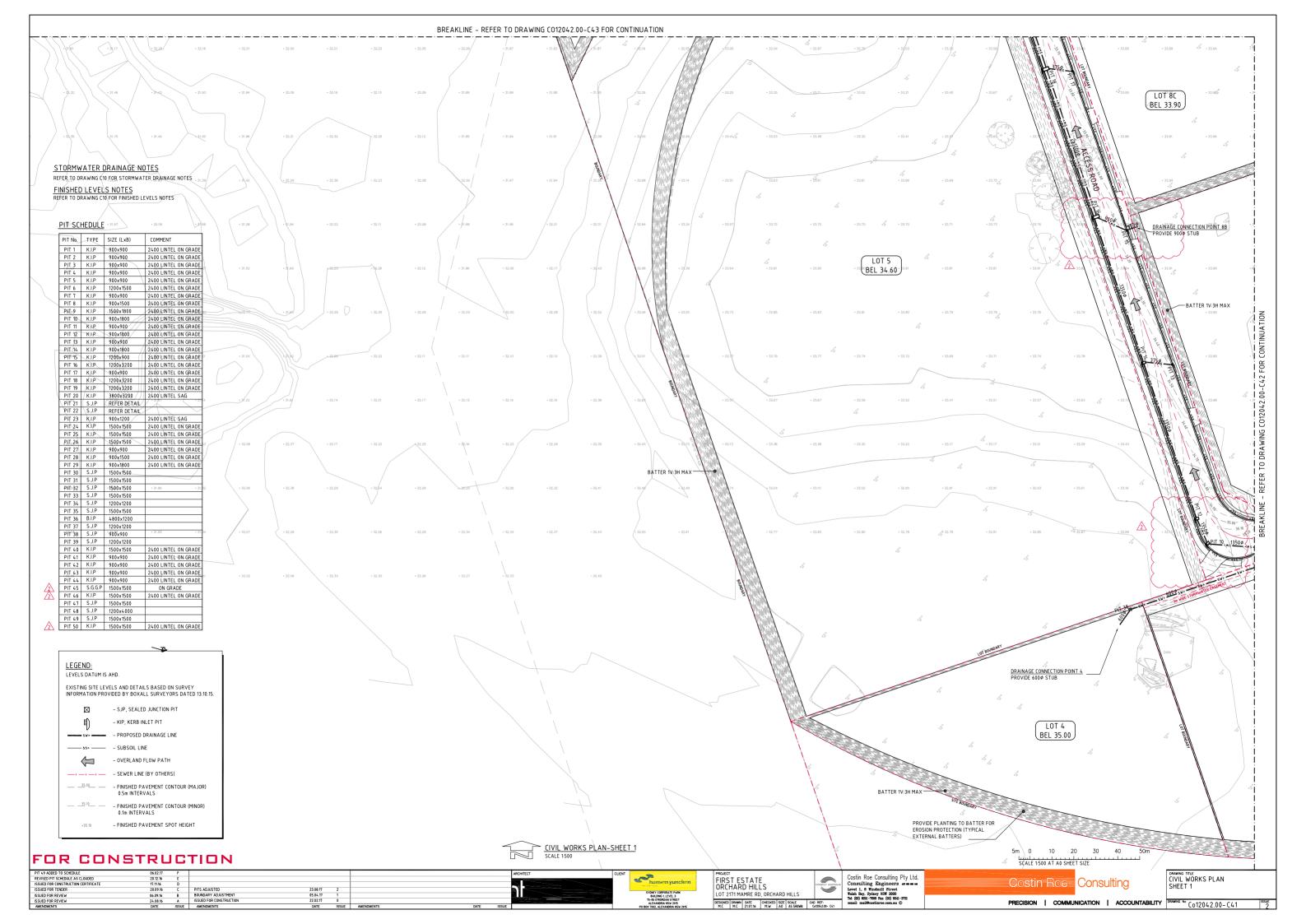


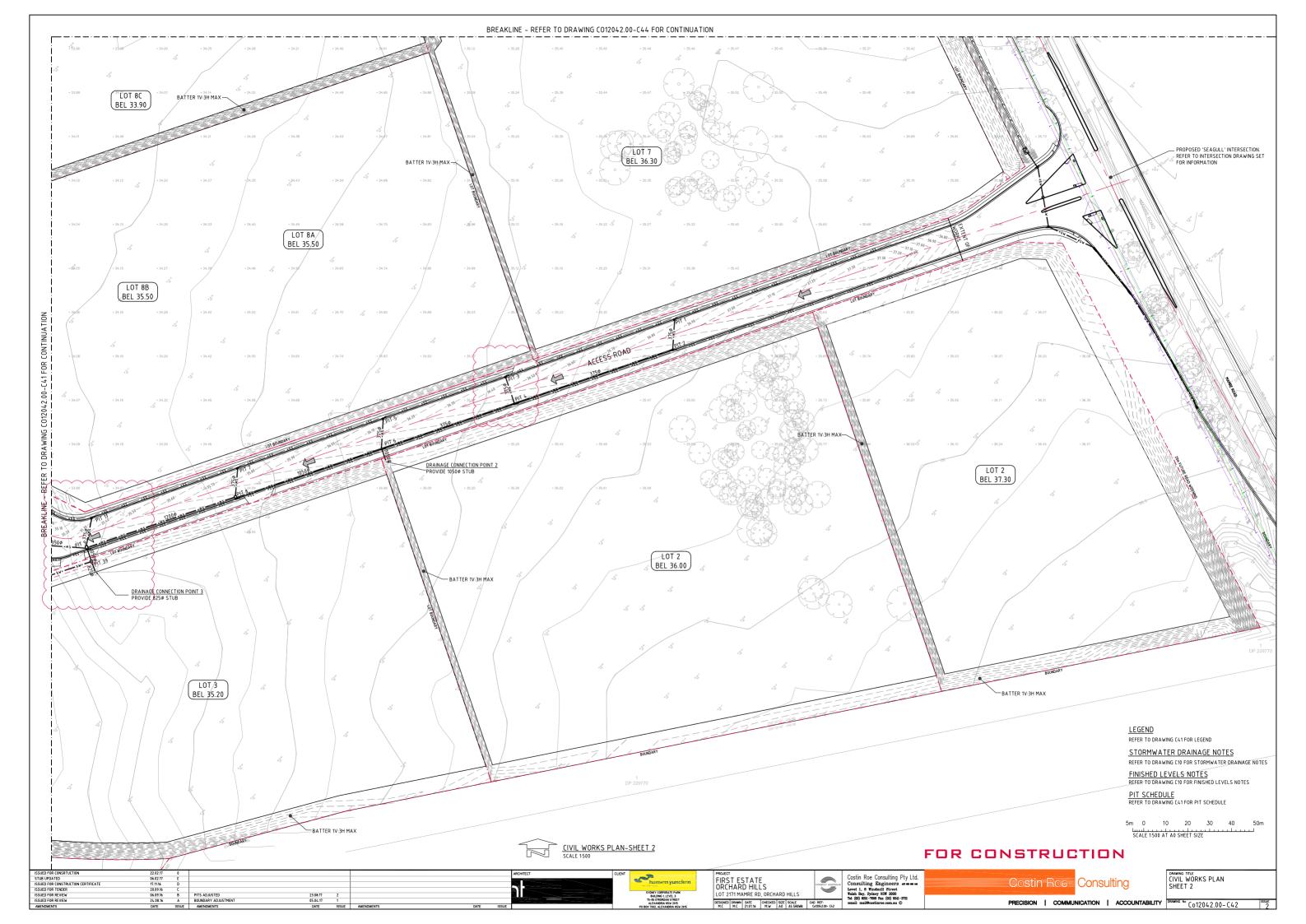
## CONCLUSION

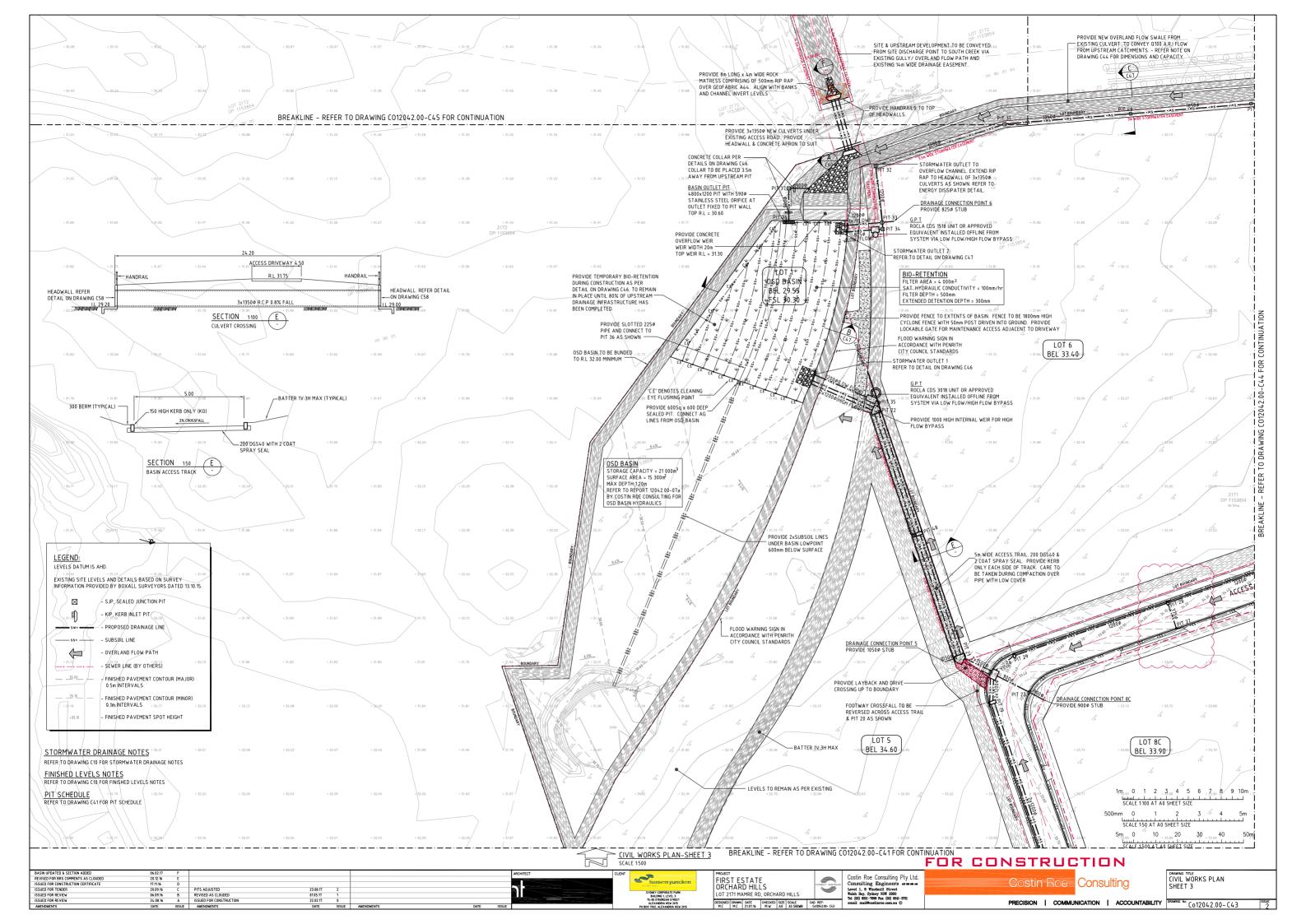
Based on the preparation of the concept stormwater drainage plans and MUSIC modeling results it is demonstrated that the principles of WSUD have been incorporated into the design and operation of the proposed development at Lot 8 585-649 Mamre Rd, Orchard Hills NSW in accordance with PDCP 20014 Part C3 and Section 5 of the MWDCP. It is demonstrated that the proposed development achieves reductions in potable water import by capturing rainwater on site and reusing this for non-potable uses including irrigation and toilet flushing, achieving reuse reduction targets set by council. That the site is located outside the 1%AEP flood extents and not affected by flooding/overland flows.

