



Buttai Gravel Pty Ltd (Daracon)

Martins Creek Quarry,
Martins Creek NSW

Traffic Impact Assessment

August 2016

SECAsolution >>>

Martins Creek Quarry

Traffic and Access Assessment

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1 Introduction

1.1 Background

Seca Solution was commissioned by Site R+D on behalf of Buttai Gravel Pty Ltd to prepare a Traffic Impact Assessment for the proposed on-going use and increase extraction capacity of the existing quarry known as Martins Creek Quarry, Martins Creek, NSW. This report will form part of an Environmental Impact Statement (EIS) being prepared for the project to support a submission to the Department of Planning and Infrastructure. The report is based upon:

- a review of the historic and current operations of the quarry;
- a review of the transport operation for the quarry based upon historic data provided by the quarry operators;
- the collection of traffic data along the primary traffic routes associated with the transport of quarry product;
- extensive consultation with the road authorities, and
- site visits to the location, surrounding road network and analysis by Seca Solution.

This assessment has been prepared in accordance with Austroads Guidelines and the "RTA Guide to Traffic Generating Developments" published by the Roads and Maritime Services (RMS).

1.2 Scope of Report

The scope of this report is to review the external traffic impacts of the on-going operation of the quarry and provides advice on the capacity, efficiency and safety issues associated with the road network. This includes a road safety review of the proposed routes and a review of access into the quarry.

1.3 Issues and Objectives of the study

The issues as they relate to the proposal are:

- Assess the impact on the local road network due to the additional traffic flows associated with an expansion of the quarry from the current extraction rate of 900,000 tonnes per annum to 1,500,000 tonnes per annum;
- To review the access arrangements for the development; and
- Assess any capacity, efficiency and safety impacts associated with the development.

The objective of the report is to document the impacts of the proposed development and provide advice on any measures to be implemented as part of the development.

1.4 Planning Context

In preparing this document, the following guides and publications were used:

- RTA Guide to Traffic Generating Developments, Version 2.2 Dated October 2002;
- RMS TDT 2013/04 "Update Traffic surveys August 2013".
- Austroads Guide to Traffic Management – Part 12 Traffic Impacts of Developments

1.5 Authority Requirements

The Department of Planning and Environment have provided a set of SEAR's (Secretary's Environmental Assessment Requirements) that provide advice on what is to be assessed for the project and the information that is required to be provided. Relevant to traffic are the following sections:

Table 1-1 – Department of Planning and Environment SEARs

Requirement	Relevant Section of Seca Solution report
Accurate predictions of the road traffic generated by the construction and operation of the development, including a description of the types of vehicles likely to be used for transportation of quarry products, the public roads in the Dungog Shire, Maitland City and Port Stephens LGAs likely to be so used and the times during which those roads would be so used.	Section 4.1
A detailed assessment of potential traffic impacts on the capacity, condition, safety and efficiency of the local and State road network (as identified above), having regard to the requirements of the RMS, Dungog Shire, Maitland City and Port Stephens Councils (see Attachment 2);	Section 4.2, Section 4.3, Section 4.4
A detailed assessment of the existing railway siding facility at the site as an alternative transport option for delivering quarry products and avoiding potential impacts associated with road delivery.	Undertaken by others
A detailed description of the measures or works (including concept plans) that would be used and/or implemented to upgrade, maintain and improve the capacity, efficiency and safety of the road network used by the development.	Undertaken by others

The Roads and Maritime Services have provided separate details on their requirements for the project and are provided below:

Table 1-2 – Roads and Maritime Services requirements

Requirement	Relevant Section of Seca Solution report
Identify all relevant vehicular traffic routes and intersections for access to / from the subject site and any connections to the classified state road network.	Section 2.5 Section 4.2
The anticipated additional vehicular traffic likely to be generated as a result of the subject development.	Section 4.1
The distribution on the road network of the trips generated by the proposed development.	Section 4.2
Consideration of the traffic impacts on existing intersections and the capacity of the intersections to safely and efficiently cater for additional vehicular traffic generated by the proposed development. The traffic impact shall also include the cumulative traffic impact of other proposed developments in the area if any.	Section 4.4

Identify any necessary road network infrastructure upgrades that are required to maintain existing levels of service as a result of the development. In this regard, preliminary concept drawings shall be submitted with the EA for any identified road infrastructure upgrades. Any road upgrades will need to be to the satisfaction of the RMS and Council.	Section 5
<p>Traffic analysis of the affected intersections, using Sidra or similar modelling and include:</p> <ul style="list-style-type: none"> • Current traffic counts and 10 year traffic growth projections • With and without development scenarios • 95th percentile back of queue lengths • Delays and level of service on all legs for the relevant intersections • Electronic data for RMS review 	Section 4.4

2 Existing Situation

2.1 Site Description and Proposed Activity

Martins Creek Quarry is located on the northern boundary of Martins Creek, 25 kms north of Maitland. The site has been operating as a quarry since approximately 1915 and has been managed by a number of different operators. The quarry is now leased and operated by the Daracon Group and has been under their control since December 2012 with a 25 year agreement.

The proposal is to continue with the existing quarry activities on the site and to:

- Extract up to 1.5 million tonnes of hard rock material per annum;
- Expand into new extraction areas, clearing approximately 36.8 hectares of vegetation;
- Increasing hours of operation for quarrying to 6am to 6pm (Monday to Saturday), processing to 6am to 10pm (Monday to Saturday), mixing and binding to 4.30am – 10pm (Monday to Friday) and 4.30am to 6pm (Saturdays), stockpiling, loading and dispatch of road transportation to 5.30am-7pm (Monday to Saturday) and train loading to 24 hours per day, 7 days per week;
- Consolidating existing operations and approvals;
- Rehabilitating the site

2.2 Site Location

The site is located on the northern edge of the township of Martins Creek, some 4 kms north of Paterson and 25 kms north of Maitland. It is located within the Dungog Local Government Area. The site currently gains access via Station Street through Martins Creek and along Grace Avenue to connect with the external road network (Dungog Road).

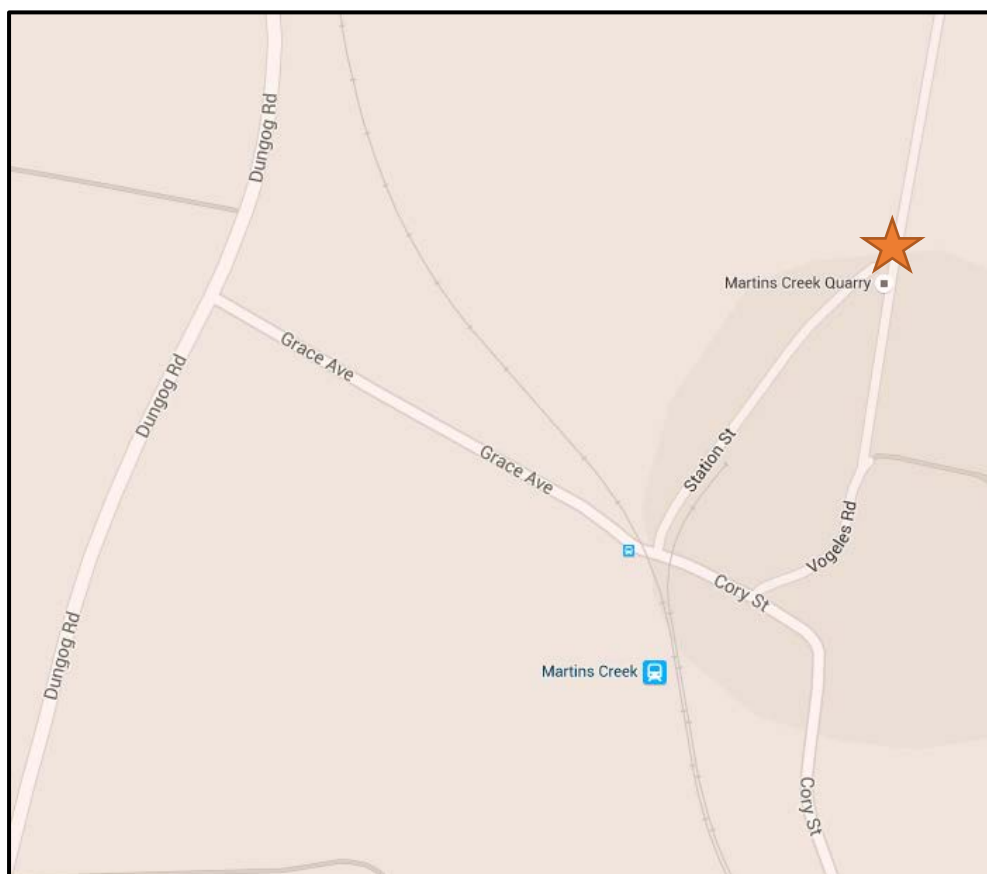


Figure 2-1 Martins Creek Quarry site in the local context

Source: Google maps

The location of the site is shown below in Figure 2-2.

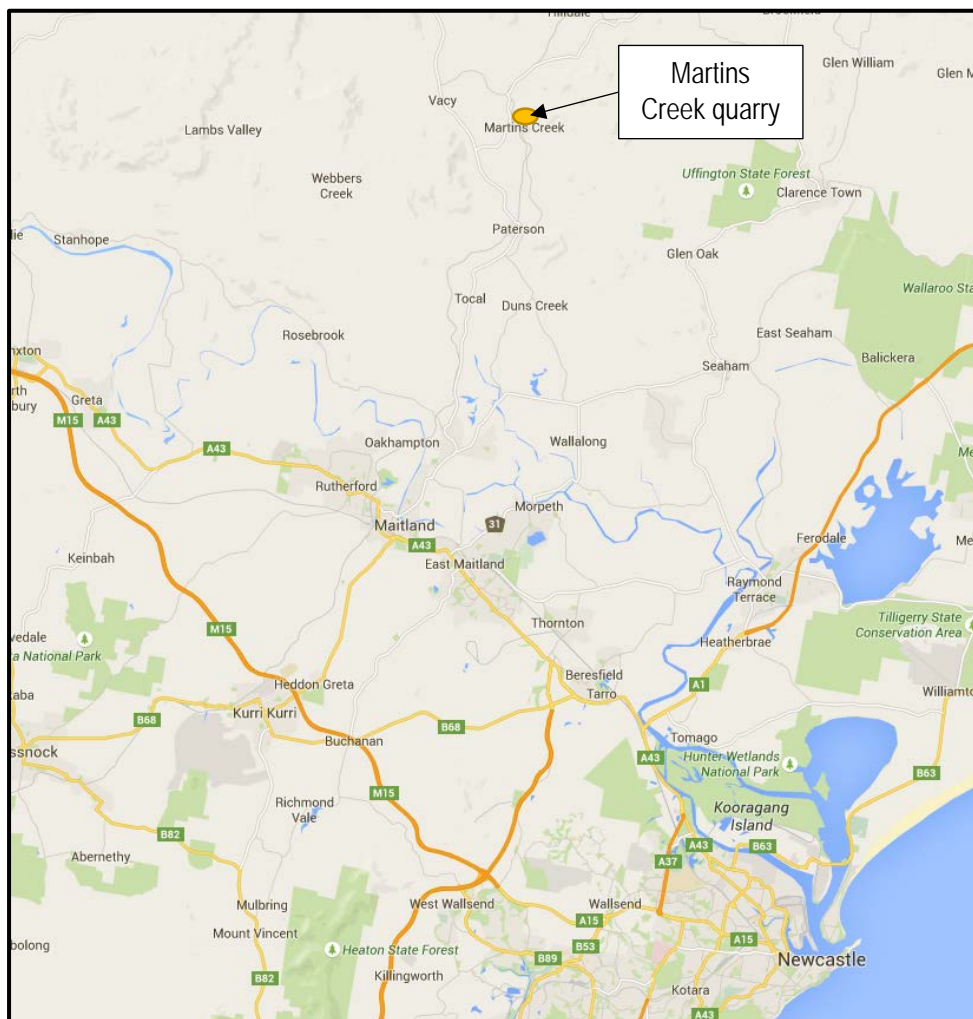


Figure 2-2 - Site Location in the regional context

Source: Google maps

2.2.1 Zoning and Adjacent Land Use

Existing land use adjacent to the site is generally rural residential and urban residential together with rural grazing land. The quarry site is currently zoned as primary production.

2.3 Site Access

Site access is available via a number of different access points that connect to the local road network. The primary site access is via Station Street that then connects with Grace Avenue and Dungog Road to provide a route through to Gresford Road, which is the primary road connection.

The site access at this location caters for both the trucks entering and exiting the site as well as light vehicles associated with staff movements.

A second access is also available via Vogeles Road to the east of Station Street.

There is a railway line running along the western boundary of the site that stops vehicle access connecting directly with Dungog Road to the west of the site.

2.4 Existing Traffic Conditions

2.4.1 Road Hierarchy

The major route through the locality is the regional road (number 101) that's connects from West Maitland (at the New England Highway) via Bolwarra, Paterson, Wallarobba, Wirragulla, Dungog, Dingadee and Walshpool Bridge to The Bucketts Way. The current route follows Belmore Road at its southern end, then Paterson Road and Tocal Road through to Paterson before changing to Gresford Road north of Paterson and Dungog Road as it passes through Martins Creek. This is the primary access route to the quarry with vehicles travelling either along Belmore Road through Lorn or along Flat Road. It has been determined that the future route will be via Flat Road, including Glenarvon Road and Pitnacree Road, and its characteristics are summarised in Table 2-1 below.

2.4.1.1 Route One

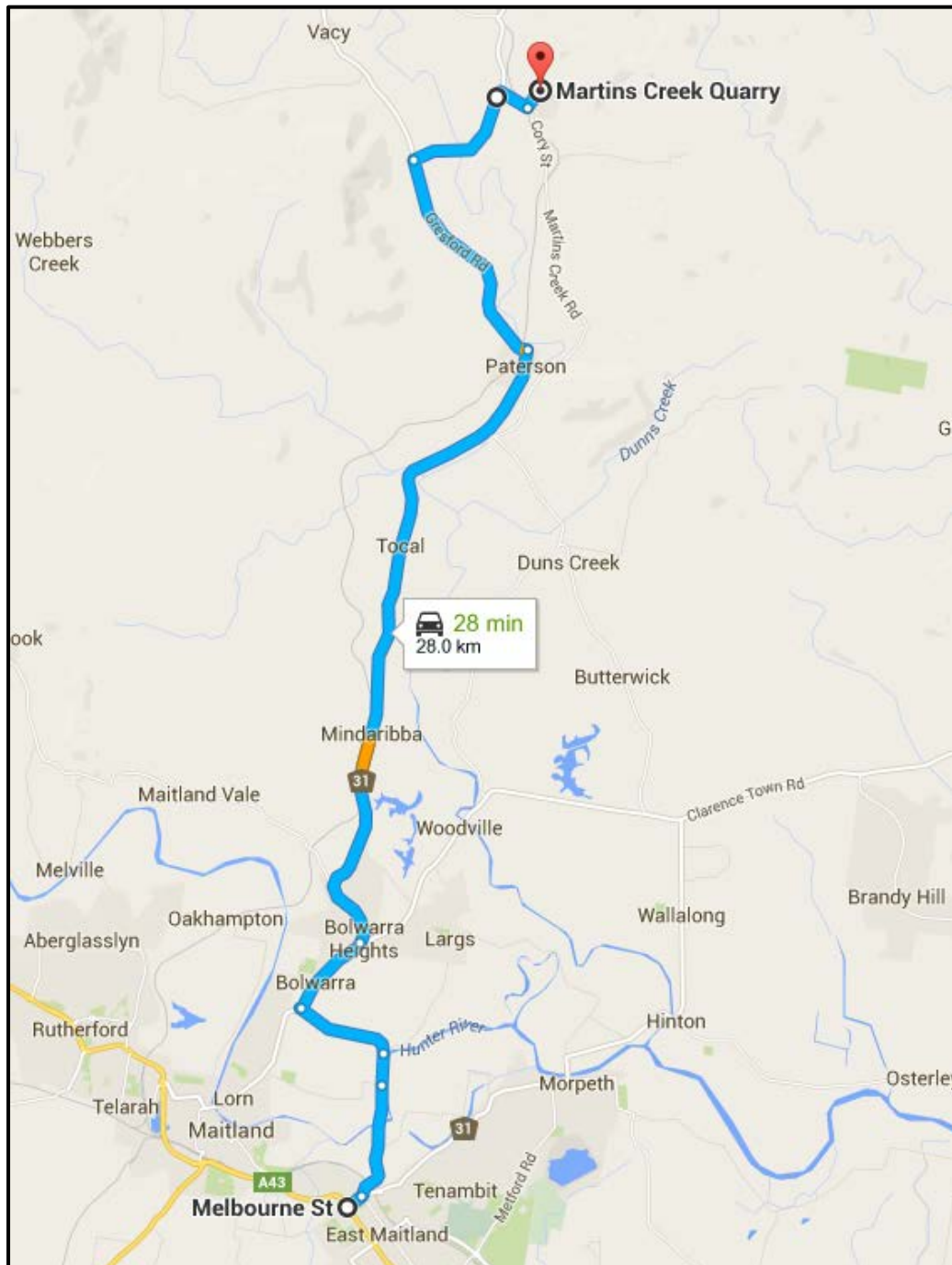


Figure 2-3 Quarry Haulage Route 1

Table 2-1 Characteristics of Route 1

Section	Length	Characteristics
Dungog Road between Grace Avenue and Gresford Road	2.4 kms	Single lane of travel in both directions, no shoulders and formed verges. Posted speed limit of 80 km/h, provides access to a number of rural holdings. Has a width restriction over the Paterson River which permits a single lane of travel only over a heritage wooden bridge. It provides a reasonably straight alignment with a noticeable curve with speed reduction sign (35 km/h advisory) on the northern approach to the bridge over the Paterson River. There are no overtaking lanes provided.
Gresford Road between Dungog Road and Paterson	4.3 kms	Single lane of travel in both directions; no shoulders nor formed verges. Posted speed limit of 80 km/h, provides access to a number of rural holdings and rural residential lots. This section of the road provides a very good alignment with a single curve with an advisory speed limit of 65 km/h. The speed limit reduces to 50 km/h at the southern end of this section of the road when it passes through the village of Paterson. There are no overtaking lanes provided.
Gresford Road / Tocal Road through Paterson	1.2 kms	This section of road provides a single lane of travel in both directions with additional road pavement to both sides to permit on-street parking. It provides access to the local facilities within Paterson as well as residential lots fronting directly onto this road. It also allows for connection to a number of side roads for the residential area of Paterson. It operates under a posted speed limit of 50 km/h and there are limited pedestrian paths except in the centre of the village. The connection of Gresford Road and Tocal Road is a right angle requiring vehicles to slow down and large vehicles e.g. semi-trailer or truck and dog combination are required to use all of the provided road pavement width to complete the turn within their lane. It provides a minimum width of 7.0 metres with 12 metres provided in the centre of the village to accommodate the on-street parking.
Tocal Road between Paterson and Dungog / Maitland LGA boundary	3.3 kms	This section of the road provides a single lane of travel in both directions with a narrow shoulder over much of its length. Where it crosses Webbers Creek there is no shoulder and the width between the kerbs is 7 metres. There are no footpaths along this section of the road and the bridge over Webbers Creek creates a pinch point. This section of the road has a posted speed limit of 100 km/h with an advisory speed limit of 65 km/h for the bend on the northbound approach to Webbers Creek.
Tocal Road between Dungog / Maitland LGA boundary and Lang Drive	6.4 kms	This section of road provides a good alignment with sealed shoulders to both sides of the road, operating under a posted speed limit of 100 km/h reducing to 80 km/h at its southern end. There are no footpaths and the overall width is 9 metres.
Tocal Road between Lang Drive and Bolwarra Heights	1.0 km	Over this length of the road, the alignment has several tighter bends, which encourage drivers approaching the built up area to reduce their vehicle speeds in line with the posted speed limit of 60 km/h through Bolwarra Heights. There is no shoulder over the majority of the length of the road and no footpaths. At its southern end it connects with Maitland Vale Road and enters the urban area of Bolwarra Heights.
Tocal Road between Maitland Vale Road and Paterson Road	1.3 kms	Provides a single lane of travel in each direction and operates under the posted speed limit of 60 km/h. There is a partial pedestrian footpath along the eastern side of the road and there are individual residential lots with driveway access direct onto the road at this location. There are a number of street lights reflective of the urban

		road environment in this locality. The road alignment is reflective of the urban built form and the posted speed limit, with a curved alignment.
Paterson Road between Tocal Road and Flat Road	1.7 kms	Provides a single lane of travel in both directions with a marked parking lane to both sides over the majority of its length. Allows for direct driveway access to residential lots and operates under the posted speed limit of 60 km/h. No footpaths are provided along the majority of its length and limited street lights. Provides an overall pavement width of 12.5 metres for the majority of its length. Generally a straight alignment.
Flat Road between Paterson Road and Melbourne Street	4.9 kms	This road forms part of the 3 rd Hunter River Project completed by the RMS. This section of the road has been built to current design standards, providing a single lane of travel in both directions with a wide sealed shoulder to both sides to cater for breakdowns and cyclists. It operates under the posted speed limit of 80 km/h and provides a good alignment. There are no pedestrian paths along this road.
Melbourne Street between Flat Road and the New England Highway	350 metres	This road forms part of the State Road network and provides two lanes of travel in both directions, with additional turn lanes at the key intersections. It connects with Flat Road and the New England Highway via 4-way traffic signals. There is kerb side parking permitted along the majority of its length together with footpaths to both sides. It operates under the posted speed limit of 60 km/h.
New England Highway (State Highway)		This road forms part of the state highway network and provides a minimum of 2 lanes of travel in both directions. Additional lanes are provided at the key intersections and there are parking lanes to both sides for the majority of its length in this location. In this location it operates under the posted speed limit of 60 km/h.

From the New England Highway, trucks can access both local and regional markets. These routes can utilise the New England Highway as well as the Hunter Expressway to access areas including the Upper Hunter Valley, Greater Newcastle and its surrounds, the M1 towards the Central Coast and Sydney as well as areas in the immediate locality e.g. Gillieston Heights, Rutherford, Thornton etc.

2.4.1.2 Route Two

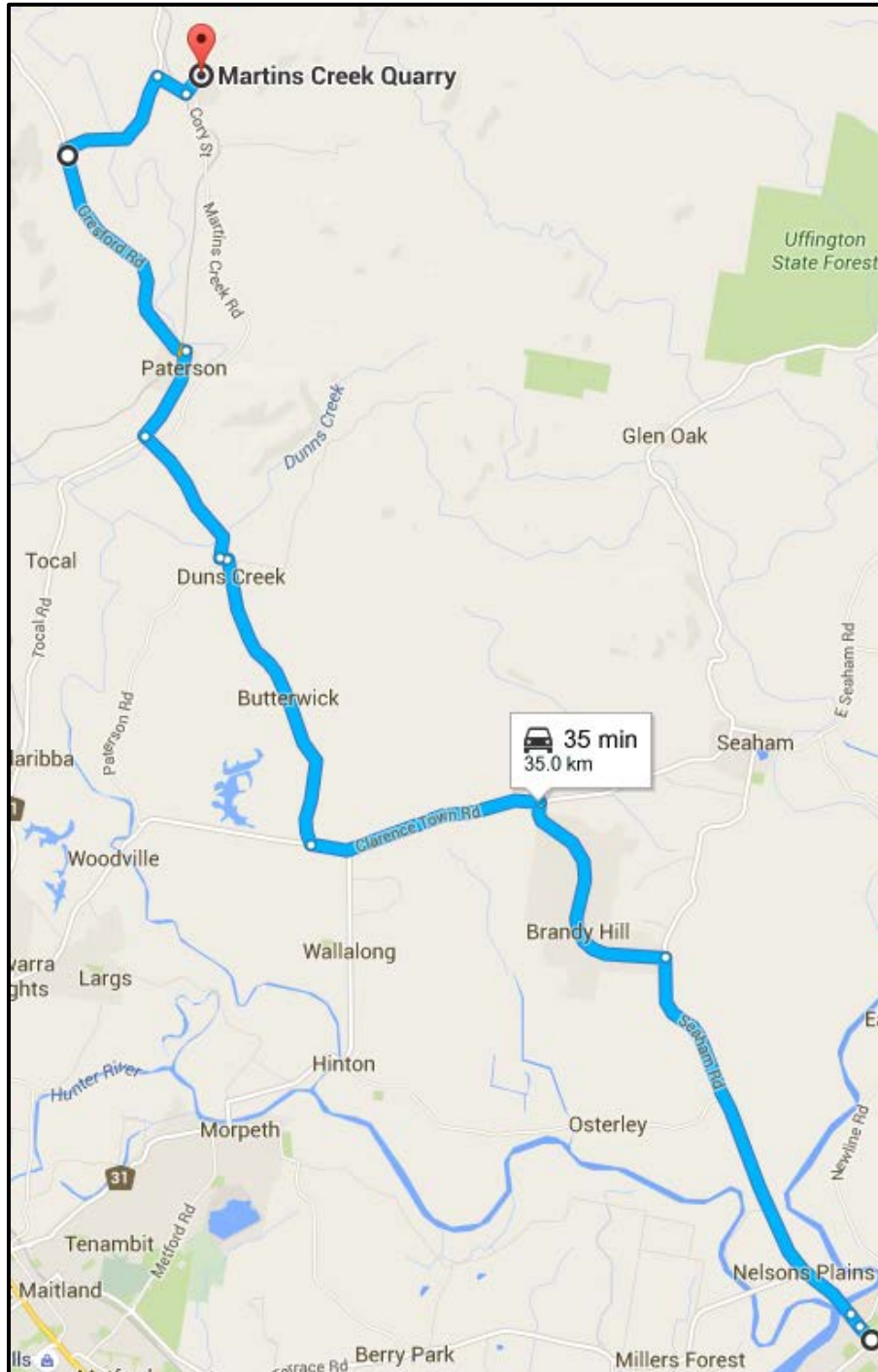


Figure 2-4 Quarry Haulage Route 2

Route Two allows for access to the east of the site, for areas within the Port Stephens LGA as well as access to the north along the Pacific Highway. This route allows for direct access to Raymond Terrace and then a connection to the greater road network from there.

Table 2-2 - Characteristics of Route 2 via Butterwick, Brandy Hill and Raymond Terrace

Section	Length	Characteristics
Paterson Road between Tocal Road and Duns Creek Road	2.7 kms	Provides a single lane of travel in both directions with a narrow sealed shoulder to both sides. It connects with Tocal Road via a simple give way controlled intersection. It operates under the posted speed limit of 80 km/h and provides a relatively level terrain but with limited over taking opportunities.
Duns Creek Road between Paterson Road and Butterwick Road	140 metres	This short length of the route provides a single lane of travel in both directions with a narrow sealed shoulder to both sides. It connects with Paterson Road at its western end and Butterwick Road at its eastern end via simple give way controlled intersections. It operates under the posted speed limit of 80 km/h.
Butterwick Road between Duns Creek Road and Clarence Town Road	5.6 kms	Provides a single lane of travel in both directions and over a long section of its length does not provide any shoulder or edge lines. There are edge lines provided over the remainder of the road length with sealed narrow shoulders. The road provides a reasonably straight alignment with a number of vertical curves which reduce driver's visibility and discourage speeding. There are a number of minor roads along its length which connect with Butterwick Road via simple give way control.
Clarence Town Road between Butterwick Road and Brandy Hill Drive	4.3 kms	Provides a single lane of travel in both directions and a sealed shoulder of varying width for the majority of its length. There is a painted edge line along the full length of this road and it operates under the posted speed limit of 100 km/h.
Brandy Hill Drive	4.5 kms	Provides a single lane of travel in both directions and operates under the posted speed limit of 80 km/h. For the majority of its length there is a solid double white line stopping vehicles overtaking. There are large lot semi-rural residential developments off both sides of the road with driveway connections to Brandy Hill Drive. It connects at both ends via simple give way controlled intersections.
Seaham Road between Brandy Hill Drive and Port Stephens Street	7.7 kms	Provides a single lane of travel in both directions and provides an edge line for the full length of the road. It provides a sealed shoulder of varying width along its full length and operates under the posted speed limit of 80 / 100 km/h until it crosses over the Hunter River where the speed limit reduces to 60 km/h reflective of the urban area of Raymond Terrace on the south-eastern side of the Hunter River. There are no footpaths provided along the majority of its length although there are footpaths provided on the bridge over the Hunter River. Seaham Road connects with William Bailey Street via a 4-way roundabout controlled intersection.
William Bailey Street	360 metres	Provides a single lane of travel in both directions and operates under the posted speed limit of 60 km/h. It provides a carriageway width in the order of 10 metres and does not provide any footpaths. It connects with Adelaide Street at its southern end via a 3-way signal controlled intersection that allows for all turning movements as well as caters for pedestrian crossing movements.
Adelaide Street (State Highway)		Adelaide Street forms part of the State Highway network and provides a mixture of 1 and 2 lanes of travel in each direction. This road used to form part of the Pacific Highway and as such is built to a high standard and now carries significantly lower traffic flows than prior to the construction of the Raymond Terrace bypass which formed part of the Pacific Highway upgrade in this location. It provides a footpath along the eastern side of the road and one along the western side of the road for the majority of its length within

		Raymond Terrace. It connects with the Pacific Highway to the north and south of this location as well as Richardson Road to the north that provides a connection through to Port Stephens.
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Martins Creek quarry provides material for local Councils in the Lower Hunter Valley as well as other projects requiring access at times via other routes. The volume of material transported along these other routes is much lower and sporadic and include:

- The use of Dungog Road to access projects within Dungog and the Dungog LGA
- The use of Gresford Road north to East Gresford and beyond
- The on-going use of Belmore Road through Lorn to access the local market in Lorn and Maitland

The major intersections along Route One, where trucks associated with the haulage are required to give way are:

- Dungog Road and Gresford Road- give way control with Gresford Road being the priority road
- Paterson Road and Flat Road – three way roundabout
- Pitnacree Road and Melbourne Street – four way traffic signal control;
- Melbourne Street and New England Highway- four way traffic signal control

The major intersections along Route Two where trucks associated with the haulage are required to give way are:

- Dungog Road and Gresford Road- give way control with Gresford Road being the priority road
- Tocal Road and Paterson Road – give way control with Tocal Road the priority road
- Paterson Road and Duns Creek Road – give way control with Paterson Road being the priority road
- Duns Creek Road and Butterwick Road – give way control with Duns Creek Road being the priority road
- Butterwick Road and Clarence Town Road- give way control with Clarence Town Road being the priority road
- Clarence Town Road and Brandy Hill Drive – 4-way give way control with Clarence Town Road being the priority road
- Brandy Hill Drive and Seaham Road – give way control with Seaham Road being the priority road
- William Bailey Street and Newline Road / Port Stephens Street – 4-way roundabout control
- William Bailey Street and Adelaide Street – 3 way traffic signals

2.4.2 Roadworks (Current and Proposed)

Discussion with the road authorities (various Councils and the RMS) indicates that there are no proposed road network changes along either of the major transport routes used by Martins Creek Quarry nor other roads works that will be impacted upon by the proposed expansion.

The RMS has completed the construction of the Hunter Expressway and this has significantly altered the traffic patterns along the New England Highway. In particular, it has reduced the through traffic movements along the length of the New England Highway which has improved the capacity of the side roads, which at Melbourne Street provides a benefit for the project. The RMS is currently upgrading the intersection of the New England Highway and Cessnock Road, which suffers from significant delays and congestion during the traditional peak periods. This part of the road network is used by haulage trucks from Martins Creek that travel along Cessnock Road to link with the Hunter Expressway to the south of this location.

The various Councils have indicated that other than routine maintenance, there are no planned road upgrades within the general vicinity of the subject site or along the haul routes.

Discussion has also been held with the RMS with regards to the heritage listed wooden bridge on Dungog Road over the Paterson River. The RMS has indicated that this bridge is subject to on-going maintenance and has been approved for on-going use by heavy vehicles which include the truck and dog combinations utilised at Martins Creek Quarry.

2.4.3 Traffic Management Works

There are currently no traffic management works occurring in the vicinity of the subject site.

It is noted that during the project study period there has been repair work completed on the wooden bridge on Dungog Road. During this time, when the bridge was closed in association with on-going maintenance works, a temporary diversion was installed to direct vehicles north along Dungog Road and then west along Horns Crossing Road to connect with Gresford Road. These vehicles then headed south along Gresford Road through Vacy to continue south via Paterson.

2.4.4 Pedestrian and Cycling Facilities

There are no pedestrian or cyclist's facilities provided along the majority of the length of the local roads along either of the major haulage routes. This is typical of the rural setting for the project and is reflective of the low traffic volumes and the very low demand for pedestrians or cyclists along these routes. Through Paterson there are footpaths provided in a non-continual manner. These paths are in the centre of the town where there are some pedestrian demands.

