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15 May 2019

KF110816- C020

Dr Mark Jackson
Jackson Environment & Planning Pty Ltd
Suite 102, Level 1
25-29 Berry Street
NORTH SYDNEY 2060

Dear Mark,

Re SSD 5300 Mod 2 for Resource Recovery Facility, 30 Wyllie Road, Kembla Grange

This letter replaces our advice dated 24 April 2017.

I understand that an application to modify SSD5300 is being prepared. This application is referred to as SSD5300 Mod #2.

The purpose of the application is to seek approval for the following:

- a. Installation of rainwater storage tanks on the premises in a location which differs from the approved plans under SSD 5300 Mod 1;
- b. Installation of firewater storage tanks and a pump room; and
- c. An outdoor picking station (roof area 110 m²) and associated equipment in the general processing area.

The site works involved are of a very minor nature and will have the following effects:

- Increase in impervious area of 110 m² from the picking station roof (100% impervious).
- Increase in impervious area of 105 m² due to the fire fighting water tanks and pump room (100% impervious).
- Increase in impervious area of 74 m² due to the rainwater base slabs (100% impervious).

The total increase in impervious area under SSD 5300 Mod #2 will therefore be 289 m². This represents a cumulative increase in the overall site impervious area of 1.68%

I have reviewed the following documents prepared to support the application under SSD5300 Mod #2:

- a. Site Water Impact Assessment addendum report (KFW, 24 April 2017)
- b. Site Water Balance (KFW, 5 May 2016)
- c. WSUD and Flood Analysis Report (KFW, June 2014)

The stormwater model ,DRAINS, was modified to include the total additional impervious area draining to Pit 1G which includes the picking station roof and Pit 1J which includes the fire fighting water tanks and pump house.

As stated above, the overall increase in impervious area remains insignificant.

Reference to Attachment #1 indicates the following:

- Peak local runoff at pit 1G increase from 0.153 m³/s to 0.154 m³/s during a 100yr ARI storm burst.
- Peak local runoff at pit 1J increase from 0.361 m³/s to 0.363 m³/s during a 100yr ARI storm burst.
- The additional 0.003 m³/s runoff (Pit 1G and Pit 1J) is routed through the detention basin and has no significant effect on site discharge.

Please note that the apparent very small increase in discharge shown on Attachment #1 from the detention basin from 0.89 m³/s with 1.51 m³/s weir flow to 0.89 m³/s with 1.52 m³/s during a 100 year ARI storm burst is due to the fact that DRAINS model reports results to three significant figures.

- The Picking Station roof, hardstand under the rainwater tanks and the hardstand areas built for the firewater tank and pump room will not perceptibly increase the peak runoff from the site.

The MUSIC model was modified as follows:

- Operational area reduced by 36m² and impervious increased by 1% (additional recycled concrete pavement)
- New source node for the weighbridge office with a new treatment node (500 litre rainwater tank).

Reference to Attachment #2 indicates the following

- ❖ Table A2.1 shows that there is no perceptible increase residual pollutant load.
- ❖ Table A2.2 shows that water reuse demand met from the permanent pool is largely unaffected. The apparent reduction (ie 75.1% to 74.8%) is due to the weighbridge rainwater tank intercepting runoff which would have otherwise been captured by the permanent pool.
- ❖ Table A2.3 shows that the installation of a 500 litre rainwater tank will provide a small but additional source of water for site operations and mitigate the increase in runoff volume.

It can be reasonably concluded without further modelling that the impact of Modification #2 and associated site works is insignificant and imperceptible.

Yours faithfully,

K F WILLIAMS & ASSOCIATES PTY LTD


Wal Mullany

BE, Grad Dip LGE, ME(Hons), MIE Aust, CPEng, NER

Attachments:

Attachment #1: Modified DRAINS stormwater model output

Attachment #2: Modified MUSIC model output

Attachment #3: Drawing KF110816 C301 Rev F

Attachment #4: Drawing KF110816 C305 Rev D

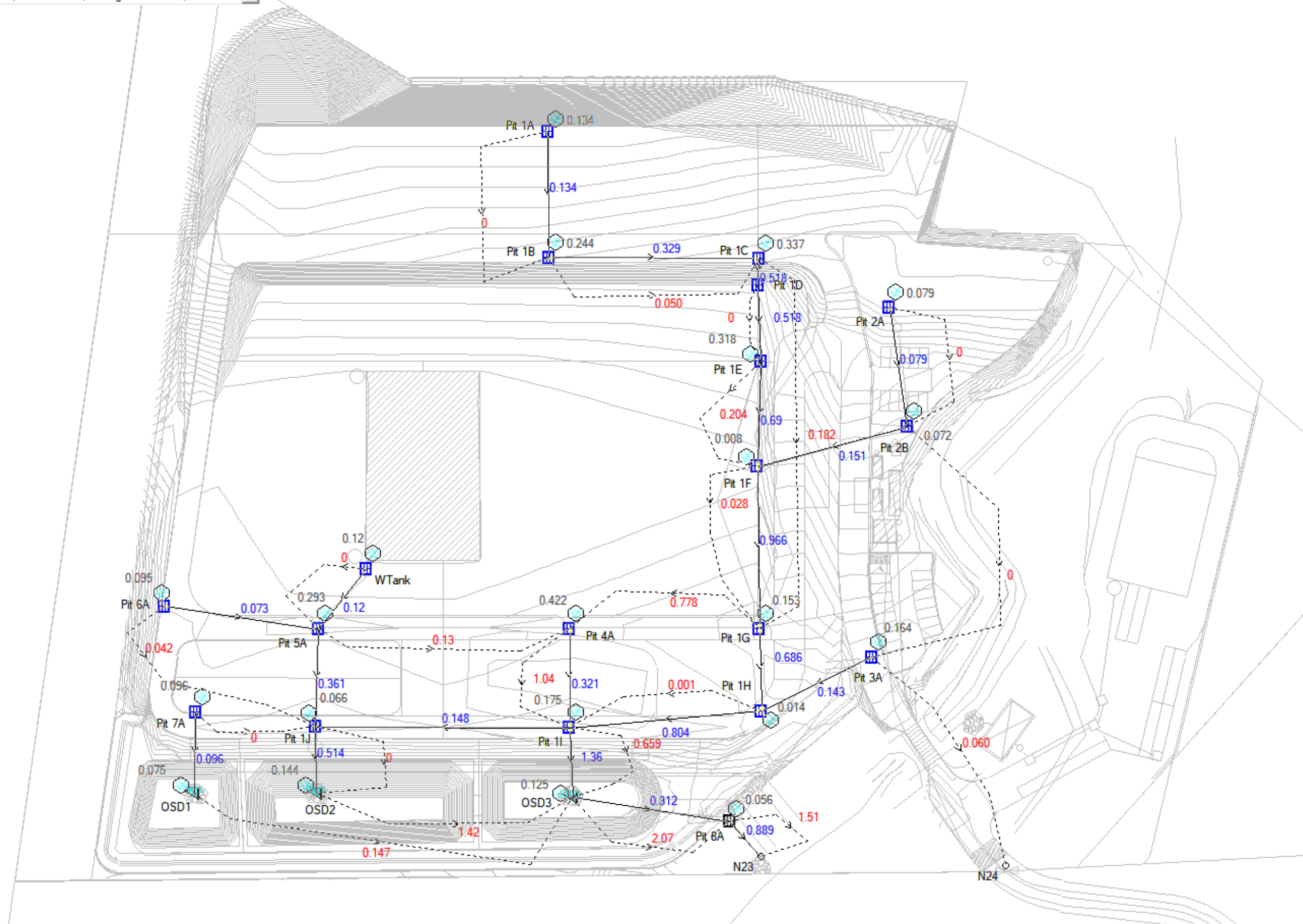
A1.1: Drains Model as Approved

KF110816-Drains-CC_2019 05 16_comparison.drn - DRAINS

File Edit Project View Draw Run Help



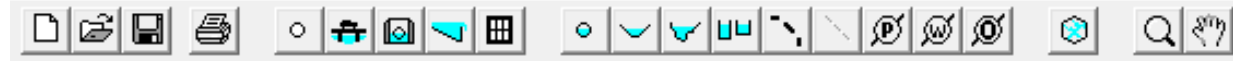
AR&R 100 year, 2 hours storm, average 71.9 mm/h, Zone 1