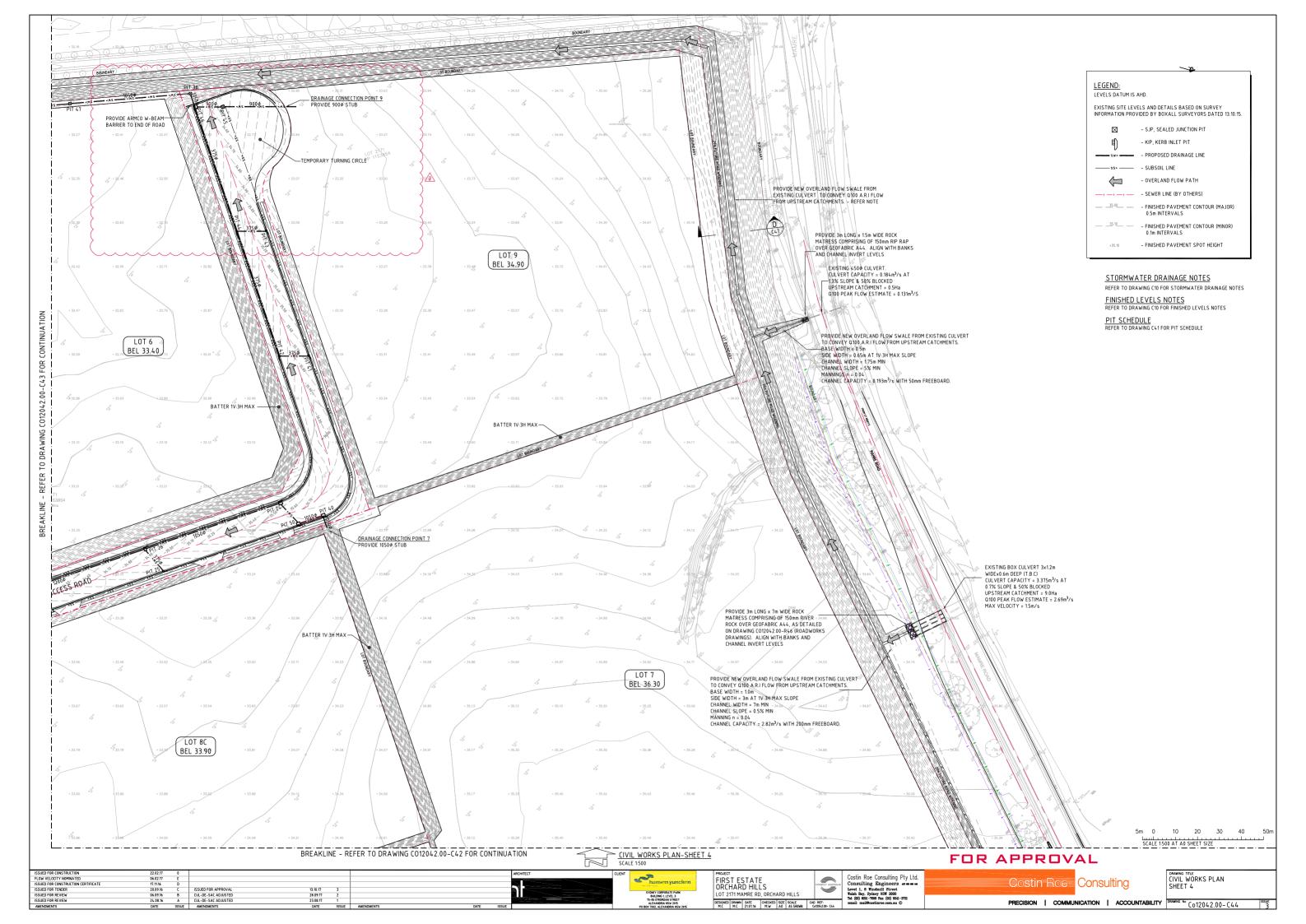


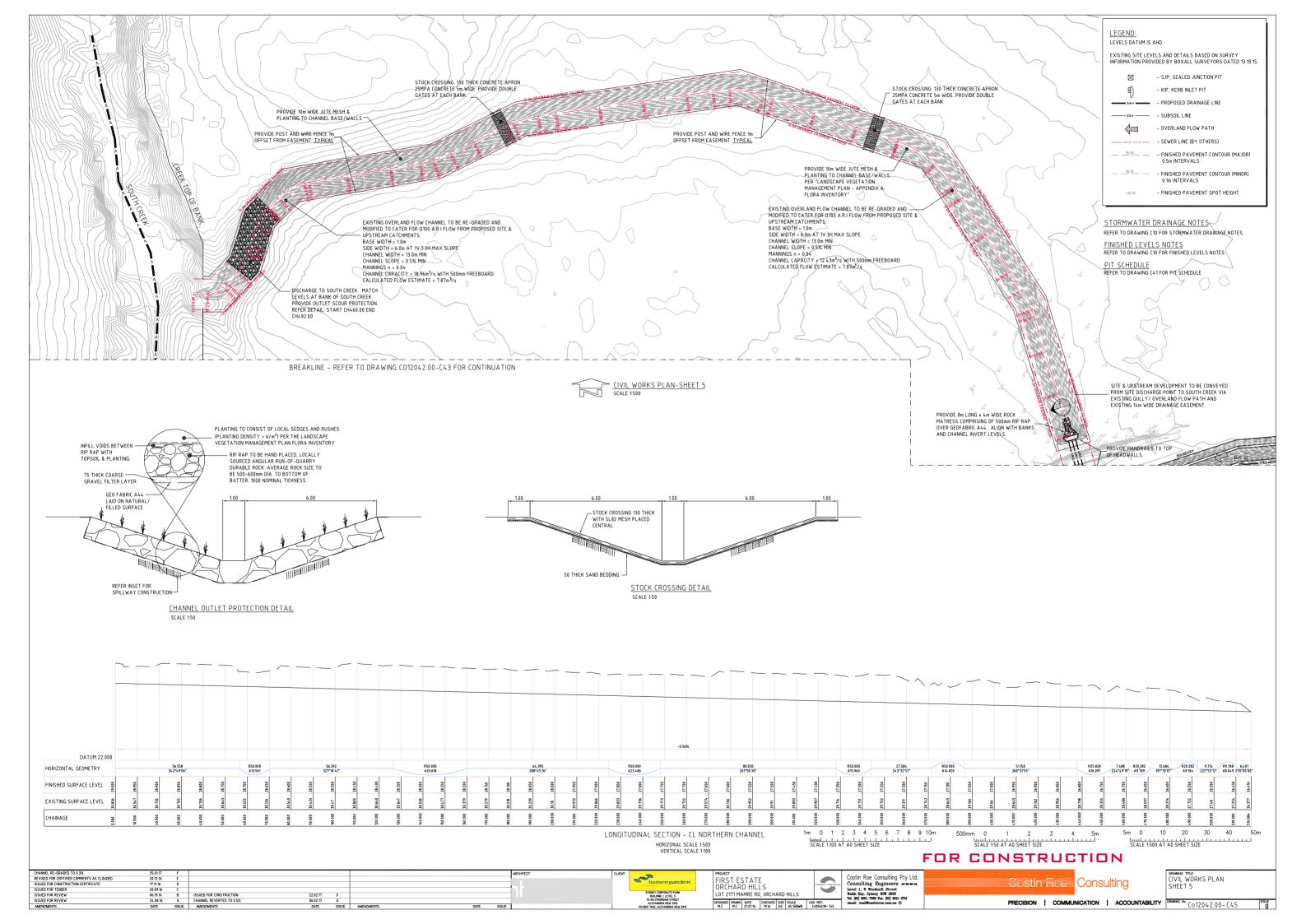
## **APPENDIX A. ESTATE INFRSTRUCTURE PLANS**













## **APPENDIX B. CONCEPT STORMWATER MANAGEMENT PLANS**

## LOT 8 FIRST ESTATE MAMRE RD ORCHARD HILLS CONCEPT STORMWATER MANAGEMENT

#### SITE WORKS - GENERAL

- 1. ALL WORKS ARE TO BE UNDERTAKEN IN ACCORDANCE WITH LOCAL COUNCIL, AUSTRALIAN AND AUTHORITY STANDARDS.
- 2. ALL TRENCHING WORKS ARE TO BE RESTORED TO ORIGINAL CONDITION.
- 3. THE INTEGRITY OF ALL EXISTING AND NEW SERVICES IS TO BE MAINTAINED THROUGHOUT THE CONSTRUCTION PERIOD
- 4. ALL PLANS ARE TO BE READ IN CONJUNCTION WITH APPROVED ARCHITECTS, STRUCTURAL ENGINEERS AND OTHER CONSULTANT'S PLANS. ANY DISCREPANCES ARE TO BE NOTIFIED TO THE ENGINEER FOR

#### SEDIMENT AND EROSION CONTROL

- 1. THE CONTRACTOR SHALL INSTIGATE ALL SEDIMENT AND EROSION CONTROL MEASURES IN ACCORDANCE WITH PENRITH CITY COUNCIL REQUIREMENTS AND THE "BLUE BOOK" (MANAGING URBAN STORMWATER SOILS AND CONSTRUCTION PRODUCED BY THE DEPARTMENT OF HOUSING). THESE MEASURES ARE TO BE REGULARLY INSPECTED AND MAINTAINED.
- 2. THE SEDIMENT & EROSION CONTROL PLAN PRESENTS CONCEPTS ONLY THE CONTRACTOR SHALL AT ALL TIMES BE RESPONSIBLE FOR THE ESTABLISHMENT & MANAGEMENT OF A DETAILED SCHEME MEETING COUNCIL'S DESIGN, AND ALL OTHER REGULATORY AUTHORITY
- 3. WHERE PRACTICAL, THE SOIL EROSION HAZARD ON THE SITE SHALL BE KEPT AS LOW AS POSSIBLE. TO THIS END, WORKS SHOULD BE UNDERTAKEN IN THE FOLLOWING SEQUENCE:
- a. NISTALL ALL TEMPORARY SEDMENT FENCES AND BARRIER FENCES.
  WHERE FENCES ARE ADJACENT TO EACH OTHER THE SEDMENT
  FENCE CAN BE INCORPORATED INTO THE BARRIER FENCE.
- b. CONSTRUCT TEMPORARY STABILISED SITE ACCESS. INCLUDING
- c. INSTALL SEDIMENT CONTROL MEASURES AS OUTLINED ON THESE SEDIMENT AND CONTROL PLANS (ONCE APPROVED)

  4. THE CONTRACTOR SHALL UNDERTAKE SITE DEVELOPMENT WORKS SO
- THAT LAND DISTURBANCE IS CONFINED TO AREAS OF MINIMUM
