

West Culbura Subdivision Development Transport and Accessibility Impact Assessment

# West Culburra Subdivision Development 

## Transport a nd Accessibility Impact Assessment

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Quality Record


MELBOURNE - SYDNEY - BRISBANE - CANBERRA ADELAIDE • GOLD COAST • TOWNSVILLE

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## 1. Introduction

### 1.1 Background

The West Culburra development involves a mixed use subdivision development over approximately 110 hectares (ha) on land bounded to the north by the Crookhaven River, Lake Woollumboola and the existing urban area of Culburra to the east, Jervis Bay National Park to the south and Coonamia Road to the west.

The proposed subdivision is comprised of six key stages, proposed to be undertaken in stages over a period of approximately 10 years. On completion, the West Culburra Development will include a mixture of medium density housing types, ranging from small lots for the 55+ aged group to multistorey units. A Collector Road is proposed to provide access to the development from Culburra Road.

A major project application (no. 09-0088) was lodged with the NSW Department of Planning and Infrastructure in April 2010 seeking approval for the Concept Plan under Part 3A of the Environmental Planning and Assessment Act 1979.

GTA Consultants was commissioned by Reality Realizations Pty Ltd in May 2012 to undertake a transport and accessibility impact assessment for the proposed development and in particular to address Section 5 (Traffic and Access) of the Director-General's Environmental Assessment Requirements (DGR's) dated 27 May 2010 as follows. Table 1.1 lists the DGR's and the corresponding sections of the report where these are addressed.

Table 1.1: DGR's and Relevant Report Sections

| Section 5 - Traffic and Access | Addressed in |
| :--- | :---: |
| 5.1) Prepare a Transport and Accessibility Impact Study in accordance with Table |  |
| 2.1 of the RMS's Guide to Traffic Generating Developments, having regard to |  |
| the principles of the NSW Planning Guidelines for Walking and Cycling and the |  |
| NSW State Plan (2010) to include: |  |$\quad$ -

### 1.2 Purpose of this Report

This report sets out an assessment of the anticipated transport implications of the proposed development, including consideration of the following:
i existing traffic conditions surrounding the site
ii pedestrian and bicycle requirements
iii the traffic generating characteristics of the proposed development
iv suitability of the proposed access arrangements for the site
v the transport impact of the development proposal on the surrounding road network.

### 1.3 References

In preparing this report, reference has been made to the following:

- an inspection of the site and its surrounds
- Austroads Guide to Road Design, Part 4B: Roundabouts (second edition), 2011
- Austroads Part 6A: Pedestrian and Cyclist Paths, 2009
- Comments from Scott Wells (Traffic and Transport Unit, Shoalhaven City Council) on Long Bow Point Golf Course Traffic and Parking Assessment, dated 31 May 2012
- Environmetrics, 2006, Sydney Cycling Research: Internet Survey. For the City of Sydney
- Integrating Land Use and Transport, NSW Department of Urban Affairs and Planning, 2001
- NSW Bicycle Guidelines, Roads and Maritime Services, 2005
- NSW 2021: A Plan to Make NSW Number One, 2011
- NSW 2021: A Plan to Make NSW Number One, Regional Action Plan: Illawarra Community Discussion Paper, 2011
- NSW Planning Guidelines for Walking and Cycling, Department of Infrastructure, Planning and Natural Resources, 2004
- NSW Speed Zoning Guidelines (RMS, 2011).
- Portland Bureau of Transportation (PBT), 2010, Four Types of Transportation Cyclists. Assessed at: http://www.portlandonline.com/transportation/index.cfm? $\mathrm{a}=158497 \& \mathrm{c}=44671$
- Shoalhaven City Council, Traffic and Transport Unit, Calculation of Traffic Growth Factors \& Trip Generation Rates, correspondence dated 19 February 2013
- Shoalhaven City Council Subdivision Code (DCP 100), 2002
- Shoalhaven City Council Car Parking Code, Development Control Plan (DCP) 18, 1996
- Shoalhaven City Council DCP No. 67, Culburra Expansion Area, 1996
- Shoalhaven Draft Local Environmental Plan (LEP) 2009
- Shoalhaven LEP 1985
- Shoalhaven Integrated Transport Strategy, 2000
- Traffic and Parking Assessment - Proposed 18 Hole Championship Golf Course, Long Bow Point, Culburra, prepared by Traffic Solutions Pty Ltd, March 2012
- traffic surveys undertaken by Skyhigh in May 2012 as referenced in the context of this report
- plans for the proposed development site constraints prepared by Allen, Price and Associates: Site Constraints, revision Po4, dated 25 July 2012

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- plans for the proposed development site constraints prepared by John Toon Pty Ltd, plan no's 1-6, dated April 2010
- other documents and data as referenced in this report.


## 2. Existing Conditions

The subject site is located at Culburra, 180km south of Sydney and 20km east of Nowra.
The West Culburra subdivision development, as specified in the Part 3A submission, covers an area of approximately 110 ha. The land area is currently unoccupied and subject to the following land use classifications under Shoalhaven City Council Local Environmental Plan (LEP) 1985 (amendment no. 41):

- 2(c) - Residential 'C' (Living Area)
- 3 (f) - Business ' $\mathrm{F}^{\prime}$ (Village)
- $\quad 4(\mathrm{a})$ - Industrial ' A ' (General).

Under Shoalhaven Draft LEP 2009, which was exhibited in July 2011, the land area is subject to the following land use classifications:

- $\quad$ R1-General Residential
- $\quad \mathrm{IN}_{1}$ - General Industrial
- B2 - Local Centre.

In addition to these, certain land areas within the development area are subject to a zoning of E2Environmental Conservation. The overall effect of the of Draft LEP 2009 was to confine the area allocated to residential development to the Crookhaven River catchment whilst maintaining the nonresidential uses as proposed in LEP 1985.

The surrounding properties predominantly include residential and commercial uses to the east. The location of the subject site and its surrounding environs is shown in Figure 2.1.

Figure 2.1: Subject Site and Its Environs


Image Source: Google Maps

### 2.1 Road Network

### 2.1.1 Adjoining Roads

## Culburra Road

Culburra Road is a Regional Road (RR7632) generally aligned in an east-west direction and carries approximately 5,200 vehicles per day ${ }^{1}$. It is a two-way, 7 metre wide road set within a 20 metre wide road reserve (approx.), configured with one lane in each direction. Culburra Road is the key link between Culburra and Nowra to the west and in the vicinity of the subject site is subject to a $100 \mathrm{~km} / \mathrm{hr}$ posted speed limit. North of Mayfield Road, the name of the road changes to Pyree Lane.

## Pyree Lane

Pyree Lane is a Regional Road (RR7632) aligned in a north-south direction and carries approximately 5,200 vehicles per day ${ }^{1}$. It is a two-way, 6 metre wide road set within an 11 metre wide road reserve (approx.), configured with one lane in each direction. Pyree Lane is the key link between Culburra and Nowra to the west and is subject to a $100 \mathrm{~km} / \mathrm{hr}$ posted speed limit. South of Mayfield Road, the name of the road changes to Culburra Road.

[^0]
## Existing Conditions

## Coonamia Road

Coonamia Road is a Local Road to the west of the site and is aligned in a north-south direction. It is a two-way, 7 metre wide road set within a 20 metre wide road reserve (approx.), configured with one lane in each direction and carries approximately 2,600 vehicles per day ${ }^{1}$. Coonamia Road is the sole link between Culburra and the coastal villages of Callala Bay, Callala Beach and Currarong to the south.

## Greenwell Point Road/ Kalandar Street

Greenwell Point Road is a Regional Road (RR7632) aligned in an east-west direction. It is a two-way, 6 metre wide road set within a 13 metre wide road reserve (approx.), configured with one lane in each direction and carries approximately 6,000 vehicles per day ${ }^{1}$. Greenwell Point Road provides the sole road access between Nowra and the coastal village of Greenwell Point. West of McKay Street in East Nowra, the name of the road changes to Kalandar Street.

## Forest Road

Forest Road is a Local Road aligned in an east-west direction. It is a two-way, 7 metre wide road set within a 20 metre wide road reserve (approx.), configured with one lane in each direction and carries approximately 2,600 vehicles per day ${ }^{1}$. Forest Road is the key link between the coastal villages of Callala and Currarong and the Princes Highway. Forest Road was recently upgraded as a flood free connection to the Princes Highway.

## Princes Highway

The Princes Highway is a State Road ( $\mathrm{HW}_{1}$ ) aligned in a north-south direction and is the key coastal route between Sydney and the Victorian border. Through Nowra, the Princes Highway is a two-way, 12.8 metre wide road set within a 22 metre wide road reserve (approx.), configured with two lanes in each direction and carries approximately 35,000 vehicles per day ${ }^{1}$.

### 2.1.2 Surrounding Intersections

The following key intersections currently exist in the vicinity of the site:

- Culburra Road/ Coonamia Road (unsignalised)
- Pyree Lane/ Greenwell Point Road (unsignalised)
- Princes Highway/ Kalandar Street (signalised)
- Princes Highway/ Forest Road (unsignalised).


### 2.2 Traffic Volumes

GTA Consultants commissioned traffic movement counts and travel time surveys on key intersections and roads surrounding the site as shown in Figure 2.2. The intersection traffic movement counts were undertaken by Skyhigh during the following peak periods:

- Friday 04 May 2012: 7:00am to 9:00am and 4:00pm to 6:00pm
- Saturday 05 May 2012: 12:00pm to 2:00pm.

The existing weekday AM and PM peak hour traffic volumes are summarised in Figure 2.3 with Saturday peak hour traffic volumes summarised in Figure 2.4. Full results of the traffic movement counts are contained in Appendix A.

Figure 2.2: Traffic Count and Travel Time Survey Locations

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## Existing Conditions

### 2.3 Relevant Transport Studies

### 2.3.1 Long Bow Point Golf Course, Traffic and Parking Assessment

A traffic and parking assessment was prepared by Traffic Solutions Pty Ltd in March 2012 to support a Development Application (DA) for a proposed 18 hole championship golf course at Long Bow Point, Culburra. The proposed golf course is located west of the established residential areas of Culburra on the southern side of Culburra Road as shown in Figure 2.5.

The report states that vehicle access to the golf course is proposed directly from Culburra Road via a new intersection approximately 1km west of Strathstone Street as shown in Figure 2.5 .

Figure 2.5: Proposed Golf Course, Long Bow Point, Culbura


Image Source: Google Maps
Traffic generation estimates for the proposed golf course development were based on surveys undertaken on a Wednesday and Saturday at Nowra Golf Club. It was estimated that the proposed golf course would generate 33 and 53 vehicle movements (two-way) during the weekday AM and PM peak periods, and 66 vehicle movements (two-way) during a Saturday peak hour.

As GTA Consultants understands it, the DA (DA11/1728) for the development has been submitted to Shoalhaven City Council and is currently under review.

### 2.3.2 Princes Highway Upgra de REF

Construction works for the upgrade of a 6.3 km section of the Princes Highway between Kinghorne Street and Forest Road, south of Nowra have commenced. Key features of the upgrade include:

## Existing Conditions

- Duplication of the Princes Highway from two to four lanes
- Realignment of the Princes Highway between Warra Warra Road and Forest Road, west of the roads present alignment
- Reconstructing the Forest Road intersection to allow all turning movements
- Relocation of the BTU Road intersection approximately 400 metres north of its existing location
- New pedestrian and cycling facilities.

On completion the upgrade will provide consistent four lane conditions between Bomaderry and Jervis Bay Road ( 4.5 km south of Forest Road).

The Review of Environmental Factors (REF) for the project was completed in November 2009 and estimated a $2.5 \%$ linear growth rate in traffic volumes on this section of the Princes Highway up to 2028. This growth rate was based on the recorded Annual Average Daily Traffic (AADT) counts taken on the Princes Highway (station number 07.707) over a five year period; 25,636 in 2003 to 27,888 in 2008. The REF projected traffic volumes are summarised in Table 2.1.

Table 2.1: Princes Highway Projected Traffic Volumes (Princes Highway Upgrade REF, 2009)

| Year | Annual Average Daily Traffic (AADT) |
| :---: | :---: |
| 2012 | 29,511 |
| 2018 | 33,688 |
| 2022 | 34,919 |
| 2028 | 39,250 |

### 2.4 Public Transport

Culburra is served by one public bus service, route 729, which operates between Bomaderry Railway Station, Nowra, Orient Point and Culburra Beach via Greenwell Point Road, Pyree Lane and Culburra Road as shown in Figure 2.6. This service is operated by Kennedy's Bus and Coach with the weekday frequency summarised in Table 2.2. The nearest bus stops to the proposed development are located on Prince Edward Avenue in the vicinity of Culburra shops, east of the site.

Table 2.2: Route 729 Bus Service Frequency

| Direction | No. of AM Weekday Services | No. of PM Weekday Services |
| :---: | :---: | :---: |
| Culbura to Nowra | 3 | 2 |
| Nowra to Culbura | 2 | 3 |

It is noted that this service does not operate on weekends or public holidays.
Kennedy's Bus and Coach also provide school bus services to Culburra and Orient Point.

Figure 2.6: Public Bus Services


Source: Kennedy's Bus and Coach website: www.kennedystours.com.au/ (accessed 06September 2012)

### 2.5 Pedestria n Infrastruc ture

There is no pedestrian infrastructure in the immediate vicinity of the subject site. The nearest dedicated pedestrian infrastructure is located in the established residential areas of Culburra to the east of the proposed development. The footpath network within the urban area of Culburra is limited with many streets having wide verges in lieu of paved footpaths.

### 2.6 Cycle Infrastruc ture

The nearest dedicated cycle infrastructure to the site is a 2.5 metre wide shared path adjacent to Prince Edward Avenue between The Lake Circuit and Penguins Head Road as shown in Figure 2.7. Further discussion on cycling infrastructure is contained in Section 3 and Section 5.

Figure 2.7: Prince Edward Avenue Existing Shared Path, Proposed and Possible Future Cycleways


Source of Base Plan: Bicycle Information NSW website www.bicycleinfo.nsw.gov.au/maps/ (accessed 06 September 2012)

### 2.7 Crash Analysis

GTA Consultants obtained vehicle crash data from RMS for the following eleven key intersections between Culburra and Nowra for the five year period to June 2012:
i Culburra Road/ Coonamia Road
ii Culburra Road/ Mayfield Road
iii Greenwell Point Road/ Pyree Lane
iv Greenwell Point Road/ Jindy Andy Lane
v Greenwell Point Road/ Mayfield Road
vi Greenwell Point Road/ Millbank Road/ Worrigee Road
vii Princes Highway/ Kalandar Street
viii Coonamia Road/ Currarong Road/ Forest Road

## Existing Conditions

ix Kalandar Street/ Kinghorne Street/ Albatross Road
x Princes Highway/ Forest Road
xi Princes Highway/ Moss Street.
The accident history within 100 m of the approaches to the above intersections were analysed to determine whether there any accident clusters or safety issues at these locations. The results of the crash analysis are presented below and full details are contained in Appendix B.

Table 2.3: Reported Crash Summary (J uly 2007 - J une 2012)

| Intersection | No. of C rashes <br> (within 100 m of intersection) |
| :--- | :---: |
| Culbura Road/ Coonamia Road | 2 (resulting in 3 people injured) |
| Culbura Road/ Mayfield Road | 2 (resulting in 1 person injured) |
| Greenwell Point Road/ Pyree Lane | 2 (resulting in 2 people injured) |
| Greenwell Point Road/ Jindy Andy Lane | 5 (resulting in no injuries) |
| Greenwell Point Road/ Mayfield Road | 8 (resulting in 1 person injured) |
| Greenwell Point Road/ Millbank Road/ Worigee Road | 4 (resulting in 4 people injured) |
| Princes Highway/ Kalandar Street | 18 (11 injury crashes resulting in 13 people injured) |
| Coonamia Road/ Curarong Road/ Forest Road <br> (includes Forest Road/ Callala Bay Road intersection) | 4 (1 injury crash resulting in 1 person injured) |
| Kalandar Street/ Kinghome Street | 4 (3 injury crashes resulting in 3 people injured) |
| Princes Highway/ Forest Road | 18 (12 injury crashes resulting in 22 people injured) |
| Princes Highway/ Moss Street | 29 (13 injury crashes resulting in 14 people injured) |

As shown in Table 2.3 the more notable accident clusters occurred at the following intersections:

- Greenwell Point Road/ Mayfield Road
- Princes Highway/ Kalandar Street
- Princes Highway/ Moss Street
- Princes Highway/ Forest Road.

The location of crashes in the vicinity of these intersections is shown graphically in Figure 2.8 to Figure 2.11.

Figure 2.8: Reported Crashes- Greenwell Point Rd/ Mayfield Rd intersection (J uly 2007 - J une 2012)


Source: RMS
Figure 2.9: Reported Crashes - Princes Highway/ Kalandar Street intersection (J uly 2007 - J une 2012)


Source: RMS

Figure 2.10: Reported Crashes - Princes Highway/ Moss Street intersection (J uly 2007 - J une 2012)


Source: RMS
Figure 2.11: Reported Crashes - Princes Highway/ Forest Road intersection (J uly 2007 - J une 2012)


Source: RMS

## Existing Conditions

The Princes Highway/ Forest Road intersection is currently a painted seagull arrangement as shown in Figure 2.12 and Figure 2.13 .

Figure 2.12: Princes Highway at Forest Road (looking south)


Figure 2.13: Princes Highway at Forest Road (looking north)


Image Source: Google Maps
As stated in Section 2.3.2, works have commenced for the upgrade of the Princes Highway between Kinghorne Street and Forest Road. As part of these upgrade works the Forest Road intersection will be upgraded to a kerbed seagull intersection allowing all turning movements as shown in Figure 2.14.

Figure 2.14: Princes Highway/ Forest Road Intersection - Proposed Seagull Arrangement


Source: RMS
The upgraded works will significantly improve safety at the intersection by providing a dedicated left turn slip lane from the Princes Highway and a vegetated median along the Princes Highway, providing greater protection for turning vehicles.

The intersections of Princes Highway with Kalandar Street and Moss Street are the most heavily trafficked and congested intersections in the study area. Due to these significantly higher traffic volumes and the congestion, which results in many rear end shunts, it is not unexpected that there are more crashes at this location.

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## Existing Conditions

The road alignment on the eastbound approach to Mayfield Road is relatively tight and warning signage (Reduce Speed, 45km/h advisory signage and Chevron Alignment Markers \& safety barrier has been provided to try and minimise the crash risk.

### 2.8 Intersection Operation

The Director-General's Environmental Assessment Requirements (DGR's) dated 27 May 2010 stipulated that network modelling be undertaken using TRACKS modelling software to assess the current (and future) performance of the intersections in the study area.

TRACKS is a suite of software programs produced by Gabites Porter Consultants of Christchurch, New Zealand. The traffic authority has a TRACKS model of the area that is required for use as the basis of our analysis. However, we have been unable to obtain a model to date. On $14^{\text {th }}$ May 2012, Scott Wells, Traffic and Transport Unit Manager, Shoalhaven City Council wrote:
"There has been an ITUC meeting to discuss third party use of TRACKS models, I am yet to see the minutes, however there was general acceptance, subject to conditions. It was agreed there would be no fee for use however a condition would be to ensure the level of model validation in the area required for testing was improved prior to use. Engagement for that purpose would be by Council at your clients cost, the updated model and all data would be Council's. Once the model is updated and agreed sufficient for use for your purposes, and all costs to achieve the improved level of validation have been paid for, you could then use the model subject to conditions."

Negotiations between GTA Consultants and Shoalhaven City Council took place for the release of the TRACKS model to undertake to the required network modelling. In subsequent correspondence Scott Wells wrote on $24^{\text {th }}$ January 2013:
"... we (Council Traffic Unit) never asked for TRACKS modelling, it was an RMS request for DPI to include in DGRs and this was included in the DGRs without consultation with Council. The only available TRACKS model that covers this area is an AADT model and there has never been specific validation in the area subject of assessment. This means without checking against field data there is no high level confidence in regards to the strategic distributions to/from the site and Princes Highway.... For the purposes of your study use of SIDRA at Princes Highway/Moss street and Princes Highway/Kalandar Street should suffice in my view".

Consequently, assessment of the traffic impact of the proposed development has been undertaken using SIDRA INTERSECTION ${ }^{2}$, a computer based modelling package which calculates intersection performance on an individual intersection basis. Conversely TRACKS software assesses traffic impacts on a network wide scale.

The commonly used measure of intersection performance, as defined by Roads and Maritime Services (RMS), is vehicle delay. SIDRA INTERSECTION determines the average delay that vehicles encounter and provides a measure of the level of service.

Table 2.4 shows the criteria that SIDRA INTERSECTION adopts in assessing the level of service.

[^1]
## Existing Conditions

Table 2.4: SIDRA INTERSECTION Level of Service Criteria

| Level of Service (LOS) | Average Delay per <br> vehicle (secs/veh) | Traffic Signals, <br> Roundalbout | Give Way \& Stop Sign |
| :---: | :---: | :---: | :---: |
| A | Less than 14 | Good operation | Good operation |
| B | 15 to 28 | Good with acceptable <br> delaysand spare <br> capacity | Acceptable delays and <br> spare capacity |
| C | 43 to 4256 | Satisfactory | Satisfactory, but accident <br> study required |
| D | 57 to 70 | Nearcapacity | Nearcapacity, accident <br> study required |
| E | Greater than 70 | At capacity, at signals <br> incidents will cause <br> excessive delays | At capacity, requires other <br> control mode |
| F | Extra capacity required | Extreme delay, major <br> treatment required |  |

The road network under consideration as part of this assessment is shown in Figure 2.3 and Figure 2.4 and includes the following eleven intersections:
i Culburra Road/ Coonamia Road (priority controlled)
ii Culburra Road/ Mayfield Road (priority controlled)
iii Greenwell Point Road/ Pyree Lane (priority controlled)
iv Greenwell Point Road/ Jindy Andy Lane (priority controlled)
v Greenwell Point Road/ Mayfield Road (priority controlled)
vi Greenwell Point Road/ Millbank Road/ Worrigee Road (stop controlled)
vii Princes Highway/ Kalandar Street (signalised)
viii Coonamia Road/ Currarong Road/ Forest Road (priority controlled)
ix Kalandar Street/ Kinghorne Street (roundabout)
x Princes Highway/ Forest Road (priority controlled)
xi Princes Highway/ Moss Street (signalised).

### 2.8.1 Base Scenario - 120th Highest Annual Hour

As stated in Section 2.2, traffic volumes were recorded in May 2012. The queuing at the intersection was also recorded so that the base year model could be validated.

However, as the NSW South Coast is a popular tourist destination subject to influxes of tourists over long weekends and during school holidays, particularly during the summer school holidays. To reflect this seasonal increase in traffic volumes in the vicinity of the development site, the $120^{\text {th }}$ highest annual hour (HH) has been used as the Design Hourly Volume (DHV) for the base traffic scenario as instructed by Scott Wells of Shoalhaven City Council in correspondence dated $24^{\text {th }}$ January 2013:
"We would also consider the assessment incomplete without undertaking adjustment of the surveyed flows to equivalent $120^{\text {th }}$ HH demand flow levels consistent with AUSTROADS guidelines".

Use of the $120^{\text {th }} \mathrm{HH}$ as the DHV reflects a peak hour within the highest $1 \%$ of all hourly volumes recorded over a year and as such represents a period of high seasonal traffic volumes.

The traffic counts undertaken as part of this assessment were undertaken in May 2012 during a period of low tourist activity. Subsequently, the recorded peak hour traffic flows require application of an appropriate growth factor to represent the $120^{\text {th }} \mathrm{HH}$.

## Existing Conditions

## Calculation of Growth Factors

The calculation of an appropriate growth factor to be applied to the May 2012 recorded traffic flows was undertaken by the Traffic and Transport Unit of Shoalhaven City Council. The growth factors were calculated by analysing 2008 annual hourly traffic volume data from Greenwell Point Road and Forest Road as well as data from the RMS permanent traffic count stations at Falls Creek, north of Jervis Bay Road (approximately 4 km south of Forest Road, count station 07.053). The growth factors to be applied to the recorded May 2012 traffic flows are summarised below with further details of the calculation of growth factors contained in Appendix C:

Table 2.5: Growth Factors to be Applied to May 2012 Recorded Fows to Calculate $\mathbf{1 2 0} \mathbf{0 4}$ HH Rows

| Road | Friday AM <br> Peak Hour <br> $(8-9 a m)$ | Friday PM <br> Peak Hour <br> $(4-5 p m)$ | Saturday Peak <br> Hour <br> $(12-1 p m)$ |
| :--- | :---: | :---: | :---: |
| Local Roads \& Traffic to/ from Princes Highway | 1.12 | 1.41 | 1.25 |
| Princes Highway through traffic (north-south movements)* | 1.13 | 1.07 | 1.18 |

* Relates to the Princes Highway intersections with Moss Street, Kalandar Street and Forest Road.

Source: Shoalhaven City Council (Appendix C)
Application of the growth factors shown in Table 2.5 to the surveyed May 2012 peak hour volumes and the equivalent $120^{\text {th }} \mathrm{HH}$ traffic flows used for the base scenario assessments of the Friday AM, Friday PM and Saturday Peak hours are shown graphically in Figure 2.15, Figure 2.16 and Figure 2.17 respectively.

### 2.8.2 Existing Intersection Operation

Table 2.6 presents a summary of the existing operation of the eleven intersections within the road network under consideration using $120^{\text {th }} \mathrm{HH}$ traffic volumes to account for the seasonal growth in traffic in the region. Full results presented in Appendix D of this report while the intersection layouts from SIDRA are contained in Appendix E.




## Existing Conditions

Table 2.6: Existing Operating Conditions (Equivalent 120 ${ }^{\mathrm{Hh}}$ Highest Annual Hour)

| Intersection | Peak | Degree of Saturation (DOS) | Delay (sec) | 95th Percentile Queue (m) | Level of Senice (LOS) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Culburra Road/ Coonamia Road | Friday AM | 0.223 | 6.8 | 5 | NA |
|  | Friday PM | 0.145 | 6.4 | 4 | NA |
|  | Saturday | 0.140 | 6.7 | 3 | NA |
| Culbura Road/ Mayfield Road | Friday AM | 0.198 | 0.6 | 4 | NA |
|  | Friday PM | 0.214 | 0.8 | 11 | NA |
|  | Saturday | 0.144 | 0.8 | 7 | NA |
| Greenwell Point Road/ Pyree Lane | Friday AM | 0.224 | 8.9 | 7 | NA |
|  | Friday PM | 0.558 | 10.0 | 37 | NA |
|  | Saturday | 0.212 | 7.5 | 6 | NA |
| Greenwell Point Road/J Jindy Andy Lane | Friday AM | 0.227 | 3.5 | 7 | NA |
|  | Friday PM | 0.215 | 4.1 | 6 | NA |
|  | Saturday | 0.181 | 3.6 | 5 | NA |
| Greenwell Point Road/ Mayfield Road | Friday AM | 0.200 | 1.8 | 9 | NA |
|  | Friday PM | 0.235 | 2.3 | 21 | NA |
|  | Saturday | 0.170 | 2.3 | 15 | NA |
| Greenwell Point Road/ Millbank Road/ Womigee Road | Friday AM | 0.391 | 7.5 | 15 | NA |
|  | Friday PM | 0.235 | 6.2 | 7 | NA |
|  | Saturday | 0.153 | 5.6 | 4 | NA |
| Princes Highway/ Kalandar Street | Friday AM | 1.049 | 86.4 | 403 | F |
|  | Friday PM | 1.101 | 119.6 | 558 | F |
|  | Saturday | 0.968 | 63.3 | 389 | E |
| Coonamia Road/ Currarong Road/ Forest Road | Friday AM | 0.117 | 11.9 | 3 | NA |
|  | Friday PM | 0.249 | 12.1 | 8 | NA |
|  | Saturday | 0.202 | 12.2 | 6 | NA |
| Kalandar Street/ Kinghome Street | Friday AM | 0.729 | 15.4 | 69 | B |
|  | Friday PM | 0.774 | 15.7 | 78 | B |
|  | Saturday | 0.373 | 9.8 | 18 | A |
| Princes Highway/ Forest Road | Friday AM | 0.739 | 2.0 | 5 | NA |
|  | Friday PM | 0.766 | 4.7 | 17 | NA |
|  | Saturday | 0.598 | 3.2 | 13 | NA |
| Princes Highway/ Moss Street | Friday AM | 1.025 | 89.2 | 388 | F |
|  | Friday PM | 1.237 | 199.9 | 796 | F |
|  | Saturday | 0.887 | 49.5 | 200 | D |

On the basis of the above assessment, under equivalent $120^{\text {th }} \mathrm{HH}$ traffic volumes:

- the priority controlled intersections operate well during the three respective peak periods with minimal delays and queues on all approaches
- it is clear from this analysis that the existing Princes Highway intersections at Kalandar Street and Moss Street experience significant delays during these peak periods, particularly during the Friday AM and Friday PM peak periods. The other intersections in the study area currently operate satisfactorily.


### 2.9 Performance of Rural Road Network

GTA Consultants undertook an assessment of the existing local road network surrounding the development site to assess road design aspects (cross-section parameters) for compliance with AUSTROADS Standards and RMS Guidelines in relation to:

- Lane widths
- Rural turning lanes
- Intersection configurations
- Warrants for overtaking lanes

As advised in correspondence from Scott Wells dated $19{ }^{\text {th }}$ February 2013, this assessment was to be limited to the local road network surrounding the site as a similar assessment of the State Road network in the vicinity of the site (i.e. the Princes Highway) was not required.

This issue is considered in detail at Section8.

## Development Proposal

## 3. Development Proposal

### 3.1 Land Uses

The proposed mixed use subdivision development is comprised of six stages as shown in Figure 3.1.
Figure 3.1: West Culbura Subdivision - Proposed Stages


Background Image Source: J ohn Toon Pty Ltd, 2010
As GTA Consultants understands it, the main areas to be developed are Stages 3,4 and 5 which have an indicative capacity of approximately 800 dwellings and units on lots ranging from $550 \mathrm{~m}^{2}$ to $900 \mathrm{~m}^{2}$. It is anticipated that these areas will be constructed and sold in about an eight years period from 2014.

Stage 1 is proposed for 60 small-lot housing units for the 55 plus aged group and three five-storey apartment buildings as well as commercial and tourist-orientated uses. It is anticipated that this development will commence at the end of 2013 and be completed within three years.

Stage 2, south of Culburra Road, is proposed as the site for 80 small-lot housing units for the 55 plus aged group, six five-storey apartment blocks with each block estimated to contain 40 two bedroom apartments. It is anticipated that the development of the five-storey apartments will be developed over a 20 year period.

It is understood that 26 industrial lots are also proposed as part of the development over an area of 6.8ha (approx.) in Stage 6. This area is located on the northern side of Culburra Road, east of the existing industrial area as shown in Figure 3.1.

An indicative development schedule is summarised in Table 3.1.

## Development Proposal

Table 3.1: Indicative Development Schedule

| Stage | Area (approx) | $\begin{aligned} & \text { Zoning } \\ & \text { (LEP 1985) } \end{aligned}$ | Zoning (Draft LEP 2009) | Land Use/ Capacity |
| :---: | :---: | :---: | :---: | :---: |
| 1 | 1 ha | 3(f) Business Zone | B2 Local Centre | Tourist/ commercial use |
| 2 | 1.6 ha | 2(c) <br> Residential | R1 General Residential E2 Environmental Conservation | $48 \times 2$ bedroom apartments <br> $21 \times$ small-lot 2 bedroom single storey villas for the $55+$ aged group |
| 3-5 | 83.5 ha | 2(c) <br> Residential | R1 General Residential | $500 \times$ dwelling houses <br> $30 \times$ mixed-use, 3 bedroom town houses (The Circus) <br> $26 \times 2$ bedroom small lot dwellings for the $55+$ aged group (behind The Circus) <br> $10 \times 1$ bedroom units <br> $35 \times 2$ bedroom units <br> $15 \times 3$ bedroom units <br> $3 x$ tourist sites (1.25 ha total area) - motel, cafe, gift shop and restaurant |
| 6 | 6.8 ha | 4(a) <br> Industrial | IN1 General Industrial | 28 industrial lots |
| Total |  |  |  | Tourist/ commercial use <br> $83 \times 2$ bedroom a partments <br> $47 \times$ sma ll-lot 2 bedroom single storey villas for the <br> $55+$ aged group <br> $500 \times$ dwelling houses <br> $30 \times$ mixed-use, 3 bedroom town houses (The Circus) <br> $10 \times 1$ bedroom units <br> $15 \times 3$ bedroom units <br> $3 \times$ tourist sites ( 1.25 ha total area) - motel, cafe, gift shop and restaurant <br> 28 industrial lots |

As shown in Table 3.1 following full site development, Stage 2 to 5 will contain 685 residential dwellings and Stage 6 will contain 28 industrial lots. The development will be phased with Stages 3-5 developed over a period of approx. 8 years.

An indicative concept layout of the West Culburra subdivision is shown in Figure 3.2.


## Development Proposal

### 3.2 Vehicle Access

It is anticipated that vehicle access to Stage 1 will be via Canal Street East and the extension of Brighton Parade. Access to Stage 2 will be via Culburra Road. Access to the new industrial area is anticipated to be direct from Culburra Road or via Regmoore Close, in the adjacent existing industrial area which currently has access to Culburra Road via Strathstone Street.

A new Collector Road is proposed as part of the development to provide access to Stages 3, 4 and 5 with two connections to Culburra Road as shown in Figure 3.3.

Figure 3.3: Proposed Collector Road and Access Locations


Image Source: J ohn Toon Pty Ltd, 2010
As GTA Consultants understands it, the eastern access will be the primary means of accessing Stages 3, 4 and 5. Plans prepared by Allen, Price and Associates propose the new Collector Road to be set within a 25 metre wide road reserve. It is understood that the western access to the development will not be provided at this stage. The location of the western access is anticipated to be in the vicinity of the existing unsealed access track.

Further discussion on vehicle access arrangements are contained in Section 4 of this report.

### 3.3 Pedestrian and Bicycle Facilities

The new Collector Road is considered to be the optimal alignment for a cycleway connecting the development Stages 3,4 and 5 to Culburra shops in the established area of Culburra to the east of the

Development Proposal
development. A cycleway following the alignment of the foreshore adjacent to Stages 3,4 and 5 is also proposed as part of the development.

The indicative alignment of cycleways proposed as part of the development is shown in Figure 3.4.
Figure 3.4: Proposed Cycleways


Source: Modified from Allen, Price \& Associates (J uly, 2012)
It is anticipated that the proposed cycleway shown in Figure 3.4 will operate as off-road shared pedestrian and cyclist paths or as separated off-road paths.

Further discussion on pedestrian and bicycle facilities are contained in Section 5 of this report.

### 3.4 Parking

The car parking requirements for different development types are contained in Shoalhaven City Council Car Parking Code (DCP 18). It is anticipated that car parking for the development will be provided in accordance with the requirements of DCP 18 .

## 4. Vehicle Access

### 4.1 Introduction

It is proposed that access to the site is provided by means of a roundabout at the proposed eastern intersection of the new Collector Road with Culburra Road. Ultimately a western access to the development will be provided which will also form an intersection at Culburra Road.

### 4.2 Intersection Assessment and Concept Design

GTA Consultants undertook an assessment of the proposed eastern intersection of the new Collector Road with Culburra Road to determine the most appropriate location, layout and dimensional requirements with consideration of the topography, sight distances and road geometry. Based on this assessment, GTA Consultants produced an indicative concept design of the intersection consisting of a four arm single lane roundabout layout as shown in Figure 4.1.

The roundabout has been sketched to be generally in accordance with Austroads Guide to Road Design, Part 4 B: Roundabouts (second edition, 2011) and has an island diameter of 28 m and a circulating carriageway width of 7.2 m . The roundabout has been designed to accommodate an Austroads standard 19 metre long articulated semi-trailer. GTA Consultants undertook a swept path assessment of the proposed layout to confirm vehicle manoeuvrability.

Figure 4.1: Eastem Access Preliminary Concept Layout


The southern leg of the roundabout has been included in the concept design to show an alternative access point to the proposed Long Bow Point golf course described in Section 2.3. This location is approximately 350 metres north of the golf course access proposed on the straight section of Culburra Road shown in Figure 2.5. An unsealed access road to the land on which the Long Bow Golf Point golf

## Vehicle Access

course is proposed is located on the inside of the bend, where the southern leg of the roundabout is proposed.

Combining the eastern access of the Collector Road with an access for the proposed Long Bow golf course would offer safety advantages over an independent, priority controlled intersection for the golf course, particularly on a rural road within a $100 \mathrm{~km} / \mathrm{hr}$ speed limit zone. It is understood that the golf course development is subject to consent by Shoalhaven City Council.

DCP 100 requires the subdivision road network to connect with the external road network in a manner which maximises movement efficiency for all traffic routes. A roundabout will offer the most effective means of managing traffic at the intersection, minimising the average delay on all approaches.

The preliminary concept design of the proposed roundabout, including long sections is contained in Appendix F.

### 4.2.1 Speed Zonings

It is anticipated that the existing 50km/hr speed limit in place on Culburra Road, approximately 350 metres east of Strathstone Street, will be extended to a point west of the roundabout intersection to provide a $50 \mathrm{~km} / \mathrm{hr}$ speed limit on the western approach in line with the NSW Speed Zoning Guidelines (RMS, 2011). It is also anticipated that the proposed Collector Road will be subject to a $50 \mathrm{~km} / \mathrm{hr}$ speed limit in line with the NSW Speed Zoning Guidelines and Shoalhaven City Council Subdivision Code - DCP 100 (2002)

The proposed roundabout will form the entrance to the largest section of the development within Stages 3,4 and 5, providing access to 800 dwellings. It is considered appropriate for the $50 \mathrm{~km} / \mathrm{hr}$ speed zone to be extended westwards beyond the proposed roundabout to provide a $50 \mathrm{~km} / \mathrm{hr}$ speed limit on all approaches to the intersection. Such a speed zoning is considered appropriate to the proposed road environment and conducive to a safer environment for pedestrians, cyclists and vehicles. The lowering of vehicle speeds along Culburra Road will result in intersection spacing becoming more appropriate to road speed.

## 5. Susta ina ble Transport Infrastruc ture

### 5.1 Policy and Planning Guidelines

### 5.1.1 NSW 2021: A Plan to Make NSW NumberOne (2011)

NSW 2021: A Plan to Make NSW Number One, Illawarra Regional Action Plan (2011) sets out priorities for the Illawarra subregion which is comprised of Wingecarribee, Wollongong, Shellharbour, Kiama and Shoalhaven LGA's in line with the State plan; NSW 2021. The Regional Action Plan states that to provide effective and integrated regional transport, 'whole of region' transport planning is required to improve:

- Public transport links
- Patronage on public transport
- Public transport access to key employment areas
- Planning for local and regional road infrastructure.

These local priorities relate to the following NSW 2021 goals:

- Goal 7: Reduce travel times
- Goal 8: Grow patronage on public transport by making it a more attractive choice
- Goal 10: Improve road safety.

The Illawarra Regional Action Plan contains a priority action to develop an Illawarra transport strategy in line with the NSW Long Term Master Plan to provide a clear direction for all transport modes.

### 5.1.2 Integrating Land Use and Transport (2001)

The NSW Department of Urban Affairs and Planning's Integrating Land Use and Transport (2001) policy package provides guidelines for planning and development which aim to encourage development that:

- increases access to public transport, walking and cycling
- encourages people to travel shorter distances and make fewer trips
- reduces car dependency.

The aim of integrating land use and transport is to ensure that urban structures, building forms, land use locations, development designs, subdivisions and street layouts achieve the following planning objectives:

- improving access to housing, jobs and services by walking, cycling and public transport
- increasing the choice of available transport and reducing dependence on cars
- reducing travel demand including the number of trips generated by development and the distances travelled, especially by car
- supporting the efficient and viable operation of public transport services.

Integrating Land Use and Transport identifies the following key transport planning concepts which recognise people's basic travel needs:

- Convenience - the transport mode needs to be easy to find and use, and to transfer from one mode to another.


## Sustaina ble Transport Infrastructure

- Information - reliable information at accessible locations is essential to encourage use of various travel alternatives.
- Proximity - transport facilities and services, such as cycle paths and bus services, need to be in close, convenient and obvious locations to people's trip origins and destinations.
- Destination choice - the more destinations that can be linked on a public transport route, the more attractive it will be.
- Directness - routes should take the shortest and least deviating course, with priority to achieve fast travel times for walking, cycling and public transport (e.g. pedestrian links, dedicated bus lanes, and bikeways).
- Security - the environment for walking and waiting needs to be comfortable and safe from personal attack or conflicts with traffic (e.g. waiting areas sheltered from the elements, natural surveillance, good lighting, bike lanes on major roads).


### 5.1.3 NSW Planning Guidelines for Wa lking and Cycling (2004)

The NSW Planning Guidelines for Walking and Cycling (2004) aims to assist land-use planners and related professionals to improve consideration of walking and cycling in their work. The guidelines have been designed to provide a walking and cycling focus to the NSW Government's Integrating Land Use \& Transport Planning policy package.

### 5.1.4 Shoa Iha ven Integrated Transport Strategy (2000)

The Shoalhaven Integrated Transport Strategy (2000) aims to promote the Shoalhaven as a place where:

- Public transport is readily available, safe, efficient and regarded as a viable alternative to private car use
- Alternative forms of transport, other than motor car are promoted and encouraged.

The Transport Strategy highlights that the difficulties of public bus transport provision in Shoalhaven LGA, given the dispersed coastal communities and population:
"At the moment the bus operators are providing services that are generally only just viable. This does not include the school routes. The bus operators would provide improved services (frequencies and destinations) if the patronage justified it".

The Transport Strategy notes the issues raised by residents related to public transport around the Shoalhaven as:

- Lack of timetable integration for bus to bus and bus to train
- Door to door journey time
- Fare structure; both cost and lack of a single ticket system
- Safety
- Comfort and convenience
- Lack of transport after hours and during weekends, public holidays and school holidays.

The Transport Strategy contains the following critical implementation actions:

- Action 11 - Require the provision of bus routes, footpaths and cycleways in all new housing development areas.


