

Memorandum

То	Lend Lease Building	Johanna Nolan	johanna.nolan@lendlease.com					
From	Peter Oitmaa		Date	22 October 2013				
Subject	Preliminary Geotechnical In 4 Murray Rose Avenue, Syd		Project No.	45153.03				

This memorandum provides preliminary geotechnical information for the above project. The geotechnical report is currently being prepared and will expand upon the information contained herein.

- Five boreholes (BH1 to BH3, BH5 & BH6) were drilled to depths of 15.0 m to 15.2 m for geotechnical investigation purposes. The locations of the geotechnical boreholes are shown in red on the attached aerial photograph. BH4 was not drilled due to access constraints. The conditions encountered in the boreholes are shown on the logs and core photographs which are also attached.
- The boreholes indicate that the site is underlain by the following profile:
 - Filling: concrete, sandy/silty clay topsoil, shaly clay, shale and roadbase filling to depths of 0.2 m to 0.8 m.
 - Residual Soil: stiff to very stiff silty clay and clay with traces of ironstone gravel to depths of 0.6 m to 3.0 m.
 - Bedrock: initially extremely low strength shale (Class V), becoming low to medium strength (Class III) below depths of 1.5 m to 4.4 m, and medium, medium to high or high strength (Class II/I) below 3.5 m to 6.9 m depth.
- Free groundwater was not observed during augering and is likely to be well below the bedrock surface. Confirmation of groundwater levels will be made once the results of the groundwater sampling programme have been compiled.
- The rock profile on the site is similar to that encountered at 3 and 5 Murray Rose Avenue.
- Excavations in filling, soil and extremely low to very low strength shale should be readily achievable using conventional equipment such as a hydraulic excavator with bucket attachment. Excavations in low strength shale and stronger will require the use of ripping equipment and/or rock hammers.
- Excavations could be battered temporarily at 1(H):1(V) until such time as the basement walls have been constructed and the void backfilled. However, batters are unlikely to be practical for the proposed depth of excavation.
- Excavations that cannot be battered will need to be supported by shoring walls. Soldier pile walls with infill shotcrete panels would be suitable for the site. Walls could be designed on the basis of the parameters shown in Table 1 and should be constructed over the full excavation depth.

 Integrated Practical Solutions

 Brisbane • Cairns • Campbelltown • Canberra • Darwin • Gold Coast • Melbourne • Newcastle • Perth • Sunshine Coast • Sydney • Townsville • Wollongong • Wyong



	-					
Material	Bulk Density (kN/m³)	Coefficient of Earth Pressure at Rest (K _o)	Ultimate Passive Earth Pressure (kPa)			
Filling	20	0.6	-			
Residual Soil	20	20 0.25 0.4		-		
Class V Rock	22	0.15 ¹	0.25 ¹	750 ²		
Class III Rock 23		0 ¹	0 ¹	3,000 ²		
Class II/I Rock	23	0 ¹	0 ¹	6,000 ²		

Table 1: Material and Strength Parameters for Excavation Support Structures

Notes: ¹Unless unfavourably jointed; ²Only below bulk/detailed excavation level and where jointing is favourable

- Additional lateral loads from potential rock wedges should also be considered in the shoring wall design as was recommended for 3 and 5 Murray Rose Avenue.
- Temporary anchors will be required to support the soldier pile walls and could be designed using the parameters provided in Table 2.

Table 2: Allowable Bond Stresses for Anchor Design

Material Description	Allowable Bond Stress (kPa)					
Class V Rock	100					
Class III Rock	250					
Class II/I Rock	500					

- Groundwater is likely to be well below the top of the bedrock surface. Drainage provisions will be required in the basement to collect, store and remove seepage water.
- Spread footings (pads and strips) would be suitable for supporting the proposed structure. These could be proportioned on the basis of an allowable bearing pressure of 6000 kPa in the Class II/I shale, subject to spoon testing to check for the presence of seams below the footings.
- Bored piles used for shoring support could also be used to support structural loads providing they are founded below the bulk excavation level. Piles could also be used to support structural loads outside the basement area. Bored piles could be proportioned on the basis of the design parameters provided in Table 3.



Table 3: Design Parameters for Bored Piles

Material Description	Allowable End-Bearing Pressure (kPa)	Allowable Shaft Adhesion ¹ (kPa)			
Class V Rock	750	50			
Class III Rock	3500	300			
Class II/I Rock	6000	500			

Notes: ¹Provided adequate socket roughness is achieved

We trust the above information meets your present requirements. These comments will be expanded upon in the geotechnical report which is currently being prepared.