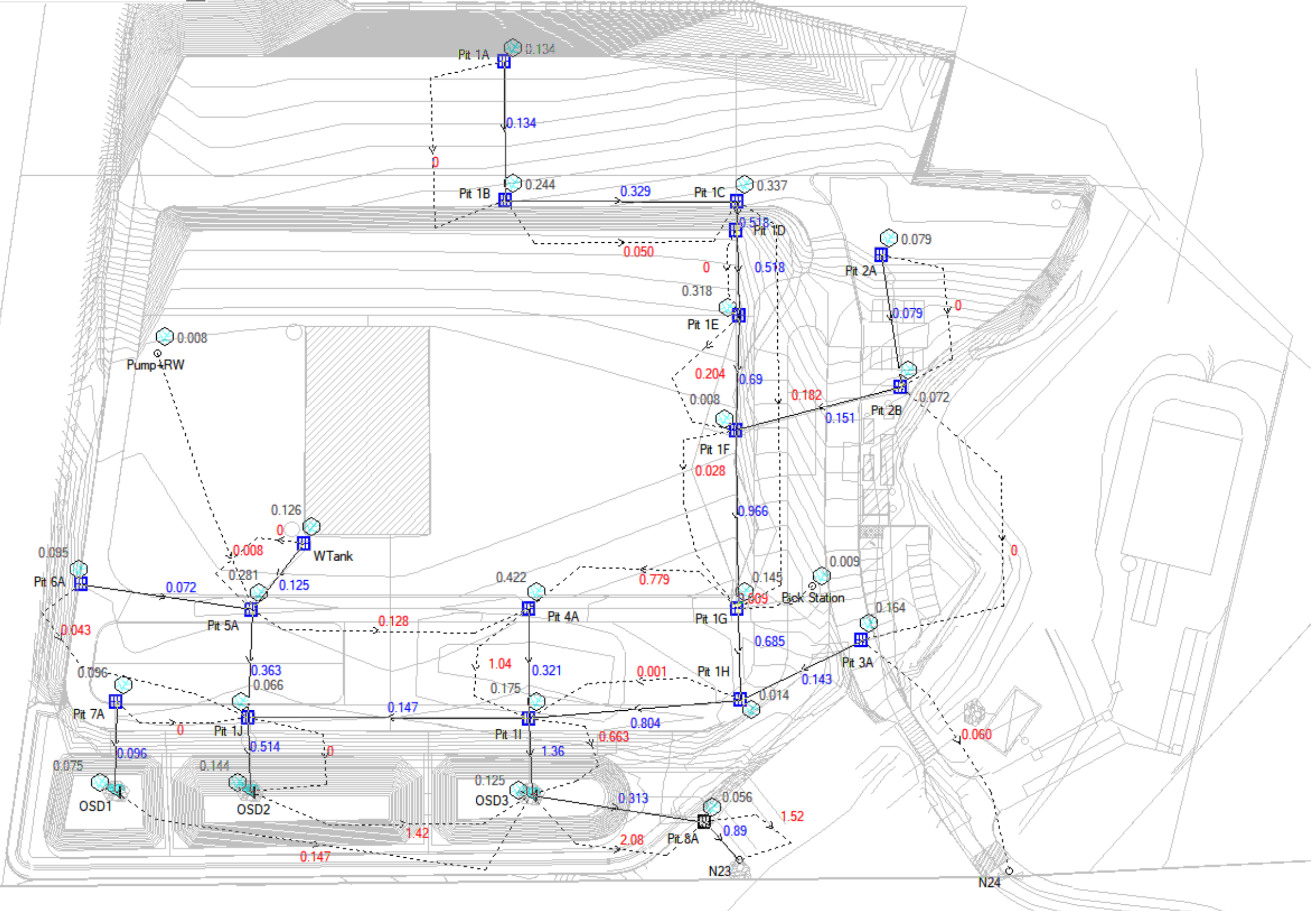
1.2: Drains Model for Mod #2

KF110816-SSD_Mod #2_2019 05 16.drn - DRAINS

File Edit Project View Draw Run Help

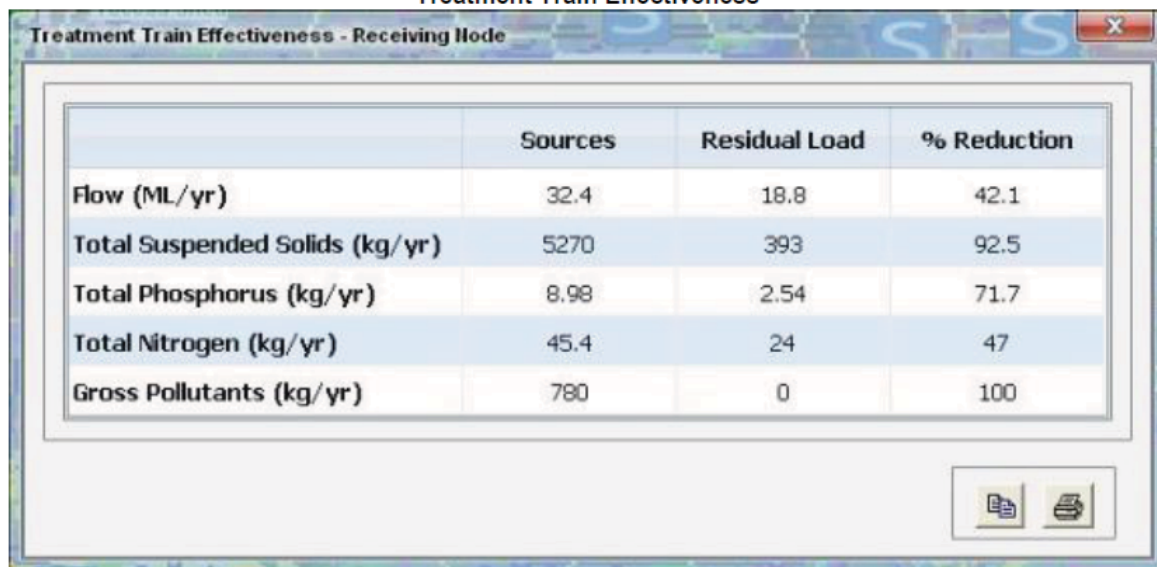


AR&R 100 year, 2 hours storm, average 71.9 mm/h, Zone 1



A2.1 Treatment Train Effectiveness

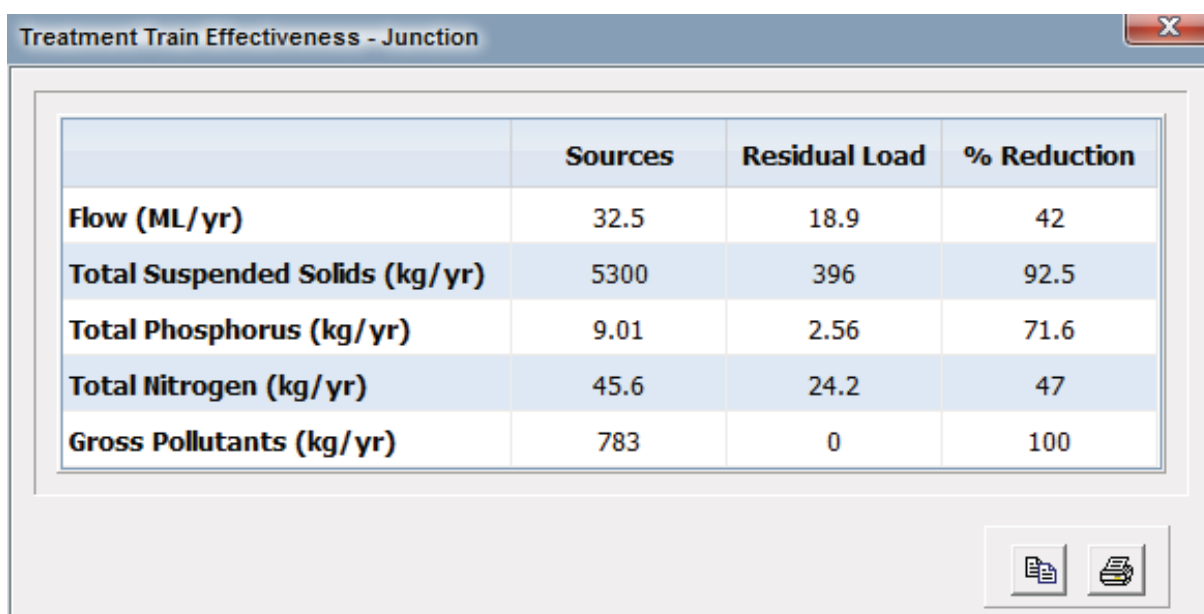
Table 4.6.1
Treatment Train Effectiveness



	Sources	Residual Load	% Reduction
Flow (ML/yr)	32.4	18.8	42.1
Total Suspended Solids (kg/yr)	5270	393	92.5
Total Phosphorus (kg/yr)	8.98	2.54	71.7
Total Nitrogen (kg/yr)	45.4	24	47
Gross Pollutants (kg/yr)	780	0	100

Table 4.6.1 extracted from WSUD Report

Table A2.1
Extracted from modified MUSIC[#] Model



	Sources	Residual Load	% Reduction