## Sustaina ble Transport Infrastructure

### 5.1.5 Shoa Iha ven Subdivision Code - DCP 100 (2002)

DCP 100 contains the following objectives to promote sustainable transport as part of subdivision developments in the Shoalhaven LGA:
> - All residents should have the opportunity to walk or cycle to the nearest community facilities, such as shops and schools. They should also be provided with safe and convenient links to other major destinations external to the neighbourhood. The design of the street network should encourage walking and cycling along quieter local streets, reducing the need for separate rights of way for cycle and pedestrian linkages.
> - Increase opportunities for choice in mode of transport and provide cost effective and energy efficient public transport services that are accessible and convenient to the community.

### 5.2 Bus Routes

Culburra and the adjoining village of Orient Point currently have very limited public transport with only five weekday bus services between Nowra and no weekend or public holiday services.

It is envisaged that the proposed Collector Road through Stages 3,4 and 5 will serve as the key route for the Culburra-Nowra public bus service and for school bus services. Given this, all access points to the development from Culburra Road must be designed to accommodate bus turning manoeuvres. As a result, the concept layout for the eastern Collector Road/ Culburra Road intersection described in Section 4 has been designed to accommodate bus turning movements.

DCP 67 states that the road network should be designed so that generally all residential development is within 400 metres of the bus network and possible bus stops. Integrating Land Use and Transport states that 400 metres is a desirable walking distance to access a bus route as it is within a 5 minute walk. DCP 100 suggests that $85 \%$ of dwellings within a subdivision be within 500 metres safe walking distance from an existing or potential bus route. Approximately $90 \%$ of the lots in Stages 3, 4 and 5 are within 400 metre walking distance of the proposed Collector Road, the likely key bus route through the development. All lots within Stages 1 and 2, as well as the industrial area, are proposed to be located within 200 metres of Culburra Road.

Following full site development it is anticipated that the Culburra to Nowra bus services will deviate from Culburra Road into the proposed development at the eastern access, travel the new Collector Road and return to Culburra Road at the western access, with the reverse scenario occurring for Nowra to Culburra services. In the interim, Stages 3,4 and 5 will be accessible solely by the eastern access. As such, it is anticipated that bus services both to and from Culburra will travel a 'loop' route, entering and exiting the development at the eastern access.

The key destinations for residents of the development are likely to be similar to destinations for current Culburra and Orient Point residents; Nowra town centre, Bomaderry Railway Station and Culburra shops.

The development offers the opportunity to provide improved public transport services to all residents of Culburra and Orient Point through higher frequency weekday services and the provision of weekend and public holiday services. It is anticipated that increased residential density attributed to the development will improve the economic viability of increased public transport provision and justify significant improvements in public transport infrastructure for Culburra and Orient Point.

## Sustaina ble Transport Infrastructure

### 5.2.1 Consultation with Bus Operator

Shoalhaven Integrated Transport Strategy highlights the importance of early consultation with bus operators to ensure early provision of bus services in new residential developments which offers advantages for:

- The vendor - the availability of public transport can be a strong selling point
- The bus operator - if new families move into a new development and have a service available immediately it is likely that the operator will have regular patrons. Once a family has to purchase a second car it is unlikely that the family will use the bus again.

GTA Consultants liaised with the current bus service provider of the Culburra-Nowra service, Kennedy's Bus and Coach to discuss future bus service provision in light of the proposed development. Initial consultation with David Tagg of Kennedy's has indicated that the operator welcomes additional patronage within their normal bus operations and would be happy to extend their current service arrangements.

The operator highlighted the road geometry requirements for the development to allow for a minimum 12.5 to 13.5 metre long bus and noted that consideration needs to be given to the provision of disabled access for low floor wheelchair buses which will be compulsory on all route services by 2020.

Correspondence with Kennedy's Coaches is contained in Appendix G.

### 5.3 Bus Stops

Integrating Land Use and Transport outlines that public transport stops should be designed and managed to provide the following:

- good pedestrian access from surrounding areas, including direct, safe and well-lit street connections or pedestrian links, safe pedestrian crossings and clear lines of sight to the stop
- safe, well-lit and comfortable waiting areas with shelter and information on available services
- direct and convenient connections from the footpath to the shelter/ waiting area and from the shelter/ waiting area to the doors of the public transport vehicle, and vice versa
- clear identification of the public transport nodes and access points by attractive design and signage
- access for all users, including appropriate provision for people with disabilities
- bus stops with adequate lighting, shelter and passive security.

DCP 100 contains the requirements for the location and design of bus stops within subdivision developments:

- Public transport stops provide for pedestrian safety, security, comfort and convenience
- Bus stops are designed to prevent vehicles from overtaking a stationary bus, or vehicle speeds are reduced to ensure safe pedestrian crossing
- Bus stops are located and designed to provide shelter, seats, adequate lighting and timetable information, are overlooked from nearby buildings, and are located to minimise adverse impact on the amenity of nearby dwellings.

DCP 100 suggests the following measures are considered as part of the development:

## Sustaina ble Transport Infrastructure

- Routes for regular bus services are designed for a minimum pavement width of 9.0 metres
- Bus stops are, or are planned for 400 metre spacings where the route serves residential development
- The siting of bus stops is related to the pedestrian path network.

Given the proximity of Stages 3, 4 and 5 to the proposed Collector Road, the location of bus stops is crucial to ensure ease of access for residents and in turn encourage the use of sustainable transport. Figure 5.1 has been prepared to show indicative locations of three possible bus stops along the Collector Road and to show the residential and industrial lots within a 400 metre/ 5 minute walking catchment of each stop. It is anticipated that new bus stops will also be provided on Culburra Road adjacent to Stages 1 and 2 to encourage public transport usage by residents.

It is recommended that all new bus stops provide the following as a minimum:

- Shelter
- Seating
- Lighting
- Timetable information.

Figure 5.1: Potential bus stop locations and indicative 400 metre walking catchment


### 5.4 Walking and Cycling Network

### 5.4.1 Street Pattem

The NSW Planning Guidelines for Walking and Cycling highlights the importance of street pattern as a determinant of walkability and cycleability. Street pattern determines how far a person can travel by foot or by bicycle within a set timeframe, as well as the feel of a neighbourhood from a pedestrian's or

## Sustaina ble Transport Infrastructure

cyclist's perspective. Local streets such as those within Stages 3,4 and 5 should be highly interconnected with many junctions onto main road making walking and cycling trips short and direct. In contrast, layouts with unconnected cul-de-sacs make walking and cycling trips longer and less interesting as sight lines are limited and there are few (if any) alternative route options available for any one trip as shown in Figure 5.2.

Figure 5.2: Street Pattems and Accessibility
 stops (Ref 5.10)


A more organic street pattern for direct access to centres and public transport stops (Ref 5.10)


Disconnected culs-de-sac design showing long walk from A to B and only one route available

Modified grid design showing shorter walk from $\mid A$ to $B$ and several routes available


Source: NSW Planning Guidelines for Walking and Cycling
The indicative street layout for development Stages 3, 4 and 5 shown in Figure 3.2 has many similarities to the 'perfect' street pattern shown in Figure 5.2, offering many options for access through the development. This through site permeability is vital for encouraging walking and cycling for short trips within these stages and to key destinations.

### 5.4.2 Key Destinations

The key walking and cycling destinations in proximity of development areas are likely to be:

- Culburra shops - east of the development area
- Crookhaven River foreshore area - north of the development area
- Beaches - east of the established urban area of Culburra.

Providing direct and easy walking and cycling access to these locations via dedicated, high quality facilities is imperative to promote walking. The indicative street layout for Stages 3,4 and 5 shown in Figure 3.2 has numerous streets aligned in a north-south direction enabling direct walking access to the Crookhaven River foreshore area from within the development. The proposed public reserves shown as green in Figure 3.2 provide further options for direct walking and cycling access to the foreshore area.

### 5.4.3 Wa lking a nd Cycling Infrastructure

The Shoalhaven Integrated Transport Strategy lists the following as a critical implementation action:

- Action 4 - Build a network of cycleways and footpaths which link schools, shops, employment areas, bus interchanges and also link outlying villages to Park and Ride interchanges along the trunk corridor.

The cycle network proposed as part of the development (Figure 3.4) includes two key routes:
i East-west route along the foreshore area providing access to Culburra shops
ii East-west route along the proposed Collector Road and the northern side of Culburra Road providing access to Culburra shops and the sports oval.

## Sustaina ble Transport Infrastructure

As stated previously, it is anticipated that these key routes will be either a shared pedestrian/ cyclist paths or as separate pedestrian and cyclist paths. The foreshore route offers an excellent opportunity as a recreational walking and cycling route and to promote cycle tourism. The connection of both of these routes to the established areas of Culburra and in particular to Culburra shops is crucial in discouraging private car use for short trips within the area. The connection of these proposed routes to the existing footpath network requires further consideration. It is recommended that the walking and cycling network within the established areas of Culburra be upgraded to provide a consistent standard of facility and in particular to improve access to the beaches, east of the town as shown in Figure 2.7 and Figure 3.4 .

The provision of a dedicated, high quality facility adjacent to the proposed Collector Road is imperative to improve accessibility within the development area. As stated previously, approximately $80 \%$ of the land in spatial units 3,4 and 5 are within 400 metre walking distance of the Collector Road and as such a dedicated facility aligned along the Collector will serve as the key route for cycling and walking within the development area.

## Bic yc le Infrastruc ture Selection

The NSW Bicycle Guidelines (RMS, 2005) derive appropriate cycling facilities based on vehicle speeds and volumes as shown in Figure 5.3 with the different methods of separation shown in Figure 5.4.

The traffic generation of the development is discussed in Section 7, where it is estimated that Stages 3, 4 and 5 will generate in the order of 6,480 vehicle movements (two-way) per day along the proposed Collector Road. Given this, and assuming a $50 \mathrm{~km} / \mathrm{hr}$ speed limit along the Collector Road, Figure 5.3 indicates that bicycle lanes or shoulders would likely be inappropriate for the Collector Road. As such it is recommended that a separated facility be provided along this alignment.

Given the higher traffic volumes along Culburra Road, it is recommended to provide a separated facility along the northern side of Culburra Road to provide access between Stages 3, 4 and 5, Culburra shops and Stage 1.

## Sustaina ble Transport Infrastructure

Figure 5.3: Appropriate cycling facilities based on vehicle speeds and volumes


Source: NSW Bicycle Guidelines (RMS)

Figure 5.4: Methods of Separation


Physical separation - off-road bicycle paths


Visual separation - on-road bicycle lanes or shoulder


Source: NSW Bicycle Guidelines (RMS)

## Separated and Shared Paths

There are advantages and disadvantages associated with shared and separated paths as summarised in Table 5.1.

Table 5.1: Pros and Cons for Shared and Separated Paths

| Type of Path | Pros | Cons |
| :--- | :--- | :--- |
| Shared | Suitable formost users <br> Lower construction cost than separated <br> paths <br> Require less road reserve space | Not satisfactory in high usage areas-may <br> discourage walking amongst the young, <br> seniors and people with disabilities <br> May be ineffective if thorough planning <br> (consultation, observation and demand <br> estimation) has not been ca mied out |
| Separated | Eliminatesconflicts occuming between <br> different user types <br> Will allow the bicycle path to operate at a <br> higher speed than in the case of shared <br> paths, which may make it more desirable for <br> commutercyclists | Ineffective if pedestrian movement along or <br> acrossthe bicycle path is not minimised <br> Ineffective if both paths are not of a <br> comparable standard relative to the <br> requirements of each type of user |

The provision of separated cycleways is an important element in creating an attractive cycling environment. As shown in Figure 5.5, the general population fit into four categories of cyclists: strong and fearless, enthused and confident, interested but concerned and no way, no how.

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Figure 5.5: Four Types of Cyclists


The "strong and the fearless" demographic ride regardless of road conditions: riding is a strong part of their identity and they are undeterred by road conditions.

The "enthused and confident" demographic are and could be attracted to regular riding by continuing to address the barriers to cycling: shorter trip distances, better bicycle facilities, better end-of-trip facilities.

The "interested but concerned" demographic hear messages about how easy it is to cycle, but they are afraid to ride. They don't like the cars speeding down their streets. They get nervous thinking about what would happen to them on a bicycle when a driver runs a red light, or passes too fast and too close.

The "no Way, no How" group is not interested in cycling at all, for reasons of topography, inability, or simply a complete and utter lack of interest (PBT,2010).

For NSW, 2006 Census data indicates that less than $1 \%$ of work trips were made by bicycles - the cyclists that are "strong and fearless". To achieve the NSW 2021 target to more than double the mode share of all bicycle trips, the occasional or non-riders of the "enthused and confident" and "interested but concerned" demographic must take up cycling.

Australian research conducted in 2008 by RACQ Market and Communications Research (December 2008) revealed $85 \%$ of people support the provision of off-road bicycle paths and $69 \%$ of males and $74 \%$ of females would bicycle more regularly if dedicated lanes and off-road routes were more readily available (refer to Table 5.2). Recent bicycle counts in the City of Sydney show increases of $60 \%$ and $48 \%$ in the AM and PM periods, respectively, over the past year on routes where separated cycleways have been constructed. Dedicated cycling lanes and off-road routes must connect to all popular destinations to encourage high levels of uptake.

Table 5.2: Non-regular cyclists preference for separation from vehicles

| What would make you bicycle more regularly? | Male | Female |
| :--- | ---: | ---: |
| Ava ila bility of bicycle dedic ated lanes and off road routes | $69 \%$ | $74 \%$ |
| Increased driver a wa reness of bicycle safety and sha ring the road | $49 \%$ | $56 \%$ |
| Availa bility of bicycle parking or bicycle lockers | $33 \%$ | $41 \%$ |
| Having more cyclists on the road | $31 \%$ | $36 \%$ |
| Increased knowledge of the road rules | $12 \%$ | $22 \%$ |

Source: Environmetrics Pty Ltd (2006) Sydney Cycleway Research: Intemet survey forthe City of Sydney
Implementing safe cycling routes, separated from vehicles, is more likely to encourage this group and increase levels of cycling.

### 5.4.4 Design Requirements

DCP 100 outlines the design requirements for walking and cycling infrastructure within subdivision developments. A minimum of a 1.2 metre wide footpath is required on local and collector streets within

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a subdivision. As such it is anticipated that footpaths will be provided on both sides of all local streets within the subdivision at a minimum 1.2 metres wide.

The design requirements for bicycle facilities are included in the NSW Bicycle Guidelines and Austroads Part 6A: Pedestrian and Cyclist Paths (2009). The width requirements for shared paths are summarised in Table 5.3.

Table 5.3: Separated Path Widths

|  | Path width (m) |  |  |
| :---: | :---: | :---: | :---: |
|  | Local access path | Commuter path | Recreational path |
| Desirable minimum width | 2.5 | 3.0 | 3.5 |
| Minimum width - typical maximum | $2.5^{1}-3.0^{2}$ | $2.5^{1}-4.0^{2}$ | $3.0^{1}-4.0^{2}$ |

1. A lesser width should only to be adopted where cyclist volumes and operational speeds will remain low
2. A greater width may be required where the numbers of cyclists and pedestrians are very high or there is a high probability of conflict between users (e.g. people walking dogs, roller bladders and skaters etc.).

Source: Austroads Part 6A: Pedestrian and Cyclist Paths
If the cycleways proposed as part of the development are determined to be shared paths, it is recommended to provide a minimum 3 metre width given their potential as recreational routes.

### 5.5 Susta ina ble Tra nsport Infra struc ture Summa ry

- The development offers the opportunity to provide improved public transport services to all residents of Culburra and Orient Point through higher frequency weekday bus services and the provision of weekend and public holiday services.
- The early provision of bus services for the development areas is considered paramount in promoting public transport usage. Initial consultation with the bus operator indicates that the operator welcomes additional patronage within their normal bus operations and would be happy to extend their current service arrangements.
- The indicative street layout for Stages 3, 4 and 5 offers many options for access through the development area, providing vital through site permeability. The street layout is consistent with the aims of the NSW Planning Guidelines for Walking and Cycling to promote walking and cycling, particularly for short trips.
- A minimum of a 1.2 metre wide footpath is required on local and collector streets within a subdivision in line with DCP 100.
- The proposed alignment of cycleways; adjacent to the foreshore and Collector Road, and also providing access to Culburra shops is considered conducive to promoting pedestrian and cyclist access, particularly for short trips.
- The foreshore route is considered to be an excellent opportunity as a recreational cycle route and to promote cycle tourism in the region.
- With consideration of likely vehicle speeds and volumes along the Collector Road, it is recommended that a separated cycle facility be provided along this alignment in line with the NSW Bicycle Guidelines.
- For shared pedestrian and cycle paths associated within the development, it is recommended to provide a minimum 3 metre width given their potential as recreational routes.


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- Given the traffic volumes along Culburra Road, it is recommended to provide a separated facility along the northern side of Culburra Road to provide access between Stages 3, 4 and 5, Culburra shops and Stage 1.


## 6. Loading Facilities

It is understood that refuse collection for the residential areas of the subdivision will involve kerbside collection by a 12.5 metre long Council garbage vehicle and as such the eastern and western accesses from Culburra Road to Stages 3,4 and 5 must be able to accommodate such a vehicle.

The concept roundabout layout at the eastern access of the Collector Road and Culburra Road has been designed to accommodate a 12.5 m long rigid vehicle and subject to a swept path assessment.

## 7. Traffic Impact Assessment

### 7.1 Traffic Generation

### 7.1.1 Design Rates

Traffic generation estimates for the proposed development would usually be sourced from the Guide to Traffic Generating Developments (RMS, 2002). Estimates of peak hour and daily traffic volumes using this guide are set out in Table 7.1.

Table 7.1: Estimated Development Traffic Generation (RMS Rates)

| Stage | Land Use | Design Generation Rates |  | Traffic Generation Estimates (vehicles) |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Peak Hour | Daily | Peak Hour | Daily |
| 2 | Residential ( $48 \times 2$ bedroom apartments) | $0.4-0.5$ vehicle movements/ dwelling | $4-5$ vehicle movements/ dwelling | 20-24 vehicle movements/ hour | 192-240 vehicle movements/ day |
|  | Residential 55+aged group ( $21 \times$ small-lot2 bedroom single storey villas) | 0.2 vehicle movements/ dwelling | $1-2$ vehicle movements/ dwelling | 5 vehicle movements/ hour | 21-44 vehicle movements/ day |
| 3-5 | Residential 500 dwelling houses | 0.85 vehicle movements/ dwelling | 9 vehicle movements/ dwelling | 425 vehicle movements/ hour | 4,500 vehicle movements/ day |
|  | Residential $30 \times$ mixed-use, 3 bedroom town houses (The Circus) | $0.5-0.65$ vehicle movements/ dwelling | $5-6.5$ vehicle movements/ dwelling | 15-20 vehicle movements/ hour | 150-195 vehicle movements/ day |
|  | Residential <br> $26 \times 2$ bedroom small lot dwellings for the $55+$ aged group (behind The Circus) | 0.2 vehicle movements/ dwelling | 1-2 vehicle movements/ dwelling | $3-6$ vehicle movements/ hour | 26-52 vehicle movements/ day |
|  | Residential <br> $10 \times 1$ bedroom units | $0.4-0.5$ vehicle movements/ dwelling | $4-5$ vehicle movements/ dwelling | $4-5$ vehicle movements/ hour | 40-50 vehicle movements/ day |
|  | Residential <br> $35 \times 2$ bedroom units | $0.4-0.5$ vehicle movements/ dwelling | $4-5$ vehicle movements/ dwelling | 14-18 vehicle movements/ hour | 140-175 vehicle movements/ day |
|  | Residential $15 \times 3$ bedroom units | $0.5-0.65$ vehicle movements/ dwelling | 5-6.5 vehicle movements/ dwelling | $8-10$ vehicle movements/ hour | 75-98 vehicle movements/ day |
| Sub-Total |  |  |  | 494-513 vehicle movements/ hour | 5,144-5,354 vehicle movements/ day |
| 6 | Industrial (6.8 hec tares) | 8.9 vehicle movements/ perhectare | - | 60 vehicle movements/ hour | - |
|  |  |  | Total | 554-573 vehicle movements/ hour | - |


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Table 7.1 indicates that based on the adoption of the traffic generation rates contained in the RMS' Guide to Traffic Generating Developments:

- the residential component of the proposed development (Stages 2 to 5) could be expected to generate up to 513 vehicle movements (two-way) and the industrial component of the development (Stage 6) up to 60 vehicle movements (two-way) during a typical weekday peak hour.
- Following full-site development, the west Culburra subdivision development could be expected to generate up to 573 vehicle movements (two-way) during a typical weekday peak hour.


### 7.1.2 Empirical Traffic Generation Leaving the Culbura Area

The Traffic and Transport Unit of Shoalhaven City Council provided empirical traffic generation rates for the established area of Culburra. The trip rates were calculated by using traffic volume data from 2008 annual hourly counts on Greenwell Point Road and Forest Road and residential occupancy data from the 2011 census to determine the number of vehicle trips entering the regional road network (west of Culburra) per occupied dwelling in the relevant peak hours. Table 7.2 summarises the empirical traffic generation rates with further details contained in Appendix C.

Table 7.2: Empirical Traffic Generation Rates (Shoalhaven City Council)

| Peak Hour Scenario | Traffic Generation Rate <br> (vehicles per occupied dwelling per peak hour) |
| :---: | :---: |
| Friday AM | 0.22 |
| Friday PM | 0.21 |
| Saturday | 0.23 |

Source: Shoalhaven City Council (Appendix C)
As shown in Table 7.2, based on analysis of the existing traffic generating characteristics of the established urban area of Culburra, it is anticipated that the proposed development would generate $0.22,0.21$ and 0.23 vehicle trips per dwelling during the respective Friday AM, Friday PM and Saturday peak hours. As advised by Shoalhaven City Council these rates are based on detached dwellings and reductions could be justified for semi-detached or non-detached dwellings.

Application of the empirical traffic generation rates provided by Shoalhaven City Council to the proposed development results in an estimated traffic generation estimates as summarised in Table 7.3.

Table 7.3: Estimated Development Traffic Generation (Shoalhaven City Council Empirical Traffic Generation Rates)

| Peak Hour <br> Scenario | Traffic Generation Rate <br> (Shoallhaven City Council) | Proposed Residential <br> Dwellings <br> (Stages 2-5) | Traffic Generation <br> Estimates (vehicles) |
| :---: | :---: | :---: | :---: |
| Friday AM | 0.22 |  | 151 |
| Friday PM | 0.21 | 685 | 144 |
| Saturday | 0.23 |  | 158 |

As shown in Table 7.3, based on the adoption of the traffic generation rates provided by Shoalhaven City Council the proposed development is expected to generate 151,144 and 158 vehicle trips per occupied dwelling during the respective Friday AM, Friday PM and Saturday peak hours on the regional road network (west of Culburra).

It is noted that approx one quarter of the residential dwellings proposed as part of the development are either semi- detached or non-detached ( 160 of the 685 total dwellings proposed). As such the traffic generation estimates contained in Table 7.3 represents a conservative estimate of the traffic that is likely to be generated by the development (i.e. greater than what could be expected).

Given the existing traffic generating characteristics of the established area of Culburra on the regional road network (west of Culburra) and the proportion of dedicated housing for the over 55 aged group proposed as part of the development, the adoption of the empirical traffic generation estimate is considered appropriate.

Council did however indicate that the remainder of the peak hour traffic generation (that is the RMS rate of 0.85 per dwelling minus the 0.22 leaving Culburra) would travel to the village of Culburra, This is discussed later in Section 7.4.

### 7.2 Distribution and Assignment

The directional distribution and assignment of traffic generated by the proposed development will be influenced by a number of factors, including the:
i configuration of the arterial road network in the immediate vicinity of the site
ii existing operation of intersections providing access between the local and arterial road network
iii distribution of households in the vicinity of the site
iv surrounding employment centres, retail centres and schools in relation to the site
$\checkmark$ configuration of access points to the site.
The distribution and assignment of traffic generated by the proposed development has been informed by the following:

- Analysis 2006 Census Journey to Work Data
- Analysis of the May 2012 Traffic Count Data
- Consultation with Shoalhaven City Council's Traffic and Transport Unit.


### 7.2.1 2006 CensusJ oumey to Work Data

To determine the distribution of development traffic on the surrounding road network, 2006 Census Journey to Work (JTW) data (Bureau of Transport Statistics, 2001) has been analysed to provide an understanding of the existing travel patterns of residents of the area. JTW data provides information relating to the origin and destination of journeys to work and includes the mode of travel. The smallest geographical area for which Journey to Work data is available is a Travel Zone. The development areas are located in Travel Zone 3733; Kinghorne, as shown in Figure 7.1.

Figure 7.1: Bureau of Transport Statistics Travel Zone


Source: Bureau of Transport Statistics website: http://www.bts.nsw.gov.au/ (accessed 12 September 2012)
GTA Consultants undertook analysis of all trips made from Travel Zone 3733 by the JTW mode of 'Car as Driver (including 'Truck and Motorbike'), which represented $65 \%$ of all journeys to work in the travel zone. Destinations of these trips were grouped into four broad geographical categories; north, south, east of Princes Highway and west of Princes Highway. There were also a number of trips that were classified as 'Unknown', 'Sydney Undefined', 'NSW Undefined' or 'No Fixed Address' in the data. These results have been grouped together into a category referred to as 'Unknown'.

The results of the 2006 Census JTW data for residents of Travel Zone 3733 travelling by a mode of car/ truck /motorcycle as driver is summarised in Figure 7.2 below.

Figure 7.2: 2006 Census J oumey to Work Destinations


Data Source: Bureau of Transport Statistics website: http://www.bts.nsw.gov.au/ (accessed 12 September 2012)
Further analysis of the JTW data revealed the following:

- Approximately $26 \%$ of all journeys to work made from travel zone 3733 had a destination within the same travel zone
- Approximately $30 \%$ of all journeys to work made from travel zone 3733 were to destinations east of the Princes Highway, i.e. they did not cross the Princes Highway

It is noted that whilst a high proportion of existing residents have work destinations within travel zone 3733, new residents are more likely to work further afield and as such are likely to travel to Nowra and beyond for work.

### 7.2.2 May 2012 Traffic Count Data

The May 2012 traffic count data was analysed to determine the relative distribution of vehicles into and out of the road network under consideration. This was determined by calculating the percentage of vehicles entering or exiting this network during the respective peak hours based on the May 2012 intersection traffic counts. The six entry/exit points to the network are shown graphically in Figure 7.3 and includes the following intersections:

- NORTH - Princes Highway/ Moss Street
- WEST - Princes Highway/ Moss Street, Kalandar Street/ Kinghorne Street
- SOUTH - Princes Highway/ Forest Road
- EAST - Greenwell Point/ Pyree, Culburra Road/ Coonamia Road and the Coonamia Road/ Currarong Road.

Figure 7.3: Directional Distribution Analysis - Road Network Entry/ Exit Locations


Background Image Source: Google Maps
While this area does not represent a 'closed' network, whereby there are other entry and exit points to the road network, the analysis was undertaken to provide an indication of the existing directional distribution of vehicles into and out of the road network under consideration. The existing directional distribution of vehicles into and out of this road network during the Friday AM, Friday PM and Saturday peak hours is summarised in Table 7.4, Table 7.5 and Table 7.6 respectively.

Table 7.4: Existing Directional Distribution - Friday AM Peak Hour (May 2012 Traffic Counts)

| Direction | Entry/Exit Location | Outbound |  | Inbound |  |
| :--- | :--- | :---: | :---: | :---: | :---: |
| West | Moss Street | 518 | $19 \%$ | 260 | $7 \%$ |
|  | Kinghome Street | 417 | $15 \%$ | 405 | $10 \%$ |
| North | North of Moss Street | 1085 | $40 \%$ | 1680 | $42 \%$ |
|  | South of Forest Road | 468 | $17 \%$ | 1211 | $30 \%$ |
| East | Culbura Road | 143 | $6 \%$ | 225 | $6 \%$ |
|  | Currarong Road | 22 | $1 \%$ | 45 | $1 \%$ |
|  | Greenwell Point Road | 66 | $\mathbf{2 \%}$ | $\mathbf{1 4 2}$ | $4 \%$ |

Table 7.5: Existing Directional Distribution - Friday PM Peak Hour (May 2012 Traffic Counts)

| Direction | Entry/ Exit Location | Outbound |  | Inbound |  |  |  |  |  |  |  |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Moss Street | 383 | $10 \%$ | 597 | $17 \%$ |  |  |  |  |  |  |
|  | Kinghome Street | 388 | $10 \%$ | 432 | $13 \%$ |  |  |  |  |  |  |
| North | North of Moss Street | 1489 | $37 \%$ | 1580 | $46 \%$ |  |  |  |  |  |  |
|  | South of Forest Road | 1342 | $33 \%$ | 600 | $17 \%$ |  |  |  |  |  |  |
| East | Culbura Road | 239 | $6 \%$ | 127 | $4 \%$ |  |  |  |  |  |  |
|  | Curarong Road | 48 | $1 \%$ | 20 | $1 \%$ |  |  |  |  |  |  |
|  | Greenwell Point Road | 137 | $3 \%$ | 64 | $2 \%$ |  |  |  |  |  |  |
|  |  |  |  |  |  |  | Total | $\mathbf{4 0 2 6}$ | $\mathbf{1 0 0 \%}$ | $\mathbf{3 4 2 0}$ | $\mathbf{1 0 0 \%}$ |

Table 7.6: Existing Directional Distribution - Saturday Peak Hour (May 2012 Traffic Counts) Development Traffic Distribution

| Direction | Entry/ Exit Location | Outbound |  | Inbound |  |
| :--- | :--- | :---: | :---: | :---: | :---: |
|  | Moss Street | 266 | $8 \%$ | 431 | $14 \%$ |
|  | Kinghome Street | 269 | $8 \%$ | 299 | $9 \%$ |
| North | North of Moss Street | 1287 | $40 \%$ | 1332 | $42 \%$ |
|  | South of Forest Road | 962 | $30 \%$ | 683 | $22 \%$ |
| East | Culbura Road | Curarong Road | 176 | $6 \%$ | 190 |
|  | Greenwell Point Road | 36 | $1 \%$ | $\mathbf{2 8}$ | $6 \%$ |

### 7.2.3 Consultation with Shoalhaven City Council

Shoalhaven City Council's Traffic and Transport Unit was consulted extensively in relation to the directional distribution of traffic generated by the proposed development in an effort to gain an understanding of known local traffic patterns and key trip generators. In addition to the empirical traffic generation rates shown in Table 7.3, Council's Traffic and Transport Unit provided directional splits for traffic generated by the development in the relevant peak hours as shown in Table 7.7.

Table 7.7: Empirical Traffic Generation Rates and $120^{\mathrm{h}} \mathrm{HH}$ Directional Splits (Shoallhaven City Counc il)

| Peak Hour Scenario | Traffic Generation Rate <br> (Shoallhaven City Council) | Directional Split- $\mathbf{1 2 0}^{\mathrm{H}} \mathbf{~ H H}$ |  |
| :---: | :---: | :---: | :---: |
|  |  | Outbound <br> (westbound) | Inboundl <br> (eastbound) |
| Friday AM | 0.22 | $76 \%$ | $24 \%$ |
| Friday PM | 0.21 | $25 \%$ | $75 \%$ |
| Saturday | 0.23 | $50 \%$ | $50 \%$ |

Source: Shoalhaven City Council (Appendix C)
On the basis of the above, the directional distribution of traffic generated by the development on the road network west of Culburra during the Friday AM, Friday PM and Saturday peak hours are summarised in Table 7.8, Table 7.9 and Table 7.10 respectively (the numbers in brackets in the 'Outbound' and 'Inbound' columns represent the corresponding number of vehicles).

This distribution is also shown graphically in Figure 7.4, Figure 7.5 and Figure 7.6.
Figure 7.7 , Figure 7.8 and Figure 7.9 have been prepared to show the estimated increase in turning movements on the surrounding road network following full site development.

Table 7.8: Proposed Directional Distribution - Friday AM Peak Hour

| Direction | Route | Outbound | Inbound |
| :---: | :---: | :---: | :---: |
| West | West of Princes Highway (via J indy Andy Lane) | 10\% (15) | 2\% (3) |
|  | West of Princes Highway (via Millbank Road) |  |  |
|  | West of Princes Highway (via Kalandar Street) | 11\%(17) | 2\% (3) |
| North | North of the Shoalhaven River (via J indy Andy Lane) | 11\%(17) | 3\% (5) |
|  | North of the Shoalhaven River (via Millbank Road) |  |  |
|  | North of the Shoalhaven River (via Kalandar Street) | 16\% (23) | 5\% (7) |
| South | South of Forest Road | 8\% (12) | 6\% (9) |
| East | East of Princes Highway (via J indy Andy Lane) | 2\% (3) | 0\% (0) |
|  | East of Princes Highway (via Millbank Road) |  |  |
|  | East of Princes Highway (via Kala ndar Street) | 13\% (20) | 2\% (3) |
|  | East of Princes Highway (via Womigee Road orOld Southem Road) | 2\% (3) | 2\% (3) |
|  | East of Princes Highway (via Forest Road) | 3\% (5) | 2\% (3) |
|  | Total | 76\% (115) | 24\% (36) |

Table 7.9: Proposed Directional Distribution - Friday PM Peak Hour

| Direction | Route | Outbound | Inbound |
| :---: | :---: | :---: | :---: |
| West | West of Princes Highway (via Jindy Andy Lane) | 2\% (3) | 9\% (13) |
|  | West of Princes Highway (via Millbank Road) |  |  |
|  | West of Princes Highway (via Kalandar Street) | 3\% (4) | 11\%(16) |
| North | North of the Shoalhaven River (via J indy Andy Lane) | 2\% (3) | 10\% (14) |
|  | North of the Shoalhaven River (via Millbank Road) |  |  |
|  | North of the Shoalhaven River (via Kalandar Street) | 2\% (3) | 24\% (35) |
| South | South of Forest Road | 7\% (10) | 8\% (12) |
| East | East of Princes Highway (via J indy Andy Lane) | 1\% (2) | 0\% (0) |
|  | East of Princes Highway (via Millbank Road) |  |  |
|  | East of Princes Highway (via Kalandar Street) | 3\% (4) | 3\% (4) |
|  | East of Princes Highway (via Worrigee Road or Old Southem Road) | 2\% (3) | 2\% (3) |
|  | East of Princes Highway (via Forest Road) | 3\% (4) | 8\% (11) |
|  | Total | 25\% (36) | 75\% (108) |

Table 7.10: Proposed Directional Distribution - Saturday Peak Hour

| Direction | Route | Outbound | Inbound |
| :---: | :---: | :---: | :---: |
| West | West of Princes Highway (via Jindy Andy Lane) | 3\% (5) | 6\% (9) |
|  | West of Princes Highway (via Millbank Road) |  |  |
|  | West of Princes Highway (via Kalandar Street) | 5\% (8) | 3\% (5) |
| North | North of the Shoalhaven River (via J indy Andy Lane) | 5\% (8) | 4\% (6) |
|  | North of the Shoalhaven River (via Millbank Road) |  |  |
|  | North of the Shoalhaven River (via Kalandar Street) | 9\% (15) | 6\% (9) |
| South | South of Forest Road | 10\% (16) | 7\% (11) |
| East | East of Princes Highway (via J indy Andy Lane) | 2\% (3) | 0\% (0) |
|  | East of Princes Highway (via Millbank Road) |  |  |
|  | East of Princes Highway (via Kalandar Street) | 2\% (3) | 1\% (2) |
|  | East of Princes Highway (via Womigee Road or Old Southem Road) | 2\% (3) | 2\% (3) |
|  | East of Princes Highway (via Forest Road) | 6\% (9) | 7\% (11) |
|  | Greenwell Point | 6\% (9) | 14\% (23) |
|  | Total | 50\% (79) | 50\% (79) |








### 7.3 Traffic Impact

The West Culburra subdivision development will be the major source of growth in the Culburra area over the next 10 years. It is anticipated that the development will be completed in stages, with full site development reached approx. 8 years after commencement.

An assessment of the impacts that the anticipated development traffic would have on the surrounding road network can be made by comparing intersection performance prior to and following full site development.

The proposed development is anticipated to generate an additional 151,144 and 158 vehicle movements (two-way) on the road network west of Culburra during the respective Friday AM, Friday PM and Saturday peak hours.

Table 7.11 presents a summary of intersection operating conditions following full site development while full results are contained in Appendix D.

Table 7.11: Future Operating Conditions (Equivalent $\mathbf{1 2 0}{ }^{\mathrm{Hh}} \mathrm{HH}$ plus Development Traffic)

| Intersection | Peak | Degree of Saturation (DOS) | Delay (sec) | 95th Percentile Queue (m) | Level of Service (LOS) |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Culbura Road/ Coonamia Road | Friday AM | 0.234 | 6.3 | 6 | NA |
|  | Friday PM | 0.191 | 6.0 | 5 | NA |
|  | Saturday | 0.177 | 6.4 | 5 | NA |
| Culburra Road/ Mayfield Road | Friday AM | 0.251 | 0.8 | 5 | NA |
|  | Friday PM | 0.261 | 1.0 | 15 | NA |
|  | Saturday | 0.173 | 1.0 | 9 | NA |
| Greenwell Point Road/ Pyree Lane | Friday AM | 0.306 | 9.7 | 11 | NA |
|  | Friday PM | 0.701 | 12.1 | 68 | NA |
|  | Saturday | 0.307 | 8.4 | 10 | NA |
| Greenwell Point Road/J Jindy Andy Lane | Friday AM | 0.283 | 3.7 | 9 | NA |
|  | Friday PM | 0.273 | 4.4 | 8 | NA |
|  | Saturday | 0.218 | 3.8 | 6 | NA |
| Greenwell Point Road/ Mayfield Road | Friday AM | 0.237 | 2.1 | 11 | NA |
|  | Friday PM | 0.270 | 2.6 | 25 | NA |
|  | Saturday | 0.183 | 2.5 | 18 | NA |
| Greenwell Point Road/ Millbank Road/ Worigee Road | Friday AM | 0.448 | 7.6 | 18 | NA |
|  | Friday PM | 0.270 | 6.3 | 8 | NA |
|  | Saturday | 0.163 | 5.6 | 4 | NA |
| Princes Highway/ Kalandar Street | Friday AM | 1.082 | 100.2 | 461 | F |
|  | Friday PM | 1.109 | 129.3 | 539 | F |
|  | Saturday | 0.983 | 67.6 | 415 | E |
| Coonamia Road/ Curarong Road/ Forest Road | Friday AM | 0.124 | 12.1 | 4 | NA |
|  | Friday PM | 0.274 | 12.3 | 9 | NA |
|  | Saturday | 0.238 | 12.4 | 8 | NA |
| Kalandar Street/ <br> Kinghome Street | Friday AM | 0.738 | 15.8 | 73 | B |
|  | Friday PM | 0.786 | 16.2 | 82 | B |
|  | Saturday | 0.377 | 9.9 | 18 | A |
| Princes Highway/ Forest Road | Friday AM | 0.739 | 2.1 | 6 | NA |
|  | Friday PM | 0.766 | 5.7 | 20 | NA |
|  | Saturday | 0.598 | 3.8 | 15 | NA |
| Princes Highway/ Moss Street | Friday AM | 1.066 | 109.1 | 472 | F |
|  | Friday PM | 1.243 | 223.2 | 795 | F |
|  | Saturday | 0.873 | 48.3 | 199 | D |

On the basis of the above assessment, under equivalent $120^{\text {th }} \mathrm{HH}$ traffic volumes with the addition of traffic generated by the development:

- the priority controlled intersections operate well with minimal delays and queues on all approaches during the three respective peak periods
- As stated earlier, the Princes Highway intersections at Kalandar Street and Moss Street experience significant delays particularly during the Friday AM and Friday PM peak periods. However there is no significant change to the intersection Level of Service with the addition of development traffic.


### 7.3.1 Princes Highway/ Forest Road Intersection

It is noted that the Princes Highway/ Forest Road intersection will be upgraded as part of the upgrade of the Princes Highway between Kinghorne Street and Forest Road. No detailed plans of the upgraded intersection were available for this assessment but it is understood that a kerbed seagull arrangement will be provided allowing all turning movements as shown in the concept intersection layout shown in Figure 2.14. Given this, the SIDRA assessment with the addition of development traffic was undertaken using the existing intersection arrangement. In reality the new intersection will be upgraded prior to the development being completed.

### 7.3.2 Princes Highway Signalised Intersections

Table 7.12 provides a summary of the increase in traffic volumes from development traffic at the key Princes Highway signalised intersections of Kalandar Street and Moss Street. Unfortunately no detailed

Table 7.12: Signalised Intersection Traffic Volume Comparison

| Intersection | Existing Equivalent 120th HH <br> Traffic Volumes through Intersection <br> (vehicles) |  |  | Development Traffic Inc rease through <br> Intersection |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Friday AM | Friday PM | Saturday | Friday AM | Friday PM | Saturday |
| Princes Highway/ <br> Kalandar Street | 3,611 | 4,463 | 4,089 | $73(\% 2)$ | $66(2 \%)$ | $41(1 \%)$ |
| Princes Highway/ <br> Moss Street | 3,690 | 4,246 | 3,595 | $75(2 \%)$ | $75(2 \%)$ | $57(2 \%)$ |

As shown in Table 7.12 the addition of development traffic at the Princes Highway intersections of Kalandar Street and Moss Street represents only a marginal increase in the total volume of traffic travelling through the intersections following full site development. During the three peak hours examined, a maximum increase of $2 \%$ on existing traffic volumes is anticipated.

The impact of this additional traffic on intersections on the wider road network has been assessed using SIDRA INTERSECTION. Table 7.11 presents a summary of the anticipated future operation of the intersections following the full development of the site under $120^{\text {th }} \mathrm{HH}$ equivalent traffic volumes, with full results included in Appendix D.

### 7.3.3 Culburra Road/ new Collector Road intersection

GTA Consultants assessed the operation of the proposed eastern roundabout intersection of the new Collector Road with Culburra Road following full site development using SIDRA under equivalent $120^{\text {th }}$ HH traffic volumes.

For the purposes of assessment a 28 metre wide roundabout was modelled without a southern arm to the golf course (not withstanding this, a sensitivity test was undertaken using the predicted golf club traffic). A $1 \%$ linear growth rate was applied to the existing Culburra Road through traffic for full site development around 2032.

Table 7.13 presents a summary of the operation of a roundabout at the intersection following full site development, with full results presented in Appendix D of this report.

