

## **Appendix H: Preliminary Road Investigation**



# RYE PARK WINDFARM

Preliminary Road Upgrade Investigation

Version 4, 3 April 2020

### Rye Park Windfarm

#### Preliminary Road Upgrade Investigation

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#### 1. Introduction

#### 1.1 Background

The Rye Park Wind Farm is an approved \$700 million development north of Yass and east of Boorowa near the village of Rye Park. The project area extends into the three Local Government Areas (LGA's) of Hilltops, Upper Lachlan (ULSC) and Yass Valley (YVC).

The Rye Park Wind Farm received development consent in May 2017 and since this time there have been advancements in wind turbine technology and the proponents are intending to submit an application to modify the current development consent to incorporate the use of the latest, most efficient wind turbines on the project.

The current consent contains a number of options with regard to access routes for the site. In order to provide certainty and allow more detailed environmental assessment to be undertaken, it is intended to identify and commit to a preferred route/s as part of the proposed modification application.

In order to narrow down options for road access and obtain further agreement with partner Council's, a preliminary road investigation is required to assess the various access options and develop high level cost estimates which will assist decision making processes.

#### 1.2 Scope and Objectives

The scope of the project is to undertake visual inspections of each of the proposed access routes to:

- Identify likely upgrade requirements along each road section;
- Identify bridges and major culverts which may require upgrade;
- Locate obvious areas where road upgrades may lead to works within private property which would require land acquisition;
- Identify approximate areas of impact on vegetation as a result of any identified upgrade work;
- Determine high level cost estimates for identified upgrade works.

The objective of the above scope of works is to provide sufficient high level information on each of the various access routes to enable discussion and consideration of the most appropriate route/s for the project moving forward. This will allow adoption of a preferred route/s and enable completion of detailed environmental assessment, and engineering design such that early agreement can be reached with the partner Council's.

#### 1.3 Route Options

A project locality plan showing proposed wind turbine locations is included as **Attachment A**. A plan showing the current approved access route options which includes the project site boundary, access route options, and LGA boundaries has been included as **Attachment B**.

For the purposes of this assessment the access routes have been broken into six separate road sections as follows:

Road Section No.	Description	Approx. Length
1	Jerrawa Rd/Coolalie Rd/Bushs Rd Roads from Hume Highway to southern end of site	13.5km
2	Rye Park/Dalton Rd from site access 12, to site access 13	11km
3	Rye Park/Dalton Rd from the Boorowa-Rye Park Road intersection to site access 12	14.7km
4	Grassy Creek Rd from Boorowa-Rye Park Rd to site access 2	5.3km
5	Grassy Creek Rd from site access 2 to site access 10	1.2km
6	Trucking Yard Road /Long Street/Boorowa-Rye Park Rd from Lachlan Valley Way to Yass Street in Rye Park	22km

Table 1 – Sections of Road Assessed

The following map shows the location of each of the six road sections:

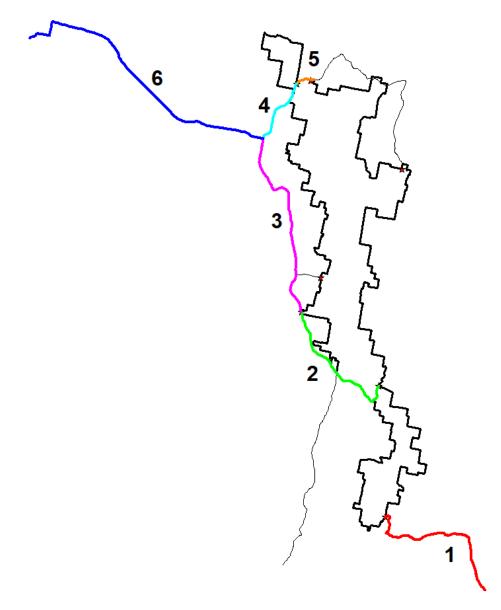


Figure 1 – Sections of Road Assessed

### 2. Existing Road Conditions

#### 2.1 Jerrawa Road

Jerrawa Road runs between the Hume Highway and the village of Jerrawa and is 4.1km long. The road is sealed and is generally 6.0m wide with some sections up to 6.5m. The road is generally in satisfactory condition with some sections showing signs of age with potholes and minor pavement defects present.

The road pavement is likely to have been constructed of local materials and would be unlikely to have sufficient pavement strength or to cater for large numbers of heavy vehicles. In addition the existing narrow road width would not be suitable for use by heavy vehicles.

Upper Lachlan Council has indicated that all roads proposed for use as access roads for the wind farm, would need to be brought up to current Austroads standards including widening and pavement strengthening.

The road reserve is heavily vegetated and contains Box Gum Woodland which is identified as a Threatened Ecological Community.

#### 2.2 Coolalie Road

Coolalie Road runs from Jerrawa, west to Yass with the section between Jerrawa and Bushs Road (6.6 km) proposed for use as an access route for the wind farm. The first 880m of the road is sealed but is very narrow at 5.0 - 6.0 m wide. The remainder of the road is unsealed and is generally 6.0 - 6.5 m within ULSC and 7.0 - 8.0 m wide on the section within YVC.

The alignment would generally meet an 80km/h horizontal design speed apart from two 90 degree bends located approximately 2km west of Jerrawa. This section of road is likely to need realignment to meet Council requirements and also to allow for use by over dimension vehicles.

The current vertical alignment of the road is substandard and significant earthworks would be expected to enable the road to be upgraded to the required standards.

The road reserve has significant amounts of native vegetation and earthworks associated with widening and vertical alignment improvements would impact on this vegetation.

#### 2.3 Bushs Road/Days Road

Bushs Road and Days Road are very low standard unsealed roads that currently provide access to a handful of farms and residences. Both roads are approximately 3.0 – 4.0m wide and would need significant works to upgrade them to meet Council standards.

There is an existing railway crossing at the intersection of Bushs Road/Days Road which is also likely to require upgrading to enable use by large numbers of heavy vehicles and/or over dimension vehicles.

#### 2.4 Rye Park-Dalton Road

Rye Park-Dalton Road as the name suggests, runs from Rye Park, south to the intersection with Blakney Creek South Road, and then heads predominantly east to the village of Dalton.

The sections proposed for use as part of the wind farm project include the 25.7km from the village of Rye Park to proposed site access 13 located approximately 1.5km north of Blakney Creek South Road. This section traverses both Hilltops and Upper Lachlan Council areas.

The portion of the road within the Hilltops Council area is all sealed and generally has a width of 8.0-8.2m. The portion in the Upper Lachlan Council area is predominantly unsealed with widths varying from 6.0-8m. Parts of the unsealed sections will need significant widening and earthworks to bring them up to the required standard.

There are numerous bridges, major culverts and causeways along the length of the road that may require upgrading or widening to cater for the proposed heavy vehicle usage.

The road reserve is well vegetated with large sections of the road being identified to contain the Threatened Ecological Community, Bow Gum Woodland. There are likely to be impacts to this community as a result of upgrade works on this section of the road.

#### 2.5 Grassy Creek Road

Grassy Creek Road runs from the village of Rye Park, north toward the village of Rugby. The road is sealed but is quite narrow with the existing seal being 5.4 - 5.6m wide. The alignment of the road is meandering and there are numerous curves that would likely need realignment to meet design requirements.

The road reserve is well vegetated with large sections of the road being identified to contain the Threatened Ecological Community, Bow Gum Woodland. There are likely to be impacts to this community as a result of upgrade works on this section of the road.

#### 2.6 Boorowa-Rye Park Road

Boorowa-Rye Park Road links Boorowa and Rye Park and the length proposed to be used by the wind farm is 18.9km long. The road is sealed for its entire length and is generally of a suitable standard to allow use by heavy vehicles on the project. There are isolated sections which will require widening and or pavement strengthening but this would involve minimal earthworks or disturbance.

There are numerous bridges and major culverts along this section of road which may require upgrading and/or widening to meet Council requirements.

It is not expected that there will be any significant impact on vegetation as a result of any upgrades to this road.

#### 2.7 Trucking Yard Rd/Long Street

Trucking Yard Road and Long Street are urban streets within Boorowa. They are generally in reasonable condition but are quite narrow with some sections between 5.3-6.0m wide. It is likely that some sections will need widening and/or pavement strengthening to allow their use by large numbers of heavy vehicles.

#### 3. Road Standards

#### 3.1 Council Requirements

Meetings were held with relevant Council officers from Hilltops Council (20 September 2019) and Upper Lachlan Shire Council (1 November 2019) to confirm their requirements for road upgrades and identify any other expectation that would need consideration. A meeting was not held with Yass Valley Council due to the very small sections of road within their jurisdiction. It has been assumed that YVC requirements are as per previous discussions and the requirements of the development consent. A summary of upgrade requirements is below:

#### Hilltops Council

- Hilltops Council are happy to stick to the agreed standard contained in the Development Consent and EIS as follows:
  - Unsealed roads to be sealed: 200 mm road base topped with double spray seal (14/7 double/double). 7.0 m seal and 8.5 m formation width.
  - Unsealed Roads: Construction width minimum 6 metres wide, maximum 8 metres wide. Pavement minimum thickness 100 mm on existing sheeted road.
- 80km/h design speed is acceptable. Higher design speeds are likely to be unachievable.