Flow (ML/yr)	32.5	18.9	42
Total Suspended Solids (kg/yr)	5300	396	92.5
Total Phosphorus (kg/yr)	9.01	2.56	71.6
Total Nitrogen (kg/yr)	45.6	24.2	47
Gross Pollutants (kg/yr)	783	0	100

Table 4.6.4
Water Balance Permanent Pool

	Flow (ML/yr)	TSS (kg/yr)	TP (kg/yr)	TN (kg/yr)	GP (kg/yr)
Flow In	28.7	972.2	5.9	29.2	694.4
ET Loss	1.3	0.0	0.0	0.0	0.0
Infiltration Loss	0.0	0.0	0.0	0.0	0.0
Low Flow Bypass Out	0.0	0.0	0.0	0.0	0.0
High Flow Bypass Out	0.0	0.0	0.0	0.0	0.0
Pipe Out	6.6	133.6	0.9	8.6	0.0
Weir Out	9.5	214.1	1.4	11.6	0.0
Transfer Function Out	0.0	0.0	0.0	0.0	0.0
Reuse Supplied	10.8	223.2	1.5	16.1	0.0
Reuse Requested	14.4	0.0	0.0	0.0	0.0
% Reuse Demand Met	75.1	0.0	0.0	0.0	0.0
% Load Reduction	43.9	64.2	61.6	31.1	100.0