Table 7.13: Post-Development Intersection Operating Conditions- Three Amm Roundabout

| Intersection | Peak | Leg | Degree of Saturation (DOS) | Average Delay (sec) | 95th Percentile Queue (m) | Level of Service (LOS) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Culbura <br> Road Road/ <br> Collector <br> Road (east) | AM | East | 0.212 | 4.1 | 9 | A |
|  |  | North | 0.254 | 4.8 | 11 | A |
|  |  | West | 0.155 | 2.9 | 6 | A |
|  | PM | East | 0.285 | 6.4 | 14 | A |
|  |  | North | 0.099 | 5.4 | 4 | A |
|  |  | West | 0.319 | 4.3 | 14 | A |
|  | Sat | East | 0.255 | 5.3 | 12 | A |
|  |  | North | 0.171 | 4.8 | 7 | A |
|  |  | West | 0.194 | 3.5 | 7 | A |

On the basis of the above assessment, it is clear that a roundabout at the intersection of Culburra Road and the eastern access to the new Collector Road would be expected to operate well with minimal delays on all approaches.

Furthermore, a sensitivity test was undertaken to assess the additional effect of a fourth, southern arm roundabout to access the proposed Long Bow Point Golf Course. Traffic volumes for the golf club development were used in the assessment were taken from the Traffic and Parking Assessment of the golf course development prepared by Traffic Solutions Pty Ltd (Section 2.3.1). Table 7.14 presents a summary of the operation of a four arm roundabout at the intersection following full site development, with full results presented in Appendix D of this report.

Table 7.14: Post-Development Intersection Operating Conditions- Four Arm Roundabout

| Intersection | Peak | Leg | Degree of Saturation (DOS) | Average Delay (sec) | 95th <br> Percentile Queue (m) | Level of Service (LOS) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Culbura <br> Road/ <br> Collector <br> Road (east)/ <br> Golf Course | AM | South | 0.009 | 9.5 | 0 | A |
|  |  | East | 0.237 | 4.3 | 10 | A |
|  |  | North | 0.259 | 4.9 | 11 | A |
|  |  | West | 0.166 | 3.5 | 6 | A |
|  | PM | South | 0.047 | 10.6 | 2 | A |
|  |  | East | 0.298 | 6.4 | 14 | A |
|  |  | North | 0.103 | 5.5 | 4 | A |
|  |  | West | 0.331 | 4.6 | 14 | A |
|  | Sat | South | 0.036 | 10.2 | 1 | A |
|  |  | East | 0.286 | 5.4 | 13 | A |
|  |  | North | 0.177 | 5.0 | 7 | A |
|  |  | West | 0.212 | 4.2 | 8 | A |

On the basis of the above assessment, it is clear that with the addition of a four arm roundabout to provide access to the proposed Long Bow Point Golf Course, the intersection would be expected to operate well with minimal delays on all approaches.

### 7.4 Possible Traffic Impact In Culbura

As stated in Section 7.1.1, Council believe that the traffic generation from the site to Culburra would be 0.63 trips per dwelling in a Friday AM peak based on the premise that one dwelling generates 0.85 trips.

GTA undertook a survey of 3 residential areas in Culburra which appeared fully occupied on 26 February 2013. The traffic entering Glenhouse Way, Eastwood Avenue and Wentworth Street was recorded on a weekday AM \& PM peak. These 71 dwellings generated 45 trips which equates to 0.64 trips per dwelling. Assuming 0.22 of these head to destinations west of Culburra, the proposed development could generate 0.42 trips per dwelling to/within Culburra. This equates to 288 trips per hour, which is 4/5 per minute in the busiest hour.

The intersections in Culburra are very lightly trafficked and most are observed to operate at Level of Service A/B. The addition of the development traffic will not cause any significant changes in their operational performance.

## 8. Rural Road Assessment

As part of the assessment, Shoalhaven City Council has requested that GTA Consultants consider the following:
i Austroads cross-section warrants based on existing road characteristics.
ii Annual Average Daily Traffic (AADT) of study roads to understand the expected impact on an average day.
iii Peak Seasonal Daily Traffic (PSDT) to understand the expected impact during peak seasonal times of the year such as school holidays and across the summer period. This is based on the $120^{\text {th }}$ highest hour.
iv Warrants for overtaking lanes on roads in the study area.
GTA Consultants has responded to each assessment criteria as requested and they are set out in the following sections.

### 8.1 Austroads C ross-Section Wa rants

Guidance on single land rural road cross-sections have been sourced from Austroads Guide to Road Design Part 3: Geometric Design Table 4.5 which is reproduced in Figure 8.1.

Figure 8.1: Austroads Table 4.5: Single caniageway rural road widths (m)

| Element | Design AADT |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: |
|  | $\mathbf{1 - 1 5 0}$ | $\mathbf{1 5 0 - 5 0 0}$ | $\mathbf{5 0 0 - 1 , 0 0 0}$ | $\mathbf{1 , 0 0 0 - 3 , 0 0 0}$ | $>3,000$ |
| Traffic laness $^{(1)}$ | 3.7 <br> $(1 \times 3.7)$ | 6.2 <br> $(2 \times 3.1)$ | $6.2-7.0$ <br> $(2 \times 3.1 / 3.5)$ | 7.0 <br> $(2 \times 3.5)$ | 7.0 <br> $(2 \times 3.5)$ |
| Total shoulder | 2.5 | 1.5 | 1.5 | 2.0 | 2.5 |
| Minimum shoulder <br> seal <br> ${ }^{(2),(3),(4),(5),(6) ~}$ | 0 | 0.5 | 0.5 | 1.0 | 1.5 |
| Total carriageway | 8.7 | 9.2 | $9.2-10.0$ | 11.0 | 12.0 |

1. Traffic lane widths include centre-lines but are exclusive of edge-lines.
2. Where significant numbers of cyclists use the roadway, consideration should be given to fully sealing the shoulders. Suggest use of a maximum size 10 mm seal within a 20 km radius of towns.
3. Wider shoulder seals may be appropriate depending on requirements for maintenance costs, soil and climatic conditions or to accommodate the tracked width requirements for Large Combination Vehicles.
4. Short lengths of wider shoulder seal or lay-bys to be provided at suitable locations to provide for discretionary stops.
5. Full width shoulder seals may be appropriate adjacent to safety barriers and on the high side of superelevation.
6. A minimum 7.0 m seal should be provided on designated heavy vehicle routes (or where the AADT contains more than $15 \%$ heavy vehicles).

GTA Consultants has reviewed each of the study area roads identified in Figure 8.2 based on information from aerial photography (Nearmap, Six Maps, Google), Google Streetview and our previous site inspections of the study area, to categorise each section of road according to Figure 8.1. The results for rural sections of roads are summarised in Table 8.1 with the urban roads summarised in Table 8.2.

A rural road was considered any road without a formal kerb and an urban road was considered as any of those roads with a formal kerb passing through residential areas.

Rural Road Assessment

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Table 8.1: Summary of Existing Rural Road Characteristics

| Road | From | To | Urban / Rural | Speed Limit | Caniageway Width (m) | Average Sealed Shoulder (N/W) | Average Sealed Shoulder (S/E) | Average Gravel Shoulder (N/W) | Average Gravel Shoulder (S/E) | Total Camiageway Width | Rural <br> Design <br> AADT |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Forest Road | Coonamia Road | Callala Beach Road | Rural | 80 | 6 | 0 | 0 | 0.5 | 0.5 | 7 | 150 to 500 |
| Forest Road | Callala Beach Road | East Stump Road | Rural | 80/90/100 | 6 | 1 | 1 | 0 | 0 | 8 | 150 to 500 |
| Forest Road | East Stump Road | Chesnut Road | Rural | 100 | 6 | 0.5 | 0.5 | 1 | 1 | 9 | 150 to 500 |
| Forest Road | Chesnut Road | Manuka Road | Rural | 100 | 6 | 1 | 1 | 0.5 | 0.5 | 9 | 150 to 500 |
| Forest Road | Manuka Road | Gimlet Road | Rural | 100/90 | 6 | 1 | 1 | 1 | 1 | 10 | 150 to 500 |
| Forest Road | Gimlet Road | Vineyard Road | Rural | 90 | 6 | 1 | 1 | 1 | 1 | 10 | 150 to 500 |
| Forest Road | Vineyard Road | Westem Road | Rural | 90 | 6 | 1 | 1 | 1 | 1 | 10 | 150 to 500 |
| Forest Road | Westem Road | Princes Highway | Rural | 60 | 6 | 0 | 0 | 0 | 0 | 6 | 150 to 500 |
| Comerong Island Road | J indy Andy Lane | 90 degree left tum | Rural | 60 | 6 | 0 | 0 | 0 | 0 | 6 | 150 to 500 |
| Comerong Island Road | 90 degree left tum | Milbank Road | Rural | 60/80 | 6 | 0 | 0 | 0 | 0 | 6 | 150 to 500 |
| Terara Road | Milbank Road | Wondalga Crescent | Rural | 50/60 | 6 | 0 | 0 | 0 | 0 | 6 | 150 to 500 |
| Greenwell Point Road | West Street | Pyree Lane | Rural | 100/80/50 | 5.4 | 0 to 0.3 | 0 to 0.3 | 0 | 0 | 5.4 to 6 | 150 to 500 |
| Greenwell Point Road | Pyree Lane | J indy Andy Lane | Rural | 80 | 6.4 | 0.3 to 0.5 | 0.3 to 0.5 | 0 | 0 | 7 to 8.4 | 500 to 1000 |
| Greenwell Point Road | J indy Andy Lane | Apperleys Lane | Rural | 80 | 6.2 | 0 to 0.3 | 0 to 0.3 | 0 | 0 | 6.2 to 6.8 | 150 to 500 |
| Greenwell Point Road | Apperleys Lane | Worigee Road | Rural | 60/80 | 7 | 0.5 to 1 | 0.5 to 1 | 0 | 0 | 8 to 9 | 1000 to 3000 |
| Greenwell Point Road | Worigee Road | Old Southem Road | Rural | 60 | 6.2 | 0 to 3m | 0 to 2m | 0 | 0 | 6.2 to 11.2 | 500 to 1000 |
| Greenwell Point Road | Old Southem Road | Clipper Road | Rural | 60 | 6.7 | 0.3 | 0.3 | 0 | 0 | 7.3 | 150 to 500 |

Table 8.2: Summary of Existing Rural Road Characteristics

| Road | From | To | Urban / Rural | Speed Limit | Caniageway Width (m) | Average Sealed Shoulder (N/W) | Average Sealed Shoulder (S/E) | Average Gravel Shoulder (N/W) | Average Gravel Shoulder (S/E) | Total Camiageway Width | Rural Design AADT |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Moss Street | Wondalga Crescent | Princes Highway | Urban | 50 | 12 | Kerb | Kerb | N/A | N/A | 12 | N/A |
| Greenwell Point Road | Clipper Road | Mc Kay Street | Urban | 60 | 12 | Kerb | Kerb | N/A | N/A | 12 | N/A |
| Kalandar Street | McKay Street | Stua rt Street | Urban | 60 | 10.4 | Kerb | Kerb | N/A | N/A | 10.4 | N/A |
| Kala ndar Street | Stua rt Street | Wallace Street | Urban | 60 | 10.8 | Kerb | Kerb | N/A | N/A | 10.8 | N/A |
| Kala ndar Street | Wallace Street | Princes Highway | Urban | 60 | 12 | Kerb | Kerb | N/A | N/A | 12 | N/A |

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### 8.2 Existing Daily Traffic

Shoalhaven City Council provided GTA Consultants with peak to daily traffic conversion factors to apply to the May 2012 volumes counted at the study intersections to determine AADT and PSDT. These factors have been applied to the existing May 2012 turning movement volumes, and are shown in Figure 8.2. Shoalhaven City Council provided two conversion factors for both AADT and PSDT, and each were based on the Friday ( $8-9 a m$ ) or Saturday ( $12-1 \mathrm{pm}$ ) peak hours. When applied to the turning volumes, in some cases the factors yielded different daily volumes. In these cases, as requested by Council, the higher or 'worst case' value has been selected for assessment.

Figure 8.2: Existing AADTand Seasonal Traffic Volumes


The existing daily traffic based on the factors provided by Shoalhaven City Council has been compared to the design capacity based on Austroads requirements. The comparison is provided in Table 8.3

Rural Road Assessment

Table 8.3: Design AADTand Existing Daily Traffic

| Road | From | To | Rural Design |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Existing <br> AADT | Existing <br> PSDT |  |  |  |
| Coonamia Road | Culbura Road | Forest Road | 150 to 500 | 3,079 | 3,786 |
| Forest Road | Coonamia Road | Callala Beach Road | 150 to 500 | 3,529 | 4,184 |
| Forest Road | Callala Beach Road | Princes Highway | 150 to 500 | 3,178 | 3,767 |
| Greenwell Point Road | West Street | Pyree Lane | 150 to 500 | 5,526 | 6,551 |
| Greenwell Point Road | Pyree Lane | Jindy Andy Lane | 500 to 1000 | 7,888 | 9,352 |
| Greenwell Point Road | J indy Andy Lane | Apperleys Lane | 150 to 1000 | 5,947 | 7,051 |
| Greenwell Point Road | Apperleys Lane | Womigee Road | 1,000 to 3,000 | 6,144 | 7,285 |
| Jindy Andy Lane | Greenwell Point Road | Comerong Island Road | 150 to 500 | 2,250 | 2,667 |
| Pyree Lane | Coonamia Road | Jindy Andy Lane | 150 to 500 | 5,624 | 6,668 |

### 8.3 Antic ipated Daily Development Traffic

The anticipated average and peak seasonal daily traffic has been estimated based on discussions with Council and investigations by GTA Consultants. The additional development traffic and expected future traffic post development is summarised in Figure 8.3 and Figure 8.4 respectively.

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Figure 8.3: Anticipated Development Daily Traffic


Base Map Source: maps.google.com.au
The methods for calculating the traffic volumes generated by the development are explained in Section 7. According to GTA calculations, the worst case day for peak hour traffic generation onto the road network west of Culburra was found to be Saturday and traffic distribution was assigned accordingly.

Figure 8.4: Anticipated Post Development Daily Traffic


Base Map Source: maps.google.com.au
Figure 8.3 and Figure 8.4 indicate that the rural road network surrounding Culburra is expected to experience increases of between 368 and 2,179 vehicles on an average day, and between 500 and 2,584 vehicles at seasonal peaks. Culburra Road is expected to experience the greatest increases, originating directly from the development at 2,179 vpd (AADT) and 2,584 vpd (PSDT). Greenwell Point Road, Pyree Lane and Forest Road are also expected to see increased volumes.

However, it is also recognised that the performance of the road is more likely to be dictated by the peak hour performance of the intersections along its length. As shown in Section 7 the surveyed intersections Level of Service is unchanged with the addition of development traffic under $120^{\text {th }} \mathrm{HH}$ conditions.

### 8.4 S94 Requirements

Council currently has a Section 94 Plan which will raise funding towards parts of the roadway network which are considered deficient. This includes the following road related works:

- Pyree Lane Improvements
- Greenwell Point Road
$\$ 129.86$ per dwelling
\$34.44 per dwelling
- Culburra Road/ Prince Highway $\$ 213.69$ per dwelling.

The developer will pay these S94 contributions to help address Councils concerns about the deficiencies of certain sections of the road to the site.

### 8.5 Wa rrants for Overtaking Lanes

GTA Consultants has also investigated the warrants for overtaking lanes for the study roads identified in Figure 8.5. Guidance on the warrants and installation of overtaking lanes is provided in Austroads Guide to Road Design Part 3: Geometric Design Section 9.4 and Section 5.6.4.

Section 9.4 notes that "in deciding whether an overtaking lane is warranted, the evaluation needs to be carried out over a significant route length and not be isolated to the particular length over which the additional lane may be constructed."

Table 9.1 of Austroads Guide to Road Design Part 3: Geometric Design provides the traffic volume guidelines for providing overtaking lanes. The document also states that "Table 9.1 gives the currentyear design volumes (AADT) at which overtaking lanes would normally be justified. These guidelines apply for short low-cost overtaking lanes at spacings of 10 to 15 km or more along a road in a given direction. If spacing is less than this, a specific cost benefit analysis will need to justify the construction at the shorter spacing."

The existing speed limits and road section lengths are provided in Figure 8.5.

Figure 8.5: Existing Speed Limit and Road Section Lengths


Base Map Source: maps.google.com.au
Figure 8.5 shows that study roads contain a mixture of $50,60,80$ and $100 \mathrm{~km} / \mathrm{h}$ speed limits. Speed limits on all roads reduce as they approach the Princes Highway from the proposed development.

Typically, overtaking lanes are provided on high speed rural roads or where there are significant grades that could result in slow moving vehicles. The routes to and from the development from Princes Highway are mostly flat with minor grades with a single lane in each direction.

The longest stretch of existing rural road is 12.2 km and that is through a recently upgraded section of Forest Road. For the vast majority of this road, double barrier lines are in place and sight distance is not sufficient for any overtaking.

The longest stretch of $100 \mathrm{~km} / \mathrm{h}$ speed limit on Culburra Road is 5 km and a review of that stretch shows there are only a couple of short sections (approximately 500 m long) without barrier lines. None of those sections of Culburra Road is considered appropriate to provide an overtaking lane on.

Jindy Andy Lane and Coonemia Road are also $100 \mathrm{~km} / \mathrm{h}$ roads but their length is not considered long enough to warrant overtaking lanes.

Given the existing geometry and speed zones on the study roads, and the typical guidelines which suggest providing overtaking lanes every 10 to 15 km , overtaking lanes are not considered necessary for any of the study roads and are not proposed to be provided.

### 8.6 Summary

The following conclusions from this rural road analysis can be drawn:

- The existing shoulder widths, including sealed shoulders, of the rural roads assessed do not accord with current Austroads guidance.
- Notwithstanding this, the road and the intersections along it are operating satisfactorily (this reports shows that the intersections will continue to operate adequately following completion of the development).
- Council has a Section 94 Plan to address a number of the existing deficiencies. The development will contribute the requisite financial sums to address the identified issues.
- GTA believe there are no locations where overtaking lanes could be easily introduced and does not believe they are necessary.


## Conclusion

## 9. Conclusion

Based on the analysis and discussions presented within this report, the following conclusions are made:
i The West Culburra subdivision development involves approximately 110 ha on land west of the established area of Culburra.
ii The subdivision is comprised of six key stages, proposed to be developed in phases over a period of approximately 8 years. On completion, the West Culburra Development will include a mixture of medium density housing types, ranging from small lots 2 bedroom villas for the $55^{+}$aged group to multi-storey units.
iii The overall development includes a total of 685 dwellings consisting of:

- 500 dwelling houses
- $47 \times$ small-lot two bedroom, single storey villas for the $55^{+}$aged group
- $30 \times$ mixed-use, 3 bedroom town houses
- $10 \times 1$ bedroom units
- $83 \times 2$ bedroom apartments
- $15 \times 3$ bedroom units
iv The majority of development will be concentrated in Stages 3,4 and 5 which will include:
- 500 dwelling houses
- $30 \times$ mixed-use, 3 bedroom town houses (The Circus)
- $26 \times 2$ bedroom small lot dwellings for the $55^{+}$aged group
- $10 \times 1$ bedroom units
- $35 \times 2$ bedroom units
- $15 \times 3$ bedroom units.
v A new Collector Road within a 25 metre wide road reserve is proposed through Stages 3, 4 and 5 which will have two connections to Culburra Road. The eastern access will be the primary means of accessing these areas, as the western access will not be provided until a later stage of the development.
vi GTA Consultants undertook an assessment of the proposed eastern intersection of the Collector Road with Culburra Road to determine the most appropriate location, layout and dimensional requirements of the intersection. Based on this assessment, GTA Consultants produced an indicative concept design of the intersection consisting of a four arm single lane roundabout layout. The southern leg of the roundabout has been included in the concept design to show an alternative access point to the proposed Long Bow Point golf course.
vii It is anticipated that the existing 50km/hr speed limit in place on Culburra Road, approximately 350 metres east of Strathstone Street, will be extended west of the intersection to provide a $50 \mathrm{~km} / \mathrm{hr}$ speed limit on the western approach in line with the NSW Speed Zoning Guidelines (RMS, 2011).
viii The cycle network proposed as part of the development includes two key routes:
- East-west route along the foreshore area providing access to Culburra shops
- East-west route along the proposed Collector Road and the northern side of Culburra Road providing access to Culburra shops.

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## Conclusion

The foreshore route is considered to be an excellent opportunity for a recreational cycle route and to promote cycle tourism in the region. The new Collector Road is considered to be the optimum alignment for a cycleway through Stages 3, 4 and 5 to connect with Culburra shops to the east.
ix The Collector Road through Stages 3,4 and 5 will serve as the key route for the CulburraNowra public bus service and for school bus services and as such all accesses to the Collector Road are required to accommodate bus turning movements.
$x$ It is recommended that the existing bus stops within Culburra and Orient Point be upgraded as part of the development to improve amenity and promote the use of public transport for existing residents outside the development areas.
xi It is recommended that all new bus stops provide shelter, seating, lighting, timetable information as a minimum.
xii A minimum of a 1.2 metre wide footpath is required on local and collector streets within a subdivision in line with DCP 100.
xiii With consideration of likely vehicle speeds and volumes along the Collector Road, it is recommended that a separated cycle facility be provided along this alignment in line with the NSW Bicycle Guidelines.
xiv For shared pedestrian and cycle paths associated within the development, it is recommended to provide a minimum 3 metre width given their potential as recreational routes.
xv Given the traffic volumes along Culburra Road, it is recommended to provide a separated facility along the northern side of Culburra Road to provide access between Stages 3,4 and 5, Culburra shops and Stage 1.
xvi Further consideration is required for the connection of footways and cycleways constructed as part of the development with the existing cycling network to provide a consistent standard of facility.
xvii It is anticipated that refuse collection for the new development areas will be undertaken by a standard 12.5 metre long Council garbage vehicle.
xviii Based on empirical traffic generation rates calculated from analysis of historical traffic volume data and residential occupancy data, the proposed development is expected to generate 151, 144 and 158 vehicle trips per occupied dwelling during the respective Friday AM, Friday PM and Saturday peak hours on the regional road network (west of Culburra).
xix In assessing intersection performance on the road network surrounding the site, growth factors were applied to the recorded traffic volumes (May 2012) to represent the equivalent $120^{\text {th }}$ Highest Annual Hour (HH). This was done to reflect the significant seasonal increases in traffic volumes in the region.
xx Under equivalent $120^{\text {th }} \mathrm{HH}$ traffic volumes the performance of intersections surrounding the site was not significantly changed with the addition of development traffic
xxi Under equivalent $120^{\text {th }} \mathrm{HH}$ traffic volumes the Princes Highway intersections at Kalandar Street and Moss Street currently experience significant delays, particularly during the Friday AM and Friday PM peak periods. The addition of development traffic at these intersections (which would compromise only $2 \%$ of the flow at these intersections) would not result in any discernible change in intersection performance.
xxii There would be traffic increases in Culburra but the additional traffic generated by the development would not cause any existing roads/intersections to experience any operational problems.

## Conclusion

xxiii The existing shoulder widths, including sealed shoulders, of the rural roads assessed do not accord with current Austroads guidance.
xxiv Notwithstanding this, the road and the intersections along it are operating satisfactorily (this reports shows that the intersections will continue to operate adequately following completion of the development).
xxv Council has a Section 94 Plan to address a number of the existing deficiencies. The development will contribute the requisite financial sums to address the identified issues.
xxvi GTA believe there are no locations where overtaking lanes could be easily introduced and does not believe they are necessary.

In conclusion, provided that the developer provides the roundabout access into the site and pays the requisite S94 contributions to upgrade deficiencies in the road network, the traffic generated by the development can be successfully accommodated.

## AppendixA

Survey Results







| Job No. | : N790 |
| :--- | :--- |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $:$ 1. Culburra Rd / Coonamia Rd |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | : Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |








| Approach | Gulburra Rd |  |  |  |  |  |  |  |  | Mayfield Rd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  |  | Direction 12 <br> （Right Turn） |  | Direction 12U <br> （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{\circ}} \\ & \hline \end{aligned}$ | $$ |  | $\begin{aligned} & \text { ज口 } \\ & \stackrel{1}{0} \end{aligned}$ |  |  |  | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \text { ভ } \\ & \stackrel{\rightharpoonup}{\circ} \\ & \hline \end{aligned}$ | 皆 |  | $\begin{array}{r} \overline{\boxed{\circ}} \\ \stackrel{\rightharpoonup}{\circ} \\ \hline \end{array}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | ¢ |
| 7：00 to 7：15 | 11 | 9 | 20 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 2 | 3 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7：15 to $7: 30$ | 16 | 10 | 26 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 7：30 to 7：45 | 12 | 1 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 |
| 7：45 to 8：00 | 18 | 1 | 19 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8：00 to 8：15 | 18 | 4 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8：15 to 8：30 | 28 | 1 | 29 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 |
| 8：30 to $8: 45$ | 34 | 1 | 35 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8：45 to 9：00 | 20 | 1 | 21 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 1 | 0 | 0 | 0 |
| AM Totals | 157 | 28 | 185 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 2 | 4 | 3 | 0 | 3 | 0 | 0 | 0 |
| 16：00 to 16：15 | 62 | 3 | 65 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 |
| 16：15 to 16：30 | 71 | 1 | 72 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 16：30 to 16：45 | 76 | 1 | 77 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 16：45 to 17：00 | 63 | 0 | 63 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 17：00 to 17：15 | 84 | 0 | 84 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 |


| 17:15 to 17:30 | 89 | 0 | 89 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 17:30 to 17:45 | 83 | 1 | 84 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 |
| 17:45 to 18:00 | 76 | 1 | 77 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| PM Totals | 604 | 7 | 611 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 1 | 3 | 0 | 0 | 0 |


| Job No. | : N790 |
| :--- | :--- |
| Client | : Realty Realizations |
| Suburb | : Nowra |
| Location | : 2. Gulburra Rd / Mayfield Rd |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | : Fine |
| Description | : Classified Intersection Count |
|  | : Hourly Summary |






SKYHIGH - THE TRAFFIC SURVEY COMPANY





| Job No. | : N790 |
| :--- | :--- |
| Client | : Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $: 3$. Greenwell Point Rd / Pyree Ln |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | : Fine |
| Description | : Classified Intersection Count |
|  | : Hourly Summary |



SKYHIGH - THE TRAFFIC SURVEY COMPANY






Description ：Classified Intersection Count
： 15 mins Data

| Approach | Jindy Andy Ln |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 <br> （Through） |  |  | Direction 12 U （U Turn） |  |  |
| Time Period | $$ | ふ <br> $\substack{\text { en } \\ \text { ¢ }}$ | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{\circ}{1} \\ & \hline \end{aligned}$ | 菏 |  | $\begin{aligned} & \overline{\oplus 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline 1 \end{aligned}$ |  | $\begin{aligned} & \overline{\oplus 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  |  | 董 | 入 <br> $\substack{0 \\ \text { ¢ }}$ | \} |
| 7：00 to 7：15 | 3 | 0 | 3 | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 1 | 2 | 15 | 17 | 32 | 0 | 0 | 0 |
| 7：15 to 7：30 | 5 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 2 | 20 | 14 | 34 | 0 | 0 | 0 |
| 7：30 to 7：45 | 1 | 1 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 16 | 3 | 19 | 0 | 0 | 0 |
| 7：45 to 8：00 | 3 | 0 | 3 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 22 | 5 | 27 | 0 | 0 | 0 |
| 8：00 to 8：15 | 8 | 0 | 8 | 3 | 1 | 4 | 0 | 0 | 0 | 2 | 0 | 2 | 19 | 5 | 24 | 0 | 0 | 0 |
| 8：15 to 8：30 | 5 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 32 | 0 | 32 | 0 | 0 | 0 |
| 8：30 to 8：45 | 10 | 1 | 11 | 1 | 0 | 1 | 0 | 0 | 0 | 2 | 2 | 4 | 33 | 2 | 35 | 0 | 0 | 0 |
| 8：45 to 9：00 | 9 | 0 | 9 | 3 | 2 | 5 | 1 | 0 | 1 | 0 | 1 | 1 | 21 | 3 | 24 | 0 | 0 | 0 |
| AM Totals | 44 | 2 | 46 | 8 | 4 | 12 | 1 | 0 | 1 | 7 | 6 | 13 | 178 | 49 | 227 | 0 | 0 | 0 |


| 16:00 to 16:15 | 27 | 1 | 28 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 65 | 2 | 67 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16:15 to 16:30 | 24 | 1 | 25 | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 1 | 2 | 67 | 0 | 67 | 0 | 0 | 0 |
| 16:30 to 16:45 | 28 | 0 | 28 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 73 | 3 | 76 | 0 | 0 | 0 |
| 16:45 to 17:00 | 27 | 0 | 27 | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 59 | 0 | 59 | 0 | 0 | 0 |
| 17:00 to 17:15 | 32 | 0 | 32 | 3 | 0 | 3 | 0 | 0 | 0 | 1 | 0 | 1 | 77 | 0 | 77 | 0 | 0 | 0 |
| 17:15 to 17:30 | 37 | 0 | 37 | 2 | 0 | 2 | 0 | 0 | 0 | 1 | 0 | 1 | 79 | 0 | 79 | 0 | 0 | 0 |
| 17:30 to 17:45 | 26 | 0 | 26 | 1 | 0 | 1 | 0 | 0 | 0 | 3 | 0 | 3 | 77 | 0 | 77 | 0 | 0 | 0 |
| 17:45 to 18:00 | 23 | 1 | 24 | 1 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 1 | 64 | 2 | 66 | 0 | 0 | 0 |
| PM Totals | 224 | 3 | 227 | 9 | 0 | 9 | 0 | 0 | 0 | 14 | 1 | 15 | 561 | 7 | 568 | 0 | 0 | 0 |









| Job No. | : N790 |
| :--- | :--- |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $: 5$. Greenwell Point Rd / Mayfield Rd |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |



SKYHIGH - THE TRAFFIC SURVEY COMPANY


| Approach |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction |  |  | Direction 11 (Through) |  |  | Direction 12 (Right Turn) |  |  | Direction 12U (U Turn) |  |  |
| Time Period |  |  |  | ¢ | $\begin{aligned} & \overline{5} \\ & \stackrel{\circ}{1} \end{aligned}$ | 苛 | त § ¢ ¢ | ¢ |  | $\xrightarrow{\substack{\text { ¢ } \\ \text { ¢ }}}$ | 픈 |
| 7:00 to 8:00 |  |  | 75 | 41 | 116 | 3 | 0 | 3 | 0 | 0 | 0 |
| 7:15 to $8: 15$ |  |  | 79 | 28 | 107 | 3 | 0 | 3 | 0 | 0 | 0 |
| 7:30 to 8:30 |  |  | 92 | 13 | 105 | 2 | 0 | 2 | 0 | 0 | 0 |
| 7:45 to 8:45 |  |  | 110 | 12 | 122 | 4 | 0 | 4 | 0 | 0 | 0 |
| 8:00 to 9:00 |  |  | 109 | 11 | 120 | 5 | 0 | 5 | 0 | 0 | 0 |
| AM Totals |  |  | 184 | 52 | 236 | 8 | 0 | 8 | 0 | 0 | 0 |
| 16:00 to 17:00 |  |  | 274 | 4 | 278 | 10 | 0 | 10 | 0 | 0 | 0 |
| 16:15 to 17:15 |  |  | 285 | 2 | 287 | 12 | 0 | 12 | 0 | 0 | 0 |
| 16:30 to 17:30 |  |  | 292 | 2 | 294 | 9 | 0 | 9 | 0 | 0 | 0 |
| 16:45 to 17:45 |  |  | 302 | 0 | 302 | 10 | 0 | 10 | 0 | 0 | 0 |
| 17:00 to 18:00 |  |  | 300 | 2 | 302 | 10 | 0 | 10 | 0 | 0 | 0 |
| PM Totals |  |  | 574 | 6 | 580 | 20 | 0 | 20 | 0 | 0 | 0 |



| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 岩 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $$ | $\begin{aligned} & 3 \\ & \begin{array}{l} 3 \\ \frac{0}{x} \end{array} \end{aligned}$ | $\begin{aligned} & \overline{\text { In }} \\ & \text { ○ } \end{aligned}$ |  |  | $\begin{aligned} & \overline{\mathrm{W}} \\ & \stackrel{-}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 芌 } \\ & \stackrel{O}{J} \end{aligned}$ | $\begin{aligned} & 3 \\ & \begin{array}{c} 3 \\ \text { I } \end{array} \end{aligned}$ | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{\rightharpoonup}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 들 } \\ & \\ & \hline \end{aligned}$ | ¢ | ¢ |
| 7：00 to 7：15 | 6 | 0 | 6 | 4 | 2 | 6 | 0 | 2 | 2 | 0 | 0 | 0 | 5 | 0 | 5 | 47 | 2 | 49 | 7 | 0 | 7 | 0 | 0 | 0 |
| 7：15 to $7: 30$ | 7 | 1 | 8 | 4 | 0 | 4 | 1 | 1 | 2 | 0 | 0 | 0 | 5 | 0 | 5 | 43 | 0 | 43 | 8 | 1 | 9 | 0 | 0 | 0 |
| 7：30 to 7：45 | 9 | 1 | 10 | 2 | 0 | 2 | 3 | 0 | 3 | 0 | 0 | 0 | 3 | 0 | 3 | 81 | 6 | 87 | 7 | 0 | 7 | 0 | 0 | 0 |
| 7：45 to 8：00 | 21 | 1 | 22 | 9 | 0 | 9 | 2 | 0 | 2 | 0 | 0 | 0 | 4 | 3 | 7 | 73 | 4 | 77 | 3 | 1 | 4 | 0 | 0 | 0 |
| 8：00 to 8：15 | 18 | 2 | 20 | 19 | 3 | 22 | 4 | 0 | 4 | 0 | 0 | 0 | 3 | 0 | 3 | 100 | 1 | 101 | 4 | 2 | 6 | 0 | 0 | 0 |
| 8：15 to 8：30 | 15 | 1 | 16 | 30 | 0 | 30 | 8 | 0 | 8 | 0 | 0 | 0 | 1 | 0 | 1 | 59 | 2 | 61 | 5 | 0 | 5 | 0 | 0 | 0 |
| $8: 30$ to $8: 45$ | 8 | 0 | 8 | 35 | 0 | 35 | 4 | 0 | 4 | 0 | 0 | 0 | 2 | 0 | 2 | 70 | 3 | 73 | 13 | 0 | 13 | 0 | 0 | 0 |
| 8：45 to 9：00 | 15 | 0 | 15 | 13 | 0 | 13 | 2 | 0 | 2 | 0 | 0 | 0 | 4 | 0 | 4 | 65 | 3 | 68 | 3 | 0 | 3 | 0 | 0 | 0 |
| AM Totals | 99 | 6 | 105 | 116 | 5 | 121 | 24 | 3 | 27 | 0 | 0 | 0 | 27 | 3 | 30 | 538 | 21 | 559 | 50 | 4 | 54 | 0 | 0 | 0 |
| 16：00 to 16：15 | 10 | 0 | 10 | 4 | 0 | 4 | 2 | 0 | 2 | 0 | 0 | 0 | 5 | 0 | 5 | 38 | 3 | 41 | 2 | 0 | 2 | 0 | 0 | 0 |
| 16：15 to $16: 30$ | 9 | 0 | 9 | 12 | 0 | 12 | 6 | 0 | 6 | 0 | 0 | 0 | 3 | 0 | 3 | 21 | 2 | 23 | 3 | 0 | 3 | 0 | 0 | 0 |
| 16：30 to 16：45 | 7 | 0 | 7 | 7 | 0 | 7 | 7 | 1 | 8 | 0 | 0 | 0 | 4 | 0 | 4 | 22 | 1 | 23 | 2 | 0 | 2 | 0 | 0 | 0 |
| 16：45 to 17：00 | 13 | 0 | 13 | 9 | 0 | 9 | 3 | 0 | 3 | 0 | 0 | 0 | 1 | 1 | 2 | 23 | 0 | 23 | 2 | 0 | 2 | 0 | 0 | 0 |
| 17：00 to 17：15 | 9 | 1 | 10 | 7 | 0 | 7 | 10 | 0 | 10 | 0 | 0 | 0 | 8 | 0 | 8 | 26 | 0 | 26 | 4 | 0 | 4 | 0 | 0 | 0 |
| 17：15 to 17：30 | 5 | 0 | 5 | 4 | 0 | 4 | 6 | 0 | 6 | 0 | 0 | 0 | 3 | 0 | 3 | 32 | 0 | 32 | 3 | 0 | 3 | 0 | 0 | 0 |
| 17：30 to 17：45 | 9 | 0 | 9 | 7 | 0 | 7 | 6 | 0 | 6 | 0 | 0 | 0 | 3 | 0 | 3 | 32 | 0 | 32 | 1 | 1 | 2 | 0 | 0 | 0 |
| 17：45 to 18：00 | 11 | 0 | 11 | 6 | 0 | 6 | 5 | 0 | 5 | 0 | 0 | 0 | 2 | 0 | 2 | 20 | 1 | 21 | 6 | 0 | 6 | 0 | 0 | 0 |
| PM Totals | 73 | 1 | 74 | 56 | 0 | 56 | 45 | 1 | 46 | 0 | 0 | 0 | 29 | 1 | 30 | 214 | 7 | 221 | 23 | 1 | 24 | 0 | 0 | 0 |

： 15 mins Data
Millbank Rd

| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 <br> （Right Turn） |  |  | Direction 12 U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \hline \underline{I} \end{aligned}$ |  | $\begin{aligned} & \bar{\Pi} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{aligned} & \overline{\# ँ} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \underline{I} \end{aligned}$ |  | $\begin{aligned} & \overline{50} \\ & \stackrel{0}{1} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \text { IO } \end{aligned}$ |  | $\begin{aligned} & \overline{50} \\ & \text { 으 } \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\oplus 5} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ | $\frac{\stackrel{\rightharpoonup}{5}}{\underline{3}}$ |  | $\begin{aligned} & \bar{\Pi} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \underline{I} \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{IN}} \\ & \stackrel{0}{1} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | 듳 |
| 7：00 to 7：15 | 2 | 1 | 3 | 1 | 0 | 1 | 1 | 1 | 2 | 0 | 0 | 0 | 3 | 0 | 3 | 20 | 8 | 28 | 2 | 0 | 2 | 0 | 0 | 0 |
| 7：15 to 7：30 | 0 | 0 | 0 | 0 | 1 | 1 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 16 | 6 | 22 | 1 | 1 | 2 | 0 | 0 | 0 |
| 7：30 to 7：45 | 3 | 0 | 3 | 0 | 0 | 0 | 4 | 0 | 4 | 0 | 0 | 0 | 2 | 0 | 2 | 16 | 4 | 20 | 6 | 1 | 7 | 0 | 0 | 0 |
| 7：45 to 8：00 | 3 | 0 | 3 | 2 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 20 | 4 | 24 | 5 | 0 | 5 | 0 | 0 | 0 |
| 8：00 to 8：15 | 4 | 0 | 4 | 3 | 1 | 4 | 2 | 0 | 2 | 0 | 0 | 0 | 11 | 1 | 12 | 23 | 2 | 25 | 3 | 0 | 3 | 0 | 0 | 0 |
| 8：15 to 8：30 | 1 | 0 | 1 | 8 | 1 | 9 | 2 | 0 | 2 | 0 | 0 | 0 | 14 | 0 | 14 | 32 | 4 | 36 | 5 | 0 | 5 | 0 | 0 | 0 |
| 8：30 to 8：45 | 3 | 0 | 3 | 6 | 1 | 7 | 3 | 0 | 3 | 0 | 0 | 0 | 9 | 0 | 9 | 23 | 1 | 24 | 5 | 1 | 6 | 0 | 0 | 0 |
| 8：45 to 9：00 | 2 | 2 | 4 | 2 | 0 | 2 | 3 | 0 | 3 | 0 | 0 | 0 | 3 | 2 | 5 | 32 | 4 | 36 | 36 | 2 | 38 | 0 | 0 | 0 |
| AM Totals | 18 | 3 | 21 | 22 | 4 | 26 | 16 | 1 | 17 | 0 | 0 | 0 | 44 | 3 | 47 | 182 | 33 | 215 | 63 | 5 | 68 | 0 | 0 | 0 |


| 16：00 to 16：15 | 2 | 0 | 2 | 9 | 1 | 10 | 4 | 0 | 4 | 0 | 0 | 0 | 4 | 0 | 4 | 72 | 0 | 72 | 21 | 0 | 21 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16：15 to 16：30 | 5 | 0 | 5 | 9 | 0 | 9 | 3 | 0 | 3 | 0 | 0 | 0 | 3 | 0 | 3 | 69 | 1 | 70 | 17 | 1 | 18 | 0 | 0 | 0 |
| 16：30 to 16：45 | 2 | 0 | 2 | 8 | 0 | 8 | 3 | 0 | 3 | 0 | 0 | 0 | 3 | 0 | 3 | 67 | 1 | 68 | 19 | 0 | 19 | 0 | 0 | 0 |
| 16：45 to 17：00 | 6 | 0 | 6 | 9 | 0 | 9 | 2 | 0 | 2 | 0 | 0 | 0 | 1 | 0 | 1 | 66 | 0 | 66 | 19 | 0 | 19 | 0 | 0 | 0 |
| 17：00 to 17：15 | 4 | 0 | 4 | 8 | 0 | 8 | 2 | 0 | 2 | 0 | 0 | 0 | 4 | 0 | 4 | 64 | 1 | 65 | 21 | 0 | 21 | 0 | 0 | 0 |
| 17：15 to 17：30 | 2 | 0 | 2 | 8 | 0 | 8 | 4 | 0 | 4 | 0 | 0 | 0 | 2 | 0 | 2 | 83 | 0 | 83 | 22 | 1 | 23 | 0 | 0 | 0 |
| 17：30 to 17：45 | 2 | 0 | 2 | 8 | 0 | 8 | 4 | 0 | 4 | 0 | 0 | 0 | 1 | 0 | 1 | 71 | 5 | 76 | 15 | 0 | 15 | 0 | 0 | 0 |
| 17：45 to 18：00 | 4 | 0 | 4 | 6 | 0 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 57 | 0 | 57 | 17 | 0 | 17 | 0 | 0 | 0 |
| PM Totals | 27 | 0 | 27 | 65 | 1 | 66 | 22 | 0 | 22 | 0 | 0 | 0 | 21 | 0 | 21 | 549 | 8 | 557 | 151 | 2 | 153 | 0 | 0 | 0 |


| Job No． | $:$ N790 |
| :--- | :--- |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $: 6$. Greenwell Point Rd／Millbank Rd |
|  |  |
| Day／Date | ：Fri，4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |



SKYHICH－THE TRAFFIC SURVEY COMPANY

| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{aligned} & \bar{\Xi} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\frac{\stackrel{7}{0}}{\underline{I}}$ |  | $\begin{aligned} & \overline{๊ ๊} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{O}} \\ & \stackrel{y}{\mathrm{O}} \end{aligned}$ | $\begin{aligned} & \text { ふ } \\ & \text { オ } \\ & \text { d } \end{aligned}$ | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \underline{3} \end{aligned}$ |  | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\text { Z }}{\substack{x}} \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline 1 \end{aligned}$ | $\begin{aligned} & \underset{\text { Z }}{\substack{\text { I }}} \end{aligned}$ | $\begin{gathered} \overline{\mathrm{I}} \\ \stackrel{-}{\circ} \end{gathered}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\text { Z }}{\substack{\text { I } \\ \hline}} \end{aligned}$ | Пّ |
| 7：00 to 8：00 | 43 | 3 | 46 | 19 | 2 | 21 | 6 | 3 | 9 | 0 | 0 | 0 | 17 | 3 | 20 | 244 | 12 | 256 | 25 | 2 | 27 | 0 | 0 | 0 |
| 7：15 to 8：15 | 55 | 5 | 60 | 34 | 3 | 37 | 10 | 1 | 11 | 0 | 0 | 0 | 15 | 3 | 18 | 297 | 11 | 308 | 22 | 4 | 26 | 0 | 0 | 0 |
| 7：30 to 8：30 | 63 | 5 | 68 | 60 | 3 | 63 | 17 | 0 | 17 | 0 | 0 | 0 | 11 | 3 | 14 | 313 | 13 | 326 | 19 | 3 | 22 | 0 | 0 | 0 |
| 7：45 to 8：45 | 62 | 4 | 66 | 93 | 3 | 96 | 18 | 0 | 18 | 0 | 0 | 0 | 10 | 3 | 13 | 302 | 10 | 312 | 25 | 3 | 28 | 0 | 0 | 0 |
| 8：00 to 9：00 | 56 | 3 | 59 | 97 | 3 | 100 | 18 | 0 | 18 | 0 | 0 | 0 | 10 | 0 | 10 | 294 | 9 | 303 | 25 | 2 | 27 | 0 | 0 | 0 |
| AM Totals | 99 | 6 | 105 | 116 | 5 | 121 | 24 | 3 | 27 | 0 | 0 | 0 | 27 | 3 | 30 | 538 | 21 | 559 | 50 | 4 | 54 | 0 | 0 | 0 |
| 16：00 to 17：00 | 39 | 0 | 39 | 32 | 0 | 32 | 18 | 1 | 19 | 0 | 0 | 0 | 13 | 1 | 14 | 104 | 6 | 110 | 9 | 0 | 9 | 0 | 0 | 0 |
| 16：15 to 17：15 | 38 | 1 | 39 | 35 | 0 | 35 | 26 | 1 | 27 | 0 | 0 | 0 | 16 | 1 | 17 | 92 | 3 | 95 | 11 | 0 | 11 | 0 | 0 | 0 |
| 16：30 to 17：30 | 34 | 1 | 35 | 27 | 0 | 27 | 26 | 1 | 27 | 0 | 0 | 0 | 16 | 1 | 17 | 103 | 1 | 104 | 11 | 0 | 11 | 0 | 0 | 0 |
| 16：45 to 17：45 | 36 | 1 | 37 | 27 | 0 | 27 | 25 | 0 | 25 | 0 | 0 | 0 | 15 | 1 | 16 | 113 | 0 | 113 | 10 | 1 | 11 | 0 | 0 | 0 |
| 17：00 to 18：00 | 34 | 1 | 35 | 24 | 0 | 24 | 27 | 0 | 27 | 0 | 0 | 0 | 16 | 0 | 16 | 110 | 1 | 111 | 14 | 1 | 15 | 0 | 0 | 0 |
| PM Totals | 73 | 1 | 74 | 56 | 0 | 56 | 45 | 1 | 46 | 0 | 0 | 0 | 29 | 1 | 30 | 214 | 7 | 221 | 23 | 1 | 24 | 0 | 0 | 0 |


| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline 1 \end{aligned}$ | $\begin{aligned} & 3 \\ & \begin{array}{l} 7 \\ \text { I } \end{array} \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\rightharpoonup}{1} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \underline{J} \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{\circ}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\frac{\stackrel{\rightharpoonup}{0}}{\underline{O}}$ |  | $\begin{gathered} \overline{\boxed{\circ}} \\ \stackrel{\circ}{\circ} \end{gathered}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \mathrm{I} \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \bar{Ð} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{V}} \\ & \stackrel{\rightharpoonup}{J} \end{aligned}$ | 入 <br> $\substack{\text { ¢ } \\ \text { ¢ }}$ <br>  | \} |
| 7：00 to 8：00 | 8 | 1 | 9 | 3 | 1 | 4 | 6 | 1 | 7 | 0 | 0 | 0 | 7 | 0 | 7 | 72 | 22 | 94 | 14 | 2 | 16 | 0 | 0 | 0 |
| 7：15 to 8：15 | 10 | 0 | 10 | 5 | 2 | 7 | 7 | 0 | 7 | 0 | 0 | 0 | 15 | 1 | 16 | 75 | 16 | 91 | 15 | 2 | 17 | 0 | 0 | 0 |
| 7：30 to 8：30 | 11 | 0 | 11 | 13 | 2 | 15 | 8 | 0 | 8 | 0 | 0 | 0 | 29 | 1 | 30 | 91 | 14 | 105 | 19 | 1 | 20 | 0 | 0 | 0 |
| 7：45 to 8：45 | 11 | 0 | 11 | 19 | 3 | 22 | 7 | 0 | 7 | 0 | 0 | 0 | 36 | 1 | 37 | 98 | 11 | 109 | 18 | 1 | 19 | 0 | 0 | 0 |
| 8：00 to 9：00 | 10 | 2 | 12 | 19 | 3 | 22 | 10 | 0 | 10 | 0 | 0 | 0 | 37 | 3 | 40 | 110 | 11 | 121 | 49 | 3 | 52 | 0 | 0 | 0 |
| AM Totals | 18 | 3 | 21 | 22 | 4 | 26 | 16 | 1 | 17 | 0 | 0 | 0 | 44 | 3 | 47 | 182 | 33 | 215 | 63 | 5 | 68 | 0 | 0 | 0 |
| 16：00 to 17：00 | 15 | 0 | 15 | 35 | 1 | 36 | 12 | 0 | 12 | 0 | 0 | 0 | 11 | 0 | 11 | 274 | 2 | 276 | 76 | 1 | 77 | 0 | 0 | 0 |
| 16：15 to 17：15 | 17 | 0 | 17 | 34 | 0 | 34 | 10 | 0 | 10 | 0 | 0 | 0 | 11 | 0 | 11 | 266 | 3 | 269 | 76 | 1 | 77 | 0 | 0 | 0 |
| 16：30 to 17：30 | 14 | 0 | 14 | 33 | 0 | 33 | 11 | 0 | 11 | 0 | 0 | 0 | 10 | 0 | 10 | 280 | 2 | 282 | 81 | 1 | 82 | 0 | 0 | 0 |
| 16：45 to 17：45 | 14 | 0 | 14 | 33 | 0 | 33 | 12 | 0 | 12 | 0 | 0 | 0 | 8 | 0 | 8 | 284 | 6 | 290 | 77 | 1 | 78 | 0 | 0 | 0 |
| 17：00 to 18：00 | 12 | 0 | 12 | 30 | 0 | 30 | 10 | 0 | 10 | 0 | 0 | 0 | 10 | 0 | 10 | 275 | 6 | 281 | 75 | 1 | 76 | 0 | 0 | 0 |
| PM Totals | 27 | 0 | 27 | 65 | 1 | 66 | 22 | 0 | 22 | 0 | 0 | 0 | 21 | 0 | 21 | 549 | 8 | 557 | 151 | 2 | 153 | 0 | 0 | 0 |



| Approach <br> Direction | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U <br> （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 <br> （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{gathered} \text { 历̈ } \\ \stackrel{\circ}{\circ} \end{gathered}$ | $$ |  | $\begin{aligned} & \bar{\leftrightarrows} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $$ | $\begin{aligned} & \underset{0}{\lambda} \\ & \text { ָin } \end{aligned}$ | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\boxed{\circ}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $$ |  | $\begin{aligned} & \text { 历̈ } \\ & \stackrel{\rightharpoonup}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { ㄷㅡㅡㄹ } \\ & \hline 0 \end{aligned}$ | $\begin{aligned} & \underset{\substack{3 \\ \text { I } \\ \text { I }}}{ } \end{aligned}$ | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $$ | $\begin{aligned} & \underset{\substack{3 \\ \mathbf{x} \\ \text { In }}}{ } \end{aligned}$ | $\begin{aligned} & \stackrel{\pi}{\circ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & 3 \\ & \begin{array}{c} 3 \\ \text { I } \end{array} \end{aligned}$ | 픙 |
| 7：00 to 7：15 | 0 | 0 | 0 | 67 | 23 | 90 | 5 | 2 | 7 | 0 | 0 | 0 | 9 | 1 | 10 | 35 | 0 | 35 | 47 | 2 | 49 | 0 | 0 | 0 |
| 7：15 to 7：30 | 0 | 0 | 0 | 120 | 4 | 124 | 9 | 0 | 9 | 0 | 0 | 0 | 6 | 1 | 7 | 36 | 2 | 38 | 78 | 0 | 78 | 0 | 0 | 0 |
| 7：30 to 7：45 | 1 | 0 | 1 | 136 | 20 | 156 | 6 | 0 | 6 | 0 | 0 | 0 | 8 | 0 | 8 | 29 | 0 | 29 | 83 | 3 | 86 | 0 | 0 | 0 |
| 7：45 to 8：00 | 0 | 0 | 0 | 164 | 7 | 171 | 6 | 1 | 7 | 0 | 0 | 0 | 13 | 1 | 14 | 62 | 0 | 62 | 114 | 0 | 114 | 0 | 0 | 0 |
| 8：00 to 8：15 | 1 | 0 | 1 | 184 | 15 | 199 | 9 | 0 | 9 | 0 | 0 | 0 | 5 | 0 | 5 | 47 | 1 | 48 | 130 | 5 | 135 | 0 | 0 | 0 |
| 8：15 to 8：30 | 1 | 0 | 1 | 185 | 8 | 193 | 4 | 0 | 4 | 0 | 0 | 0 | 7 | 0 | 7 | 75 | 2 | 77 | 172 | 6 | 178 | 0 | 0 | 0 |
| 8：30 to 8：45 | 1 | 0 | 1 | 200 | 10 | 210 | 11 | 0 | 11 | 0 | 0 | 0 | 8 | 2 | 10 | 64 | 0 | 64 | 182 | 5 | 187 | 0 | 0 | 0 |
| 8：45 to 9：00 | 0 | 0 | 0 | 195 | 12 | 207 | 8 | 0 | 8 | 0 | 0 | 0 | 9 | 1 | 10 | 66 | 5 | 71 | 160 | 3 | 163 | 0 | 0 | 0 |
| AM Totals | 4 | 0 | 4 | 1251 | 99 | 1350 | 58 | 3 | 61 | 0 | 0 | 0 | 65 | 6 | 71 | 414 | 10 | 424 | 966 | 24 | 990 | 0 | 0 | 0 |
| 16：00 to 16：15 | 5 | 0 | 5 | 163 | 6 | 169 | 18 | 0 | 18 | 0 | 0 | 0 | 18 | 1 | 19 | 44 | 0 | 44 | 101 | 3 | 104 | 0 | 0 | 0 |
| 16：15 to $16: 30$ | 0 | 0 | 0 | 184 | 4 | 188 | 22 | 1 | 23 | 0 | 0 | 0 | 15 | 0 | 15 | 42 | 1 | 43 | 88 | 1 | 89 | 0 | 0 | 0 |
| 16：30 to $16: 45$ | 1 | 0 | 1 | 161 | 2 | 163 | 22 | 0 | 22 | 0 | 0 | 0 | 18 | 0 | 18 | 35 | 2 | 37 | 98 | 3 | 101 | 0 | 0 | 0 |
| 16：45 to 17：00 | 2 | 0 | 2 | 171 | 6 | 177 | 11 | 0 | 11 | 0 | 0 | 0 | 15 | 0 | 15 | 40 | 1 | 41 | 88 | 0 | 88 | 0 | 0 | 0 |
| 17：00 to 17：15 | 1 | 0 | 1 | 156 | 4 | 160 | 16 | 2 | 18 | 0 | 0 | 0 | 20 | 1 | 21 | 44 | 1 | 45 | 99 | 0 | 99 | 0 | 0 | 0 |
| 17：15 to 17：30 | 0 | 0 | 0 | 130 | 2 | 132 | 14 | 0 | 14 | 0 | 0 | 0 | 19 | 0 | 19 | 47 | 1 | 48 | 110 | 1 | 111 | 0 | 0 | 0 |
| 17：30 to 17：45 | 1 | 0 | 1 | 162 | 5 | 167 | 15 | 0 | 15 | 0 | 0 | 0 | 13 | 0 | 13 | 38 | 0 | 38 | 91 | 1 | 92 | 0 | 0 | 0 |
| 17：45 to 18：00 | 3 | 0 | 3 | 126 | 4 | 130 | 10 | 0 | 10 | 0 | 0 | 0 | 15 | 0 | 15 | 51 | 0 | 51 | 100 | 0 | 100 | 0 | 0 | 0 |
| PM Totals | 13 | 0 | 13 | 1253 | 33 | 1286 | 128 | 3 | 131 | 0 | 0 | 0 | 133 | 2 | 135 | 341 | 6 | 347 | 775 | 9 | 784 | 0 | 0 | 0 |



| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 <br> （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 <br> （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 鹍 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \sum_{J}^{\gtrless} \\ & \text { din } \end{aligned}$ | $\begin{aligned} & \text { 픔 } \\ & \hline \end{aligned}$ |  |  | $\begin{aligned} & \bar{\cong} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline .0 \end{aligned}$ | $\begin{aligned} & \text { 入 } \\ & \text { 조 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{50} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ | $\begin{aligned} & \text { 入 } \\ & \text { 조 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{5} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ | $\begin{aligned} & \sum_{J}^{7} \\ & \frac{0}{1} \end{aligned}$ | $\begin{aligned} & \overline{\ddagger 5} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \underline{I} \end{aligned}$ | $\begin{aligned} & \text { 入 } \\ & \text { 조 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline .0 \end{aligned}$ | ス <br> $\substack{\text { ¢ } \\ \text { ¢ }}$ | $\begin{aligned} & \overline{50} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\frac{\text { 苛 }}{3}$ | 橘 | ¢ |
| 7：00 to 7：15 | 22 | 5 | 27 | 101 | 15 | 116 | 18 | 2 | 20 | 0 | 0 | 0 | 2 | 2 | 4 | 11 | 5 | 16 | 19 | 0 | 19 | 0 | 0 | 0 |
| 7：15 to 7：30 | 29 | 4 | 33 | 118 | 17 | 135 | 9 | 1 | 10 | 0 | 0 | 0 | 3 | 2 | 5 | 12 | 1 | 13 | 19 | 0 | 19 | 0 | 0 | 0 |
| 7：30 to 7：45 | 21 | 2 | 23 | 151 | 15 | 166 | 18 | 3 | 21 | 0 | 0 | 0 | 11 | 1 | 12 | 11 | 3 | 14 | 14 | 1 | 15 | 0 | 0 | 0 |
| 7：45 to 8：00 | 41 | 2 | 43 | 178 | 12 | 190 | 21 | 0 | 21 | 0 | 0 | 0 | 6 | 2 | 8 | 14 | 1 | 15 | 28 | 0 | 28 | 0 | 0 | 0 |
| 8：00 to 8：15 | 49 | 2 | 51 | 195 | 10 | 205 | 27 | 4 | 31 | 0 | 0 | 0 | 8 | 2 | 10 | 20 | 1 | 21 | 34 | 4 | 38 | 0 | 0 | 0 |
| 8：15 to 8：30 | 40 | 2 | 42 | 185 | 12 | 197 | 35 | 5 | 40 | 0 | 0 | 0 | 10 | 2 | 12 | 26 | 0 | 26 | 33 | 1 | 34 | 0 | 0 | 0 |
| 8：30 to 8：45 | 46 | 5 | 51 | 150 | 16 | 166 | 33 | 2 | 35 | 0 | 0 | 0 | 12 | 1 | 13 | 27 | 0 | 27 | 30 | 0 | 30 | 0 | 0 | 0 |
| 8：45 to 9：00 | 69 | 7 | 76 | 170 | 17 | 187 | 23 | 4 | 27 | 0 | 0 | 0 | 8 | 1 | 9 | 52 | 1 | 53 | 28 | 2 | 30 | 0 | 0 | 0 |
| AM Totals | 317 | 29 | 346 | 1248 | 114 | 1362 | 184 | 21 | 205 | 0 | 0 | 0 | 60 | 13 | 73 | 173 | 12 | 185 | 205 | 8 | 213 | 0 | 0 | 0 |


| 16：00 to 16：15 | 150 | 6 | 156 | 289 | 8 | 297 | 29 | 2 | 31 | 0 | 0 | 0 | 15 | 3 | 18 | 55 | 0 | 55 | 61 | 1 | 62 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16：15 to 16：30 | 128 | 0 | 128 | 259 | 8 | 267 | 20 | 1 | 21 | 0 | 0 | 0 | 24 | 1 | 25 | 49 | 0 | 49 | 53 | 0 | 53 | 0 | 0 | 0 |
| 16：30 to 16：45 | 128 | 0 | 128 | 268 | 7 | 275 | 22 | 2 | 24 | 0 | 0 | 0 | 15 | 3 | 18 | 50 | 0 | 50 | 50 | 1 | 51 | 0 | 0 | 0 |
| 16：45 to 17：00 | 148 | 0 | 148 | 304 | 13 | 317 | 17 | 0 | 17 | 0 | 0 | 0 | 13 | 0 | 13 | 52 | 0 | 52 | 38 | 0 | 38 | 0 | 0 | 0 |
| 17：00 to 17：15 | 149 | 1 | 150 | 284 | 4 | 288 | 15 | 1 | 16 | 0 | 0 | 0 | 16 | 0 | 16 | 67 | 0 | 67 | 45 | 0 | 45 | 0 | 0 | 0 |
| 17：15 to 17：30 | 156 | 2 | 158 | 282 | 6 | 288 | 12 | 0 | 12 | 0 | 0 | 0 | 8 | 0 | 8 | 47 | 1 | 48 | 44 | 0 | 44 | 0 | 0 | 0 |
| 17：30 to 17：45 | 157 | 4 | 161 | 280 | 7 | 287 | 22 | 1 | 23 | 0 | 0 | 0 | 8 | 0 | 8 | 51 | 2 | 53 | 38 | 0 | 38 | 0 | 0 | 0 |
| 17：45 to 18：00 | 127 | 3 | 130 | 258 | 7 | 265 | 10 | 1 | 11 | 0 | 0 | 0 | 10 | 2 | 12 | 52 | 0 | 52 | 41 | 0 | 41 | 0 | 0 | 0 |
| PM Totals | 1143 | 16 | 1159 | 2224 | 60 | 2284 | 147 | 8 | 155 | 0 | 0 | 0 | 109 | 9 | 118 | 423 | 3 | 426 | 370 | 2 | 372 | 0 | 0 | 0 |


| Job No． | ：N790 |
| :--- | :--- |
| Client | ：Realty Realizations |
| Suburb | ：Nowra |
| Location | $:$ 7．Kalandar St／Princes Hwy |
|  |  |
| Day／Date | ：Fri，4th May 2012 |
| Weather | ：Fine |
| Description | ：Classified Intersection Count |
|  | $:$ Hourly Summary |



Princes Hwy

| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ |  |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\frac{\stackrel{7}{0}}{\underline{I}}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{1} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ |  | $\begin{aligned} & \text { ふ } \\ & \text { ふ } \\ & \text { ¹ } \end{aligned}$ | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ |  |  | $\begin{aligned} & \overline{\mathrm{O}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { ふ } \\ & \text { 㐅} \\ & \text { © } \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{O}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { ふ } \\ & \text { § } \\ & \text { ² } \end{aligned}$ | 픙 |
| 7：00 to 8：00 | 1 | 0 | 1 | 487 | 54 | 541 | 26 | 3 | 29 | 0 | 0 | 0 | 36 | 3 | 39 | 162 | 2 | 164 | 322 | 5 | 327 | 0 | 0 | 0 |
| 7：15 to 8：15 | 2 | 0 | 2 | 604 | 46 | 650 | 30 | 1 | 31 | 0 | 0 | 0 | 32 | 2 | 34 | 174 | 3 | 177 | 405 | 8 | 413 | 0 | 0 | 0 |
| 7：30 to 8：30 | 3 | 0 | 3 | 669 | 50 | 719 | 25 | 1 | 26 | 0 | 0 | 0 | 33 | 1 | 34 | 213 | 3 | 216 | 499 | 14 | 513 | 0 | 0 | 0 |
| 7：45 to 8：45 | 3 | 0 | 3 | 733 | 40 | 773 | 30 | 1 | 31 | 0 | 0 | 0 | 33 | 3 | 36 | 248 | 3 | 251 | 598 | 16 | 614 | 0 | 0 | 0 |
| 8：00 to 9：00 | 3 | 0 | 3 | 764 | 45 | 809 | 32 | 0 | 32 | 0 | 0 | 0 | 29 | 3 | 32 | 252 | 8 | 260 | 644 | 19 | 663 | 0 | 0 | 0 |
| AM Totals | 4 | 0 | 4 | 1251 | 99 | 1350 | 58 | 3 | 61 | 0 | 0 | 0 | 65 | 6 | 71 | 414 | 10 | 424 | 966 | 24 | 990 | 0 | 0 | 0 |
| 16：00 to 17：00 | 8 | 0 | 8 | 679 | 18 | 697 | 73 | 1 | 74 | 0 | 0 | 0 | 66 | 1 | 67 | 161 | 4 | 165 | 375 | 7 | 382 | 0 | 0 | 0 |
| 16：15 to 17：15 | 4 | 0 | 4 | 672 | 16 | 688 | 71 | 3 | 74 | 0 | 0 | 0 | 68 | 1 | 69 | 161 | 5 | 166 | 373 | 4 | 377 | 0 | 0 | 0 |
| 16：30 to $17: 30$ | 4 | 0 | 4 | 618 | 14 | 632 | 63 | 2 | 65 | 0 | 0 | 0 | 72 | 1 | 73 | 166 | 5 | 171 | 395 | 4 | 399 | 0 | 0 | 0 |
| 16：45 to 17：45 | 4 | 0 | 4 | 619 | 17 | 636 | 56 | 2 | 58 | 0 | 0 | 0 | 67 | 1 | 68 | 169 | 3 | 172 | 388 | 2 | 390 | 0 | 0 | 0 |
| 17：00 to 18：00 | 5 | 0 | 5 | 574 | 15 | 589 | 55 | 2 | 57 | 0 | 0 | 0 | 67 | 1 | 68 | 180 | 2 | 182 | 400 | 2 | 402 | 0 | 0 | 0 |
| PM Totals | 13 | 0 | 13 | 1253 | 33 | 1286 | 128 | 3 | 131 | 0 | 0 | 0 | 133 | 2 | 135 | 341 | 6 | 347 | 775 | 9 | 784 | 0 | 0 | 0 |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{O}} \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \mathrm{I} \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{\circ}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{I}} \\ & \hline \mathrm{~J} \end{aligned}$ | $\begin{aligned} & \text { ス } \\ & \text { त⿹丁口 } \\ & \text { in } \end{aligned}$ | $\begin{gathered} \overline{\mathrm{O}} \\ \stackrel{-}{\circ} \end{gathered}$ | $\frac{\stackrel{\rightharpoonup}{\mathrm{V}}}{\underline{\mathrm{~J}}}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{V}} \\ & \stackrel{\rightharpoonup}{\mathrm{O}} \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{I}} \\ & \hline \mathrm{~J} \end{aligned}$ | $\begin{aligned} & \text { } \\ & \text { त } \\ & \frac{0}{x} \end{aligned}$ | ¢ |
| 7：00 to 8：00 | 113 | 13 | 126 | 548 | 59 | 607 | 66 | 6 | 72 | 0 | 0 | 0 | 22 | 7 | 29 | 48 | 10 | 58 | 80 | 1 | 81 | 0 | 0 | 0 |
| 7：15 to 8：15 | 140 | 10 | 150 | 642 | 54 | 696 | 75 | 8 | 83 | 0 | 0 | 0 | 28 | 7 | 35 | 57 | 6 | 63 | 95 | 5 | 100 | 0 | 0 | 0 |
| 7：30 to 8：30 | 151 | 8 | 159 | 709 | 49 | 758 | 101 | 12 | 113 | 0 | 0 | 0 | 35 | 7 | 42 | 71 | 5 | 76 | 109 | 6 | 115 | 0 | 0 | 0 |
| 7：45 to 8：45 | 176 | 11 | 187 | 708 | 50 | 758 | 116 | 11 | 127 | 0 | 0 | 0 | 36 | 7 | 43 | 87 | 2 | 89 | 125 | 5 | 130 | 0 | 0 | 0 |
| 8：00 to 9：00 | 204 | 16 | 220 | 700 | 55 | 755 | 118 | 15 | 133 | 0 | 0 | 0 | 38 | 6 | 44 | 125 | 2 | 127 | 125 | 7 | 132 | 0 | 0 | 0 |
| AM Totals | 317 | 29 | 346 | 1248 | 114 | 1362 | 184 | 21 | 205 | 0 | 0 | 0 | 60 | 13 | 73 | 173 | 12 | 185 | 205 | 8 | 213 | 0 | 0 | 0 |
| 16：00 to 17：00 | 554 | 6 | 560 | 1120 | 36 | 1156 | 88 | 5 | 93 | 0 | 0 | 0 | 67 | 7 | 74 | 206 | 0 | 206 | 202 | 2 | 204 | 0 | 0 | 0 |
| 16：15 to 17：15 | 553 | 1 | 554 | 1115 | 32 | 1147 | 74 | 4 | 78 | 0 | 0 | 0 | 68 | 4 | 72 | 218 | 0 | 218 | 186 | 1 | 187 | 0 | 0 | 0 |
| 16：30 to $17: 30$ | 581 | 3 | 584 | 1138 | 30 | 1168 | 66 | 3 | 69 | 0 | 0 | 0 | 52 | 3 | 55 | 216 | 1 | 217 | 177 | 1 | 178 | 0 | 0 | 0 |
| 16：45 to 17：45 | 610 | 7 | 617 | 1150 | 30 | 1180 | 66 | 2 | 68 | 0 | 0 | 0 | 45 | 0 | 45 | 217 | 3 | 220 | 165 | 0 | 165 | 0 | 0 | 0 |
| 17：00 to 18：00 | 589 | 10 | 599 | 1104 | 24 | 1128 | 59 | 3 | 62 | 0 | 0 | 0 | 42 | 2 | 44 | 217 | 3 | 220 | 168 | 0 | 168 | 0 | 0 | 0 |
| PM Totals | 1143 | 16 | 1159 | 2224 | 60 | 2284 | 147 | 8 | 155 | 0 | 0 | 0 | 109 | 9 | 118 | 423 | 3 | 426 | 370 | 2 | 372 | 0 | 0 | 0 |




|  |  |
| :--- | :--- |
| Job No. | $:$ N790 |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $:$ 8. Forest Rd / Coonamia Rd |
| Day/Date | $:$ Fri, 4th May 2012 |
| Weather | : Fine |
| Description | $:$ Classified Intersection Count |
|  | $: 15$ mins Data |



| 16:00 to 16:15 | 9 | 0 | 9 | 32 | 2 | 34 | 0 | 0 | 0 | 32 | 2 | 34 | 9 | 0 | 9 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16:15 to 16:30 | 9 | 0 | 9 | 32 | 2 | 34 | 0 | 0 | 0 | 26 | 1 | 27 | 7 | 1 | 8 | 0 | 0 | 0 |
| 16:30 to 16:45 | 4 | 0 | 4 | 28 | 0 | 28 | 0 | 0 | 0 | 15 | 1 | 16 | 3 | 0 | 3 | 0 | 0 | 0 |
| 16:45 to 17:00 | 3 | 0 | 3 | 28 | 1 | 29 | 0 | 0 | 0 | 14 | 1 | 15 | 3 | 0 | 3 | 0 | 0 | 0 |
| 17:00 to 17:15 | 6 | 0 | 6 | 35 | 0 | 35 | 0 | 0 | 0 | 18 | 0 | 18 | 2 | 0 | 2 | 0 | 0 | 0 |
| 17:15 to 17:30 | 4 | 0 | 4 | 42 | 0 | 42 | 0 | 0 | 0 | 19 | 0 | 19 | 4 | 0 | 4 | 0 | 0 | 0 |
| 17:30 to 17:45 | 9 | 0 | 9 | 25 | 0 | 25 | 0 | 0 | 0 | 16 | 1 | 17 | 4 | 0 | 4 | 0 | 0 | 0 |
| 17:45 to 18:00 | 5 | 0 | 5 | 33 | 0 | 33 | 0 | 0 | 0 | 15 | 0 | 15 | 5 | 0 | 5 | 0 | 0 | 0 |
| PM Totals | 49 | 0 | 49 | 255 | 5 | 260 | 0 | 0 | 0 | 155 | 6 | 161 | 37 | 1 | 38 | 0 | 0 | 0 |


| Job No. | : N790 |
| :--- | :--- |
| Client | : Realty Realizations |
| Suburb | : Nowra |
| Location | $:$ 8. Forest Rd / Coonamia Rd |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | : Fine |
| Description | : Classified Intersection Count |
|  | $:$ Hourly Summary |


: Hourly Summary




| Approach <br> Direction | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U <br> （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 <br> （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \end{aligned}$ |  | $\begin{gathered} \text { 历̈ } \\ \stackrel{\circ}{\circ} \end{gathered}$ | $\begin{aligned} & \frac{7}{0} \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\pi}{3} \\ & \text { © } \\ & \text { In } \end{aligned}$ | $\begin{aligned} & \overline{\amalg ٓ} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{5} \\ & \stackrel{O}{J} \end{aligned}$ | $\begin{aligned} & \underset{0}{\lambda} \\ & \text { ָin } \end{aligned}$ | $\begin{aligned} & \bar{\leftrightarrows} \\ & \stackrel{\rightharpoonup}{\circ} \\ & \hline \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\boxed{\circ}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\pi}{\square} \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\text { In }} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { ㄷㅡㅡㄹ } \\ & \hline 0 \end{aligned}$ | $\begin{aligned} & \underset{\substack{3 \\ \text { I } \\ \text { I }}}{ } \end{aligned}$ | $\begin{aligned} & \stackrel{\Xi}{\circ} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $$ | $\begin{aligned} & \underset{\substack{3 \\ \mathbf{x} \\ \text { In }}}{ } \end{aligned}$ | $\begin{aligned} & \stackrel{\pi}{\circ} \\ & \stackrel{1}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & 3 \\ & \begin{array}{c} 3 \\ \text { I } \end{array} \end{aligned}$ | 픙 |
| 7：00 to 7：15 | 3 | 2 | 5 | 21 | 1 | 22 | 6 | 1 | 7 | 0 | 0 | 0 | 1 | 0 | 1 | 47 | 2 | 49 | 4 | 0 | 4 | 0 | 0 | 0 |
| 7：15 to 7：30 | 5 | 3 | 8 | 37 | 0 | 37 | 8 | 0 | 8 | 0 | 0 | 0 | 2 | 0 | 2 | 34 | 2 | 36 | 11 | 1 | 12 | 0 | 0 | 0 |
| 7：30 to 7：45 | 6 | 0 | 6 | 55 | 1 | 56 | 4 | 1 | 5 | 0 | 0 | 0 | 8 | 0 | 8 | 28 | 3 | 31 | 11 | 0 | 11 | 0 | 0 | 0 |
| 7：45 to 8：00 | 6 | 0 | 6 | 71 | 2 | 73 | 5 | 1 | 6 | 0 | 0 | 0 | 6 | 0 | 6 | 49 | 0 | 49 | 27 | 0 | 27 | 0 | 0 | 0 |
| 8：00 to 8：15 | 3 | 1 | 4 | 87 | 3 | 90 | 11 | 0 | 11 | 0 | 0 | 0 | 5 | 0 | 5 | 49 | 4 | 53 | 21 | 1 | 22 | 0 | 0 | 0 |
| 8：15 to 8：30 | 7 | 0 | 7 | 106 | 4 | 110 | 10 | 0 | 10 | 0 | 0 | 0 | 5 | 0 | 5 | 70 | 5 | 75 | 41 | 1 | 42 | 0 | 0 | 0 |
| 8：30 to 8：45 | 7 | 1 | 8 | 109 | 2 | 111 | 14 | 0 | 14 | 0 | 0 | 0 | 3 | 0 | 3 | 54 | 2 | 56 | 41 | 0 | 41 | 0 | 0 | 0 |
| 8：45 to 9：00 | 7 | 0 | 7 | 94 | 5 | 99 | 15 | 0 | 15 | 0 | 0 | 0 | 12 | 0 | 12 | 49 | 9 | 58 | 26 | 1 | 27 | 0 | 0 | 0 |
| AM Totals | 44 | 7 | 51 | 580 | 18 | 598 | 73 | 3 | 76 | 0 | 0 | 0 | 42 | 0 | 42 | 380 | 27 | 407 | 182 | 4 | 186 | 0 | 0 | 0 |
| 16：00 to 16：15 | 3 | 0 | 3 | 69 | 0 | 69 | 15 | 0 | 15 | 0 | 0 | 0 | 9 | 0 | 9 | 43 | 4 | 47 | 24 | 0 | 24 | 0 | 0 | 0 |
| 16：15 to $16: 30$ | 7 | 1 | 8 | 73 | 1 | 74 | 21 | 0 | 21 | 0 | 0 | 0 | 8 | 0 | 8 | 35 | 2 | 37 | 20 | 0 | 20 | 0 | 0 | 0 |
| 16：30 to $16: 45$ | 4 | 0 | 4 | 44 | 0 | 44 | 9 | 0 | 9 | 1 | 0 | 1 | 5 | 1 | 6 | 39 | 2 | 41 | 13 | 0 | 13 | 0 | 0 | 0 |
| 16：45 to 17：00 | 8 | 0 | 8 | 33 | 0 | 33 | 7 | 0 | 7 | 0 | 0 | 0 | 4 | 0 | 4 | 35 | 0 | 35 | 19 | 1 | 20 | 1 | 0 | 1 |
| 17：00 to 17：15 | 3 | 1 | 4 | 40 | 1 | 41 | 10 | 0 | 10 | 0 | 0 | 0 | 5 | 0 | 5 | 27 | 2 | 29 | 23 | 0 | 23 | 0 | 0 | 0 |
| 17：15 to 17：30 | 8 | 0 | 8 | 43 | 0 | 43 | 10 | 0 | 10 | 0 | 0 | 0 | 7 | 0 | 7 | 32 | 2 | 34 | 15 | 0 | 15 | 0 | 0 | 0 |
| 17：30 to 17：45 | 3 | 0 | 3 | 32 | 0 | 32 | 13 | 1 | 14 | 0 | 0 | 0 | 14 | 0 | 14 | 36 | 1 | 37 | 18 | 0 | 18 | 1 | 0 | 1 |
| 17：45 to 18：00 | 1 | 0 | 1 | 32 | 0 | 32 | 15 | 0 | 15 | 0 | 0 | 0 | 6 | 0 | 6 | 30 | 1 | 31 | 23 | 0 | 23 | 1 | 0 | 1 |
| PM Totals | 37 | 2 | 39 | 366 | 2 | 368 | 100 | 1 | 101 | 1 | 0 | 1 | 58 | 1 | 59 | 277 | 14 | 291 | 155 | 1 | 156 | 3 | 0 | 3 |


| Job No． | $:$ N790 |
| :--- | :--- |
| Client | ：Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $: 9$. Kalandar St／Kinghorne St |
|  |  |
| Day／Date | ：Fri，4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |

： 15 mins Data
 15 лериегея


SKYHIGH－THE TRAFFIC SURVEY COMPANY

| Approach | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 <br> （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 哥 } \\ & \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{\circ}{1} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & .0 \end{aligned}$ |  | $\begin{gathered} \overline{Ð 5} \\ \stackrel{\circ}{\circ} \\ \hline \end{gathered}$ | $\frac{\text { 苛 }}{3}$ | $\begin{aligned} & \underset{\substack{\lambda \\ \\ ~}}{ } \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \sum_{\nwarrow}^{\lambda} \\ & \text { © } \end{aligned}$ | $\begin{aligned} & \overline{\ddagger 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\mathrm{IN}} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \underline{I} \end{aligned}$ | $\begin{aligned} & \gtrless_{0}^{7} \\ & \text { 오 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $$ |  | П |
| 7：00 to 7：15 | 9 | 1 | 10 | 2 | 0 | 2 | 110 | 1 | 111 | 0 | 0 | 0 | 19 | 0 | 19 | 15 | 5 | 20 | 0 | 1 | 1 | 0 | 0 | 0 |
| 7：15 to 7：30 | 15 | 1 | 16 | 2 | 0 | 2 | 101 | 1 | 102 | 0 | 0 | 0 | 27 | 1 | 28 | 10 | 2 | 12 | 2 | 0 | 2 | 0 | 0 | 0 |
| 7：30 to 7：45 | 12 | 1 | 13 | 4 | 0 | 4 | 42 | 1 | 43 | 0 | 0 | 0 | 27 | 2 | 29 | 21 | 3 | 24 | 1 | 0 | 1 | 0 | 0 | 0 |
| 7：45 to 8：00 | 18 | 0 | 18 | 6 | 0 | 6 | 52 | 2 | 54 | 0 | 0 | 0 | 33 | 0 | 33 | 21 | 3 | 24 | 0 | 0 | 0 | 0 | 0 | 0 |
| 8：00 to 8：15 | 19 | 1 | 20 | 7 | 0 | 7 | 35 | 1 | 36 | 0 | 0 | 0 | 43 | 1 | 44 | 45 | 7 | 52 | 1 | 1 | 2 | 0 | 0 | 0 |
| 8：15 to 8：30 | 16 | 0 | 16 | 3 | 0 | 3 | 39 | 2 | 41 | 2 | 0 | 2 | 54 | 3 | 57 | 35 | 3 | 38 | 1 | 0 | 1 | 0 | 0 | 0 |
| 8：30 to 8：45 | 20 | 0 | 20 | 7 | 0 | 7 | 38 | 1 | 39 | 1 | 0 | 1 | 63 | 3 | 66 | 38 | 1 | 39 | 3 | 0 | 3 | 0 | 0 | 0 |
| 8：45 to 9：00 | 49 | 3 | 52 | 9 | 0 | 9 | 32 | 1 | 33 | 2 | 0 | 2 | 51 | 3 | 54 | 44 | 2 | 46 | 2 | 1 | 3 | 0 | 0 | 0 |
| AM Totals | 158 | 7 | 165 | 40 | 0 | 40 | 449 | 10 | 459 | 5 | 0 | 5 | 317 | 13 | 330 | 229 | 26 | 255 | 10 | 3 | 13 | 0 | 0 | 0 |


| 16：00 to 16：15 | 60 | 1 | 61 | 14 | 1 | 15 | 52 | 2 | 54 | 1 | 0 | 1 | 65 | 0 | 65 | 53 | 2 | 55 | 2 | 0 | 2 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16：15 to 16：30 | 48 | 0 | 48 | 15 | 0 | 15 | 49 | 3 | 52 | 1 | 0 | 1 | 58 | 1 | 59 | 61 | 1 | 62 | 3 | 0 | 3 | 0 | 0 | 0 |
| 16：30 to 16：45 | 47 | 1 | 48 | 16 | 1 | 17 | 45 | 2 | 47 | 0 | 0 | 0 | 42 | 1 | 43 | 61 | 1 | 62 | 4 | 0 | 4 | 0 | 0 | 0 |
| 16：45 to 17：00 | 55 | 0 | 55 | 7 | 0 | 7 | 52 | 0 | 52 | 0 | 0 | 0 | 35 | 0 | 35 | 41 | 0 | 41 | 1 | 0 | 1 | 0 | 0 | 0 |
| 17：00 to 17：15 | 75 | 0 | 75 | 16 | 0 | 16 | 42 | 2 | 44 | 0 | 0 | 0 | 50 | 2 | 52 | 56 | 1 | 57 | 3 | 0 | 3 | 0 | 0 | 0 |
| 17：15 to 17：30 | 46 | 0 | 46 | 12 | 0 | 12 | 53 | 2 | 55 | 2 | 0 | 2 | 40 | 0 | 40 | 42 | 0 | 42 | 4 | 0 | 4 | 0 | 0 | 0 |
| 17：30 to 17：45 | 49 | 0 | 49 | 9 | 0 | 9 | 53 | 1 | 54 | 0 | 0 | 0 | 31 | 1 | 32 | 50 | 1 | 51 | 0 | 0 | 0 | 0 | 0 | 0 |
| 17：45 to 18：00 | 39 | 0 | 39 | 10 | 0 | 10 | 35 | 1 | 36 | 0 | 0 | 0 | 46 | 1 | 47 | 37 | 1 | 38 | 1 | 0 | 1 | 0 | 0 | 0 |
| PM Totals | 419 | 2 | 421 | 99 | 2 | 101 | 381 | 13 | 394 | 4 | 0 | 4 | 367 | 6 | 373 | 401 | 7 | 408 | 18 | 0 | 18 | 0 | 0 | 0 |


| Job No． | $:$ N790 |
| :--- | :--- |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $: 9$. Kalandar St／Kinghorne St |
|  |  |
| Day／Date | ：Fri，4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |



SKYHIGH－THE TRAFFIC SURVEY COMPANY

| Approach | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\text { 号 }}{3} \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{0} \\ & \hline \end{aligned}$ |  | 픈 | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{O}} \\ & \stackrel{y}{\mathrm{O}} \end{aligned}$ | $\begin{aligned} & \text { ふ } \\ & \text { オ } \\ & \text { d } \end{aligned}$ | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline 1 \end{aligned}$ |  | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\text { Z }}{\substack{x}} \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ |  |  | 픈 | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { \} } \\ {\text { 㐅}} \\ {\text { 오 }} \end{aligned}$ | \} |
| 7：00 to 8：00 | 20 | 5 | 25 | 184 | 4 | 188 | 23 | 3 | 26 | 0 | 0 | 0 | 17 | 0 | 17 | 158 | 7 | 165 | 53 | 1 | 54 | 0 | 0 | 0 |
| 7：15 to 8：15 | 20 | 4 | 24 | 250 | 6 | 256 | 28 | 2 | 30 | 0 | 0 | 0 | 21 | 0 | 21 | 160 | 9 | 169 | 70 | 2 | 72 | 0 | 0 | 0 |
| 7：30 to 8：30 | 22 | 1 | 23 | 319 | 10 | 329 | 30 | 2 | 32 | 0 | 0 | 0 | 24 | 0 | 24 | 196 | 12 | 208 | 100 | 2 | 102 | 0 | 0 | 0 |
| 7：45 to 8：45 | 23 | 2 | 25 | 373 | 11 | 384 | 40 | 1 | 41 | 0 | 0 | 0 | 19 | 0 | 19 | 222 | 11 | 233 | 130 | 2 | 132 | 0 | 0 | 0 |
| 8：00 to 9：00 | 24 | 2 | 26 | 396 | 14 | 410 | 50 | 0 | 50 | 0 | 0 | 0 | 25 | 0 | 25 | 222 | 20 | 242 | 129 | 3 | 132 | 0 | 0 | 0 |
| AM Totals | 44 | 7 | 51 | 580 | 18 | 598 | 73 | 3 | 76 | 0 | 0 | 0 | 42 | 0 | 42 | 380 | 27 | 407 | 182 | 4 | 186 | 0 | 0 | 0 |
| 16：00 to 17：00 | 22 | 1 | 23 | 219 | 1 | 220 | 52 | 0 | 52 | 1 | 0 | 1 | 26 | 1 | 27 | 152 | 8 | 160 | 76 | 1 | 77 | 1 | 0 | 1 |
| 16：15 to 17：15 | 22 | 2 | 24 | 190 | 2 | 192 | 47 | 0 | 47 | 1 | 0 | 1 | 22 | 1 | 23 | 136 | 6 | 142 | 75 | 1 | 76 | 1 | 0 | 1 |
| 16：30 to 17：30 | 23 | 1 | 24 | 160 | 1 | 161 | 36 | 0 | 36 | 1 | 0 | 1 | 21 | 1 | 22 | 133 | 6 | 139 | 70 | 1 | 71 | 1 | 0 | 1 |
| 16：45 to 17：45 | 22 | 1 | 23 | 148 | 1 | 149 | 40 | 1 | 41 | 0 | 0 | 0 | 30 | 0 | 30 | 130 | 5 | 135 | 75 | 1 | 76 | 2 | 0 | 2 |
| 17：00 to 18：00 | 15 | 1 | 16 | 147 | 1 | 148 | 48 | 1 | 49 | 0 | 0 | 0 | 32 | 0 | 32 | 125 | 6 | 131 | 79 | 0 | 79 | 2 | 0 | 2 |
| PM Totals | 37 | 2 | 39 | 366 | 2 | 368 | 100 | 1 | 101 | 1 | 0 | 1 | 58 | 1 | 59 | 277 | 14 | 291 | 155 | 1 | 156 | 3 | 0 | 3 |


| Approach | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 吉 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ | $\frac{\stackrel{7}{0}}{\underline{I}}$ | $\begin{aligned} & 3 \\ & \begin{array}{l} \lambda \\ \\ \hline \end{array} \end{aligned}$ | $\begin{aligned} & \overline{Ð 5} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{V}} \\ & \hline \mathrm{~J} \end{aligned}$ |  | $\begin{gathered} \overline{\boxed{\circ}} \\ \stackrel{-}{\circ} \end{gathered}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{I}} \\ & \hline \mathrm{~J} \end{aligned}$ | $\begin{aligned} & \lambda_{\pi}^{\lambda} \\ & \text { N్土 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{\boxed{5}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & 7 \\ & \substack{0 \\ \text { N } \\ \hline} \end{aligned}$ | $\begin{gathered} \overline{\boxed{0}} \\ \stackrel{-}{\circ} \end{gathered}$ | $\frac{\stackrel{\rightharpoonup}{\mathrm{I}}}{\underline{\mathrm{I}}}$ |  | $\begin{gathered} \overline{\boxed{O}} \\ \stackrel{-}{\circ} \end{gathered}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{I}} \\ & \hline \mathrm{~J} \end{aligned}$ |  | 든 |
| 7：00 to 8：00 | 54 | 3 | 57 | 14 | 0 | 14 | 305 | 5 | 310 | 0 | 0 | 0 | 106 | 3 | 109 | 67 | 13 | 80 | 3 | 1 | 4 | 0 | 0 | 0 |
| 7：15 to 8：15 | 64 | 3 | 67 | 19 | 0 | 19 | 230 | 5 | 235 | 0 | 0 | 0 | 130 | 4 | 134 | 97 | 15 | 112 | 4 | 1 | 5 | 0 | 0 | 0 |
| 7：30 to 8：30 | 65 | 2 | 67 | 20 | 0 | 20 | 168 | 6 | 174 | 2 | 0 | 2 | 157 | 6 | 163 | 122 | 16 | 138 | 3 | 1 | 4 | 0 | 0 | 0 |
| 7：45 to 8：45 | 73 | 1 | 74 | 23 | 0 | 23 | 164 | 6 | 170 | 3 | 0 | 3 | 193 | 7 | 200 | 139 | 14 | 153 | 5 | 1 | 6 | 0 | 0 | 0 |
| 8：00 to 9：00 | 104 | 4 | 108 | 26 | 0 | 26 | 144 | 5 | 149 | 5 | 0 | 5 | 211 | 10 | 221 | 162 | 13 | 175 | 7 | 2 | 9 | 0 | 0 | 0 |
| AM Totals | 158 | 7 | 165 | 40 | 0 | 40 | 449 | 10 | 459 | 5 | 0 | 5 | 317 | 13 | 330 | 229 | 26 | 255 | 10 | 3 | 13 | 0 | 0 | 0 |
| 16：00 to 17：00 | 210 | 2 | 212 | 52 | 2 | 54 | 198 | 7 | 205 | 2 | 0 | 2 | 200 | 2 | 202 | 216 | 4 | 220 | 10 | 0 | 10 | 0 | 0 | 0 |
| 16：15 to 17：15 | 225 | 1 | 226 | 54 | 1 | 55 | 188 | 7 | 195 | 1 | 0 | 1 | 185 | 4 | 189 | 219 | 3 | 222 | 11 | 0 | 11 | 0 | 0 | 0 |
| 16：30 to 17：30 | 223 | 1 | 224 | 51 | 1 | 52 | 192 | 6 | 198 | 2 | 0 | 2 | 167 | 3 | 170 | 200 | 2 | 202 | 12 | 0 | 12 | 0 | 0 | 0 |
| 16：45 to 17：45 | 225 | 0 | 225 | 44 | 0 | 44 | 200 | 5 | 205 | 2 | 0 | 2 | 156 | 3 | 159 | 189 | 2 | 191 | 8 | 0 | 8 | 0 | 0 | 0 |
| 17：00 to 18：00 | 209 | 0 | 209 | 47 | 0 | 47 | 183 | 6 | 189 | 2 | 0 | 2 | 167 | 4 | 171 | 185 | 3 | 188 | 8 | 0 | 8 | 0 | 0 | 0 |
| PM Totals | 419 | 2 | 421 | 99 | 2 | 101 | 381 | 13 | 394 | 4 | 0 | 4 | 367 | 6 | 373 | 401 | 7 | 408 | 18 | 0 | 18 | 0 | 0 | 0 |


|  |  |
| :--- | :--- |
| Job No. | : N790 |
| Client | : Realty Realizations |
| Suburb | : Nowra |
| Location | $:$ 10. Forest Rd / Princes Hwy |
|  | $:$ Fri, 4th May 2012 |
| Day/Date | : Fine |
| Weather |  |
| Description | $:$ Classified Intersection Count |
|  | $: 15$ mins Data |






| 16:00 to 16:15 | 20 | 0 | 20 | 359 | 10 | 369 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16:15 to 16:30 | 28 | 2 | 30 | 306 | 10 | 316 | 0 | 0 | 0 |
| 16:30 to 16:45 | 29 | 0 | 29 | 279 | 5 | 284 | 1 | 0 | 1 |
| 16:45 to 17:00 | 21 | 0 | 21 | 339 | 1 | 340 | 0 | 0 | 0 |
| 17:00 to 17:15 | 32 | 1 | 33 | 340 | 8 | 348 | 0 | 0 | 0 |
| 17:15 to 17:30 | 29 | 0 | 29 | 324 | 7 | 331 | 0 | 0 | 0 |
| 17:30 to 17:45 | 30 | 0 | 30 | 294 | 5 | 299 | 0 | 0 | 0 |
| 17:45 to 18:00 | 16 | 1 | 17 | 263 | 4 | 267 | 0 | 0 | 0 |
| PM Totals | 205 | 4 | 209 | 2504 | 50 | 2554 | 1 | 0 | 1 |


| Job No. | $:$ N790 |
| :--- | :--- |
| Client | $:$ Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $:$ 10. Forest Rd / Princes Hwy |
|  |  |
| Day/Date | : Fri, 4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |






| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 <br> （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\text { IN }} \\ & \stackrel{0}{1} \\ & \hline \end{aligned}$ |  | $\begin{array}{r} 3 \\ \substack{3 \\ \text { I } \\ \hline \\ \hline} \end{array}$ | $\begin{aligned} & \bar{\Pi} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\text { IN }} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $$ | $\begin{aligned} & \underset{\text { J }}{\substack{\text { In }}} \end{aligned}$ |  |  | $\begin{array}{r} \underset{\pi}{3} \\ \stackrel{\pi}{I} \\ \hline \end{array}$ | $\begin{aligned} & \overline{\mathrm{O}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{array}{r} \substack{3 \\ \vdots \\ \text { 0 } \\ \hline \\ \hline} \end{array}$ | $\begin{aligned} & \text { Г̄ } \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $$ | $\begin{array}{r} 3 \\ \begin{array}{c} 3 \\ \text { 0 } \\ \hline \end{array} \\ \hline \end{array}$ | 픙 |
| 7：00 to 7：15 | 0 | 0 | 0 | 95 | 26 | 121 | 3 | 0 | 3 | 0 | 0 | 0 | 3 | 0 | 3 | 13 | 0 | 13 | 6 | 0 | 6 | 0 | 0 | 0 |
| 7：15 to 7：30 | 2 | 0 | 2 | 138 | 11 | 149 | 5 | 1 | 6 | 0 | 0 | 0 | 2 | 0 | 2 | 5 | 0 | 5 | 19 | 5 | 24 | 0 | 0 | 0 |
| 7：30 to 7：45 | 0 | 0 | 0 | 145 | 22 | 167 | 4 | 1 | 5 | 0 | 0 | 0 | 4 | 0 | 4 | 18 | 0 | 18 | 36 | 0 | 36 | 0 | 0 | 0 |
| 7：45 to 8：00 | 0 | 0 | 0 | 153 | 12 | 165 | 9 | 0 | 9 | 0 | 0 | 0 | 1 | 0 | 1 | 33 | 0 | 33 | 23 | 0 | 23 | 0 | 0 | 0 |
| 8：00 to 8：15 | 0 | 0 | 0 | 143 | 16 | 159 | 12 | 2 | 14 | 0 | 0 | 0 | 5 | 0 | 5 | 37 | 3 | 40 | 42 | 1 | 43 | 0 | 0 | 0 |
| $8: 15$ to $8: 30$ | 0 | 0 | 0 | 188 | 15 | 203 | 22 | 3 | 25 | 0 | 0 | 0 | 4 | 1 | 5 | 41 | 2 | 43 | 42 | 4 | 46 | 0 | 0 | 0 |
| $8: 30$ to $8: 45$ | 3 | 0 | 3 | 202 | 13 | 215 | 47 | 0 | 47 | 0 | 0 | 0 | 8 | 0 | 8 | 48 | 3 | 51 | 66 | 5 | 71 | 0 | 0 | 0 |
| 8：45 to 9：00 | 0 | 0 | 0 | 169 | 14 | 183 | 48 | 0 | 48 | 0 | 0 | 0 | 11 | 0 | 11 | 59 | 0 | 59 | 58 | 0 | 58 | 0 | 0 | 0 |
| AM Totals | 5 | 0 | 5 | 1233 | 129 | 1362 | 150 | 7 | 157 | 0 | 0 | 0 | 38 | 1 | 39 | 254 | 8 | 262 | 292 | 15 | 307 | 0 | 0 | 0 |
| 16：00 to 16：15 | 2 | 0 | 2 | 250 | 8 | 258 | 21 | 0 | 21 | 1 | 0 | 1 | 5 | 0 | 5 | 28 | 0 | 28 | 51 | 2 | 53 | 0 | 0 | 0 |
| 16：15 to 16：30 | 1 | 0 | 1 | 216 | 8 | 224 | 22 | 0 | 22 | 0 | 0 | 0 | 6 | 1 | 7 | 31 | 0 | 31 | 37 | 0 | 37 | 0 | 0 | 0 |
| 16：30 to 16：45 | 0 | 0 | 0 | 273 | 11 | 284 | 21 | 0 | 21 | 0 | 0 | 0 | 5 | 0 | 5 | 23 | 0 | 23 | 34 | 1 | 35 | 0 | 0 | 0 |
| 16：45 to 17：00 | 4 | 0 | 4 | 222 | 4 | 226 | 13 | 0 | 13 | 0 | 0 | 0 | 6 | 0 | 6 | 19 | 0 | 19 | 31 | 0 | 31 | 0 | 0 | 0 |
| 17：00 to 17：15 | 0 | 0 | 0 | 272 | 4 | 276 | 9 | 0 | 9 | 0 | 0 | 0 | 2 | 0 | 2 | 18 | 0 | 18 | 28 | 1 | 29 | 0 | 0 | 0 |
| 17：15 to 17：30 | 1 | 0 | 1 | 239 | 3 | 242 | 21 | 0 | 21 | 0 | 0 | 0 | 1 | 0 | 1 | 11 | 0 | 11 | 27 | 0 | 27 | 0 | 0 | 0 |
| 17：30 to 17：45 | 1 | 0 | 1 | 217 | 6 | 223 | 11 | 0 | 11 | 1 | 0 | 1 | 2 | 0 | 2 | 25 | 0 | 25 | 32 | 0 | 32 | 0 | 0 | 0 |
| 17：45 to 18：00 | 1 | 0 | 1 | 194 | 6 | 200 | 7 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 0 | 15 | 25 | 0 | 25 | 0 | 0 | 0 |
| PM Totals | 10 | 0 | 10 | 1883 | 50 | 1933 | 125 | 0 | 125 | 2 | 0 | 2 | 27 | 1 | 28 | 170 | 0 | 170 | 265 | 4 | 269 | 0 | 0 | 0 |


： 15 mins Data
Princes Hwy

| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 <br> （Left Turn） |  |  | Direction 8 <br> （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 <br> （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period |  |  | $\begin{aligned} & \overline{\# 5} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{5} \\ & \stackrel{\circ}{1} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \underline{I} \end{aligned}$ |  | $\begin{aligned} & \overline{5} \\ & \stackrel{\circ}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline . . \end{aligned}$ |  | $\begin{aligned} & \overline{5} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ | $$ |  | $\begin{aligned} & \overline{\mathrm{N}} \\ & \stackrel{-}{\circ} \end{aligned}$ | $$ | $\begin{aligned} & \sum_{J}^{\gtrless} \\ & \text { © } \end{aligned}$ | $\begin{aligned} & \overline{5} \\ & \stackrel{0}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | \} | $\begin{aligned} & \overline{\mathrm{IN}} \\ & \stackrel{0}{1} \end{aligned}$ |  | \} $\\ {\substack{\text { ¢ } \\ \text { ¢ }}} \\ {\hline}$ | П |
| 7：00 to 7：15 | 9 | 1 | 10 | 214 | 22 | 236 | 34 | 2 | 36 | 0 | 0 | 0 | 10 | 0 | 10 | 0 | 0 | 0 | 2 | 1 | 3 | 0 | 0 | 0 |
| 7：15 to 7：30 | 6 | 1 | 7 | 220 | 16 | 236 | 27 | 2 | 29 | 0 | 0 | 0 | 13 | 1 | 14 | 7 | 1 | 8 | 5 | 1 | 6 | 0 | 0 | 0 |
| 7：30 to 7：45 | 14 | 1 | 15 | 247 | 15 | 262 | 57 | 3 | 60 | 0 | 0 | 0 | 16 | 2 | 18 | 7 | 0 | 7 | 5 | 2 | 7 | 0 | 0 | 0 |
| 7：45 to 8：00 | 23 | 1 | 24 | 282 | 19 | 301 | 82 | 4 | 86 | 0 | 0 | 0 | 16 | 0 | 16 | 10 | 1 | 11 | 7 | 0 | 7 | 0 | 0 | 0 |
| 8：00 to 8：15 | 27 | 0 | 27 | 271 | 17 | 288 | 72 | 0 | 72 | 0 | 0 | 0 | 21 | 4 | 25 | 20 | 0 | 20 | 6 | 2 | 8 | 0 | 0 | 0 |
| 8：15 to 8：30 | 43 | 2 | 45 | 289 | 23 | 312 | 74 | 3 | 77 | 0 | 0 | 0 | 30 | 3 | 33 | 25 | 2 | 27 | 9 | 1 | 10 | 0 | 0 | 0 |
| 8：30 to 8：45 | 39 | 4 | 43 | 261 | 15 | 276 | 73 | 5 | 78 | 0 | 0 | 0 | 20 | 2 | 22 | 34 | 2 | 36 | 14 | 2 | 16 | 0 | 0 | 0 |
| 8：45 to 9：00 | 42 | 2 | 44 | 296 | 27 | 323 | 91 | 4 | 95 | 0 | 0 | 0 | 25 | 2 | 27 | 28 | 0 | 28 | 8 | 0 | 8 | 0 | 0 | 0 |
| AM Totals | 203 | 12 | 215 | 2080 | 154 | 2234 | 510 | 23 | 533 | 0 | 0 | 0 | 151 | 14 | 165 | 131 | 6 | 137 | 56 | 9 | 65 | 0 | 0 | 0 |


| 16：00 to 16：15 | 28 | 1 | 29 | 267 | 15 | 282 | 81 | 1 | 82 | 0 | 0 | 0 | 85 | 0 | 85 | 40 | 0 | 40 | 20 | 0 | 20 | 0 | 0 | 0 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 16：15 to 16：30 | 36 | 2 | 38 | 273 | 6 | 279 | 69 | 1 | 70 | 0 | 0 | 0 | 91 | 2 | 93 | 42 | 0 | 42 | 28 | 1 | 29 | 0 | 0 | 0 |
| 16：30 to 16：45 | 31 | 1 | 32 | 343 | 13 | 356 | 62 | 0 | 62 | 0 | 0 | 0 | 65 | 2 | 67 | 27 | 0 | 27 | 14 | 0 | 14 | 0 | 0 | 0 |
| 16：45 to 17：00 | 34 | 1 | 35 | 247 | 7 | 254 | 61 | 0 | 61 | 0 | 0 | 0 | 96 | 0 | 96 | 56 | 0 | 56 | 28 | 0 | 28 | 1 | 0 | 1 |
| 17：00 to 17：15 | 35 | 1 | 36 | 308 | 7 | 315 | 74 | 0 | 74 | 0 | 0 | 0 | 83 | 0 | 83 | 51 | 0 | 51 | 35 | 1 | 36 | 0 | 0 | 0 |
| 17：15 to 17：30 | 25 | 0 | 25 | 273 | 10 | 283 | 48 | 1 | 49 | 0 | 0 | 0 | 70 | 1 | 71 | 32 | 0 | 32 | 17 | 0 | 17 | 0 | 0 | 0 |
| 17：30 to 17：45 | 33 | 0 | 33 | 280 | 7 | 287 | 58 | 1 | 59 | 0 | 0 | 0 | 78 | 0 | 78 | 32 | 0 | 32 | 25 | 0 | 25 | 0 | 0 | 0 |
| 17：45 to 18：00 | 19 | 0 | 19 | 268 | 10 | 278 | 47 | 0 | 47 | 0 | 0 | 0 | 60 | 0 | 60 | 26 | 0 | 26 | 15 | 0 | 15 | 0 | 0 | 0 |
| PM Totals | 241 | 6 | 247 | 2259 | 75 | 2334 | 500 | 4 | 504 | 0 | 0 | 0 | 628 | 5 | 633 | 306 | 0 | 306 | 182 | 2 | 184 | 1 | 0 | 1 |


| Job No． | $:$ N790 |
| :--- | :--- |
| Client | ：Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $:$ 11．Moss St／Princes Hwy |
|  |  |
| Day／Date | ：Fri，4th May 2012 |
| Weather | $:$ Fine |
| Description | $:$ Classified Intersection Count |
|  | $:$ Hourly Summary |



SKYHIGH－THE TRAFFIC SURVEY COMPANY

| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 <br> （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 <br> （Through） |  |  | Direction 6 （Right Turn） |  |  | Direction 6U （U Turn） |  |  |
| Time Period |  |  | $\begin{aligned} & \overline{\amalg ँ} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \mathrm{J} \end{aligned}$ |  | $\begin{aligned} & \overline{\amalg ँ} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{\mathrm{I}} \\ & \hline \mathrm{~J} \end{aligned}$ |  | $\begin{aligned} & \overline{\text { In }} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\frac{\text { 苛 }}{3}$ | $\begin{aligned} & \underset{\nwarrow}{\lambda} \\ & \stackrel{\text { x }}{\mathbf{I}} \end{aligned}$ | $\begin{aligned} & \overline{\text { In }} \\ & \stackrel{1}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \underset{\pi}{\grave{~}} \\ & \stackrel{y}{x} \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{-}{\circ} \end{aligned}$ | 苛 | $\begin{aligned} & {\underset{刃}{\mathbf{J}}}_{\substack{\text { I }}} \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{O}} \\ & \stackrel{-}{\circ} \end{aligned}$ |  | $\begin{aligned} & \underset{\pi}{\grave{~ N}} \\ & \stackrel{y}{I} \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $$ | 入 <br> ¢ <br> ¢ | \} |
| 7：00 to 8：00 | 2 | 0 | 2 | 531 | 71 | 602 | 21 | 2 | 23 | 0 | 0 | 0 | 10 | 0 | 10 | 69 | 0 | 69 | 84 | 5 | 89 | 0 | 0 | 0 |
| 7：15 to 8：15 | 2 | 0 | 2 | 579 | 61 | 640 | 30 | 4 | 34 | 0 | 0 | 0 | 12 | 0 | 12 | 93 | 3 | 96 | 120 | 6 | 126 | 0 | 0 | 0 |
| 7：30 to 8：30 | 0 | 0 | 0 | 629 | 65 | 694 | 47 | 6 | 53 | 0 | 0 | 0 | 14 | 1 | 15 | 129 | 5 | 134 | 143 | 5 | 148 | 0 | 0 | 0 |
| 7：45 to 8：45 | 3 | 0 | 3 | 686 | 56 | 742 | 90 | 5 | 95 | 0 | 0 | 0 | 18 | 1 | 19 | 159 | 8 | 167 | 173 | 10 | 183 | 0 | 0 | 0 |
| 8：00 to 9：00 | 3 | 0 | 3 | 702 | 58 | 760 | 129 | 5 | 134 | 0 | 0 | 0 | 28 | 1 | 29 | 185 | 8 | 193 | 208 | 10 | 218 | 0 | 0 | 0 |
| AM Totals | 5 | 0 | 5 | 1233 | 129 | 1362 | 150 | 7 | 157 | 0 | 0 | 0 | 38 | 1 | 39 | 254 | 8 | 262 | 292 | 15 | 307 | 0 | 0 | 0 |
| 16：00 to 17：00 | 7 | 0 | 7 | 961 | 31 | 992 | 77 | 0 | 77 | 1 | 0 | 1 | 22 | 1 | 23 | 101 | 0 | 101 | 153 | 3 | 156 | 0 | 0 | 0 |
| 16：15 to 17：15 | 5 | 0 | 5 | 983 | 27 | 1010 | 65 | 0 | 65 | 0 | 0 | 0 | 19 | 1 | 20 | 91 | 0 | 91 | 130 | 2 | 132 | 0 | 0 | 0 |
| 16：30 to 17：30 | 5 | 0 | 5 | 1006 | 22 | 1028 | 64 | 0 | 64 | 0 | 0 | 0 | 14 | 0 | 14 | 71 | 0 | 71 | 120 | 2 | 122 | 0 | 0 | 0 |
| 16：45 to 17：45 | 6 | 0 | 6 | 950 | 17 | 967 | 54 | 0 | 54 | 1 | 0 | 1 | 11 | 0 | 11 | 73 | 0 | 73 | 118 | 1 | 119 | 0 | 0 | 0 |
| 17：00 to 18：00 | 3 | 0 | 3 | 922 | 19 | 941 | 48 | 0 | 48 | 1 | 0 | 1 | 5 | 0 | 5 | 69 | 0 | 69 | 112 | 1 | 113 | 0 | 0 | 0 |
| PM Totals | 10 | 0 | 10 | 1883 | 50 | 1933 | 125 | 0 | 125 | 2 | 0 | 2 | 27 | 1 | 28 | 170 | 0 | 170 | 265 | 4 | 269 | 0 | 0 | 0 |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ |  |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ |  |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 吉 } \\ & \hline 1 \end{aligned}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \stackrel{\circ}{\circ} \end{aligned}$ | $\frac{\text { 岢 }}{3}$ |  | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{1}{\circ} \end{aligned}$ | $\frac{\stackrel{7}{0}}{\underline{I}}$ | $\begin{aligned} & 3 \\ & \begin{array}{l} \lambda \\ \frac{0}{1} \end{array} \end{aligned}$ | $\begin{aligned} & \overline{\boxed{0}} \\ & \stackrel{\rightharpoonup}{\circ} \end{aligned}$ | $\frac{\stackrel{7}{0}}{\underline{I}}$ | $\begin{aligned} & \underset{\sim}{\lambda} \\ & \stackrel{\pi}{\top} \end{aligned}$ | $\begin{aligned} & \bar{\Pi} \\ & \stackrel{0}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | ふ ¢ ¢ ¢ | 工 |
| 7：00 to 8：00 | 52 | 4 | 56 | 963 | 72 | 1035 | 200 | 11 | 211 | 0 | 0 | 0 | 55 | 3 | 58 | 24 | 2 | 26 | 19 | 4 | 23 | 0 | 0 | 0 |
| 7：15 to 8：15 | 70 | 3 | 73 | 1020 | 67 | 1087 | 238 | 9 | 247 | 0 | 0 | 0 | 66 | 7 | 73 | 44 | 2 | 46 | 23 | 5 | 28 | 0 | 0 | 0 |
| 7：30 to 8：30 | 107 | 4 | 111 | 1089 | 74 | 1163 | 285 | 10 | 295 | 0 | 0 | 0 | 83 | 9 | 92 | 62 | 3 | 65 | 27 | 5 | 32 | 0 | 0 | 0 |
| 7：45 to 8：45 | 132 | 7 | 139 | 1103 | 74 | 1177 | 301 | 12 | 313 | 0 | 0 | 0 | 87 | 9 | 96 | 89 | 5 | 94 | 36 | 5 | 41 | 0 | 0 | 0 |
| 8：00 to 9：00 | 151 | 8 | 159 | 1117 | 82 | 1199 | 310 | 12 | 322 | 0 | 0 | 0 | 96 | 11 | 107 | 107 | 4 | 111 | 37 | 5 | 42 | 0 | 0 | 0 |
| AM Totals | 203 | 12 | 215 | 2080 | 154 | 2234 | 510 | 23 | 533 | 0 | 0 | 0 | 151 | 14 | 165 | 131 | 6 | 137 | 56 | 9 | 65 | 0 | 0 | 0 |
| 16：00 to 17：00 | 129 | 5 | 134 | 1130 | 41 | 1171 | 273 | 2 | 275 | 0 | 0 | 0 | 337 | 4 | 341 | 165 | 0 | 165 | 90 | 1 | 91 | 1 | 0 | 1 |
| 16：15 to 17：15 | 136 | 5 | 141 | 1171 | 33 | 1204 | 266 | 1 | 267 | 0 | 0 | 0 | 335 | 4 | 339 | 176 | 0 | 176 | 105 | 2 | 107 | 1 | 0 | 1 |
| 16：30 to 17：30 | 125 | 3 | 128 | 1171 | 37 | 1208 | 245 | 1 | 246 | 0 | 0 | 0 | 314 | 3 | 317 | 166 | 0 | 166 | 94 | 1 | 95 | 1 | 0 | 1 |
| 16：45 to 17：45 | 127 | 2 | 129 | 1108 | 31 | 1139 | 241 | 2 | 243 | 0 | 0 | 0 | 327 | 1 | 328 | 171 | 0 | 171 | 105 | 1 | 106 | 1 | 0 | 1 |
| 17：00 to 18：00 | 112 | 1 | 113 | 1129 | 34 | 1163 | 227 | 2 | 229 | 0 | 0 | 0 | 291 | 1 | 292 | 141 | 0 | 141 | 92 | 1 | 93 | 0 | 0 | 0 |
| PM Totals | 241 | 6 | 247 | 2259 | 75 | 2334 | 500 | 4 | 504 | 0 | 0 | 0 | 628 | 5 | 633 | 306 | 0 | 306 | 182 | 2 | 184 | 1 | 0 | 1 |


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| Job No. | : N790 |
| :--- | :--- |
| Client | : Realty Realizations |
| Suburb | $:$ Nowra |
| Location | $:$ 1. Culburra Rd / Coonamia Rd |
|  |  |
| Day/Date | : Sat, 5th May 2012 |
| Weather | : Fine |
| Description | :Classified Intersection Count |
|  | $:$ Hourly Summary |



SKYHIGH - THE TRAFFIC SURVEY COMPANY




| Approach | Gulburra Rd |  |  |  |  |  |  |  |  | Mayfield Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 8 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 9 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | Direction 9U （U Turn） |  |  | $\begin{gathered} \hline \begin{array}{c} \text { Direction } 10 \\ \text { (Left Turn) } \end{array} \\ \hline \end{gathered}$ |  |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{array}{\|c\|} \hline \begin{array}{c} \text { Direction } 120 \\ \text { (U Turn) } \end{array} \\ \hline \end{array}$ |  |  |
| Time Period | 河 | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { di } \\ \hline \end{array}$ | $\begin{gathered} \overline{\mathrm{I}} \\ \stackrel{y}{\circ} \end{gathered}$ | $\begin{array}{\|c} \hline \mathbf{5} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\text { 㐅⿳亠丷厂犬土}} \\ \text { n } \end{array}$ | $\begin{array}{\|l\|} \hline \mathrm{I} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 总 } \end{array}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { à } \end{array}$ | $\begin{array}{\|l\|} \hline \mathrm{\Xi} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\begin{aligned} & \overline{\mathrm{g}} \\ & \stackrel{y}{\circ} \end{aligned}$ |  | $\begin{array}{\|c} \stackrel{\rightharpoonup}{9} \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \hline \mathrm{O} \end{array}$ | $\begin{array}{\|c} \stackrel{\rightharpoonup}{\mathrm{F}} \\ \hline \end{array}$ |  | $\stackrel{\text { ¢ }}{\stackrel{\text { ¢ }}{\circ}}$ |
| 12：00 10 12：15 | 47 | 0 | 47 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 | 0 | 1 | 0 | 0 | 0 |
| $12: 15$ 10 $12: 30$ | 45 | 0 | 45 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 |
| $12: 30$ 10 $12: 45$ <br> 1   <br> 1   | 57 | 0 | 57 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 | 0 | 1 | 0 | 0 | 0 |
| 12：45 to 13：00 | 39 | 1 | 40 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 |
| 13：00 10－13：15 | 44 | 1 | 45 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 | 0 | 1 | 0 | 0 | 0 |
| 13：15 to 13：30 | 53 | 0 | 53 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 | 0 |
| $13: 30$ to $13: 45$ | 60 | 0 | 60 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 | 0 | 1 | 0 | 0 | 0 |
| 13：45 to 14：00 | 46 | 0 | 46 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 1 | 0 | 1 | 0 | 0 | 0 |
| Totals | 391 | 2 | 393 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 5 | 0 | 5 | 0 | 0 | 0 |



| Approach | Gulburra Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction |  | ectio <br> eft T |  |  | $\begin{aligned} & \text { rection } \\ & \text { hroua } \end{aligned}$ |  |  | Tur |  |  |
| Time Period | $\begin{array}{\|l\|} \hline \text { 觅 } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \substack{\text { İ } \\ \text { İ工 } \\ \hline} \\ \hline \end{array}$ |  | $\begin{array}{\|c} \hline \text { 总 } \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \substack{3 \\ \text { xim }} \\ \hline \end{array}$ | $\frac{\overline{\mathrm{a}}}{\underline{\mathrm{o}}}$ | 营 |  | $\begin{aligned} & \stackrel{\overline{\mathrm{o}}}{\stackrel{1}{\circ}} \\ & \hline \end{aligned}$ |  |
| 12：00 to 13：00 | 2 | 0 | 2 | 209 | 1 | 210 | 0 | 0 | 0 |  |
| 12：15 to 13：15 | 2 | 0 | 2 | 204 | 0 | 204 | 0 | 0 | 0 |  |
| 12：30 to 13：30 | 2 | 0 | 2 | 174 | 0 | 174 | 0 | 0 | 0 |  |
| 12：45 to 13：45 | 1 | 0 | 1 | 139 | 1 | 140 | 0 | 0 | 0 |  |
| $13: 00$ to $14: 00$ | 1 | 0 | 1 | 139 | 1 | 140 | 0 | 0 | 0 |  |
| Totals | 3 | 0 | 3 | 348 | 2 | 350 | 0 | 0 | 0 |  |


| Approach | Gulburra Rd |  |  |  |  |  |  |  |  | Mayfield Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | $\begin{aligned} & \begin{array}{l} \text { Direction } 8 \\ \text { (Through) } \end{array} \end{aligned}$ |  |  | $\begin{gathered} \text { Direction } 9 \\ \text { (Right Turn) } \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \text { Direction } 10 \\ \text { (Left Turn) } \end{gathered}$ |  |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{gathered} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{array}{\|l\|} \hline \text { 吡 } \end{array}$ |  | $\begin{array}{\|l\|l\|} \hline \mathrm{I} \\ \hline 1 \end{array}$ | $\begin{array}{\|l\|} \hline \frac{5}{9} \\ \hline \end{array}$ | $\begin{aligned} & \text { 言 } \\ & \text { İ } \\ & \hline \end{aligned}$ |  | 皆 |  | $\begin{array}{\|l\|l\|} \hline \frac{\mathrm{⿺⿻}}{\mathrm{o}} \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 学 } \end{array}$ |  | $\begin{array}{\|l\|l\|} \hline \frac{\overline{\mathrm{g}}}{\mathrm{D}} \end{array}$ |  | $\begin{array}{\|l\|l\|} \hline \stackrel{\rightharpoonup}{9} \\ \hline \end{array}$ | $\begin{aligned} & \text { 䧺 } \\ & \text { In } \end{aligned}$ | $\begin{array}{\|l\|} \hline \mathrm{I} \\ \hline \mathrm{O} \end{array}$ | $\begin{array}{\|l\|l\|} \hline \text { 喜 } \\ \hline \end{array}$ | $\begin{aligned} & \text { 訔 } \\ & \text { İ } \end{aligned}$ | － |
| 12：00 to 13：00 | 188 | 1 | 189 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |  | 2 | 0 | 2 | 0 | 0 | 0 |
| 12：15 to 13：15 | 185 | 2 | 187 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 2 | 0 | 2 | 0 | 0 | 0 |
| 12：30 to 13：30 | 193 | 2 | 195 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 2 | 0 | 2 | 0 | 0 | 0 |
| 12：45 to $13: 45$ | 196 | 2 | 198 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 2 | 0 | 2 | 0 | 0 | 0 |
| 13：00 to 14：00 | 203 | 1 | 204 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 3 | 0 | 3 | 0 | 0 | 0 |
| Totals | 391 | 2 | 393 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 |  | 5 | 0 | 5 | 0 | 0 | 0 |











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| Job No. | : N790 |
| :--- | :--- |
| Client | : Realty Realizations |
| Suburb | : Nowra |
| Location | $: 5$. Greenwell Point Rd / Mayfield Rd |
|  |  |
| Day/Date | : Sat, 5th May 2012 |
| Weather | : Fine |
| Description | : Classified Intersection Count |
|  | : Hourly Summary |