Yours faithfully, Douglas Partners Pty Ltd

ilus

Peter Oitmaa Senior Associate

Attachments:	Aerial Photograph
	Borehole Logs
	Core Photographs



SURFACE LEVEL: 11.5 AHD EASTING: NORTHING:

DIP/AZIMUTH: 90°/--

BORE No: 1 PROJECT No: 45153.03 DATE: 27/9/2013 SHEET 1 OF 2

			Description	Degree of		Rock	Fracture	Discontinuities	S	amnli	na & I	n Situ Testing
RL		Depth	Description of	Weathering	aphic 0	Strength Might High Strength Age Age	Spacing					-
R R		(m)	_	Weathering	Gra	Very Low Medium High Very High Ex High	0.01 0.05 0.50 1.00 (W)	B - Bedding J - Joint S - Shear F - Fault	Type	Core	RQD %	&
			CONCRETE	H H M M M M M M M M M M M M M M M M M M			0.01			- 22	_	Comments
11		0.15-	SILTY CLAY - very stiff, light grey brown silty clay with a trace of ironstone gravel, moist						E			9,16,14
-	-1		SHALE - extremely low strength, grey brown shale with ironstone bands					Unless otherwise stated	S	-		N = 30
10	-	1.5-	SHALE - very low to low strength,					rock is fractured along rough planar bedding dipping 0°-10°	S			7,9,14 N = 23
	Ē	1.8	grey shale		<u></u> i							
- 6 - 6			SHALE - low to medium strength, highly and highly to moderately weathered, fractured, grey brown shale, some very low strength bands					1.9m: B5°, fe 2.0 to 2.5m: B(x3) 0°-5°, cly 2.5 to 2.7m: B(x4) 0°, cly , fe 2.7m: J45° to 55°, cu, ro, cly 2.75 to 3.0m: B(x5) 0°, fe, cly 3m: J35°, un, ro, fe 3.12m: B5°, fe, cly 10mm 3.17 & 3.23m: B0°, fe 3.4 & 3.5m: J40°, pl, ro, fe 3.68m: B0°, fe, cly 10mm 3.85m: J80°, he 4.07 to 4.7m: B(x13) 0°, cly co 1-2mm	С	100	17	PL(A) = 0.3 PL(A) = 0.2 PL(A) = 0.3
4			SHALE - medium strength, fresh stained then fresh, slightly fractured and unbroken grey shale. Approximately 10% fine grained sandstone laminations. Some high strength siderite bands					4.7m: 175°, pl, ro, fe 4.9 to 5.13m: B(x3) 0°, fe 5.13 & 5.21m: J30°, pl, ro, cln 5.52m: J45°, pl, ro, cly 5.67m: J75°, pl, ro, fe 5.9m: J55°, un, ro, fe 6.04m: J80°, un, ro, fe 6.28m: J30°, pl, ro, cly 6.35m: J70°-90°, st, he, fe	С	100	92	PL(A) = 0.4 PL(A) = 1.3 PL(A) = 0.9
	- 9							7.94m: J60°, pl, ro, cln 8.1 to 8.65m: J(x3) 20°-30°, pl, sm, cln	С	100	96	PL(A) = 0.6 PL(A) = 0.8

RIG: Scout 1

CLIENT:

PROJECT:

Lend Lease Building Pty Ltd

Proposed Commercial Development

LOCATION: 4 Murray Rose Avenue, Sydney Olympic Park

DRILLER: LC

LOGGED: SI

CASING: HW to 1.2m

TYPE OF BORING: Diatube to 0.15m; Solid flight auger to 1.0m; Rotary to 1.8m; NMLC coring to 15.05m

WATER OBSERVATIONS: No free groundwater observed whilst augering

REMARKS: Standpipe installed to 15.05m (Screen 3.5m to 15.05m; Gravel 1.5m to 15.05m; Bentonite 1.0m to 1.5m; Backfill to Ground Level then Gatic cover)

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample G Gas sample PID Photo ionisation detector (ppm)	
B Bulk sample P Piston sample PL(A) Point load axial test Is(50) (MPa)	
BLK Block sample U, Tube sample (x mm dia.) PL(D) Point load diametral test Is(50) (MPa)	I Dolidiae Partnere
C Core drilling W Water sample pp Pocket penetrometer (kPa)	Douglas Partners
D Disturbed sample D Water seep S Standard penetration test	
E Environmental sample 📱 Water level V Shear vane (kPa)	Geotechnics Environment Groundwater

CLIENT:

PROJECT:

Lend Lease Building Pty Ltd

LOCATION: 4 Murray Rose Avenue, Sydney Olympic Park

Proposed Commercial Development

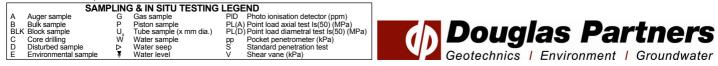
SURFACE LEVEL: 11.5 AHD EASTING: NORTHING: DIP/AZIMUTH: 90°/-- BORE No: 1 PROJECT No: 45153.03 DATE: 27/9/2013 SHEET 2 OF 2