A footpath to the eastern side of Tocal Road is provided through Bolwarra Heights which ensures pedestrian safety is maintained through the narrow section of the road reserve. There are no footpaths provided on Paterson Road further south, including the recently upgraded section to the north of Flat Road.

Footpaths are provided within East Maitland, reflective of the much higher pedestrian demands and the urban development. Similarly there are footpaths within Raymond Terrace along the main road network.

For other minor routes utilised by the Martins Creek quarry, the majority of the roads are in rural areas with no footpaths or cyclist facilities, reflective of the low traffic flows and demands by pedestrians or cyclists. Within the local town centres e.g. Dungog there are footpaths provided within the central business district only, reflective of the higher demands in these areas.

2.5 Traffic Flows

2.5.1 Peak Hour Flows

The proposed development is for an expansion of the current quarry activities. The quarry is open over extended hours and creates higher truck movements during the morning period, due to the demands of construction work.

As part of the project work, Seca Solution has collected extensive traffic data along Routes One and Two utilised by the project traffic. Tube counters were installed in week beginning 17th July 2015. These traffic surveys have involved using automatic tube counts, which collect data over a 24 hour period over a minimum of 7 days. These traffic counters also provide the split in traffic movements by direction as well as a breakdown of vehicle numbers by class as defined by Austroads. A summary of the peak hour traffic volumes by location is provided below.

Table 2-3 – Summary of hourly traffic flows by direction and two-way

Location	AM Peak			PM peak		
	Inbound Direction	Outbound Direction	2-way	Inbound Direction	Outbound Direction	2-way
Dungog Road, midway between Grace Ave and Gresford Road	57	77	134	71	60	131
Gresford Road north of Paterson	111	181	292	168	126	294
Paterson Road at Bolwarra	369	744	1113	669	400	1069
Flat Road midway between Paterson Road and Melbourne Street	263	507	770	487	318	805
Melbourne Street	637	852	1489	862	662	1524
Butterwick Road midway between Tocal Road and Clarence Town Road	47	83	130	86	51	137
Clarence Town Road, midway between Butterwick Road and Brandy Hill	124	184	308	170	143	313
Brandy Hill Drive	62	90	152	94	68	162
Seaham Road, midway between Brandy Hill Drive and Port Stephens Street	199	363	562	366	231	597

Note: Inbound direction equates to vehicle movements heading towards the Martins Creek quarry project whilst outbound is vehicles heading away from the Martins Creek quarry project.

2.5.2 Daily Traffic Flows

The traffic data for existing daily traffic flows via the tube counts also provides details on the Annual Average Daily Traffic flow (AADT) and the AADT for the various sections of the major transport routes is provided below.

Table 2-4 – Daily traffic volumes

Location	Daily traffic flows
Dungog Road, midway between Grace Ave and Gresford Rd	1,445
Gresford Road north of Paterson	3,078
Tocal Road, midway between Paterson and Bolwarra Heights	3,616
Paterson Road at Bolwarra	11,688
Flat Road midway between Paterson Rd and Melbourne St	8,659
Melbourne Street	9,007
Butterwick Road midway between Tocal Rd and Clarence Town Rd	1,423
Clarence Town Road, midway between Butterwick Rd and Brandy Hill	3,389
Brandy Hill Drive	1,816
Seaham Road, midway between Brandy Hill Dr and Port Stephens St	6,623

2.5.3 Daily Traffic Flow Distribution

The daily traffic volumes are reasonably balanced in both directions, with the above data indicating a slight bias in movements southbound (outbound) in the AM peak towards Maitland and the New England Highway and towards Raymond Terrace. These would be reflective of commuter type trips to the major centres such as Maitland and Newcastle for work and education requirements. During the PM peak periods, the traffic flows with a bias for northbound (inbound) movements. These would be reflective of reverse commuter trips heading home after work, educational and retail requirements, with these centres all being to the south of Martins Creek.

2.5.4 Vehicle Speeds

The traffic data collected by the automatic tube counters also included the collection of vehicle speeds. A summary of the vehicle speeds from the surveys is provided below.

Table 2-5 – Summary of 85th percentile speeds and posted speed limits

Location	85 th percentile speed	Posted speed limit
Dungog Road, midway between Grace Avenue and Gresford Road	93 km/h	80 km/h
Gresford Road north of Paterson	95 km/h	80 km/h
Tocal Road, midway between Paterson and Bolwarra Heights	98 km/h	100 km/h
Paterson Road at Bolwarra	68 km/h	60 km/h
Flat Road midway between Paterson Road and Melbourne Street	87 km/h	80 km/h
Melbourne Street	56 km/h	60 km/h
Butterwick Road midway between Tocal Road and Clarence Town Road	105 km/h	100 km/h
Clarence Town Road, midway between Butterwick Road and Brandy Hill	105 km/h	100 km/h
Brandy Hill Drive	88 km/h	80 km/h
Seaham Road, midway between Brandy Hill Drive and Port Stephens Street	102 km/h	100 km/h

The 85th percentile speed is the speed at or below which 85% of all vehicles travelled.

The above data shows that in the more rural areas of the transport routes the 85th percentile vehicles speeds exceed the posted speed limit by a reasonable margin, but within the more residential areas the vehicle speeds are nearer to the posted speed limit.

Additional data was extrapolated from the tube counters to determine the vehicle speeds by vehicle class (as defined by Austroads Standards). A summary of the results of the data is provided below.

Table 2-6 – Summary of speeds by vehicle class (cars and quarry haulage vehicles) for typical working day

Location	Light vehicle speeds	Quarry haulage vehicle speeds	Posted speed limit
Gresford Road, north of railway crossing at Paterson	46.3 km/h	37.2km/hr	50 km/h
Paterson Road, Bolwarra to immediate south of Tocal Road	66.8 km/h	63.0 km/h	60 km/h
Flat Road	87.9 km/h	87.0 km/h	80 km/h
Butterwick Road southbound	103.3 km/h	97.7 km/h	100 km/h
Butterwick Road northbound	104.6 km/h	98.7 km/h	100 km/h

The above results demonstrate that the trucks associated with the quarry haulage travel slower than the light vehicles. Trucks typically travel within the posted speed limit except on Paterson Road (Bolwarra to the immediate south of Tocal Road) and along Flat Road. It can be seen that Flat Road provides a high standard of road design and construction which leads to all drivers regularly driving over the posted speed limit of 80 km/h.

2.5.5 Existing Site Flows

The site is currently an operational quarry with associated truck and light vehicle movements in and out of the site. All vehicle access is currently via the main access point on Station Street. This access connects with Grace Avenue to then provide access to the greater road network. The historic tonnage removed from Martins Creek and associated truck movements, based on 32.5 tonnes per truck load, associated with Martins Creek quarry are shown below.

Table 2-7 – Historic haulage values (Tonnes) and associated truck movements for Martins Creek Quarry (Source: Daracon)

Martins Creek Quarry Sales (Financial Year Tonnes)						
Operator	Financial Year	Sales for Financial Year	Truck Movements per Year	Per Wk (50 wks/pa)	Trucks per day Monday-Friday	Trucks per day Saturday
RailCorp	2003/2004	772,984.00	23,784	476	86	43
	2004/2005	652,991.00	20,092	402	73	37
	2005/2006	828,684.00	25,498	510	93	46
	2006/2007	609,487.00	18,753	375	68	34
	2007/2008	687,287.00	21,147	423	77	38
	2008/2009	633,397.00	19,489	390	71	35
	2009/2010	645,821.00	19,871	397	72	36
	2010/2011	569,930.00	17,536	351	64	32
	2011/2012	834,254.00	25,669	513	93	47
Railcorp/ Buttai Gravel	2012/2013	940,326.00	28,933	579	105	53
Buttai Gravel	2013/2014	1,152,228.76	35,453	709	129	64
	2014/2015	906,524.12	27,893	558	101	51
Summary	Average	769,492.82	23,677	493	90	45
	Median	730,135.50	22,466	468	85	43

Note that the above values are for *average* outbound truck movements only based on tonnes. For every outbound laden truck there is a corresponding inbound un-laden truck movement.

The above values represent the average truck movements per day, but operations on site vary considerably by day and also by hour through the normal working day. The operational characteristics of the quarry allows a maximum throughput of up to 40 trucks per hour outbound. The table below shows the average weekday haulage of material for the years 2013 and 2014 by month, based upon the data provided from the weighbridge on site.

Table 2-8 – Average weekday demand by month for 2013 and 2014 for existing operations at Martins Creek quarry (Source: Daracon)

Year	2013 weekday average (tonnes)	2013 weekday average (trucks)	2014 weekday average (tonnes)	2014 weekday average (trucks)
January	1362	42	3330	103
February	2959	91	5985	184
March	2771	85	6720	207
April	2168	67	4821	148
May	3045	94	5671	175
June	2556	79	3958	122
July	4467	138	3270	101
August	3159	98	3190	98
September	3967	122	4162	128
October	3408	105	3330	103
November	2050	63	3742	116
December	2849	88	unavailable	unavailable
Average	2897	90	4380	136

Note that the above values are for outbound truck movements only. For every outbound laden truck there is a corresponding inbound un-laden truck movement.

In reviewing the data in Table 2-8 related to the current and historic operation of the quarry (2013/2014), it is noted that there can be significant variation in demands, based upon the end market demands and weather conditions i.e. reduced or no operations within the quarry during periods of prolonged heavy rain. March 2014 was an absolute peak month for the quarry operations which coincided with peak demands at a number of major projects in the Lower Hunter. There was an average daily outbound flow of 6,720 tonnes corresponding to 207 outbound truck movements per day. The absolute peak demand occurred on Monday 17th of March 2014 with 9,449 tonnes which equates to 291 outbound truck movements, giving 582 two-way truck movements on that day. During the week commencing Monday 17th March, on average 8212 tonnes of material was supplied per day over the full working week, some 22% higher than the average per week for that month and 87% higher than the overall average weekly flow for 2014.

Overall this data for the two years 2013/2014 shows that:

- Average number of daily truck movements 111 inbound, 111 outbound
- 85th percentile of daily truck movements 159 inbound, 159 outbound
- Actual peak of daily truck movements 291 inbound, 291 outbound
- Theoretical daily peak of truck movements 400 inbound, 400 outbound (based on 10 hour working day)

The data provided for the quarry for 2013 and 2014 also shows the material demands for a Saturday morning, when the quarry works a half day. The Saturday average for 2013 was 765 tonnes which equates to 24 laden trucks whilst for 2014 the Saturday average was 997 tonnes which equates to 31 laden trucks.

2.5.6 Heavy Vehicle Flows

The traffic data collected by Seca Solution included a break down in vehicle classification as per the Austroads Guidelines. The traffic data shows that north of the town of Paterson the truck numbers as a percentage of the total flows is higher, but then as the analysis moves south along the route, the truck numbers remain near constant but the overall volume of traffic increases and hence the number of trucks as a percentage of the overall flow decreases. A summary of the heavy vehicle flows is given below.

Table 2-9 – Summary of the heavy vehicle flows along the transport routes from tube counters

Location	Total daily flow	Heavy vehicle daily flows	Heavy vehicle - percentage of daily flows
Dungog Road, midway between Grace Ave and Gresford Road	1,445	113	7.8%
Gresford Road north of Paterson	3,078	182	5.9%
Tocal Road, midway between Paterson and Bolwarra Heights	3,616	209	5.8%
Paterson Road at Bolwarra	11,688	386	3.3%
Flat Road midway between Paterson Road and Melbourne Street	8,659	268	3.1%
Melbourne Street	9,007	405	4.5%
Butterwick Road midway between Tocal Road and Clarence Town Road	1,423	51	3.6%
Clarence Town Road, midway between Butterwick Road and Brandy Hill	3,389	122	3.6%
Brandy Hill Drive	1,816	91	5.0%
Seaham Road, midway between Brandy Hill Drive and Port Stephens Street	6,623	265	4.0%

Heavy vehicles shown above are vehicles from medium rigid upwards, classes 3 to 12 inclusive as per Austroads Vehicle Classifications.

A closer examination of the survey results show that the percentage of heavy vehicles is greater in the morning and gradually decreases through the working day and from 5.00 PM the percentage of heavy vehicles drop off significantly.

2.5.7 Current Road Network Operation

Observations on site during the peak periods show that the road network, along the major transport routes associated with the Martins Creek quarry operations, all operate well with minimal delays and congestion, with the exception of the length of Melbourne Street on the approach to the New England Highway. Observations show that in the morning peak in particular, the queue back from the traffic lights at the New England Highway and Melbourne Street extends back beyond the traffic signals at Pitnacree Road / Lawes Street which creates significant delays and congestion for traffic on Melbourne Street. This is created by the high volume of traffic using the New England Highway in this location. Whilst the Hunter Expressway has provided some relief along this road corridor, the local demands along the New England Highway in this location remain high, hence the delays and queues created at this signal controlled intersection.

Other delays occur on the current haulage route via Belmore Road, Lorn. This is created by the traffic signals within Maitland at High Street as well as the more intense residential development in this area. As part of the approval process for the expansion of Martins Creek quarry, the operator of the quarry has stated that future truck movements will be directed to use the route via Flat Road and Melbourne Street to access the New England Highway and will not continue to use Belmore Road through Lorn. This route through Lorn will however be used to service the local markets in Maitland and its environs as required.

2.6 Traffic Safety and Accident History

The length of the two major haulage routes for the quarry operations have been reviewed as part of the project under the guidelines for the preparation of Road Safety Audits published by Austroads. The RMS has also provided accident data along the full length of the two haul routes over the last 5 years, which provides details on the accidents types as well as the type of vehicles involved.

Advice from the study team indicates that there have been no recorded accidents along the two major haulage routes associated with trucks working for the existing Martins Creek quarry.

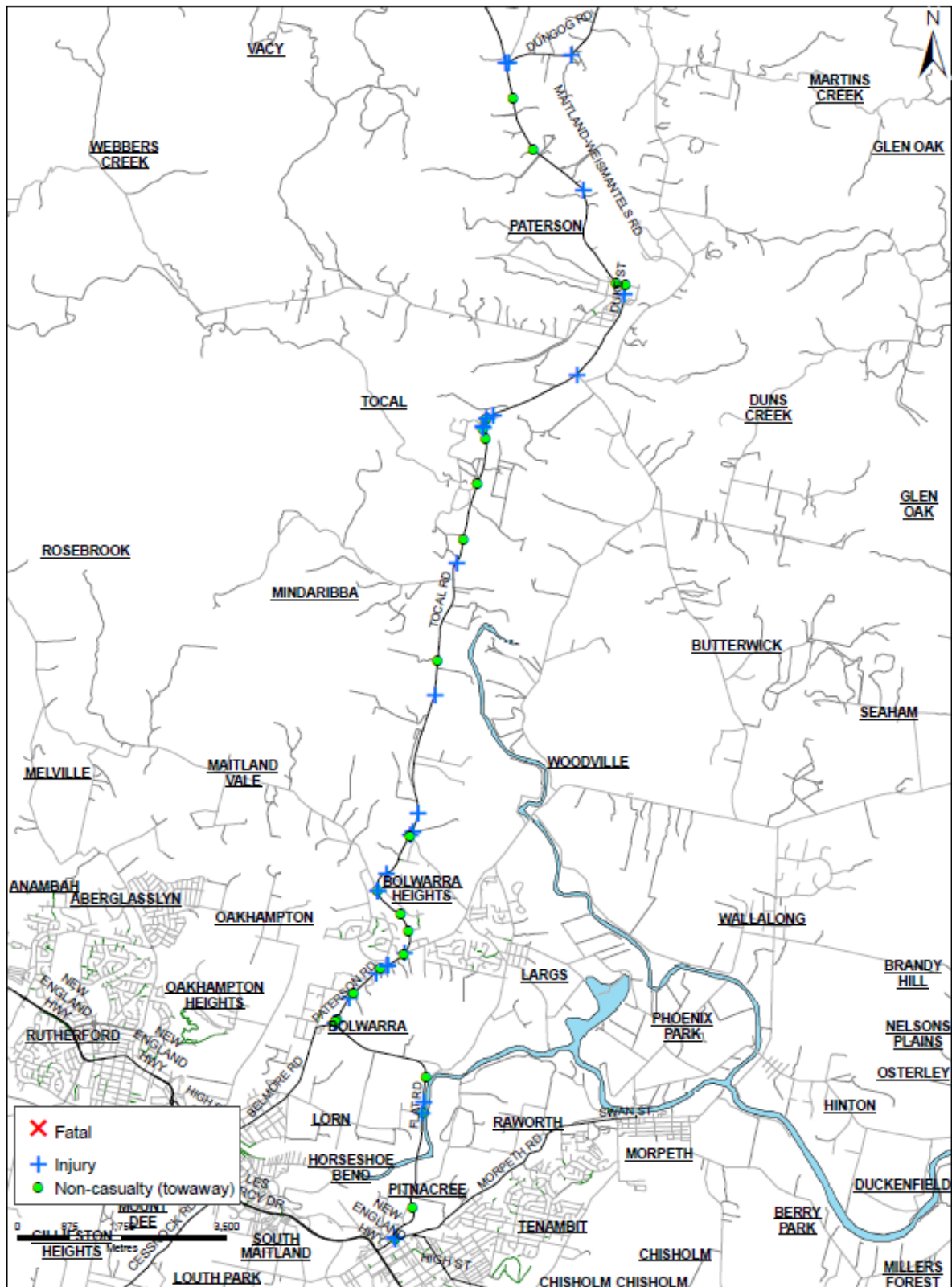
A summary of the accident data is provided below.

Martins Creek Quarry to East Maitland via Bolwarra Heights

Crash period 01/01/2010 to 31/12/2014



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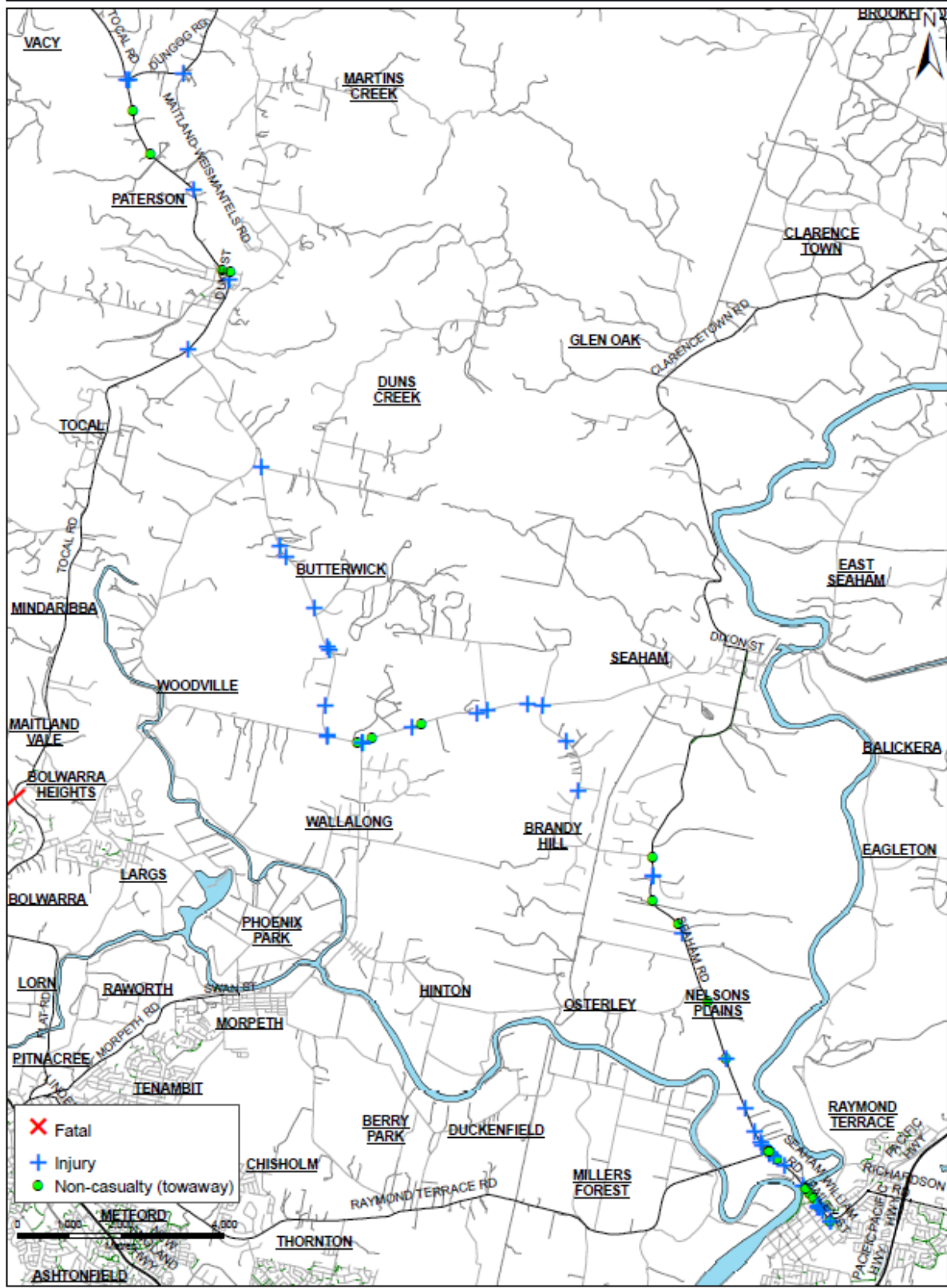
Summary Crash Report									
# Crash Type		Contributing Factors		Crash Movement		CRASHES 54		CASUALTIES 39	
Car Crash	45 83.3%	Speeding	16 29.6%	Intersection, adjacent approaches	5 9.3%	Fatal	0 0.0%	Killed	0 0.0%
Light Truck Crash	9 16.7%	Fatigue	5 9.3%	Head-on (not overtaking)	5 9.3%	Serious inj.	8 14.8%	Seriously inj.	12 30.8%
Rigid Truck Crash	1 1.9%			Opposing vehicles; turning	4 7.4%	Moderate inj.	12 22.2%	Moderately inj.	16 41.0%
Articulated Truck Crash	2 3.7%			U-turn	1 1.9%	Minor/Other inj.	6 11.1%	Minor/Other inj.	8 20.5%
"Heavy Truck Crash	(3) (5.6%)			Rear-end	10 18.5%	Uncategorised inj.	3 5.6%	Uncategorised inj.	3 7.7%
Bus Crash	0 0.0%			Lane change	0 0.0%	Non-casualty	25 46.3%	Unrestrained	0 0.0%
"Heavy Vehicle Crash	(3) (5.6%)			Parallel lanes; turning	0 0.0%				
Emergency Vehicle Crash	0 0.0%			Vehicle leaving driveway	1 1.9%				
Motorcycle Crash	2 3.7%			Overtaking; same direction	1 1.9%				
Pedal Cycle Crash	0 0.0%			Hit parked vehicle	0 0.0%				
Pedestrian Crash	1 1.9%			Hit railway train	0 0.0%				
* Rigid or Artic. Truck * Heavy Truck or Heavy Bus				Hit pedestrian	1 1.9%				
# These categories are NOT mutually exclusive				Permanent obstruction on road	0 0.0%				
Location Type				Hit animal	0 0.0%				
"Intersection	22 40.7%			Off road, on straight	2 3.7%				
Non intersection	32 59.3%			Off road on straight, hit object	3 5.6%				
* Up to 10 metres from an intersection				Out of control on straight	1 1.9%				
Collision Type				Off road, on curve	3 5.6%				
Single Vehicle	23 42.6%			Off road on curve, hit object	14 25.9%				
Multi Vehicle	31 57.4%			Out of control on curve	0 0.0%				
				Other crash type	3 5.6%				
Road Classification									
Freeway/Motorway	0 0.0%								
State Highway	0 0.0%								
Other Classified Road	47 87.0%								
Unclassified Road	7 13.0%								
~ 07:30-09:30 or 14:30-17:00 on school days									
~ 40km/h or less									
~ School Travel Time Involvement									
Day of the Week									
Monday	6 11.1%	Wednesday	6 11.1%	Friday	11 20.4%	Sunday	5 9.3%	WEEKEND	10 18.5%
Tuesday	7 13.0%	Thursday	14 25.9%	Saturday	5 9.3%	WEEKDAY	44 81.5%		
#Holiday Periods									
New Year	0 0.0%	Easter	0 0.0%	Queen's BD	1 1.9%	Christmas	1 1.9%	Easter SH	0 0.0%
Aust. Day	1 1.9%	Anzac Day	0 0.0%	Labour Day	1 1.9%	January SH	3 5.6%	June/July SH	2 3.7%
								Sept./Oct. SH	1 1.9%
								December SH	1 1.9%
Crashid dataset Martins Creek Quarry to East Maitland via Bolwarra Heights - crash data from 01/01/2010 to 31/12/2014									
Note: Crash self reporting, including self reported injuries began in Oct 2014. Trends from 2014 are expected to vary from previous years. More unknowns are expected in self reported data.									
Reporting years 2014 onwards contain uncategorised injury crashes.									
Percentages are percentages of all crashes. Unknown values for each category are not shown on this report.									
<div> <div>Rep ID: REG01</div> <div>Office: Hunter</div> <div>User ID: hampsona</div> <div>Page 1 of 1</div> <div>Generated: 20/10/2015 12:25</div> </div>									


Martins Creek Quarry to Raymond Terrace via Brandy Hill

Crash period 01/01/2010 to 31/12/2014



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Transport for NSW

Centre for Road Safety

Summary Crash Report

<div># Crash Type</div> <table> <tr><td>Car Crash</td><td>71</td><td>89.9%</td></tr> <tr><td>Light Truck Crash</td><td>18</td><td>22.8%</td></tr> <tr><td>Rigid Truck Crash</td><td>1</td><td>1.3%</td></tr> <tr><td>Articulated Truck Crash</td><td>0</td><td>0.0%</td></tr> <tr><td>*Heavy Truck Crash</td><td>(1)</td><td>(1.3%)</td></tr> <tr><td>Bus Crash</td><td>0</td><td>0.0%</td></tr> <tr><td>"Heavy Vehicle Crash</td><td>(1)</td><td>(1.3%)</td></tr> <tr><td>Emergency Vehicle Crash</td><td>0</td><td>0.0%</td></tr> <tr><td>Motorcycle Crash</td><td>7</td><td>8.9%</td></tr> <tr><td>Pedal Cycle Crash</td><td>1</td><td>1.3%</td></tr> <tr><td>Pedestrian Crash</td><td>2</td><td>2.5%</td></tr> </table> <div> <div>* Rigid or Artic. Truck * Heavy Truck or Heavy Bus</div> <div># These categories are NOT mutually exclusive</div> </div>	Car Crash	71	89.9%	Light Truck Crash	18	22.8%	Rigid Truck Crash	1	1.3%	Articulated Truck Crash	0	0.0%	*Heavy Truck Crash	(1)	(1.3%)	Bus Crash	0	0.0%	"Heavy Vehicle Crash	(1)	(1.3%)	Emergency Vehicle Crash	0	0.0%	Motorcycle Crash	7	8.9%	Pedal Cycle Crash	1	1.3%	Pedestrian Crash	2	2.5%	<div>Contributing Factors</div> <table> <tr><td>Speeding</td><td>14</td><td>17.7%</td></tr> <tr><td>Fatigue</td><td>12</td><td>15.2%</td></tr> </table> <div>Weather</div> <table> <tr><td>Fine</td><td>60</td><td>75.9%</td></tr> <tr><td>Rain</td><td>11</td><td>13.9%</td></tr> <tr><td>Overcast</td><td>6</td><td>7.6%</td></tr> <tr><td>Fog or mist</td><td>2</td><td>2.5%</td></tr> <tr><td>Other</td><td>0</td><td>0.0%</td></tr> </table> <div>Road Surface Condition</div> <table> <tr><td>Wet</td><td>14</td><td>17.7%</td></tr> <tr><td>Dry</td><td>65</td><td>82.3%</td></tr> <tr><td>Snow or ice</td><td>0</td><td>0.0%</td></tr> </table> <div>Natural Lighting</div> <table> <tr><td>Dawn</td><td>5</td><td>6.3%</td></tr> <tr><td>Daylight</td><td>48</td><td>60.8%</td></tr> <tr><td>Dusk</td><td>2</td><td>2.5%</td></tr> <tr><td>Darkness</td><td>24</td><td>30.4%</td></tr> </table>	Speeding	14	17.7%	Fatigue	12	15.2%	Fine	60	75.9%	Rain	11	13.9%	Overcast	6	7.6%	Fog or mist	2	2.5%	Other	0	0.0%	Wet	14	17.7%	Dry	65	82.3%	Snow or ice	0	0.0%	Dawn	5	6.3%	Daylight	48	60.8%	Dusk	2	2.5%	Darkness	24	30.4%	<div>Crash Movement</div> <table> <tr><td>Intersection, adjacent approaches</td><td>12</td><td>15.2%</td></tr> <tr><td>Head-on (not overtaking)</td><td>2</td><td>2.5%</td></tr> <tr><td>Opposing vehicles; turning</td><td>10</td><td>12.7%</td></tr> <tr><td>U-turn</td><td>4</td><td>5.1%</td></tr> <tr><td>Rear-end</td><td>17</td><td>21.5%</td></tr> <tr><td>Lane change</td><td>0</td><td>0.0%</td></tr> <tr><td>Parallel lanes; turning</td><td>0</td><td>0.0%</td></tr> <tr><td>Vehicle leaving driveway</td><td>1</td><td>1.3%</td></tr> <tr><td>Overtaking; same direction</td><td>0</td><td>0.0%</td></tr> <tr><td>Hit parked vehicle</td><td>0</td><td>0.0%</td></tr> <tr><td>Hit railway train</td><td>0</td><td>0.0%</td></tr> <tr><td>Hit pedestrian</td><td>2</td><td>2.5%</td></tr> <tr><td>Permanent obstruction on road</td><td>0</td><td>0.0%</td></tr> <tr><td>Hit animal</td><td>3</td><td>3.8%</td></tr> <tr><td>Off road, on straight</td><td>2</td><td>2.5%</td></tr> <tr><td>Off road on straight, hit object</td><td>8</td><td>10.1%</td></tr> <tr><td>Out of control on straight</td><td>1</td><td>1.3%</td></tr> <tr><td>Off road, on curve</td><td>2</td><td>2.5%</td></tr> <tr><td>Off road on curve, hit object</td><td>7</td><td>8.9%</td></tr> <tr><td>Out of control on curve</td><td>1</td><td>1.3%</td></tr> <tr><td>Other crash type</td><td>7</td><td>8.9%</td></tr> </table>	Intersection, adjacent approaches	12	15.2%	Head-on (not overtaking)	2	2.5%	Opposing vehicles; turning	10	12.7%	U-turn	4	5.1%	Rear-end	17	21.5%	Lane change	0	0.0%	Parallel lanes; turning	0	0.0%	Vehicle leaving driveway	1	1.3%	Overtaking; same direction	0	0.0%	Hit parked vehicle	0	0.0%	Hit railway train	0	0.0%	Hit pedestrian	2	2.5%	Permanent obstruction on road	0	0.0%	Hit animal	3	3.8%	Off road, on straight	2	2.5%	Off road on straight, hit object	8	10.1%	Out of control on straight	1	1.3%	Off road, on curve	2	2.5%	Off road on curve, hit object	7	8.9%	Out of control on curve	1	1.3%	Other crash type	7	8.9%	<div>CRASHES 79</div> <table> <tr><td>Fatal</td><td>0</td><td>0.0%</td></tr> <tr><td>Serious inj.</td><td>17</td><td>21.5%</td></tr> <tr><td>Moderate inj.</td><td>19</td><td>24.1%</td></tr> <tr><td>Minor/Other inj.</td><td>6</td><td>7.6%</td></tr> <tr><td>Uncategorised inj.</td><td>9</td><td>11.4%</td></tr> <tr><td>Non-casualty</td><td>28</td><td>35.4%</td></tr> </table> <div>Self Reported Crash 1 1.27%</div> <div>Time Group % of Day</div> <table> <tr><td>00:01 - 02:59</td><td>1</td><td>1.3%</td><td>12.5%</td></tr> <tr><td>03:00 - 04:59</td><td>1</td><td>1.3%</td><td>8.3%</td></tr> <tr><td>05:00 - 05:59</td><td>1</td><td>1.3%</td><td>4.2%</td></tr> <tr><td>06:00 - 06:59</td><td>4</td><td>5.1%</td><td>4.2%</td></tr> <tr><td>07:00 - 07:59</td><td>2</td><td>2.5%</td><td>4.2%</td></tr> <tr><td>08:00 - 08:59</td><td>4</td><td>5.1%</td><td>4.2%</td></tr> <tr><td>09:00 - 09:59</td><td>5</td><td>6.3%</td><td>4.2%</td></tr> <tr><td>10:00 - 10:59</td><td>4</td><td>5.1%</td><td>4.2%</td></tr> <tr><td>11:00 - 11:59</td><td>3</td><td>3.8%</td><td>4.2%</td></tr> <tr><td>12:00 - 12:59</td><td>4</td><td>5.1%</td><td>4.2%</td></tr> <tr><td>13:00 - 13:59</td><td>7</td><td>8.9%</td><td>4.2%</td></tr> <tr><td>14:00 - 14:59</td><td>2</td><td>2.5%</td><td>4.2%</td></tr> <tr><td>15:00 - 15:59</td><td>7</td><td>8.9%</td><td>4.2%</td></tr> <tr><td>16:00 - 16:59</td><td>4</td><td>5.1%</td><td>4.2%</td></tr> <tr><td>17:00 - 17:59</td><td>7</td><td>8.9%</td><td>4.2%</td></tr> <tr><td>18:00 - 18:59</td><td>7</td><td>8.9%</td><td>4.2%</td></tr> <tr><td>19:00 - 19:59</td><td>6</td><td>7.6%</td><td>4.2%</td></tr> <tr><td>20:00 - 21:59</td><td>4</td><td>5.1%</td><td>8.3%</td></tr> <tr><td>22:00 - 24:00</td><td>6</td><td>7.6%</td><td>8.3%</td></tr> </table> <div>Street Lighting Off/Nil % of Dark</div> <table> <tr><td>12</td><td>of</td><td>24</td><td>in Dark</td><td>50.0%</td></tr> </table>	Fatal	0	0.0%	Serious inj.	17	21.5%	Moderate inj.	19	24.1%	Minor/Other inj.	6	7.6%	Uncategorised inj.	9	11.4%	Non-casualty	28	35.4%	00:01 - 02:59	1	1.3%	12.5%	03:00 - 04:59	1	1.3%	8.3%	05:00 - 05:59	1	1.3%	4.2%	06:00 - 06:59	4	5.1%	4.2%	07:00 - 07:59	2	2.5%	4.2%	08:00 - 08:59	4	5.1%	4.2%	09:00 - 09:59	5	6.3%	4.2%	10:00 - 10:59	4	5.1%	4.2%	11:00 - 11:59	3	3.8%	4.2%	12:00 - 12:59	4	5.1%	4.2%	13:00 - 13:59	7	8.9%	4.2%	14:00 - 14:59	2	2.5%	4.2%	15:00 - 15:59	7	8.9%	4.2%	16:00 - 16:59	4	5.1%	4.2%	17:00 - 17:59	7	8.9%	4.2%	18:00 - 18:59	7	8.9%	4.2%	19:00 - 19:59	6	7.6%	4.2%	20:00 - 21:59	4	5.1%	8.3%	22:00 - 24:00	6	7.6%	8.3%	12	of	24	in Dark	50.0%	<div>CASUALTIES 75</div> <table> <tr><td>Killed</td><td>0</td><td>0.0%</td></tr> <tr><td>Seriously inj.</td><td>20</td><td>26.7%</td></tr> <tr><td>Moderately inj.</td><td>32</td><td>42.7%</td></tr> <tr><td>Minor/Other inj.</td><td>9</td><td>12.0%</td></tr> <tr><td>Uncategorised inj.</td><td>14</td><td>18.7%</td></tr> <tr><td>^ Unrestrained</td><td>0</td><td>0.0%</td></tr> </table> <div>^ Belt fitted but not worn, No restraint fitted to position OR No helmet worn</div> <div>Crashes Casualties</div> <table> <tr><td>11</td><td>2014</td><td>14</td></tr> <tr><td>16</td><td>2013</td><td>10</td></tr> <tr><td>14</td><td>2012</td><td>9</td></tr> <tr><td>16</td><td>2011</td><td>22</td></tr> <tr><td>22</td><td>2010</td><td>20</td></tr> </table> <div>McLean Periods % Week</div> <table> <tr><td>A</td><td>9</td><td>11.4%</td><td>17.9%</td></tr> <tr><td>B</td><td>3</td><td>3.8%</td><td>7.1%</td></tr> <tr><td>C</td><td>18</td><td>22.8%</td><td>17.9%</td></tr> <tr><td>D</td><td>3</td><td>3.8%</td><td>3.5%</td></tr> <tr><td>E</td><td>4</td><td>5.1%</td><td>3.6%</td></tr> <tr><td>F</td><td>17</td><td>21.5%</td><td>10.7%</td></tr> <tr><td>G</td><td>12</td><td>15.2%</td><td>7.1%</td></tr> <tr><td>H</td><td>4</td><td>5.1%</td><td>7.1%</td></tr> <tr><td>I</td><td>4</td><td>5.1%</td><td>12.5%</td></tr> <tr><td>J</td><td>5</td><td>6.3%</td><td>10.7%</td></tr> </table>	Killed	0	0.0%	Seriously inj.	20	26.7%	Moderately inj.	32	42.7%	Minor/Other inj.	9	12.0%	Uncategorised inj.	14	18.7%	^ Unrestrained	0	0.0%	11	2014	14	16	2013	10	14	2012	9	16	2011	22	22	2010	20	A	9	11.4%	17.9%	B	3	3.8%	7.1%	C	18	22.8%	17.9%	D	3	3.8%	3.5%	E	4	5.1%	3.6%	F	17	21.5%	10.7%	G	12	15.2%	7.1%	H	4	5.1%	7.1%	I	4	5.1%	12.5%	J	5	6.3%	10.7%
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From the accident data above, the following points are made:

- The vast majority of the accidents involved light vehicles and light trucks only, which excludes the size / type of vehicle associated with haulage of quarry material;
- Speed and fatigue represent over 30% of the cause of the accidents along the two routes;
- The majority of accidents involve rear end accidents, turning traffic and at the approach to intersections. These are typically created at driveways and intersections where there are no sheltered right turn lanes, to protect traffic waiting to turn right off the road.

The road safety review undertaken along the length of the two major haulage routes identified a number of safety concerns with the existing layout of intersections and roads. It is noted that these concerns do not have a nexus to the haulage vehicles associated with quarry use at Martins Creek (or Brandy Hill). These are existing safety issues on the road network that do not comply with current design standards. A summary of the safety concerns is provided below.

Table 2-10 – Summary of findings of Road Safety review along the two key haulage routes

Issue	Potential safety concern	Suggested upgrade	Comment
Lack of space between intersection of Station Street and railway crossing and road alignment across railway crossing	Could lead to driver confusion over traffic controls leading to blockage of railway crossing	1 - Remove railway crossing with grade separation 2 - Upgrade level crossing and alignment	ARTC has prepared plan for upgrade but no timeframe for works.
One-way bridge operation on Dungog Road	Could lead to head on collisions due to drivers not giving way	1 – Widen bridge 2 – Provide new wider bridge	RMS has stated that the current bridge can continue to operate as one-way.
Lack of sheltered right turn lane on Gresford Road for drivers turning right into Dungog Road	Could lead to rear end type crashes	Upgrade intersection to provide a dedicated sheltered right turn lane	Existing traffic demands warrant this road upgrade
Tight road alignment on 90-degree bend at Gresford Road / Duke Street in Paterson	Could lead to drivers cutting the corner and resulting in head on collisions	Provide physical guidance for vehicles to reinforce traffic manoeuvre around the bend	
Lack of pavement width on Tocal Road at Bolwarra Heights	Could lead to safety concerns for cyclists and potential issue for vehicles stopping	1 - Widen road pavement or 2 – Provide an off road footway / cycleway Provide No Stopping signage along length of the road	Council has recently completed works in this section to improve delineation
Lack of shoulders along Butterwick Road	No run off area or space for cyclists	Provide shoulder widening in both directions along affected length	Existing traffic volumes require this road upgrade
Lack of sheltered right turn lane on Clarence Town Road for drivers turning right into Butterwick Road	Could lead to rear end type accidents	Upgrade intersection to provide a dedicated sheltered right turn lane	Existing traffic demands warrant this road upgrade

Issue	Potential safety concern	Suggested upgrade	Comment
Lack of sheltered right turn lane on Clarence Town Road for drivers turning right into Brandy Hill Drive	Could lead to rear end type accidents	Upgrade intersection to provide a dedicated sheltered right turn lane	Existing traffic demands warrant this road upgrade. May require upgrade due to Brandy Hill quarry

Overall, it is considered that the road network is generally satisfactory for road safety issues, with the major concerns being the lack of sheltered right turn lanes at the key intersections identified above.

2.7 Parking Supply and Demand

2.7.1 On-street Parking Provision

Currently, vehicles can be parked on both sides of the local roads in the vicinity of the site. However, on-site observations indicate that current on street demands are negligible with the majority of local residents having off street parking available.

2.7.2 Off-Street Parking Provision

There is adequate off-street parking in the general locality of the subject site to satisfy the local demand.

2.7.3 Parking Demand and Utilisation

There was no on-street parking noted in the general vicinity of the site. All parking was accommodated within the private properties.

2.7.4 Short term Set down or pick up areas

There are no set down or pick up areas in the locality of the site.

2.8 Public Transport

2.8.1 Rail Station Locations

Martins Creek railway station is less than 500m from the subject site. It is located on the Hunter railway line providing a link to Newcastle and beyond through to Sydney to the south and to Dungog to the north. Maitland station, located on the Hunter line 27 kms to the south of the site provides access to services to the west towards Singleton and beyond.

2.8.2 Bus Routes and Associated Facilities

Local bus services are limited. Linq Buslines provides a local school run for Martins Creek public school, as well as for students travelling to Dungog High School. Shelton Services of Dungog operate local school bus runs and connect with Busways and Hunter Valley Buses.

There are no bus stops within the general locality of the subject site.

2.8.3 Rail and Bus Service Frequencies

There are five northbound rail services and five southbound rail services daily, Monday to Friday, with less frequent services of a weekend.

Northbound	Southbound
5.25	7.11
9.25	10.33
11.25	12.49
16.21	17.24
18.25	19.49

The local school bus services only operate on school days.

2.9 Pedestrian Network

There are very limited pedestrian facilities within the general locality of the subject site, reflective of the rural setting and the combination of low traffic flows and low pedestrian demands.

Within the major urban centres e.g. Paterson, Bolwarra Heights, Lorn, Maitland and Raymond Terrace footpaths are provided which allow for safe and appropriate pedestrian movements.

2.10 Other Proposed Developments

The other major development proposed within the general locality of the subject site is the expansion of the Brandy Hill Quarry. Brandy Hill Quarry is proposing to expand their operations to allow for the extraction of 1.5 million tonnes of hard rock per year for 30 years, which will allow for 24 hour operations, sales and despatch. The Environmental Impact Assessment (EIS) for the project has been produced to assess the development, the SEARs have been issued and addressed and the project has been submitted to the Department of Planning for a test of adequacy prior to the formal submission of the EIS.

Further information on this project with regard to routes for trucks, split of demand for the end products or details on truck movements generated by this project has not been made available by Hansen to the study team for Martins Creek.

3 Proposed Development

3.1 The Development

The project seeks to continue existing operations to complete the extraction of material in existing areas in conjunction with expansion into the proposed new areas to maximise the utilisation of the resource. Mining methods are expected to remain the same as currently used with rock being broken by Drill and Blast techniques in the pit with Run of Mine (ROM) material being trucked to the crushing plant for further processing before being stockpiled and loaded on to road trucks for delivery to market.

The components of the development comprise:

- extracting up to 1.5 million tonnes of hard rock material per annum;
- expanding into new extraction areas and clearing of vegetation;
- increasing the hours of operation,
 - for quarrying to 6am-6pm (Monday to Saturday)
 - processing to 6am -10pm (Monday to Saturday)
 - mixing and binding to 4.30am – 10pm (Monday to Friday) and 4.30-6pm (Saturdays),
 - train loading retained at 24 hours per day, 7 days per week,
 - maintenance works retained at 24 hours per day, 7 days per week,
- consolidating existing operations and approvals; and
- rehabilitating the site

3.1.1 Projected number of employees

The proposal may generate the requirement for additional staffing levels which will increase over time as the output of material increases. The extent of additional staffing levels is not know at this time.

3.1.2 Hours and days of operations

A summary of the operational hours for Martins Creek quarry is provided below increasing the hours of operation to:

- for quarrying to 6am-6pm (Monday to Saturday)
- processing to 6am -10pm (Monday to Saturday)
- mixing and binding to 4.30am – 10pm (Monday to Friday) and 4.30-6pm (Saturdays),
- train loading retained at 24 hours per day, 7 days per week,
- maintenance works retained at 24 hours per day, 7 days per week,

3.1.3 Phasing and Timing

The proposed expansion of the quarry to the full annual output of 1.5 million tonnes will be driven by market demand. The timing and phasing of the expansion will therefore be reliant upon market forces.

3.1.4 Selection of appropriate design vehicles for access and circulation requirements

The development will generally need to accommodate both light vehicles and heavy vehicles with the largest vehicle being a truck and dog combination. Light vehicles will include staff vehicles as well as small trucks

associated with general maintenance for the works on site. The majority of the current material hauled by road from the site utilise a truck and dog combination. Other product is moved using semi-trailers.

Daracon also have need for the occasional larger vehicle, including low loader floats for moving heavy machinery on and off site as required.

3.2 Access

3.2.1 Driveway Location

All vehicle access to the subject site will be via the existing access on Station Street. Station Street then connects with Grace Avenue to allow for connection to the greater road network. The existing access on Station Street has been the single vehicle access to the quarry for the majority of the life of the quarry and allows for safe and appropriate two-way traffic movements.

As part of the on-going development of the quarry, a new access option has been identified that will allow for direct vehicle access between the quarry and Dungog Road, to the north of Grace Avenue which involves a bridge crossing over the railway line. Once this new access has been constructed, all heavy vehicle movements will be via this new access. This new road access will be designed and constructed in accordance with Council standards and will allow for a sheltered right turn lane on Dungog Road to ensure that the new access can operate in a safe and appropriate manner. The bridge works will be designed and constructed in accordance with ARTC standards.

When this new access is constructed, the existing access on Station Street will remain as per the current layout, allowing for emergency access as well as light vehicle movements in and out of the site. This current layout is sufficient to continue to allow for this use.

3.2.2 Sight Distances

The connection of the current site access to the road network is via the intersection of Grace Avenue and Station Street. This intersection is a simple T-intersection with Stop sign control with the roads connecting at 90 degrees. Visibility is good in both directions, especially when taking into account the raised seating position for drivers of trucks. Visibility to the right for drivers exiting Station Street is impacted upon by the vertical alignment of the road over the level crossing, but there remains adequate distance for drivers to determine a suitable gap for exiting Station Street.

For the posted speed limit of 50 km/h the sight distance requirement is 80 metres. This distance has been assessed on site and exceeds 100 metres in both directions. It is noted that whilst the posted speed limit is 50 km/h in this location, the actual vehicle speeds are lower due to the interaction with the at-grade train level crossing to the immediate west of the intersection.



Figure 3-1 – Aerial photo showing intersection of Station Street and Grace Avenue



Photo 1 – View west along Cory Street on approach to level crossing. Note Station Street is to right of photo

3.2.3 Service Vehicle Access

The proposed expansion of the quarry may generate additional servicing requirements for plant and vehicles with all maintenance carried out on site. Typical service vehicles include light to medium trucks and these vehicles can all be accommodated on site as appropriate. Currently, all of these vehicles gain access via Station Street but in the future will be able to gain access via the new connection direct onto Dungog Road. The current site access allows for access for all vehicles including the truck and dog combinations and thus cater for the swept path movement associated with service vehicles.

The new access on Dungog Road will be designed in accordance with Council requirements and will be designed to accommodate the normal vehicle access requirements for the project, which will include the service vehicles.

3.2.4 Queuing at entrances

Given the overall traffic flows along Grace Avenue, it is considered that there will be minimal queuing associated with the traffic movements in and out of the subject site. Observations of the current operation show that there is very limited delays for trucks accessing the site, with the major delays occurring when trains are travelling through the location causing hold ups across the at-grade level crossing. However, there is a limited train service in this location and accordingly the delays are minimal.

With the construction of the future access direct onto Dungog Road, the delays at the existing site access at Station Street will be significantly reduced. The new access on Dungog Road will allow for a safe and appropriate connection and will provide a sheltered right turn lane for traffic entering the site. The delays for the through traffic movements will be minimal and any delay for exiting traffic will result in a queue located within the site.

3.2.5 Current access compared with proposed access

Vehicle access to the subject site will be the same as the existing access to the site, via Station Street.

Access will be improved with the construction of new access road and connection to Dungog Road. At this stage, all heavy vehicles will be able to access the site via the new access and will not need to use Station Street. The Station Street access will remain as per the current layout and will allow for heavy vehicle access if required.

3.2.6 Access to Public Transport

There will be no need for public transport to access the site.

3.3 Circulation

3.3.1 Pattern of circulation

The size of the site and the layout of the internal roads allows for vehicles to enter and exit the site in a forward direction.

3.3.2 Internal Road width

The width of the internal roads is in accordance with normal design standards for quarries and allow for two way traffic movements as appropriate. All internal traffic movements are governed by mining rules and regulations and Daracon have a drivers' code of conduct to manage internal traffic movements. There is no public access within the quarry site.

3.3.3 Internal Bus Movements

It is considered that there will be neither internal bus movements nor a requirement for a bus to travel within the development site.

3.3.4 Service Area Layout

Servicing is completed across the site as required adjacent to the various equipment and plant. Vehicles associated with maintenance and servicing park within the site adjacent to the plant as required. A separate maintenance shed is provided that allows for vehicle maintenance.

3.4 Parking

3.4.1 Proposed Supply

All parking for this proposed development will be contained within the site.

Due to the unique nature of the proposed expanded project it is not covered by Council's parking code. The existing parking provision will be retained and expanded on site to meet the potential future expansion of the staffing needs.

3.4.2 Parking provision per State Government policy

No specific provision required.

3.4.3 Council code and local parking policies and plans

No specific provision required.

3.4.4 Parking Layout

The site layout will allow for the safe parking of vehicles within the site. This will include a stand over area for the trucks parked on site overnight. These parking areas are typically informal and located within the quarry footprint as appropriate and will alter as the quarry expands.

3.4.5 Projected peak demand

Peak parking demand will be reliant upon staffing levels. The site area allows for all staff parking to be provided on site. The site allows for trucks to be stored on site over-night and staff arrive and depart from the site in their own vehicles.

3.4.6 Service Vehicle Parking

The site area allows for all service vehicle parking to be provided on site as required.

3.4.7 Pedestrian and Bicycle Facilities

There are no specific pedestrian and bicycle facilities proposed.

4 Transportation Analysis

4.1 Traffic Generation

The quarry expansion is proposed to allow for a maximum annual allowance of 1.5 million tonnes. Currently it is proposed that the majority of the material will be removed from the site via road haulage, due to the operation restrictions associated with the use of the rail siding and rail network. Use of the railway sidings will continue at similar levels to the current use, which is in the order of 50,000 tonnes per annum.

Based upon the typical truck and dog combination used at the quarry carrying 32.5 tonnes per load, the future expansion to 1.5 million tonnes could generate:

- 46,153 laden truck movements per year;
- Corresponding 46,153 empty truck movements accessing the site.

Based upon the current operation, allowing 50 weeks per year and 5.5 days per week operation, this will generate 923 truckloads per week on average and 168 truckloads per day Monday to Friday. This gives a typical daily two-way rate of 336 trucks per day. Based on a half day for the Saturday the quarry could generate up to 84 truckloads on a Saturday, however it is noted that demands for major construction material is much lower on a Saturday morning and as such, the normal Saturday demands are much lower than 84 loads. Data from the Martins Creek weighbridge indicates that for year 2013 – 2014 the average Saturday tonnage was 765 which equates to less than 25 trucks.

Advice from the study team indicates that the operational characteristics of the quarry allow for a maximum sales throughput of 40 trucks loading and exiting the site per hour. This is consistent with the current hourly operations of the site however the total daily throughput is dependent upon the throughput capacity of the quarry as well as market demands. Market demands require that the majority of product be delivered early within the working day and as such must leave the quarry between 5.30-8am, after this the hourly truck movements reduce, dropping off significantly after 11am and further again at 3pm. The timeline for quarry product deliveries are shown below in Figure 4-1.

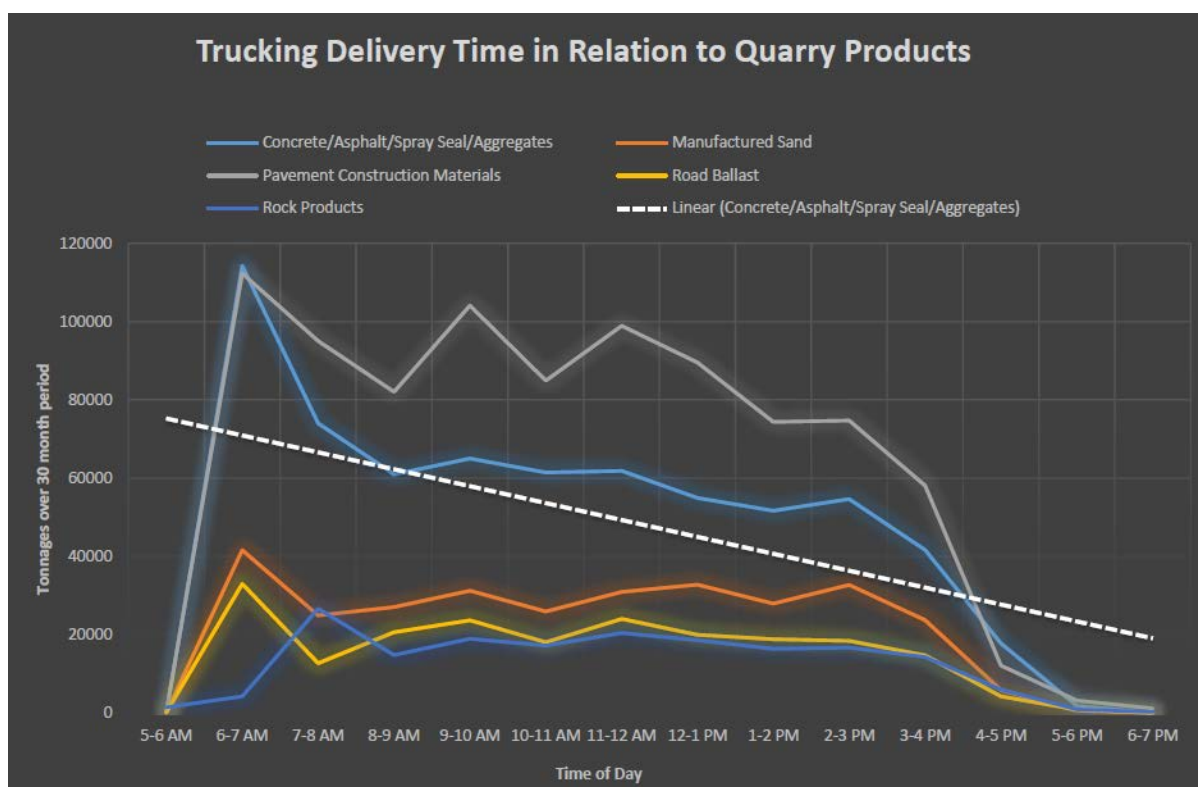


Figure 4-1 Truck movements associated with outbound quarry products (Source: Daracon)

The development could generate some 40 inbound and 40 outbound trucks per hour at its peak when the site is working at maximum capacity. Based on 8 hours for deliveries of outbound product demand per working day, as a worst case scenario this would equate to an absolute peak daily demand of 320 trucks entering and exiting the site per day, 640 two way truck movements. However it is important to note that this type of demand is dependent upon Daracon working concurrently on a number of major projects and all of these projects requiring quarry product all at the same time. A review of the weighbridge data shows that the absolute peak demand for 2013/2014 was 291 trucks on the 17th March 2014, which would have generated 582 two-way truck movements. For the following Monday (24th March) the weighbridge records show 179 truckloads were removed from the site whilst the average for the entire year for 2014 was 136 truckloads per day.

In reviewing data related to the current and historic operation of the quarry (2013/2014) overall:

- Average number of daily truck movements 111 inbound, 111 outbound
- 85th percentile of daily truck movements 159 inbound, 159 outbound
- Peak of daily truck movements 291 inbound, 291 outbound
- Theoretical daily peak of truck movements 320 inbound, 320 outbound

This fluctuation in deliveries by year over the last 12 years is shown below in Figure 4-2.

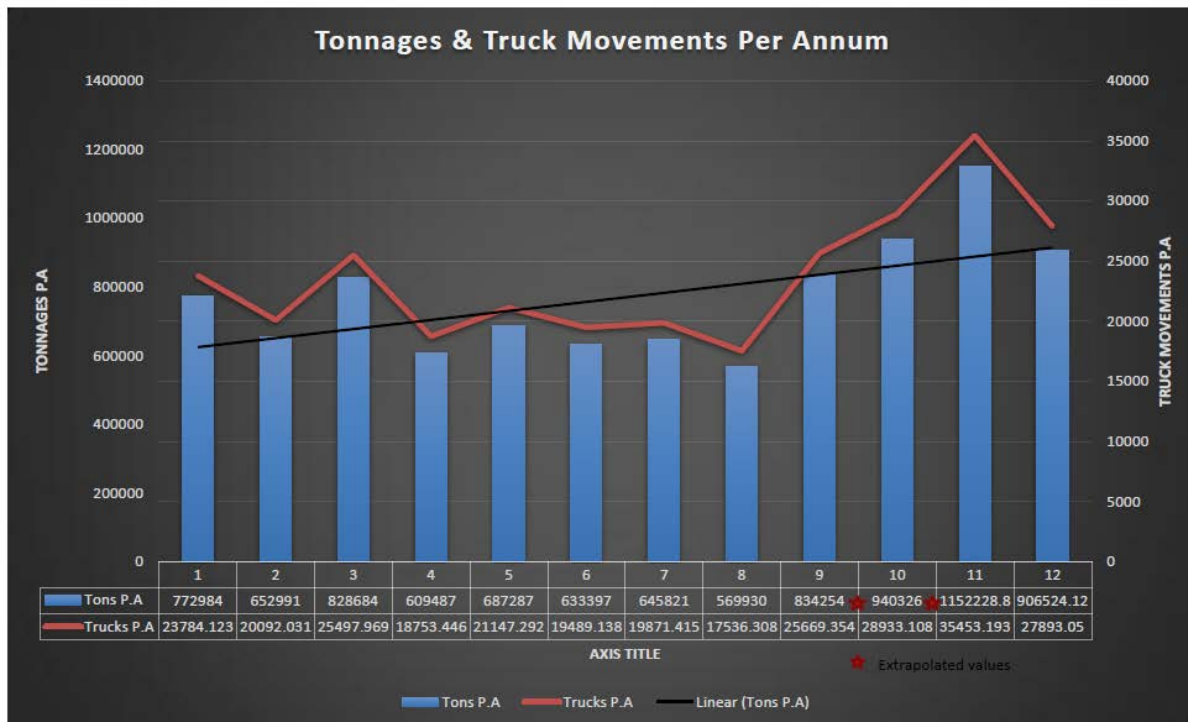


Figure 4-2 Demonstrating the fluctuations in tonnage and associated truck movement over last 12 years of operation (Source: Daracon)

In addition, there will be traffic movements associated with staff and maintenance vehicles, which will be similar to the existing operations on site.

4.1.1 Daily and Seasonal Factors

The nature of the development can lead to significant variations in daily traffic flows, with additional variation created by market demands. Daily flows will vary due to market demands as well as weather conditions. These are typically outside of the control of the quarry management.

The major impact upon the traffic flows generated by the development is created by the demands of the end market.

Daracon's external customer base includes, but is not limited to, government and major industry players including RMS, ARTC, Local Government, Sydney Trains, Lend Lease, Thiess, Leightons, Boral, Hanson, Holcim, Fulton Hogan and Metromix. Supply to these customers occurs for various reasons but mainly due to the quality and availability of aggregates and road pavement materials produced at Martins Creek that readily meet the rigorous requirements of State Government and associated geotechnical specifications. Whilst Martins Creek Quarry primarily produces aggregates, Daracon also focus on the design and manufacture of high quality road pavement materials, in particular Stabilbase (RMS Dense Graded Base) and Stabilstone (RMS Heavily Bound Base). For the past 15 years (approximately) Stabilstone has been one of only two products that meet RMS heavily bound specification. Heavily Bound Base is critical to road infrastructure for state and local government roads subjected to increased traffic volume and heavy loading. Recent changes to RMS specifications has seen a small increase of suppliers in the Heavily Bound Base (HBB) market, however due to some specific site requirements they do not always comply. The extensive experience of Daracon staff and a history of 15 years designing these materials means that Stabilstone readily meets all site and specification requirements. Both Stabilstone and Stabilbase produced at Martins Creek Quarry incorporate a percentage of flyash. Flyash is a by-product (waste) of the combustion of pulverised coal in thermal power plants, Martins Creek Quarry uses approximately 10-15kt per annum of flyash which is delivered to the Martins Creek quarry in 19.0 metre semi-trailer combinations.

The Lower Hunter Regional Strategy represents an agreed NSW government position on the future of the Lower Hunter, the current strategy applies to the period 2006 – 2031. The primary purpose of the regional strategy is to

ensure that adequate land is available and appropriately located to sustainably accommodate the projected housing and employment needs of the Region's population over the 25 year period. The strategy aims to provide for up to an additional 115 000 new dwellings ensuring that housing can accommodate the projected 160 000 additional people that is forecasted to populate the Region over the period. In 2011, the Government established the HIF to promote economic growth and enhance the liveability of the Hunter region. The Government initially committed \$350 million to the HIF over four years. Of this amount \$332 million has now been allocated to projects, including \$60 million towards the revitalisation of the Newcastle CBD.

NSW State Government has committed \$60 billion to rebuilding state infrastructure in the 2014-2015 budget. The budget has an allowance of \$43 million allocated to road infrastructure upgrades in the Hunter Region consisting of;

Hunter Roads – Major Projects	\$43 million (2014-15 financial year)	Planning and investment for works to address localised impact of mining related activity and population growth. A corridor study on Hillsborough Road, Warners Bay will commence in 2014-15. Some projects include Commonwealth Government funding contributions.
<ul style="list-style-type: none"> ▪ Cormorant Road, Industrial Drive to Stockton Bridge (planning) ▪ Nelson Bay Road, Bobs Farm to Anna Bay (Stage 3) ▪ Newcastle Inner City Bypass, Rankin Park to Jesmond (planning) ▪ New England Highway, Belford to Golden Highway duplication (planning) ▪ New England Highway, Gowrie Gates, widen Rail Underpass (planning) ▪ New England Highway, Singleton Bypass (planning) ▪ New England Highway, Scone Bypass and Rail Level Crossing removal (planning) ▪ New England Highway, Upgrade of Maitland roundabouts ▪ Pacific Motorway (M1) Extension to Raymond Terrace (planning) 		

This forecasted growth within the Hunter, and government commitment to upgrading infrastructure, will increase market need for hard rock quarry products in the region including from Martins Creek, to service the demand for building and construction materials.

Martins Creek Quarry is critical to the ongoing development and growth of the Hunter area and the Regional market. It is anticipated that it will have a significant place in the state market due to the rail siding and the limited number of hard rock quarries in NSW that are forecast to have major road and rail infrastructure upgrades completed. In particular, it is anticipated that the Pacific Highway Upgrade in Northern NSW will require quarry materials in excess of volume and quality available locally to meet project demands. There have been early suggestions that material may be sourced from distant locations using rail freight for transport. In addition to the Pacific Highway Upgrade the State's infrastructure program has a particular focus on transport projects designed to reduce the costs of Sydney's road congestion, which costs the State economy an estimated \$5.1 billion each year. Due to the close proximity to Sydney, and the advantage of the rail siding, Daracon are currently assessing locations that are suitable to unload trains into the Sydney area that can be used as a transfer point where re-loading onto trucks for delivery to projects is achievable.

All of these future projects, as well as other projects yet to be commissioned or determined, will impact upon the end market demands for quarry products from Martins Creek quarry.

4.1.2 Pedestrian Movements

Pedestrian access to the site will be available using the local road network, but with the site locality pedestrian movements will be low.