Mayfield Rd




|  |  |  | Millbank Rd |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Job No． | ：N790 |  | $94.98{ }^{8}{ }^{7}$ |  |  |
| Client | ：Realty Realizations | ¢ |  | 할 |  |
| Suburb | ：Nowra | ！ |  | ！ |  |
| Location | ：6．Greenwell Point Rd／Millbank Rd | $\frac{\bar{\circ}}{\stackrel{\circ}{0}}$ |  | $\begin{aligned} & \text { ò } \\ & \hline \overline{\mathrm{o}} \end{aligned}$ |  |
| Day／Date | ：Sat，5th May 2012 | 気 | 気 ${ }^{\text {c }}$ | ${ }_{0}$ | SKYHIGH－THE TRAFFIC SURUEY COMPANY |
| Weather | ：Fine | － |  | O |  |
| Description | ：Classified Intersection Count |  | $\left\|\begin{array}{llll}1 & 2 & 3 & 3 U\end{array}\right\|$ |  |  |
|  | ： 15 mins Data |  | Millbank Rd |  |  |


| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7（Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 9 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | Direction 9U （U Turn） |  |  | $\begin{gathered} \hline \begin{array}{c} \text { Direction } 10 \\ \text { (Left Turn) } \end{array} \\ \hline \end{gathered}$ |  |  | Direction 11（Through） |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{gathered} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{array}{\|l\|l} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \frac{\mathrm{x}}{\mathrm{⿺}} \\ \hline \end{array}$ | 苛 |  | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \stackrel{1}{2} \end{array}$ | $\begin{aligned} & \stackrel{\rightharpoonup}{5} \\ & \hline \mathrm{~J} \end{aligned}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\text { 㐅⿳亠丷厂犬土}} \\ \text { n } \end{array}$ | $\begin{array}{\|l\|} \hline \mathrm{I} \\ \hline 1 \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 总 } \end{array}$ |  | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \stackrel{1}{2} \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \frac{\mathrm{⿺⿻} 丷 冖 ⿱ 丶 万 ⿱ ⿰ ㇒ 一 乂 。 ~}{2} \end{array}$ | $\begin{aligned} & \text { 蒿 } \end{aligned}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { di } \end{array}$ | $\stackrel{\overline{\mathrm{I}}}{\stackrel{\mathrm{I}}{\circ}}$ | $\begin{array}{\|c} \stackrel{5}{9} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { an } \end{array}$ | $\begin{array}{\|l\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心.} \end{array}$ | 苛 |  | $\stackrel{\text { ¢ }}{\text { ¢ }}$ |
| 12：00 to 12：15 | 0 | 0 | 0 | 5 | 2 | 7 | 5 | 0 | 5 | 0 | 0 | 0 | 4 | 0 | 4 | 55 | 0 | 55 | 14 | 0 | 14 | 0 | 0 | 0 |
| $\begin{array}{lllll}12: 15 & \text { to } & 12: 30\end{array}$ | 2 | 0 | 2 | 2 | 0 | 2 | 4 | 0 | 4 | 0 | 0 | 0 | 1 | 0 | 1 | 52 | 0 | 52 | 7 | 0 | 7 | 0 | 0 | 0 |
| 12：30 to 12：45 | 0 | 0 | 0 | 3 | 0 | 3 | 2 | 0 | 2 | 0 | 0 | 0 | 3 | 1 | 4 | 47 | 2 | 49 | 8 | 0 | 8 | 0 | 0 | 0 |
| 12：45 to 13：00 | 3 | 1 | 4 | 6 | 0 | 6 | 1 | 0 | 1 | 0 | 0 | 0 | 7 | 0 | 7 | 36 | 0 | 36 | 12 | 0 | 12 | 0 | 0 | 0 |
| 13：00 to 13：15 | 1 | 0 | 1 | 2 | 0 | 2 | 2 | 0 | 2 | 0 | 0 | 0 | 3 | 0 | 3 | 42 | 1 | ${ }^{43}$ | 9 | 0 | 9 | 0 | 0 | 0 |
| 13：15 to 13：30 | 2 | 0 | 2 | 6 | 0 | 6 | 3 | 0 | 3 | 0 | 0 | 0 | 7 | 0 | 7 | 47 | 1 | 48 | 10 | 0 | 10 | 0 | 0 | 0 |
| 13：30 to 13：45 | 1 | 0 | 1 | 2 | 0 | 2 | 2 | 0 | 2 | 0 | 0 | 0 | 8 | 0 | 8 | 57 | 2 | 59 | 11 | 0 | 11 | 0 | 0 | 0 |
| 13：45 to 14：00 | 1 | 0 | 1 | 3 | 0 | 3 | 3 | 0 | 3 | 0 | 0 | 0 | 4 | 0 | 4 | 49 | 1 | 50 | 11 | 0 | 11 | 0 | 0 | 0 |
| Totals | 10 | 1 | 11 | 29 | 2 | 31 | 22 | 0 | 22 | 0 | 0 | 0 | 37 | 1 | 38 | 385 | 7 | 392 | 82 | 0 | 82 | 0 | 0 | 0 |


|  |  |  |  | Millbank Rd |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Job No． | ：N790 |  |  |  |  |
| Client | ：Realty Realizations | 『 | ${ }^{\mathrm{D}}$－ | $\uparrow \downarrow \downarrow$ ¢ | \％ |
| Suburb | ：Nowra | 号 | 家 | $\overrightarrow{+}$ | 䓂 |
| Location | ：6．Greenwell Point Rd／Millbank Rd | $\stackrel{\text { 잉 }}{ }$ | $\pm$ | $\uparrow$ | $\stackrel{\circ}{\circ}$ |
|  |  | $\stackrel{0}{3}$ | $\stackrel{\rightharpoonup}{\sim}$ | $\stackrel{+}{\square}$ | $\stackrel{0}{3}$ |
| Day／Date | Sat，5th May 2012 | む̀ | $\stackrel{\rightharpoonup}{\sim}$ | $\checkmark$ | む |
| Weather | Fine | － |  | $\dot{\nu}_{B}$ | 0 |
| Description | ：Classified Intersection Count |  |  | $1{ }_{1}^{1} 23300$ |  |
|  | Hourly Summary |  |  | Millbank Rd |  |


| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 （Through） |  |  | $\begin{gathered} \text { Direction } 3 \text { 3 } \\ \text { (Right Turn) } \end{gathered}$ |  |  | $\begin{gathered} \text { Direction 3U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 6 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 6U } \\ \text { (U Turn) } \end{gathered}$ |  |  |
| Time Period | 总 |  | $\begin{array}{\|l\|} \hline \stackrel{\text { ® }}{\mathrm{o}} \\ \hline \end{array}$ | 营 | $\begin{aligned} & 3 \\ & \begin{array}{l} \text { In } \\ \text { In } \\ \hline \end{array} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 厄⿳一巛口口口阝 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l} \hline \text { 寽 } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { In } \\ \hline \end{array} \\ \hline \end{array}$ |  | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 3 } \\ & \text { I⿷匚⿳ } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 厄⿳一巛口口口阝 } \\ & \hline \end{aligned}$ | 总 | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { In } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \stackrel{\text { 玉̈ }}{\circ} \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 喜 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline 3 \\ \text { 㙜 } \\ \hline \end{array}$ | $\begin{aligned} & \overline{\mathrm{o}} \\ & \stackrel{\rightharpoonup}{\mathrm{o}} \end{aligned}$ | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { In } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{array}{\|} \stackrel{\text { ® }}{\stackrel{\circ}{\circ}} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline 5 \\ \hline \end{array}$ |  | － |
| 12：00 to 13：00 | 58 | 0 | 58 | 21 | 1 | 22 | 28 | 1 | 29 | 0 | 0 | 0 | 14 | 1 | 15 | 181 | 2 | 183 | 12 | 0 | 12 | 0 | 0 | 0 |
| 12：15 to 13：15 | 45 | 0 | 45 | 21 | 1 | 22 | 28 | 1 | 29 | 0 | 0 | 0 | 15 | 1 | 16 | 177 | 0 | 177 | 13 | 0 | 13 | 0 | 0 | 0 |
| 12：30 10 13：30 | 41 | 0 | 41 | 27 | 1 | 28 | ${ }^{30}$ | 1 | 31 | 0 | 0 | 0 | 15 | 1 | 16 | 155 | 1 | 156 | 6 | 0 | 6 | 0 | 0 | 0 |
| 12：45 to $13: 45$ | 35 | 0 | 35 | 28 | 2 | 30 | 25 | 1 | 26 | 0 | 0 | 0 | 16 | 0 | 16 | 130 | 2 | 132 | 7 | 1 | 8 | 0 | 0 | 0 |
| 13：00 to 14：00 | 31 | 1 | 32 | 28 | 1 | 29 | 25 | 1 | 26 | 0 | 0 | 0 | 15 | 14 | 29 | 139 | 2 | 141 | 7 | 1 | 8 | 0 | 0 | 0 |
| Totals | 89 | 1 | 90 | 49 | 2 | 51 | 53 | 2 | 55 | 0 | 0 | 0 | 29 | 15 | 44 | 320 | 4 | 324 | 19 | 1 | 20 | 0 | 0 | 0 |


| Approach | Millbank Rd |  |  |  |  |  |  |  |  |  |  |  | Greenwell Point Rd |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 <br> （Left Turn） |  |  | Direction 8 （Through） |  |  | Direction 9 （Right Turn） |  |  | Direction 9U （ U Turn） |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） |  |  | Direction 12 U （U Turn） |  |  |
| Time Period | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 入 } \\ & \text { 崖 } \end{aligned}$ |  | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ |  |  | 喜 |  |  | 总 |  |  | $\begin{array}{\|l\|l} \hline \text { 总 } \\ \hline \end{array}$ |  | $\begin{aligned} & \stackrel{\overline{\mathrm{g}}}{\stackrel{\circ}{\circ}} \end{aligned}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ |  | $\stackrel{\text { 항 }}{ }$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ |  |  | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 各 } \\ & \text { I⿷匚⿳ } \end{aligned}$ |  |
| 12：00 to 13：00 | 5 | 1 | 6 | 16 | 2 | 8 | 12 | 0 | 12 | 0 | 0 | 0 | 15 | 1 | 16 | 190 | 2 | 192 | 41 | 0 | 41 | 0 | 0 | 0 |
| 12：15 to 13：15 | 6 | 1 | 7 | 13 | 0 | 13 | 9 | 0 | 9 | 0 | 0 | 0 | 14 | 1 | 15 | 177 | 3 | 180 | 36 | 0 | 36 | 0 | 0 | 0 |
| 12：30 to 13：30 | 6 | 1 | 7 | 17 | 0 | 17 | 8 | 0 | 8 | 0 | 0 | 0 | 20 | 1 | 21 | 172 | 4 | 176 | 39 | 0 | 39 | 0 | 0 | 0 |
| 12：45 to 13：45 | 7 | 1 | 8 | 16 | 0 | 16 | 8 | 0 | 8 | 0 | 0 | 0 | 25 | 0 | 25 | 182 | 4 | 186 | 42 | 0 | 42 | 0 | 0 | 0 |
| 13：00 to 14：00 | 5 | 0 | 5 | 13 | 0 | 13 | 10 | 0 | 10 | 0 | 0 | 0 | 22 | 0 | 22 | 195 | 5 | 200 | 41 | 0 | 41 | 0 | 0 | 0 |
| Totals | 10 | 1 | 11 | 29 | 2 | 31 | 22 | 0 | 22 | 0 | 0 | 0 | 37 | 1 | 38 | 385 | 7 | 392 | 82 | 0 | 82 | 0 | 0 | 0 |



|  |  |  | Princes Hwy |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Job No． | ：N790 |  |  |  |  |
| Client | ：Realty Realizations |  | $\mathrm{D}_{\text {¢ }}$ ¢ $\downarrow \downarrow \downarrow \square$ |  |  |
| Suburb | ：Nowra | $\stackrel{\square}{\omega}$ | $\overrightarrow{\overrightarrow{0} \uparrow} \xrightarrow{+1}$ | あ | 7 |
| Location | ：7．Kalandar St／Princes Hwy | 䟴 | $\stackrel{\text { ！}}{\substack{\text { ¢ }}}$ | 年 | 2） |
| Day／Date | ：Sat，5th May 2012 | $\stackrel{\text { ¢ }}{\text { ¢ }}$ | $\overrightarrow{\text { co }}$ | $\stackrel{\text { ¢ }}{\text { ¢ }}$ | SKYHIGH－THE TRAFFIC SURUEY COMPANY |
| Weather | ：Fine |  | $\dot{\dot{*}}_{\text {B }}$ |  |  |
| Description | ：Classified Intersection Count |  | A $\left\|\begin{array}{llll}1 & 2 & 3 & 3 U\end{array}\right\|$ |  |  |
|  | ： 15 mins Data |  | Princes Hwy |  |  |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | $\begin{aligned} & \text { Direction } 88 \\ & \text { (Through) } \end{aligned}$ |  |  | $\begin{gathered} \begin{array}{c} \text { Direction } 9 \\ \text { (Right Turn) } \end{array} \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction } 10 \\ \text { (Left Turn) } \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction } 11 \\ \text { (Through) } \\ \hline \end{gathered}$ |  |  | Direction 12 （Right Turn） <br> （Right Turn） |  |  | $\begin{array}{\|c\|} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{array}$ |  |  |
| Time Period | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \begin{array}{c} \text { Iam } \\ \text { In } \end{array} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \hline \end{array}$ | $\begin{array}{\|l} \hline \frac{5}{9} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{c} \text { àd } \\ \text { an } \\ \hline \end{array} \\ \hline \end{array}$ | $\frac{\text { 镸 }}{}$ | $\begin{array}{\|l} \hline \mathrm{F} \\ \hline \mathrm{I} \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} \text { Ia } \\ \text { an } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{\sigma}} \\ \hline \end{array}$ | $\begin{array}{\|c} \hline \stackrel{\text { F }}{\mathrm{I}} \\ \hline \end{array}$ |  | $\frac{\text { 惑 }}{}$ | $\begin{array}{\|l\|l} \hline \frac{\mathrm{F}}{9} \end{array}$ | $\begin{array}{\|c} \substack{\stackrel{\rightharpoonup}{I} \\ \stackrel{\rightharpoonup}{I} \\ \hline} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \hline \end{array}$ | $\begin{aligned} & \text { 暿 } \\ & \hline \end{aligned}$ |  | $\frac{\text { 硕 }}{}$ | $\begin{array}{\|l} \hline \frac{5}{9} \\ \hline \end{array}$ |  |  | $\begin{array}{\|c} \stackrel{5}{5} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { dix } \\ \hline \end{array}$ | 产 |
| 12：00 to 12：15 | 98 | 0 | 98 | 253 | 2 | 255 | 12 | 2 | 14 | 0 | 0 | 0 | 10 | 0 | 10 | 40 | 0 | 40 | ${ }^{35}$ | 0 | 35 | 0 | 0 | 0 |
| 12：15 to 12：30 | 95 | 1 | 96 | 298 | 3 | 301 | 10 | 2 | 12 | 0 | 0 | 0 | 20 | 0 | 20 | 19 | 0 | 19 | 24 | 0 | 24 | 0 | 0 | 0 |
| 12：30 to 12：45 | 97 | 0 | 97 | 314 | 4 | 318 | 17 | 4 | 21 | 0 | 0 | 0 | 10 | 1 | 11 | 34 | 0 | 34 | 39 | 1 | 40 | 0 | 0 | 0 |
| 12：45 to 13：00 | 86 | 0 | 86 | 240 | 2 | 242 | 22 | 0 | 22 | 0 | 0 | 0 | 9 | 0 | 9 | 35 | 0 | 35 | 32 | 0 | 32 | 0 | 0 | 0 |
| 13：00 to 13：15 | ${ }^{112}$ | 0 | 112 | 231 | 2 | 233 | 24 | 1 | 25 | 0 | 0 | 0 | 6 | 0 | 6 | 42 | 0 | 42 | 26 | 0 | 26 | 0 | 0 | 0 |
| 13：15 to 13：30 | 79 | 0 | 79 | 275 | 8 | 283 | 27 | 0 | 27 | 0 | 0 | 0 | 7 | 1 | 8 | 34 | 0 | 34 | 36 | 2 | 38 | 0 | 0 | 0 |
| 13：30 to 13：45 | 94 | 1 | 95 | 253 | 3 | 256 | 11 | 0 | 11 | 0 | 0 | 0 | 10 | 0 | 10 | 25 | 1 | 26 | 27 | 0 | 27 | 0 | 0 | 0 |
| 13：45 to 14：00 | 84 | 2 | 86 | 271 | 2 | 273 | 17 | 0 | 17 | 0 | 0 | 0 | 7 | 1 | 8 | 21 | 0 | 21 | 29 | 1 | 30 | 0 | 0 | 0 |
| Totals | 745 | 4 | 749 | 2135 | 26 | 2161 | 140 | 9 | 149 | 0 | 0 | 0 | 79 | 3 | 82 | 250 | 1 | 251 | 248 | 4 | 252 | 0 | 0 | 0 |


|  |  |  |  | Princes Hwy |
| :---: | :---: | :---: | :---: | :---: |
| Job No． | ：N790 |  |  | 9U $9.9 .8 .{ }^{8}$ |
| Client | ：Realty Realizations |  | 今 | $\uparrow \downarrow \downarrow$ |
| Suburb | ：Nowra | あ | 䎡 |  |
| Location | ：7．Kalandar St／Princes Hwy | \％ | $\stackrel{+}{\square}$ | 4 ¢ |
|  |  |  | $\vec{\sim}$ | ＋${ }^{+}$ |
| Day／Date | ：Sat，5th May 2012 | $\stackrel{\text { ® }}{ }$ | 式 | $\downarrow$ |
| Weather | ：Fine |  |  |  |
| Description | ：Classified Intersection Count |  |  | $\left\|\begin{array}{llll}1 & 2 & 3 & 3 U\end{array}\right\|$ |
|  | Hourly Summary |  |  | Princes Hw |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 （Left Turn） |  |  | Direction 2 （Through） |  |  | $\begin{gathered} \begin{array}{c} \text { Direction } 3 \\ \text { (Right Turn) } \end{array} \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 3U } \\ \text { (U Turn) } \end{gathered}$ |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | Direction 6（Right Turn） |  |  | $\begin{gathered} \hline \text { Direction 6U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { 玉̈x } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \text { 등 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \frac{5}{9} \\ \hline \end{array}$ |  | $\begin{gathered} \stackrel{\text { ® }}{\stackrel{\circ}{6}} \\ \hline \end{gathered}$ | $\begin{array}{\|l} \hline \frac{\mathrm{F}}{9} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \text { B } \\ \text { 玉्工 } \\ \hline \end{array}$ | $\begin{aligned} & \text { 응 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l} \hline \text { E } \\ \hline \end{array}$ |  | $\begin{aligned} & \overline{\mathrm{I}} \\ & \hline 1 \end{aligned}$ | $\begin{array}{\|l} \hline \frac{5}{9} \\ \hline \end{array}$ | $$ |  | 总 | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { 区 } \\ \hline \end{array} \\ \hline \end{array}$ |  | $\begin{array}{\|l\|l\|} \hline \text { 亳 } \\ \hline \end{array}$ |  | $\begin{array}{\|l} \stackrel{\text { ® }}{\stackrel{1}{6}} \\ \hline \end{array}$ | 总 | $\begin{array}{r} \text { 3 } \\ \text { 圌 } \\ \hline \end{array}$ | － |
| 12：00 to 13：00 | 3 | 1 | 4 | 753 | 20 | 773 | 84 | 4 | 88 | 0 | 0 | 0 | 68 | 0 | 68 | 159 | 1 | 160 | 409 | 4 | 413 | 0 | 0 | 0 |
| 12：15 to 13：15 | 6 | 1 | 7 | 724 | 16 | 740 | 92 | 3 | 95 | 0 | 0 | 0 | 64 | 0 | 64 | 134 | 1 | 135 | 377 | 3 | 380 | 0 | 0 | 0 |
| 12：30 to 13：30 | 4 | 0 | 4 | 699 | 12 | 711 | 95 | 2 | 97 | 0 | 0 | 0 | 61 | 0 | 61 | 122 | 1 | 123 | 332 | 3 | 335 | 0 | 0 | 0 |
| 12：45 to 13：45 | 7 | 0 | 7 | 695 | 9 | 704 | 87 | 1 | 88 | 0 | 0 | 0 | 57 | 0 | 57 | 129 | 1 | 130 | 341 | 1 | 342 | 0 | 0 | 0 |
| 13：00 to 14：00 | 7 | 0 | 7 | 681 | 7 | 688 | 73 | 0 | 73 | 0 | 0 | 0 | 53 | 0 | 53 | 113 | 2 | 115 | 326 | 0 | 326 | 0 | 0 | 0 |
| Totals | 10 | 1 | 11 | 1434 | 27 | 1461 | 157 | 4 | 161 | 0 | 0 | 0 | 121 | 0 | 121 | 272 | 3 | 275 | 735 | 4 | 739 | － | 0 | 0 |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{gathered} \begin{array}{c} \text { Direction } 9 \\ \text { (Right Turn) } \end{array} \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | Direction 10（Left Turn） （Left Turn） |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） <br> （Right Turn） |  |  | $\begin{array}{\|c\|} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{array}$ |  |  |
| Time Period | $\begin{array}{\|l\|} \hline \text { 䜳 } \\ \hline \end{array}$ |  | $\stackrel{\text { 픙 }}{ }$ | $\begin{aligned} & \text { 吡 } \end{aligned}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { as } \\ \hline \end{array} \\ \hline \end{array}$ |  | $\begin{array}{\|l} \hline \text { 寽 } \\ \hline \end{array}$ |  | $\stackrel{\text { 高 }}{\circ}$ | $\begin{aligned} & \text { 畐 } \\ & \hline \end{aligned}$ | $$ | $\begin{array}{\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心.} \end{array}$ | $\begin{array}{\|l\|l\|} \hline \text { 吡 } \\ \hline \end{array}$ |  |  | $\begin{array}{\|l\|l\|} \hline \text { 吡 } \end{array}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { am } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \frac{\overline{\mathrm{g}}}{\mathrm{i}} \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|l} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \frac{\overline{\mathrm{x}}}{\mathrm{i}} \\ \hline \end{array}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { im } \\ \hline \end{array} \\ \hline \end{array}$ | $\stackrel{\text { ¢ }}{\text { ¢ }}$ |
| 12：00 to 13：00 | 376 | 1 | 377 | 1105 | 11 | 1116 | 61 | 8 | 69 | 0 | 0 | 0 | 49 | 1 | 50 | 128 | 0 | 128 | 130 | 1 | 131 | 0 | 0 | 0 |
| 12：15 to 13：15 | 390 | 1 | 391 | 1083 | 11 | 1094 | 73 | 7 | 80 | 0 | 0 | 0 | 45 | 1 | 46 | 130 | 0 | 130 | 121 | 1 | 122 | 0 | 0 | 0 |
| $\begin{array}{llll}12: 30 & 10 & 13: 30\end{array}$ | 374 | 0 | 374 | 1060 | 16 | 1076 | 90 | 5 | 95 | 0 | 0 | 0 | 32 | 2 | 34 | 145 | 0 | 145 | 133 | 3 | 136 | 0 | 0 | 0 |
| 12：45 to 13：45 | 371 | 1 | 372 | 999 | 15 | 1014 | 84 | 1 | 85 | 0 | 0 | 0 | 32 | 1 | 33 | 136 | 1 | 137 | 121 | 2 | 123 | 0 | 0 | 0 |
| 13：00 to 14：00 | 369 | 3 | 372 | 1030 | 15 | 1045 | 79 | 1 | 80 | 0 | 0 | 0 | 30 | 2 | 32 | 122 | 1 | 123 | 118 | 3 | 121 | 0 | 0 | 0 |
| Totals | 745 | 4 | 749 | 2135 | 26 | 2161 | 140 | 9 | 149 | 0 | 0 | 0 | 79 | 3 | 82 | 250 | 1 | 251 | 248 | 4 | 252 | 0 | 0 | 0 |






| Approach |  | Currarong Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction |  |  | Direction 5 (Through) |  |  | $\begin{gathered} \text { Direction } 6 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 6U } \\ \text { (U Turn) } \end{gathered}$ |  |  |
| Time Period |  |  | $\begin{aligned} & \text { 镸 } \\ & \hline \end{aligned}$ |  | $\begin{array}{\|l\|} \hline \stackrel{\text { Ï }}{\circ} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \text { 喜 } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { In } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \overline{\mathrm{\circ}} \mathrm{i} \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|l} \hline 5 \\ \hline \end{array}$ |  | ¢ |
| $12: 00$ 10 $13: 00$ |  |  | 8 | 0 | 8 | 20 | 0 | 20 | 0 | 0 | 0 |
| $12: 15$ to $13: 15$ <br> 12 lo  |  |  | 9 | 0 | 9 | 18 | 0 | 18 | 0 | 0 | 0 |
| $\begin{array}{llll}12: 30 & 10 & 13: 30\end{array}$ |  |  | 7 | 0 | 7 | 17 | 0 | 17 | 0 | 0 | 0 |
| 12:45 to $13: 45$ |  |  | 6 | 0 | 6 | 19 | 0 | 19 | 0 | 0 | 0 |
|  |  |  | 8 | 0 | 8 | 20 | 0 | 20 | 0 | 0 | 0 |
| Totals |  |  | 16 | 0 | 16 | 40 | 0 | 40 | 0 | 0 | 0 |





| Approach | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 9 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (UTurn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \begin{array}{c} \text { Direction } 10 \\ \text { (Left Turn) } \end{array} \\ \hline \end{gathered}$ |  |  | Direction 11 （Through） |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{gathered} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{array}{\|c} \hline \stackrel{\rightharpoonup}{\Xi} \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \mathrm{y} \\ \hline 1 \end{array}$ | 苛 | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { dis } \end{array}$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \stackrel{1}{2} \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 吡 } \end{array}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\text { 㐅⿳亠丷厂犬土}} \\ \text { n } \end{array}$ | $\begin{array}{\|l\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心} \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 总 } \end{array}$ |  | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \stackrel{1}{2} \end{array}$ | $\begin{array}{\|c} \hline \stackrel{\rightharpoonup}{\Xi} \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \bar{\circ} \mathrm{I} \\ \hline \end{array}$ |  | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { di } \end{array}$ | $\stackrel{\overline{\mathrm{I}}}{\stackrel{\mathrm{I}}{\circ}}$ | $\begin{array}{\|c} \stackrel{5}{9} \\ \hline \end{array}$ | $\begin{aligned} & \text { ? } \\ & \text { Ï } \\ & \text { In } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心.} \end{array}$ |  |  | － |
| 12：00 to 12：15 | 32 | 1 | 33 | 3 | 0 | 3 | 21 | 1 | 22 | 1 | 0 | 1 | 24 | 0 | 24 | 41 | 0 | 41 | 2 | 1 | 3 | 1 | 0 | 1 |
| $\begin{array}{lllll}12: 15 & \text { to } & 12: 30\end{array}$ | 28 | 0 | 28 | 4 | 0 | 4 | 27 | 0 | 27 | 0 | 0 | 0 | 28 | 1 | 29 | 34 | 0 | 34 | 3 | 0 | 3 | 0 | 0 | 0 |
| 12：30 to 12：45 | 27 | 0 | 27 | 6 | 0 | 6 | 29 | 2 | 31 | 1 | 1 | 2 | 32 | 0 | 32 | 45 | 1 | 46 | 5 | 0 | 5 | 1 | 1 | 2 |
| 12：45 to 13：00 | 31 | 0 | 31 | 3 | 0 | 3 | 35 | 0 | 35 | 0 | 0 | 0 | 36 | 1 | 37 | 41 | 0 | 41 | 3 | 1 | 4 | 0 | 0 | 0 |
| 13：00 to 13：15 | 27 | 0 | 27 | 2 | 0 | 2 | 26 | 1 | 27 | 2 | 0 | 2 | 27 | 1 | 28 | 40 | 1 | 41 | 5 | 0 | 5 | 2 | 0 | 2 |
| 13：15 to 13：30 | 26 | 0 | 26 | 5 | 0 | 5 | 28 | 2 | 30 | 0 | 1 | 1 | 26 | 0 | 26 | ${ }^{33}$ | 1 | 34 | 4 | 0 | 4 | 0 | 1 | 1 |
| 13：30 to 13：45 | 26 | 0 | 26 | 3 | 0 | 3 | 33 | 0 | 33 | 2 | 0 | 2 | 25 | 1 | 26 | 29 | 1 | 30 | 3 | 0 | 3 | 2 | 0 | 2 |
| 13：45 to 14：00 | 27 | 1 | 28 | 3 | 0 | 3 | 29 | 1 | 30 | 1 | 0 | 1 | 30 | 1 | 31 | 27 | 1 | 28 | 5 | 1 | 6 | 1 | 0 | 1 |
| Totals | 224 | 2 | 226 | 29 | 0 | 29 | 228 | 7 | 235 | 7 | 2 | 9 | 228 | 5 | 233 | 290 | 5 | 295 | 30 | 3 | 33 | 7 | 2 | 9 |



| Approach | Kinghorne St |  |  |  |  |  |  |  |  |  |  |  | Kalandar St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 9 \\ \text { (Right Turn) } \end{gathered}$ |  |  | Direction 9U （U Turn） |  |  | $\begin{gathered} \hline \text { Direction } 10 \\ \text { (Left Turn) } \\ \hline \end{gathered}$ |  |  | Direction 11 （Through） |  |  | Direction 12 （Right Turn） <br> （Right Turn） |  |  | $\begin{array}{\|c\|} \hline \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{array}$ |  |  |
| Time Period | $\begin{array}{\|l\|} \hline \text { 䜳 } \\ \hline \end{array}$ |  | $\stackrel{\text { 玉َ }}{\stackrel{\rightharpoonup}{\circ}}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { as } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \mathrm{y} \\ \hline 1 \end{array}$ | $\begin{array}{\|l} \hline \frac{5}{9} \\ \hline \end{array}$ | $$ | $\stackrel{\text { 高 }}{\circ}$ | $\begin{array}{\|l\|} \hline \text { 总 } \\ \hline \end{array}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { an } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心.} \end{array}$ | $\begin{array}{\|l} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\stackrel{\text { 흥 }}{ }$ | $\begin{array}{\|l\|l\|} \hline \text { 吡 } \end{array}$ | $\begin{array}{\|l} \hline \begin{array}{l} \text { I } \\ \text { am } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \frac{\overline{\mathrm{g}}}{\mathrm{i}} \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \text { 寽 } \\ \hline \end{array}$ |  | $\begin{array}{\|l\|} \hline \frac{\overline{\mathrm{x}}}{\mathrm{i}} \\ \hline \end{array}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $$ | 产 |
| 12：00 to 13：00 | 118 | 1 | 119 | 16 | 0 | 16 | 112 | 3 | 115 | 2 | 1 | 3 | 120 | 2 | 122 | 161 | 1 | 162 | 13 | 2 | 15 | 2 | 1 | 3 |
| 12：15 to 13：15 | 113 | 0 | 113 | 15 | 0 | 15 | 117 | 3 | 120 | 3 | 1 | 4 | 123 | 3 | 126 | 160 | 2 | 162 | 16 | 1 | 17 | 3 | 1 | 4 |
| 12：30 | 111 | 0 | 111 | 16 | 0 | 16 | 118 | 5 | 123 | 3 | 2 | 5 | 121 | 2 | 123 | 159 | 3 | 162 | 17 | 1 | 18 | 3 | 2 | 5 |
| 12：45 to 13：45 | 110 | 0 | 110 | 13 | 0 | 13 | 122 | 3 | 125 | 4 | 1 | 5 | 114 | 3 | 117 | 143 | 3 | 146 | 15 | 1 | 16 | 4 | 1 | 5 |
| 13：00 to 14：00 | 106 | 1 | 107 | 13 | 0 | 13 | 116 | 4 | 120 | 5 | 1 | 6 | 108 | 3 | 111 | 129 | 4 | 133 | 17 | 1 | 18 | 5 | 1 | 6 |
| Totals | 224 | 2 | 226 | 29 | 0 | 29 | 228 | 7 | 235 | 7 | 2 | 9 | 228 | 5 | 233 | 290 | 5 | 295 | 30 | 3 | 33 | 7 | 2 | 9 |





| Approach | Princes Hwy |  |  |  |  |  |  |  |  | Forest Rd |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 2 （Through） |  |  | Direction 3 （Right Turn） |  |  | Direction 3U （U Turn） |  |  | Direction 4 （Left Turn） |  |  |  | $\begin{gathered} \hline \text { Direction } 6 \\ \text { (Right Turn) } \end{gathered}$ |  |  | Direction 6U （U Turn） |  |  |
| Time Period | 营 | $\begin{array}{\|l\|l\|} \substack{3 \\ \text { In } \\ \hline} \\ \hline \end{array}$ | 坒 | 总 |  |  | 夁 |  | 家 | $\begin{aligned} & \text { 苛 } \\ & \hline \end{aligned}$ | $\begin{aligned} & \text { 㽞 } \\ & \underline{\text { n }} \end{aligned}$ |  | 宕 | 夁 |  | 年 | 苛 | $\begin{aligned} & \underline{\mathrm{ax}} \\ & \underline{1} \\ & \hline \end{aligned}$ | 产 |
| $\begin{array}{llll}12.00 & \text { to } & 13.00\end{array}$ | ${ }^{642}$ | 11 | 653 | 30 | 0 | \％ | 0 | 0 | － | 34 | 0 |  | 34 | 65 | 2 | 67 | 0 | 0 | 0 |
| 12.15 to 13.15 <br> 123   | 629 | 10 | 639 | 26 | 0 | 26 | 0 | 0 | － | 36 | 0 |  | 36 | 70 | 2 | 72 | 0 | 0 | 0 |
| $\begin{array}{lllll}1230 & \text { 10 } & 13.30\end{array}$ | 621 | 11 | 632 | 31 | 0 | 1 | 0 | 0 | － | 28 | 0 |  | 28 | 59 | 0 | 59 | 0 | 0 | 0 |
| 12.45 10 13：45 | 618 | 12 | 630 | 31 | 0 | 1 | 0 | 0 | 0 | 32 | 1 |  | 33 | 59 | 0 | 59 | 0 | 0 | 0 |
| 13.00 to 14.00 | 604 | 18 | 622 | 29 | 0 | 9 | 0 | 0 | 0 | 41 | 2 |  | 43 | 70 | 0 | 70 | 0 | 0 | 0 |
| Totals | 1246 | 29 | 1275 | 59 | 0 | 9 | 0 | 0 | 0 | 75 | 2 |  | 77 | 135 | 2 | 137 | 0 | 0 | 0 |




| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 （Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{array}{\|c} \hline \begin{array}{c} \text { Direction } 9 \\ \text { (Right Turn) } \end{array} \\ \hline \end{array}$ |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | Direction 10 （Left Turn） |  |  | Direction 11 （Through） |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{gathered} \text { Direction 12U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{array}{\|l\|l} \hline \frac{\mathrm{F}}{\mathrm{I}} \\ \hline \end{array}$ | $$ | $\begin{array}{\|l\|} \hline \mathrm{y} \\ \hline 1 \end{array}$ | $\begin{aligned} & \text { 总 } \end{aligned}$ |  | $\begin{gathered} \overline{\mathrm{g}} \mathrm{⿺} \\ \hline \end{gathered}$ | $\begin{array}{\|l} \hline \mathrm{F} \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \stackrel{\rightharpoonup}{\mathbf{a}} \\ \text { ix } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{g}} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \stackrel{\rightharpoonup}{\mathrm{I}} \\ \hline \end{array}$ |  | $\begin{aligned} & \overline{\mathrm{g}} \\ & \hline 1 \end{aligned}$ | $\begin{array}{\|l} \hline \frac{5}{9} \\ \hline \end{array}$ | $\begin{array}{\|l} \hline \stackrel{3}{3} \\ \text { id } \\ \hline \end{array}$ | $\begin{array}{\|l} \overline{\mathrm{g}} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|l} \hline \text { 坒 } \end{array}$ | $\begin{array}{\|l\|} \hline \stackrel{\rightharpoonup}{3} \\ \text { à } \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \overline{\mathrm{I}} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline \stackrel{\rightharpoonup}{\mathrm{g}} \\ \hline \end{array}$ | $\begin{aligned} & \text { ? } \\ & \text { İ } \\ & \text { İ } \\ & \hline \end{aligned}$ | $\begin{aligned} & \overline{\mathrm{g}} \\ & \hline \stackrel{y}{\circ} \end{aligned}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|l\|} \hline ⿳ 亠 㐅 \\ \text { İ } \\ \text { In } \\ \hline \end{array}$ | $\stackrel{\text { ¢ }}{\text { ¢ }}$ |
| 12：00 10－12：15 | ${ }^{26}$ | 2 | 28 | ${ }^{288}$ | 6 | 294 | 47 | 0 | 47 | 0 | 0 | 0 | 61 | 0 | 61 | 41 | 0 | 41 | 21 | 0 | 21 | 0 | 0 | 0 |
| 12：15 to 12：30 | 21 | 0 | 21 | 245 | 5 | 250 | 65 | 2 | 67 | 0 | 0 | 0 | 58 | 0 | 58 | 35 | 0 | 35 | 22 | 0 | 22 | 0 | 0 | 0 |
| 12：30 to 12：45 | 20 | 0 | 20 | 271 | 2 | 273 | 44 | 0 | 44 | 0 | 0 | 0 | 57 | 0 | 57 | 17 | 0 | 17 | ${ }^{15}$ | 1 | 16 | 0 | 0 | 0 |
| 12：45 to 13：00 | 18 | 0 | 18 | 229 | 3 | 232 | 38 | 0 | 38 | 0 | 0 | 0 | 60 | 0 | 60 | 24 | 0 | 24 | 19 | 0 | 19 | 0 | 0 | 0 |
| 13：00 to 13：15 | 27 | 0 | 27 | 234 | 2 | 236 | 58 | 1 | 59 | 0 | 0 | 0 | 73 | 0 | 73 | 34 | 0 | 34 | 23 | 1 | 24 | 0 | 0 | 0 |
| $13: 15$ to $13: 30$ | 17 | 0 | 17 | 271 | 6 | 277 | 54 | 0 | 54 | 0 | 0 | 0 | 57 | 0 | 57 | 18 | 0 | 18 | 13 | 2 | 15 | 0 | 0 | 0 |
| 13：30 to 13：45 | 25 | 0 | 25 | 277 | 4 | 281 | 37 | 0 | 37 | 0 | 0 | 0 | 46 | 0 | 46 | 20 | 0 | 20 | 10 | 1 | 11 | 0 | 0 | 0 |
| 13：45 to 14：00 | 25 | 0 | 25 | 271 | 3 | 274 | 58 | 0 | 58 | 0 | 0 | 0 | 51 | 0 | 51 | 34 | 0 | 34 | 11 | 0 | 11 | 0 | 0 | 0 |
| Totals | 179 | 2 | 181 | 2086 | 31 | 2117 | 401 | 3 | 404 | 0 | 0 | 0 | 463 | 0 | 463 | 223 | 0 | 223 | 134 | 5 | 139 | 0 | 0 | 0 |



| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 1 <br> （Left Turn） |  |  | Direction 2 （Through） |  |  | $\begin{gathered} \hline \text { Direction } 3 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | Direction 3 U （U Turn） |  |  | Direction 4 （Left Turn） |  |  | Direction 5 （Through） |  |  | $\begin{gathered} \text { Direction } 6 \\ \text { (Right Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \hline \text { Direction 6U } \\ \text { (U Turn) } \end{gathered}$ |  |  |
| Time Period | 喜 |  |  | 皆 |  | $\stackrel{\text { 틍 }}{ }$ | $\begin{array}{\|l\|} \hline \text { 总 } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} 3 \\ \text { 区 } \\ \text { In } \\ \hline \end{array} \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \stackrel{\text { II }}{\circ} \end{array}$ | 喜 | $\begin{array}{\|l\|} \hline \begin{array}{l} \text { İ } \\ \text { İ工 } \\ \hline \end{array} \\ \hline \end{array}$ | $\stackrel{\text { 틍 }}{ }$ | $\begin{array}{\|l\|} \hline \text { 总 } \\ \hline \end{array}$ | $\begin{array}{\|l\|} \hline \begin{array}{l} \text { 3 } \\ \text { İ } \end{array} \\ \hline \end{array}$ | $\begin{aligned} & \text { 厄⿳一巛口亍刂。 } \\ & \hline \end{aligned}$ | $\begin{array}{\|l\|} \hline \stackrel{\rightharpoonup}{\Xi} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline \text { B } \\ \text { 区ix } \\ \hline \end{array}$ | $\begin{array}{\|l\|l\|} \hline \stackrel{\text { IIO}}{\circ} \\ \hline \end{array}$ | $\begin{array}{\|l} \hline \mathrm{F} \\ \hline \end{array}$ |  | $\begin{aligned} & \overline{\mathrm{g}} \\ & \stackrel{1}{\circ} \\ & \hline \end{aligned}$ | 总 | $\begin{array}{\|l\|l\|} \hline \begin{array}{l} 3 \\ \text { ax } \\ \hline \end{array} \\ \hline \end{array}$ | － |
| 12：00 to 13：00 | 9 | 0 | 9 | 915 | 25 | 940 | 79 | 0 | 79 | 0 | 0 | 0 | 24 | 0 | 24 | ${ }^{61}$ | 0 | 61 | 109 | 2 | 111 | 0 | 0 | 0 |
| 12：15 to 13：15 | 7 | 0 | 7 | 856 | 17 | 873 | 74 | 0 | 74 | 0 | 0 | 0 | 24 | 0 | 24 | 63 | 0 | 63 | 109 | 2 | 111 | 0 | 0 | 0 |
| 12：30 to 13：30 | 6 | 0 | 6 | 823 | 16 | 839 | 67 | 1 | 68 | 1 | 0 | 1 | 21 | 0 | 21 | 47 | 0 | 47 | 111 | 2 | 113 | 0 | 0 | 0 |
| 12：45 to 13：45 | 9 | 0 | 9 | 811 | 15 | 826 | 65 | 1 | 66 | 2 | 0 | 2 | 23 | 0 | 23 | 47 | 0 | 47 | 106 | 2 | 108 | 0 | 0 | 0 |
| 13：00 to 14：00 | 9 | 0 | 9 | 764 | 12 | 776 | 67 | 1 | 68 | 2 | 0 | 2 | 26 | 0 | 26 | 49 | 0 | 49 | 102 | 2 | 104 | 0 | 0 | 0 |
| Totals | 18 | 0 | 18 | 1679 | 37 | 1716 | 146 | 1 | 147 | 2 | 0 | 2 | 50 | 0 | 50 | 110 | 0 | 110 | 211 | 4 | 215 | 0 | 0 | 0 |


| Approach | Princes Hwy |  |  |  |  |  |  |  |  |  |  |  | Moss St |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Direction 7 <br> （Left Turn） |  |  | Direction 8 （Through） |  |  | $\begin{array}{c\|} \hline \text { Direction } 9 \\ \text { (Right Turn) } \end{array}$ |  |  | $\begin{gathered} \hline \text { Direction 9U } \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  | $\begin{gathered} \text { Direction } 10 \\ \text { (Left Turn) } \\ \hline \end{gathered}$ |  |  | Direction 11 （Through） |  |  | $\begin{aligned} & \hline \text { Direction } 12 \\ & \text { (Right Turn) } \\ & \hline \end{aligned}$ |  |  | $\begin{gathered} \hline \text { Direction } 12 \mathrm{U} \\ \text { (U Turn) } \\ \hline \end{gathered}$ |  |  |
| Time Period | $\begin{array}{\|l\|l\|} \hline \text { 吡 } \\ \hline \end{array}$ |  | $\stackrel{\text { 픙 }}{ }$ | $\begin{aligned} & \text { 皆 } \end{aligned}$ |  |  | $\begin{array}{\|l\|} \hline \text { 寽 } \\ \hline \end{array}$ | $$ | $\begin{array}{\|l\|} \hline \mathrm{I} \\ \hline 1 \end{array}$ | $\begin{array}{\|l\|} \hline \text { 总 } \\ \hline \end{array}$ |  | $\begin{aligned} & \overline{\mathrm{g}} \\ & \hline 1 \end{aligned}$ | $\begin{array}{\|l} \hline \mathbf{F} \\ \hline \end{array}$ | $$ | $\stackrel{\overline{\mathrm{g}}}{\stackrel{\mathrm{I}}{\circ}}$ | $\begin{array}{\|l\|l\|} \hline \text { 吡 } \\ \hline \end{array}$ | $$ | $\begin{array}{\|l\|} \hline \overline{\mathrm{I}} \\ \hline \end{array}$ | $\begin{array}{\|l\|l} \hline \text { 学 } \\ \hline \end{array}$ | $$ | $\begin{array}{\|l\|} \hline \mathrm{⿺⿻⿻一㇂㇒丶⿱口一心.} \\ \hline \end{array}$ | $\begin{aligned} & \text { 总 } \\ & \hline \end{aligned}$ |  | － |
| 12：00 to 13：00 | 85 | 2 | 87 | 1033 | 16 | 1049 | 194 | 2 | 196 | 0 | 0 | 0 | 236 | 0 | 236 | 117 | 0 | 117 | 77 | 1 | 78 | 0 | 0 | 0 |
| 12：15 to 13：15 | 86 | 0 | 86 | 979 | 12 | 991 | 205 | 3 | 208 | 0 | 0 | 0 | 248 | 0 | 248 | 110 | 0 | 110 | 79 | 2 | 81 | 0 | 0 | 0 |
| 12：30 | 82 | 0 | 82 | 1005 | 13 | 1018 | 194 | 1 | 195 | 0 | 0 | 0 | 247 | 0 | 247 | 93 | 0 | 93 | 70 | 4 | 74 | 0 | 0 | 0 |
| 12：45 to 13：45 | 87 | 0 | 87 | 1011 | 15 | 1026 | 187 | 1 | 188 | 0 | 0 | 0 | 236 | 0 | 236 | 96 | 0 | 96 | 65 | 4 | 69 | 0 | 0 | 0 |
| 13：00 to 14：00 | 94 | 0 | 94 | 1053 | 15 | 1068 | 207 | 1 | 208 | 0 | 0 | 0 | 227 | 0 | 227 | 106 | 0 | 106 | 57 | 4 | 61 | 0 | 0 | 0 |
| Totals | 179 | 2 | 181 | 2086 | 31 | 2117 | 401 | 3 | 404 | 0 | 0 | 0 | 463 | 0 | 463 | 223 | 0 | 223 | 134 | 5 | 139 | 0 | 0 | 0 |

## Appendix B

## RMSCrash Data

|  |  | $\stackrel{0}{\underline{E}}$ |  |  | $\begin{aligned} & 00 \\ & \underset{\sim}{2} \\ & 0 \\ & \hline 1 \end{aligned}$ |  | $$ |  |  | $$ |  | $\begin{aligned} & \times \\ & \stackrel{\text { © }}{\substack{C}} \\ & \stackrel{\text { O}}{2} \end{aligned}$ |  |  | 0 <br> $\vdots$ <br> 0 <br> 0 <br> 0 <br> 10 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | AS F |
| Southern Region |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Shoalhaven City LGA |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Culburra |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 715825 28/05/2010 | Fri | 16:40 |  | at CULBURRA RD | TJN | STR | Fine | Dry | 80 | 2 | CAR | M41 | N in COONEMIA RD | 20 T |  |  | 1 | 0 | 2 |
| E41295549 |  |  |  |  | RUM: | 13 | Right near |  |  |  | CAR | F75 | W in CULBURRA RD | 60 P |  |  |  |  |  |
| Wollumboola |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Culburra Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 668769 30/04/2009 | Thu | 13:20 | 20 mW | W Coonamia Rd | 2WY | STR | Raining | Wet | 80 | 2 | CAR | M69 | W in Culburra rd | 80 P |  |  | 1 | 0 | 1 |
| E38076353 |  |  |  |  | RUM: | 32 | Right rear |  |  |  | CAR | F70 | W in Culburra RD |  |  |  |  |  |  |
| Report Totals: |  | Total C | hes: 2 | F | : 0 |  | Inju | Crashes |  |  |  |  | Killed: 0 |  |  |  |  |  |  |
| Crashid dataset 1 - Culbu | lburra | a Road | onamia | ia Road - July 200 | 12 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