												- 2
		Description	Degree of Weathering	. <u>0</u>	Rock Strength	Ļ	Fracture	Discontinuities	Sa	amplii	ng &	In Situ Testing
R	Depth (m)	of		-og		Water	Spacing (m)	B - Bedding J - Joint	ЭС	Core Rec. %	0	Test Results
	(11)	Strata	M H M M M H M M M M M M M M M M M M M M	_ق_	Ex Low Very Low Medium Very High	EX Hig		S - Shear F - Fault	Type	Re C	RQ%	& Comments
	-	SHALE - medium strength, fresh stained then fresh, slightly fractured and unbroken grey shale. Approximately 10% fine grained sandstone laminations.							с	100	96	PL(A) = 0.5
	- 11	Some high strength siderite bands (continued)						11.13m: J35°, pl, ro, cln				PL(A) = 1.5
	- 12							12.15m: J85°, pl, ro, cin	с	100	100	PL(A) = 0.7
	- 13											PL(A) = 0.6
	- 14								с	100	100	PL(A) = 0.8
	^{- 15} 15.05			e								
	. 15.05	Bore discontinued at 15.05m										
-4	- - - -											
- · ·	- 16 - - -											
	- 17											
_φ -	- - - - - - 18											
· · · · · · ·												
	- 19											
- φ - φ	-											
RIC	G: Scou	it 1 DRILL	ER: LC		I	LOGO	GED: SI	CASING: HW	/ to 1	.2m		

TYPE OF BORING: Diatube to 0.15m; Solid flight auger to 1.0m; Rotary to 1.8m; NMLC coring to 15.05m

WATER OBSERVATIONS: No free groundwater observed whilst augering

REMARKS: Standpipe installed to 15.05m (Screen 3.5m to 15.05m; Gravel 1.5m to 15.05m; Bentonite 1.0m to 1.5m; Backfill to Ground Level then Gatic cover)



SURFACE LEVEL: 12.3 AHD EASTING: NORTHING: DIP/AZIMUTH: 90°/-- BORE No: 2 PROJECT No: 45153.03 DATE: 30/9/2013 SHEET 1 OF 2

	Description	Dearee of	Roc	k	Frac	ture	Discontinuities	0	ampli	na 8	In Situ Testing
Depth	Description of	Degree of Weathering	≌ ∣ Stren	ath 🔤	Spa	cing			_		
(m)	Strata	H H W K K K K K K K K K K K K K K K K K	Crapt Loc	High Very High Ex High	0.05 0.10 U)		B - Bedding J - Joint S - Shear F - Fault	Type	Con Con	RQD %	& Comments
-	TOPSOIL - grey sandy clay topsoil	<u>μ</u> πΣομπ						E			Commenta
- 0.2 -	with some grass rootlets, moist										
	brown clay, moist										6,9,11
0.8	CLAY - stiff to very stiff, light grey				li ii	ii		S			N = 20
- 1	mottled brown clay, moist								1		3,6,9
								S			N = 15
-					li ii	ii					
- 1.8	SHALE - extremely low strength,				1 11		Unless otherwise stated rock is fractured along				
-2	light grey brown shale		<u></u>				rough planar bedding dipping 0°-10°				
2.3 2.4	SHALE - very low to low strength,						2.3m: CORE LOSS:				
-	highly weathered, fragmented to highly fractured, light grey brown						100mm 2.4 to 3.0m: fg, fe, cly				PL(A) = 0.
- 3	shale. Some medium strength		╤╡╎┍┲┚		LH				0		
	bands		크님님				3.0 to 3.6m: B5°, fe, cly	С	89	0	
-			3 H		li N						
-											
- - -4 4.0			\times	\leq	\square	\triangleleft	3.75m: CORE LOSS: 250mm				
-	SHALE - medium then low to medium strength, moderately		<u> </u>				⁴ .0 to 4.4m: B(x4) 0°, fe				PL(A) = 0
-	weathered, fractured and slightly fractured, brown shale						4.4 to 5.3m: B(x18) 0°,				
-							∫ fe, he 4.6m: J35°, pl, ro, fe				
- -5			크네		li i l	ii	4.85m: J55°, pl, ro, fe				
-			ヨニ	ii				с	92	51	
			크무님		1 1		5.3m: J45°-50°, he, fe 5.4 to 6.05m: J(x5)				PL(A) = 0
-							60°-70°, pl, ro, fe				
- 6					li li						
6.17	SHALE - medium strength, fresh,	┤╎┖╍╍╻╞] [ነ 6.15m: B0°, fe, cly ነ 10mm				PL(A) = 2
-	slightly fractured, grey shale. Approximately 10% fine grained						6.2 to 6.3m: B(x3) 0°, fe				
-	sandstone laminations. Some high strength siderite bands				li ii	di i -	^c 6.35m: J90°, pl, ro, cln 6.72m: J65° to 70°, cu,				
-7			<u></u>			Ľ	sm, cln 7.0 to 7.2m: J80°, un, ro,				
-							cln				
-			크네		li ii	iii					PL(A) = 0
							7.75 to 7.9m: J45°, pl,				
- 8							sm, cln				
-			크네		li ii	ii		с	100	98	PL(A) = 0
			크님!				8.65 to 9.3m: J(x3) 70°, pl, ro, cln				
-9			크니								
-			크님비			h					PL(A) = 0
-											
-			크니								
				• • •					·		
G: Scou	It 1 DRILI BORING: Solid flight auger to 1.0n	LER: LC			GED: S	SI	CASING: HW	/ to 1	.2m		