- 5. AT ALL TIMES AND IN PARTICULAR DURING WINDY AND DRY WEATHER, LARGE, UNPROTECTED AREAS WILL BE KEPT MOIST (NOT WET) BY SPRINKLING WITH WATER TO KEEP DUST UNDER CONTROL. TACIFIERS MAY BE USED TO CONTROL DUST DURING EXTENDED PERIODS OF DRY
- 6. ANY SAND USED IN THE CONCRETE CURING PROCESS (SPREAD OVER THE SURFACE) SHALL BE REMOVED AS SOON AS POSSIBLE AND WITHIN 10 WORKING DAYS FROM PLACEMENT.
- 7. WATER SHALL BE PREVENTED FROM ENTERING THE PERMANENT DRAINAGE SYSTEM UNLESS THE CATCHMENT AREA HAS BEEN STABILISED AND/OR ANY LIKELY SEDMENT HAS BEEN FILTERED OUT.
- 8. TEMPORARY SOIL AND WATER MANAGEMENT STRUCTURES SHALL BE REMOVED ONLY AFTER THE LANDS THEY ARE PROTECTING ARE STABILISED / REHABILITATED.
- 9. THE CONTRACTOR SHALL ALLOW FOR THE ESTABLISHMENT OF ANY OTHER EROSION PROTECTION MEASURES. (IF APPLICABLE).
- IOTHE CONTRACTOR SHALL REGULARLY INSPECT (AINTIMUM TWICE PER WEEK) ALL EROSION AND SEDIMENT CONTROL MEASURES TO ENSURE THEY ARE OPERATING EFFECTIVELY. REPAIRS AND/OR MAINTENANCE SHALL BE UNDERTAKEN REGULARLY AND AS REQUIRED, PARTICULARLY FOLLOWING STORM EVENTS.
- 11.ACCEPTABLE RECEPTORS SHALL BE USED FOR CONCRETE AND MORTAR SUURRIES, PAINTS, ACID WASHINGS, LICHT-WEIGHT WASTE MATERIALS AND LITTER WASTE FROM THESE RECEPTORS SHALL BE DISPOSED OF IN ACCORDANCE WITH REGULATORY AUTHORITY REQUIREMENTS, PAY ALL FEES AND PROVIDE EVIDENCE OF SAFE DISPOSAL

#### SITE WORKS - ACCESS AND SAFETY

- 1. ALL WORKS ARE TO BE UNDERTAKEN IN A SAFE MANNER IN ACCORDANCE WITH ALL STATUTORY AND INDUSTRIAL RELATION REQUIREMENTS.
- 2. ACCESS TO ADJACENT BUILDINGS AND PROPERTIES SHALL BE MAINTAINED AT ALL TIMES.
- 3. WHERE NECESSARY SAFE PASSAGE SHALL BE PROVIDED FOR VEHICLES AND PEDESTRIANS THROUGH OR ADJACENT TO THE SITE

#### **STORMWATER**

- 1 ALL WORKS ARE TO BE UNDERTAKEN IN ACCORDANCE WITH THE FOLLOWING AUSTRALIAN STANDARDS AS2032, AS3500 AND AS3725 AS
- 2. ALL PIPES LESS THAN OR ECLAL TO Ø300mm IN SIZE ARE TO BE
- SOLVENT WELD-JOINTED UPVC CLASS SN6 U.N.O.