- Where possible roads to be designed to follow existing road alignments to minimise impacts on vegetation and possible land acquisition. Some minor realignment may be required in isolated locations and at intersections;
- Council are willing to help with land acquisition issues surrounding road reserves if required;
- Council will require a dilapidation assessment, prefer visual assessment, report and video;
- Tilt Renewables will undertake an updated Traffic Impact Assessment when final route is decided on;
- Council will supply a copy of their Endangered Flora and Fauna Register;
- Council are happy to have a workshop with Tilt and OEH regarding road design and vegetation clearing if required;
- As routes are refined and design work progresses, Tilt Renewables will need to confirm size/mass of heavy vehicle loads and assess the capacity of existing bridges as necessary. Options such as strengthening or construction of temporary bypass tracks/crossings may be considered if current capacity is unsuitable.

#### Yass Valley Council

- Road standards as per the agreed standard contained in the Development Consent and EIS as follows:
  - Unsealed roads to be sealed: 200 mm road base topped with double spray seal (14/7 double/double). 7.0 m seal and 8.5 m formation width.
  - Unsealed Roads: Construction width minimum 6 metres wide, maximum 8 metres wide. Pavement minimum thickness 100 mm on existing sheeted road.
- 80km/h design speed in accordance with YVC Road Standards policy.

#### **Upper Lachlan Shire Council**

- Any roads to be upgraded will need to be upgraded to a sealed standard;
- Roads to be designed in accordance with Austroads standards. No specific design speed specified and will be determined as part of design process. Some flexibility will be allowed on a case by case basis to design the road generally along existing alignments and to minimise vegetation removal;
- Road widths required for Regional Roads (Rye Park-Dalton Road) is a 9m formation with 8m seal. Widths of Local Roads to be agreed as part of design process.
- Pavement construction works to utilise DGB20 road base or equivalent. Some potential
  for use of local gravel pits or alternate equivalent pavement designs (eg. stabilisation)
  where this can be justified;
- Sight distance to be addressed at all intersections;
- Cadastral survey of all road reserve boundaries is required to confirm that the road is within the existing road reserve;
- Geotechnical investigation is required to inform pavement design. Pavement designed in accordance with Austroads standard for 25 year design life;
- Council fees apply for review of designs. Approval for works to be via a Works Authorisation Deed (WAD) based on RMS template. Section 138 approval also required;
- Specific consideration to be given to upgrade of the Jerrawa Road and Cooks Hill Road intersections to meet community expectations.

#### 3.2 Hilltops Council Workshops

Following initial meetings with stakeholder Council's a follow up workshop was held via telephone conference between Tilt Renewables, Hilltops Council and the NSW Biodiversity Conservation Department (BCD) on 31 January 2020. Outcomes of this meeting in relation to road upgrades were as follows:

- BCD raised concerns over the extent of vegetation removal required due to road widening
  as a result of the 80km/h design speed, and expressed particular concerns over impacts
  along Grassy Creek Road;
- Tilt Renewables outlined the process that had been undertaken to assess multiple routes to minimise impacts on vegetation communities. This has resulted in some proposed routes being abandoned;
- Discussion took place around opportunities to reduce the design standard in some locations in order to reduce overall impact on native vegetation and in particular along Grassy Creek Road. Hilltops Council indicated they would be open to considering reduced road design standards providing road safety objectives could be achieved. To this end a road safety audit of the route was suggested;
- Hilltops Council noted several bridges and culverts will require upgrades and tilt Renewables undertook to complete structural assessment to identify which structures would require upgrades.

This workshop was followed by a further teleconference meeting with Hilltops Council engineering staff on 7 February 2020 to further discuss road standards. Outcomes of this meeting were:

- Hilltops Council happy to accept a design that complies with the current road speed environment on Grassy Creek Road (estimated to be approx.. 60km/h);
- Minimum 7m formation/seal width required (reduced from 8.5m formation/7m seal width).
   Desirable standard is 7.4m wide formation and seal with edge lines to delineate travel lanes;
- The design process is to include a review by a Road Safety Auditor to identify potential safety issues that can be addressed as part of the design process.

#### 3.3 Pavement Design

Upper Lachlan Shire Council requires a pavement design to be undertaken in accordance with the *Austroads Guide to Pavement Technology, Part 2: Pavement Structural Design 2017.* Essentially the two inputs into this design process are:

(i) **Traffic volumes** – traffic volumes are measured in Equivalent Standard Axles (ESA's) calculated over the 25 year design life of the pavement. An Equivalent Standard Axle is defined as a dual tyred single axle that transmits a load of 80kN (8.2 Tonne) to the road pavement. ESA's are used as a standard measure against which various size and configurations of heavy vehicle can assessed.

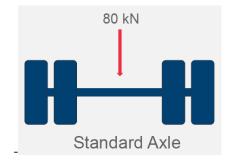


Figure 2 - Equivalent Standard Axle (ESA)

Given that heavy vehicle impacts as a result of the Rye Park Wind Farm development will be largely limited to the 18 month construction phase it is somewhat difficult to calculate ESA's over a 25 year period. In order to calculate design traffic ESA's, the following traffic volumes were adopted from SMEC 2020:

- Total of 67,896 (two way) heavy vehicle movements associated with wind farm construction over a 93 week construction period;
- Total of 58,240 (two way) light vehicle trips associated with construction over the 93 week construction period;
- Total 126,136 (two way) construction vehicle trips with 51% being heavy vehicles;
- 8 trips per day associated with O&M of the wind farm during the operational phase (Epuron Pty Ltd (2016), Rye Park Windfarm Traffic and Transport Assessment – April 2016).

Limited traffic data is available for the Council roads in question. A conservative existing traffic volume of 200 vehicles per day with 15% of these being heavy vehicles was adopted for use in the pavement design.

The table below provides details of key inputs and outputs of the design traffic calculation:

Traffic Source	AADT	%HV	Comment
Existing Traffic	200	15%	estimate only
Construction Traffic	13	51%	Averaged over 25 years
O&M Traffic	8	25%	Assumed %HV
Totals	221	17%	
Design Period	25 y	ears	
Heavy Vehicle Growth Rate	0.5	5%	Assumed
Axle groups per Heavy vehicle (N <sub>HVAG</sub> )	2.	8	Based on presumptive values provided in Austroads
Design ESA's	4.08	x 10 <sup>5</sup>	

Table 2 – Design Traffic

As a sensitivity analysis the AADT was increased by 10% and a 2% heavy vehicle growth rate was adopted. This resulted in a higher Design ESA value of  $5.41 \times 10^5$  and an alternative pavement design was undertaken for this higher traffic volume.

(ii) Subgrade CBR – To undertake pavement design, a geotechnical investigation would normally be completed to obtain subgrade samples for analysis. These samples are tested to determine the soaked California Bearing Ratio (CBR) of the underlying subgrade material.

CBR provides an indication of the bearing capacity of the layer of soil directly below the road pavement which in turn provides an understanding of its ability to withstand repetitions of heavy vehicle loading. The lower the CBR, the thicker the road pavement is required to be.

For this high level analysis a conservative CBR value of 5 has been adopted for use in the pavement design. Based on experience and the predominantly clay soils in the region it is considered that this is a realistic representation of likely lower bound CBR values that would be encountered.

Using the above inputs and utilising the empirical design method for flexible pavement design as outlined in Austroads, pavement thicknesses of 335mm – 365mm were calculated. Copies of the detailed calculation sheets have been provided as **Attachment G**. For the purposes of cost estimation

a more conservative pavement thickness of 380mm was adopted for Upper Lachlan Shire Roads. A pavement thickness of 200mm was adopted for Yass Valley and Hilltops roads as previously agreed with these parties and specified in the development consent.

#### 4. Cost Estimation

#### 4.1 Road Construction

It is difficult to accurately estimate likely road construction costs without detailed design and quantities. In particular earthworks quantities are difficult to estimate and have a significant impact on the construction cost per kilometre.

Earthworks quantities can vary from project to project due to the following:

- Topography;
- Amount of widening required (a function of design standards and existing road width);
- Existing horizontal alignment and the need for improvements;
- Existing vertical alignment and need for additional cuts and fills.

In addition to earthworks costs, the biggest impact on rural road construction costs comes from pavement construction with the main variables being the distance from the worksite to a suitable gravel source, and pavement thickness (pavement design).