WATER OBSERVATIONS: No free groundwater observed whilst augering

CLIENT:

PROJECT:

Lend Lease Building Pty Ltd

Proposed Commercial Development

LOCATION: 4 Murray Rose Avenue, Sydney Olympic Park

REMARKS: Standpipe installed to 15.1m (Screen 3.1m to 15.1m; Gravel 2.0m to 15.1m; Bentonite 1.5m to 2.0m; Backfill to Ground Level then Gatic cover)

SAME	PLIN	G & IN SITU TESTING	LEG	END			
A Auger sample	G	Gas sample	PID	Photo ionisation detector (ppm)			
B Bulk sample	Р	Piston sample) Point load axial test Is(50) (MPa)			Douglas Partners
BLK Block sample	U,	Tube sample (x mm dia.)	PL(C) Point load diametral test Is(50) (MPa)	1	. \	Dolidias Partners
C Core drilling	Ŵ	Water sample	pp	Pocket penetrometer (kPa)			
D Disturbed sample	⊳	Water seep	S	Standard penetration test			
E Environmental sample	Ŧ	Water level	V	Shear vane (kPa)	J .		Geotechnics Environment Groundwater
· · · · · · · · · · · · · · · · · · ·				,			

SURFACE LEVEL: 12.3 AHD EASTING: NORTHING:

DIP/AZIMUTH: 90°/--

BORE No: 2 **PROJECT No:** 45153.03 DATE: 30/9/2013 SHEET 2 OF 2

		Description	Degree of	Rock Strength	Fracture	Discontinuities	Sa	amplii	ng &	In Situ Testing
R	Depth (m)	of Strata	Degree of Weathering	Ex Low Very Low Medium High Ex High Water	Spacing (m) 00100 00100 0011	B - Bedding J - Joint S - Shear F - Fault	Type	Core Rec. %	RQD %	Test Results & Comments
2	- 11 - 11.2 - 12 - 12.25	SHALE - medium strength, fresh, slightly fractured, grey shale. Approximately 10% fine grained sandstone laminations. Some high strength siderite bands (<i>continued</i>) SHALE - medium to high strength, fresh, slightly fractured, grey shale with high strength siderite bands SHALE - medium strength, fresh, slightly fractured grey shale, with approximately 10% fine grained sandstone laminations				10.1 to 10.6m: J(x4) 70°, pl, ro, cln 10.35m: J65°, he 10.97m: J90°, pl, sm, cln 11.65 & 11.71m: J65°, pl, ro, cln 12.2m: J60°, Cz10mm	с	100	93	PL(A) = 0.8 PL(A) = 1.2 PL(A) = 1
	- 13					13.1 & 13.3m: J30°, un, ro, cln 13.6 & 13.85m; J70°, pl, sm, cln 13.9 & 14.33m: B0°-5°,		100	05	PL(A) = 0.9
	- 14 - - - - - - - - - - - - - - - - - - -					cly 10-20mm	С	100	95	PL(A) = 0.6
	- 16 - 17 - 17 - 18 - 19	Bore discontinued at 15.1m								
T١	RIG: Scout 1 DRILLER: LC LOGGED: SI CASING: HW to 1.2m YPE OF BORING: Solid flight auger to 1.0m; Rotary to 2.3m; NMLC coring to 15.1m CASING: HW to 1.2m VATER OBSERVATIONS: No free groundwater observed whilst augering CASING: HW to 1.2m									

REMARKS: Standpipe installed to 15.1m (Screen 3.1m to 15.1m; Gravel 2.0m to 15.1m; Bentonite 1.5m to 2.0m; Backfill to Ground Level then Gatic cover)

	00101)				
	SAM	IPLIN	G & IN SITU TESTING	LEGEND	
A	Auger sample	G	Gas sample	PID Photo ionisation detector (ppm)	
B	Bulk sample	Р	Piston sample	PL(A) Point load axial test Is(50) (MPa)	
BL	K Block sample	U,	Tube sample (x mm dia.)	PL(D) Point load diametral test Is(50) (MPa)	I Dollalas Partners
C	Core drilling	Ŵ	Water sample	pp Pocket penetrometer (kPa)	Douglas Partners
D	Disturbed sample	⊳	Water seep	S Standard penetration test	
E	Environmental sample	Ŧ	Water level	V Shear vane (kPa)	Geotechnics Environment Groundwater



Lend Lease Building Pty Ltd Proposed Commercial Development

LOCATION: 4 Murray Rose Avenue, Sydney Olympic Park

SURFACE LEVEL: 12.1 AHD EASTING: NORTHING: DIP/AZIMUTH: 90°/-- BORE No: 3 PROJECT No: 45153.03 DATE: 30/9/2013 SHEET 1 OF 2