  3. ALL PIPES @375mmCR CREATER IN SIZE ARE TO BE MN CLASS 2 RENFORCED CONCRETE PIPE (RCP) WITH SPIGGOT AND SOCKETED JOINT OR VANTAGE PIPE PLUS (VPIPE+) FIBRE RENFORCED CONCRETE (FRC) WITH VANTAGE PIPE PLUS JOINT U.N.O.
- 4. ALL PIPES ARE TO BE LAID AT MIN. 1.0% GRADE U.N.O.
- 5. PIPE BEDDING IS TO BE HS2 UNDER ROADS AND TRAFFICKED AREAS AND SHALL BE H2 IN LANDSCAPED AND PEDESTRIAN TRAFFICKED AREAS U.N.O.
- 6. ALL PIPE BENDS AND JUNCTIONS ARE TO BE MADE WITH EITHER PURPOSE MADE FITTINGS OR STORMWATER DRAINAGE PITS.
- 7. MINIMUM COVER FROM THE OBVERT OF THE STORMWATER PIPE OF 300mm IS TO BE PROVIDED IN LANDSCAPED AREAS AND 600mm IN VEHICULAR TRAFFICKED AREAS U.N.O.
- 8. WHERE MINMOM COVER CANNOT BE ACHIEVED CONCRETE ENCASEMENT OF THE AFFECTED PIPE IS MAY BE UNDERTAKEN WITH 20MPA CONCRETE WITH A MIN. COVER OF 150mm TO ALL SIDES OF THE PIPE. THE CONTRACTOR SHALL CONFRM THIS REQUIREMENT WITH THE ENGINEER OR SUPERINTENDENT.
- 9. LAID PIPELINES ARE TO HAVE THE FOLLOWING CONSTRUCTED
- a. HORIZONTAL-1:300 ANGULAR DEVIATION FROM REQUIRED ALIGNMENT; b. VERTICAL-1:300 ANGULAR DEVIATION FROM REQUIRED ALIGNMENT.
- 10. ALL DRAINAGE PITS ARE TO BE CAST IN-SITU, PRECAST DRAINAGE THIS MAY BE USED WITH APPROVAL FROM THE ENGINEER. THE CONTRACTOR SHALL SUBMIT A PRECAST PIT INSTALLATION WORK METHOD STATEMENT FOR ASSESSMENT BY THE ENGINEER FOR APPROVAL.
- 11. DRAINAGE PIT COVERS ARE TO BE EITHER GALVANISED STEEL OR CAST IRON CLASS 'B' IN LANDSCAPED AND PEDESTRIAN TRAFFICKED AREAS AND CLASS 'D' IN ALL VEHICULAR TRAFFICKED AREAS U.N.O.
- 12. DRAINAGE PIT COVERS ARE TO BE 'HEELSAFE' TYPE IN ALL PEDESTRIAN TRAFFICKED AREAS U.N.O.
- 13. EXISTING STORMWATER PIT LOCATIONS AND INVERT LEVELS TO BE CONFIRMED PRIOR TO COMMENCING WORKS ON SITE. 14. PROVIDE CLEANING EYES (RODDING POINTS) TO PIPES AT ALL CORNERS
- AND T-JUNCTIONS WHERE NO PITS ARE PRESENT 15. DOWN PIPES CONNECTED DIRECT TO PIPES TO BE CONNECTED AT 45° TO THE FLOW DIRECTION WITH A CLEANING EYE PROVIDED AT GROUND

#### FINISHED LEVELS

- 1. LEVELS BASED ON ESTATE INFRASTRUCTURE PLANS PREPARED BY COSTINBOE CONSULTING, Ref: 12042.00 & SURVEY PREPARED BY BOXALL SURVEYORS Ref: 10116. THE CONTRACTOR SHALL VERIFY LEVELS PRIOR TO CONSTRUCTION COMMENCEMENT ANY DISCREPANCIES SHALL BE NOTIFIED TO THE ENGINEER OR SUPERINTENDENT FOR CLARIFICATION.
- 2. FINISHED LEVELS SHOWN ARE CONCEPTUAL ONLY AND SUBJECT TO DETAILED DESIGN PRIOR TO CONSTRUCTION CERTIFICATE APPLICATION. FINAL FINISHED LEVELS TO BE +/- 0.5m FROM LEVELS SHOWN.
- 3. CARPARK & SERVICE AREA LAYOUT AND GRADES TO COMPLY WITH
- 4. DRIVEWAY LAYOUT AND DESIGN TO COMPLY WITH PENRITH CITY COUNCIL ACCESS DRIVEWAY DESIGN AND CONSTRUCTION SPECIFICATION.
- 5. ALL CONTOUR LINES & SPOT LEVELS INDICATE FINISHED PAVEMENT
- 6. PERMANENT BATTER SLOPES ARE TO HAVE A MAXIMUM GRADE OF
- 7. ALL FOOTPATHS ARE TO FALL AWAY FROM THE BUILDING AT 2.5% NOMINAL GRADE.
- 8. ALL PAVEMENTS ARE TO BE SET AT 50mm BELOW THE FINISHED FLOOR