In order to provide per kilometre rates that more accurately reflect the potential variances in quantities due to the above factors, cost estimates were undertaken for a range of scenarios and each section of road was classified into one of the following types for the purposes of cost estimation:

Type	Earthworks	Formation Width (m)	Seal Width (m)	Pavement Thickness (mm)	Cost/km	Comment
Type 1	None/minimal	8.5	7	200		
Type 2	Minor	8.5	7	200		
Type 3	Significant	8.5	7	200		
Type 4	None/minimal	9	8	380		ULSC standard
Type 5	Minor	9	8	380		ULSC standard
Type 6	Significant	9	8	380		ULSC standard
Type 7	Significant	8.5	7	200		Road realignment
Type 8	Minor	8.5	7	200		Widening only

Table 4 – Road Types for Cost Estimation

Copies of the detailed cost estimates undertaken for each upgrade type have been included as **Attachment F.** Survey costs were added on the basis of \$5000/km for detailed survey only, and \$7000/km where full cadastral survey is also required (Upper Lachlan Shire Council requirement).

It was assumed that any required fill material would be available on site or within close proximity and as such earthworks costs are based on cut to fill scenarios where significant importation of fill material is not required.

Road pavement costs are based on a suitable available commercial gravel source within 100km of the sites. Sealing rates assume that more than 1km would be constructed and sealed at a time, minimising establishment costs.

No allowance has been made for design, geotechnical investigation, or project management and it is assumed that these costs will be essentially the same on a per kilometre basis for each road section.

#### 4.2 Intersections

Intersections were identified for upgrade based on visual assessment and utilising swept path analysis previously undertaken by Rex J Andrews – Engineered Transportation. Indicative cost were then allocated to each intersection requiring upgrade based on the likely scope of works. It is noted that it is difficult to estimate these costs without an accurate scope and quantities and as such these costs should be treated as ballpark estimates only and are intended only for comparison purposes.

Intersection upgrade costs are estimated to vary from \$50,000 to \$500,000 depending on the scale of the required works.

#### 4.3 Structures

As part of the assessment structures including bridges, major culverts, causeways, and railway crossings were identified and a brief visual assessment undertaken to determine dimensions, likely age and condition. From this a list of assumed upgrades has been developed.

Costs estimates for these structures are based on the following unit rates:

- /m2 for bridge construction/replacement;
- /m2 for concrete causeway construction/widening;
- Large box culvert extensions based on indicative costs for each culvert;
- Railway crossing upgrade indicative cost only.

#### 4.4 Land Acquisition

Where areas of land acquisition were identified as being likely to facilitate road improvements, approximate areas were generated from online mapping tools. Land zoned RU1 – Primary Production generally sells for around \_\_\_\_\_\_\_/ha (NSW GLOBE) in the region depending on location, agricultural productivity, and numerous other factors. Recognising the small areas of land to be acquired, potential resistance from wind farm detractors, and the likely premium that will need to be payed to get agreement from property owners, and unit rate of has been adopted. For very small areas (<1000m2) a unit rate of has been adopted to reflect the premium likely to be paid for these small areas and the higher proportion of fixed costs associated with the acquisition eg. survey and legal costs. Land areas identified for potential acquisition are shown in the maps provided as **Attachment H**.

#### Road Assessment Outcomes

Detailed schedules for identified upgrades to roads, intersections, structures and required land acquisition have been included as **Attachments C to E**. The following table provides a summary of the results of the assessment for each of the six road sections:

Cost Catagories	Road Section												
Cost Categories	1	2	3	4	5	6							
Road Upgrades													
Intersection Upgrades													
Structure Upgrades													
Land Acquisition													
Total													
Approx. Area of	33000	11500	2000	10600	10450	100							
Vegetation Impact (m <sup>2</sup> )	33000	11300	2000	10000	10430	100							

Table 5 - Road Upgrade Costs and vegetation impact by Road Section

Road Sections 1 and 2 are predominantly within the Upper Lachlan Shire Council who have more stringent requirements when it comes to design and construction of road upgrades. This has led to significantly higher per kilometre rates and ultimately a high overall cost for upgrade of these roads. Section 1 also has the highest assumed vegetation impact with approximately three times the area of vegetation likely to be impacted in order to upgrade these roads. This is due to narrow existing road widths, Council expectations of road widths, and poor vertical alignment which is likely to lead to significant cuts and fills in some areas.

Road Section 6 appears by far the easiest section of road to upgrade as existing formation widths are generally adequate and upgrade works are generally limited to pavement strengthening with little or no impact on roadside vegetation. The relatively large number of structures on this section of road including a number of bridges with unknown structural capacity makes up for approximately 25% of the estimated upgrade costs.

### 6. Assumptions

The intention of this report is to provide a high level assessment of the various access route options to assist with decision making. With the limited available data currently available including no detailed road design it is not possible to accurately estimate construction costs and as such the cost estimates in this report are intended to indicate relative costs of upgrading the various road sections rather than absolute costs. Numerous assumptions have been necessary in compiling this report and these are listed below:

- Identified impact areas for vegetation should be considered indicative only and are based on a
  drive through of each road to identify obvious areas of vegetation impact. It was assumed that
  generally vegetation (generally grasslands) back to the existing tree line has previously been
  disturbed and is therefore of little value. No assessment of vegetation types, or qualitative
  assessment was undertaken:
- In order to determine which sections of road require upgrade, a visual assessment was undertaken to determine road width and pavement condition. Council requirements and previous correspondence with Councils was taken into account but ultimately the sections of road listed for upgrade were based on a judgement call and need to be confirmed with the Council's once the preferred routes are identified;
- There has been no structural assessment of bridges and other major structures to inform
  decision making. Where possible the age of some structures was identified and a visual
  inspection undertaken to determine condition. Where doubt existed over the structural
  capacity of structures it has been assumed that they will require full replacement. A detailed
  assessment of these structures will be needed and should be undertaken as soon as possible
  after the preferred access routes are confirmed;
- Major culverts and causeways have been individually listed where they are considered likely to require widening or replacement. No assessment was undertaken of smaller road culverts but allowance has been made for the extension of 5 small culverts (375mm – 600mm diameter) per kilometre.
- In order to determine an appropriate pavement design it was necessary to assume subgrade CBR values and also existing traffic volumes utilising the roads. Conservative values have been adopted for this purpose. A 380mm pavement thickness was adopted for ULSC roads;
- Land acquisition areas have been determined based on a desktop assessment of the road alignments based on achieving an 80km/h design speed. This is not intended to be an exhaustive list but rather to provide some relativity between the various road sections for comparison purposes;
- It has been assumed that Yass Valley Council road upgrade requirements have not changed since agreement was reached prior to issue of the current consent;
- Intersection upgrade costs are based on high level ballpark estimates in the absence of any detailed quantities;
- Quantities used in preparing costs estimates are estimated quantities only based on indicative works required for a typical kilometre of road. Unit rates were derived from previous projects in the region;
- It was assumed that the Council's would not accept unsealed roads being used as heavy vehicle access routes and as such all unsealed roads have been identified for upgrade to a sealed standard;
- It was assumed that any required fill material would be available on site or within close proximity and as such earthworks costs are based on cut to fill scenarios where significant importation of fill material is not required;

- Road pavement costs are based on a suitable available commercial gravel source within 100km of the sites.
- Sealing rates assume that more than 1km would be constructed and sealed at a time, minimising establishment costs.
- No allowance has been made for design, geotechnical investigation, structural assessment or
  project management and it is assumed that these costs will be essentially the same on a per
  kilometre basis for each road section.

#### 7. References

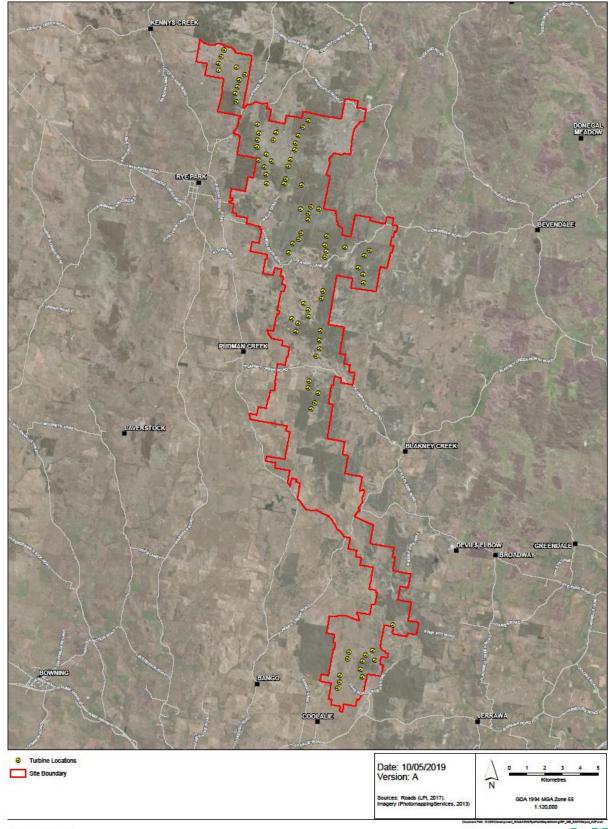
ZEM Energy Pty Ltd (2016), Rye Park Windfarm Transport Route Assessment, (0005-RC-002)