LOCATION
Culburra Rd Coonamia Road Wollumboola

Crash Data Period 01/07/2007 to 30/06/2012


Classified Roads
$\longrightarrow$ State Road
Regional Road


Prepared 05/02/2013
Planning \& Analysis
Southern Region


|  | L | L |  | Meters |  |
| :--- | :--- | :--- | :--- | :--- | :--- |
| 0 | 50 | 100 | 200 | 300 | 400 |

Road Safety

|  |  |  | $\stackrel{0}{\underline{E}}$ | $\begin{aligned} & \ddot{0} \\ & \stackrel{0}{\Pi} \\ & \stackrel{\#}{0} \\ & \hline 0 \end{aligned}$ |  | $\begin{aligned} & 0.0 \\ & \stackrel{2}{2} \\ & 0 \\ & 0 \end{aligned}$ |  | $\begin{aligned} & \text { 亠凶 } \\ & \frac{\text { TV }}{\tilde{W}} \\ & \stackrel{1}{0} \end{aligned}$ |  |  |  | $\stackrel{\text { ® }}{\stackrel{\text { ® }}{\text { ¢ }}}$ |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | AS F |

## Southern Region <br> Shoalhaven City LGA <br> Pyree

Culburra Rd


80 Proceeding in lane
I $0 \quad 1 \quad S$

50 Proceeding in lane
N 00

Report Totals:
Total Crashes: 2
Fatal Crashes: $0 \quad$ Injury Crashes: Killed: 0

Injured: 1
Crashid dataset 2 - Culburra Road - Mayfield Road - July 2007 to June 2012


Centre for
Road Safety
NSW


## Southern Region <br> Shoalhaven City LGA <br> Pyree <br> Greenwell Pt Rd

678982 18/08/2009 Tue 17:34

E38122832
731681 12/11/2010
Fri 15:10
Total Crashes: 2
at PYREE LANE
$\begin{array}{rcc}\text { TJN } & \text { CRV } & \text { Fine } \\ \text { RUM: } & 13 & \text { Right near } \\ \text { TJN } & \text { CRV } & \text { Fine }\end{array}$
802 TRK M23 N in PYREE LANE
CAR UU W in GREENWELL PT RD
TRK M57 N in PYREE LANE
VAN M44 W in GREENWELL PT RD
Killed: 0
Injury Crashes: 1

10 Turning right
102
50 Proceeding in lane
10 Proceeding in lane 70 Proceeding in lane Injured: 2

Crashid dataset 3 - Greenwell Point Road - Pyree Lane - July 2007 to June 2012

LOCATION
Greenwell Point Road Pyree Lane Pyree

Crash Data Period 01/07/2007 to 30/06/2012



Crashid dataset 4 - Greenwell Point Road - Jindy Andy Lane - July 2007 to June 2012

LOCATION
Greenwell Point Road Jindy Andy Lane Pyree

Crash Data Period 01/07/2007 to 30/06/2012
Legend
Daily Dataset
Fatal Crash
Injury Crash
Non-casualty Crash
Classified Roads
State Road


Prepared 05/02/2013 Planning \& Analysis Southern Region


## Southern Region <br> Shoalhaven City LGA <br> Brundee <br> Greenwell Pt Rd

641033 04/10/2008 Sat 07:15

E35320004
651418 24/12/2008 Wed $13 \cdot 20$
a
at MAYFIELD RD
E36118404
659105 15/02/2009 Sun 12:40
2 m E MAYFIELD RD
E121076898
652318 29/12/2008 Mon 13:45 100 m W MAYFIELD RD E36210068

## Pyree

Greenwell Pt Rd

| 735743 | $17 / 12 / 2010$ | Fri 20:40 |  | at MAYFIELD RD |  |
| ---: | :--- | :--- | :--- | :--- | :--- |
| E42674444 |  |  |  |  |  |
| 683645 | $20 / 09 / 2009$ | Sun 14:20 | 75 m | E MAYFIELD RD |  |
| E38233237 |  |  |  |  |  |
| 621840 | $19 / 04 / 2008$ | Sat $01: 00$ | 60 m | S MAYFIELD RD |  |
| E33394859 |  |  |  |  |  |

692057 26/11/2009 Thu 06:20 2.115 km W PYREE LANE E162083794

Total Crashes: 8
$\begin{array}{lcc}\text { Report Totals: } \quad \text { Total Crashes: } 8 & \text { Fatal Crashes: } 0 \\ \text { Crashid dataset } 5 & \text { - Greenwell Point Road - Mayfield Road - July } 2007 \text { to June } 2012\end{array}$


801 CAR F53 E in GREENWELL PT RD Fence
802 CAR M20 E in GREENWELL PT RD CAR M53 W in GREENWELL PT RD
801 CAR F43 E in GREENWELL PT RD Fence
801 CAR F32 E in GREENWELL PT RD Fence


801 CAR M44 E in GREENWELL PT RD
801 UTE F20 E in GREENWELL PT RD Fence RUM: 87 Off Ift/ft bnd=>obj RUM: 87 Off lft/ft bnd=>obj
2WY CRV Raining Wet RUM: 85 Off rt/ft bnd=>obj
2WY CRV Fog or mist Dry RUM: 81 Off left/rt bnd=>obj

801 UTE M18 N in GREENWELL PT RD Utility pole
801 TRK M42 W in GREENWELL PT RD Guide Post

Killed: 0

Injury Crashes: 1

60 Proceeding in lane
$\begin{array}{llll}\mathrm{N} & 0 & 0 & \mathrm{~S}\end{array}$
75 Incorrect side
15 Proceeding in lane 70 Proceeding in lane

80 Proceeding in lane N 00

80 Proceeding in lane $\quad$ I |  | 1 |
| :--- | :--- | :--- |

50 Proceeding in lane $\quad$ N $0 \quad 0 \quad$ S

55 Proceeding in lane
$\mathrm{N} \quad 0 \quad 0 \quad \mathrm{~S}$
80 Proceeding in lane
$\mathrm{N} \quad 0 \quad 0$

Crashid dataset 5 - Greenwell Point Road - Mayfield Road - July 2007 to June 2012

## LOCATION

Greenwell Point Road Mayfield Road Pyree

## Crash Data Period

 01/07/2007 to 30/06/2012| Legend |  |
| :--- | :--- |
| Daily Dataset |  |
| Fatal Crash |  |
| Injury Crash |  |
| Non-casualty Crash |  |

Classified Roads
State Road


Prepared 05/02/2013
Planning \& Analysis
Southern Region

NSW Road Safety

|  | $\stackrel{\text { O}}{\stackrel{0}{0}}$ |  | $\stackrel{0}{\underline{E}}$ | $\begin{aligned} & \mathscr{0} \\ & \underline{E} \\ & \tilde{\Pi} \\ & 0 \\ & 0 \end{aligned}$ | 을 I © 0 0 | $\begin{aligned} & 000 \\ & \underset{\sim}{2} \\ & 0 \\ & \hline 0 \end{aligned}$ |  |  |  |  | $\begin{aligned} & \text { n } \\ & \stackrel{1}{0} \\ & \dot{O} \end{aligned}$ |  |  |  |  |  |  | $\begin{array}{ll} \text { 을 } \\ \underline{\overline{2}} \\ \underline{\bar{y}} \\ \hline \end{array}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

## Southern Region <br> Shoalhaven City LGA <br> Terara <br> Millbank Rd

759631 44100337

## Worrigee

Greenwell Pt Rd
E40060048 10:55
760610 18/07/2011 Mon 07:
at MILLBANK RD
at WORRIGEE RD

2WY STR Fine Dry
502 TRK M59 E in MILLBANK RD
Unk Forward from drive Unk Proceeding in lane

5064254
735967 04/10/2010 Mon 07:05
at WORRIGEE RD

| XJN | STR | Fine | Dry | 80 | 2 | CAR | M69 | S in MILLBANK RD |
| ---: | :---: | :---: | :---: | :---: | :--- | :--- | :--- | :--- |
| RUM: | 10 | Cross traffic |  |  |  | CAR | F22 | W in GREENWELL PT RD |
| XJN | STR | Fine | Dry | 80 | 2 | CAR | F19 | N in MILLBANK RD |
| RUM: | 10 | Cross traffic |  |  |  | TRK | M28 | E in GREENWELL PT RD |
| XJN | STR | Overcast | Dry | 100 | 2 | M/C | M25 | N in WORRIGEE RD |
| RUM: | 10 | Cross traffic |  |  |  | CAR | F29 | W in GREENWELL PT RD |

Report Totals:
Total Crashes: 4
Fatal Crashes: 0
Injury Crashes: 3
Killed: 0

15 Proceeding in lane
70 Proceeding in lane 20 Proceeding in lane 80 Proceeding in lane 20 Proceeding in lane 70 Proceeding in lane Injured: 4

## LOCATION

Greenwell Point Road Millbank Road Worrigee

## Crash Data Period

 01/07/2007 to 30/06/2012| Legend |
| :---: |
| Daily Dataset Fatal Crash Injury Crash Non-casualty Crash <br> Classified Roads $\qquad$ State Road $\qquad$ Regional Road |
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Prepared 05/02/2013 Planning \& Analysis
Southern Region


| 0 | 50 | 100 | 200 | 300 | 400 |
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NSW


Crashid dataset 7 - Princes Highway - Kalandar Street - July 2007 to June 2012

LOCATION
Princes Highway Kalander Street Nowra

Crash Data Period 01/07/2007 to 30/06/2012


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Southern Region


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| 0 | 50 | 100 | 200 | 300 | 400 |  |



Crashid dataset 8 - Coonamia Road - Currarong Road - Forest Road - July 2007 to June 2012

LOCATION
Coonamia Road Currarong Road Forest Road Wollumboola

Crash Data Period 01/07/2007 to 30/06/2012


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 Road Safety

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| Southern Region |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
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| Shoalhaven City LGA |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Nowra |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Albatross Rd |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 668933 19/05/2009 | Tue | 08:00 |  | at | KINGHORNE ST | RDB | STR | Overcast | Wet | 50 | 2 | CAR | F48 | N in KINGHORNE ST |
| E37994942 |  |  |  |  |  | RUM: | 10 | Cross traffic |  |  |  | M/C | M35 | W in ALBATROSS RD |
| 725541 13/09/2010 | Mon | 07:15 |  | at | KINGHORNE ST | RDB | STR | Fine | Dry | 50 | 2 | P/C | M31 | S in KINGHORNE ST |
| E42458229 |  |  |  |  |  | RUM: | 21 | Right through |  |  |  | CAR | F28 | N in KINGHORNE ST |
| Kalandar St |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 684279 02/09/2009 | Wed | 11:26 |  | at | KINGHORNE ST | RDB | STR | Fine | Dry | 50 | 2 | WAG | F78 | N in KINGHORNE ST |
| E38838374 |  |  |  |  |  | RUM: | 10 | Cross traffic |  |  |  | M/C | M65 | W in Kalandar st |
| Kinghorne St |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 731597 10/11/2010 | Wed | 08:00 | 10 m | S | ALBATROSS RD | RDB | STR | Fine | Dry | 50 | 3 | CAR | M33 | N in KINGHORNE ST |
| E42296544 |  |  |  |  |  | RUM: | 30 | Rear end |  |  |  | CAR | F41 | N in KINGHORNE ST |
|  |  |  |  |  |  |  |  |  |  |  |  | 4WD | F25 | N in KINGHORNE ST |
| Report Totals: |  | Total C | es: 4 |  | Fa | s: 0 |  | Injury | rash | 3 |  |  |  | Killed: 0 |

Crashid dataset 9 - Kalandar Street - Kinghorne Street - July 2007 to June 2012

LOCATION
Kalandar Street Kinghorne Street Nowra

Crash Data Period 01/07/2007 to 30/06/2012


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Southern Region


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| 0 | 50 | 100 | 200 | 300 | 400 |  |


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| Southern Region |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Shoalhaven City LGA |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Falls Creek |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Princes Hwy |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 761750 | 21/07/2011 | Thu | 09:00 | 400 m | S | BTU RD | 2WY | CRV | Raining | Wet | 100 | 2 | CAR | M21 | N in PRINCES HWY | 80 Proceeding in lane | N | 0 | 0 |
| E45355440 |  |  |  |  |  |  | RUM: | 30 | Rear end |  |  |  | CAR | F60 | N in PRINCES HWY | 80 Proceeding in lane |  |  |  |
| 592332 | 02/10/2007 | Tue | 10:05 |  | at | FOREST RD | TJN | STR | Fine | Dry | 100 | 2 | TRK | F45 | N in PRINCES HWY | 65 Proceeding in lane | N | 0 | 0 |
| E31455472 |  |  |  |  |  |  | RUM: | 30 | Rear end |  |  |  | 4WD | F42 | N in PRINCES HWY | 50 Proceeding in lane |  |  |  |
| 646005 | 26/10/2008 | Sun | 17:15 |  | at | FOREST RD | TJN | STR | Overcast | Dry | 100 | 2 | M/C | M53 | S in PRINCES HWY | 80 Proceeding in lane | 1 | 0 | 1 |
| E35606150 |  |  |  |  |  |  | RUM: | 74 | On road-out of | cont. |  |  | TRK | F45 | W in FOREST RD | 15 Turning right |  |  |  |
| 677928 | 11/08/2009 | Tue | 16:55 |  | at | FOREST RD | TJN | CRV | Fine | Dry | 100 | 2 | BUS | MU | W in FOREST RD | 10 Turning right | 1 | 0 | 3 |
| E38331666 |  |  |  |  |  |  | RUM: | 13 | Right near |  |  |  | CAR | F18 | S in PRINCES HWY | 85 Proceeding in lane |  |  |  |
| 692953 | 17/12/2009 | Thu | 08:00 |  | at | FOREST RD | TJN | CRV | Fine | Dry | 100 | 2 | CAR | M83 | W in FOREST RD | 8 Turning right | 1 | 0 | 1 |
| E39272232 |  |  |  |  |  |  | RUM: |  | Right near |  |  |  | TRK | M55 | $S$ in PRINCES HWY | 80 Proceeding in lane |  |  |  |
| 766035 | 23/03/2011 | Wed | 17:00 | 100 m | S | FOREST RD | 2WY | CRV | Fine | Dry | 100 | 2 | TRK | M47 | $S$ in PRINCES HWY | 20 Perform U-turn | 1 | 0 | 4 |
| E44183231 |  |  |  |  |  |  | RUM: | 40 | U turn |  |  |  | CAR | F60 | $S$ in PRINCES HWY | 100 Proceeding in lane |  |  |  |
| Nowra Hill |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Princes Hwy |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 594902 | 20/10/2007 | Sat | 09:15 |  | at | FOREST RD | TJN | STR | Fine | Dry | 80 | 2 | 4WD | F54 | W in FOREST RD | 10 Turning right | N | 0 | 0 |
| E132417394 |  |  |  |  |  |  | RUM: | 13 | Right near |  |  |  | CAR | M67 | S in PRINCES HWY | 30 Proceeding in lane |  |  |  |
| 615468 | 16/01/2008 | Wed | 17:32 |  | at | FOREST RD | TJN | CRV | Raining | Wet | 80 | 2 | CAR | F21 | W in FOREST RD | Unk Turning right | 1 | 0 | 2 |
| E32645106 |  |  |  |  |  |  | RUM: | 13 | Right near |  |  |  | CAR | F36 | S in PRINCES HWY | 80 Proceeding in lane |  |  |  |
| 646934 | 05/11/2008 | Wed | 17:30 |  | at | FOREST RD | TJN | CRV | Fine | Dry | 80 | 2 | CAR | M19 | W in FOREST RD | 10 Turning right | N | 0 | 0 |
| E68611801 |  |  |  |  |  |  | RUM: | 13 | Right near |  |  |  | CAR | F27 | $S$ in PRINCES HWY | 80 Proceeding in lane |  |  |  |
| 763500 | 02/08/2011 | Tue | 06:10 |  | at | FOREST RD | TJN | CRV | Fine | Dry | 100 | 1 | TRK | M48 | N in PRINCES HWY | 100 Proceeding in lane | N | 0 | 0 |
| E45271948 |  |  |  |  |  |  | RUM: | 67 | Struck animal |  |  |  | Kanga |  |  |  |  |  |  |
| 793882 | 27/12/2011 | Tue | 11:00 |  | at | FOREST RD | TJN | CRV | Fine | Dry | 100 | 2 | CAR | M51 | W in FOREST RD | 30 Turning right | 1 | 0 | 5 |
| E46414811 |  |  |  |  |  |  | RUM: | 13 | Right near |  |  |  | TRK | M79 | S in PRINCES HWY | 80 Proceeding in lane |  |  |  |
| 789871 | 04/03/2012 | Sun | 10:30 |  | at | FOREST RD | TJN | CRV | Unk | Wet | 100 | 2 | TRK | M33 | W in FOREST RD | 5 Turning right | 1 | 0 | 1 S |
| E47416866 |  |  |  |  |  |  | RUM: | 13 | Right near |  |  |  | M/C | M52 | S in PRINCES HWY | 70 Proceeding in lane |  |  |  |
| 794640 | 24/04/2012 | Tue | 19:15 |  | at | FOREST RD | TJN | CRV | Fine | Dry | 100 | 2 | WAG | M19 | W in FOREST RD | Unk Turning left | 1 | 0 | 1 |
| E47860950 |  |  |  |  |  |  | RUM: | 16 | Left near |  |  |  | TRK | M25 | S in PRINCES HWY | 95 Proceeding in lane |  |  |  |

NSW Road Safety


LOCATION
Princes Highway
Forest Rd
Nowra Hill

## Crash Data Period

 01/07/2007 to 30/06/2012

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| Southern Region |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Shoalhaven City LGA |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Nowra |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Moss St |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 713852 | 11/06/2010 | Fri | 15:10 | 20 m |  | PRINCES HWY | 2WY | STR | Fine | Dry | 50 | 2 | TRK | M51 | W in MOSS ST | 1 Turning right | N | 0 | 0 |
| E42025553 |  |  |  |  |  |  | RUM: | 21 | Right through |  |  |  | CAR | M37 | E in MOSS ST | 20 Proceeding in lane |  |  |  |
| 731581 | 09/11/2010 | Tue | 11:45 | 50 m |  | PRINCES HWY | 2WY | STR | Fine | Dry | 50 | 2 | LOR | M57 | W in MOSS ST | 7 Reversing in lane | N | 0 | 0 |
| E42945471 |  |  |  |  |  |  | RUM: | 45 | Reversing |  |  |  | CAR | F64 | E in MOSS ST | 0 Stationary |  |  |  |
| Princes Hwy |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 583723 | 18/07/2007 | Wed | 15:40 |  |  | MOSS ST | XJN | STR | Fine | Dry | 70 | 2 | CCH | M49 | $S$ in PRINCES HWY | 20 Turning left | N | 0 | 0 |
| E30787823 |  |  |  |  |  |  | RUM: | 37 | Left turn sidesw | wipe |  |  | CAR | M26 | $S$ in PRINCES HWY | 30 Proceeding in lane |  |  |  |
| 587626 | 24/08/2007 | Fri | 15:10 |  |  | MOSS ST | XJN | STR | Fine | Dry | 70 | 2 | CAR | M30 | N in PRINCES HWY | 40 Proceeding in lane | 1 | 0 | 1 |
| E31076117 |  |  |  |  |  |  | RUM: | 0 | Ped nearside |  |  |  | PED | M17 | PRINCES HWY | Run across carriageway |  |  |  |
| 609063 | 04/02/2008 | Mon | 07:50 |  |  | MOSS ST | XJN | CRV | Raining | Wet | 70 | 1 | CAR | M20 | $S$ in PRINCES HWY | 50 Proceeding in lane | N | 0 | 0 S |
| E32615033 |  |  |  |  |  |  | RUM: | 81 | Off left/rt bnd=> | >obj |  |  | Signal |  |  |  |  |  |  |
| 609630 | 07/02/2008 | Thu | 14:05 |  |  | MOSS ST | XJN | STR | Raining | Wet | 70 | 2 | TRK | M17 | $S$ in PRINCES HWY | 60 Proceeding in lane | 1 | 0 | 1 |
| E33169929 |  |  |  |  |  |  | RUM: | 10 | Cross traffic |  |  |  | CAR | F32 | E in MOSS ST | 15 Proceeding in lane |  |  |  |
| 630665 | 06/07/2008 | Sun | 15:00 |  |  | MOSS ST | XJN | STR | Overcast | Dry | 70 | 2 | CAR | M40 | $S$ in PRINCES HWY | 40 Proceeding in lane | N | 0 | 0 |
| E36001082 |  |  |  |  |  |  | RUM: | 10 | Cross traffic |  |  |  | CAR | F57 | W in MOSS ST | 10 Proceeding in lane |  |  |  |
| 651142 | 31/10/2008 | Fri | 23:19 |  |  | MOSS ST | XJN | CRV | Fine | Dry | 70 | 2 | CAR | F17 | $S$ in PRINCES HWY | 40 Turning right | N | 0 | 0 |
| E35349311 |  |  |  |  |  |  | RUM: | 21 | Right through |  |  |  | CAR | UU | $N$ in PRINCES HWY | 60 Proceeding in lane |  |  |  |
| 645679 | 11/11/2008 | Tue | 14:00 |  |  | MOSS ST | XJN | CRV | Fine | Dry | 70 | 2 | CAR | UU | $S$ in PRINCES HWY | 10 Turning left | 1 | 0 | 1 |
| E35721962 |  |  |  |  |  |  | RUM: | 37 | Left turn sidesw | wipe |  |  | WAG | F18 | S in PRINCES HWY | 10 Proceeding in lane |  |  |  |
| 676027 | 21/07/2009 | Tue | 07:30 |  |  | MOSS ST | XJN | CRV | Fine | Dry | 70 | 2 | VAN | M50 | E in MOSS ST | 40 Turning left | 1 | 0 | 1 |
| E38481963 |  |  |  |  |  |  | RUM: | 16 | Left near |  |  |  | TRK | M22 | N in PRINCES HWY | 70 Proceeding in lane |  |  |  |
| 687360 | 05/11/2009 | Thu | 22:40 |  | at M | MOSS ST | XJN | STR | Raining | Wet | 70 | 2 | CAR | M18 | $S$ in PRINCES HWY | 35 Turning right | N | 0 | 0 |
| E39611042 |  |  |  |  |  |  | RUM: | 21 | Right through |  |  |  | CAR | F19 | $N$ in PRINCES HWY | 65 Proceeding in lane |  |  |  |
| 716195 | 29/06/2010 | Tue | 19:05 |  | at M | MOSS ST | XJN | CRV | Fine | Dry | 70 | 2 | CAR | M22 | E in MOSS ST | Unk Proceeding in lane | N | 0 | 0 |
| E41092152 |  |  |  |  |  |  | RUM: | 10 | Cross traffic |  |  |  | CAR | F62 | $S$ in PRINCES HWY | 60 Proceeding in lane |  |  |  |
| 755191 | 02/06/2011 | Thu | 20:45 |  | at M | MOSS ST | XJN | CRV | Fine | Dry | 70 | 2 | CAR | M19 | $N$ in PRINCES HWY | 40 Turning right | N | 0 | 0 |
| E142468698 |  |  |  |  |  |  | RUM: | 21 | Right through |  |  |  | CAR | M57 | $S$ in PRINCES HWY | 55 Proceeding in lane |  |  |  |
| 779872 | 30/11/2011 | Wed | 10:20 |  | at M | MOSS ST | XJN | CRV | Overcast | Dry | 70 | 2 | P/C | F38 | W in PRINCES HWY | Along footpath | 1 | 0 | 1 |
| E46271072 |  |  |  |  |  |  | RUM: | 48 | From footpath |  |  |  | CAR | F34 | N in PRINCES HWY | 50 Proceeding in lane |  |  |  |

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| 807368 | 27/03/2012 | Tue | 16:25 |  | at | MOSS ST |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| E48015874 |  |  |  |  |  |  |
| 803031 | 25/06/2012 | Mon | 18:09 |  | at | MOSS ST |
| E47440410 |  |  |  |  |  |  |
| 626943 | 28/11/2007 | Wed | 11:40 | 5 m | N | MOSS ST |
| E32187245 |  |  |  |  |  |  |
| 735985 | 23/10/2010 | Sat | 16:00 | 10 m | N | MOSS ST |
| E42380113 |  |  |  |  |  |  |
| 782473 | 30/11/2011 | Wed | 14:07 | 20 m | N | MOSS ST |
| E48155887 |  |  |  |  |  |  |
| 779748 | 31/12/2011 | Sat | 11:15 | 50 m | N | MOSS ST |
| E46977777 |  |  |  |  |  |  |
| 791426 | 16/04/2012 | Mon | 18:29 | 1 m | S | MOSS ST |
| E47469227 |  |  |  |  |  |  |
| 694839 | 12/12/2009 | Sat | 06:30 | 2 m | S | MOSS ST |
| E233306692 |  |  |  |  |  |  |
| 594131 | 06/10/2007 | Sat | 18:10 | 5 m | S | MOSS ST |
| E31911549 |  |  |  |  |  |  |
| 687362 | 02/10/2009 | Fri | 15:09 | 5 m | S | MOSS ST |
| E134752896 |  |  |  |  |  |  |
| 758674 | 21/05/2011 | Sat | 13:35 | 15 m | S | MOSS ST |
| E44591534 |  |  |  |  |  |  |
| 799593 | 07/06/2012 | Thu | 15:45 | 40 m | S | MOSS ST |
| E48621043 |  |  |  |  |  |  |
| 735971 | 07/10/2010 | Thu | 13:00 |  | at | NORTH ST |
| E42310048 |  |  |  |  |  |  |
| 782720 | 25/01/2012 | Wed | 16:10 |  | at | NORTH ST |
| E46347009 |  |  |  |  |  |  |
| 678116 | 27/05/2009 | Wed | 14:00 | 5 m | S | NORTH ST |
| E37815107 |  |  |  |  |  |  |



| 50 Proceeding in lane | 1 | 0 | 1 |  |
| :---: | :---: | :---: | :---: | :---: |
| Proceeding in lane |  |  |  |  |
| Unk Turning right | N | 0 | 0 |  |
| 70 Proceeding in lane |  |  |  |  |
| 40 Veering right | N | 0 | 0 |  |
| 0 Stationary |  |  |  |  |
| 40 Proceeding in lane | 1 | 0 | 1 | S |
| 0 Stationary |  |  |  |  |
| 20 Veering left | N | 0 | 0 |  |
| 50 Proceeding in lane |  |  |  |  |
| 40 Veering right | N | 0 | 0 |  |
| 40 Proceeding in lane |  |  |  |  |
| 40 Proceeding in lane |  |  |  |  |
| 65 Proceeding in lane | 1 | 0 | 1 |  |
| Riding skateboard |  |  |  |  |
| Unk Turning left | 1 | 0 | 1 |  |
| Walk across carriageway |  |  |  |  |
| Unk Proceeding in lane |  |  |  |  |
| Unk Proceeding in lane | 1 | 0 | 1 |  |
| 0 Stationary |  |  |  |  |
| 30 Proceeding in lane | 1 | 0 | 1 |  |
| With traffic, not edge |  |  |  |  |
| Unk Veering right | N | 0 | 0 |  |
| Unk Proceeding in lane |  |  |  |  |
| 10 Proceeding in lane | N | 0 | 0 |  |
| 0 Parked other |  |  |  |  |
| 60 Proceeding in lane |  |  |  |  |
| 10 Turning left | N | 0 | 0 |  |
| 10 Proceeding in lane |  |  |  |  |
| 60 Veering left | 1 | 0 | 1 |  |
| 60 Proceeding in lane |  |  |  |  |



Crashid dataset 11 - Princes Highway - Moss Street - July 2007 to June 2012

LOCATION
Princes Highway Moss Street Nowra

## Crash Data Period

 01/07/2007 to 30/06/2012

Prepared 05/02/2013 Planning \& Analysis
Southern Region


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| 0 | 50 | 100 | 200 | 300 | 400 |  |

## Appendix C

## Calculation of Traffic Growth Factors \& Trip Generation Rates (Shoalha ven City Council)

## From:

Sent:
To:
Subject:
Attachments:

Ken Hollyoak
Wednesday, 20 February 2013 9:21 AM
Wayne Johnson; Justin Murphy
FW: UPDATED Trip Generation Analysis - Culburra 3A Development - assumptions for traffic study
Hourly Data 200407053 Falls Creek Correction.xls; Greenwell Point Rd Annual Analysis based on 2008.xlsx; Forest Rd Annual Analysis based on 2008.xlsx; Culburra Traffic Gen Analysis.xlsx

From: Wells, Scott [mailto:WELLSS@shoalhaven.nsw.gov.au]
Sent: Tuesday, 19 February 2013 6:37 PM
To: Ken Hollyoak
Cc: Britton, John; Williams, Brett; 'MILLET Chris P'
Subject: UPDATED Trip Generation Analysis - Culburra 3A Development - assumptions for traffic study

## Hi Ken

The following is summary of our analysis of external traffic generation rates to be applied, and what adjustments are required in our view to the May 2012 base survey data you are intending to use as basis for your analysis. The spreadsheet used to base this analysis summary is attached (Culburra Traffic Gen analysis) and the annual traffic data used to derive AADT and $120^{\text {th }} \mathrm{HH}$ factors are also attached (Greenwell Pt Rd and Forest Rd for local adjustment factors, Princes Highway Falls Creek for Highway adjustment factors).

The analysis is detailed, undertaken by our Transport Engineer. I have reviewed and support the findings. The impact of the development is likely to be considerable and we have undertaken the analysis to ensure the analysis of the developments impacts is sufficiently detailed, robust and realistic in order for Council and RMS to better understand the developments likely impacts.

As previously stated this area is objected to significant seasonal fluctuations in traffic levels, thus the request to consider an AADT scenario as well as $120^{\text {th }} \mathrm{HH}$ scenario in accordance with RMS guidelines and AUSTROADS.

## Peak Hour Development Traffic Generation

To be applied to proposed residential development - these rates are based on detached dwellings, reduction may need to be considered for any proposed non-detached dwellings. Note this is for external regional traffic distribution only. Directional split data obtained from examining local road annual data in equivalent AADT \& $120^{\text {th }}$ Highest Annual Hour periods.

| Peak Hour <br> Scenario | Factor (vehicles per <br> hour per occupied <br> dwelling) | Directional Split - AADT <br> (eastbound / westbound) | Directional Split - 120 <br> (eastbound $/$ HH <br> (estbound) |
| :---: | :---: | :---: | :---: |
| Friday AM | 0.22 | $22 \% / 78 \%$ | $24 \% / 76 \%$ |
| Friday PM | 0.21 | $65 \% / 35 \%$ | $75 \% / 25 \%$ |
| Saturday MD | 0.23 | $53 \% / 47 \%$ | $50 \% / 50 \%$ |

Note: The balance of peak hour trips per dwelling (in accordance with RTA's Guide to Traffic Generating Developments) must be assigned to/from Culburra to complete the external distribution analysis.

## Survey Data Conversion Rates for Peak Hour Traffic Volumes - NON-HIGHWAY DATA - Based on Local Peaks

These factors convert 1-hour data from the applicants May 2012 surveys to theoretical AADT \& Seasonal Peak flows for the intersection analysis. These factors apply to all surveyed local road peak hour flows, including all movements to/from the Highway, but not north-south through movements on the Highway (refer factors below for highway analysis). The additional factors for Friday ( $3-4 \mathrm{pm}$ ) \& Saturday ( $8-9 \mathrm{am}$ ) are provided for separate analysis as the local and highway peaks do not coincide at these times. Note the Friday AM peak ( $8-9 \mathrm{am}$ ) for local \& Highway traffic coincided, therefore separate analysis is not required. Because of the conflicting peak times our recommendation is for all of the following scenarios to be assessed for worst case in each of the AADT and $120^{\text {th }} \mathrm{HH}$ scenarios.

| Peak Hour Scenario <br> AADT Analysis | Factor <br> (converts 2012 survey data to theoretical AADT <br> values - LOS C target for intersection analysis) |
| :---: | :---: |
| Friday AM (8-9am) | 0.92 |
| Friday PM (2-3pm) | $1.10^{*}$ |
| Saturday MD (12-1pm) | 1.11 |
| Friday PM (3-4pm) | 1.10 |
| Saturday AM (8-9am) | 1.07 |


| Peak Hour Scenario <br> Seasonal Peak (120 <br> Analysis | FH) |
| :---: | :---: |
| (converts 2012 survey data to theoretical Seasonal <br> Peak values i.e. 120 <br> LOS D target for intersection anallysis) |  |
| Friday AM (8-9am) | 1.12 |
| Friday PM (2-3pm) | $1.41^{*}$ |
| Saturday MD (12-1pm) | 1.25 |
| Friday PM (3-4pm) | 1.41 |
| Saturday MD (8-9am) | 1.17 |

* Note: the Friday PM 1-hour analysis has also been factored to account for the actual local peak which occurred between $2-3 \mathrm{pm}$, rather than the $4-5 \mathrm{pm}$ peak reported in the 2012 survey data, which was approx $15 \%$ lower than the $2-3 \mathrm{pm}$ volume (local roads analysis).
* Note: the above factors are derived from the combined analysis of annual traffic data from the Greenwell Point Road and Forest Road data.


## Survey Data Conversion Rates for Peak Hour Traffic Volumes - PRINCES HIGHWAY DATA Based on Local \& Highway Peaks

This converts 1-hour data from May 2012 survey to theoretical AADT \& Seasonal Peak flows for intersection analysis. These factors apply only to all surveyed north-south through movements on the Highway for analysis based on either local (Coastal Villages) or Highway peak hour flows. The additional factors for Friday ( $3-4 \mathrm{pm}$ ) \& Saturday ( $8-9 \mathrm{am}$ ) are provided for separate analysis as the local and highway peaks do not coincide at these times. Note the Friday AM peak ( $8-9 \mathrm{am}$ ) for local \& Highway traffic coincided, therefore separate analysis is not required. Because of the conflicting peak times our recommendation is for all of the following scenarios to be assessed for worst case in each of the AADT and $120^{\text {th }} \mathrm{HH}$ scenarios.

| Peak Hour Scenario <br> AADT Analysis | Factor <br> (converts 2012 survey data to theoretical AADT <br> values - LOS C target for intersection analysis) |
| :---: | :---: |
| Friday AM (8-9am) | 0.88 |
| Friday PM (2-3pm) ${ }^{\star *}$ | 0.89 |
| Saturday MD (12-1pm) | 1.27 |
| Friday PM (3-4pm) | 0.88 |
| 1.32 |  |
| Peaturday Hour Scenario $(8-9 a m)$  <br> Seasonal Peak (120  <br> H. HH) | Factor <br> (converts 2012 survey data to theoretical Seasonal |


| Analysis | Peak values i.e. $120^{\text {th }}$ Highest Annual Hour - <br> LOS D target for intersection analysis) |
| :---: | :---: |
| Friday AM (8-9am) | 1.25 |
| Friday PM (2-3pm)** | 1.13 |
| Saturday MD (12-1pm) $^{\dagger}$ | 1.37 |
| Friday PM (3-4pm) | 1.15 |
| Saturday MD (8-9am) | 1.43 |

Note: The above factors are based on a permanent count station on the Princes Highway at Falls Creek (ie outside of the Nowra Urban Area) which is subject to AADT volumes of approx 20,000 veh/day. As the Princes Highway through the Nowra Urban Area is subject to volumes approximately twice as high (ie 40,000 veh/day), the influence of these factors can be reduced by half for analysis of the urban Princes Highway intersections (ie at Kalandar Street \& Moss Street). This results in the following factors (reading down the page): 0.94, 0.95, 1.14, 0.94, 1.16, 1.13, 1.07, 1.18, 1.07 \& 1.21 for urban area intersection analysis, and it is our view that it would be appropriate for the latter factors to be applied to all north-south through movements on the Highway to avoid un-justified increases or decreases to through-flows on the Highway for analysis.
** Note: the Friday PM peak hour on the Princes Highway at Falls Creek was 3-4pm, differed from both the surveyed peak (4-5pm) and the local peak (2-3pm). Accordingly, separate factors for undertaking a "Highway" peak hour analysis are also provided (the actual likely Friday PM peak hour).
${ }^{\dagger}$ Note: the Saturday peak hour on the Princes Highway at Falls Creek occurred between 8-9am, which was different to the local peak (12-1pm). Accordingly, separate factors for undertaking a "Highway" peak hour analysis are also provided (the actual likely Saturday peak hour).

## Survey Data Conversion Rates for "Daily" Traffic Volumes - NON-HIGHWAY DATA

These factors convert the applicant's May 2012 survey data to theoretical daily flows from AADT \& Seasonal Peak (120 ${ }^{\text {th }}$ Highest Annual Hour) equivalent peak hour flow levels. This is required to be undertaken to assess road design aspects (cross-section parameters) ie lane widths, clear zones, overtaking lanes etc for all scenarios including BOTH with / without the development.

This analysis is for local roads only, ie cross section assessment is not required to be undertaken on the Princes Highway.

The factors were determined by combining a peak-to-daily factor for each of the 6 scenarios (determined by analysing 2008 AADT \& Seasonal peak-to-daily factors separately) with a conversion factor for either Friday or Saturday, which compared the equivalent survey dates in 2008 (based on proximity to School Holidays) to the AADT \& Seasonal ( $120^{\text {th }}$ HH ) volumes accordingly.

Because of the conflicting peak times our recommendation is for all of the following scenarios to be assessed for worst case in each of the AADT and $120^{\text {th }}$ HH daily flow scenarios.

| AADT Peak-to-Daily Factors <br> (applies to surveyed 1-hour data <br> from May 2012) | Factor <br> (converts 2012 survey data to theoretical Annual <br> Average Daily volume level) |
| :---: | :---: |
| Friday AM (8-9am) | 10.22 |
| Saturday MD (12-1pm) | 14.06 |


| Seasonal Peak-to-Daily <br> Factors (applies to surveyed 1- <br> hour data from May 2012) | Factor <br> (converts 2012 survey data to theoretical Seasonal <br> Peak $\left(120^{\text {th }} \mathrm{HH}\right)$ equivalent daily volume level) |
| :---: | :---: |
| Friday AM (8-9am) | 14.34 |
| Saturday MD (12-1pm) | 16.67 |

* Note: the actual Friday PM local peak occurred between 2-3pm, rather than the 4-5pm peak reported in the 2012 survey data, which was approx $15 \%$ lower than the $2-3 p m$ volume. Accordingly the adjustment has only been applied to the Friday AM data, although only one daily flow calculation is required for the Friday in any case.
* Note: the above factors are to be applied direct to the base 2012 May survey data (local roads only).
* Note: the above factors are to calculate base daily flow levels for the AADT and $120^{\text {th }}$ HH equivalent daily flow scenarios. Your assessment will need to take into account the case with / without the development. To estimate the developments external daily traffic generation, refer to the top table for peak hour generation rates and use the same ratio of (external regional peak hour generation / RMS peak hour generation) to the RMS daily traffic generation rates to estimate external regional daily traffic generation. Similar to the peak hour analysis the balance of daily trips (between the RMS daily rate and the external regional daily development traffic) will then be assigned to/from Culburra Village to assess those more local impacts, in addition to the regional road impact analysis.

I hope all of that makes sense. We have tried to explain it in as simple terms as possible. We don't want to over complicate the assessment, but we do have an obligation to Council and the local community that the assessment has been undertaken correctly.

In regards to the future analysis scenario Council has adopted an ID forecast data set which provides population and dwelling projections in 5 year increments to 2036.

In addition to the above we will review this forecast data to provide our best advice in regards to an appropriate background traffic growth rate to apply for your assessment of future impacts. le as previously advised your analysis will need to consider a more realistic future time upon which the development will fully impact the surrounding road network. A ten year assessment is supported as industry practice however the growth rate to be applied needs to be agreed. We will review our ID data and provide advice in the coming days.

We will also provide our advice regarding external traffic distribution when you can clarify some of the points raised in my previous email and accordingly provide the additional information required.