4.2 Traffic Distribution and Assignment

4.2.1 Hourly Distribution of Trips

The information provide above shows that for the proposed annual production of 1.5 million tonnes, the daily number of truck loads will be 168 loads per day. This is an *average* volume per day and will vary both up and down. In addition, the end user demands for material from the quarry are guided by standards for material and their application on site. This has a direct bearing upon the flow of trucks outbound from the site, with the majority of the demand for road base material for example occurring at the beginning of the working day at the quarry, so that it can be laid and spread within the normal working day on the construction site. This leads to a demand for typically 40 outbound trucks from the site per hour within the first three or more hours of operation within the quarry and a significant drop off in the supply of material in the afternoon period. The hourly distribution of existing trips is discussed and shown in Figure 4-1 above.

4.2.2 Origin / destinations assignment

The haulage routes to and from the quarry are reliant upon the end user demands, but within the immediate locality of the subject site there are three primary access routes that are currently being used by Daracon and account for more than 95% of the product haulage. These are:

- Martins Creek Quarry – Grace Avenue – Dungog Road – Gresford Road – Tocal Road – Paterson Road – Flat Road (Route 1)
- Martins Creek Quarry – Grace Avenue – Dungog Road – Gresford Road – Butterwick Road – Clarence Town Road – Brandy Hill Drive – Seaham Road (Route 2)
- Martins Creek Quarry – Grace Avenue – Dungog Road – Gresford Road – Tocal Road – Paterson Road – Belmore Road (Route 3)

Daracon have stated that as part of the approval process for the expansion of Martins Creek Quarry, all trucks will be diverted away from the route via Lorn (Route 3) and will use Flat Road instead, unless they have a destination in Lorn or the Maitland city centre or emergency related projects. Therefore, the future expansion will require the vast majority of the traffic to use Route 1 and Route 2 identified above.

With regard to the distribution of product from the site, a review has been completed by the study team of the product deliveries over the twelve months from November 2013 to October 2014. This is considered reflective of normal operation demands and can be used to predict the future expansion of the quarry. The approximate volume for the period totalled 1.1 million tonnes.

A distribution of percentages to each LGA are included in the table below. Consideration should be given to the major projects that Daracon supplied over this period, which generated short term increased volumes and indicates a larger market than would normally be expected. Major projects during this period included;

1. Hexham Rail upgrades – Newcastle
2. Nelson Bay Road Upgrade – Port Stephens
3. Inner City Bypass – Newcastle

Table 4-1 – Distribution of material from Martins Creek quarry by LGA areas

Market By Local Government Area	% of total volume
Newcastle	40.2%
Port Stephens	18.2%
Lake Macquarie	15.4%
Maitland	12.6%
Cessnock	3.6%
Singleton	1.7%
Gosford / Wyong	1.6%
Dungog	0.7%
Muswellbrook	0.6%
Upper Hunter	0.1%
Gloucester	0.1%
Sydney (Botany)	0.1%
Other	
Ex Bin / COD's with no Address	2.6%
Ballast Trains	2.5%

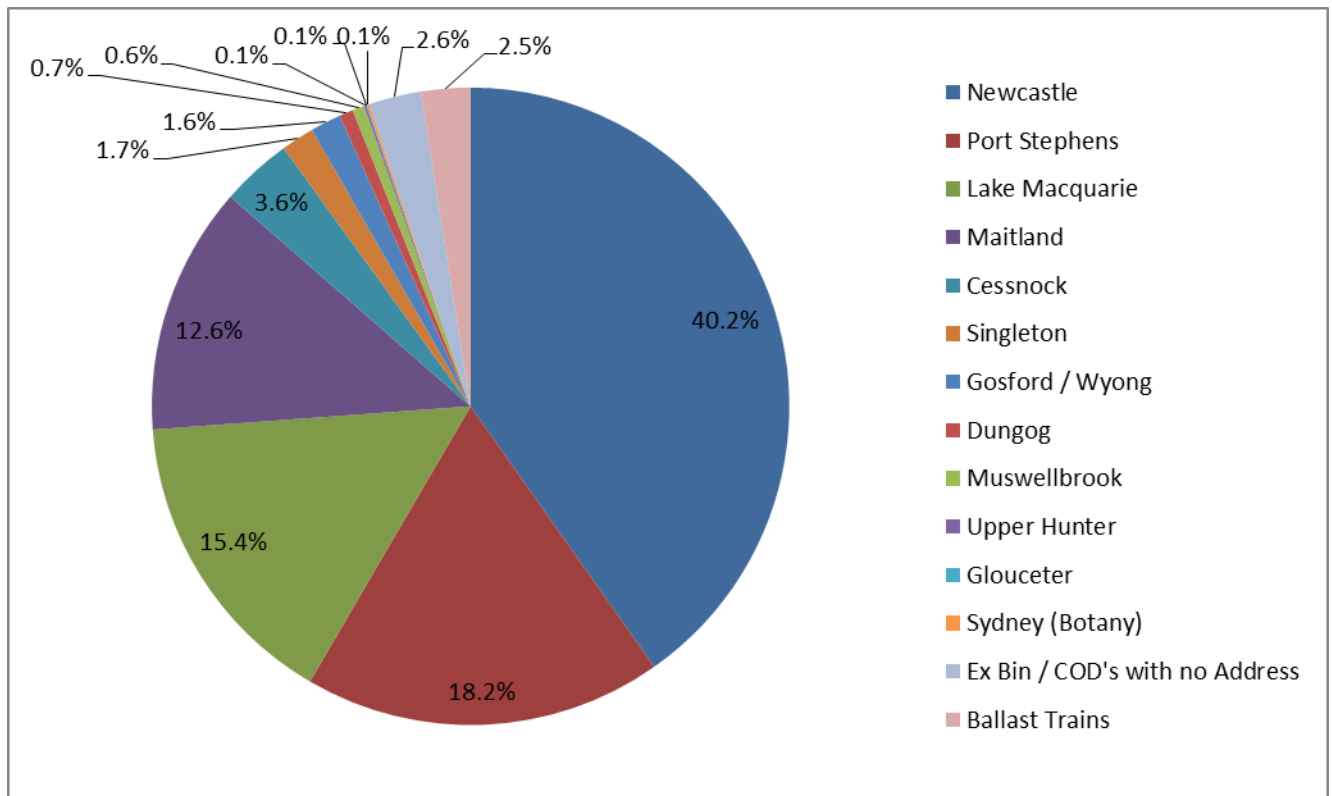


Figure 4-3 – Distribution of material from Martins Creek quarry by LGA areas

Daracon conducted a study of haulage routes based on heavy vehicles leaving the quarry for a period of 88 days from August 2014. The study involved the weighbridge personnel discussing and documenting each driver's intended route. The data collected and recorded was primarily based on the 3 major routes identified above, with vehicles not using these routes being recorded as other. The table below summarises the findings.

Table 4-2 – Identified trips generated by Martins Creek quarry (August to December 2014)

Month	Flat Road	Lorn	Brandy Hill	Other	Flat Road via Vacy	Lorn via Vacy	Brandy Hill via Vacy
Aug-14	24.7%	11.0%	18.0%	17.6%	12.2%	5.9%	10.6%
Sep-14	38.4%	14.6%	20.6%	9.7%	8.6%	2.9%	5.2%
Oct-14	52.0%	19.1%	21.5%	7.4%	0.0%	0.0%	0.0%
Nov-14	38.2%	29.8%	13.7%	18.3%	0.0%	0.0%	0.0%
Dec-14	55.5%	28.3%	10.4%	5.8%	0.0%	0.0%	0.0%

Due to the temporary closure of Gostwyck Bridge by the Roads and Maritime Services (RMS) for maintenance work, routes above also include the detour via Vacy that was implemented for the duration of each bridge closure.

The method used to breakdown the distribution of the Other destinations shown above was from data provided from Apex, Daracon's operational system that tracks quotes, orders and supply information. Data was extracted for the survey period (27th August 2014 – 9th December 2014) and was assessed by Daracon transport staff with haul routes determined based on job locations.

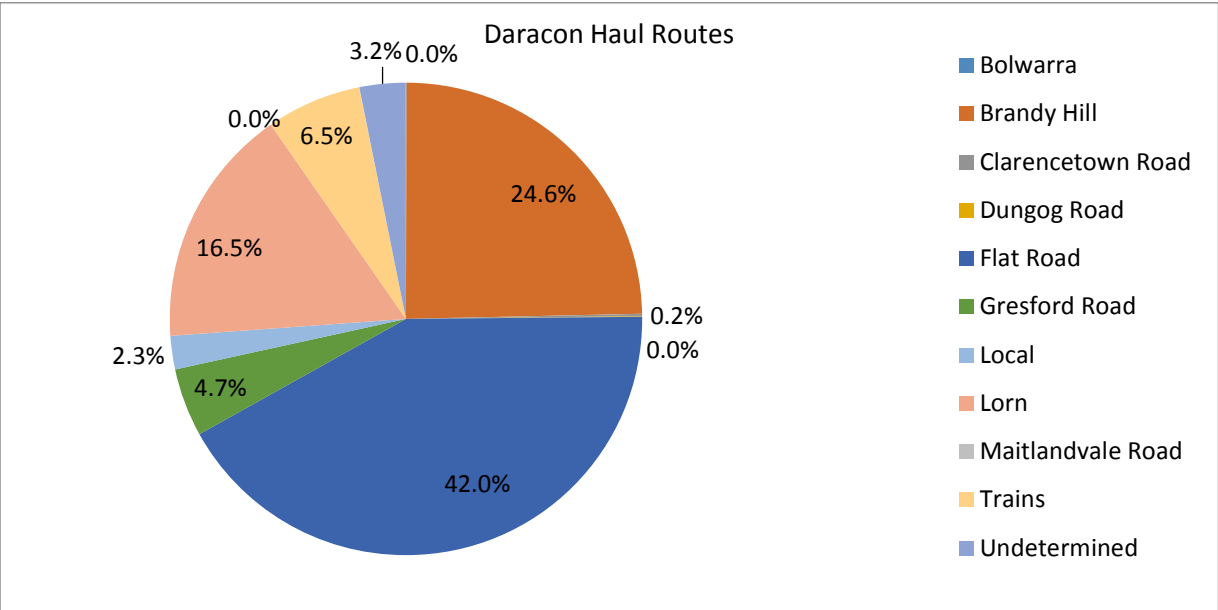


Figure 4-4 – Distribution of addition “other” trips based upon Daracon sales

Metromix haul routes were assessed separately (shown separately below) as they are a significant customer that has their own transport and picks material up directly from the quarry. Daracon's Apex system doesn't track the location of Metromix jobs and therefore the same method could not be used to determine the haul routes. Metromix previously provided an estimated distribution of materials supplied to each Local Government Area (LGA), determination of haul routes were therefore assumed based on the percentages provided.

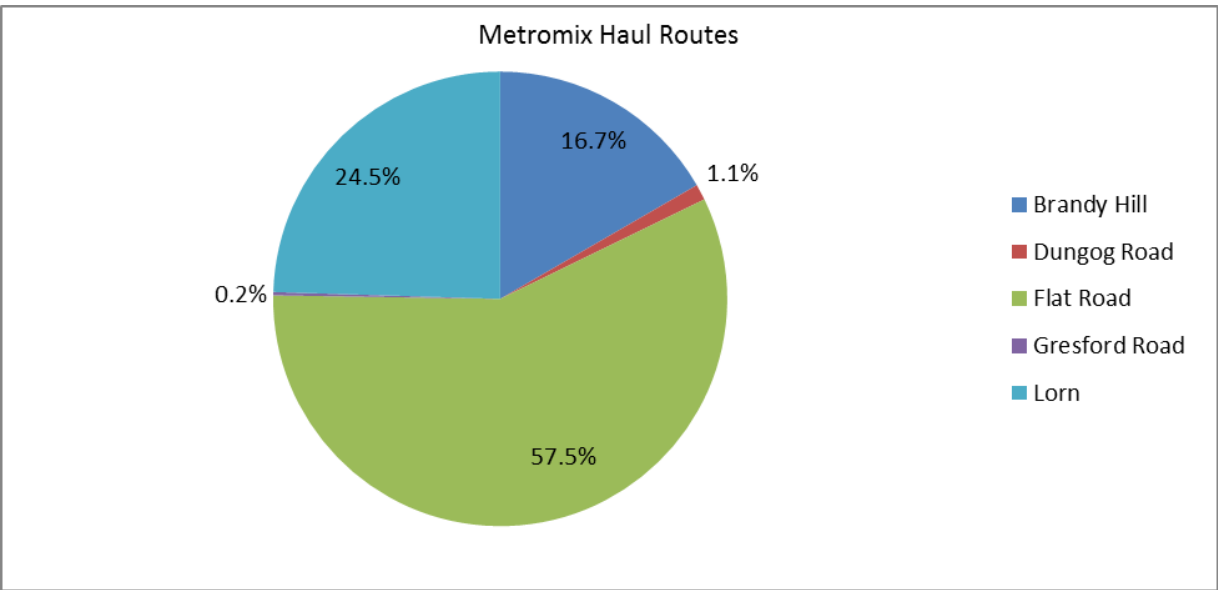


Figure 4-5 – Distribution of Metromix Haul Routes

Based upon the historic data for the quarry sales the following split of traffic volumes are given:

- 61.1% via Flat Road
- 25.1% via Brandy Hill
- 6.6% via train
- 2.4% local market
- 4.8% north

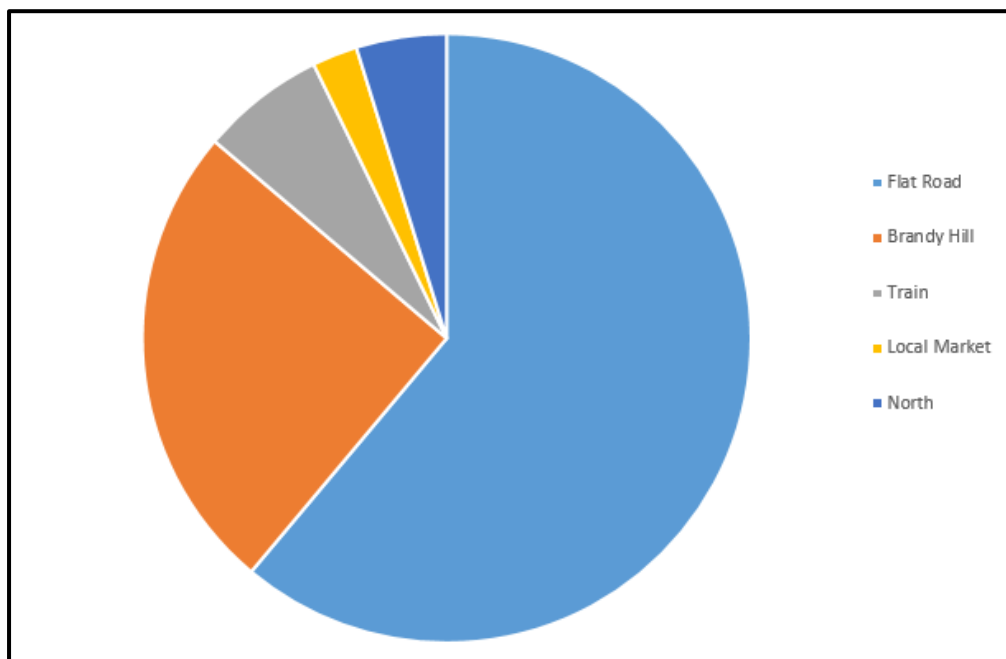
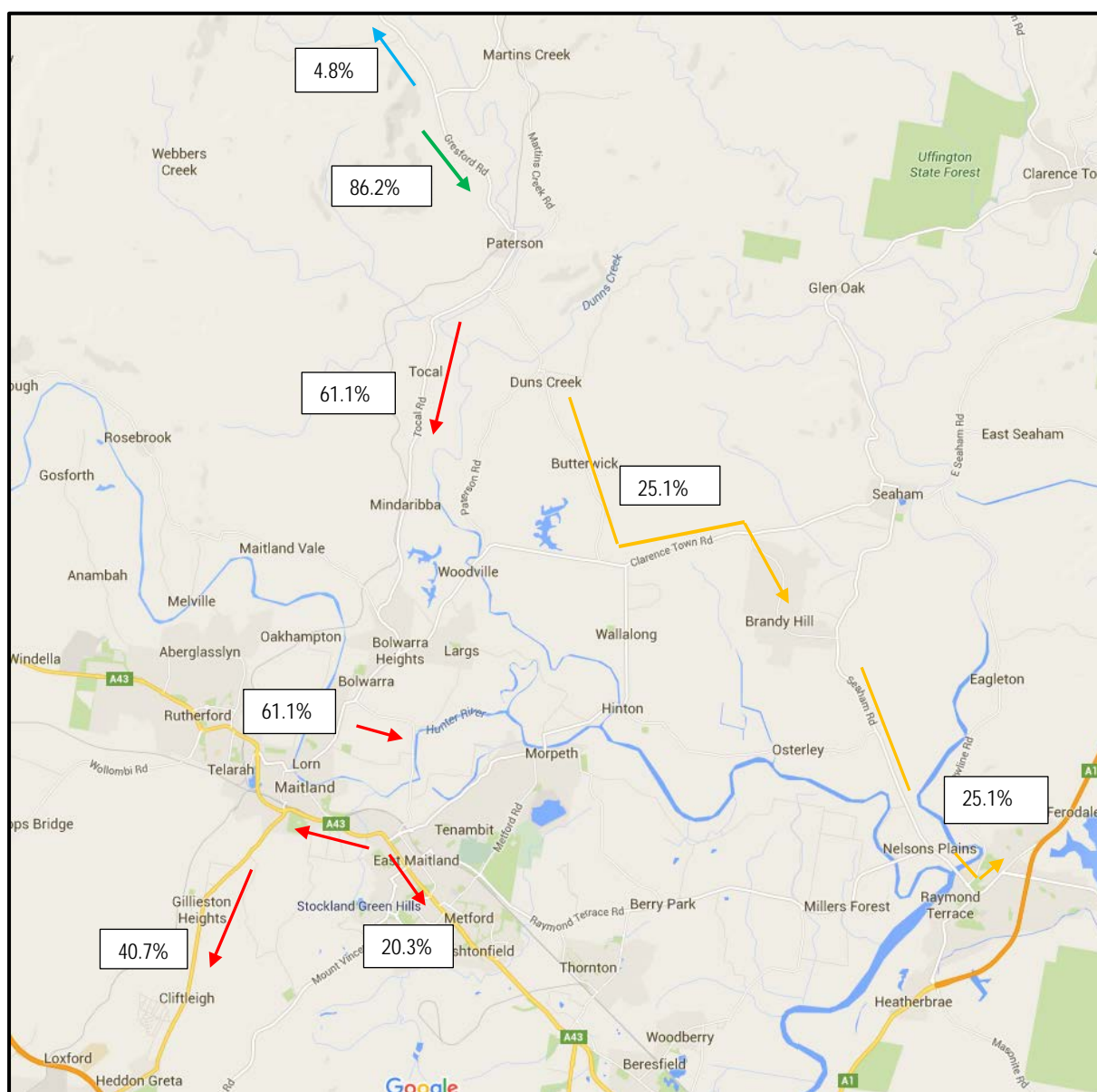


Figure 4-6– Distribution of material from Martins Creek Quarry



Based upon the full development allowing for 1.5 million tonnes per annum the volume of traffic for each route specified above is provided below.

Table 4-3 – Volume of traffic based upon existing distribution demands

Route	Percentage %	Average daily one way truck movements	Average hourly one way truck movements
Gresford Road via Paterson	86.2	145	18-19
Route via Flat Road	61.1	103	13
West along New England Highway towards Hunter Expressway	40.7	69	8-9
East along New England Highway from Melbourne Street	20.3	34	4-5
Route via Brandy Hill	25.1	42	5-6
Route north	4.8	8	1

Note the above values represent the outbound truck movements – there is a corresponding inbound truck movement per loaded truck.

It is worth noting that the maximum number of trucks per hour on any one route would be 40 outbound and 40-50 inbound. There can be a higher number of inbound trucks, as some truck drivers can arrive early on the site (especially when it is the second run for the day) and will have a break on site prior to being loaded and exiting the site. All outbound movements provide a maximum of 40 trucks per hour, due to the operational characteristics on site.

4.3 Impact on Road Safety

The review of the existing road network has highlighted a number of locations where there is an existing road safety concern relating to the layout of the road network which are not directly related to the existing activities of Martins Creek Quarry. These safety issues are due to the existing road system being built to historic rather than current design standards. These can be summarised as:

- Lack of space between intersection of Station Street and railway crossing and road alignment across railway crossing
- One-way bridge operation at Gostwyck Bridge on Dungog Road
- Lack of sheltered right turn lane on Gresford Road for drivers turning right into Dungog Road
- Tight road alignment on 90 degree bend at Gresford Road / Duke Street in Paterson
- Lack of pavement width on Tocal Road at Bolwarra Heights
- Lack of shoulders along Butterwick Road
- Lack of sheltered right turn lane on Clarence Town Road for drivers turning right into Butterwick Road
- Lack of sheltered right turn lane on Clarence Town Road for drivers turning right into Brandy Hill Drive

As part of the project, it is proposed to provide a new access that will provide direct access onto Dungog Road. This will remove the safety issues currently occurring at the railway crossing adjacent to Station Street. However, this new access to Dungog Road, requiring a new bridge to be constructed over the railway line, will require extensive planning and design prior to construction and the existing access to the quarry via Station Street will remain in operation to service the site. A review of the accident data provided by the RMS shows that the intersection of Station Street and Grace Avenue operates well and that there are no recorded accidents at this intersection. Whilst the intersection does not comply with current best practice, it can be seen that it continues to operate in a safe manner, mainly due to the high level of familiarity with drivers using this section of the road together with the low traffic flows on Grace Avenue. Visibility at this location also allows drivers in trucks exiting

the quarry to safely determine suitable gaps in the through traffic movements along Grace Avenue to exit Station Street.

Plans have been previously prepared by ARTC to upgrade the existing level crossing at Grace Avenue but there is no planned date for this upgrade work to commence. In the meantime, it is appropriate to consider that ARTC have reviewed the existing operation of the level crossing and have determined that it is suitable for on-going use. It is considered that whilst the project will increase the daily traffic flows over this level crossing, there will be minimal impacts upon road safety created by this. The existing controls for the level crossing will continue to advise vehicle drivers of approaching trains which enhances road safety.

For the one-way bridge on Dungog Road, discussion with the RMS has not highlighted any specific safety concerns with this one-way operation and the RMS has not identified the requirement to upgrade or bypass this bridge. The RMS has indicated that the bridge can continue to cater for the traffic demands associated with the expansion of the Martins Creek Quarry. Given the lack of accidents at this location it is proposed that the increased use of this route by trucks associated with the expansion of the quarry will not have a significant impact upon the safety at this location.

As part of the project, it has been recognised that the intersection of Gresford Road for drivers turning right into Dungog Road will be upgraded to provide a sheltered right turn lane in accordance with RMS and Austroads Guidelines.

As part of the project, it has been identified that the 90 degree bend at Gresford Road / Duke Street in Paterson can be upgraded to provide a raised central median which will direct vehicle movements and reduce the potential for collisions whilst also providing for a pedestrian crossing at this location.

Recent work completed by Council on Tocal Road through Bolwarra Heights has improved the road surface and line markings along this section of the road. However this has not increased the running width of the road. Given the apparent existing width constraint of the road reserve in this location, Council are not able to widen the road beyond the current alignment. The accident data at this location does not highlight any specific concerns and there have been no recorded incidents involving heavy vehicles in this location or any accidents resulting in injuries. Whilst the proposal will increase the total number of trucks passing along this section of the road, the intensity of truck movements will remain at similar levels to the existing operations and therefore there will be a minimal impact upon the road safety at this location.

For Butterwick Road, it can be seen that the existing width of the sealed road pavement does not comply with Austroad Guidelines, based upon the daily volume of traffic using this length of the road. It can be seen that the upgrade of the section of Butterwick Road with no sealed shoulders is required irrespective of the expansion of Martins Creek Quarry, due to the current traffic volumes. Whilst the expansion of Martins Creek Quarry will increase the volume of traffic using this road, there is no specific road safety concern raised by the increasing flows with the accident data indicating there is no specific accident location.

The lack of sheltered right turn lanes on Clarence Town Road at Butterwick Road and Brandy Hill Drive have been identified and are required for the current traffic movements, irrespective of the development of Martins Creek Quarry (and Brandy Hill Quarry). The current layout of these two intersections does not comply with the requirements of the RMS's Road Design Guide or Austroads requirements, which require the provision of a sheltered right turn lane.

4.4 Impact of Generated Traffic (Capacity and Level of service)

The impact of the development has been assessed against the requirements of the Authorities against the road capacity for both hourly flows and daily flows. The impact on the hourly flows has been checked against the traffic volume collected as part of the project and has covered the critical morning and afternoon peak hours. It can be seen that outside of the peak hours, the background traffic flows are much lower. At this time there is significant spare capacity in the road network which can cater for additional traffic demands without creating any capacity issues at intersections or on the road links.

4.4.1 Impact on daily Traffic Flows

The daily traffic flows for the site, based upon historic data, are given as:

- Average number of daily truck movements 111 inbound, 111 outbound
- 85th percentile of daily truck movements 159 inbound, 159 outbound
- Peak of daily truck movements 291 inbound, 291 outbound
- Theoretical daily peak of truck movements 320 inbound, 320 outbound

The impact of these flows on the daily traffic numbers is provided below. For the results below, this has presented an absolute worst case scenario by allowing:

1. ALL trucks from Martins Creek to use one route only. This maybe applicable if the quarry is only servicing one site, however normal practice allows for delivery of material to more than one site per day and using more than one route accordingly;
2. Allows for the quarry to run at full capacity for the full day

When determining the daily impact below, the traffic data at Dungog Road to the south of Martins Creek showed a daily heavy vehicle flow of 152 trucks (Monday-Friday 5am-5pm associated with the operational hours of Martins Creek quarry). It is assumed that 100% of these Class 9 vehicles relate to Martins Creek quarry. The difference between this and the potential two way peak flow of 640 truck movements has been applied to the balance of the route to assess the potential daily impact on the daily flows. At full operating capacity these 640 trucks give additional daily two-way flows of 488 trucks over those operating during the survey period.

Similarly, the traffic data breakdown per hour at Dungog Road provides details on the truck numbers per hour associated with Martins Creek quarry. The difference between these and the predicted peak flows of 80 trucks two way have been applied through each route and is presented in Table 4-4 and Table 4-5 below.

Table 4-4 Hourly and Daily impact of the Martins Creek quarry proposal by road segment – Route 1

Time	Current Traffic Flows (7 Days)	Weekly Quarry Flows (Class 9 - 7 Day)	Daily Average Quarry Flows (Class 9)	Expansion to Martins Creek	Gresford Rd, North of Paterson		Total Rd, South of Paterson		Paterson Rd, Bolwarra		Flat Rd	
					Current Flows (Class 9)	Future Flows (Class 9)	Current Flows (Class 9)	Future Flows (Class 9)	Current Flows (Class 9)	Future Flows (Class 9)	Current Flows (Class 9)	Future Flows (Class 9)
Midnight - 1am	6	2	1	0	0	0	0	0	1	1	0	0
1am - 2am	12	2	1	0	1	1	1	1	1	1	1	1
2am - 3am	13	2	1	0	1	1	1	1	1	1	1	1
3am - 4am	5	2	1	0	1	1	1	1	1	1	1	1
4am - 5am	8	1	1	0	1	1	1	1	1	1	1	1
5am - 6am	67	18	4	6	5	11	6	12	8	14	4	10
6am - 7am	169	115	20	60	20	80	14	74	15	75	7	67
7am - 8am	236	72	14	66	16	82	13	79	19	85	7	73
8am - 9am	317	77	14	66	16	82	11	77	15	81	5	71
9am - 10am	350	82	14	66	16	82	14	80	19	85	6	72
10am - 11am	333	94	19	61	20	81	13	74	18	79	6	67
11am - Midday	329	88	16	64	17	81	14	78	18	82	7	71
Midday - 1pm	336	75	15	65	19	84	14	79	19	84	7	72
1pm - 2pm	334	64	13	17	13	30	11	28	15	32	5	22
2pm - 3pm	358	54	11	9	13	22	12	21	17	26	5	14
3pm - 4pm	450	59	12	8	15	23	10	18	11	19	4	12
4pm - 5pm	438	7	2	0	2	2	3	3	5	5	2	2
5pm - 6pm	371	2	1	0	1	1	1	1	2	2	1	1
6pm - 7pm	228	4	1	0	1	1	1	1	1	1	0	0
7pm - 8pm	144	1	1	0	0	0	0	0	1	1	1	1
8pm - 9pm	86	2	1	0	0	0	0	0	0	0	0	0
9pm - 10pm	84	5	1	0	1	1	0	0	1	1	0	0
10pm - 11pm	42	0	0	0	0	0	0	0	0	0	0	0
11pm - Midnight	29	1	1	0	0	0	0	0	0	0	0	0
Totals	4745	829	165	488	179	667	141	629	189	677	71	559

Table 4-5 – Hourly and Daily impact of the Martins Creek quarry proposal by road segment – Route 2

Time	Current Traffic Flows (7 Days)	Weekly Quarry Flows (Class 9 - 7 Day)	Daily Average Quarry Flows (Class 9)	Expansion to Martins Creek	Gresford Rd, North of Paterson		Total Rd, South of Paterson		Butterwick Rd		Clarence Town Rd, between Butterwick Rd & Brandy Hill Dr		Brandy Hill Dr		Seaham Rd, North of William River	
					Current Flows (Class 9)	Future Flows (Class 9)	Current Flows (Class 9)	Future Flows (Class 9)	Current Flows (Class 9)	Future Flows (Class 9)	Current Flows (Class 9)	Future Flows (Class 9)	Current Flows (Class 9)	Future Flows (Class 9)	Current Flows (Class 9)	Future Flows (Class 9)
Midnight - 1am	6	2	1	0	0	0	0	0	0	0	0	0	0	0	0	0
1am - 2am	12	2	1	0	1	1	1	1	0	0	0	0	0	0	0	0
2am - 3am	13	2	1	0	1	1	1	1	0	0	0	0	0	0	1	1
3am - 4am	5	2	1	0	1	1	1	1	0	0	0	0	0	0	0	0
4am - 5am	8	1	1	0	1	1	1	1	0	0	0	0	0	0	0	0
5am - 6am	67	18	4	6	5	11	6	12	1	7	1	7	7	13	7	13
6am - 7am	169	115	20	60	20	80	14	74	2	62	6	66	18	78	18	78
7am - 8am	236	72	14	66	16	82	13	79	3	69	5	71	12	78	15	81
8am - 9am	317	77	14	66	16	82	11	77	5	71	6	72	13	79	15	81
9am - 10am	350	82	14	66	16	82	14	80	4	70	4	70	16	82	16	82
10am - 11am	333	94	19	61	20	81	13	74	4	65	4	65	14	75	17	78
11am - Midday	329	88	16	64	17	81	14	78	6	70	5	69	13	77	16	80
Midday - 1pm	336	75	15	65	19	84	14	79	4	69	5	70	13	78	15	80
1pm - 2pm	334	64	13	17	13	30	11	28	4	21	5	22	9	26	9	26
2pm - 3pm	358	54	11	9	13	22	12	21	2	11	4	13	8	17	11	20
3pm - 4pm	450	59	12	8	15	23	10	18	4	12	3	11	8	16	9	17
4pm - 5pm	438	7	2	0	2	2	3	3	1	1	2	2	3	3	5	5
5pm - 6pm	371	2	1	0	1	1	1	1	0	0	1	1	1	1	1	1
6pm - 7pm	228	4	1	0	1	1	1	1	0	0	0	0	0	0	1	1
7pm - 8pm	144	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0
8pm - 9pm	86	2	1	0	0	0	0	0	0	0	0	0	0	0	0	0
9pm - 10pm	84	5	1	0	1	1	0	0	0	0	0	0	0	0	0	0
10pm - 11pm	42	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
11pm - Midnight	29	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0
Totals	4745	829	165	488	179	667	141	629	40	528	51	539	135	623	156	644

4.4.2 Peak Hour Impacts on Intersections

As part of the assessment for the proposed development, the intersection capacity analysis program Sidra has been used. The Sidra analysis has reviewed the impact of the generated traffic at the key intersections of Pitnacree Road / Melbourne Street and Melbourne Street / New England Highway. Both of these intersections are controlled by traffic signals and form part of the regional road network. The traffic signals are controlled by the RMS.

As part of the project work, the current traffic flows at these two intersections were surveyed during the typical morning and afternoon peak periods. The surveys were completed on Thursday 26th February 2015. A summary of the results of these surveys is provide in Appendix B to this report.

The surveys show that the AM peak was generally 8.15 to 9.15 AM when the Martins Creek quarry is busy, as is the surrounding road network, and 4.30 to 5.30 PM when the traffic movements associated with Martins Creek are minimal, due to the significant drop off in material delivery beyond 4 PM.