Epuron Pty Ltd (2016), Rye Park Windfarm Traffic and Transport Assessment

Rex J Andrews Engineered Transportation (2019), *Route Study: Vestas, Rye Park Windfarm: (82 Metre Blade), Ex Newcastle.* 

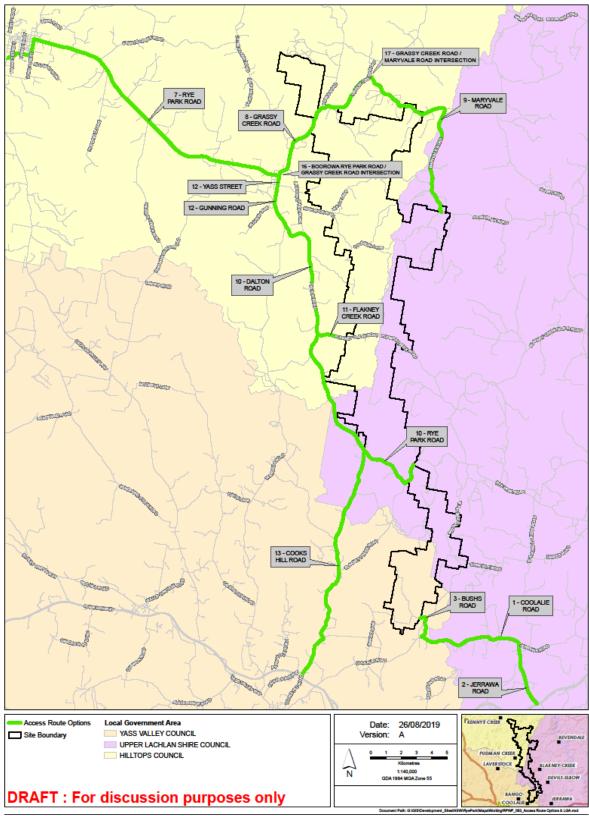
SMEC 2020, RPWF\_Construction Traffic Input\_800\_NOSQ\_RevD.xlsx

## Attachment A – Project Locality Plan



Rye Park 80 WTG Layout

### Attachment B – Current Access Route Options



Rye Park Wind Farm

Access Route Options & LGA



## Attachment C – Detailed Road Upgrade Schedule

Road	LGA	Road Name	Start Loation	Start CH	End CH	Length	Existing	Existing	Req'd	Pavement	Req'd	Upgrade	Upgrade	Total Upgrade	Approx.	Comments
Section				(km)	(km)	(km)	Surface	Width (m)	P'ment	Thickness	Seal	Category	Cost per km	Cost	Veg	
-	~	•	,						Width	(mm)	Width				Impact	
1	ULSC	Jerrawa Road	Hume Highway	0	4.1	4.1	Sealed	6.0 - 6.5	9	380	8	5			8000	Narrow and likely low pavement strength. Likley high impact on vegetation
1	ULSC	Coolalie Road	Jerrawa Road	0	0.88	0.88	Sealed	5.0 - 6.0	9	380	8	6			1000	Very narrow. Fair to poor pavement with low pavement strength. Powerline relocations may be required
																and likely high vegetation impacts.
1	ULSC	Coolalie Road	Jerrawa Road	0.88	5.2	4.32	Unsealed	6.0 - 6.5	9	380	8	6			17000	Powerlines likely to need relocation. Poor vertical alignment (sharp crests). High vegetation impact.
1	YVC	Coolalie Road	Shire Boundary (CH5.2)	5.2	6.6	1.4	Unsealed	7	8.5	200	7	3			7000	Very High vegetation impact
1	YVC	Bushs Road	Coolalie Road	0	1.3	1.3	Unsealed	3.0 - 4.0	8.5	200	7	3			0	Some earthworks required. Negligible vegetation impact. Possible rail crossing upgrade.
1	YVC	Days Road	Bushs Road	0	0.4	0.4	Unsealed	3	8.5	200	7	3			0	Significant earthworks required. Major culvert needed at Ch0.19km.
2	ULSC	Rye Park-Dalton Road	Site Access 12	1.3	1.57	0.27	Unsealed	7.0 - 8.0	9	380	8	4			0	Requires upgrade to sealed standard
2	ULSC	Rye Park-Dalton Road	Site Access 12	2.1	3.75	1.65	Unsealed	7.0 - 8.0	9	380	8	4			0	Requires upgrade to sealed standard. Minimal vegetation or earthworks required.
2	ULSC	Rye Park-Dalton Road	Site Access 12	4	5.6	1.6	Unsealed	7.0 - 8.0	9	380	8	4			1500	Requires upgrade to sealed standard. Minimal vegetation or earthworks required.
2	ULSC	Rye Park-Dalton Road	Site Access 12	5.6	9.15	3.55	Unsealed	6.0 - 6.5	9	380	8	6			10000	Requires upgrade to sealed standard. Significant wideing, earthworks & vegetation removal required.
																Significant realignment required approaching Blakney Creek causeway.
2	ULSC	Rye Park-Dalton Road	Site Access 12	9.15	10.5	1.35	Sealed	7.0 - 7.8	9	380	8	4			0	Road in good condition. Council likley to require widening and pavement strengthening
3	Hilltops	Rye Park-Dalton Road	Boorowa-Rye Park Road	0	1.05	1.05	Sealed	5.5 - 6.0	8.5	200	7	8			0	Yass Street Rye Park. Existing pavement OK. Widen and reseal only.
3	Hilltops	Rye Park-Dalton Road	Boorowa-Rye Park Road	3.95	4.5	0.55	Sealed	8	8.5	200	7	7			2000	Realignment of substandard bend near Pudman Creek Bridge
3	Hilltops	Rye Park-Dalton Road	Site Access 12	0	1.3	1.3	Sealed	8	8.5	200	7	N/A			0	Some Heavy patching required to address pavement defects. Assume 10% of road area.
4	Hilltops	Grassy Creek Road	Boorowa-Rye Park Road	0	0.1	0.1	Sealed	5.4 - 5.6	8.5	200	7	8			0	Narrow and good to fair pavement condition. Assume widening and allowance for 10% heavy patching
4	Hilltops	Grassy Creek Road	Boorowa-Rye Park Road	0.1	0.4	0.3	Sealed	5.4 - 5.6	8.5	200	7	7			0	Realignment of substandard bend near Boorowa/Rye Park Road
4	Hilltops	Grassy Creek Road	Boorowa-Rye Park Road	0.4	2.4	2	Sealed	5.4 - 5.6	8.5	200	7	8			0	Narrow and good to fair pavement condition. Assume widening and allowance for 10% heavy patching
4	Hilltops	Grassy Creek Road	Boorowa-Rye Park Road	2.4	2.6	0.2	Sealed	5.4 - 5.6	8.5	200	7	7			1000	Realignment of substandard bend
4	Hilltops	Grassy Creek Road	Boorowa-Rye Park Road	2.6	4.5	1.9	Sealed	5.4 - 5.6	8.5	200	7	8			300	Narrow and good to fair pavement condition. Assume widening and allowance for 10% heavy patching
4	Hilltops	Grassy Creek Road	Boorowa-Rye Park Road	4.5	4.7	0.2	Sealed	5.4 - 5.6	8.5	200	7	7			6000	Realignment of substandard bend
4		Grassy Creek Road	Boorowa-Rye Park Road	4.7	5	0.3	Sealed	5.4 - 5.6	8.5	200	7	8			300	Narrow and good to fair pavement condition. Assume widening and allowance for 10% heavy patching
4		Grassy Creek Road	Boorowa-Rye Park Road	5	5.2	0.2	Sealed	5.4 - 5.7	8.5	200	7	7			3000	Realignment of substandard bend. Site Access 2
5		Grassy Creek Road	Boorowa-Rye Park Road	0	0.65	0.65	Sealed	5.4 - 5.6	8.5	200	7	8			4000	Narrow and good to fair pavement condition. Assume widening and allowance for 10% heavy patching
5	Hilltops	Grassy Creek Road	Boorowa-Rye Park Road	0.65	0.95	0.3	Sealed	5.4 - 5.6	8.5	200	7	7			6000	Realignment of substandard bend
5	Hilltops	Grassy Creek Road	Boorowa-Rye Park Road	0.95	1.1	0.15	Sealed	5.4 - 5.6	8.5	200	7	8			450	Narrow and good to fair pavement condition. Assume widening and allowance for 10% heavy patching
6	Hilltops	Trucking Yard Road/Dillon Street	Lachlan Valley Way	0	1.6	1.6	Sealed	5.3 - 7.0	8.5	200	7	1			0	Good to fair pavement condition. Likely to require widening
6	Hilltops	Long Street	Dillon Street	0	0.4	0.4	Sealed	5	8.5	200	7	1			0	Narrow section of road. Requires widening
6	Hilltops	Boorowa-Rye Park Road	Long Street	0	1.2	1.2	Sealed	7.2	8.5	200	7	1			0	Poor pavement condition. Needs widening and reconstruction of pavement
6	Hilltops	Boorowa-Rye Park Road	Long Street	1.2	1.75	0.55	Sealed	5.7	8.5	200	7	1			0	Narrow and poor pavement condition. Needs widening and reconstruction
6	Hilltops	Boorowa-Rye Park Road	Long Street	5.85	6.7	0.85	Sealed	6	8.5	200	7	2			100	Narrow assume upgrade is required
6	Hilltops	Boorowa-Rye Park Road	Long Street	7.2	11.4	4.2	Sealed	6.0 - 6.5	8.5	200	7	2			0	Narrow and poor pavement condition. Needs widening and reconstruction
6	Hilltops	Boorowa-Rye Park Road	Long Street	17.8	18.9	1.1	Sealed	6.7 - 7.3	8.5	200	7	2			0	Fair to poor pavement condition. Assume upgrade
						39.92										