							/AZINUTH:		HE			-
		Description	Degree of Weathering	<u>.</u>	Rock Streng		Fracture	Discontinuities	Sa	amplir	ng & I	In Situ Testing
	Depth (m)	of Strata	M M M S L S L S L S L S L S L S L S L S	Loc	Ex Low Very Low Low Medium	Xater Water	Spacing (m)	B - Bedding J - Joint S - Shear F - Fault	Type	Core Rec. %	RQD %	Test Results & Comments
. <u>6</u> -	0.16	CONCRETE			<u> </u>	<u>->-ш</u>			E			Commenta
Ē	0.3	FILLING - shaly clay filling, damp		$\not\mapsto$						1		
-		CLAY - very stiff, grey brown clay with a trace of irosntone gravel, moist							s			4,9,6
-1		most										N = 15
-	4.5							Unless otherwise stated rock is fractured along rough planar bedding	s			4,8,20 N = 28
-	1.5	SHALE - extremely low strength, light grey brown shale with ironstone						dipping 0°-10°				
-2	1.8-	bands						1.8 to 2.08m: fg, fe				
² - ²		SHALE- low strength, highly weathered, fragmented and fractured, light grey brown shale						2.08 to 3.0m: B0°, fe, cly				
-									с	100	0	PL(A) = 0.2
3	3.0	SHALE - very low and very low to						3.0 to 3.4m: Ds				
ļ		low strength, highly weathered, light grey and grey shale										
-				X		$\downarrow \downarrow$		3.4m: CORE LOSS: 450mm				
-4	3.85					+						
- 00								4.2 to 4.7m: fg, cly				
-						ΝĿ						
-	4.7	SHALE - low strength, highly then slightly weathered, slightly fractured,				i N			с	85	45	PL(A) = 0.2
-5	5	grey brown shale				i i N		5.0 to 5.06m: fg 5.06m: J35°, pl, ro, fe	-			
				X				5.4m: J90°, pl, ro, cln				PL(A) = 0.2
-				K				, 5.75m: J70°, un, ro, fe,				
6- ص	6.0	SHALE - medium and medium to		X			╟╴┟┽┛╎╎╴	∖ cly 5.85m: B0°, fe and J80°,				
Ē		high strength, slightly weathered then fresh, slightly fractured and				i i I		un, ro, fe 6.1 to 6.4m: B(x6) 0°, fe				PL(A) = 0.6
-		unbroken, grey shale. Approximately 5% fine grained sandstone				i i		6.55m: J90°, pl, ro, fe				
-7	.	laminations						6.75m: B0°, cly				
' - ^م												
-								7.45 to 7.9m: J(x5)				
F								25°-30°, pl, ro, cln				PL(A) = 0.7
4-8	3								с	100	94	
-												
ļ												PL(A) = 0.6
-9	,											
- n -								∖ 9.15m: J60°, pl, ro, cln 9.2m: J85°, un, ro, cln				
Ē												
-								9.67m: J45°, pl, ro, cln	с	100	100	PL(A) = 0.7
	Scout		ER: LC				GED: SI	Casing: HW		_	1	

RIG: Scout 4

CLIENT:

PROJECT:

Lend Lease Building Pty Ltd

Proposed Commercial Development

LOCATION: 4 Murray Rose Avenue, Sydney Olympic Park

DRILLER: LC

LOGGED: SI

CASING: HW to 1.8m

TYPE OF BORING: Diatube to 0.16m; Solid flight auger to 1.0m; Rotary to 1.8m; NMLC coring to 15.0m WATER OBSERVATIONS: No free groundwater observed whilst augering REMARKS:

	SAM	PLIN	G & IN SITU TESTING	LEG	END		
A	Auger sample	G	Gas sample	PID	Photo ionisation detector (ppm)		
B	Bulk sample	Р	Piston sample	PL(A	A) Point load axial test Is(50) (MPa)		
BL	K Block sample	U,	Tube sample (x mm dia.)	PL(C	D) Point load diametral test Is(50) (MPa)		Indudiae Parthere
C	Core drilling	Ŵ	Water sample	, aa	Pocket penetrometer (kPa)		Douglas Partners
D	Disturbed sample	⊳	Water seep	S	Standard penetration test		
Е	Environmental sample	Ŧ	Water level	V	Shear vane (kPa)		Geotechnics Environment Groundwater