Decimal Places: 1

Table 4.6.4 extracted from WSUD Report

Table A2.2
Permanent Pool Water Balance
Extracted from Modified MUSIC# Model

	Flow (ML/yr)	TSS (kg/yr)	TP (kg/yr)	TN (kg/yr)	GP (kg/yr)
Flow In	27.5	931.8	5.6	28.0	665.3
ET Loss	1.3	0.0	0.0	0.0	0.0
Infiltration Loss	0.0	0.0	0.0	0.0	0.0
Low Flow Bypass Out	0.0	0.0	0.0	0.0	0.0
High Flow Bypass Out	0.0	0.0	0.0	0.0	0.0
Pipe Out	6.2	127.1	0.8	8.2	0.0
Weir Out	8.7	195.5	1.3	10.6	0.0
Transfer Function Out	0.0	0.0	0.0	0.0	0.0
Reuse Supplied	10.8	225.6	1.5	15.6	0.0
Reuse Requested	14.4	0.0	0.0	0.0	0.0
% Reuse Demand Met	74.8	0.0	0.0	0.0	0.0
% Load Reduction	45.6	65.4	62.8	33.0	100.0

Decimal Places: 1

Table

Table A2.3
Weighbridge Rainwater Tank Water Balance
Extracted from Modified MUSIC# Model

Node Water Balance - Weighbridge RWT					
	Flow (ML/yr)	TSS (kg/yr)	TP (kg/yr)	TN (kg/yr)	GP (kg/yr)
Flow In	0.0	4.9	0.0	0.0	0.7
ET Loss	0.0	0.0	0.0	0.0	0.0
Infiltration Loss	0.0	0.0	0.0	0.0	0.0
Low Flow Bypass Out	0.0	0.0	0.0	0.0	0.0
High Flow Bypass Out	0.0	0.0	0.0	0.0	0.0
Pipe Out	0.0	1.5	0.0	0.0	0.0
Weir Out	0.0	0.0	0.0	0.0	0.0
Transfer Function Out	0.0	0.0	0.0	0.0	0.0
Reuse Supplied	0.0	0.4	0.0	0.0	0.0
Reuse Requested	0.1	0.0	0.0	0.0	0.0
% Reuse Demand Met	15.9	0.0	0.0	0.0	0.0
% Load Reduction	50.1	69.0	60.0	50.4	100.0

Decimal Places



1. RAINWATER TANKS TO HAVE FIRST FLUSH BYPASS SYSTEM
2. 2 x 100,000 LITRE TANKS WILL PROVIDE APPROXIMATELY 82% OF DAILY DEMAND OF 500 LITRES/DAY
3. PRIOR TO COMMENCING CONSTRUCTION WORK, THE OWNER/CONTRACTOR SHALL ENGAGE A REGISTERED SURVEYOR TO ACCURATELY DELINEATE THE GAS PIPELINE BOUNDARY. IN ORDER TO PREVENT CONSTRUCTION PLANT FROM ENCRANCHING ONTO THE GAS PIPELINE EASEMENT, THE OWNER/CONTRACTOR SHALL INSTALL A HIGH VISIBILITY ORANGE PARA-WEB FENCING OR 18M HIGH CHAINWIRE FENCE TO CLEARLY DELINEATE THE LIMIT OF WORK. THE FENCE SHALL NOT ENCRANCH UPON THE GAS PIPELINE EASEMENT AND SHALL BE INSTALLED 0.15 M OFF THE EASEMENT BOUNDARY.
4. WATER QUALITY/RECUTTLING POND EMBANKMENT IS TO BE COMPACTED TO 100% MOD COMPACTION UNDER LEVEL 2 GEOTECHNICAL CONTROL
5. DUST CONTROL TO BE ACHIEVED USING WATER CART. DEMAND ESTIMATED AT 40,000 L/DAY
6. LEACHATE COLLECTION TANKS PUMPED OUT BY SERVICE TANKER
7. BYPASS CONTROL PIT TO DIVERT 220 l/sec TO WATER QUALITY POND. DISCHARGE EXCEEDING 230 l/sec TO DRAIN DIRECT TO DETENTION BASIN.
8. DETENTION BASIN TO HAVE 1953m³ STORAGE WITH 675mm DIA PIPE OUTLET. WEIR RL @ APPROX. 18.85m.
9. WATER QUALITY/RECYCLING POND TO HAVE PERMANENT VOLUME OF 3428 m³ WITH 673 m³ EXTENDED STORAGE. OVERFLOW PIPE TO BE 100mm DIA PIPE. OVERFLOW PIPE TO DISCHARGE TO DETENTION BASIN.
10. SHREDDING RUNOFF POND. SHREDDING RUNOFF POND TO HAVE VOLUME OF 781m³. OVERFLOW PIPE TO BE DISCHARGED TO WATER QUALITY POND. OVERFLOW PIPE TO BE 100mm DIA.
11. NO STORAGE OF BUOYANT MATERIAL IN PMF INUNDATION AREA. MINIMUM CRUSHED AGGREGATE SIZE IN PMF INUNDATION AREA TO BE 75mm
12. GREEN WASTE TO BE COVERED BY TARPULIN TO PREVENT INFILTRATION OF RAINWATER WHEN STOCKPILE TO BE IN PLACE LONGER THAN 21 DAYS.
13. SEPTIC TANK SERVICING OFFICE COMPLEX SHALL BE SERVICED/PUMPED OUT WEEKLY UNDER SERVICE CONTRACT