## Attachment D – Detailed Structure Upgrade Schedule

Road	LGA	Road Name	Start Location	Approx	Easting	Northing	Description	Trafficable	Quantitiy	Unit Rate	Cost	Comments
Section				Chainage				Width (m)				
				(km)								
1	ULSC	Jerrawa Road	Hume Highway	2.62	691004	6147246	4 x 2100 x 1500 RCBC	6.5	1		Will require	widening
1	ULSC	Coolalie Road	Jerrawa Road	0.78	690168	6149091	box culvert and concrete causeway	5-6m	135		Will require	widening or replacement
1	ULSC	Coolalie Road	Jerrawa Road	2	688980	6149109	Concrete causeway with small culvert	5	1		Relocate as p	part of road realignmnet
1	YVC	Days Road	Bushs Road	0.19	684683	6150475	Large culvert or causeway required (approx. 2.1m dia)		1		New culvert	required
2	ULSC	Rye Park-Dalton Road	Boorowa-Rye Park Road	17.7	678939	6163175	4 x 1500 x 900 box culvert	8	1		3 cell culvert	requires widening
2	ULSC	Rye Park-Dalton Road	Boorowa-Rye Park Road	19.45	680320	6162002	2 x 2100 x 1800 box culvert	8	1		2 cell culvert	requires widening
2	ULSC	Rye Park-Dalton Road	Boorowa-Rye Park Road	22.3	682496	6160381	21 x 6 concrete causeway	6	84		Barlows Cree	ek causeway
2	ULSC	Rye Park-Dalton Road	Boorowa-Rye Park Road	23.75	683391	6159194	18 x 6 concrete causeway	6	72		Blakney Cree	ek causeway
3	Hilltops	Rye Park-Dalton Road	Boorowa-Rye Park Road	4.5	676316	6175360	20 x 6.1 concrete bridge	6.1	180		Pudman Cree	ek - DMR 1951
3	Hilltops	Rye Park-Dalton Road	Boorowa-Rye Park Road	8.1	677358	6172377	16 x 6.2 concrete causeway & 2 x 1500 x 450 culverts	6.2	48		Flakney Cree	k causeway - reasonable condition assume widening only
4	Hilltops	Grassy Creek Road	Boorowa-Rye Park Road	0.3	675323	6179458	3 x 3000mm dia corrugated metal pipes	5.6	108		Poor condition	on assume replacement
4	Hilltops	Grassy Creek Road	Boorowa-Rye Park Road	4.4	677457	6182711	7 x 16m concrete causeway	7	144		Poor condition	on assume replacement
6	Hilltops	Trucking Yard Road	Lachlan Valley Way	0.55	657987	6187169	Narrow cuseway with 450mm culvert	6.1	120		Will require	widening
6	Hilltops	Boorowa-Rye Park Road	Long Street	8.9	666284	6183088	27 x 7.5 concrete bridge	7.5	243		Harry's Creel	Bridge - DMR 1969. Signs of deterioration to piers and unlikley to meet current standards. Assume replacement
6	Hilltops	Boorowa-Rye Park Road	Long Street	15.3	671613	6180054	16 x 7.6 concrete bridge	7.6	144		Dirthole Cree	ek Bridge - DMR 1934
							<u> </u>		,		·	

## Attachment E – Detailed Intersection and Land Acquisition Schedules

### Intersection Upgrades

Road Section	LGA	Road Name 1	Road Name 2	Easting	Northing	Est. Cost
1	ULSC	Jerrawa Road	Hume Highway	692082	6144807	
1	ULSC	Jerrawa Road	Coolalie Road	690886	6148789	
1	YVC	Coolalie Road	Bushs Road	684524	6149185	
1	YVC	Bushs Road	Railway Crossing/Days Road	684828	6150377	
2	ULSC	Rye Park-Dalton Road	Blakney Creek South Road	683463	6159178	
6	Hilltops	Rye Park-Dalton Road	Boorowa-Rye Park Road	675192	6179196	
6	Hilltops	Boorowa-Rye Park Road	Long Street	659123	6188100	
6	Hilltops	Dillon Street	Long Street	658939	6187018	
6	Hilltops	Trucking Yard Lane	Lachlan Valley Way	657413	6186849	

### Land Acquisition

Ref.	Road	LGA	Road Name	Start Loation	Approx	Easting	Northing	Lot/DP	Approx.	Cost/m2	Total Cost
	Section				Chainage				<b>Land Area</b>		
					(km)				(m2)		
1	1	ULSC	Coolalie Road	Jerrawa Road	1.6	689241	6148983	Lot 239 DP 754122	2900		
2	1	ULSC	Coolalie Road	Jerrawa Road	2	688928	6149111	Lot 31 DP 754122	10400		
								Lot 2 DP 1245971			
	1	YVC	Bushs Road	Assume 2 x	substanda	rd bends v	vill be acce	pted			
				by Council g	iven very lo	ow traffic	volumes or	road			
3	2	ULSC	Rye Park-Dalton Road	Site Access 12	6.9	618831	6160702	Lot 1 DP 838933	3550		
4	2	ULSC	Rye Park-Dalton Road	Site Access 12	8.4	682969	6159684	Lot 92 DP 754102	2600		
5	2	ULSC	Rye Park-Dalton Road	Site Access 12	8.95	683267	6159272	Lot 4 DP 1066057	3850		
6	3	Hilltops	Rye Park-Dalton Road	Boorowa-Rye Park Road	3.95	675986	6175291	Lot 60 DP 754135	26000		
								Lot 61 DP 754135			
7	4	Hilltops	Grassy Creek Road	Boorowa-Rye Park Road	0.1	675249	6179419	Lot 2 DP 591580	2650		
8	4	Hilltops	Grassy Creek Road	Boorowa-Rye Park Road	2.4	676120	6181333	Lot 154 DP 754145	3500		
9	4	Hilltops	Grassy Creek Road	Boorowa-Rye Park Road	4.5	677479	6182772	Lot N DP 439287	3000		
10	4	Hilltops	Grassy Creek Road	Boorowa-Rye Park Road	5	677710	6183344	Lot D DP 440134	5200		
11	5	Hilltops	Grassy Creek Road	Boorowa-Rye Park Road	0.65	678428	6183728	Lot B DP 439287	3200		
12	1	ULSC	Jerrawa road	Hume Highway	4.1	690860	6148792	Lot 63 DP 754122	650		
								Lot 39 754122			
13	1	YVC	Bushs Road	Coolaie Road	0	684566	6149204	Lot 215 DP 754122	5350		
14	1	YVC	Days Road	Bushs Road	0	684804	6150466	Lot 42 DP 754122	7000		
15	6	Hilltops	Dillon Street	Lachlan Valley Way	1.5	658909	6187007	Lot 14 DP 1055548	1250		
16	6	Hilltops	Boorowa-Rye Park Rd	Dillon Street	0	659149	6188067	Lot 1 DP 219928	530		
17	6	Hilltops	Boorowa-Rye Park Rd	Dillon Street	0	659155	6188118	Lot 63 DP 754103	530		
18	4	Hilltops	Grassy Creek Road	Boorowa-Rye Park Road	0	675173	6179228	Lot 1 DP 580999	3500		
19	3	Hilltops	Rye Park-Dalton Road	Boorowa-Rye Park Road	0	675179	6179181	Lot 1 DP 1810	190		
									85850		