LOCALITY PLAN NOT TO SCALE - COURTESY OF SIX MAPS

#### DRAWING SCHEDULE

DATOL COVER SHEET, LOCALITY PLAN AND DRAWING SCHEDULE

DA2.01 CONCEPT SEDIMENT AND EROSION CONTROL PLAN

DA2.02 SEDIMENT AND EROSION CONTROL DETAILS

DA4.01 CONCEPT GRADING AND DRAINAGE PLAN SHEET 1 DA4.02 CONCEPT GRADING AND DRAINAGE PLAN SHEET 2

DA4.03 CONCEPT GRADING AND DRAINAGE PLAN SHEET 3

DA4.04 CONCEPT GRADING AND DRAINAGE PLAN SHEET 4

DA4.11 CONCEPT TYPICAL SECTIONS SHEET 1

DA4.12 CONCEPT TYPICAL SECTIONS SHEET 2

#### ISSUED FOR SSD APPLICATION

DATE No AMENDMENT DATE No AMENDMENT 21 06 18 ISSUED FOR SSD APPLICATION SK 1 June 21, 2013 NOT TO SCALE





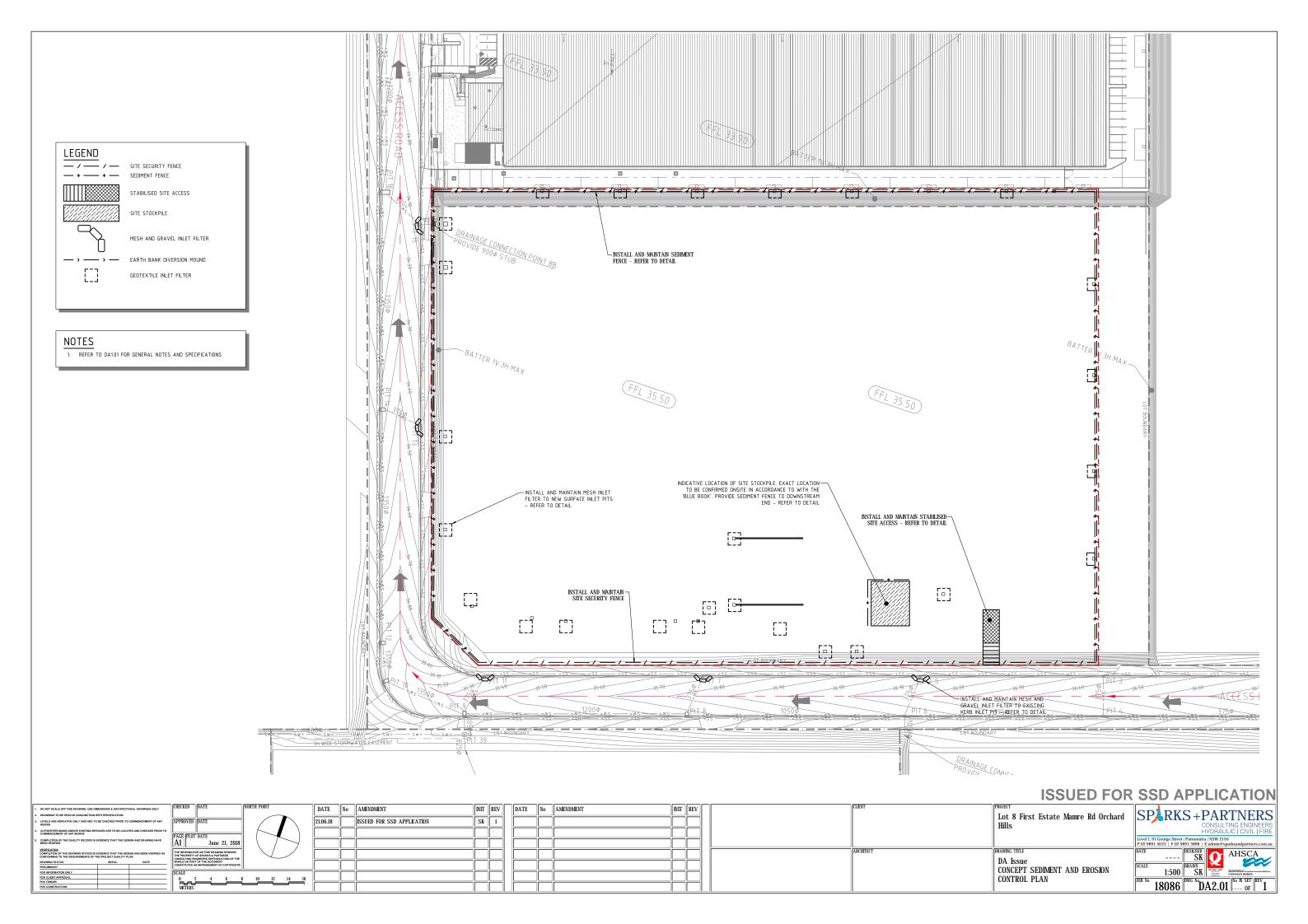


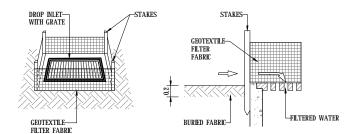
Orchard Hills DA Issue COVER SHEET, LOCALITY PLAN

AND DRAWING SCHEDULE

Lot 8 First Estate Mamre Rd

SPÅRKS+PARTNERS NTS DEAWN SK CONTRACT GROWN CONTRACT 18086 DWG No DA1.01 No IN SET REV





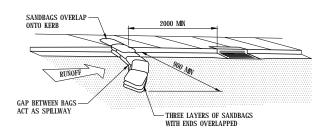
## GEOTEXTILE INLET FILTER DROP INLET SEDIMENT TRAP

#### NOTES:

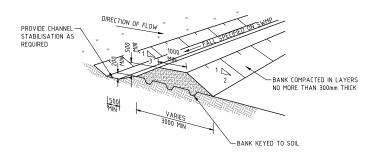
- FARRICATE A SEDIMENT BARRIER MADE FROM CEOTEXTILE OR STRAW BALES
- PADRICATE A SEDIMENT DARKER WHILE TROUGH EDUTE ALLE OR STRAW DALES.
  CUT A 200mm DEEP TRENCH ALONG THE UPSLOPE LINE OF THE FENCE FOR THE BOTTOM OF
  THE FABRIC TO BE ENTRENCHED.
  DRIVE 1.0m LONG STAR PICKETS INTO GROUND AT THE FOUR CORNERS OF PIT WALLS.
- ENSURE ANY STAR PEKETS ARE FITTED WITH SAFETY CAPS.
  FIX SELF-SUPPORTING GEOTEXTILE TO THE UPSLOPE SIDE OF THE POSTS ENSURING IT GOES
  TO THE BASE OF THE TRENCH. FIX THE GEOTEXTILE WITH WIRE TIES OR AS RECOMMENDED BY THE MANUFACTURER, ONLY USE GEOTEXTILE SPECIFICALLY PRODUCED FOR SEDIMENT
- FENCING. THE USE OF SHADE CLOTH FOR THIS PURPOSE IS NOT SATISFACTORY.

  JOIN SECTIONS OF FABRIC AT A SUPPORT POST WITH A 150mm OVERLAP.

  BACKFILL THE TRENCH OVER THE BASE OF THE FABRIC AND COMPACT IT THOROUGHLY
- OVER THE GEOTEXTILE.

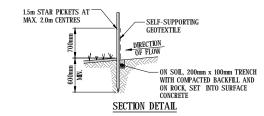


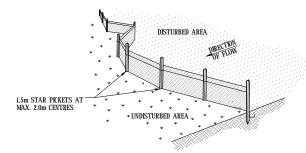
## SEDIMENT TRAP FOR KERB INLET (ON GRADE - SANDBAG)

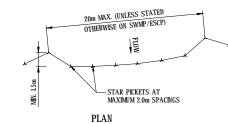


#### DIVERSION BANK AND CHANNEL

(FOR CATCHMENT GREATER THAN 2ha



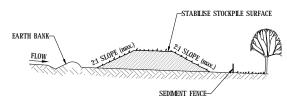




## SEDIMENT FENCE

#### NOTES:

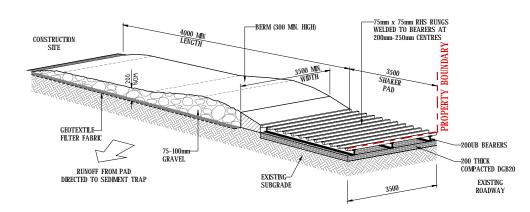
- CONSTRUCT SEDIMENT FENCES AS CLOSE AS POSSIBLE TO BEING PARALLEL TO THE CONTOURS OF THE SITE, BUT WITH SMALL RETURNS AS SHOWN IN THE DRAWING TO LIMIT THE CATCHMENT AREA OF ANY ONE SECTION. THE CATCHMENT AREA SHOULD BE SMALL ENOUGH TO LIMIT WATER FLOW IF CONCENTRATED AT ONE POINT TO 50L/s IN THE DESIGN STORM EVENT. USUALLY THE 10-YEAR EVENT.
- CUT A 200mm DEEP TERNICH ALONG THE UPSLOPE LINE OF THE FENCE FOR THE BOTTOM OF THE FABRIC TO BE ENTRENCHED. DRIVE 1.5m LONG STAR PICKETS INTO GROUND AT 2.0m INTERVALS
- (MAX) AT THE DOWNSLOPE EDGE OF THE TRENCH ENSURE ANY STAR PICKETS ARE FITTED WITH SAFETY CAPS.
  FOX SELF-SUPPORTING GEOTEXTILE TO THE UPSLOPE SIDE OF THE POSTS ENSURING IT GOES TO THE BASE OF THE TRENCH FIX THE GEOTEXTILE WITH WIRE TIES OR AS RECOMMENDED BY THE MANUFACTURER. ONLY USE GEOTEXTILE SPECIFICALLY PRODUCED FOR SEDIMENT FENCING. THE USE OF SHADE CLOTH FOR THIS PURPOSE IS NOT SATISFACTORY.
- JOIN SECTIONS OF FABRIC AT A SUPPORT POST WITH A 150mm OVERLAP.
- BACKFILL THE TRENCH OVER THE BASE OF THE FABRIC AND COMPACT IT THOROUGHLY OVER THE GEOTEXTILE.



## STOCKPILE

#### NOTES:

- PLACE STOCKPILES MORE THAN 2 (PREFERABLY 5) METRES FROM EXISTING VEGETATION, CONCENTRATED WATER FLOW, ROADS AND HAZARD AREAS. CONSTRUCT ON THE CONTOUR AS LOW, FLAT, ELONGATED MOUNDS.
- WHERE THERE IS SUFFICIENT AREA, TOPSOIL STOCKPILES SHALL BE LESS THAN 2
- WHERE THE HEIGHT.
  WHERE THEY ARE TO BE IN PLACE FOR MORE THAN 10 DAYS, STABILISE FOLLOWING THE
- APPROVED ESCP OR SWMP TO REDUCE THE C-FACTOR TO LESS THAN 0.10. CONSTRUCT EARTH BANKS ON THE UPSLOPE SIDE TO DIVERT WATER AROUND STOCKPILES AND SEDMENT FENCES 1 TO 2 METRES DOWNSLOPE.



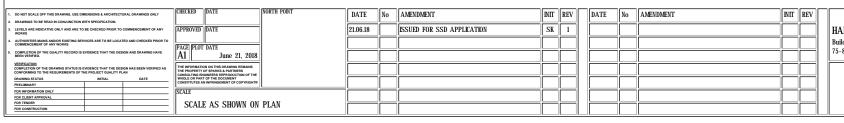
## STABILISED SITE ACCESS

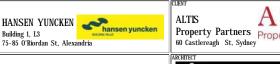
#### MAINTENANCE

- THE TEMPORARY ACCESS SHALL BE MAINTAINED IN A CONDITION THAT PREVENTS TRACKING OR FLOWING OF SEDIMENT
- ONTO PUBLIC RICHTS OF WAY,

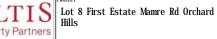
  THIS MAY REQUIRE PERIODIC TOP DRESSING WITH ADDITIONAL GRAVEL AS CONDITIONS DEMAND AND REPAIR AND/OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT,
- ALL SEDMENT SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC RIGHTS OF WAY MUST BE REMOVED IMMEDIATELY. NSTALL BARRER ON EITHER SIDE OF SHAKER PAD TO ENSURE VEHICLES ARE GUIDED ON TO THE PAD.
- NVERT OF SHAKER PAD TO BE DRANED VIA AGRICULTURAL PIPE WRAPPED IN GEOTEXTILE FABRIC.