LEGEND

- DENOTES PROPOSED BUILDING
- DENOTES EXISTING CONTOURS
- DENOTES PROPOSED SURFACE LEVELS
- DENOTES PROPOSED BATTER (1 IN 2) TO BE LANDSCAPED
- DENOTES PROPOSED LANDSCAPING/BATTER PROTECTION
- DENOTES 10m BUFFER LINE FROM TOP BANK
- DENOTES 1% AEP FLOOD LIMIT
- DENOTES SUBSOIL DRAINAGE
- DENOTES CATCH DRAIN
- DENOTES DIRECTION OF TRAFFIC FLOW
- DENOTES STORMWATER FLOW
- DENOTES HUMCEPTOR DOWNSTREAM DEFLECTOR BLOCKS OR BOULDERS WITH MIN. MASS OF 900kg MAY BE USED
- DENOTES BOLLARD & SIGNAGE
- DENOTES STORMWATER PITS AND PIPES
- DENOTES STAGE 1 LIMITS
- DENOTES FUTURE STAGE LIMITS
- DENOTES PROPOSED SUB SOIL LINE



SIGN - ARR-104

F	LABELS FIXED	05.02.2019	J.D.	W.M.
E	AMENDED AS PER DEWCAPE	09.10.2018	J.D.	W.M.
D	AMENDED AS PER CLOUDS	21/6/2018	AM	W.M.
C	1% AEP FLOOD EXTENT SHOWN	24 APRIL 2017	WM	WM
B	ADDITIONAL WEIGHBRIDGE NOTATIONS ADDED	31.03.2017	C.R.	W.M.
A	WEIGHBRIDGE LOCATION AMENDED	13.01.2017	B.M	W.M
Revision	Amendment or reason for issue	Issue date	Drawn by	Authorised