Project Reference:   Rye Park Windfarm   Designed by:   Simon Cassidy   Date:   3/04/2020   3/04/2020   3/04/2020   Date:	Genium Civil Granular Pavement D				Surfacin	g - Austroads 201	2		Genium CIVIL ENGINEERING
Date		_							CIVIL ENGINEERING
Project Description: Upper Lachlan Shire Council - Typical pavement Design Rye Park Road  Design Parameter S	Project Reference:	Rye	Park	Windfarm				Designed by:	Simon Cassidy
Rye Park Road	Client:	Tilt	Rene	wables				Date:	3/04/2020
Design Parameters Austroads Ref. Symbol Value    Presumptive Design Parameters   Austroads Ref. Symbol   Value	Project Description:				Council -	Typical paveme	ent Design		
Design Parameters   Austroads Ref.   Symbol   Value		куе	Fair	Roau					
Note   Enter values in green cells	DESIGN TRAFFIC C	ALCUL	ATIO	NS	(Refer A	ustroads 2012 Se	ection 7)		
	Design Parameters		Н			Austroads Ref.	Symbol	Value	
Person			s)						
			ĹΠ			7.4.2	(P)	25	(Typically 20-40 years)
7.4.1   (DF)   0.5		Traffic	ш			7.4.1, 7.4.4	(AADT)	122	vehicles/day
Table   Tabl			ш			7.4.1	(DF)	0.5	
Lane Distribution Factor   7.4.3 (LDF)   1   Refer Table 2 in Tables Tab)   7.4.5 (R)   7.4.5 (R)   7.4.5 (R)   7.4.5 (R)   7.5 (TLD)   Rural Collector   Refer Table 2 in Tables Tab)   7.4.5 (R)   7.5 (TLD)   Rural Collector   Refer Table 2 in Tables Tab)   7.4.5 (R)   7.4.5 (R)   7.4.5 (R)   7.5 (TLD)   Rural Collector   Refer Table 2 in Tables Tab)   7.4.5 (R)   7.5 (TLD)   Rural Collector   Refer Table 2 in Tables Tab)   7.5 (TLD)   Rural Collector   Refer Table 2 in Tables Tab)   7.5 (TLD)   Rural Collector   Refer Table 2 in Tables Tab)   Refer Table 2 in Tables Tab)   7.5 (TLD)   Rural Collector   Refer Table 2 in Tables Tab)   Refer Table 2 in Tables Tab)   7.5 (TLD)   Rural Collector   Refer Table 2 in Tables Tab)   Refer Table 2 in Tables	% Heaw Vehicles		Ш			7.4.4	(%HV)	20	%
Heavy Vehicle Growth Rate Traffic Load Distribution Traffic Load Groups per Heavy Vehicle Traffic Load Groups per Heavy Vehicle Traffic Loading (Norwa) Traffic Loading Loadin		tor	ш			7.4.3	(LDF)	1	(Refer Table 2 in Tables Tab)
Traffic Load Distribution Curnulative Growin Factor Presumptive Axis Groups per Heavy Vehicle Design Traffic  Presumptive Damage Index Values for the selected TLD  Design ESA of Loading (DESA)  PAVEMENT DESIGN  Design Traffic  Calculated Design CBR  Adopted Design CBR  Total Granular Pavement Design (Empirical Design Method)  Total Granular Pavement Thickness = 113 mm  Adopted Pavement Design  14/7mm two coat spray seal  Total Granular Pavement Thickness = 221  Total depth of granular material required = 334 mm  Total depth of granular material required = 334 mm  Total depth of granular material required = 334 mm  Select Fill Layer (95% M.D.D)  Subgrade (95% M.D.D)  Subgrade (95% M.D.D)  Subgrade (95% M.D.D)  Total depth of granular material required = 334 mm			ш			7.4.5	(R)	0.5	%
Presumptive Axle Groups per Heavy Vehicle    Commonwealth   Common	Traffic Load Distributi	on	ш			7.5	(TLD)	Rural Collector	(Select from drop down list)
Design Traffic  Eq. 14  (Nor)  3.31E+05  Heavy Vehicle axle Groups  Presumptive Damage Index Values for the selected TLD  (ESA/HV)  Design ESA of Loading (DESA)  7.6.3  (DESA)  7.6.3  (DESA)  2.65E+05  Design CBR  Adopted Design CBR = 5  %  Design period  Design Traffic  Calculated DESA = 2.65E+05  Design period  Organular Pavement Design (Empirical Design Method)  Total Granular Pavement Thickness = Remaining Pavement Thickness =	Cumulative Growth Fa	actor	ш			7.4.5 Eq. 15	(CGF)	26.6	
Design Traffic  Eq. 14  (Not)  3.31E+05  Heavy Vehicle axle Groups  Presumptive Damage Index Values for the selected TLD  (ESA/HV/AG) (ESA	Presumptive Axle Gro	ouns ne	er Hea	w Vehicle			(NHVAG)	2.8	axle groups per heavy vehicle
Presumptive Damage Index Values for the selected TLD  Design ESA of Loading (DESA)  PAVEMENT DESIGN  Designed in accordance with Figure 8.4 and Figure 12.2 of Austroads 2012  Design CBR  Adopted Design CBR	1000	ларо ро		,			( IIVAG)		
Design ESA of Loading (DESA)  PAVEMENT DESIGN  Designed in accordance with Figure 8.4 and Figure 12.2 of Austroads 2012  Design CBR  Adopted Design CBR = 5 %  Design period  Design Traffic  Calculated DESA = 0 Design period  Calculated DESA = 0 Design period  Design Traffic  Calculated Desa = 3 %  Design Period  Total Granular Pavement Thickness = 334 mm  Adopted Pavement Design  14/7mm two coat spray seal  150 mm  CBR = NA  Select Fill Layer (95% M.D.D)  Total depth of granular material required = 334 mm	Design Traffic					Eq. 14	(N <sub>DT</sub> )	3.31E+05	Heavy Vehicle axle Groups
Design ESA of Loading (DESA)  PAVEMENT DESIGN  Designed in accordance with Figure 8.4 and Figure 12.2 of Austroads 2012  Design CBR  Adopted Design CBR = 5 %  Design period  Design Traffic  Calculated DESA = 2.65E+05  Design Period  Calculated Desa = 5 %  Design period  Note: Entering a value will overide the calculated value  Granular Pavement Design (Empirical Design Method)  Total Granular Pavement Thickness = 334 mm  Adopted Pavement Design  150 mm  CBR = NA  Select Fill Layer (95% M.D.D)  Total depth of granular material required = 334 mm	D					762	(ESA/HVAG)	0.8	
Design ESA of Loading (DESA)  7.6.3  (DESA)  2.65E+05  PAVEMENT DESIGN  Design CBR  Adopted Design CBR  Adopted Design CBR  Calculated DESA = 5  Design period  Note: Entering a value will overide the calculated value  Granular Pavement Design (Empirical Design Method)  Total Granular Pavement Thickness = 334  Minimum Base Thickness = 113  Remaining Pavement Thickness = 221  mm  Adopted Pavement Design  14/7mm two coat spray seal  DGB 20 Base Course (97% M.D.D)  DGS 20 Subbase Course (97% M.D.D)  CGR = 5  Subgrade (95% M.D.D)  Total depth of granular material required = 334  mm	•	je Inde	∍x Va	lues for the		7.0.2	, ,		
Design CBR  Design CBR  Adopted Design CBR   Adopted Design CBR = 5							,		
Designed in accordance with Figure 8.4 and Figure 12.2 of Austroads 2012  Design CBR  Adopted Design CBR = 5 % Design period  Design Traffic  Calculated DESA = 2.65E+05 Note: Entering a value will overide the calculated value  Granular Pavement Design (Empirical Design Method)  Total Granular Pavement Thickness = 334 mm  Minimum Base Thickness = 113 mm  Remaining Pavement Thickness = 221 mm  Adopted Pavement Design  14/7mm two coat spray seal  DGB 20 Base Course (97% M.D.D)  DGS 20 Subbase Course (97% M.D.D)  CBR = 5 Subgrade (95% M.D.D)  Total depth of granular material required = 334 mm	Design ESA of Load	ing (DI	ESA)			7.6.3	(DESA)	2.65E+05	
Designed in accordance with Figure 8.4 and Figure 12.2 of Austroads 2012  Design CBR  Adopted Design CBR = 5 % Design period  Design Traffic  Calculated DESA = 2.65E+05 Note: Entering a value will overide the calculated value  Granular Pavement Design (Empirical Design Method)  Total Granular Pavement Thickness = 334 mm  Minimum Base Thickness = 113 mm  Remaining Pavement Thickness = 221 mm  Adopted Pavement Design  14/7mm two coat spray seal  DGB 20 Base Course (97% M.D.D)  DGS 20 Subbase Course (97% M.D.D)  CBR = 5 Subgrade (95% M.D.D)  Total depth of granular material required = 334 mm	PAVEMENT DESIGN		$\Box$						
Adopted Design CBR   Adopted Design CBR = 5 %  Design period  Design Traffic   Calculated DESA = 2.65E+05   Note: Entering a value will overide the calculated value   Calculated value	AVEINENT BEGION								
Adopted Design CBR = 5 % Design period  Design Traffic  Calculated DESA = 2.65E+05  Note: Entering a value will overide the calculated value  Granular Pavement Design (Empirical Design Method)  Total Granular Pavement Thickness = 334 mm  Minimum Base Thickness = 113 mm  Remaining Pavement Thickness = 221 mm  Adopted Pavement Design  14/7mm two coat spray seal  150 mm  DGB 20 Base Course (97% M.D.D)  DGS 20 Subbase Course (97% M.D.D)  CBR = N/A  Select Fill Layer (95% M.D.D)  Total depth of granular material required = 334 mm	Designed in accordar	ice with	ı Figu	re 8.4 and Fig	ure 12.2 d	of Austroads 2012			
Adopted Design CBR = 5 % Design period  Design Traffic  Calculated DESA = 2.65E+05  Note: Entering a value will overide the calculated value  Granular Pavement Design (Empirical Design Method)  Total Granular Pavement Thickness = 334 mm  Minimum Base Thickness = 113 mm  Remaining Pavement Thickness = 221 mm  Adopted Pavement Design  14/7mm two coat spray seal  150 mm  DGB 20 Base Course (97% M.D.D)  DGS 20 Subbase Course (97% M.D.D)  CBR = N/A  Select Fill Layer (95% M.D.D)  Total depth of granular material required = 334 mm	Design CBR								
Calculated DESA = 2.65E+05  Or Enter User Defined Value  Cranular Pavement Design (Empirical Design Method)  Total Granular Pavement Thickness = 334 mm  Minimum Base Thickness = 113 mm  Remaining Pavement Thickness = 221 mm  DGB 20 Base Course (97% M.D.D)  Total Granular Pavement Thickness = 150 mm  CBR = N/A  Select Fill Layer (95% M.D.D)  Total depth of granular material required = 334 mm	Design CBK		Н			Adopted	Design CBR =	5	%
Calculated DESA = 2.65E+05  Note: Entering a value will overide the calculated value  Granular Pavement Design (Empirical Design Method)  Total Granular Pavement Thickness = 334 mm  Minimum Base Thickness = 113 mm  Remaining Pavement Thickness = 221 mm  Adopted Pavement Design  14/7mm two coat spray seal  150 mm  DGB 20 Base Course (97% M.D.D)  DGS 20 Subbase Course (97% M.D.D)  CBR = N/A Select Fill Layer (95% M.D.D)  Total depth of granular material required = 334 mm			ш				_		Design period
Granular Pavement Design (Empirical Design Method)  Total Granular Pavement Thickness = 334 mm  Minimum Base Thickness = 113 mm  Remaining Pavement Thickness = 221 mm  Adopted Pavement Design  14/7mm two coat spray seal  DGB 20 Base Course (97% M.D.D)  DGS 20 Subbase Course (97% M.D.D)  CBR = N/A  Select Fill Layer (95% M.D.D)  Total depth of granular material required = 334 mm	Design Traffic		П						
Granular Pavement Design (Empirical Design Method)  Total Granular Pavement Thickness = 334 mm  Minimum Base Thickness = 113 mm  Remaining Pavement Thickness = 221 mm  Adopted Pavement Design  14/7mm two coat spray seal  DGB 20 Base Course (97% M.D.D)  Total depth of granular material required = 334 mm						Calc	ulated DESA =	2.65E+05	
Granular Pavement Design (Empirical Design Method)  Total Granular Pavement Thickness = 334 mm  Minimum Base Thickness = 113 mm  Remaining Pavement Thickness = 221 mm  Adopted Pavement Design  14/7mm two coat spray seal  DGB 20 Base Course (97% M.D.D)  185 mm  DGS 20 Subbase Course (97% M.D.D)  CBR = N/A  Select Fill Layer (95% M.D.D)  Total depth of granular material required = 334 mm			П			or Enter User De	efined Value		Note: Entering a value will overide the
Total Granular Pavement Thickness = 334 mm  Minimum Base Thickness = 113 mm  Remaining Pavement Thickness = 221 mm  Adopted Pavement Design  14/7mm two coat spray seal  DGB 20 Base Course (97% M.D.D)  DGS 20 Subbase Course (97% M.D.D)  CBR = N/A  Select Fill Layer (95% M.D.D)  Total depth of granular material required = 334 mm			Ш						calculated value
Minimum Base Thickness = 113 mm  Remaining Pavement Thickness = 221 mm  Adopted Pavement Design  14/7mm two coat spray seal  DGB 20 Base Course (97% M.D.D)  DGS 20 Subbase Course (97% M.D.D)  CBR = N/A Select Fill Layer (95% M.D.D)  CBR = 5 Subgrade (95% M.D.D)	Granular Pavement	Desig	n (En	npirical Desig	n Metho	d)			
Minimum Base Thickness = 113 mm  Remaining Pavement Thickness = 221 mm  Adopted Pavement Design  14/7mm two coat spray seal  DGB 20 Base Course (97% M.D.D)  DGS 20 Subbase Course (97% M.D.D)  CBR = N/A Select Fill Layer (95% M.D.D)  CBR = 5 Subgrade (95% M.D.D)			Ш						
Adopted Pavement Design  14/7mm two coat spray seal  DGB 20 Base Course (97% M.D.D)  Total depth of granular material required = 334 mm			ш		Iotal				
Adopted Pavement Design  14/7mm two coat spray seal  DGB 20 Base Course (97% M.D.D)  DGS 20 Subbase Course (97% M.D.D)  CBR = N/A  Select Fill Layer (95% M.D.D)  Total depth of granular material required = 334 mm			ш		_				
14/7mm two coat spray seal  DGB 20 Base Course (97% M.D.D)  DGS 20 Subbase Course (97% M.D.D)  O mm  CBR = N/A  Select Fill Layer (95% M.D.D)  Total depth of granular material required = 334 mm		_	+++		R	emaining Paveme	nt Thickness =	221	Jmm -
14/7mm two coat spray seal  DGB 20 Base Course (97% M.D.D)  DGS 20 Subbase Course (97% M.D.D)  O mm  CBR = N/A  Select Fill Layer (95% M.D.D)  Total depth of granular material required = 334 mm	Adamsad Davismans	Da alau							
DGB 20 Base Course (97% M.D.D)  DGS 20 Subbase Course (97% M.D.D)  O mm  CBR = N/A  Select Fill Layer (95% M.D.D)  Total depth of granular material required = 334 mm	Adopted Pavement	Design	1						
DGB 20 Base Course (97% M.D.D)  DGS 20 Subbase Course (97% M.D.D)  O mm  CBR = N/A  Select Fill Layer (95% M.D.D)  Total depth of granular material required = 334 mm		-		14/	7mm two	coat spray seal			
DGS 20 Subbase Course (97% M.D.D)  O mm CBR = N/A Select Fill Layer (95% M.D.D)  CBR = 5 Subgrade (95% M.D.D)  Total depth of granular material required = 334 mm									
DGS 20 Subbase Course (97% M.D.D)  O mm CBR = N/A Select Fill Layer (95% M.D.D)  CBR = 5 Subgrade (95% M.D.D)  Total depth of granular material required = 334 mm	150	mm						DGB 20 Base C	Course (97% M.D.D)
CBR = N/A  Select Fill Layer (95% M.D.D)  CBR = 5  Subgrade (95% M.D.D)  Total depth of granular material required = 334 mm			$\Box$						
CBR = 5 Subgrade (95% M.D.D)  Total depth of granular material required = 334 mm	185	mm	$\blacksquare$					DGS 20 Subbas	se Course (97% M.D.D)
CBR = 5 Subgrade (95% M.D.D)  Total depth of granular material required = 334 mm	1	mm	H		CBR =	N/A		Select Fill Lave	r (95% M.D.D)
Total depth of granular material required = 334 mm	0								,
	0	1			CBR =	5		Subgrade (95%	M.D.D)
	0								
	0							ma ma	
iotai ueptii oi granulai materiai provided = 335 mm	0		Tet	donth of	aules	havial results of			
	0					•			
This design has been carried out in accordance with the Austroads Guide to Pavement Technology Part 2: Pavement Structucal Design, 2012. The recommended pavement	0					•			
design/s are based on sampling and testing undertaken by others. Sampling provides a snapshot of subsurface conditions at the time of testing from a limited number of			Total	depth of grar	nular mat	erial provided =	335	mm	esign, 2012. The recommended navement