## **ISSUED FOR SSD APPLICATION**





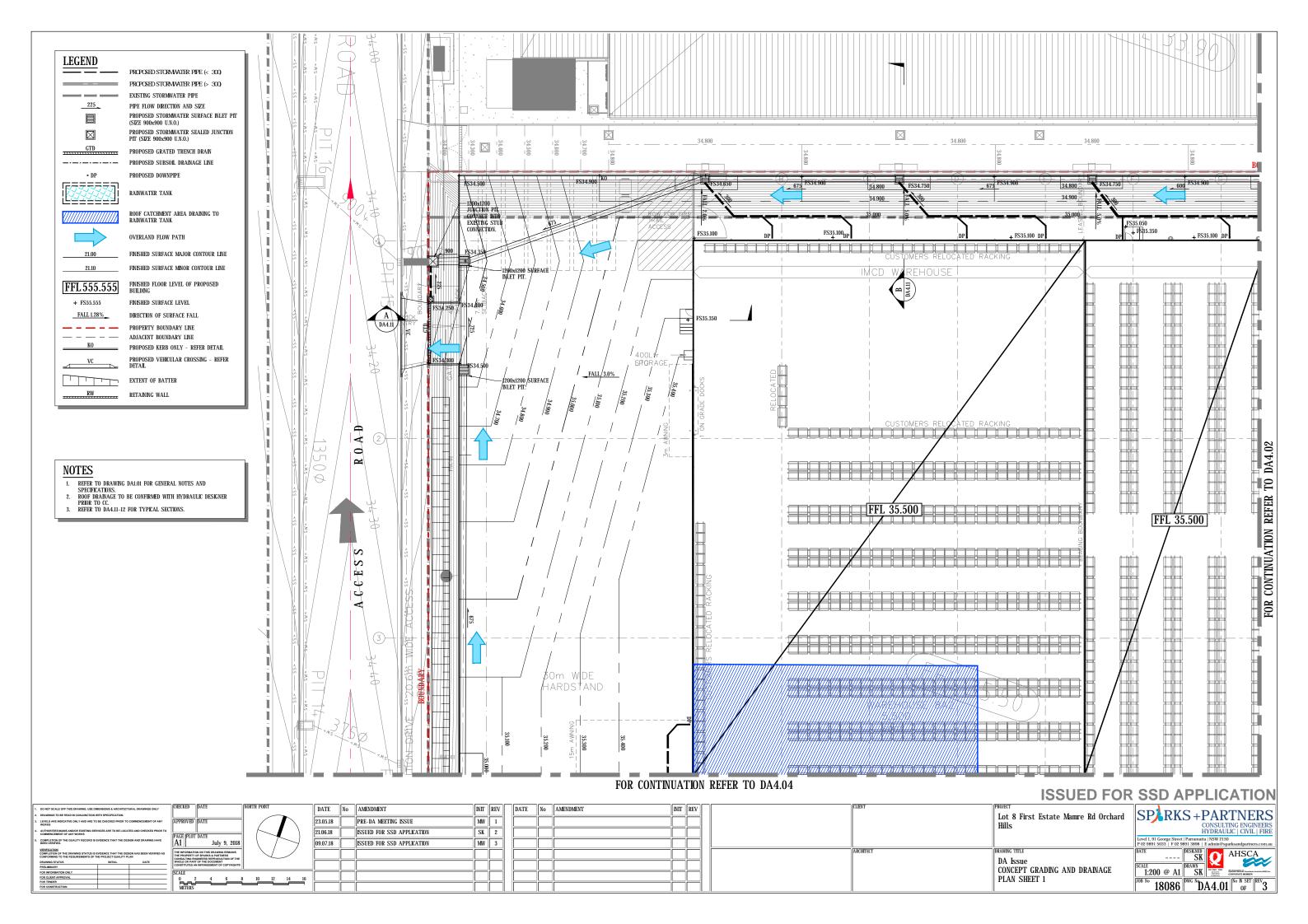


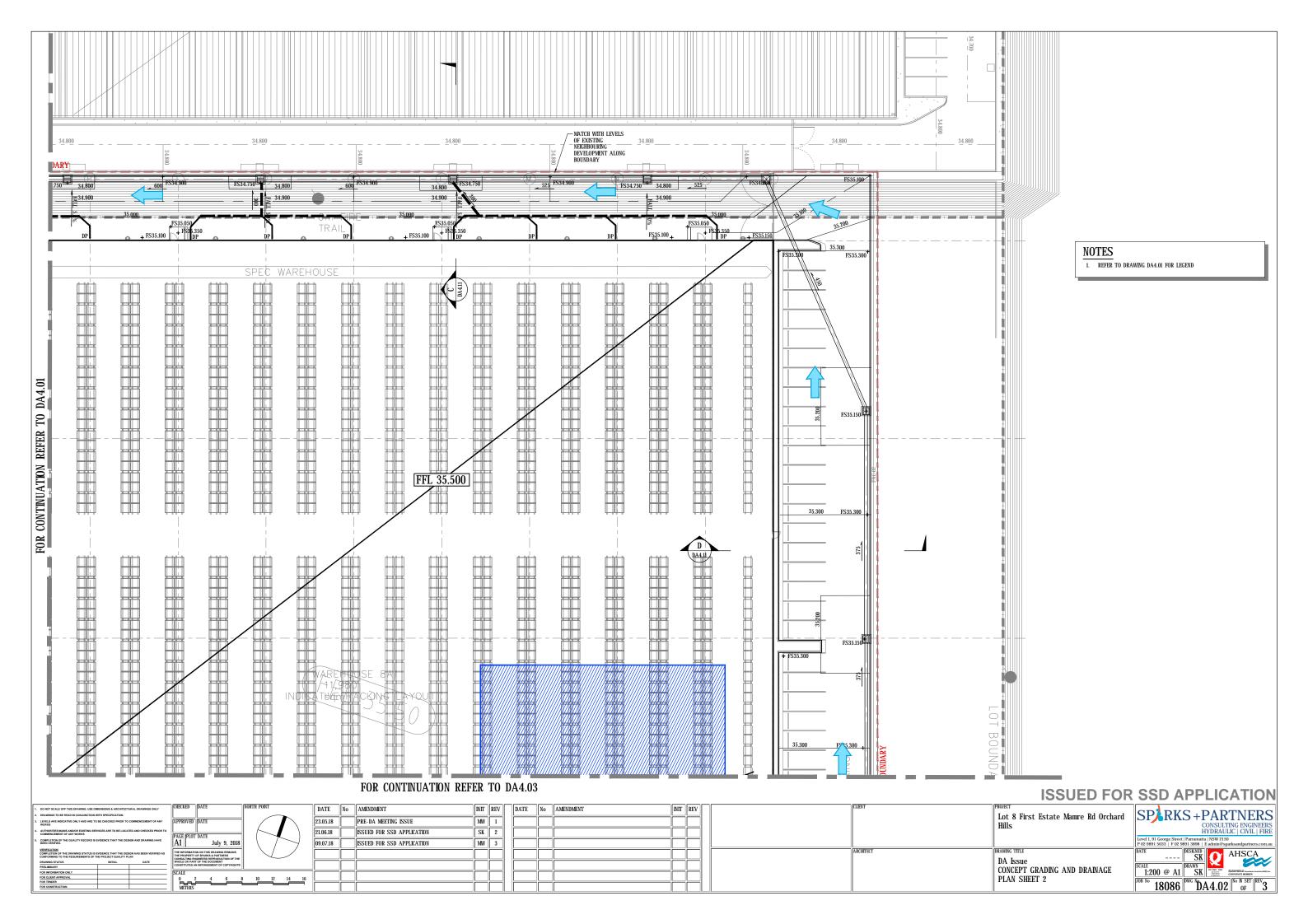


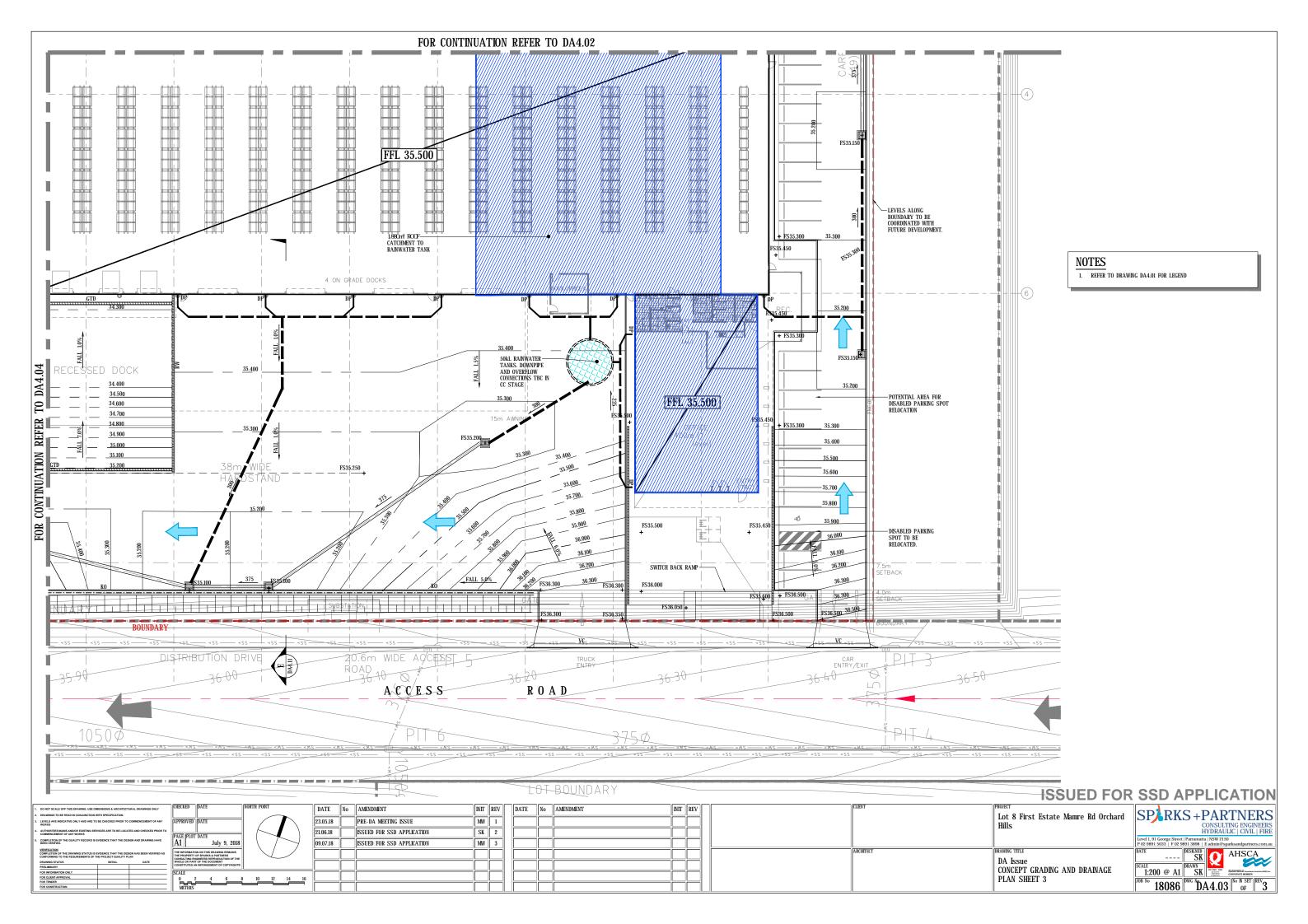
DA Issue

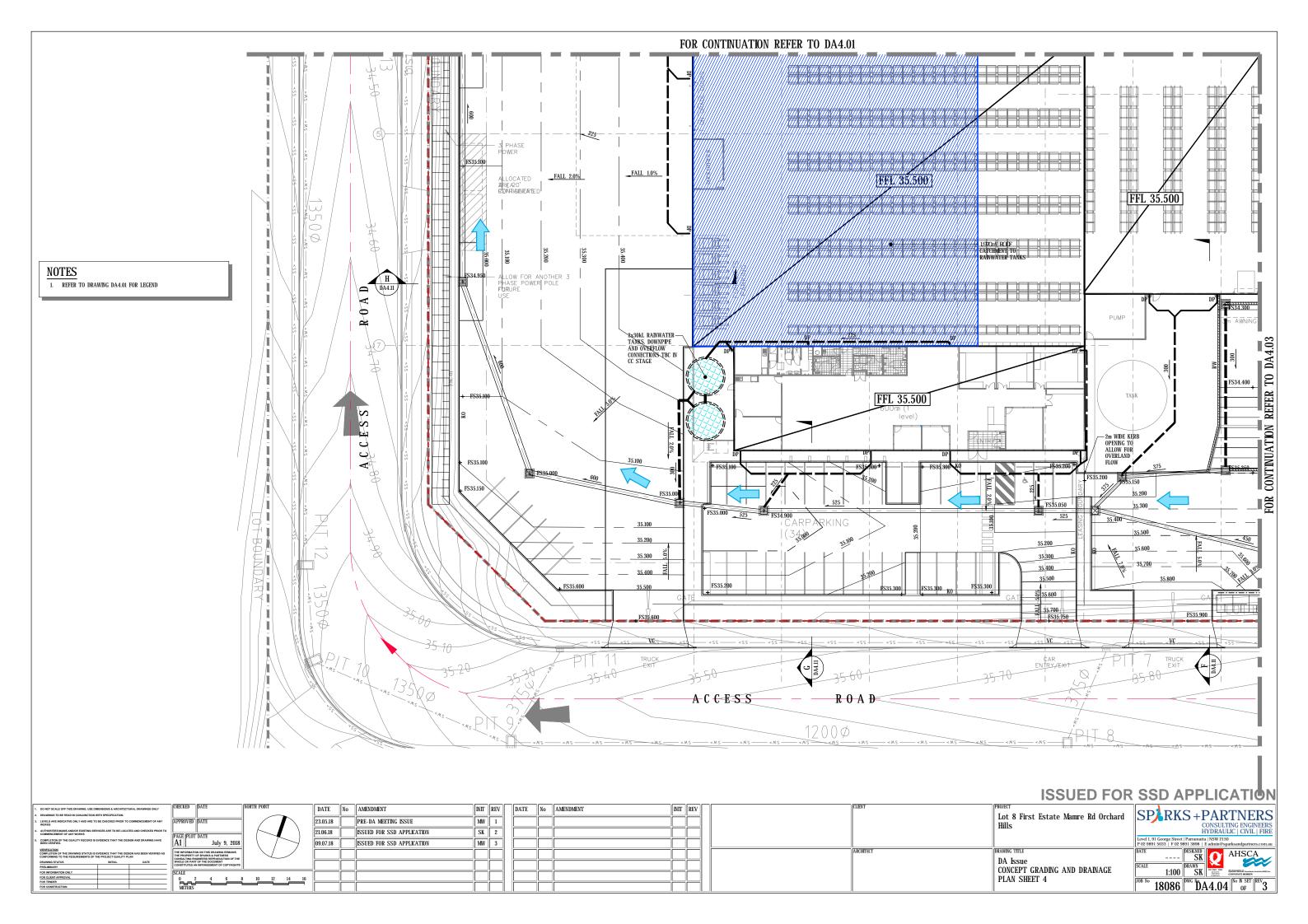
DETAILS

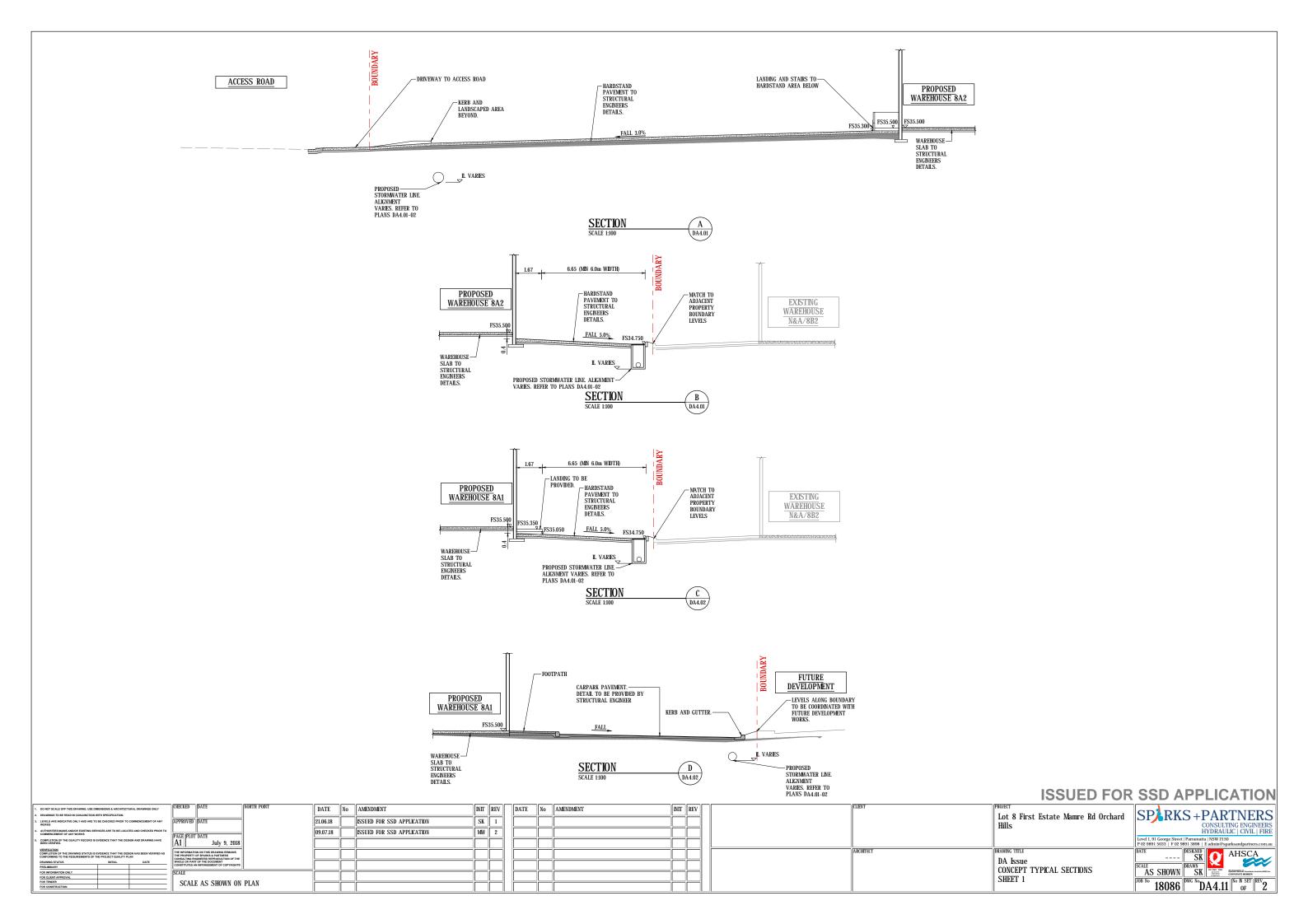


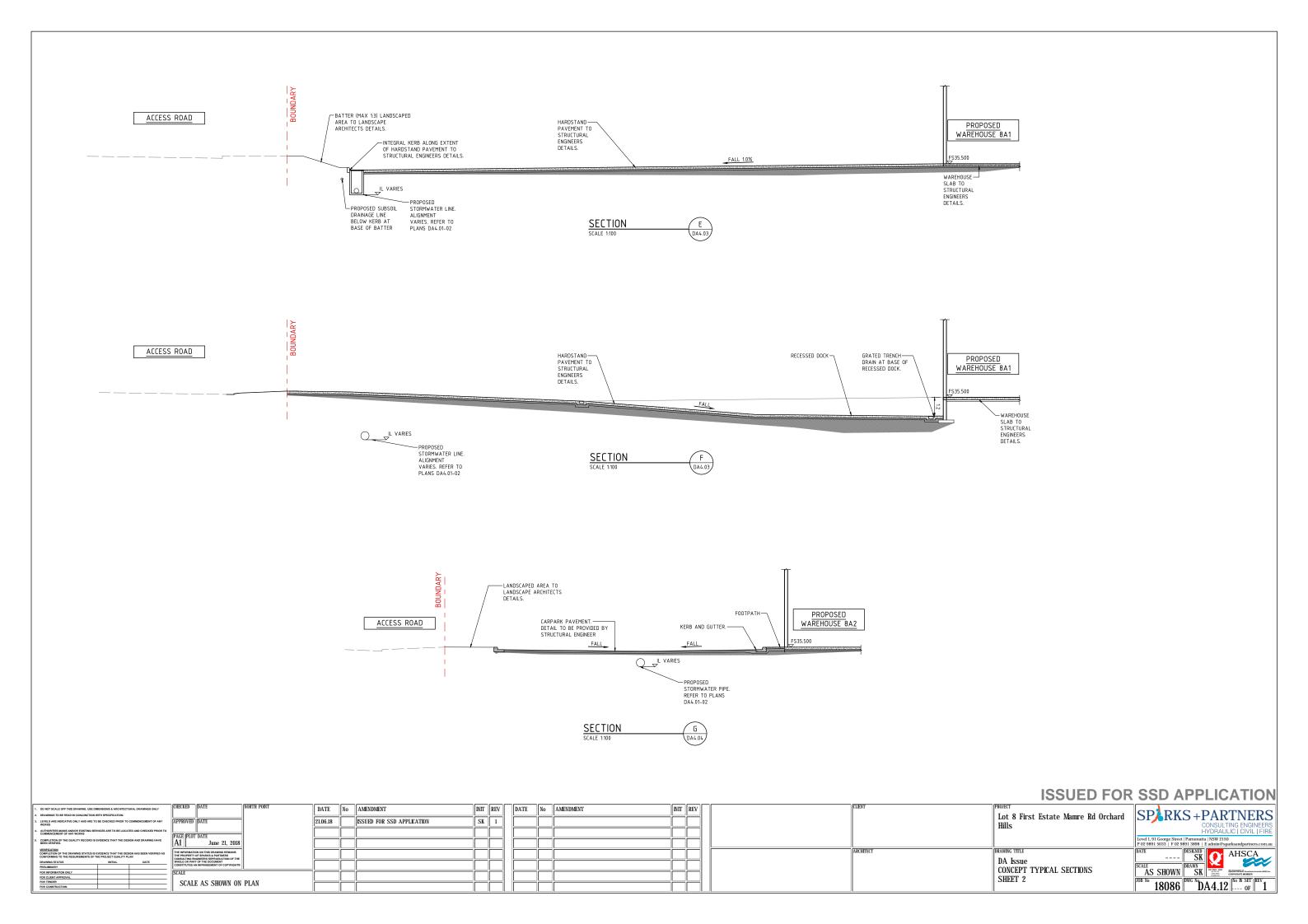














## **APPENDIX C. MUSICLINK REPORT**





#### MUSIC-link Report

Project Details Company Details

**Project:** 18081 **Company:** Sparks and Partners Consulting Engineers

Report Export Date: 21/06/2018 Contact: Simon Kapsis

Catchment Name: 18086\_MUSIC Model-RWT Sizing Address: 91 George St Parramatta

 Catchment Area:
 0.385ha
 Phone:
 9891 5033

 Impervious Area\*:
 100%
 Email:
 simon@sparksandpartners.com.au

**Rainfall Station:** 67113 PENRITH

Modelling Time-step: 6 Minutes

**Modelling Period:** 1/01/1999 - 31/12/2008 11:54:00 PM

Mean Annual Rainfall:691mmEvapotranspiration:1158mmMUSIC Version:6.3.0MUSIC-link data Version:6.31Study Area:Penrith