SURFACE LEVEL: 12.1 AHD EASTING: NORTHING: **DIP/AZIMUTH:** 90°/--

BORE No: 3 PROJECT No: 45153.03 DATE: 30/9/2013 SHEET 2 OF 2

		- I		<u> </u>							
	Der	Description	Degree of Weathering	Rock Strength		racture pacing	Discontinuities				n Situ Testing
ᆋ	Dept (m)	of		Hi Hi Lev	S Vater	(m)	B - Bedding J - Joint	Type	ore c. %	RQD %	Test Results &
			A NAW NAW NAW NAW NAW NAW NAW NAW NAW NA	Ex Low Very Low Medium High Very High	0.01	0.10	S - Shear F - Fault	Τy	С Ке	Ъ,	Comments
1	- 11	SHALE - medium and medium to high strength, slightly weathered then fresh, slightly fractured and unbroken, grey shale. Approximately 5% fine grained sandstone laminations <i>(continued)</i>					10.6m: J30°, pl, ro, cln	0	100	100	PL(A) = 1.5
	- 12						11.35m: J45°, pl, ro, cln	С	100	100	PL(A) = 0.6
-0-							12.52m: J30°, sl, sm,				PL(A) = 0.6
	- 13						cin 12.9m: J30°, si, sm, cin 13.53m: J45°, pi, sm, \cin	С	100	98	PL(A) = 0.5
	- 14						13.73 to 13.78m: Cz, cly 13.78 to 14.03m: J(x6) 20°-30°, pl, sm	C	100	90	PL(A) = 0.5
Ē	-15 1	5.0 Bore discontinued at 15.0m									
	- 16										
2-	- 17										
	- 18										
· · · · · · · · · · · · · · · · · · ·	- 19										
		cout 4 DRILL	ER: LC				Casing: HW				

RIG: Scout 4

CLIENT:

PROJECT:

Lend Lease Building Pty Ltd

LOCATION: 4 Murray Rose Avenue, Sydney Olympic Park

Proposed Commercial Development

DRILLER: LC

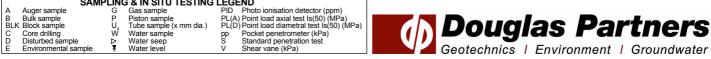
LOGGED: SI

CASING: HW to 1.8m

TYPE OF BORING: Diatube to 0.16m; Solid flight auger to 1.0m; Rotary to 1.8m; NMLC coring to 15.0m WATER OBSERVATIONS: No free groundwater observed whilst augering

REMARKS:

SAMPLING & IN SITU TESTING LEGEND



SURFACE LEVEL: 11.6 AHD EASTING: NORTHING: **DIP/AZIMUTH:** 90°/--

BORE No: 5 PROJECT No: 45153.03 DATE: 1/10/2013 SHEET 1 OF 2

\square		Description	Degree of	0	Rock	Fracture	Discontinuities	Sa	amplii	na &	In Situ Testing
님	Depth	of	Degree of Weathering	od hic	Strength High High High Water	Spacing (m)	B - Bedding J - Joint				
	(m)	Strata	H M M M M M M M M M M M M M M M M M M M	_ ق	Very Low Very Low Medium Very High Xar High	0.01 0.10 0.50 0.50	S - Shear F - Fault	Type	Rec.	RQD %	& Comments
	- 0.2	TOPSOIL - grey brown silty clay topsoil with a trace of grass rootlets, moist	-	\mathbb{P}				E			
	-	CLAY - very stiff, red brown clay with ironstone bands, moist					Unless otherwise stated rock is fractured along	s			5,7,9 N = 16
	-1 1.0 -	SHALE - extremely low to very low strength, light grey brown shale					rough planar bedding dipping at 0°-10°	s			7,9,25 N = 34
10	1.45 	highly to moderately weathered, fractured then slightly fractured, light					1.45 to 1.62m: fg, fe 1.8m: J85°, un, ro, fe				PL(A) = 0.6
	-2	grey brown shale					2.4 to 2.81m: J(x3)	с	100	95	PL(A) = 0.3
-6 	- - - - 3						55°-70°, pl, ro, cln				
	-						3.3m: J55° & 75°, st, ro,				PL(A) = 0.4
	- 3.5 - - - 	SHALE - medium strength, slightly weathered, fractured and slightly fractured, grey brown shale					cln 3.8m: J85°, pl, ro, cln				PL(A) = 6.5
	-						4m: J80°, pl, ro, fe 4.17m: B0°, cly 5mm 4.43m: B0°, fe				PL(A) = 0.5
	- - - - 5 -						4.75m: J30°, pl, ro, fe	с	100	96	PL(A) = 0.5
- 9	-						5.5 to 5.9m: J(x5) 50°-70°, pl, ro, fe				
	- 6 6.1	SHALE - medium and medium to					6.07m: J75°, un, ro, cln				
	-	high strength, fresh, slightly fractured and unbroken, grey shale. Approximately 5% fine grained \sandstone laminations									
	- - 7 -	[└] 6.7m and 10.65m: high strength siderite bands									
-4-	-						~	с	100	100	PL(A) = 0.7
	- 8										
-e	-										PL(A) = 0.6
	-9										
- 7-	-						9.7 to 9.85m: J75°, pl, ro, cln	с	100	100	PL(A) = 0.7
RIC	G: Sco	ut 1 DRILI	ER: LC	-	LOG	GED: SI	Casing: HW	/ to 1	.2m		
TΥ	PE OF	BORING: Solid flight auger to 1.0m BSERVATIONS: No free groundwat	; Rotary to		; NMLC coring to						

CLIENT:

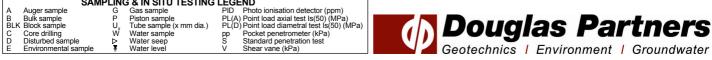
PROJECT:

Lend Lease Building Pty Ltd

Proposed Commercial Development

LOCATION: 4 Murray Rose Avenue, Sydney Olympic Park

REMARKS: Standpipe installed to 15.15m (Screen 3.15m to 15.15m; Gravel 1.5m to 15.15m; Bentonite 1.0m to 1.5m; Backfill to Ground Level then Gatic cover) **SAMPLING & IN SITU TESTING LEGEND**



CLIENT:

PROJECT:

Lend Lease Building Pty Ltd

Proposed Commercial Development

LOCATION: 4 Murray Rose Avenue, Sydney Olympic Park

SURFACE LEVEL: 11.6 AHD EASTING: NORTHING: DIP/AZIMUTH: 90°/-- BORE No: 5 PROJECT No: 45153.03 DATE: 1/10/2013 SHEET 2 OF 2

Γ		Description	Degree of	Rock Strength	Fracture	Discontinuities	Sa	amplir	ng &	In Situ Testing
R	Depth (m)	of Strata	Degree of Weathering Caphic Cond Degree Cond Cond Degree Cond Cond Cond Cond Cond Cond Cond Cond	Strength Very Low Medium Very High Very High Ex High	Spacing (m)	B - Bedding J - Joint S - Shear F - Fault	Type	Core Rec. %	RQD %	Test Results & Comments
	-11	SHALE - medium and medium to high strength, fresh, slightly fractured and unbroken, grey shale. Approximately 5% fine grained sandstone laminations <i>(continued)</i>				*>	С		100	PL(A) = 0.7 PL(A) = 2.5
- - - - - - - - - - - -	- 13									PL(A) = 1.2
	- - - - 14					13.6m: J20°, un, ro, cln	с	100	100	PL(A) = 0.6
- 	- - - - - 15					14.3m: J80°, pl, ro, cln				PL(A) = 0.6
ŀ	15.15	Bore discontinued at 15.15m								PL(A) = 0.5
	- - - 16 - -									
- - - - - - - - - - -	- 17									
-	- - 18 									
	- 19									
T١		t 1 DRILL BORING: Solid flight auger to 1.0m BSERVATIONS: No free groundwate		; NMLC coring to 1	GED: SI 5.15m	Casing: HW	/ to 1	.2m		

REMARKS: Standpipe installed to 15.15m (Screen 3.15m to 15.15m; Gravel 1.5m to 15.15m; Bentonite 1.0m to 1.5m; Backfill to Ground Level then Gatic cover)

	SAM	PLIN	G & IN SITU TESTING	LEG	END		
A	Auger sample	G	Gas sample	PID	Photo ionisation detector (ppm)		
B	Bulk sample	Р	Piston sample		A) Point load axial test Is(50) (MPa)		Douglas Partners
BI	K Block sample	U,	Tube sample (x mm dia.)	PL(E	D) Point load diametral test Is(50) (MPa)		Dollaise Partnere
C	Core drilling	Ŵ	Water sample	pp	Pocket penetrometer (kPa)		
D	Disturbed sample	⊳	Water seep	S	Standard penetration test		
E	Environmental sample	Ŧ	Water level	V	Shear vane (kPa)		Geotechnics Environment Groundwater

SURFACE LEVEL: 10.2 AHD EASTING: NORTHING:

PROJECT No: 45153.03 DATE: 2/10/2013 SHEET 1 OF 2 **DIP/AZIMUTH:** 90°/--

BORE No: 6

\square			Description	Degree of Weathering	<u>.0</u>	Rock Strength	Fracture	Discontinuities	Sa	amplii	ng & I	n Situ Testing
⊾		pth n)	of	weathering	Graphic Log	Vater High	Spacing (m)	B - Bedding J - Joint	эс	e %	۵.	Test Results
	(.	,	Strata	H M M M M M M M M M M M M M M M M M M M	ū	Ex Low Very Low Medium High Ex High Ex High	0.01	S - Shear F - Fault	Type	င်္ဂ ကို	RQD %	& Comments
-9-	-	0.14 -	CONCRETE FILLING - grey clay and crushed shale filling with some road base gravel and a trace of ash, moist						A			4,7,7
	- - - 1	0.8	CLAY - stiff then very stiff, brown clay with a trace of ironstone gravel,		\nearrow				S			N = 14
- 6 -	-		moist						S			3,6,8 N = 14
	-2	3.0-	SHALE - extremely low strength,					Unless otherwise stated rock is fractured along rough planar bedding dipping at 0°-10°	S			4,7,18 N = 25
	-		light grey brown shale					3.8m: CORE LOSS:				
- 9	- 4				\mathbb{X}			580mm				
	- 5	4.38-	SHALE - medium and low to medium strength, highly then slightly weathered, highly fractured to fractured, grey brown to grey shale, some very low strength bands					4.38 to 4.8m: B0°*10°, fe 4.8 to 5.3m: fg	С	60	0	PL(A) = 0.4
	-							5.4 to 5.7m: Ds 5.7m: CORE LOSS: 530mm				
- 4 -	-	6.23						6.23 to 6.37m: fg, fe 6.42 nto 6.57m: J(x3) 30°-35°, pl, ro, fe				
	- 7	7.35-						6.73 to 6.83m: fg, fe 6.83m: J45°, pl, ro, fe 7.03 & 7.16m: J(x2) 30°, pl, sm, cln 7.3 to 7.35m: Cz				PL(A) = 0.6
	- 8		SHALE - medium strength, fresh, slightly fractured and unbroken, grey shale. Approximately 5% fine grained sandstone laminations					7.5 10 7.5511. 02	С	94	57	PL(A) = 0.6
												PL(A) = 1
	-9							9.0 & 10.38m: B0°, cly, vn	С	100	100	PL(A) = 0.7