		J					ustroads 201			Genium CIVIL ENGINEERIN
Project Referen	ce: F	Rye Pa	rk Wind	farm					Designed by:	Simon Cassidy
Client: Project Descrip	tion:	Jpper	newable Lachlan a & Coo	Shire		I - Тур	ical paveme	ent Design	Date:	3/04/2020
DESIGN TRAFFI	C CALC	TA III	IONS		(Refer	Austro	pads 2012 Se	ction 7)		
DEGIGIT HIGHT	O OALL		L		, , ,					
Design Parame						Aus	stroads Ref.	Symbol	Value	
(Note: Enter values Design Period	in green	cells)					7.4.2	(P)	25	(Typically 20-40 years)
Annual Average [	Daily Tra	iffic				7	'.4.1, 7.4.4	(AADT)	222	vehicles/day
Direction Factor							7.4.1	(DF)	0.5	
% Heavy Vehicle							7.4.4	(%HV)	18	%
Lane Distribution						-	7.4.3	(LDF)	2	(Refer Table 2 in Tables Tab) %
Heavy Vehicle G Traffic Load Distr		ate					7.4.5	(R) (TLD)	Rural Collector	(Select from drop down list)
Cumulative Grow		or				7	.4.5 Eq. 15	(CGF)	32.0	a separation
Presumptive Axle			leavy Veh	icle				(N <sub>HVAG</sub> )	2.8	axle groups per heavy vehicle
Design Traffic							Eq. 14	(N <sub>DT</sub> )	6.54E+05	Heavy Vehicle axle Groups
							760	(ESA/HVAG)	0.8	
Presumptive Da selected TLD	ımage I	ndex	Values fo	or the			7.6.2	(ESA/HV)	2.2	
Design ESA of L	oading	(DES	A)				7.6.3	(DESA)	5.23E+05	
PAVEMENT DES	SIGN									
Designed in acco	ordance	with Fi	gure 8.4 a	and Fig	ure 12.2	of Au	stroads 2012			
Da siem CDD										
Design CBR							Adopted	Design CBR =	5	%
							•			
Design Traffic										
						<b>.</b>		ulated DESA =	5.23E+05	Note: Estados a valua villa varida da
						or I	Enter User De	etined value		Note: Entering a value will overide the calculated value
Granular Paven	nent De	sign (	Empirica	l Desig	ın Meth	od)				
					Tota	al Grar	nular Paveme	nt Thickness =	363	mm
						ı	Minimum Bas	se Thickness =	123	mm
						Remai	ning Paveme	nt Thickness =	240	mm
Adopted Paven	ent De	sign								
				14/	7mm tw	o coat	spray seal			
	105								DOD 00 D	(0704 14 7 7)
	165 r	nm							DGB 20 Base C	Course (97% M.D.D)
	200 r	nm							DGS 20 Subbas	se Course (97% M.D.D)
	0 r	nm			CBR	-	N/A		Select Fill Laye	r (95% M.D.D)
					CDD				0 1 1. (050)	M D D)
					CBR		5		Subgrade (95%	(ט.ט.)
		$\parallel$								
							l required =		mm	
		Tot	al depth	of gran	nular m	ateria	l provided =	365	mm	
This design has b	een carri	ed out in	accordance	e with the	e Austroa	ds Guide	e to Pavement Te	echnology Part 2: P	avement Structucal D	esign, 2012. The recommended pavement