The performance of these two intersections has been modelled with the Sidra Network modelling program, a standard computer program utilised by the RMS, Councils and consultants across the world. The modelling allows for an assessment of the operation of the intersections and provides details on the performance of the intersection against standard criteria applied by the RMS and Council.

A summary of the results of the Sidra analysis, for the current situation is provided below.

Table 4-6 – Sidra results, current operation of New England Highway and Melbourne Street

Approach	Level of service	Delay (seconds)	Queue (metres)
NEH from Newcastle	F / C	72.8 / 39.9	394.8 / 220.5
Melbourne St from Morpeth	F / D	72.1 / 54.9	228.8 / 123.1
NEH from Maitland	C / F	35.5 / 73.7	243.1 / 522.8
Melbourne St	D / E	53.9 / 65.3	95.7 / 115.6
Overall	E / E	57.0 / 60.7	394.8 / 522.8

Note – results above are for AM / PM peak

From the above results, it can be seen that the intersection of the New England Highway and Melbourne Street suffers from delays and congestion during both the morning and afternoon peak periods. Outside of the peak periods this intersection works reasonably well with minimal delays and congestion for the majority of the movements. Whilst the opening of the Hunter Expressway has removed a large volume of through traffic movements in this section of the New England Highway, there is still a high local demand for traffic along this road which creates some delays and congestion during the traditional peak periods. In particular, the traffic flows from Maitland / Rutherford towards Newcastle in the afternoon are very high, leading to a lengthy queue on the approach to the traffic signals on the New England Highway.

It is noted that during the surveys, Martins Creek quarry was fully functional (based on the weighbridge information provided for the same period as the surveys) and as such haulage trucks from this quarry as well as Brandy Hill Quarry were observed using this intersection, especially during the morning peak. During the afternoon peak (4.30 to 5.30 PM) there were minimal large trucks associated with quarry material haulage observed, reflective of the low demands generated by the quarry at this time.

A review of the intersection layout shows that the capacity of this intersection is constrained, due to established development on each corner. There is limited scope for any upgrades at this intersection to increase capacity, other than fine tuning of the traffic signal operation. These signals are vehicle actuated and respond to vehicle demands / queues and as such provide the maximum capacity available at this location. The operation of these traffic signals is monitored by the RMS and refined as appropriate.

The total flows through this intersection were 4,300 in the AM peak and 4,515 during the PM peak. Trucks movements associated with Martins Creek quarry could be a maximum of 80 trucks per hour, which represents 1.9% of the morning peak hour flows and 1.8% of the afternoon peak hour flows. It can thus be seen that the delays and congestion at this intersection cannot be directly linked to this quarry's operations.

Table 4-7 - Sidra results, current operation of Pitnacree Road and Melbourne Street

Approach	Level of service	Delay (seconds)	Queue (metres)
Melbourne St from NEH	C / C	34.6 / 38.6	165.5 / 286.8
Lawes Street	D / D	48.3 / 51.9	64.9 / 95.7
Melbourne St from Morpeth	C / C	38.0 / 32.2	217.8 / 124.7
Pitnacree Road	D / D	53.3 / 66.1	147.1 / 115.8
Overall	C / D	42.4 / 43.6	217.8 / 286.8

Note – results above are for AM / PM peak

From the above results, it can be seen that the intersection of Melbourne Street and Pitnacree Road suffers from delays and congestion during both the morning and afternoon peak periods. Outside of the peak periods this intersection works reasonably well with minimal delays and congestion for the majority of the movements.

It is noted that during the surveys, Martins Creek quarry was fully operational and as such haulage trucks from this quarry (as well as Brandy Hill Quarry) were observed using this intersection, especially during the morning peak. During the afternoon peak (4.30 to 5.30 PM) there were minimal large trucks associated with quarry material haulage observed, reflective of the low demands generated by the quarry at this time.

A review of the intersection layout shows that the capacity of this intersection is constrained, due to established development on each corner. There is limited scope for any upgrades at this intersection to increase capacity, other than fine tuning of the traffic signal operation. These signals are vehicle actuated and respond to vehicle demands / queues and as such provide the maximum capacity available at this location. The operation of these traffic signals is monitored by the RMS and refined as appropriate.

The total flows through this intersection are 2,216 in the AM peak and 2,303 during the PM peak. Trucks movements associated with Martins Creek quarry could be a maximum of 80 trucks per hour, which represents 3.6% of the morning peak hour flows and 3.5% of the afternoon peak hour flows. It can thus be seen that the delays and congestion at this intersection cannot be directly linked to this quarry's operations.

The intersection was then assessed with the potential additional traffic movements associated with Martins Creek quarry. When assessing the potential additional numbers, the following points have been taken into consideration:

- The trucks associated with Martins Creek quarry will be directed to avoid Lorn and use Flat Road. These outbound trucks will turn right from Pitnacree Road into Melbourne Street then right from Melbourne Street onto the New England Highway;
- There will be a corresponding reverse movement of trucks, turning left off the highway into Melbourne Street and then left into Pitnacree Road
- As a worst case scenario, a project may require all of the material to be delivered to a site west of Maitland and would thus all turn right out of Pitnacree Road then right from Melbourne Street into the New England Highway.

For the scenario below, we have assumed the worst case scenario above and have NOT discounted the existing flows to remove the Martins Creek quarry traffic that was turning right into Melbourne Street off the New England Highway and the reverse traffic movement. The results of this assessment are presented below.

Table 4-8 - Sidra results, AM peak current volumes plus Martins Creek quarry worst case scenario, New England Highway and Melbourne Street

Approach	Level of service	Delay (seconds)	Queue (metres)
NEH from Newcastle	F	83.2	422.4
Melbourne St from Morpeth	F	80.4	259.9
NEH from Maitland	C	36.7	251.5
Melbourne St	D	53.8	95.7
Overall	E	62.5	422.4

The results above indicate that the intersection will operate to a similar level as the existing, with a minor increase for the traffic heading along the highway from Newcastle. This will have a small impact upon the queue length of this approach. It can be seen that this worst case scenario overall would have a minimal impact upon the operation of this intersection and that overall it will continue to operate to similar levels of delay and congestion as the current scenario.

Note that the PM peak period between 4.30 and 5.30 PM has not been assessed, as at this point the traffic movements associated with the Martins Creek quarry are negligible.

Table 4-9 - Sidra results, AM peak current volumes plus Martins Creek quarry, Melbourne Street and Pitnacree Road

Approach	Level of service	Delay (seconds)	Queue (metres)
Melbourne St from NEH	C	34.5	165.5
Lawes Street	D	48.3	64.9
Melbourne St from Morpeth	C	38.0	217.8
Pitnacree Road	D	54.3	147.1
Overall	D	42.7	217.8

The Sidra results above demonstrate that the potential additional truck movements associated with the worst case scenario with the trucks from Martins Creek quarry would have an acceptable impact upon the overall operation of this intersection, with minor increases in delays but reduced overall queues, due to the slight change in balance of the traffic movements.

4.4.3 Impact of Construction Traffic

There will be minimal construction work associated with the proposed expansion of the quarry, with all works located on site utilising the existing equipment and plant on site. Therefore, there will be no impact upon the external road network.

4.4.4 Background traffic and other developments

Advice from Council indicates that there is limited growth expected within the general study area north of Bolwarra and through the Brandy Hill area. Some development is occurring in Morpeth which may impact upon the traffic signals along Melbourne Street. However, it is noted that a significant portion of this is aged care and retirement village development which generates very low traffic flows during the traditional peak periods.

The major development in the locality is associated with the proposed expansion of Brandy Hill quarry. The quarry has been operating at an average yearly output of 620,000 tonnes which equates to approximately 150 truck movements per day. The proposal allows for this quarry to increase production to around 1.5 million tonnes per year, with the associated increase in truck movements on the road network. Whilst details of the proposed increase and changes to the operations at Brandy Hill have been requested from the proponent, these details have not been provided.

As per the normal requirement of the RMS (and as stated by the RMS) the operation of the key intersections on Melbourne Street have been assessed for the future design year of 2026, allowing for background traffic growth to occur over this 10 year timeframe. Normal background traffic growth in the locality of the Lower Hunter Valley is 2% per annum giving some 20% over 10 years. This value has been allowed for growth along the New England Highway corridor, reflective of growth expected to occur along this corridor. As per the advice above from Council limited growth is expected to be generated by development in the Bolwarra, Brandy Hill and Morpeth area. Accordingly, a growth value of 5% has been applied over a 10 year timeframe along Melbourne Street.

The results of the future design year 2026 is shown below:

Table 4-10– Future design year, 2026 intersection of New England Highway and Melbourne Street (base data plus background growth)

Approach	Level of service	Delay (seconds)	Queue (metres)
NEH from Newcastle	F / D	126.2 / 46.8	621.5 / 303.1
Melbourne St from Morpeth	F / E	125.4 / 61.0	320.1 / 138.6
NEH from Maitland	D / F	44.7 / 140.5	337.4 / 860.5
Melbourne St	F / F	71.2 / 93.5	113.7 / 145.3
Overall	F / F	90.6 / 98.5	621.5 / 860.5

Note – results above are for AM / PM peak

The above results demonstrate that the intersection will start to suffer from increased delays and congestion, during both the AM and the PM peaks when the predicted levels of service for each approach are F. During the critical PM peak period, the traffic movements associated with Martins Creek Quarry (and Brandy Hill Quarry) would be minimal as the output of material is negligible at this time of the working day for Martins Creek Quarry. Similarly, for Brandy Hill Quarry their Preliminary Environmental Assessment indicates that 80% of the daily activities occur between 6AM and 12 PM.

Based upon the assessment above and the analysis above for the current design year (2016) plus development flows it is considered that the traffic movements associated with Martins Creek Quarry will continue to have a minimal and acceptable impact upon the overall operation of these two traffic signal controlled intersections. Whilst this intersection is predicted to suffer from increasing delays, this is due to the continual traffic growth along the New England Highway in this location rather than a direct impact of the proposal. The RMS will continue to monitor the operation of this intersection and the New England Highway road corridor and implement upgrades as required. The upgrade of the intersection of the New England Highway and Cessnock Road / Church Street to the west of this intersection demonstrates the RMS commitment to upgrade this road as required to accommodate continual traffic growth along this corridor.

4.5 Public Transport

4.5.1 Options for improving services

Minimal, if any demand for public transport will be generated by the new site proposal.

4.5.2 Pedestrian Access to Bus Stops

No specific pedestrian access to any bus stops need be provided.

4.6 Pedestrian and Cyclists

The development is not considered to be a major attractor for pedestrians or cyclists. Local access is available via the existing road network as appropriate. No changes are proposed.

5 Improvement Analysis

5.1 Improvements to Accommodate Existing Traffic

The following road safety concerns have been identified as being required to bring the road network up to current design standards to accommodate the existing traffic flows along the two key routes for the project:

- Lack of space between intersection of Station Street and railway crossing and road alignment across railway crossing
- One-way bridge operation on Dungog Road
- Lack of sheltered right turn lane on Gresford Road for drivers turning right into Dungog Road
- Tight road alignment on 90 degree bend at Gresford Road / Duke Street in Paterson
- Lack of pavement width on Tocal Road at Bolwarra Heights
- Lack of shoulders along Butterwick Road
- Lack of sheltered right turn lane on Clarence Town Road for drivers turning right into Butterwick Road
- Lack of sheltered right turn lane on Clarence Town Road for drivers turning right into Brandy Hill Drive

However, discussion with the relevant authorities shows that the above issues have been reviewed and the following advice has been provided:

- The railway crossing on Grace Avenue has been reviewed by ARTC and they have no plans to upgrade this crossing. The layout is considered acceptable given the volume and frequency of trains and the alignment of the road and the approaches;
- The RMS has stated that the bridge on Dungog Road is adequate and they have no plans to upgrade or replace this bridge.
- Council has identified and constructed upgrades to the alignment of Tocal Road at Bolwarra Heights to improve the delineation and have upgraded the road surface.

Whilst the width of Butterwick Road does not comply with current design standards, a review of the accident data shows that there are no specific safety concerns raised by the alignment and there are no recorded accidents along this road associated with trucks from Martins Creek Quarry.

These issues relate to existing traffic issues along the road network impacted upon by the development.

The review of the performance of the signal controlled intersections on Melbourne Street highlight there are capacity issues currently occurring along this length of the road, due to the volume of traffic in this location. As part of the regional road network, the RMS will continue to monitor the performance of this intersection and upgrade this intersection if required.

5.2 Improvements to Accommodate Background Traffic

Advice from Council indicates that there is no expected increase in traffic flows along the key routes due to other developments in the general locality of the subject site.

5.3 Additional Improvements to Accommodate Development Traffic

To allow for the on-going use of the major routes utilised by the trucks for Martins Creek quarry, the following road upgrades are put forward for consideration with the relevant road authority:

- 1 - Upgrade intersection to provide a dedicated sheltered right turn lane at Dungog Road and Gresford Road
- 2 - Provide physical guidance for vehicles to manoeuvre around the 90 degree bend in Paterson
- 3 - Upgrade intersection to provide a dedicated sheltered right turn lane at Butterwick Road and Clarence Town Road
- 4 - Upgrade intersection to provide a dedicated sheltered right turn lane at Clarence Town Road and Brandy Hill Drive. However this intersection is impacted upon by the Brandy Hill quarry and any upgrade works at this intersection must take into account upgrade works required as part of the expansion of this quarry with appropriate cost sharing.

The proposed road works detailed above will provide the following benefits.

- Providing a sheltered right turn lane on Gresford Road at Dungog Road will improve road safety, by reducing or eliminating the potential for rear end type accidents. The current layout (Rural Type AUR) can lead to drivers of through vehicles on Gresford Road running into the rear of a vehicle propped on Gresford Road waiting to turn right into Dungog Road. The upgrade will direct all through traffic to steer to the left of any vehicle waiting to turn right at this location. This upgrade is in line with RMS policy which no longer permits Rural Type CHR intersection controls and requires a Rural CHR type intersection.
- The upgrade at the 90 degree bend in Paterson will ensure that all vehicles drive on the correct side of the road and do not cross over the centre line. The upgrade also allows for a raised central pedestrian median to assist pedestrian crossing the road in this location and improve road safety for these pedestrians.
- The upgrade at the intersection of Butterwick Road and Clarencetown Road is similar to the upgrade at Gresford Road at Dungog Road and is in line with current RMS design requirements. It will allow for safety improvements for through traffic on Clarencetown Road.
- The upgrade at Clarencetown Road and Brandy Hill Drive will allow for an improvement to road safety for through traffic movements on Clarencetown Road, with the provision of a sheltered right turn lane.

5.4 Alternative Improvements

No other alternatives are put forward for consideration.

6 Summary and Recommendations

6.1 Summary

The proposed development of Martins Creek Quarry allows for the continual use of the quarry and for the output of the quarry to increase to 1.5 million tonnes per annum. Whilst the volume of material extracted over the year is proposed to increase, the hourly number of trucks associated with the development will not increase and will remain at the current rate of 40 trucks per hour inbound and outbound.

Traffic data along the key routes associated with the haulage of material has been collected at mid-block locations as well as at the key intersections impacted upon by the project. This data shows that the current road network impacted upon by the project currently carries traffic flows well within their capacity, reflective of the rural setting for the majority of the haulage length. In the urban settings for the haulage routes the capacity of the road, as determined by the RMS, is higher and the operation of these roads is well within their capacity. With the traffic movements associated with the Martins Creek Quarry remaining at similar hourly levels, these roads will continue to operate within acceptable limits.

As part of the development, it is proposed to divert the haulage trucks away from the existing haulage route via Lorn and to divert this traffic along Flat Road and then via Melbourne Street to connect with the New England Highway. Whilst improving the road environment through Lorn, this will impact upon the operation and capacity of the signal controlled intersection of Pitnacree Road and Melbourne Street as well as the New England Highway with Melbourne Street. As part of the project work, traffic surveys have been completed at these two intersections and a Sidra intersection analysis has been completed for each of the intersections. This analysis confirms the operation of the existing intersection and has determined that the potential impacts of the Martins Creek Quarry haulage trucks is minimal and that the delays and congestion at both of these intersections will remain at similar levels to the existing operations.

As part of the project work, a safety review has also been completed along both of the key haulage routes associated with Martins Creek Quarry as well as a review of the accident data for these routes provided by the RMS. This review shows that the existing accident rates along the two key routes are reasonable low. The road authority has not noted any particular areas of concern with regard to road safety and have no plans to upgrade any of the existing road network in the locality of the site or along the two key access routes. The road authority have provided comment with regard to lack of shoulders in some locations along the haul routes as well as intersection controls, and the safety audit noted these upgrade requirements are required to bring the road network up to current design standards.

From the project work completed for Martins Creek Quarry, it can be seen that the proposed changes and increases to the output of the quarry will have an acceptable impact upon the local road network, as the hourly traffic volumes will remain similar to the existing levels. The increased output will be market driven and demands will be met by the more efficient use of the quarry and consistent transportation of product over time.

6.2 Recommendations

From the study work, the following road upgrades are put forward for discussion with the relevant road authority to cater for the continual use of the key haulage routes for the quarry:

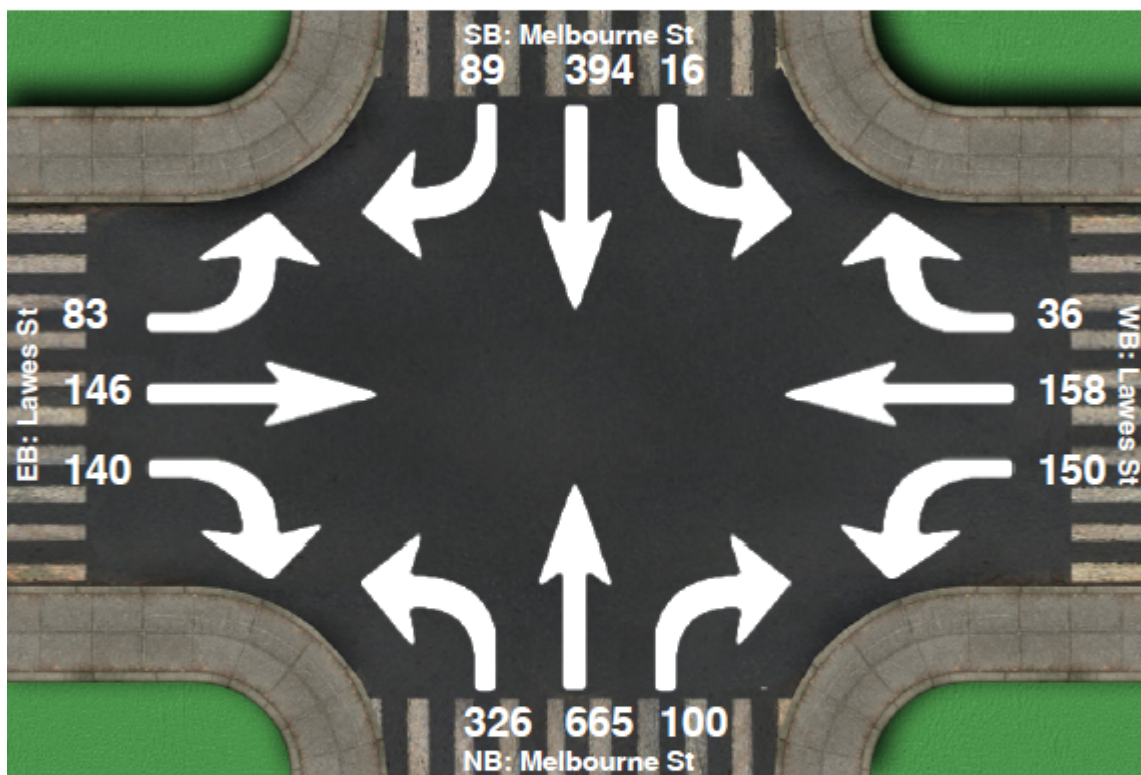
- 1 - Upgrade intersection to provide a dedicated sheltered right turn lane at Dungog Road and Gresford Road
- 2 - Provide physical guidance for vehicles to manoeuvre around the 90 degree bend in Paterson
- 3 - Upgrade intersection to provide a dedicated sheltered right turn lane at Butterwick Road and Clarence Town Road
- 4 - Upgrade intersection to provide a dedicated sheltered right turn lane at Clarence Town Road and Brandy Hill Drive. This will also need to take into account upgrades associated with the Brandy Hill quarry expansion with appropriate cost sharing.

Based upon our project work, it is considered that the proposed ongoing use of Martins Creek Quarry and its expansion to allow for 1.5 million tonnes per annum for extraction should be approved on traffic and access grounds.

Appendix A – Intersection survey data

Intersection Peak Hour

Location: Melbourne St at Lawes St, East Maitland
GPS Coordinates:
Date: 2015-02-26
Day of week: Thursday
Weather:
Analyst: BM



Intersection Peak Hour

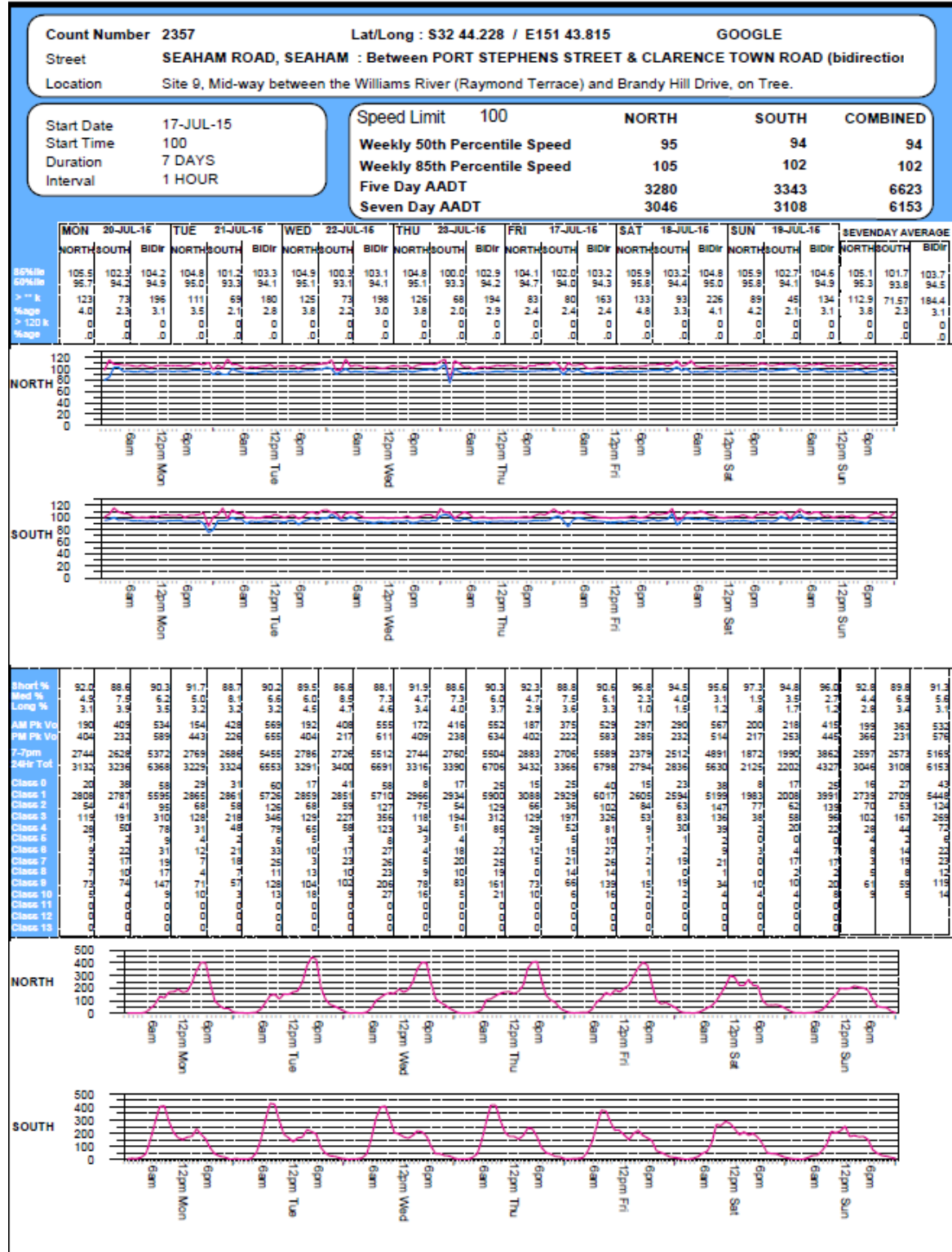
16:30 - 17:30

	SouthBound			Westbound			Northbound			Eastbound			Total
	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	
Vehicle Total	16	394	89	150	158	36	326	665	100	83	146	140	2303
Factor	0.50	0.80	0.82	0.85	0.90	0.75	0.89	0.90	0.81	0.72	0.78	0.88	0.89
Approach Factor	0.85			0.91			0.90			0.80			

Appendix B – Tube Count data

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One Page Summary

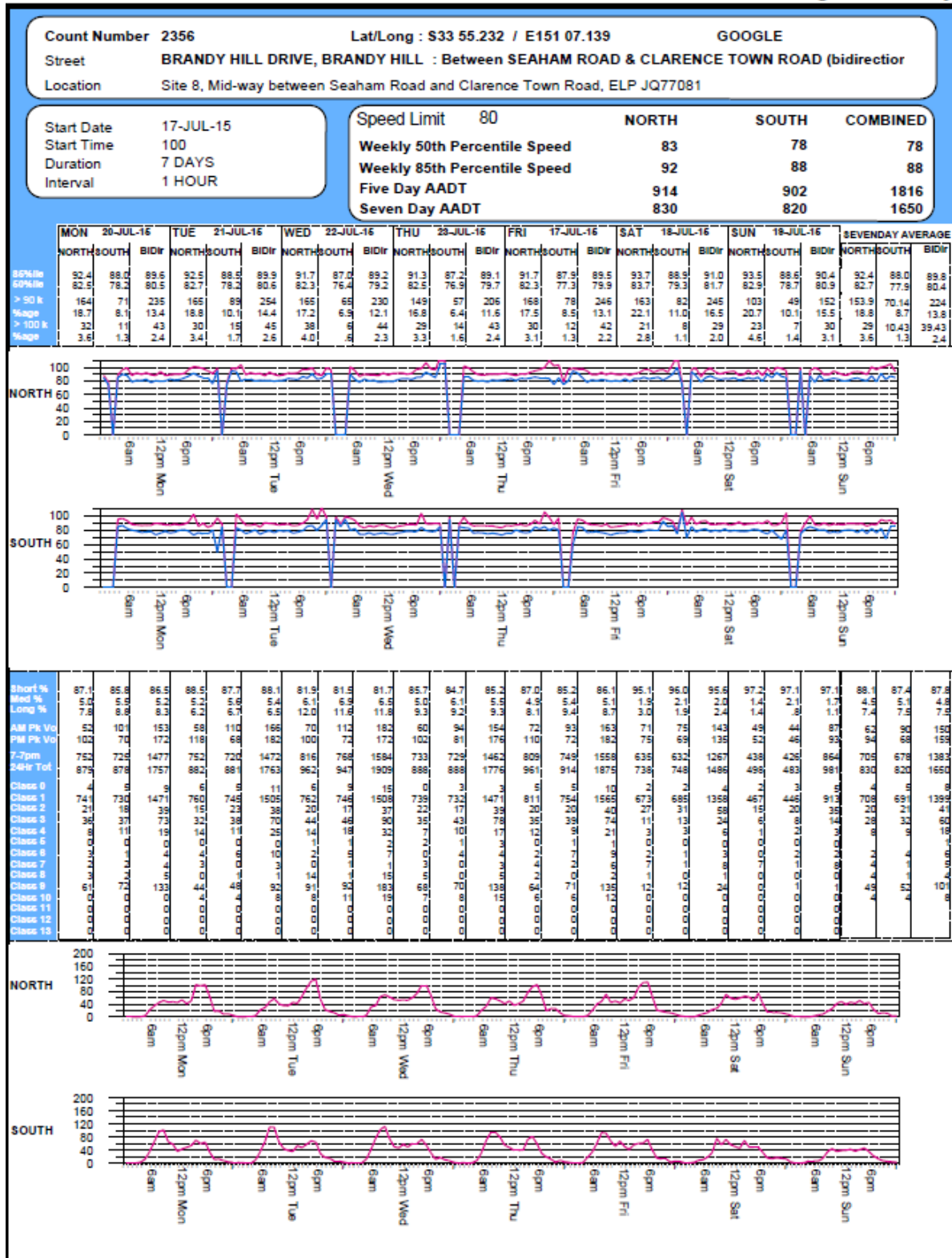


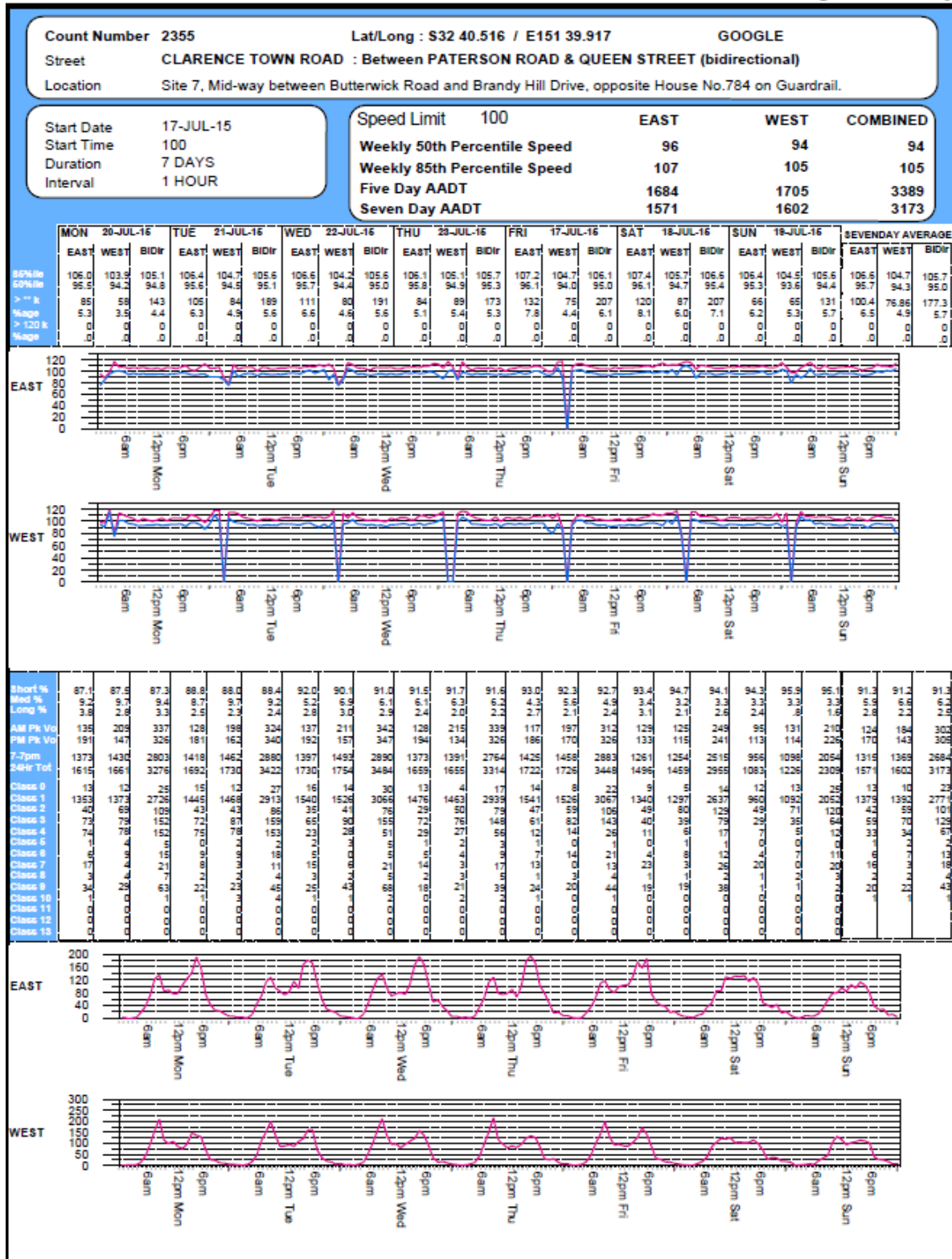
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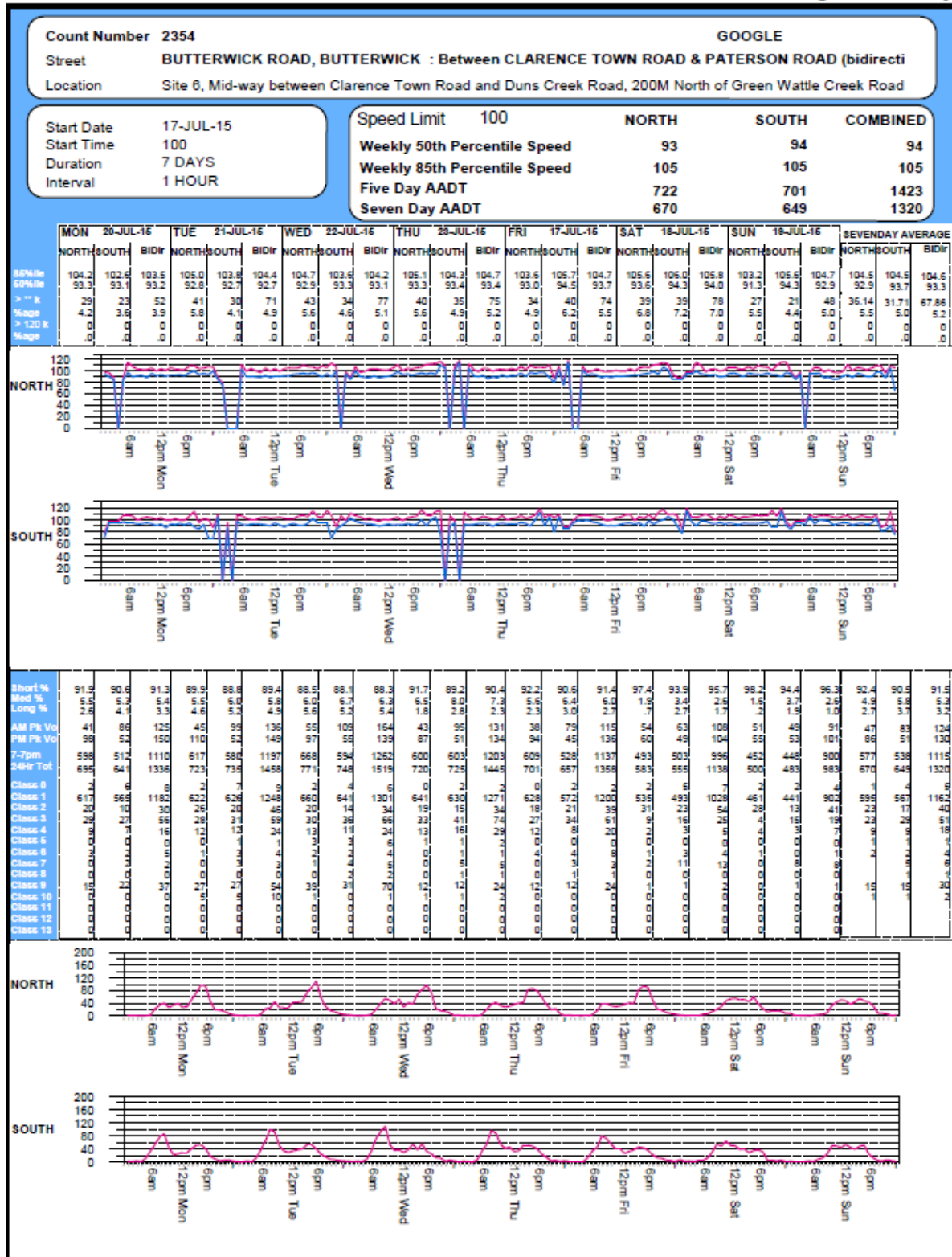
Data displayed has been compiled from pneumatic traffic count processes and is subject to the documented limitations