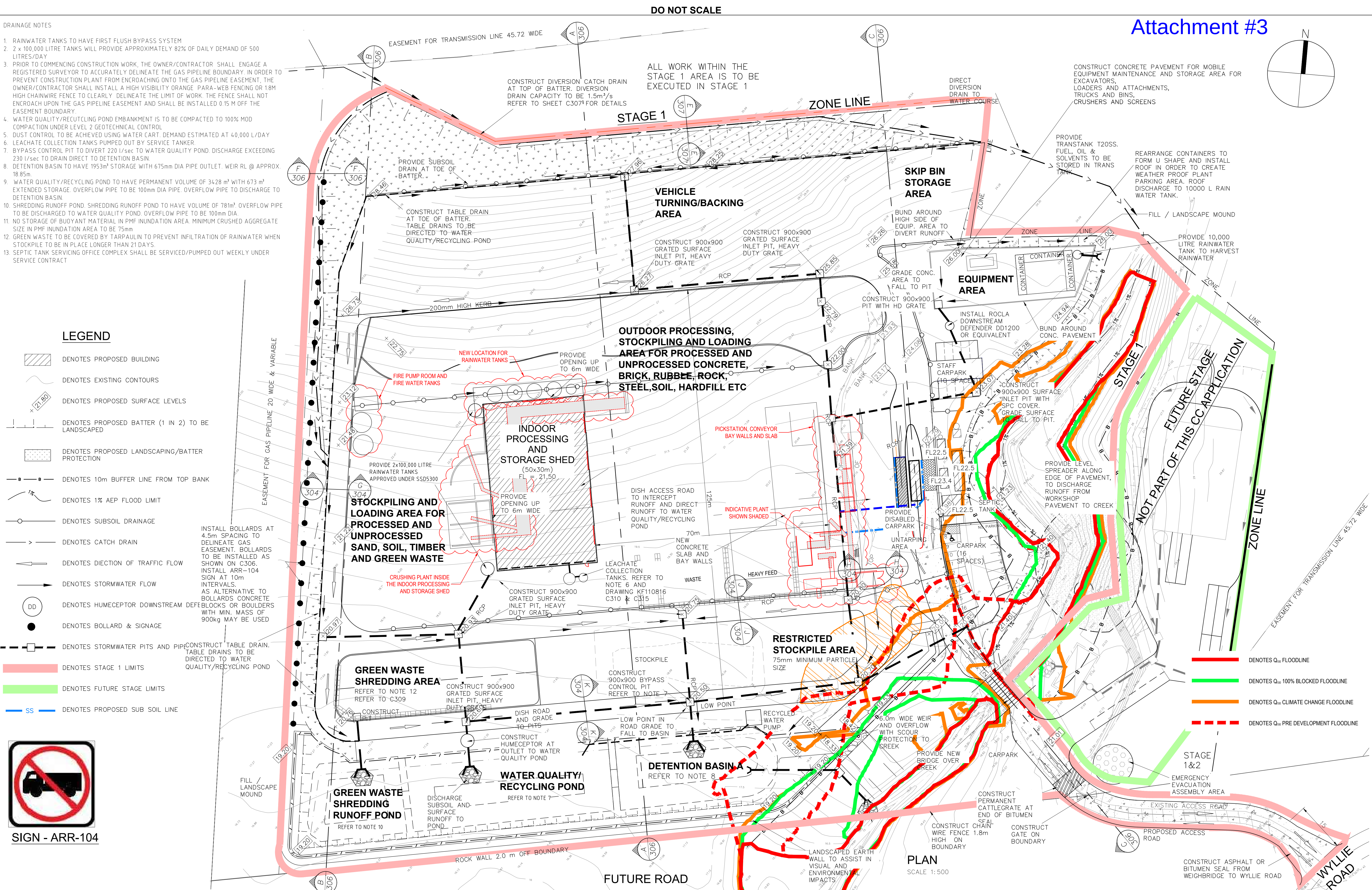


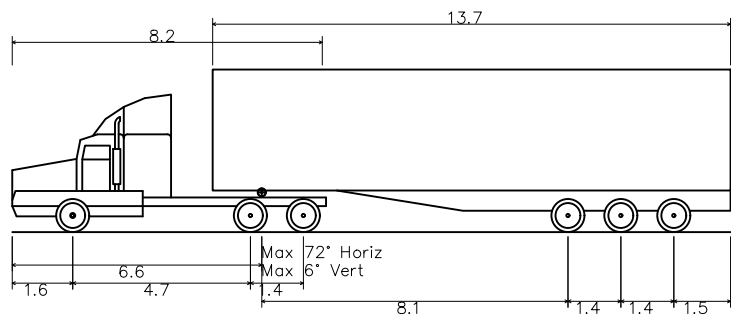
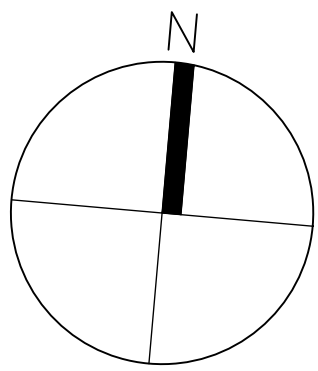
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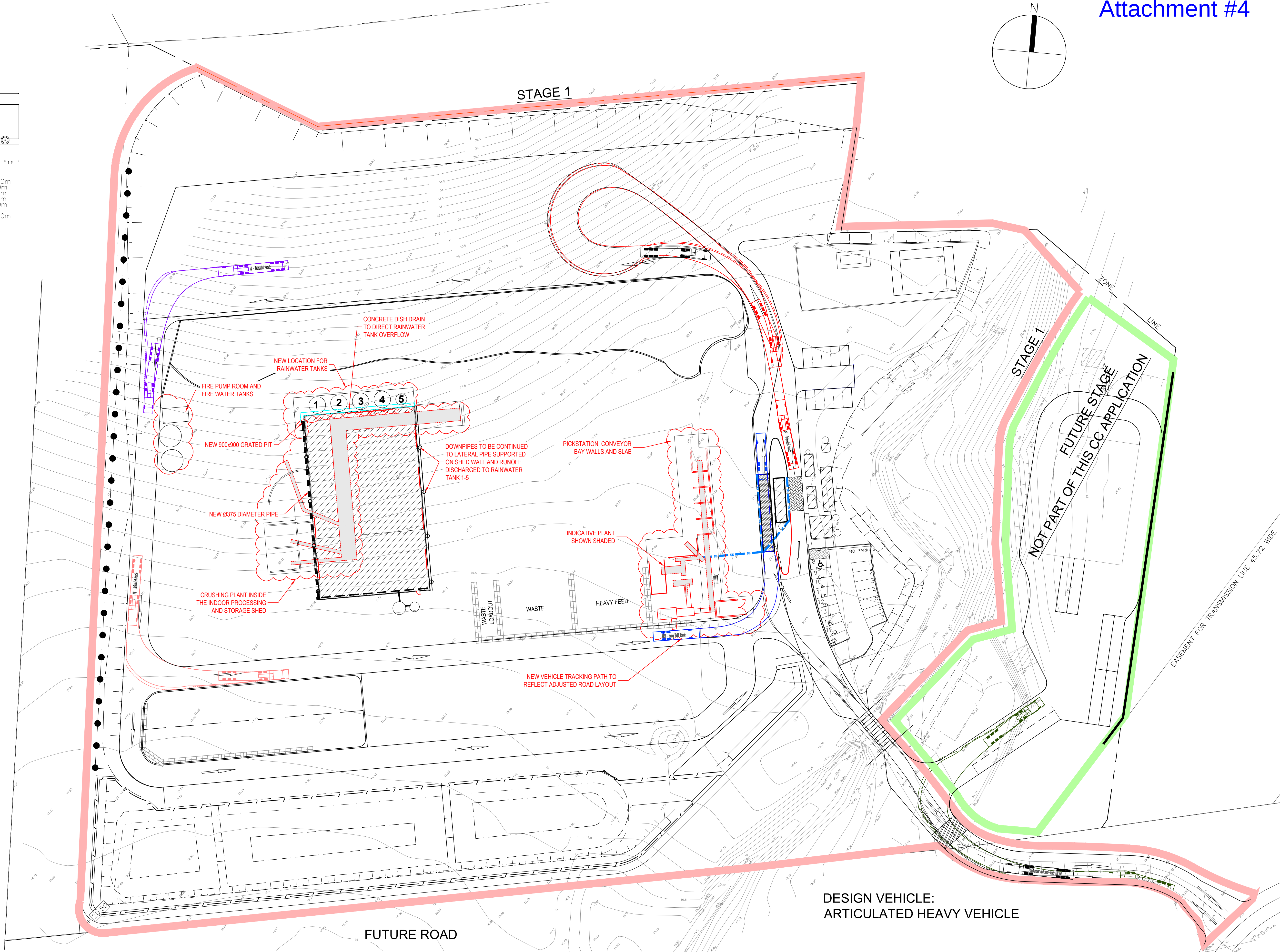
Surveyor	DENNIS SMITH	Date	APRIL 2016
Date of Survey		Drawn	B.MIDDLETON
Height Datum	AHD	Designed	W.MULLANY
Origin		Checked	
Horiz. Datum		Approved	

Drawing Title		Project No.	
PROPOSED INDUSTRIAL DEVELOPMENT		KF110816	
LOT 10 DP 878167		Drawing No.	
WYLLIE ROAD, KEMBLA GRANGE		C301	
SITE PLAN		Sheet	Revision
ISSUED FOR SSD5300 MOD2 CC		2 Of 20	F





AV — Articulated Vehicle
Overall Length 19.000m
Overall Width 2.500m
Overall Body Height 4.301m
Min Body Ground Clearance 0.418m
Track Width 2.500m
Lock-to-lock time 6.00s
Curb to Curb Turning Radius 12.500m



DESIGN VEHICLE:
ARTICULATED HEAVY VEHICLE

Revision	Amendment or reason for issue	Issue date	Drawn by	Authorised
D	SITE LAYOUT AMENDED AS PER DEWCAPE	09.10.2018	J.D.	W.M.
C	ADDITIONAL SWEEP PATH SHOWN FOR ENTRY ONTO NEW WEIGHBRIDGE	24 APRIL 2017	BM	WM
B	ADDITIONAL WEIGHBRIDGE NOTATIONS ADDED	31.03.2017	C.R.	W.M.
A	WEIGHBRIDGE LOCATION AMENDED	24.01.2017	B.M	W.M



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Surveyor DENNIS SMITH	Date APRIL 2016	Drawing Title PROPOSED INDUSTRIAL DEVELOPMENT LOT 10 DP 878167 WYLLIE ROAD, KEMBLA GRANGE TURNING SWEEP PATHS		Project No. KF110816	
Date of Survey	Drawn B.MIDDLETON			Drawing No. C305	
Height Datum AHD	Designed W.MULLANY	Drawing Status ISSUED FOR SSD5300 MOD2 CC		Sheet 6 Of 20	Revision D
Origin	Checked				
Horiz. Datum	Approved				