RIG: Scout 1

CLIENT:

PROJECT:

Lend Lease Building Pty Ltd

Proposed Commercial Development

LOCATION: 4 Murray Rose Avenue, Sydney Olympic Park

DRILLER: LC

LOGGED: SI

CASING: HW to 2.7m

TYPE OF BORING: Diatube to 0.17m; Solid flight auger to 2.5m; Rotary to 3.8m; NMLC coring to 15.0m

WATER OBSERVATIONS: No free groundwater observed whilst augering

REMARKS: Standpipe installed to 14.9m (Screen 3.5m to 14.9m; Gravel 3.0m to 14.9m; Bentonite 2.5m to 3.0m; Backfill 2.5m to Ground Level then Gatic cover)

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample G Gas sample PID Photo ionisation detector (ppm)	
B Bulk sample P Piston sample PL(A) Point load axial test Is(50) (MPa)	as Partners
BLK Block sample U, Tube sample (x mm dia.) PL(D) Point load diametral test is(50) (MPa)	ac <i>Varthor</i> c
C Core drilling W Water sample pp Pocket penetrometer (kPa)	
D Disturbed sample D water seep 5 Standard penetration test 1	
E Environmental sample V Shear vane (kPa)	Environment Groundwater

SURFACE LEVEL: 10.2 AHD EASTING: NORTHING: DIP/AZIMUTH: 90°/-- BORE No: 6 PROJECT No: 45153.03 DATE: 2/10/2013 SHEET 2 OF 2

Г			Degree of		Rock		– (1		-		•	. o:
	Depth	Description	Degree of Weathering ﷺ ≩ ≩ ଛ ଝ ଝ	a pic	Strength	ē	Fracture Spacing	Discontinuities				In Situ Testing
벅	(m)	of		Loc	Very Low Low High Very High Ex High	Water	(m)	B - Bedding J - Joint	Type	c. %	RQD %	Test Results &
		Strata	M M M M M M M M M M M M M M M M M M M		Very Medi Very Very	- 00	0.05	S - Shear F - Fault	F	Q B	Ψ,	Comments
	-11	SHALE - medium strength, fresh, slightly fractured and unbroken, grey shale. Approximately 5% fine grained sandstone laminations (continued)							С	100	100	PL(A) = 0.8 PL(A) = 0.6
	- 12 12.0 - - 13	LAMINITE - high strength, fresh, unbroken light grey and grey laminite with approximately 25% fine grained sandstone laminations										PL(A) = 1.2 PL(A) = 1.8
	- 14								С	96	96	PL(A) = 1.0
	14.88	- 14.88m to 15.0m: core left down						14 99m: CORE LOSS:				
 	-15 15.0-	Bore discontinued at 15.0m		\square				14.88m: CORE LOSS: 120mm				
	- 16							/				
	- 17											
-φ -φ 	- 18											

RIG: Scout 1

CLIENT:

PROJECT:

Lend Lease Building Pty Ltd

Proposed Commercial Development

LOCATION: 4 Murray Rose Avenue, Sydney Olympic Park

DRILLER: LC

LOGGED: SI

CASING: HW to 2.7m

TYPE OF BORING: Diatube to 0.17m; Solid flight auger to 2.5m; Rotary to 3.8m; NMLC coring to 15.0m

WATER OBSERVATIONS: No free groundwater observed whilst augering

REMARKS: Standpipe installed to 14.9m (Screen 3.5m to 14.9m; Gravel 3.0m to 14.9m; Bentonite 2.5m to 3.0m; Backfill 2.5m to Ground Level then Gatic cover)

	0000 00	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,					
	SAM	IPLIN	G & IN SITU TESTING	LEGEND]	
A	Auger sample	G	Gas sample	PID Photo ioni	isation detector (ppm)		
B	Bulk sample	Р	Piston sample		axial test Is(50) (MPa)		Douglas Partners
BL	K Block sample	U,	Tube sample (x mm dia.)	PL(D) Point load	l diametral test Is(50) (MPa)		Indialas Partners
C	Core drilling	Ŵ	Water sample	pp Pocket pe	enetrometer (kPa)		
D	Disturbed sample	⊳	Water seep	S Standard	penetration test		
E	Environmental sample	¥	Water level	V Shear var	ne (kPa)		Geotechnics Environment Groundwater



