Scenario: Penrith Development

<sup>\*</sup> takes into account area from all source nodes that link to the chosen reporting node, excluding Import Data Nodes

Treatment Train Effectiveness		Treatment Nodes		Source Nodes	
Node: Receiving Node	Reduction	Node Type	Number	Node Type	Number
Row	37%	Rain Water Tank Node	2	Urban Source Node	2
TSS	56.7%				
TP	42%				
TN	43%				
GP CP	100%				

#### Comments

Water quality has not ben modelled for the development as estate treatment measures are in place. Reference is made to SD 15\_7173, and report prepared by CostinRoe Consulting, ref: CO12042.0. The modeling provided is for determining the efficiency of the proposed rainwater tanks only in accordance with Penrith Development Control Plan 2014 (PDCP) Part C3-Water Management.





Node Type	Node Name	Parameter	Min	Max	Actual
Noue Type	Node Name	Faiailletei	IVIIII	IVIAX	Actual
Rain	Rainwater Tank 50KL	% Reuse Demand Met	80	None	85.90
Rain	Rainwater Tank 60KL	% Reuse Demand Met	80	None	86.02
Receiving	Receiving Node	% Load Reduction	None	None	37
Receiving	Receiving Node	GP % Load Reduction	90	None	100
Urban	8A1 Roof 1880sq.m	Area Impervious (ha)	None	None	0.188
Urban	8A1 Roof 1880sq.m	Area Pervious (ha)	None	None	0
Urban	8A1 Roof 1880sq.m	Total Area (ha)	None	None	0.188
Urban	8A2 Roof 1970sq.m	Area Impervious (ha)	None	None	0.197
Urban	8A2 Roof 1970sq.m	Area Pervious (ha)	None	None	0
Urban	8A2 Roof 1970sq.m	Total Area (ha)	None	None	0.197





Failing Parameters							
Node Type	Node Name	Parameter	Min	Max	Actual		
Receiving	Receiving Node	TN % Load Reduction	45	None	43		
Receiving	Receiving Node	TP % Load Reduction	60	None	42		
Receiving	Receiving Node	TSS % Load Reduction	85	None	56.7		
Only certain parameters are reported when they pass validation							