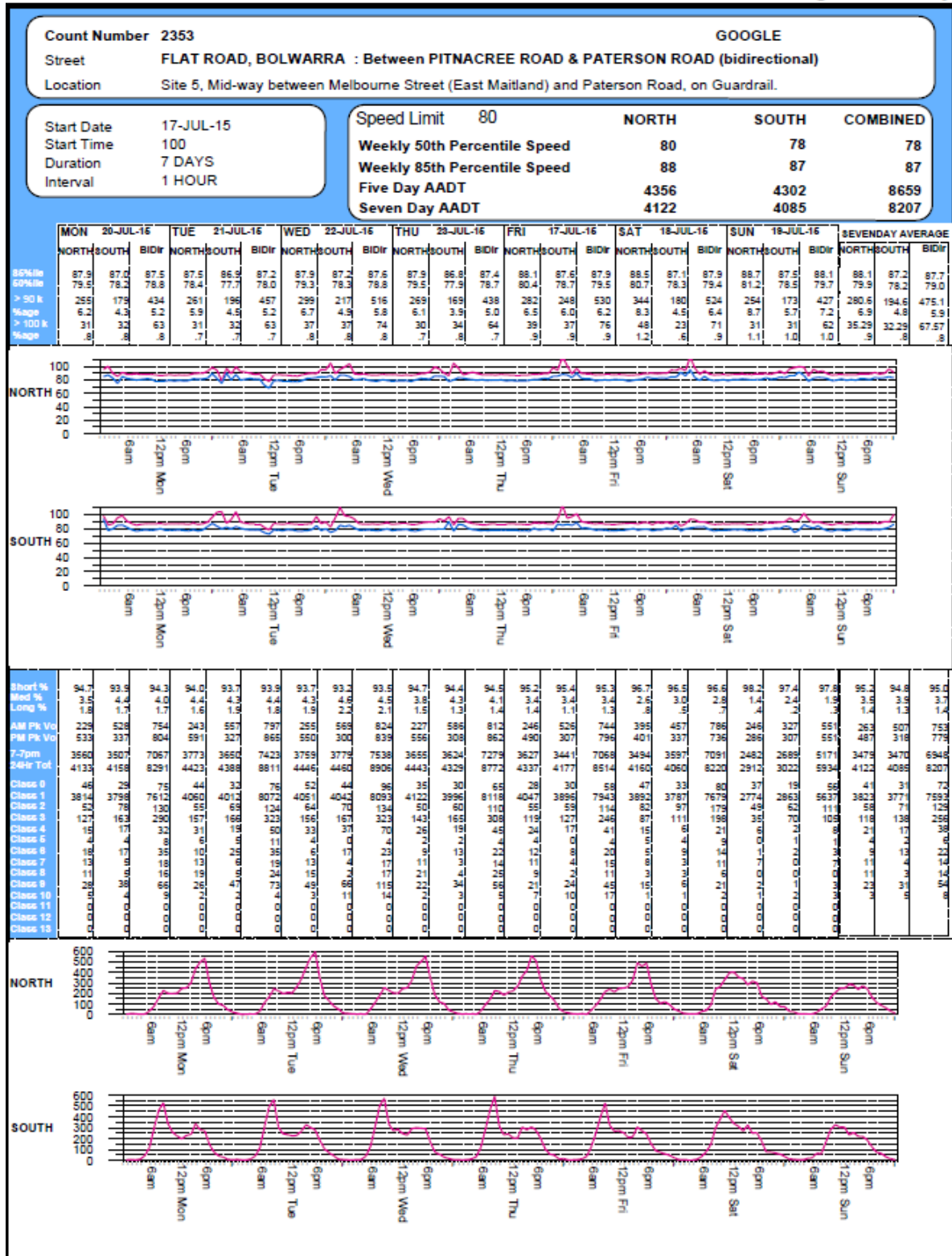
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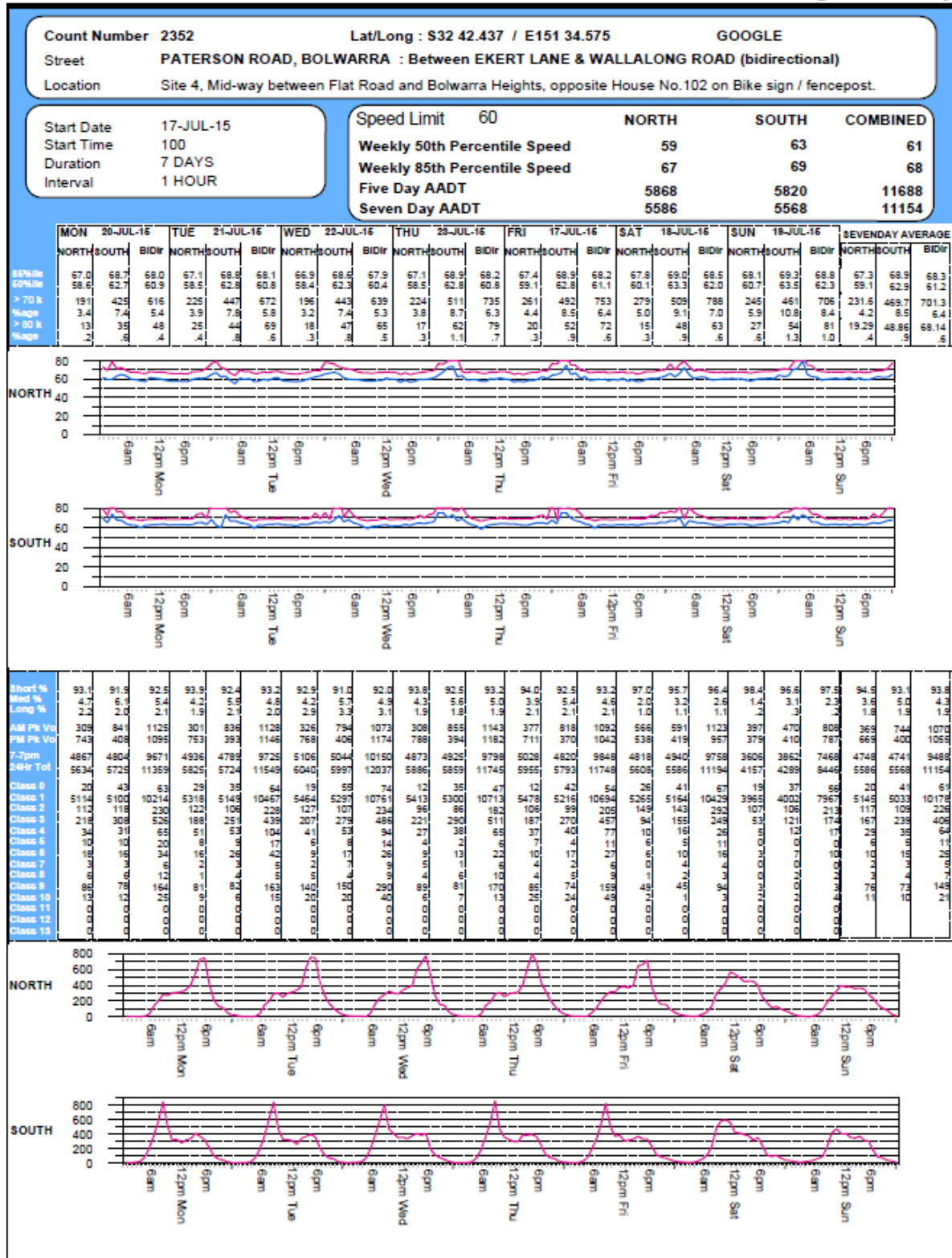


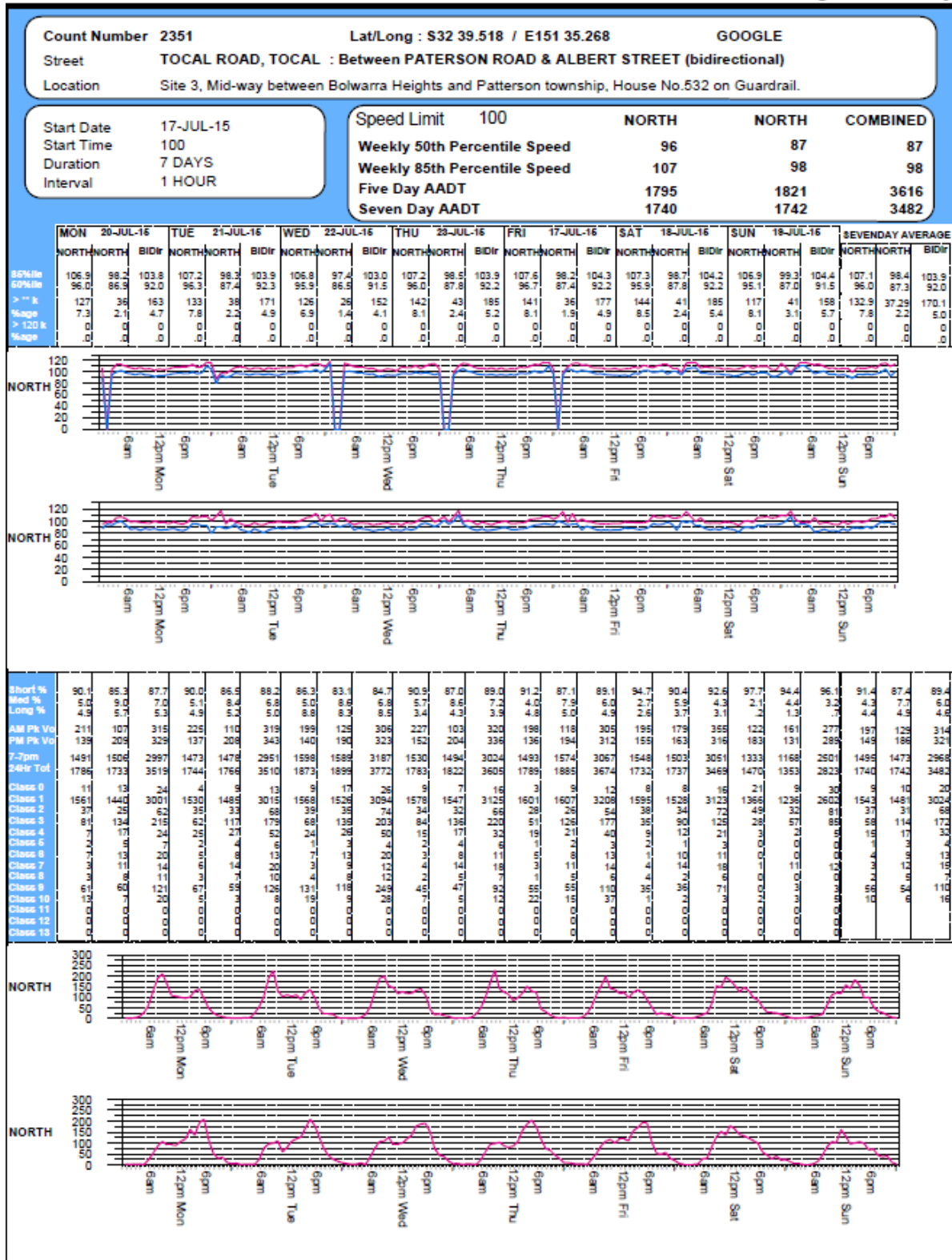




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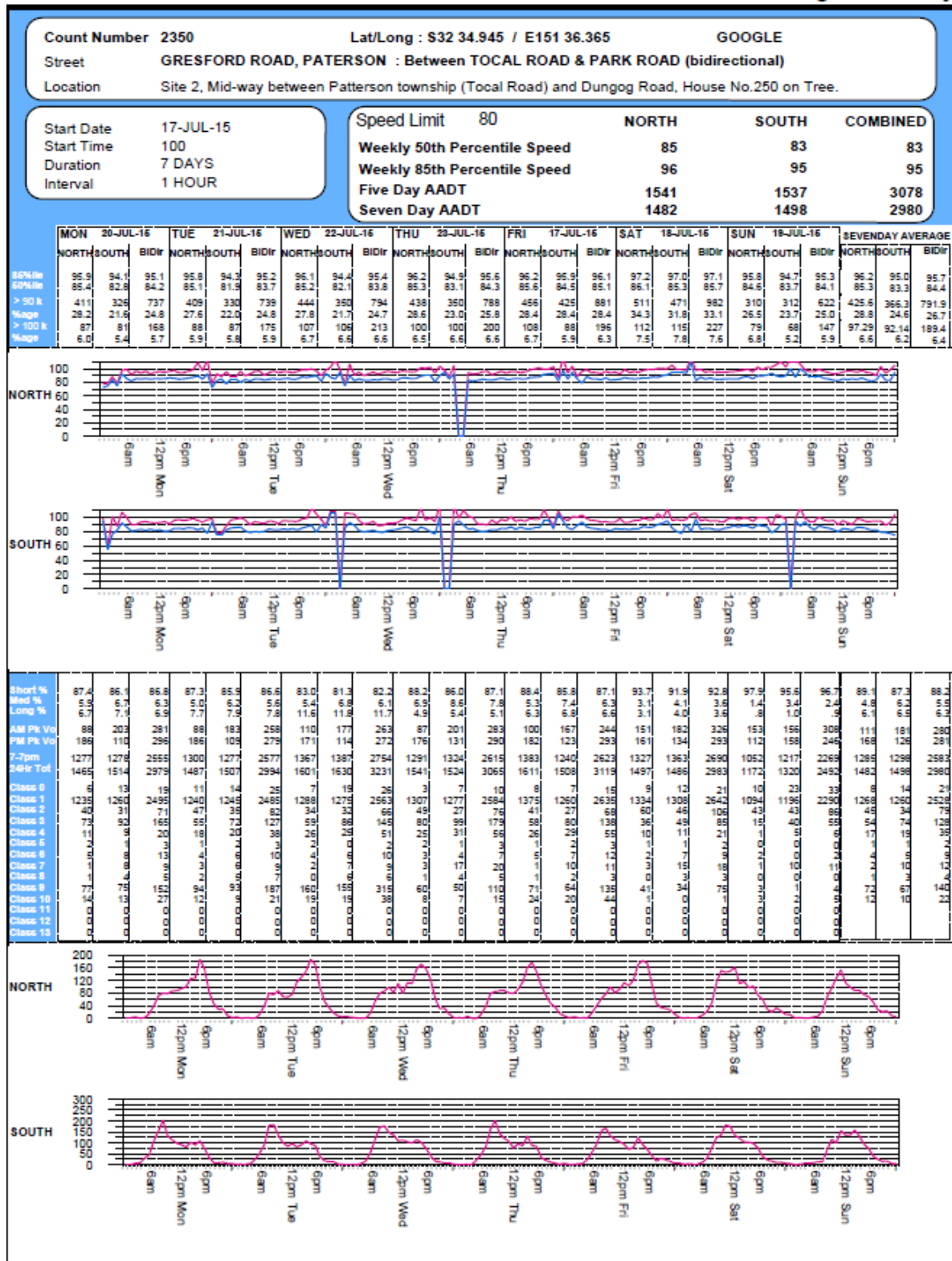
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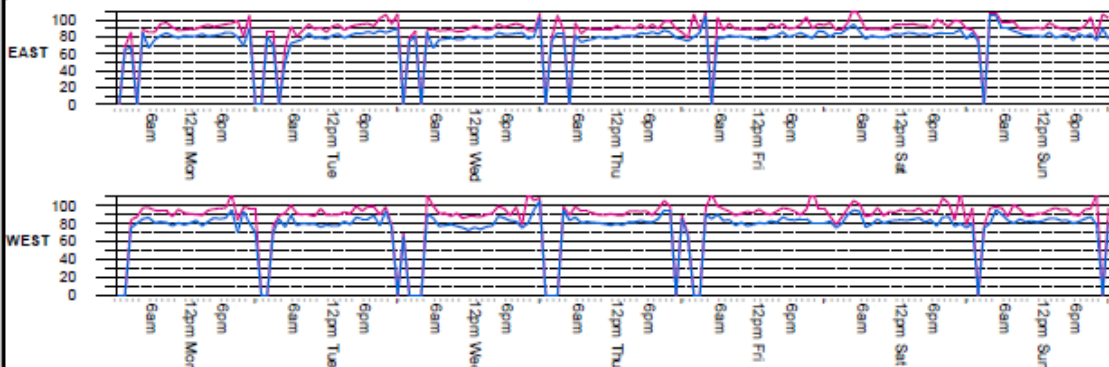


Count Number 2349 Lat/Long : S32 33.615 / E151 36.616 GOOGLE
 Street DUNGOG ROAD, MARTINS CREEK : Between GRESFORD ROAD & CLARENCE TOWN ROAD (bidirectic)
 Location Site 1, Mid-way between Gresford Road and Cory Street, on Tree.

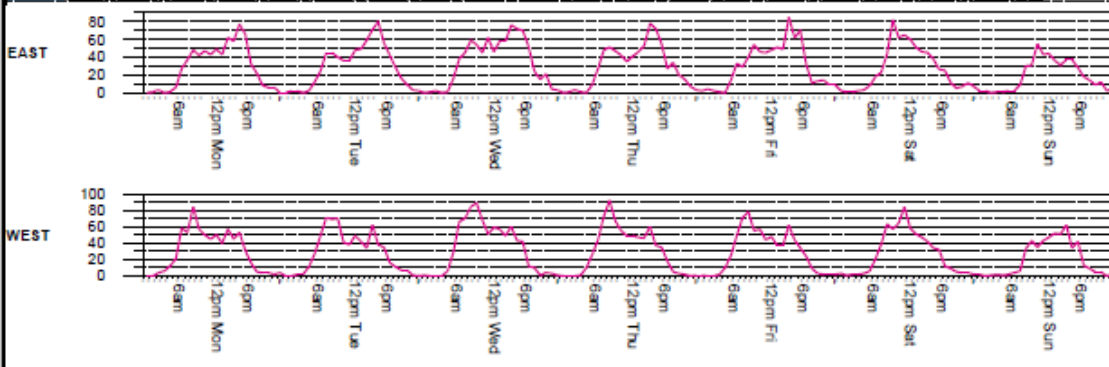
Start Date 17-JUL-15
 Start Time 100
 Duration 7 DAYS
 Interval 1 HOUR

Speed Limit 80
 Weekly 50th Percentile Speed EAST WEST COMBINED
 Weekly 85th Percentile Speed 81 81 81
 Five Day AADT 730 721 1451
 Seven Day AADT 678 677 1355

	MON 20-JUL-15			TUE 21-JUL-15			WED 22-JUL-15			THU 23-JUL-15			FRI 17-JUL-16			SAT 18-JUL-16			SUN 19-JUL-16			SEVENDAY AVERAGE		
	EAST	WEST	BIDir	EAST	WEST	BIDir	EAST	WEST	BIDir	EAST	WEST	BIDir	EAST	WEST	BIDir	EAST	WEST	BIDir	EAST	WEST	BIDir	EAST	WEST	BIDir
50%ile	93.3	93.8	93.1	92.2	92.6	92.4	89.7	91.5	90.3	90.7	92.1	91.4	91.5	94.2	93.0	93.2	94.3	93.8	90.6	93.7	92.5	91.5	93.2	92.4
50%ile	81.7	81.6	81.7	81.8	80.3	81.1	80.1	78.0	79.2	81.3	81.1	81.2	80.6	82.2	81.4	82.7	82.9	82.8	82.0	83.3	82.7	81.5	81.3	81.4
> 50 k	124	153	277	129	128	257	113	136	249	113	129	242	122	152	274	128	145	273	71	106	177	114.3	135.6	249.9
%age	18.1	21.8	20.0	18.3	18.8	18.5	13.8	17.0	15.4	15.9	18.0	17.0	16.9	21.9	19.3	19.9	22.5	21.2	15.8	21.4	18.7	16.9	20.2	18.6
> 100 k	29	28	57	24	28	52	30	29	59	20	25	45	31	38	69	30	33	63	17	19	36	25.86	28.57	54.43
%age	4.2	4.0	4.1	3.4	4.1	3.8	3.7	3.8	3.6	2.8	3.5	3.2	4.3	5.5	4.9	4.7	5.1	4.9	3.8	3.8	3.8	3.8	4.2	4.0



Short %	80.3	78.9	79.6	77.1	74.6	75.9	70.6	68.1	69.3	80.4	78.7	79.6	78.6	77.7	78.1	89.1	87.8	88.4	97.6	96.6	97.0	80.7	79.3	80.0
Med %	6.6	8.7	7.7	8.6	9.8	9.2	9.7	9.9	9.8	9.4	12.4	10.9	9.5	10.2	9.9	3.5	6.0	5.0	1.5	3.0	2.3	7.5	8.8	8.8
Long %	13.2	12.4	12.8	14.3	15.5	14.9	19.7	22.0	20.8	10.2	8.9	9.6	11.9	12.1	12.0	7.0	6.2	6.6	9.9	7.4	9.9	11.8	11.8	11.8
AM Pk Vol	48	84	132	45	71	116	62	89	143	51	92	142	54	78	118	82	84	148	55	43	90	57	77	123
PM Pk Vol	77	57	130	80	62	133	76	60	133	79	60	138	85	62	147	60	58	118	44	62	89	71	60	128
7-9pm	604	581	1185	607	565	1172	697	684	1381	595	627	1222	611	591	1202	567	583	1150	399	451	860	593	588	1167
24hr Tot	684	701	1385	707	682	1389	822	805	1627	715	719	1434	723	696	1419	643	645	1288	451	494	945	678	677	1355
Class 0	1	9	8	13	4	13	13	13	13	2	2	2	2	2	2	3	1	1	10	10	16	8	8	104
Class 1	528	512	1050	526	494	1020	563	520	1087	554	544	1098	557	526	1083	551	543	1054	423	453	876	525	518	1044
Class 2	20	46	46	11	10	21	13	19	28	20	20	40	11	13	24	19	22	41	11	14	25	19	17	32
Class 3	37	63	93	43	53	96	55	60	115	41	60	101	44	55	99	18	34	52	6	14	20	39	47	82
Class 4	8	8	11	17	13	30	25	18	43	25	28	53	24	16	40	6	9	11	1	1	1	1	1	1
Class 5	8	8	11	17	13	30	25	18	43	25	28	53	24	16	40	6	9	11	1	1	1	1	1	1
Class 6	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3	3
Class 7	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12	12
Class 8	11	11	11	11	11	11	11	11	11	11	11	11	11	11	11	11	11	11	11	11	11	11	11	11
Class 9	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Class 10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Class 11	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Class 12	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Class 13	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0



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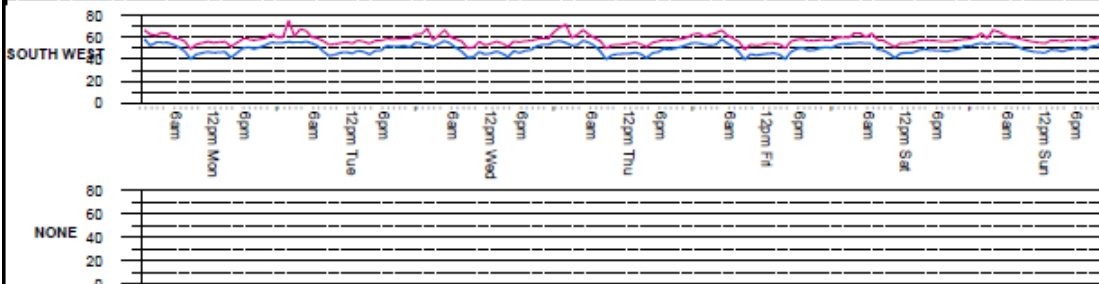
One Page Summary

Count Number 1263 Lat/Long : S32.74602 / E151.58285 UBD 51 A-5
 Street MELBOURNE STREET, EAST MAITLAND : Between WALLIS STREET & JOHN STREET (bidirectional)
 Location Combined Counts 1261 and 1262 Southwest, north of Tyrell Street

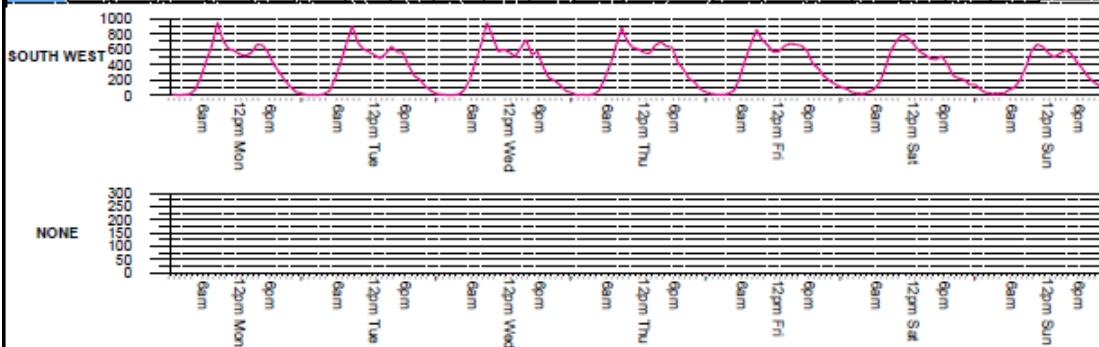
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 Start Time 100
 Duration 7 DAYS
 Interval 1 HOUR

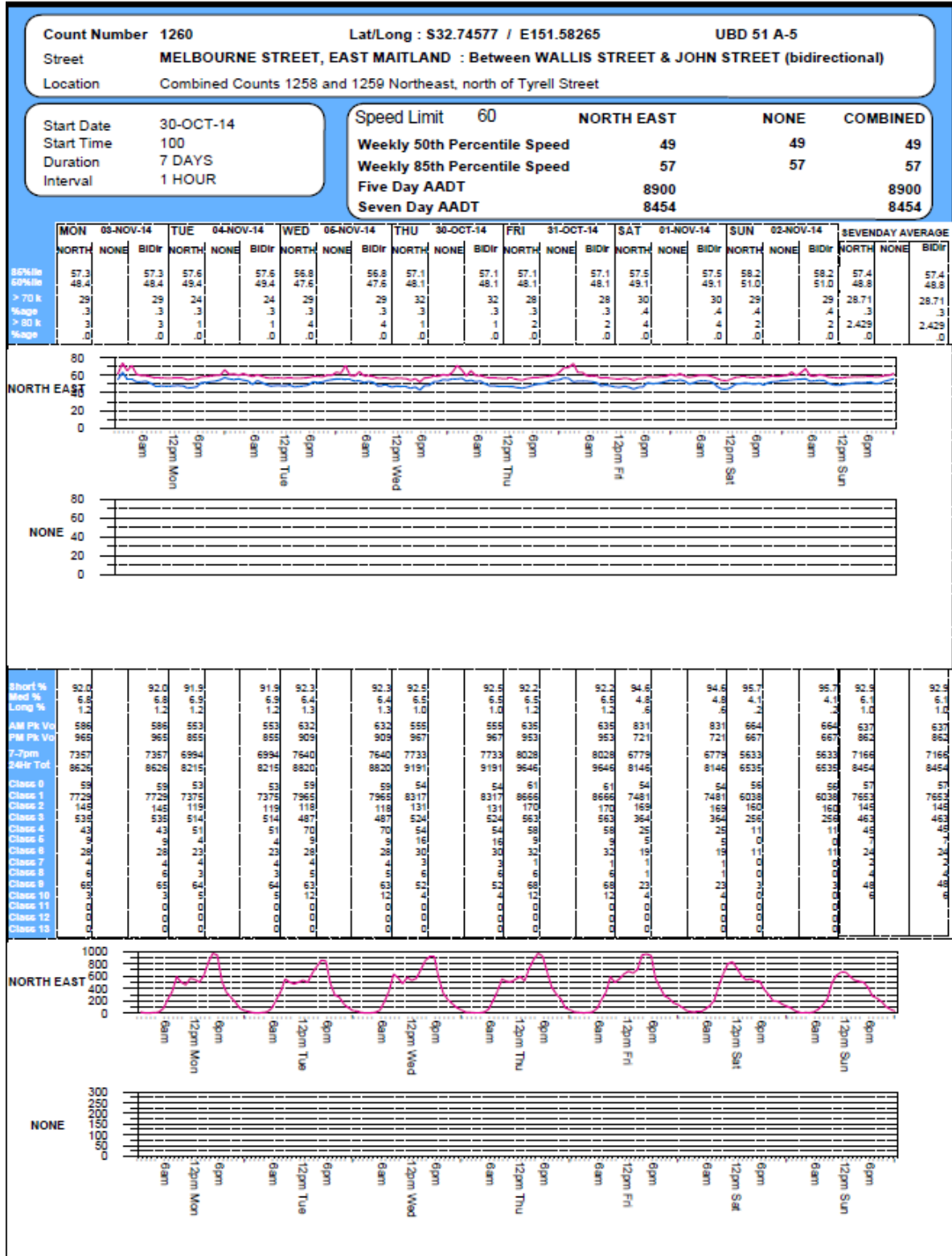
Speed Limit 60 SOUTH WEST NONE COMBINED
 Weekly 50th Percentile Speed 47 47 47
 Weekly 85th Percentile Speed 56 56 56
 Five Day AADT 9007 9007
 Seven Day AADT 8617 8617

	MON 03-NOV-14	TUE 04-NOV-14	WED 05-NOV-14	THU 06-NOV-14	FRI 07-NOV-14	SAT 08-NOV-14	SUN 09-NOV-14	SEVENDAY AVERAGE
	SOUTH NONE BIDIR	SOUTH NONE BIDIR	SOUTH NONE BIDIR	SOUTH NONE BIDIR	SOUTH NONE BIDIR	SOUTH NONE BIDIR	SOUTH NONE BIDIR	SOUTH NONE BIDIR
55thile	56.3	56.3	56.8	55.8	55.8	56.5	57.3	56.4
60thile	46.5	46.5	47.2	46.0	46.0	47.1	48.5	46.7
> 70 k	35	35	42	39	39	25	35	34.43
%age	4	4	5	4	4	3	5	4
> 80 k	5	5	8	6	4	3	5	4.571
%age	1	1	1	1	0	1	0	1



Short %	92.1	92.1	92.7	92.7	92.4	93.3	92.4	92.4	92.4	95.0	95.8	95.8	95.8	93.3	93.3
Med %	6.8	6.8	6.1	6.1	6.3	5.8	6.4	6.4	6.4	4.5	4.5	4.0	4.0	5.8	5.8
Long %	1.2	1.2	1.2	1.2	1.3	1.3	1.2	1.2	1.2	0.5	0.5	1.0	1.0	0.9	0.9
AM Pk Vo	942	942	895	895	938	875	854	854	854	796	796	663	663	852	852
PM Pk Vo	660	660	633	633	720	692	672	672	672	670	670	590	590	662	662
7-7pm	7359	7359	7129	7129	7419	7581	7581	7581	7581	6833	6833	5978	5978	7135	7135
24hr Tot	8850	8850	8583	8583	8872	9193	9193	9193	9193	9537	9537	8291	8291	8617	8617
Class 0	50	50	75	75	51	73	73	73	73	50	50	30	30	58	58
Class 1	7959	7959	7759	7759	7999	8377	8377	8377	8377	8604	8604	7652	7652	7832	7832
Class 2	144	144	135	135	151	126	126	126	126	155	155	158	158	148	148
Class 3	519	519	451	451	473	463	463	463	463	534	534	337	337	433	433
Class 4	71	71	66	66	78	62	62	62	62	74	74	26	26	57	57
Class 5	11	11	4	4	6	8	8	8	8	13	13	7	7	7	7
Class 6	37	37	23	23	25	25	25	25	25	35	35	17	17	25	25
Class 7	9	9	8	8	12	12	12	12	12	9	9	11	11	9	9
Class 8	5	5	5	5	5	4	4	4	4	3	3	0	0	4	4
Class 9	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Class 10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Class 11	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Class 12	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Class 13	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0





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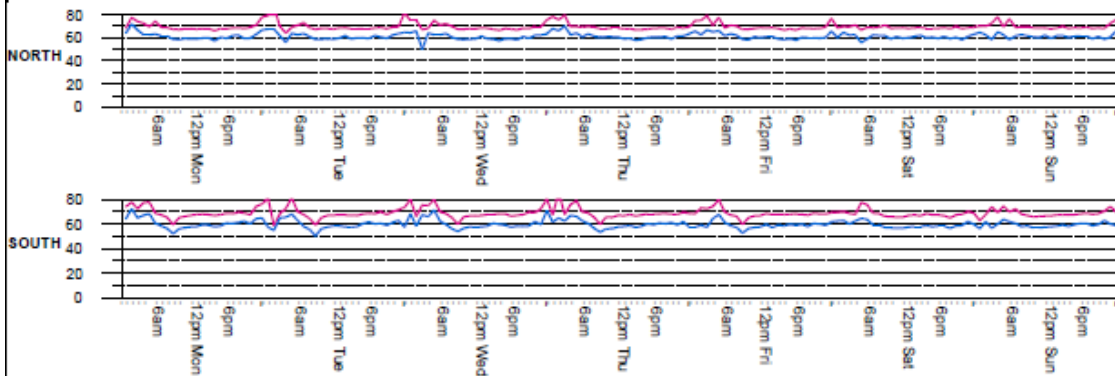
One Page Summary

Count Number 1257 Lat/Long : S32.74250 / E151.58637 GOOGLE
 Street PITNACREE ROAD, EAST MAITLAND : Between MELBOURNE STREET & GLENARVON ROAD (bidirect)
 Location At the railway crossing, On Guardrail.