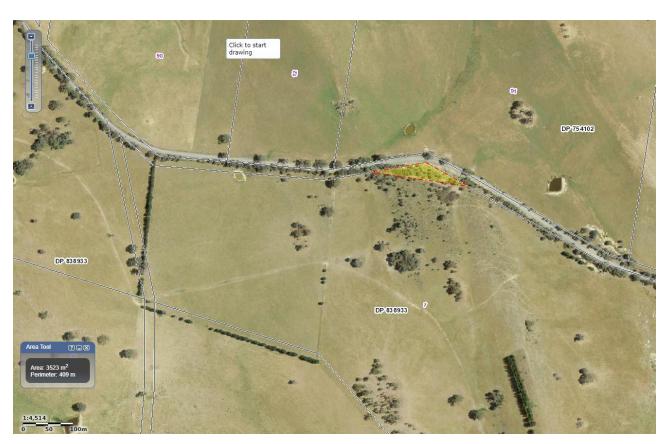
### Attachment H – Potential Land Acquisition Areas



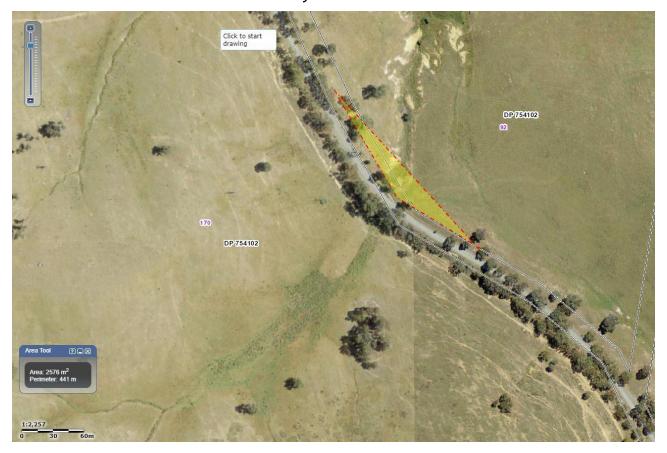
Site 1 – Coolalie Road



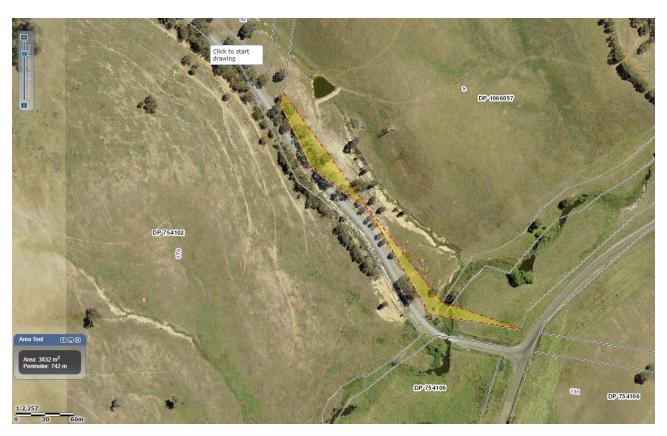
Site 2 – Coolalie Road



Site 3 – Rye Park-Dalton Road



Site 4 - Rye Park-Dalton Road



Site 5 - Rye Park-Dalton Road



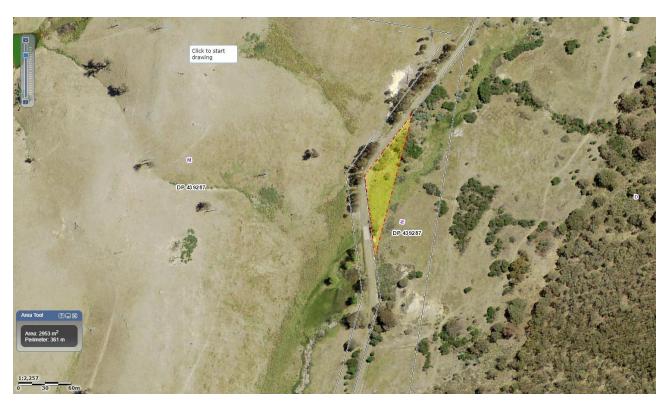
Site 6 - Rye Park-Dalton Road



Site 7 – Grassy Creek Road



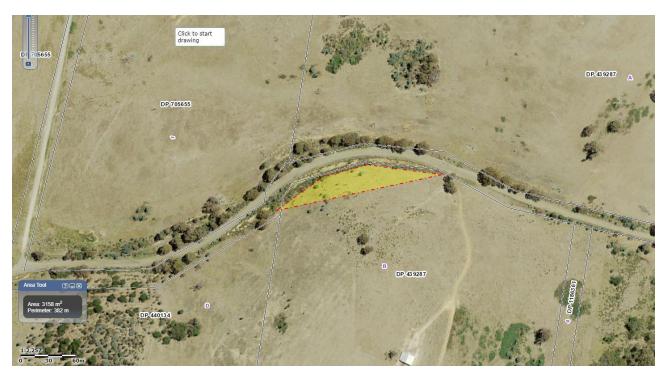
Site 8 – Grassy Creek Road



Site 9 – Grassy Creek Road



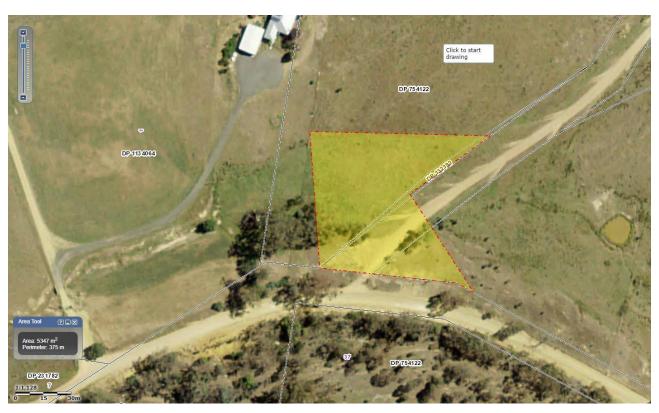
Site 10 – Grassy Creek Road



Site 11 – Grassy Creek Road



Site 12 – Jerrawa Road/Coolalie Rd Intersection



Site 13 - Bushs Road/Coolalie Rd Intersection



Site 14 - Days Road



Site 15 - Long Street/Dillon Street Intersection



Site 16 – Dillon Street/Boorowa-Rye Park Rd Intersection



Site 17 – Dillon Street/Boorowa-Rye Park Rd Intersection



Site 18 – Boorowa-Rye Park Rd/Grassy Creek Rd Intersection



Site 19 – Jerrawa Road/Coolalie Rd Intersection