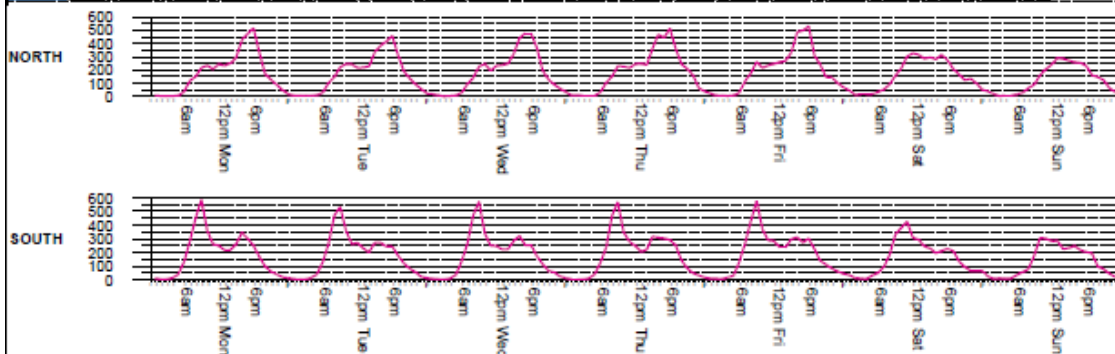
Start Date 30-OCT-14
 Start Time 1700
 Duration 7 DAYS
 Interval 1 HOUR

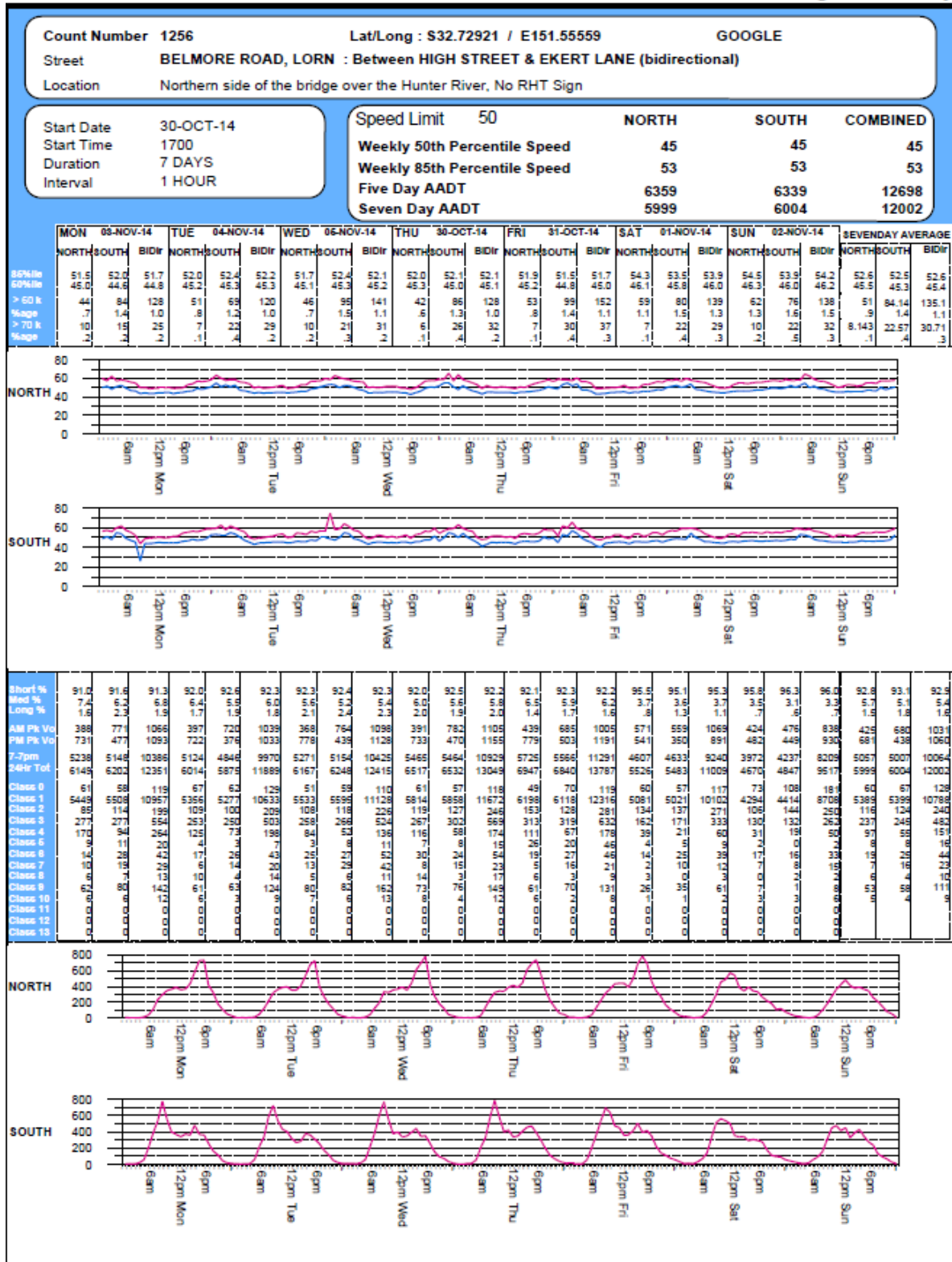
Speed Limit 60
 Weekly 50th Percentile Speed 60 NORTH SOUTH COMBINED
 Weekly 85th Percentile Speed 68 58 59
 Five Day AADT 4401 4465 8866
 Seven Day AADT 4131 4199 8329

	MON 03-NOV-14	TUE 04-NOV-14	WED 05-NOV-14	THU 06-NOV-14	FRI 07-NOV-14	SAT 08-NOV-14	SUN 09-NOV-14	SEVEN DAY AVERAGE
85thile	67.9	67.1	67.6	68.2	67.2	67.8	67.9	67.8
50thile	59.7	57.6	58.6	60.1	57.9	58.7	59.2	58.8
> 70 k	257	246	503	300	259	559	292	256
%age	6.1	5.6	5.8	7.3	6.0	6.6	6.8	6.3
> 80 k	24	26	50	33	29	62	25	30
%age	.6	.6	.6	.8	.7	.6	.6	.6



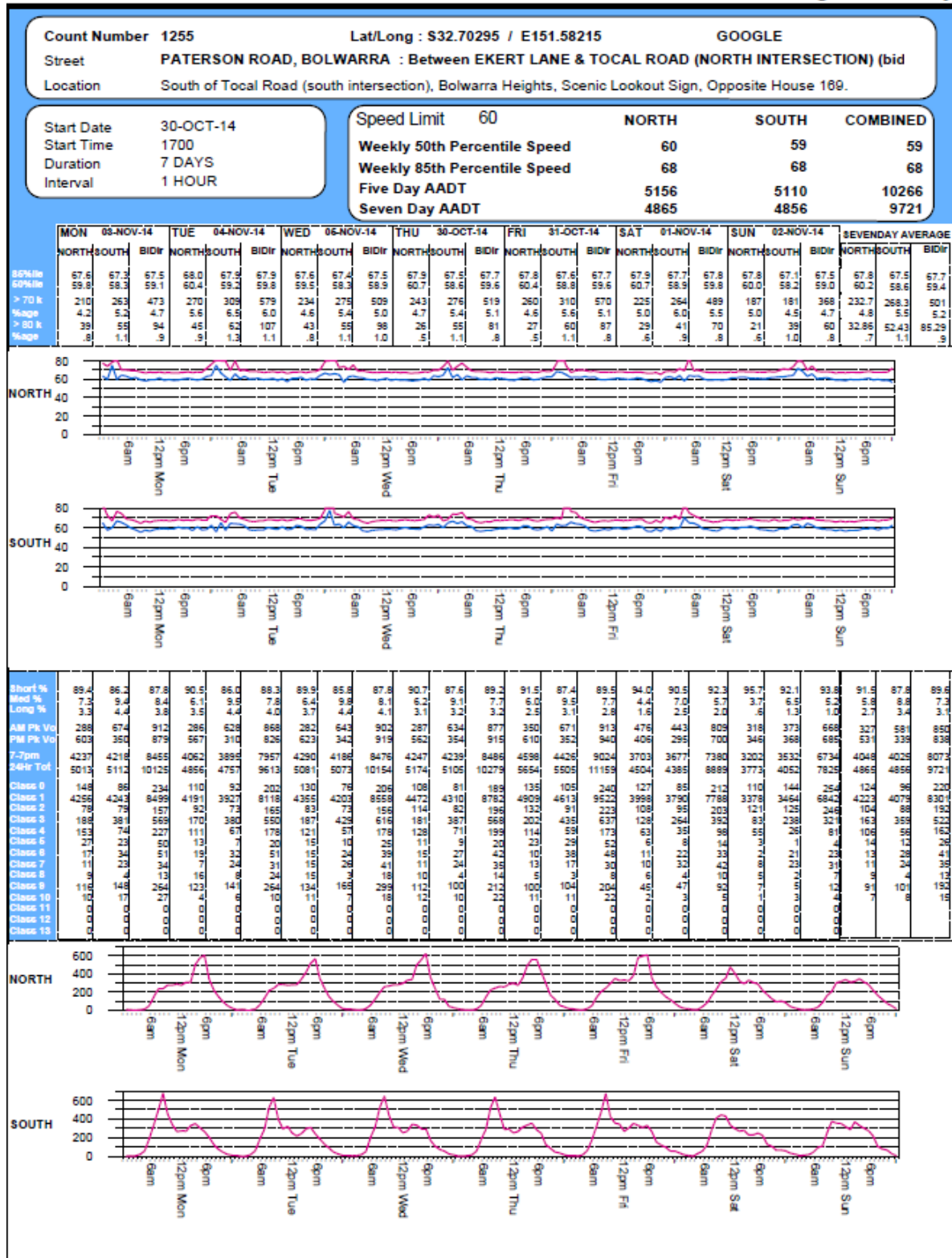
Short %	93.3	93.3	93.3	93.3	93.3	93.3	93.3	93.3
Long %	6.7	6.7	6.7	6.7	6.7	6.7	6.7	6.7
Avg Pk Vd	244	244	244	244	244	244	244	244
Pk Pk Vd	516	344	773	457	274	457	274	457
7:30pm	3581	3665	7246	3432	3535	6967	3614	3609
30m Tot	4232	4385	8617	4119	4292	8411	4303	4344
Class 0	51	51	102	46	50	96	47	52
Class 1	3833	3960	7793	3774	3881	7661	3907	3912
Class 2	63	71	134	56	72	128	61	69
Class 3	137	174	308	104	129	229	127	160
Class 4	28	28	54	15	25	40	35	38
Class 5	9	9	18	5	9	15	10	11
Class 6	1	1	2	1	1	2	1	1
Class 7	1	1	1	1	1	1	1	1
Class 8	1	1	1	1	1	1	1	1
Class 9	1	1	1	1	1	1	1	1
Class 10	1	1	1	1	1	1	1	1
Class 11	1	1	1	1	1	1	1	1
Class 12	1	1	1	1	1	1	1	1
Class 13	1	1	1	1	1	1	1	1

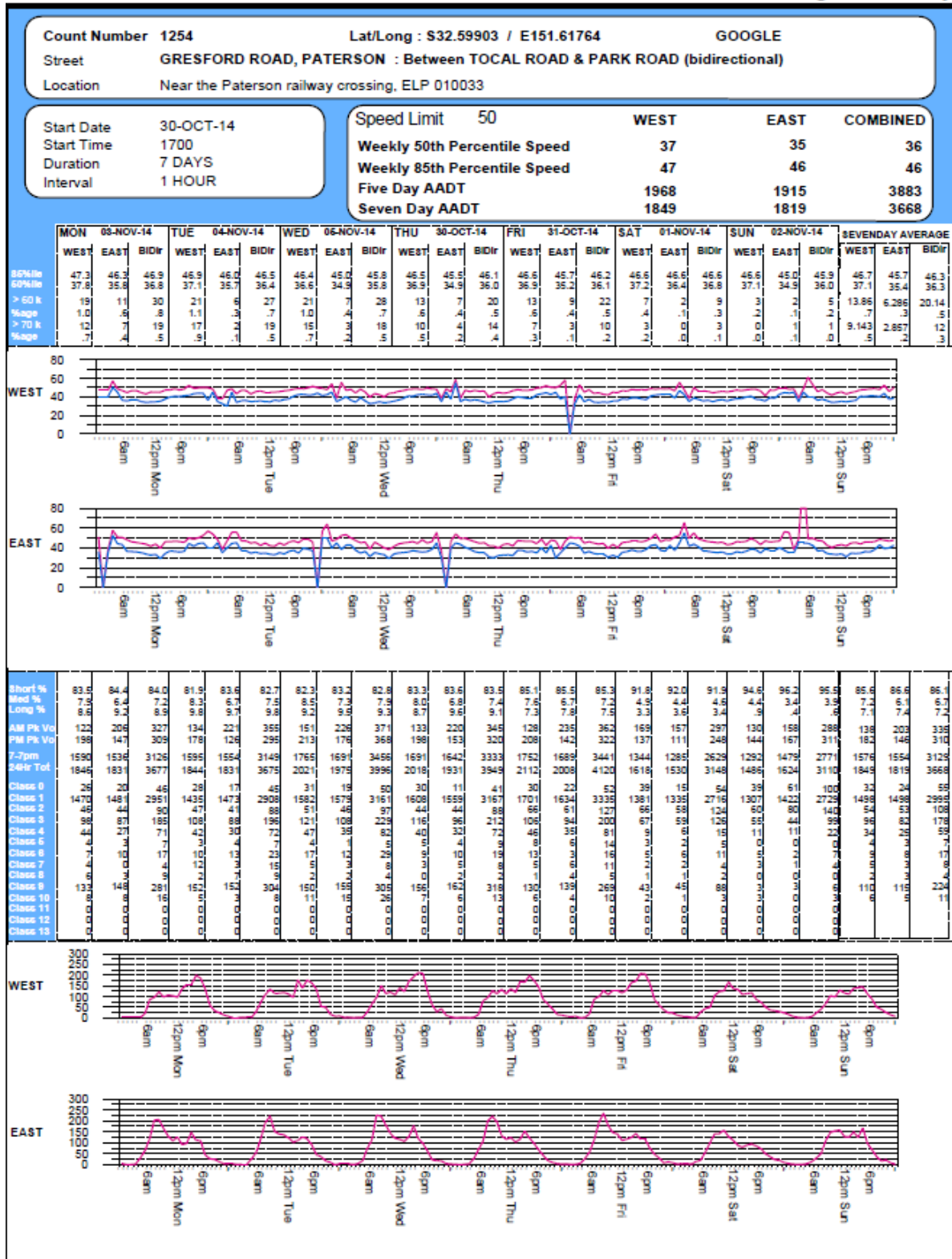




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One Page Summary





Appendix C – Sidra output

INTERSECTION SUMMARY



Site: NEH and Melbourne St AM base

AM existing flows

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Intersection Performance - Hourly Values				
Performance Measure	Vehicles	Pedestrians	Persons	
Travel Speed (Average)	30.0km/h	2.0km/h	29.7km/h	
Travel Distance (Total)	4361.6veh-km/h	3.3ped-km/h	5237.3pers-km/h	
Travel Time (Total)	145.3veh-h/h	1.7ped-h/h	176.1pers-h/h	
Demand Flows (Total)	4526veh/h	84ped/h	5432pers/h	
Percent Heavy Vehicles (Demand)	6.7%			
Degree of Saturation	0.967	0.035		
Practical Spare Capacity	-7.0%			
Effective Intersection Capacity	4679veh/h			
Control Delay (Total)	71.71veh-h/h	0.99ped-h/h	87.04pers-h/h	
Control Delay (Average)	57.0sec	42.4sec	57.7sec	
Control Delay (Worst Lane)	81.9sec			
Control Delay (Worst Movement)	81.8sec	54.2sec	81.8sec	
Geometric Delay (Average)	2.4sec			
Stop-Line Delay (Average)	54.6sec			
Idling Time (Average)	48.9sec			
Intersection Level of Service (LOS)	LOS E	LOS E		
95% Back of Queue - Vehicles (Worst Lane)	52.9veh			
95% Back of Queue - Distance (Worst Lane)	394.8m			
Queue Storage Ratio (Worst Lane)	0.50			
Total Effective Stops	4554veh/h	70ped/h	5535pers/h	
Effective Stop Rate	1.01per veh	0.83per ped	1.02per pers	
Proportion Queued	0.93	0.83	0.95	
Performance Index	484.3	2.1	486.4	
Cost (Total)	4724.33\$/h	41.97\$/h	4766.30\$/h	
Fuel Consumption (Total)	583.3L/h			
Carbon Dioxide (Total)	1391.3kg/h			
Hydrocarbons (Total)	0.130kg/h			
Carbon Monoxide (Total)	1.447kg/h			
NOx (Total)	3.459kg/h			

Level of Service (LOS) Method: Delay (RTA NSW).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

MOVEMENT SUMMARY

Site: NEH and Melbourne St AM base

AM existing flows

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Movement Performance - Vehicles

Mov ID	ODMo v	Demand Flows Total	Deg. Satn HV	Average Delay	Level of Service	95% Back of Queue Vehicles	Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed	
		veh/h	%	sec		veh	m		per veh	km/h	
South: NEH Newcastle											
1	L2	33	3.0	0.967	78.9	LOS F	52.9	394.8	1.00	1.20	26.9
2	T1	1277	8.0	0.967	73.3	LOS F	52.9	394.8	1.00	1.21	27.3
3	R2	123	5.0	0.458	66.0	LOS E	3.6	26.6	1.00	0.76	26.1
Approach		1433	7.6	0.967	72.8	LOS F	52.9	394.8	1.00	1.17	27.2
East: Melbourne St Morpeth											
4	L2	191	8.0	0.296	34.5	LOS C	7.9	58.9	0.75	0.77	35.2
5	T1	78	2.0	0.952	75.9	LOS F	28.4	210.3	1.00	1.10	23.7
6	R2	706	8.0	0.952	81.8	LOS F	30.6	228.8	1.00	1.08	23.3
Approach		975	7.5	0.952	72.1	LOS F	30.6	228.8	0.95	1.02	24.9
North: NEH Maitland											
7	L2	449	4.0	0.404	12.0	LOS A	7.4	53.6	0.55	0.74	47.4
8	T1	1086	8.0	0.828	40.3	LOS C	32.5	243.1	0.95	0.91	36.2
9	R2	123	3.0	0.903	78.8	LOS F	8.4	60.1	1.00	0.99	25.8
Approach		1659	6.5	0.903	35.5	LOS C	32.5	243.1	0.85	0.87	37.1
West: Melbourne St											
10	L2	271	3.0	0.812	37.6	LOS C	10.2	73.6	1.00	0.89	36.4
11	T1	129	3.0	0.929	75.5	LOS F	13.3	95.7	1.00	1.07	24.2
12	R2	60	3.0	0.929	81.1	LOS F	13.3	95.7	1.00	1.07	26.3
Approach		460	3.0	0.929	53.9	LOS D	13.3	95.7	1.00	0.97	30.9
All Vehicles		4526	6.7	0.967	57.0	LOS E	52.9	394.8	0.93	1.01	30.0

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians

Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back of Queue		Prop. Queued	Effective Stop Rate per ped
					Pedestrian ped	Distance m		
P1	South Full Crossing	21	54.2	LOS E	0.1	0.1	0.95	0.95
P2	East Full Crossing	21	32.3	LOS D	0.1	0.1	0.73	0.73
P3	North Full Crossing	21	54.2	LOS E	0.1	0.1	0.95	0.95
P4	West Full Crossing	21	28.7	LOS C	0.0	0.0	0.69	0.69
All Pedestrians		84	42.4	LOS E			0.83	0.83

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Project: C:\Sidra folders\Martins Creek.sip6

INTERSECTION SUMMARY



Site: Melbourne St and Pitnacree Rd AM

Melbourne and Pitnacree AM base

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Intersection Performance - Hourly Values			
Performance Measure	Vehicles	Pedestrians	Persons
Travel Speed (Average)	35.1 km/h	1.9 km/h	34.5 km/h
Travel Distance (Total)	2366.5 veh-km/h	2.8 ped-km/h	2842.6 pers-km/h
Travel Time (Total)	67.5 veh-h/h	1.5 ped-h/h	82.4 pers-h/h
Demand Flows (Total)	2333 veh/h	84 ped/h	2799 pers/h
Percent Heavy Vehicles (Demand)	4.0%		
Degree of Saturation	0.814	0.035	
Practical Spare Capacity	10.5%		
Effective Intersection Capacity	2865 veh/h		
Control Delay (Total)	27.50 veh-h/h	0.86 ped-h/h	33.86 pers-h/h
Control Delay (Average)	42.4 sec	36.8 sec	43.5 sec
Control Delay (Worst Lane)	70.5 sec		
Control Delay (Worst Movement)	70.5 sec	54.2 sec	70.5 sec
Geometric Delay (Average)	2.3 sec		
Stop-Line Delay (Average)	40.1 sec		
Idling Time (Average)	35.7 sec		
Intersection Level of Service (LOS)	LOS C	LOS D	
95% Back of Queue - Vehicles (Worst Lane)	30.1 veh		
95% Back of Queue - Distance (Worst Lane)	217.8 m		
Queue Storage Ratio (Worst Lane)	0.31		
Total Effective Stops	1950 veh/h	65 ped/h	2405 pers/h
Effective Stop Rate	0.84 per veh	0.77 per ped	0.86 per pers
Proportion Queued	0.92	0.77	0.94
Performance Index	296.1	1.8	298.0
Cost (Total)	2073.12 \$/h	36.08 \$/h	2109.20 \$/h
Fuel Consumption (Total)	260.6 L/h		
Carbon Dioxide (Total)	618.4 kg/h		
Hydrocarbons (Total)	0.055 kg/h		
Carbon Monoxide (Total)	0.665 kg/h		
NOx (Total)	1.052 kg/h		

Level of Service (LOS) Method: Delay (RTA NSW).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

MOVEMENT SUMMARY



Site: Melbourne St and Pitnacree Rd AM

Melbourne and Pitnacree AM base

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Movement Performance - Vehicles

Mov ID	ODMo v	Demand Flows Total	Deg. Satn HV	Average Delay	Level of Service	95% Back of Queue Vehicles	Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	sec		veh	m		per veh	km/h
South: Melbourne St										
1	L2	131	8.0	0.178	29.1	LOS C	4.8	35.6	0.67	39.7
2	T1	498	4.0	0.708	29.5	LOS C	22.9	165.5	0.85	40.4
3	R2	91	2.0	0.742	70.5	LOS E	5.7	40.4	1.00	27.4
Approach		719	4.5	0.742	34.6	LOS C	22.9	165.5	0.83	38.0
East: Lawes Street										
4	L2	231	2.0	0.795	38.6	LOS C	9.1	64.9	1.00	36.1
5	T1	92	4.0	0.783	63.2	LOS E	8.4	60.5	1.00	29.0
6	R2	43	2.0	0.783	68.7	LOS E	8.4	60.5	1.00	28.6
Approach		365	2.5	0.795	48.3	LOS D	9.1	64.9	1.00	33.1
North: Melbourne St										
7	L2	18	2.0	0.806	39.7	LOS C	30.1	217.8	0.91	37.7
8	T1	568	4.0	0.806	34.1	LOS C	30.1	217.8	0.91	38.4
9	R2	73	2.0	0.595	68.1	LOS E	4.4	31.4	1.00	27.9
Approach		659	3.7	0.806	38.0	LOS C	30.1	217.8	0.92	36.8
West: Pitnacree Rd										
10	L2	143	2.0	0.814	58.0	LOS E	20.7	147.1	1.00	31.1
11	T1	202	2.0	0.814	52.5	LOS D	20.7	147.1	1.00	31.6
12	R2	244	8.0	0.618	51.3	LOS D	13.0	97.5	0.96	32.0
Approach		589	4.5	0.814	53.3	LOS D	20.7	147.1	0.98	31.7
All Vehicles		2333	4.0	0.814	42.4	LOS C	30.1	217.8	0.92	35.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians

Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back of Queue		Prop. Queued	Effective Stop Rate per ped
					Pedestrian ped	Distance m		
P1	South Full Crossing	21	54.2	LOS E	0.1	0.1	0.95	0.95
P2	East Full Crossing	21	24.7	LOS C	0.0	0.0	0.64	0.64
P3	North Full Crossing	21	43.4	LOS E	0.1	0.1	0.85	0.85
P4	West Full Crossing	21	24.7	LOS C	0.0	0.0	0.64	0.64
All Pedestrians		84	36.8	LOS D			0.77	0.77

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Organisation: SECA SOLUTION | Processed: Friday, 20 November 2015 2:39:20 PM

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INTERSECTION SUMMARY



Site: Melbourne St and Pitnacree Rd PM

Melbourne and Pitnacree AM base

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Intersection Performance - Hourly Values			
Performance Measure	Vehicles	Pedestrians	Persons
Travel Speed (Average)	34.7km/h	1.9km/h	34.1 km/h
Travel Distance (Total)	2459.3veh-km/h	2.8ped-km/h	2953.9pers-km/h
Travel Time (Total)	70.9veh-h/h	1.5ped-h/h	86.6pers-h/h
Demand Flows (Total)	2424veh/h	84ped/h	2909pers/h
Percent Heavy Vehicles (Demand)	4.3%		
Degree of Saturation	0.900	0.035	
Practical Spare Capacity	0.0%		
Effective Intersection Capacity	2692veh/h		
Control Delay (Total)	29.37veh-h/h	0.87ped-h/h	36.11pers-h/h
Control Delay (Average)	43.6sec	37.1sec	44.7sec
Control Delay (Worst Lane)	75.4sec		
Control Delay (Worst Movement)	75.4sec	54.2sec	75.4sec
Geometric Delay (Average)	2.3sec		
Stop-Line Delay (Average)	41.3sec		
Idling Time (Average)	36.7sec		
Intersection Level of Service (LOS)	LOS D	LOS D	
95% Back of Queue - Vehicles (Worst Lane)	39.6veh		
95% Back of Queue - Distance (Worst Lane)	286.8m		
Queue Storage Ratio (Worst Lane)	0.54		
Total Effective Stops	2073veh/h	65ped/h	2552pers/h
Effective Stop Rate	0.85per veh	0.77per ped	0.88per pers
Proportion Queued	0.88	0.77	0.90
Performance Index	294.4	1.8	296.2
Cost (Total)	2185.99\$/h	36.30\$/h	2222.29\$/h
Fuel Consumption (Total)	275.0L/h		
Carbon Dioxide (Total)	652.8kg/h		
Hydrocarbons (Total)	0.059kg/h		
Carbon Monoxide (Total)	0.697kg/h		
NOx (Total)	1.168kg/h		

Level of Service (LOS) Method: Delay (RTA NSW).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

MOVEMENT SUMMARY



Site: Melbourne St and Pitnacree Rd PM

Melbourne and Pitnacree AM base

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Movement Performance - Vehicles

Mov ID	ODMo v	Demand Flows Total	Deg. Satn HV	Average Delay	Level of Service	95% Back of Queue Vehicles	Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed	
		veh/h	%	sec		veh	m		per veh	km/h	
South: Melbourne St											
1	L2	343	8.0	0.419	28.2	LOS B	13.2	98.6	0.71	0.79	40.1
2	T1	700	4.0	0.877	38.2	LOS C	39.6	286.8	0.91	0.92	36.8
3	R2	105	2.0	0.862	75.4	LOS F	6.9	49.4	1.00	0.94	26.4
Approach		1148	5.0	0.877	38.6	LOS C	39.6	286.8	0.86	0.88	36.4
East: Lawes Street											
4	L2	158	2.0	0.450	33.5	LOS C	5.9	42.2	0.93	0.79	38.0
5	T1	166	4.0	0.866	65.2	LOS E	13.3	95.7	1.00	0.99	28.8
6	R2	38	2.0	0.866	70.7	LOS F	13.3	95.7	1.00	0.99	28.3
Approach		362	2.9	0.866	51.9	LOS D	13.3	95.7	0.97	0.90	32.1
North: Melbourne St											
7	L2	17	2.0	0.564	29.1	LOS C	17.2	124.7	0.74	0.66	42.3
8	T1	415	4.0	0.564	23.5	LOS B	17.2	124.7	0.74	0.66	43.2
9	R2	94	2.0	0.767	71.1	LOS F	5.9	42.1	1.00	0.86	27.2
Approach		525	3.6	0.767	32.2	LOS C	17.2	124.7	0.79	0.69	39.1
West: Pitnacree Rd											
10	L2	87	2.0	0.900	73.8	LOS F	16.3	115.8	1.00	1.04	27.5
11	T1	154	2.0	0.900	68.3	LOS E	16.3	115.8	1.00	1.04	27.9
12	R2	147	8.0	0.592	59.1	LOS E	8.3	62.3	0.99	0.81	30.0
Approach		388	4.3	0.900	66.1	LOS E	16.3	115.8	0.99	0.95	28.5
All Vehicles		2424	4.3	0.900	43.6	LOS D	39.6	286.8	0.88	0.85	34.7

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians

Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back of Queue		Prop. Queued	Effective Stop Rate
					Pedestrian ped	Distance m		per ped
P1	South Full Crossing	21	54.2	LOS E	0.1	0.1	0.95	0.95
P2	East Full Crossing	21	21.0	LOS C	0.0	0.0	0.59	0.59
P3	North Full Crossing	21	52.3	LOS E	0.1	0.1	0.93	0.93
P4	West Full Crossing	21	21.0	LOS C	0.0	0.0	0.59	0.59
All Pedestrians		84	37.1	LOS D			0.77	0.77

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Project: C:\Sidra folders\Martins Creek.sip6

INTERSECTION SUMMARY



Site: Melbourne St and Pitnacree Rd AM 2026

Melbourne and Pitnacree AM base 2026 allowing for background growth

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Intersection Performance - Hourly Values			
Performance Measure	Vehicles	Pedestrians	Persons
Travel Speed (Average)	34.8km/h	1.9km/h	34.2km/h
Travel Distance (Total)	2419.8veh-km/h	2.8ped-km/h	2906.6pers-km/h
Travel Time (Total)	69.5veh-h/h	1.5ped-h/h	84.9pers-h/h
Demand Flows (Total)	2385veh/h	84ped/h	2862pers/h
Percent Heavy Vehicles (Demand)	4.0%		
Degree of Saturation	0.842	0.035	
Practical Spare Capacity	6.9%		
Effective Intersection Capacity	2833veh/h		
Control Delay (Total)	28.66veh-h/h	0.86ped-h/h	35.25pers-h/h
Control Delay (Average)	43.2sec	36.8sec	44.3sec
Control Delay (Worst Lane)	70.5sec		
Control Delay (Worst Movement)	70.5sec	54.2sec	70.5sec
Geometric Delay (Average)	2.3sec		
Stop-Line Delay (Average)	41.0sec		
Idling Time (Average)	36.4sec		
Intersection Level of Service (LOS)	LOS D	LOS D	
95% Back of Queue - Vehicles (Worst Lane)	33.7veh		
95% Back of Queue - Distance (Worst Lane)	243.7m		
Queue Storage Ratio (Worst Lane)	0.33		
Total Effective Stops	2032veh/h	65ped/h	2504pers/h
Effective Stop Rate	0.85per veh	0.77per ped	0.87per pers
Proportion Queued	0.93	0.77	0.95
Performance Index	309.3	1.8	311.1
Cost (Total)	2142.30\$/h	36.08\$/h	2178.38\$/h
Fuel Consumption (Total)	268.1L/h		
Carbon Dioxide (Total)	636.1kg/h		
Hydrocarbons (Total)	0.057kg/h		
Carbon Monoxide (Total)	0.683kg/h		
NOx (Total)	1.083kg/h		

Level of Service (LOS) Method: Delay (RTA NSW).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

MOVEMENT SUMMARY



Site: Melbourne St and Pitnacree Rd AM 2026

Melbourne and Pitnacree AM base 2026 allowing for background growth

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Movement Performance - Vehicles

Mov ID	ODMo v	Demand Flows Total veh/h	Deg. Satn HV %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: Melbourne St										
1	L2	131	8.0	0.178	LOS C	4.8	35.6	0.67	0.74	39.7
2	T1	522	4.0	0.739	LOS C	24.4	176.7	0.86	0.77	40.1
3	R2	91	2.0	0.742	LOS E	5.7	40.4	1.00	0.85	27.4
Approach		743	4.5	0.742	LOS C	24.4	176.7	0.85	0.77	37.9
East: Lawes Street										
4	L2	231	2.0	0.795	LOS C	9.1	64.9	1.00	0.88	36.1
5	T1	92	4.0	0.783	LOS E	8.4	60.5	1.00	0.89	29.0
6	R2	43	2.0	0.783	LOS E	8.4	60.5	1.00	0.89	28.6
Approach		365	2.5	0.795	LOS D	9.1	64.9	1.00	0.89	33.1
North: Melbourne St										
7	L2	18	2.0	0.842	LOS D	33.7	243.7	0.93	0.90	36.3
8	T1	597	4.0	0.842	LOS C	33.7	243.7	0.93	0.90	37.0
9	R2	73	2.0	0.595	LOS E	4.4	31.4	1.00	0.78	27.9
Approach		687	3.7	0.842	LOS C	33.7	243.7	0.94	0.89	35.7
West: Pitnacree Rd										
10	L2	143	2.0	0.814	LOS E	20.7	147.1	1.00	0.94	31.1
11	T1	202	2.0	0.814	LOS D	20.7	147.1	1.00	0.94	31.6
12	R2	244	8.0	0.618	LOS D	13.0	97.5	0.96	0.83	32.0
Approach		589	4.5	0.814	LOS D	20.7	147.1	0.98	0.89	31.7
All Vehicles		2385	4.0	0.842	LOS D	33.7	243.7	0.93	0.85	34.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians

Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back of Queue		Prop. Queued	Effective Stop Rate per ped
					Pedestrian ped	Distance m		
P1	South Full Crossing	21	54.2	LOS E	0.1	0.1	0.95	0.95
P2	East Full Crossing	21	24.7	LOS C	0.0	0.0	0.64	0.64
P3	North Full Crossing	21	43.4	LOS E	0.1	0.1	0.85	0.85
P4	West Full Crossing	21	24.7	LOS C	0.0	0.0	0.64	0.64
All Pedestrians		84	36.8	LOS D			0.77	0.77

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Project: C:\Sidra folders\Martins Creek.sip6

INTERSECTION SUMMARY



Site: Melbourne St and Pitnacree Rd PM 2026

Melbourne and Pitnacree AM base 2026 with allowance for background growth

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Intersection Performance - Hourly Values			
Performance Measure	Vehicles	Pedestrians	Persons
Travel Speed (Average)	33.9km/h	1.9km/h	33.4km/h
Travel Distance (Total)	2515.8veh-km/h	2.8ped-km/h	3021.8pers-km/h
Travel Time (Total)	74.2veh-h/h	1.5ped-h/h	90.5pers-h/h
Demand Flows (Total)	2480veh/h	84ped/h	2976pers/h
Percent Heavy Vehicles (Demand)	4.3%		
Degree of Saturation	0.916	0.035	
Practical Spare Capacity	-1.7%		
Effective Intersection Capacity	2708veh/h		
Control Delay (Total)	31.71veh-h/h	0.87ped-h/h	38.92pers-h/h
Control Delay (Average)	46.0sec	37.1sec	47.1sec
Control Delay (Worst Lane)	75.4sec		
Control Delay (Worst Movement)	75.4sec	54.2sec	75.4sec
Geometric Delay (Average)	2.2sec		
Stop-Line Delay (Average)	43.8sec		
Idling Time (Average)	39.0sec		
Intersection Level of Service (LOS)	LOS D	LOS D	
95% Back of Queue - Vehicles (Worst Lane)	46.5veh		
95% Back of Queue - Distance (Worst Lane)	336.7m		
Queue Storage Ratio (Worst Lane)	0.63		
Total Effective Stops	2190veh/h	65ped/h	2693pers/h
Effective Stop Rate	0.88per veh	0.77per ped	0.90per pers
Proportion Queued	0.89	0.77	0.91
Performance Index	309.5	1.8	311.3
Cost (Total)	2299.17\$/h	36.30\$/h	2335.47\$/h
Fuel Consumption (Total)	284.9L/h		
Carbon Dioxide (Total)	676.2kg/h		
Hydrocarbons (Total)	0.061kg/h		
Carbon Monoxide (Total)	0.720kg/h		
NOx (Total)	1.208kg/h		

Level of Service (LOS) Method: Delay (RTA NSW).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

MOVEMENT SUMMARY

Site: Melbourne St and Pitnacree Rd PM 2026

Melbourne and Pitnacree AM base 2026 with allowance for background growth

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Movement Performance - Vehicles

Mov ID	ODMo v	Demand Flows Total	Deg. Satn HV	Average Delay	Level of Service	95% Back of Queue Vehicles	Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	sec		veh	m		per veh	km/h
South: Melbourne St										
1	L2	343	8.0	0.419	28.2	LOS B	13.2	98.6	0.71	40.1
2	T1	735	4.0	0.916	47.0	LOS D	46.5	336.7	0.94	33.8
3	R2	105	2.0	0.862	75.4	LOS F	6.9	49.4	1.00	26.4
Approach		1183	5.0	0.916	44.1	LOS D	46.5	336.7	0.88	34.5
East: Lawes Street										
4	L2	158	2.0	0.450	33.5	LOS C	5.9	42.2	0.93	38.0
5	T1	166	4.0	0.866	65.2	LOS E	13.3	95.7	1.00	28.8
6	R2	38	2.0	0.866	70.7	LOS F	13.3	95.7	1.00	28.3
Approach		362	2.9	0.866	51.9	LOS D	13.3	95.7	0.97	32.1
North: Melbourne St										
7	L2	17	2.0	0.588	29.4	LOS C	18.3	132.7	0.75	42.1
8	T1	436	4.0	0.588	23.8	LOS B	18.3	132.7	0.75	43.0
9	R2	94	2.0	0.767	71.1	LOS F	5.9	42.1	1.00	27.2
Approach		546	3.6	0.767	32.1	LOS C	18.3	132.7	0.79	39.1
West: Pitnacree Rd										
10	L2	87	2.0	0.900	73.8	LOS F	16.3	115.8	1.00	27.5
11	T1	154	2.0	0.900	68.3	LOS E	16.3	115.8	1.00	27.9
12	R2	147	8.0	0.592	59.1	LOS E	8.3	62.3	0.99	30.0
Approach		388	4.3	0.900	66.1	LOS E	16.3	115.8	0.99	28.5
All Vehicles		2480	4.3	0.916	46.0	LOS D	46.5	336.7	0.89	33.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians

Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back of Queue		Prop. Queued	Effective Stop Rate per ped
					Pedestrian ped	Distance m		
P1	South Full Crossing	21	54.2	LOS E	0.1	0.1	0.95	0.95
P2	East Full Crossing	21	21.0	LOS C	0.0	0.0	0.59	0.59
P3	North Full Crossing	21	52.3	LOS E	0.1	0.1	0.93	0.93
P4	West Full Crossing	21	21.0	LOS C	0.0	0.0	0.59	0.59
All Pedestrians		84	37.1	LOS D			0.77	0.77

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Project: C:\Sidra folders\Martins Creek.sip6

INTERSECTION SUMMARY



Site: NEH and Melbourne St AM base 2026

AM existing flows allowing for background growth to 2026

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Intersection Performance - Hourly Values			
Performance Measure	Vehicles	Pedestrians	Persons
Travel Speed (Average)	23.3km/h	2.0km/h	23.2km/h
Travel Distance (Total)	4906.7veh-km/h	3.3ped-km/h	5891.4pers-km/h
Travel Time (Total)	210.3veh-h/h	1.7ped-h/h	254.0pers-h/h
Demand Flows (Total)	5072veh/h	84ped/h	6086pers/h
Percent Heavy Vehicles (Demand)	6.9%		
Degree of Saturation	1.062	0.035	
Practical Spare Capacity	-15.3%		
Effective Intersection Capacity	4775veh/h		
Control Delay (Total)	127.59veh-h/h	0.96ped-h/h	154.07pers-h/h
Control Delay (Average)	90.6sec	41.0sec	91.1sec
Control Delay (Worst Lane)	148.5sec		
Control Delay (Worst Movement)	147.2sec	54.2sec	147.2sec
Geometric Delay (Average)	2.3sec		
Stop-Line Delay (Average)	88.3sec		
Idling Time (Average)	81.4sec		
Intersection Level of Service (LOS)	LOS F	LOS E	
95% Back of Queue - Vehicles (Worst Lane)	83.2veh		
95% Back of Queue - Distance (Worst Lane)	621.5m		
Queue Storage Ratio (Worst Lane)	0.76		
Total Effective Stops	6054veh/h	69ped/h	7333pers/h
Effective Stop Rate	1.19per veh	0.82per ped	1.20per pers
Proportion Queued	0.94	0.82	0.95
Performance Index	689.2	2.1	691.2
Cost (Total)	6956.67\$/h	41.17\$/h	6997.83\$/h
Fuel Consumption (Total)	737.1L/h		
Carbon Dioxide (Total)	1757.1kg/h		
Hydrocarbons (Total)	0.177kg/h		
Carbon Monoxide (Total)	1.816kg/h		
NOx (Total)	4.309kg/h		

Level of Service (LOS) Method: Delay (RTA NSW).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

MOVEMENT SUMMARY

Site: NEH and Melbourne St AM base 2026

AM existing flows allowing for background growth to 2026

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Movement Performance - Vehicles

Mov ID	ODMo v	Demand Flows Total veh/h	Deg. Satn HV %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: NEH Newcastle										
1	L2	33	3.0	1.060	LOS F	83.2	621.5	1.00	1.52	19.0
2	T1	1533	8.0	1.060	LOS F	83.2	621.5	1.00	1.53	19.1
3	R2	129	5.0	0.542	LOS E	3.9	28.5	1.00	0.76	25.7
Approach		1695	7.7	1.060	LOS F	83.2	621.5	1.00	1.47	19.4
East: Melbourne St Morpeth										
4	L2	200	8.0	0.333	LOS C	8.7	64.8	0.79	0.78	34.2
5	T1	78	2.0	1.062	LOS F	40.8	302.0	1.00	1.34	15.6
6	R2	741	8.0	1.062	LOS F	42.8	320.1	1.00	1.30	15.7
Approach		1019	7.5	1.062	LOS F	42.8	320.1	0.96	1.21	17.5
North: NEH Maitland										
7	L2	472	4.0	0.412	LOS A	7.6	55.1	0.53	0.74	47.7
8	T1	1303	8.0	0.907	LOS D	45.1	337.4	0.98	1.04	33.0
9	R2	123	3.0	1.016	LOS F	10.4	74.8	1.00	1.15	20.5
Approach		1898	6.7	1.016	LOS D	45.1	337.4	0.87	0.97	33.9
West: Melbourne St										
10	L2	271	3.0	0.893	LOS D	12.2	87.4	1.00	0.96	33.4
11	T1	129	3.0	1.007	LOS F	15.8	113.7	1.00	1.21	19.8
12	R2	60	3.0	1.007	LOS F	15.8	113.7	1.00	1.21	21.8
Approach		460	3.0	1.007	LOS F	15.8	113.7	1.00	1.06	26.8
All Vehicles		5072	6.9	1.062	LOS F	83.2	621.5	0.94	1.19	23.3

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians

Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back of Queue		Prop. Queued	Effective Stop Rate per ped
					Pedestrian ped	Distance m		
P1	South Full Crossing	21	54.2	LOS E	0.1	0.1	0.95	0.95
P2	East Full Crossing	21	29.4	LOS C	0.1	0.1	0.70	0.70
P3	North Full Crossing	21	54.2	LOS E	0.1	0.1	0.95	0.95
P4	West Full Crossing	21	26.0	LOS C	0.0	0.0	0.66	0.66
All Pedestrians		84	41.0	LOS E			0.82	0.82

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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INTERSECTION SUMMARY



Site: NEH and Melbourne St PM base 2026

AM existing flows with background growth to 2026

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Intersection Performance - Hourly Values			
Performance Measure	Vehicles	Pedestrians	Persons
Travel Speed (Average)	22.1 km/h	2.0 km/h	22.0 km/h
Travel Distance (Total)	5151.0 veh-km/h	3.3 ped-km/h	6184.5 pers-km/h
Travel Time (Total)	232.7 veh-h/h	1.7 ped-h/h	280.9 pers-h/h
Demand Flows (Total)	5332 veh/h	84 ped/h	6398 pers/h
Percent Heavy Vehicles (Demand)	6.7%		
Degree of Saturation	1.138	0.035	
Practical Spare Capacity	-20.9%		
Effective Intersection Capacity	4685 veh/h		
Control Delay (Total)	145.82 veh-h/h	0.93 ped-h/h	175.92 pers-h/h
Control Delay (Average)	98.5 sec	40.0 sec	99.0 sec
Control Delay (Worst Lane)	197.2 sec		
Control Delay (Worst Movement)	194.0 sec	54.2 sec	194.0 sec
Geometric Delay (Average)	2.2 sec		
Stop-Line Delay (Average)	96.2 sec		
Idling Time (Average)	89.2 sec		
Intersection Level of Service (LOS)	LOS F	LOS D	
95% Back of Queue - Vehicles (Worst Lane)	115.0 veh		
95% Back of Queue - Distance (Worst Lane)	860.5 m		
Queue Storage Ratio (Worst Lane)	1.05		
Total Effective Stops	6608 veh/h	68 ped/h	7997 pers/h
Effective Stop Rate	1.24 per veh	0.80 per ped	1.25 per pers
Proportion Queued	0.94	0.80	0.95
Performance Index	660.8	2.0	662.9
Cost (Total)	7714.74 \$/h	40.59 \$/h	7755.33 \$/h
Fuel Consumption (Total)	791.4 L/h		
Carbon Dioxide (Total)	1885.6 kg/h		
Hydrocarbons (Total)	0.192 kg/h		
Carbon Monoxide (Total)	1.945 kg/h		
NOx (Total)	4.554 kg/h		

Level of Service (LOS) Method: Delay (RTA NSW).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

MOVEMENT SUMMARY

Site: NEH and Melbourne St PM base 2026

AM existing flows with background growth to 2026

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Movement Performance - Vehicles

Mov ID	ODMo v	Demand Flows Total	Deg. Satn HV	Average Delay	Level of Service	95% Back of Queue Vehicles	Distance Queue	Prop. Queued	Effective Stop Rate	Average Speed	
		veh/h	%	sec		veh	m		per veh	km/h	
South: NEH Newcastle											
1	L2	32	2.0	0.864	45.5	LOS D	40.6	303.1	0.98	0.96	35.6
2	T1	1275	8.0	0.864	39.6	LOS C	40.6	303.1	0.95	0.94	36.4
3	R2	274	5.0	0.916	80.3	LOS F	9.5	69.0	1.00	1.02	23.3
Approach		1580	7.4	0.916	46.8	LOS D	40.6	303.1	0.96	0.95	33.5
East: Melbourne St Morpeth											
4	L2	156	8.0	0.280	38.6	LOS C	6.8	51.0	0.79	0.77	33.7
5	T1	76	2.0	0.874	62.1	LOS E	18.8	138.6	1.00	0.99	26.5
6	R2	494	8.0	0.874	67.9	LOS E	18.8	138.6	1.00	0.98	25.9
Approach		725	7.4	0.874	61.0	LOS E	18.8	138.6	0.96	0.93	27.3
North: NEH Maitland											
7	L2	739	5.0	0.668	13.8	LOS A	15.2	110.9	0.70	0.81	46.1
8	T1	1733	8.0	1.138	194.0	LOS F	115.0	860.5	1.00	1.85	14.4
9	R2	162	2.0	1.062	146.9	LOS F	15.8	112.5	1.00	1.23	17.4
Approach		2634	6.8	1.138	140.5	LOS F	115.0	860.5	0.92	1.52	17.6
West: Melbourne St											
10	L2	189	2.0	0.564	33.2	LOS C	6.9	49.3	0.96	0.80	38.1
11	T1	142	2.0	1.072	148.0	LOS F	20.4	145.3	1.00	1.35	15.5
12	R2	61	2.0	1.072	153.6	LOS F	20.4	145.3	1.00	1.35	17.3
Approach		393	2.0	1.072	93.5	LOS F	20.4	145.3	0.98	1.09	22.8
All Vehicles		5332	6.7	1.138	98.5	LOS F	115.0	860.5	0.94	1.24	22.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians

Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back of Queue		Prop. Queued	Effective Stop Rate per ped
					Pedestrian ped	Distance m		
P1	South Full Crossing	21	54.2	LOS E	0.1	0.1	0.95	0.95
P2	East Full Crossing	21	27.4	LOS C	0.0	0.0	0.68	0.68
P3	North Full Crossing	21	54.2	LOS E	0.1	0.1	0.95	0.95
P4	West Full Crossing	21	24.1	LOS C	0.0	0.0	0.63	0.63
All Pedestrians		84	40.0	LOS D			0.80	0.80

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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INTERSECTION SUMMARY



Site: NEH and Melbourne St AM base+40 trucks

AM existing flows plus allowance for ALL trucks turning right out of Melbourne Street

Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Intersection Performance - Hourly Values			
Performance Measure	Vehicles	Pedestrians	Persons
Travel Speed (Average)	28.6km/h	2.0km/h	28.4km/h
Travel Distance (Total)	4434.8veh-km/h	3.3ped-km/h	5325.1pers-km/h
Travel Time (Total)	154.9veh-h/h	1.7ped-h/h	187.6pers-h/h
Demand Flows (Total)	4611veh/h	84ped/h	5533pers/h
Percent Heavy Vehicles (Demand)	6.7%		
Degree of Saturation	0.989	0.035	
Practical Spare Capacity	-9.0%		
Effective Intersection Capacity	4663veh/h		
Control Delay (Total)	80.03veh-h/h	1.00ped-h/h	97.04pers-h/h
Control Delay (Average)	62.5sec	42.7sec	63.1sec
Control Delay (Worst Lane)	91.8sec		
Control Delay (Worst Movement)	91.7sec	54.2sec	91.7sec
Geometric Delay (Average)	2.5sec		
Stop-Line Delay (Average)	60.0sec		
Idling Time (Average)	54.1sec		
Intersection Level of Service (LOS)	LOS E	LOS E	
95% Back of Queue - Vehicles (Worst Lane)	56.6veh		
95% Back of Queue - Distance (Worst Lane)	422.4m		
Queue Storage Ratio (Worst Lane)	0.52		
Total Effective Stops	4796veh/h	70ped/h	5825pers/h
Effective Stop Rate	1.04per veh	0.84per ped	1.05per pers
Proportion Queued	0.93	0.84	0.95
Performance Index	515.0	2.1	517.1
Cost (Total)	5056.34\$/h	42.17\$/h	5098.52\$/h
Fuel Consumption (Total)	605.9L/h		
Carbon Dioxide (Total)	1444.8kg/h		
Hydrocarbons (Total)	0.137kg/h		
Carbon Monoxide (Total)	1.501kg/h		
NOx (Total)	3.578kg/h		

Level of Service (LOS) Method: Delay (RTA NSW).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

MOVEMENT SUMMARY

Site: NEH and Melbourne St AM base+40 trucks

AM existing flows plus allowance for ALL trucks turning right out of Melbourne Street
 Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Movement Performance - Vehicles											
Mov ID	ODMo v	Demand Flows Total	Deg. Satn HV	Average Delay	Level of Service	95% Back of Queue Vehicles	Distance	Prop. Queued	Effective Stop Rate	Average Speed	
		veh/h	%	v/c	sec	veh	m		per veh	km/h	
South: NEH Newcastle											
1	L2	33	3.0	0.989	90.3	LOS F	56.6	422.4	1.00	1.27	24.8
2	T1	1277	8.0	0.989	84.7	LOS F	56.6	422.4	1.00	1.27	25.1
3	R2	123	5.0	0.458	66.0	LOS E	3.6	26.6	1.00	0.76	26.1
Approach		1433	7.6	0.989	83.2	LOS F	56.6	422.4	1.00	1.23	25.2
East: Melbourne St Morpeth											
4	L2	191	8.0	0.289	33.7	LOS C	7.8	58.1	0.74	0.77	35.5
5	T1	78	2.0	0.977	85.9	LOS F	31.7	234.7	1.00	1.15	22.0
6	R2	748	8.0	0.977	91.7	LOS F	34.7	259.9	1.00	1.12	21.7
Approach		1017	7.5	0.977	80.4	LOS F	34.7	259.9	0.95	1.06	23.4
North: NEH Maitland											
7	L2	492	4.0	0.441	12.2	LOS A	8.3	60.4	0.56	0.75	47.2
8	T1	1086	8.0	0.846	43.0	LOS D	33.6	251.5	0.96	0.94	35.3
9	R2	123	3.0	0.903	78.8	LOS F	8.4	60.1	1.00	0.99	25.8
Approach		1701	6.5	0.903	36.7	LOS C	33.6	251.5	0.85	0.89	36.6
West: Melbourne St											
10	L2	271	3.0	0.812	37.4	LOS C	10.1	72.4	1.00	0.89	36.5
11	T1	129	3.0	0.929	75.5	LOS F	13.3	95.7	1.00	1.07	24.2
12	R2	60	3.0	0.929	81.1	LOS F	13.3	95.7	1.00	1.07	26.3
Approach		460	3.0	0.929	53.8	LOS D	13.3	95.7	1.00	0.97	30.9
All Vehicles		4611	6.7	0.989	62.5	LOS E	56.6	422.4	0.93	1.04	28.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians								
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back of Queue		Prop. Queued	Effective Stop Rate per ped
					Pedestrian ped	Distance m		
P1	South Full Crossing	21	54.2	LOS E	0.1	0.1	0.95	0.95
P2	East Full Crossing	21	33.0	LOS D	0.1	0.1	0.74	0.74
P3	North Full Crossing	21	54.2	LOS E	0.1	0.1	0.95	0.95
P4	West Full Crossing	21	29.4	LOS C	0.1	0.1	0.70	0.70
All Pedestrians		84	42.7	LOS E			0.84	0.84

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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INTERSECTION SUMMARY



Site: Melbourne St and Pitnacree Rd AM+40 trucks

Melbourne and Pitnacree AM base plus allowance for all trucks to turn left off Flat Road to access highway
Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Intersection Performance - Hourly Values			
Performance Measure	Vehicles	Pedestrians	Persons
Travel Speed (Average)	35.0km/h	1.9km/h	34.4km/h
Travel Distance (Total)	2452.0veh-km/h	2.8ped-km/h	2945.3pers-km/h
Travel Time (Total)	70.1veh-h/h	1.5ped-h/h	85.6pers-h/h
Demand Flows (Total)	2417veh/h	84ped/h	2900pers/h
Percent Heavy Vehicles (Demand)	4.1%		
Degree of Saturation	0.814	0.035	
Practical Spare Capacity	10.5%		
Effective Intersection Capacity	2969veh/h		
Control Delay (Total)	28.67veh-h/h	0.86ped-h/h	35.26pers-h/h
Control Delay (Average)	42.7sec	36.8sec	43.8sec
Control Delay (Worst Lane)	70.5sec		
Control Delay (Worst Movement)	70.5sec	54.2sec	70.5sec
Geometric Delay (Average)	2.4sec		
Stop-Line Delay (Average)	40.3sec		
Idling Time (Average)	35.8sec		
Intersection Level of Service (LOS)	LOS D	LOS D	
95% Back of Queue - Vehicles (Worst Lane)	30.1veh		
95% Back of Queue - Distance (Worst Lane)	217.8m		
Queue Storage Ratio (Worst Lane)	0.31		
Total Effective Stops	2028veh/h	65ped/h	2498pers/h
Effective Stop Rate	0.84per veh	0.77per ped	0.86per pers
Proportion Queued	0.92	0.77	0.94
Performance Index	304.9	1.8	306.8
Cost (Total)	2158.13\$/h	36.08\$/h	2194.21\$/h
Fuel Consumption (Total)	272.0L/h		
Carbon Dioxide (Total)	645.5kg/h		
Hydrocarbons (Total)	0.058kg/h		
Carbon Monoxide (Total)	0.693kg/h		
NOx (Total)	1.126kg/h		

Level of Service (LOS) Method: Delay (RTA NSW).

Intersection LOS value for Vehicles is based on average delay for all vehicle movements.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

MOVEMENT SUMMARY

Site: Melbourne St and Pitnacree Rd AM+40 trucks

Melbourne and Pitnacree AM base plus allowance for all trucks to turn left off Flat Road to access highway
Signals - Fixed Time Isolated Cycle Time = 120 seconds (User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Movement Performance - Vehicles

Mov ID	ODMo v	Demand Flows Total veh/h	Deg. Satn HV %	Average Delay sec	Level of Service	95% Back of Queue Vehicles veh	Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: Melbourne St										
1	L2	173	8.0	0.236	LOS C	6.5	48.4	0.69	0.75	39.4
2	T1	498	4.0	0.708	LOS C	22.9	165.5	0.85	0.75	40.4
3	R2	91	2.0	0.742	LOS E	5.7	40.4	1.00	0.85	27.4
Approach		761	4.7	0.742	LOS C	22.9	165.5	0.83	0.76	38.0
East: Lawes Street										
4	L2	231	2.0	0.795	LOS C	9.1	64.9	1.00	0.88	36.1
5	T1	92	4.0	0.783	LOS E	8.4	60.5	1.00	0.89	29.0
6	R2	43	2.0	0.783	LOS E	8.4	60.5	1.00	0.89	28.6
Approach		365	2.5	0.795	LOS D	9.1	64.9	1.00	0.89	33.1
North: Melbourne St										
7	L2	18	2.0	0.806	LOS C	30.1	217.8	0.91	0.85	37.7
8	T1	568	4.0	0.806	LOS C	30.1	217.8	0.91	0.85	38.4
9	R2	73	2.0	0.595	LOS E	4.4	31.4	1.00	0.78	27.9
Approach		659	3.7	0.806	LOS C	30.1	217.8	0.92	0.84	36.8
West: Pitnacree Rd										
10	L2	143	2.0	0.814	LOS E	20.7	147.1	1.00	0.94	31.1
11	T1	202	2.0	0.814	LOS D	20.7	147.1	1.00	0.94	31.6
12	R2	286	8.0	0.724	LOS D	16.0	120.0	0.98	0.86	31.4
Approach		632	4.7	0.814	LOS D	20.7	147.1	0.99	0.90	31.4
All Vehicles		2417	4.1	0.814	LOS D	30.1	217.8	0.92	0.84	35.0

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians

Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back of Queue		Prop. Queued	Effective Stop Rate per ped
					Pedestrian ped	Distance m		
P1	South Full Crossing	21	54.2	LOS E	0.1	0.1	0.95	0.95
P2	East Full Crossing	21	24.7	LOS C	0.0	0.0	0.64	0.64
P3	North Full Crossing	21	43.4	LOS E	0.1	0.1	0.85	0.85
P4	West Full Crossing	21	24.7	LOS C	0.0	0.0	0.64	0.64
All Pedestrians		84	36.8	LOS D			0.77	0.77

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Appendix D – Interpreting Sidra output

1-Level of Service (LoS)

LoS	Traffic Signals and Roundabouts	Give Way and Stop Signs
A	Good	Good
B	Good, with acceptable delays and spare capacity	Acceptable delays and spare
C	Satisfactory	Satisfactory, but requires accident study
D	Operating near capacity	Near capacity and requires accident study
E	At capacity, excessive delay: roundabout requires other control method	At capacity, requires other control mode
F	Unsatisfactory, requires other control mode or additional capacity	Unsatisfactory, requires other control mode

2-Average Vehicle Delay (AVD)

The AVD is a measure of operational performance of an intersection relating to its LoS. The average delay should be taken as a guide only for an average intersection. Longer delays may be tolerated at some intersections where delays are expected by motorists (e.g. those in inner city areas or major arterial roads).

LoS	Average Delay / Vehicle (secs)	Traffic Signals and Roundabouts	Give Way and Stop Signs
A	Less than 15	Good operation	Good operation
B	15 to 28	Good with acceptable delays and spare capacity	Acceptable delays and spare capacity
C	28 to 42	Satisfactory	Satisfactory but accident study required
D	42 to 56	Operating near capacity	Near capacity, accident study required
E	56 to 70	At capacity, excessive delays: roundabout requires other control mode	At capacity; requires other control mode
F	Exceeding 70	Unsatisfactory, requires additional capacity	Unsatisfactory, requires other control mode

3-Degree of Saturation (D/S)

The D/S of an intersection is usually taken as the highest ratio of traffic volumes on an approach to an intersection compared with the theoretical capacity, and is a measure of the utilisation of available green time. For intersections controlled by traffic signals, both queues and delays increase rapidly as DS approaches 1.0. An intersection operates satisfactorily when its D/S is kept below 0.75. When D/S exceeds 0.9, queues are expected.