Hope all of this helps, and I apologise for the delay.
I would expect the RMS will now review this advice and indicate whether they concur to this component of the study assumptions, as they will need to do for the distribution assumptions.

```
Kind Regards,
Scott Wells
Traffic & Transport Unit Manager
Shoalhaven City Council
```



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wellss@shoalhaven.nsw.gov.au
}ttp://shoalhaven.nsw.gov.au
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```


## From 2008 Annual Counts on Forest Rd \& Greenwell Point Rd

Forest Rd AADT: 2131 (FOREST ROAD 550m EAST OF PRINCES HIGHWAY)
Greenwell Point Rd AADT: 6003 (GREENWELL POINT ROAD 737m WEST OF PYREE LANE)
Combined Data AADT: 8134
Combined Equivalent Census Day: 7576 (occurred on Tuesday 12th Aug 2008 - equivalent day to 2011 Census day on 9th Aug 2011)

> Forest Rd 120th HH: 232 2008-04-29 16:00:00 Tuesday
> Greenwell Point Rd 120th HH: 669 2008-03-24 13:00:00 Monday

Combined Data 120th HH :
885 2008-01-25 13:00:00 Friday
(based on combining hourly data from each site to determine total 120th HH )
Equivalent Census Night Occupied Dwellings: 3324 (based on 2011 Census Enumerated population - where people were on Census night)
Note: see separate tab for methodology to calculate number of "equivalent" dwellings
Equivalent 2011 Combined AADT: 8632 ( $2 \%$ compound growth assumed)
Equivalent 2011 Combined Census Day Volume: 8040 ( $2 \%$ compound growth assumed)
External Traffic Generation: 2.4 Average Daily Trips per Occupied Dwelling
Equivalent 2011 AADT Occupied Dwellings: 3569 Theoretical number of occupied dwellings for AADT traffic rates
Fri AADT AM Peak to Daily Factor: Fri AADT PM Peak to Daily Factor: Sat AADT MD Peak to Daily Factor:

Fri AADT AM External Traffic Generation: Fri AADT PM External Traffic Generation: Sat AADT MD External Traffic Generation:

Daily volume Friday 9th May 2008: 9th May 2008 to 2008 AADT Conversion Factor: 9th May 2008 to Seaonal Peak Converstion Factor:

Daily volume Saturday 10th May 2008:
10th May 2008 to 2008 AADT Conversion Factor:
10th May 2008 to Seaonal Peak Converstion Factor:
AM Peak Friday 9th May 2008:
PM Peak Friday 9th May 2008:
MD Peak Saturday 10th May 2008:
3-4pm volume Fri 9th May 2008:
4-5pm volume Fri 9th May 2008: PM Peak Hour Conversion Factor:

HW1 Peak 8-9am Sat 8th May 2004:
Fri AM Peak Conversion Factor for AADT Analysis: Fri PM Peak Conversion Factor for AADT Analysis: Sat MD Peak Conversion Factor for AADT Analysis:

Fri Seasonal Peak AM Peak to Daily Factor: 7.6\% (based on Friday 25th Jan - date of 120th HH and 2nd highest Friday observed in 2008) Fri Seasonal Peak PM Peak to Daily Factor: $7.8 \%$ (based on Friday 25th Jan - date of 120th HH and 2nd highest Friday observed in 2008) Sat Seasonal Peak MD Peak to Daily Factor: $9.6 \%$ (based on Saturday 26th Jan - 3rd highest Saturday observed in 2008)
$9.0 \%$ (based on Fridays with volumes close to AADT volume of 8,134 veh/day)
$8.5 \%$ (based on Fridays with volumes close to AADT volume of 8,134 veh/day)
$9.6 \%$ (based on Saturdays with volumes close to AADT volume of 8,134 veh/day)
0.22 Vehicles per Hour per Occupied Dwelling
0.21 Vehicles per Hour per Occupied Dwelling
0.23 Vehicles per Hour per Occupied Dwelling

8826 Vehicles
0.92 This factor should be applied to the survey data from Fri 4th May 2012 1.35 This factor should be applied to the survey data from Fri 4th May 2012 7438 Vehicles
1.09 This factor should be applied to the survey data from Sat 5th May 2012
1.60 This factor should be applied to the survey data from Sat 5th May 2012

792 8-9am (equivalent day to date of survey Friday 4th May 2012 - based on proximity to school holidays) 741 2-3pm (equivalent day to date of survey Friday 4th May 2012 - based on proximity to school holidays) 706 12-1pm (equivalent day to date of survey Saturday 5th May 2012 - based on proximity to school holidays)

738 3-4pm HW1 Peak Hour
628 4-5pm (equivalent peak hour to survey data 4-5pm - lower than actual 2-3pm peak)
1.18 (converts reported $4-5$ pm data to actual $2-3 p m$ local peak)

487 8-9am HW1 Peak Hour
0.92 (Converts consultant's 2012 survey data to AADT values for LOS C analysis) 1.10 (Converts consultant's 2012 survey data to AADT values for LOS C analysis) 1.11 (Converts consultant's 2012 survey data to AADT values for LOS C analysis)

Daily volume on Friday 25th January 2008: 11885 (Day that 120th Highest Annual Hour Occurred)
120th Highest Annual Hour to Daily Traffic Ratio: $7.4 \%$ Use this figure to convert 2012 120th HH to Daily Volumes for cross-section design analysis
Fri AM Peak Conversion Factor for Seasonal Peak Analysis: Fri PM Peak Conversion Factor for Seasonal Peak Analysis: Sat MD Peak Conversion Factor for Seasonal Peak Analysis:
1.12 (Converts consultant's 2012 survey data to Seasonal Peak values for LOS D analysis) 1.41 (Converts consultant's 2012 survey data to Seasonal Peak values for LOS D analysis) 1.25 (Converts consultant's 2012 survey data to Seasonal Peak values for LOS D analysis)

> HW1 Analysis
> Falls Creek 2004 Data
> 2004 AADT: 18845 (Count Station 07053 at Falls Creek North of Jervis Bay Road)
> 2004 Average Fridays: 22373 (Better comparison due to wide daily variation)
> 2004 Average Saturdays: 17181 (Better comparison due to wide daily variation)

Falls Creek 120th HH: 1965 Thursday 8th Jan, 2004 12pm-1pm
HW1 Fri AADT AM Peak to Daily Factor: $7.3 \%$ (based on Fridays with volumes close to the Average Friday volume of 22,373 veh/day)
HW1 Fri AADT PM Peak to Daily Factor: $8.2 \%$ (based on Fridays with volumes close to the Average Friday volume of 22,373 veh/day) HW1 Sat AADT MD Peak to Daily Factor: $9.7 \%$ (based on Saturdays with volumes close to Average Saturday volume of 17,181 veh/day)

Daily Volume Friday 7th May 2004: 21007 Vehicles (equivalent day to Friday 4th May 2012 survey date)
7th May 2004 to 2004 AADT Conversion Factor: 0.90 This factor should be applied to the survey data from Fri 4th May 2012
Daily volume Saturday 8th May 2004: 16561 Vehicles
8th May 2004 to 2004 AADT Conversion Factor: 1.14 This factor should be applied to the survey data from Sat 5th May 2012
AM Local Culburra Peak Friday 7th May 2004: 1567 8-9am (Same as HW1 peak)
PM Local Culburra Peak Friday 7th May 2004: 1603 2-3pm (HW1 peak occurred 3-4pm - see below)
MD Local Culburra Peak Saturday 8th May 2004: 1436 12-1pm (HW1 peak occurred 8-9am - see below)
HW1 Peak 3-4pm Fri 7th May 2004: 1759 3-4pm HW1 Peak Hour
HW1 4-5pm volume Fri 7th May 2004: 1732 4-5pm (To compare to collected data)
PM Peak Hour Conversion Factor: $\mathbf{0 . 9 3}$ (converts reported 4-5pm data to actual 2-3pm local peak)

Fri AM Peak Conversion Factor for AADT Analysis: Fri PM Peak Conversion Factor for AADT Analysis: Sat MD Peak Conversion Factor for AADT Analysis:
0.88 (Converts consultant's 2012 survey data to AADT values for LOS C analysis)
0.89 (Converts consultant's 2012 survey data to AADT values for LOS C analysis)
1.27 (Converts consultant's 2012 survey data to AADT values for LOS C analysis)

Daily volume on Thursday 8th January 2004: 23295 (Day that 120th Highest Annual Hour Occurred)
120th Highest Annual Hour to Daily Traffic Ratio: $8.4 \%$ Use this figure to convert 2012120 th HH to Daily Volumes for cross-section design analysis

Fri AM Peak Conversion Factor for Seasonal Peak Analysis: Fri PM Peak Conversion Factor for Seasonal Peak Analysis: Sat MD Peak Conversion Factor for Seasonal Peak Analysis:
1.25 (Converts consultant's 2012 survey data to Seasonal Peak values for LOS D analysis) 1.13 (Converts consultant's 2012 survey data to Seasonal Peak values for LOS D analysis) 1.37 (Converts consultant's 2012 survey data to Seasonal Peak values for LOS D analysis)

## HW1 Analysis - factors for Different Highway Peaks

Local Road (i.e. Non-Highway) Flows
Fri PM Highway 3-4pm Conversion Factor for AADT Analysis: Sat AM Highway 8-9am Conversion Factor for AADT Analysis:
ri PM Highway 3-4pm Conversion Factor for 120th HH Analysis: at AM Highway 8-9am Conversion Factor for 120th HH Analysis:
1.10 (Converts consultant's 2012 survey data to AADT values for LOS C analysis) 1.07 (Converts consultant's 2012 survey data to AADT values for LOS C analysis)
1.41 (Converts consultant's 2012 survey data to AADT values for LOS C analysis) 1.17 (Converts consultant's 2012 survey data to AADT values for LOS C analysis)

Highway North-South Flows

Fri PM Highway 3-4pm Conversion Factor for AADT Analysis: Sat AM Highway 8-9am Conversion Factor for AADT Analysis:
ri PM Highway 3-4pm Conversion Factor for 120th HH Analysis: at AM Highway 8-9am Conversion Factor for 120th HH Analysis:
0.88 (Converts consultant's 2012 survey data to AADT values for LOS C analysis 1.32 (Converts consultant's 2012 survey data to AADT values for LOS C analysis) 1.15 (Converts consultant's 2012 survey data to AADT values for LOS C analysis) 1.43 (Converts consultant's 2012 survey data to AADT values for LOS C analysis

| Local Peak Comparison to HW1 Peak |  |  |
| :---: | :---: | :---: |
| Forest Rd + Greenwell Point Rd Friday AM | 792 (8-9am) | Forest Rd + Greenwell Point Rd LOCAL Fri AM Peak (SAME AS HW1 PEAK) |
| Forest Rd + Greenwell Point Rd Friday PM | 741 (2-3pm) | Forest Rd + Greenwell Point Rd LOCAL Fri PM Peak |
|  | 738 (3-4pm) | Forest Rd + Greenwell Point Rd $3-4 \mathrm{pm}$ volume |
| Local to HW1 peak Conversion Factor: | 1.00 (Converts | LOCAL Peak data to HW1 peak for Highway intersection analysis) |
| Forest Rd + Greenwell Point Rd Saturday MD | 706 (12-1pm) | Forest Rd + Greenwell Point Rd LOCAL Sat MD Peak |
|  | 487 (8-9am) | Forest Rd + Greenwell Point Rd 8-9am volume |
| Local to HW1 peak Conversion Factor: | 0.69 (Converts | LOCAL Peak data to HW1 peak for Highway intersection analysis) |

## 50\% Reduction Factor for HW1 intersections in Nowra Urban Area - ie at Kalandar St \& Moss St

[^2]| Date | Forest Rd |  |  | Greenwell Point Road |  |  | Combined |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Combined | E-bound W-bound | $\begin{aligned} & \text { \% E } \\ & \% \text { W } \end{aligned}$ | Combined | E-bound W-bound | $\begin{aligned} & \hline \text { \% E } \\ & \% \text { W } \end{aligned}$ | Combined | E-bound W-bound | $\begin{aligned} & \text { \% E } \\ & \text { \% W } \end{aligned}$ |
| $\begin{array}{\|c\|} \hline \text { Fri } 29-A u g-08 \\ \text { AM Peak } \\ \hline \end{array}$ | 180 | $\begin{gathered} \hline 54 \\ 126 \\ \hline \end{gathered}$ | $\begin{aligned} & \hline 30 \% \\ & 70 \% \\ & \hline \end{aligned}$ | 556 | $\begin{aligned} & \hline 105 \\ & 451 \end{aligned}$ | $\begin{aligned} & \hline 19 \% \\ & 81 \% \end{aligned}$ | 736 | $\begin{aligned} & \hline 159 \\ & 577 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline 22 \% \\ & 78 \% \\ & \hline \end{aligned}$ |
| $\begin{array}{\|c} \hline \text { Fri 29-Aug-08 } \\ \text { PM Peak } \end{array}$ | 167 | $\begin{gathered} 112 \\ 55 \end{gathered}$ | $\begin{aligned} & \hline 67 \% \\ & 33 \% \end{aligned}$ | 562 | $\begin{aligned} & 364 \\ & 198 \end{aligned}$ | $\begin{aligned} & \hline 65 \% \\ & 35 \% \end{aligned}$ | 729 | $\begin{aligned} & \hline 476 \\ & 253 \end{aligned}$ | $\begin{aligned} & \hline 65 \% \\ & 35 \% \end{aligned}$ |
| $\begin{gathered} \text { Sat 30-Aug-08 } \\ \text { MD Peak } \\ \hline \end{gathered}$ | 186 | $\begin{aligned} & 94 \\ & 92 \\ & \hline \end{aligned}$ | $\begin{aligned} & 51 \% \\ & 49 \% \\ & \hline \end{aligned}$ | 490 | $\begin{aligned} & 261 \\ & 229 \\ & \hline \end{aligned}$ | $\begin{aligned} & 53 \% \\ & 47 \% \\ & \hline \end{aligned}$ | 676 | $\begin{aligned} & 355 \\ & 321 \end{aligned}$ | $\begin{aligned} & 53 \% \\ & 47 \% \\ & \hline \end{aligned}$ |
| $\begin{gathered} \hline \text { Fri 25-Jan-08 } \\ \text { AM Peak } \\ \hline \end{gathered}$ | 204 | $\begin{gathered} \hline 61 \\ 143 \\ \hline \end{gathered}$ | $\begin{aligned} & \hline 30 \% \\ & 70 \% \\ & \hline \end{aligned}$ | 561 | $\begin{aligned} & \hline 121 \\ & 440 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline 22 \% \\ & 78 \% \\ & \hline \end{aligned}$ | 765 | $\begin{aligned} & 182 \\ & 583 \\ & \hline \end{aligned}$ | $\begin{aligned} & \hline 24 \% \\ & 76 \% \\ & \hline \end{aligned}$ |
| $\begin{gathered} \text { Fri 25-Jan-08 } \\ \text { PM Peak } \end{gathered}$ | 226 | $\begin{gathered} \hline 160 \\ 66 \end{gathered}$ | $\begin{aligned} & \hline 71 \% \\ & 29 \% \end{aligned}$ | 700 | $\begin{aligned} & 530 \\ & 170 \end{aligned}$ | $\begin{aligned} & \hline 76 \% \\ & 24 \% \end{aligned}$ | 926 | $\begin{aligned} & 690 \\ & 236 \end{aligned}$ | $\begin{aligned} & 75 \% \\ & 25 \% \end{aligned}$ |
| Sat 26-Jan-08 MD Peak | 239 | $\begin{aligned} & \hline 125 \\ & 114 \end{aligned}$ | $\begin{aligned} & \hline 52 \% \\ & 48 \% \end{aligned}$ | 764 | $\begin{aligned} & 380 \\ & 384 \end{aligned}$ | $\begin{aligned} & \hline 50 \% \\ & 50 \% \end{aligned}$ | 1003 | $\begin{aligned} & 505 \\ & 498 \end{aligned}$ | $\begin{aligned} & 50 \% \\ & 50 \% \end{aligned}$ |

Note: Split data obtained directly from MetroCount data files


## Appendix D

SIDRA INTERSECTION Results

13S1231000 - West Culburra Subdivision
Culburra Road-Coonamia Road
Friday AM (0800-0900) - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Coonamia Rd |  |  |  |  |  |  |  |  |  |  |
| 1 L | 165 | 0.0 | 0.223 | 13.6 | LOS A | 0.7 | 4.9 | 0.37 | 0.74 | 62.3 |
| 3 R | 73 | 6.5 | 0.124 | 15.7 | LOS B | 0.5 | 3.6 | 0.46 | 0.78 | 59.9 |
| Approach | 238 | 2.0 | 0.223 | 14.3 | LOS A | 0.7 | 4.9 | 0.40 | 0.75 | 61.5 |
| East: Culburra Rd (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 40 | 2.9 | 0.022 | 11.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.74 | 63.3 |
| 5 T | 225 | 3.7 | 0.118 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 265 | 3.6 | 0.118 | 1.7 | NA | 0.0 | 0.0 | 0.00 | 0.11 | 76.6 |
| West: Culburra Rd (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 96 | 12.3 | 0.053 | 0.0 | X | X | X | X | 0.00 | 80.0 |
| 12 R | 33 | 3.6 | 0.029 | 12.5 | LOS A | 0.1 | 0.8 | 0.35 | 0.69 | 61.6 |
| Approach | 128 | 10.1 | 0.053 | 3.2 | NA | 0.1 | 0.8 | 0.09 | 0.17 | 73.9 |
| All Vehicles | 632 | 4.3 | 0.223 | 6.8 | NA | 0.7 | 4.9 | 0.17 | 0.37 | 69.8 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Culburra Road-Coonamia Road
Friday PM (1600-1700) - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Coonamia Rd |  |  |  |  |  |  |  |  |  |  |
| 1 L | 77 | 7.7 | 0.106 | 13.5 | LOS A | 0.3 | 2.1 | 0.27 | 0.70 | 62.9 |
| 3 R | 75 | 2.0 | 0.130 | 15.7 | LOS B | 0.5 | 3.7 | 0.48 | 0.79 | 59.5 |
| Approach | 152 | 4.9 | 0.130 | 14.6 | LOS B | 0.5 | 3.7 | 0.37 | 0.75 | 61.2 |
| East: Culburra Rd (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 75 | 2.0 | 0.041 | 11.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.74 | 63.3 |
| 5 T | 115 | 2.6 | 0.060 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 189 | 2.4 | 0.060 | 4.5 | NA | 0.0 | 0.0 | 0.00 | 0.29 | 71.9 |
| West: Culburra Rd (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 280 | 1.1 | 0.145 | 0.0 | X | X | X | X | 0.00 | 80.0 |
| 12 R | 152 | 3.9 | 0.133 | 12.3 | LOS A | 0.5 | 3.8 | 0.31 | 0.70 | 61.8 |
| Approach | 432 | 2.1 | 0.145 | 4.3 | NA | 0.5 | 3.8 | 0.11 | 0.24 | 71.9 |
| All Vehicles | 773 | 2.7 | 0.145 | 6.4 | NA | 0.5 | 3.8 | 0.13 | 0.35 | 69.7 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Culburra Road-Coonamia Road
Saturday - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Coonamia Rd |  |  |  |  |  |  |  |  |  |  |
| 1 L | 104 | 1.3 | 0.140 | 13.4 | LOS A | 0.4 | 2.9 | 0.32 | 0.72 | 62.6 |
| 3 R | 66 | 0.0 | 0.117 | 15.6 | LOS B | 0.5 | 3.2 | 0.48 | 0.79 | 59.4 |
| Approach | 171 | 0.8 | 0.140 | 14.3 | LOS A | 0.5 | 3.2 | 0.38 | 0.75 | 61.3 |
| East: Culburra Rd (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 80 | 0.0 | 0.043 | 11.3 | LOS A | 0.0 | 0.0 | 0.00 | 0.73 | 63.3 |
| 5 T | 169 | 0.8 | 0.087 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 249 | 0.5 | 0.087 | 3.6 | NA | 0.0 | 0.0 | 0.00 | 0.24 | 73.3 |
| West: Culburra Rd (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 166 | 0.8 | 0.086 | 0.0 | X | X | X | X | 0.00 | 80.0 |
| 12 R | 104 | 2.5 | 0.092 | 12.4 | LOS A | 0.4 | 2.7 | 0.35 | 0.71 | 61.6 |
| Approach | 271 | 1.5 | 0.092 | 4.8 | NA | 0.4 | 2.7 | 0.13 | 0.27 | 71.2 |
| All Vehicles | 691 | 1.0 | 0.140 | 6.7 | NA | 0.5 | 3.2 | 0.15 | 0.38 | 69.3 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

Culburra Road-Mayfield Road
Friday AM (0800-0900) - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Culburra Road (S) ${ }^{\text {c }}$ |  |  |  |  |  |  |  |  |  |  |
| L | 2 | 0.0 | 0.198 | 10.1 | LOS A | 0.0 | 0.0 | 0.00 | 1.73 | 57.1 |
| 2 T | 383 | 0.0 | 0.198 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 385 | 0.0 | 0.198 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 79.8 |
| North: Culburra Road ( N ) |  |  |  |  |  |  |  |  |  |  |
| 8 T | 126 | 0.0 | 0.066 | 1.9 | LOS A | 0.5 | 3.6 | 0.50 | 0.00 | 64.7 |
| 9 R | 1 | 0.0 | 0.066 | 12.0 | LOS A | 0.5 | 3.6 | 0.50 | 1.34 | 60.6 |
| Approach | 127 | 0.0 | 0.066 | 1.9 | NA | 0.5 | 3.6 | 0.50 | 0.01 | 64.7 |
| West: Mayfield Road |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 0.0 | 0.007 | 14.0 | LOS A | 0.0 | 0.2 | 0.53 | 0.65 | 46.2 |
| 12 R | 2 | 0.0 | 0.007 | 14.0 | LOS A | 0.0 | 0.2 | 0.53 | 0.74 | 46.4 |
| Approach | 3 | 0.0 | 0.007 | 14.0 | LOS A | 0.0 | 0.2 | 0.53 | 0.71 | 46.3 |
| All Vehicles | 516 | 0.0 | 0.198 | 0.6 | NA | 0.5 | 3.6 | 0.13 | 0.01 | 75.2 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

Culburra Road-Mayfield Road
Friday PM (1600-1700) - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue <br> Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Culburra Road (S) 0 |  |  |  |  |  |  |  |  |  |  |
| L | 1 | 0.0 | 0.102 | 10.1 | LOS A | 0.0 | 0.0 | 0.00 | 1.73 | 57.1 |
| 2 T | 193 | 4.6 | 0.102 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 194 | 4.6 | 0.102 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 79.8 |
| North: Culburra Road ( N ) |  |  |  |  |  |  |  |  |  |  |
| 8 T | 412 | 1.8 | 0.214 | 1.0 | LOS A | 1.6 | 11.4 | 0.39 | 0.00 | 67.7 |
| 9 R | 1 | 0.0 | 0.214 | 11.1 | LOS A | 1.6 | 11.4 | 0.39 | 1.45 | 60.0 |
| Approach | 413 | 1.8 | 0.214 | 1.0 | NA | 1.6 | 11.4 | 0.39 | 0.00 | 67.6 |
| West: Mayfield Road |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 0.0 | 0.019 | 38.1 | LOS C | 0.1 | 0.6 | 0.71 | 0.58 | 31.7 |
| 12 R | 1 | 100.0 | 0.019 | 43.5 | LOS D | 0.1 | 0.6 | 0.71 | 0.93 | 33.8 |
| Approach | 2 | 50.0 | 0.019 | 40.8 | LOS C | 0.1 | 0.6 | 0.71 | 0.75 | 32.9 |
| All Vehicles | 608 | 2.9 | 0.214 | 0.8 | NA | 1.6 | 11.4 | 0.27 | 0.01 | 70.8 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

Culburra Road-Mayfield Road
Saturday - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Culburra Road (S) 0 der |  |  |  |  |  |  |  |  |  |  |
| L | 3 | 0.0 | 0.144 | 10.1 | LOS A | 0.0 | 0.0 | 0.00 | 1.71 | 57.1 |
| 2 T | 277 | 0.5 | 0.144 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 280 | 0.5 | 0.144 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.02 | 79.7 |
| North: Culburra Road ( N ) |  |  |  |  |  |  |  |  |  |  |
| 8 T | 248 | 0.5 | 0.129 | 1.3 | LOS A | 1.0 | 6.8 | 0.44 | 0.00 | 66.2 |
| 9 R | 1 | 0.0 | 0.129 | 11.5 | LOS A | 1.0 | 6.8 | 0.44 | 1.40 | 60.4 |
| Approach | 249 | 0.5 | 0.129 | 1.4 | NA | 1.0 | 6.8 | 0.44 | 0.01 | 66.2 |
| West: Mayfield Road |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 0.0 | 0.009 | 14.5 | LOS B | 0.0 | 0.2 | 0.53 | 0.62 | 45.8 |
| 12 R | 3 | 0.0 | 0.009 | 14.5 | LOS B | 0.0 | 0.2 | 0.53 | 0.75 | 46.0 |
| Approach | 4 | 0.0 | 0.009 | 14.5 | LOS B | 0.0 | 0.2 | 0.53 | 0.72 | 45.9 |
| All Vehicles | 534 | 0.5 | 0.144 | 0.8 | NA | 1.0 | 6.8 | 0.21 | 0.02 | 72.4 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Pyree Lane
Friday AM (0800-0900) - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Pyreen Ln |  |  |  |  |  |  |  |  |  |  |
| 1 L | 376 | 1.9 | 0.205 | 11.2 | X | X | X | X | 0.69 | 58.8 |
| 3 R | 14 | 8.3 | 0.020 | 13.0 | LOS A | 0.1 | 0.6 | 0.34 | 0.69 | 56.9 |
| Approach | 389 | 2.1 | 0.205 | 11.2 | LOS A | 0.1 | 0.6 | 0.01 | 0.69 | 58.8 |
| East: Greenwell Pt Rd (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 20 | 66.7 | 0.094 | 14.9 | LOS B | 0.0 | 0.0 | 0.00 | 1.42 | 58.9 |
| 5 T | 147 | 5.6 | 0.094 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 167 | 12.9 | 0.094 | 1.8 | NA | 0.0 | 0.0 | 0.00 | 0.17 | 76.8 |
| West: Greenwell Pt Rd (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 63 | 5.6 | 0.034 | 0.0 | X | X | X | X | 0.00 | 80.0 |
| 12 R | 111 | 7.4 | 0.224 | 16.4 | LOS B | 0.9 | 6.8 | 0.56 | 0.85 | 52.5 |
| Approach | 174 | 6.8 | 0.224 | 10.4 | LOS A | 0.9 | 6.8 | 0.35 | 0.54 | 60.1 |
| All Vehicles | 731 | 5.7 | 0.224 | 8.9 | NA | 0.9 | 6.8 | 0.09 | 0.54 | 62.5 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Pyree Lane
Friday PM (1600-1700) - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \text { v/c } \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Pyreen Ln |  |  |  |  |  |  |  |  |  |  |
| 1 L | 155 | 5.8 | 0.087 | 11.3 | X | X | X | X | 0.69 | 58.9 |
| 3 R | 32 | 0.0 | 0.039 | 11.5 | LOS A | 0.2 | 1.1 | 0.23 | 0.68 | 58.0 |
| Approach | 186 | 4.8 | 0.087 | 11.4 | LOS A | 0.2 | 1.1 | 0.04 | 0.69 | 58.7 |
| East: Greenwell Pt Rd (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 26 | 0.0 | 0.050 | 10.9 | LOS A | 0.0 | 0.0 | 0.00 | 1.07 | 58.9 |
| 5 T | 68 | 4.3 | 0.050 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 95 | 3.1 | 0.050 | 3.0 | NA | 0.0 | 0.0 | 0.00 | 0.30 | 72.9 |
| West: Greenwell Pt Rd (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 173 | 1.7 | 0.090 | 0.0 | X | X | X | X | 0.00 | 80.0 |
| 12 R | 391 | 1.5 | 0.558 | 15.5 | LOS B | 5.2 | 36.6 | 0.56 | 0.82 | 53.1 |
| Approach | 563 | 1.6 | 0.558 | 10.8 | LOS A | 5.2 | 36.6 | 0.39 | 0.57 | 59.3 |
| All Vehicles | 844 | 2.5 | 0.558 | 10.0 | NA | 5.2 | 36.6 | 0.27 | 0.56 | 60.4 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Pyree Lane
Friday AM (0800-0900) - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Pyreen Ln |  |  |  |  |  |  |  |  |  |  |
| 1 L | 226 | 1.2 | 0.123 | 11.1 | X | X | X | X | 0.69 | 58.9 |
| 3 R | 45 | 0.0 | 0.065 | 12.5 | LOS A | 0.3 | 1.8 | 0.35 | 0.71 | 56.7 |
| Approach | 272 | 1.0 | 0.123 | 11.3 | LOS A | 0.3 | 1.8 | 0.06 | 0.69 | 58.5 |
| East: Greenwell Pt Rd (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 145 | 2.7 | 0.132 | 11.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.89 | 58.9 |
| 5 T | 100 | 3.9 | 0.132 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 245 | 3.2 | 0.132 | 6.6 | NA | 0.0 | 0.0 | 0.00 | 0.53 | 66.1 |
| West: Greenwell Pt Rd (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 227 | 1.2 | 0.117 | 0.0 | X | X | X | X | 0.00 | 80.0 |
| 12 R | 109 | 1.2 | 0.212 | 15.6 | LOS B | 0.9 | 6.1 | 0.54 | 0.84 | 53.0 |
| Approach | 337 | 1.2 | 0.212 | 5.1 | LOS A | 0.9 | 6.1 | 0.18 | 0.27 | 68.7 |
| All Vehicles | 854 | 1.7 | 0.212 | 7.5 | NA | 0.9 | 6.1 | 0.09 | 0.48 | 64.4 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Jindy Andy Lane
Friday AM (0800-0900) - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| North East: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |
| 25 T | 371 | 4.8 | 0.196 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| 26 R | 159 | 0.7 | 0.227 | 10.3 | LOS A | 1.0 | 7.1 | 0.39 | 0.70 | 46.7 |
| Approach | 529 | 3.6 | 0.227 | 3.1 | NA | 1.0 | 7.1 | 0.12 | 0.21 | 67.0 |
| North West: Jindy Andy Lane |  |  |  |  |  |  |  |  |  |  |
| 27 L | 39 | 3.0 | 0.038 | 11.8 | LOS A | 0.1 | 1.0 | 0.25 | 0.67 | 57.5 |
| 29 R | 12 | 30.0 | 0.077 | 30.3 | LOS C | 0.2 | 1.8 | 0.76 | 0.94 | 41.4 |
| Approach | 51 | 9.2 | 0.077 | 16.0 | LOS B | 0.2 | 1.8 | 0.37 | 0.74 | 52.8 |
| South West: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |
| 30 L | 8 | 42.9 | 0.079 | 13.5 | LOS A | 0.0 | 0.0 | 0.00 | 1.45 | 58.9 |
| 31 T | 136 | 8.7 | 0.079 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 144 | 10.7 | 0.079 | 0.8 | NA | 0.0 | 0.0 | 0.00 | 0.08 | 78.4 |
| All Vehicles | 724 | 5.4 | 0.227 | 3.5 | NA | 1.0 | 7.1 | 0.11 | 0.22 | 67.7 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Jindy Andy Lane
Friday AM (1600-1700) - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| North East: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |
| 25 T | 175 | 5.9 | 0.093 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| 26 R | 61 | 2.4 | 0.150 | 15.2 | LOS B | 0.6 | 4.0 | 0.59 | 0.87 | 42.3 |
| Approach | 236 | 5.0 | 0.150 | 3.9 | NA | 0.6 | 4.0 | 0.15 | 0.23 | 66.2 |
| North West: Jindy Andy Lane |  |  |  |  |  |  |  |  |  |  |
| 27 L | 160 | 1.9 | 0.214 | 13.8 | LOS A | 0.8 | 5.8 | 0.50 | 0.82 | 55.2 |
| 29 R | 3 | 0.0 | 0.011 | 19.3 | LOS B | 0.0 | 0.2 | 0.64 | 0.80 | 49.0 |
| Approach | 163 | 1.8 | 0.214 | 13.9 | LOS A | 0.8 | 5.8 | 0.50 | 0.82 | 55.0 |
| South West: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |
| 30 L | 14 | 11.1 | 0.215 | 11.6 | LOS A | 0.0 | 0.0 | 0.00 | 1.36 | 58.9 |
| 31 T | 399 | 1.9 | 0.215 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 413 | 2.2 | 0.215 | 0.4 | NA | 0.0 | 0.0 | 0.00 | 0.05 | 79.1 |
| All Vehicles | 812 | 2.9 | 0.215 | 4.1 | NA | 0.8 | 5.8 | 0.15 | 0.25 | 69.2 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Jindy Andy Lane
Saturday - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | $\begin{aligned} & \text { Deg. } \\ & \text { Satn } \\ & \text { v/c } \\ & \hline \end{aligned}$ | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| North East: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |
| 25 T | 254 | 3.1 | 0.133 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| 26 R | 96 | 1.4 | 0.181 | 12.6 | LOS A | 0.7 | 5.1 | 0.53 | 0.80 | 44.5 |
| Approach | 349 | 2.6 | 0.181 | 3.5 | NA | 0.7 | 5.1 | 0.14 | 0.22 | 66.8 |
| North West: Jindy Andy Lane |  |  |  |  |  |  |  |  |  |  |
| 27 L | 100 | 0.0 | 0.115 | 12.6 | LOS A | 0.4 | 3.0 | 0.39 | 0.74 | 56.6 |
| 29 R | 9 | 0.0 | 0.034 | 19.5 | LOS B | 0.1 | 0.7 | 0.65 | 0.86 | 48.7 |
| Approach | 109 | 0.0 | 0.115 | 13.2 | LOS A | 0.4 | 3.0 | 0.41 | 0.75 | 55.8 |
| South West: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |
| 30 L | 5 | 0.0 | 0.154 | 10.9 | LOS A | 0.0 | 0.0 | 0.00 | 1.33 | 58.9 |
| 31 T | 288 | 3.2 | 0.154 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 294 | 3.1 | 0.154 | 0.2 | NA | 0.0 | 0.0 | 0.00 | 0.02 | 79.5 |
| All Vehicles | 753 | 2.4 | 0.181 | 3.6 | NA | 0.7 | 5.1 | 0.13 | 0.22 | 69.2 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Mayfield Road
Friday AM (0800-0900) - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles <br> veh | Queue <br> Distance <br> m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South East: Mayfield Road |  |  |  |  |  |  |  |  |  |  |
| 21 L | 12 | 0.0 | 0.018 | 11.6 | LOS A | 0.1 | 0.4 | 0.45 | 0.69 | 48.6 |
| 23 R | 1 | 0.0 | 0.018 | 11.6 | LOS A | 0.1 | 0.4 | 0.45 | 0.78 | 48.7 |
| Approach | 13 | 0.0 | 0.018 | 11.6 | LOS A | 0.1 | 0.4 | 0.45 | 0.70 | 48.6 |
| North East: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |
| 24 L | 3 | 0.0 | 0.200 | 10.1 | LOS A | 0.0 | 0.0 | 0.00 | 1.72 | 57.1 |
| 25 T | 377 | 3.8 | 0.200 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 380 | 3.7 | 0.200 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 79.8 |
| South West: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |
| 31 T | 141 | 9.2 | 0.089 | 4.9 | LOS A | 1.2 | 8.7 | 0.68 | 0.00 | 60.3 |
| 32 R | 6 | 0.0 | 0.089 | 15.1 | LOS B | 1.2 | 8.7 | 0.68 | 1.17 | 57.6 |
| Approach | 147 | 8.8 | 0.089 | 5.4 | NA | 1.2 | 8.7 | 0.68 | 0.05 | 60.2 |
| All Vehicles | 540 | 5.0 | 0.200 | 1.8 | NA | 1.2 | 8.7 | 0.20 | 0.04 | 72.3 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Mayfield Road
Friday PM (1600-1700) - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles <br> veh | Queue <br> Distance <br> m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South East: Mayfield Road |  |  |  |  |  |  |  |  |  |  |
| 21 L | 6 | 0.0 | 0.035 | 16.7 | LOS B | 0.1 | 0.9 | 0.50 | 0.62 | 44.0 |
| 23 R | 6 | 25.0 | 0.035 | 18.0 | LOS B | 0.1 | 0.9 | 0.50 | 0.86 | 44.2 |
| Approach | 13 | 12.5 | 0.035 | 17.4 | LOS B | 0.1 | 0.9 | 0.50 | 0.74 | 44.1 |
| North East: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |
| 24 L | 1 | 0.0 | 0.097 | 10.1 | LOS A | 0.0 | 0.0 | 0.00 | 1.73 | 57.1 |
| 25 T | 181 | 5.7 | 0.097 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 182 | 5.7 | 0.097 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 79.8 |
| South West: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |
| 31 T | 413 | 1.4 | 0.235 | 2.4 | LOS A | 2.9 | 20.6 | 0.57 | 0.00 | 63.0 |
| 32 R | 15 | 0.0 | 0.235 | 12.6 | LOS A | 2.9 | 20.6 | 0.57 | 1.21 | 60.3 |
| Approach | 427 | 1.4 | 0.235 | 2.8 | NA | 2.9 | 20.6 | 0.57 | 0.04 | 62.9 |
| All Vehicles | 622 | 2.9 | 0.235 | 2.3 | NA | 2.9 | 20.6 | 0.40 | 0.05 | 66.5 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Mayfield Road
Saturday - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South East: Mayfield Road |  |  |  |  |  |  |  |  |  |  |
| 21 L | 8 | 0.0 | 0.012 | 11.1 | LOS A | 0.0 | 0.3 | 0.38 | 0.65 | 49.0 |
| 23 R | 1 | 0.0 | 0.012 | 11.1 | LOS A | 0.0 | 0.3 | 0.38 | 0.77 | 49.2 |
| Approach | 9 | 0.0 | 0.012 | 11.1 | LOS A | 0.0 | 0.3 | 0.38 | 0.66 | 49.0 |
| North East: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |
| 24 L | 4 | 66.7 | 0.139 | 13.0 | LOS A | 0.0 | 0.0 | 0.00 | 2.24 | 57.1 |
| 25 T | 263 | 1.0 | 0.139 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 267 | 2.0 | 0.139 | 0.2 | NA | 0.0 | 0.0 | 0.00 | 0.04 | 79.6 |
| South West: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |
| 31 T | 289 | 1.8 | 0.170 | 3.5 | LOS A | 2.2 | 15.4 | 0.64 | 0.00 | 61.3 |
| 32 R | 12 | 0.0 | 0.170 | 13.6 | LOS A | 2.2 | 15.4 | 0.64 | 1.18 | 59.4 |
| Approach | 301 | 1.7 | 0.170 | 3.9 | NA | 2.2 | 15.4 | 0.64 | 0.05 | 61.2 |
| All Vehicles | 578 | 1.9 | 0.170 | 2.3 | NA | 2.2 | 15.4 | 0.34 | 0.05 | 68.2 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Millbank Road-Worrigee Road
Friday AM (0800-0900) - Equivalent 120th HH
Existing
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Worrigee Road |  |  |  |  |  |  |  |  |  |  |
| 1 L | 69 | 5.1 | 0.391 | 20.4 | LOS B | 2.1 | 14.9 | 0.66 | 1.04 | 42.6 |
| 2 T | 118 | 3.0 | 0.391 | 19.0 | LOS B | 2.1 | 14.9 | 0.66 | 1.07 | 40.5 |
| 3 R | 21 | 0.0 | 0.076 | 20.6 | LOS B | 0.2 | 1.6 | 0.66 | 1.00 | 42.1 |
| Approach | 208 | 3.4 | 0.391 | 19.6 | LOS B | 2.1 | 14.9 | 0.66 | 1.05 | 41.4 |
| East: Greenwell Point Road (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 12 | 0.0 | 0.193 | 10.1 | LOS A | 0.0 | 0.0 | 0.00 | 1.65 | 57.1 |
| 5 T | 357 | 3.0 | 0.193 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| $6 \quad \mathrm{R}$ | 32 | 7.4 | 0.027 | 11.2 | LOS A | 0.1 | 0.8 | 0.30 | 0.66 | 55.3 |
| Approach | 400 | 3.2 | 0.193 | 1.2 | NA | 0.1 | 0.8 | 0.02 | 0.10 | 76.8 |
| North: Millbank Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 14 | 16.7 | 0.027 | 13.8 | LOS A | 0.1 | 0.4 | 0.29 | 0.86 | 48.2 |
| 8 T | 26 | 13.6 | 0.126 | 22.0 | LOS B | 0.5 | 3.5 | 0.70 | 1.00 | 38.8 |
| 9 R | 12 | 0.0 | 0.126 | 22.2 | LOS B | 0.5 | 3.5 | 0.70 | 1.00 | 41.1 |
| Approach | 52 | 11.4 | 0.126 | 19.9 | LOS B | 0.5 | 3.5 | 0.59 | 0.96 | 41.6 |
| West: Greenwell Point Road (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 47 | 7.5 | 0.105 | 10.4 | LOS A | 0.0 | 0.0 | 0.00 | 1.24 | 57.1 |
| 11 T | 143 | 9.1 | 0.105 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| 12 R | 61 | 5.8 | 0.063 | 12.1 | LOS A | 0.2 | 1.8 | 0.43 | 0.73 | 54.6 |
| Approach | 252 | 8.0 | 0.105 | 4.9 | NA | 0.2 | 1.8 | 0.10 | 0.41 | 68.1 |
| All Vehicles | 912 | 5.0 | 0.391 | 7.5 | NA | 2.1 | 14.9 | 0.22 | 0.45 | 60.5 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Millbank Road-Worrigee Road
Friday PM (1600-1700) - Equivalent 120th HH
Existing
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Worrigee Road |  |  |  |  |  |  |  |  |  |  |
| 1 L | 58 | 0.0 | 0.197 | 16.8 | LOS B | 0.8 | 5.3 | 0.46 | 0.84 | 44.8 |
| 2 T | 47 | 0.0 | 0.197 | 15.6 | LOS B | 0.8 | 5.3 | 0.46 | 1.00 | 42.7 |
| 3 R | 21 | 5.3 | 0.086 | 25.1 | LOS B | 0.3 | 2.1 | 0.74 | 1.00 | 39.4 |
| Approach | 126 | 0.9 | 0.197 | 17.7 | LOS B | 0.8 | 5.3 | 0.51 | 0.93 | 43.0 |
| East: Greenwell Point Road (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 21 | 7.1 | 0.099 | 10.4 | LOS A | 0.0 | 0.0 | 0.00 | 1.48 | 57.1 |
| 5 T | 163 | 5.5 | 0.099 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| 6 R | 14 | 0.0 | 0.015 | 12.0 | LOS A | 0.1 | 0.4 | 0.45 | 0.70 | 54.3 |
| Approach | 198 | 5.3 | 0.099 | 1.9 | NA | 0.1 | 0.4 | 0.03 | 0.21 | 75.0 |
| North: Millbank Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 22 | 0.0 | 0.042 | 14.1 | LOS A | 0.1 | 0.7 | 0.45 | 0.89 | 47.2 |
| 8 T | 54 | 2.8 | 0.235 | 22.5 | LOS B | 0.9 | 6.6 | 0.74 | 1.02 | 38.0 |
| 9 R | 18 | 0.0 | 0.235 | 23.4 | LOS B | 0.9 | 6.6 | 0.74 | 1.02 | 40.3 |
| Approach | 94 | 1.6 | 0.235 | 20.7 | LOS B | 0.9 | 6.6 | 0.67 | 0.99 | 40.5 |
| West: Greenwell Point Road (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 17 | 0.0 | 0.220 | 10.1 | LOS A | 0.0 | 0.0 | 0.00 | 1.62 | 57.1 |
| 11 T | 409 | 0.7 | 0.220 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| 12 R | 115 | 1.3 | 0.095 | 10.9 | LOS A | 0.4 | 2.8 | 0.30 | 0.68 | 55.3 |
| Approach | 541 | 0.8 | 0.220 | 2.6 | NA | 0.4 | 2.8 | 0.06 | 0.19 | 73.0 |
| All Vehicles | 959 | 1.8 | 0.235 | 6.2 | NA | 0.9 | 6.6 | 0.18 | 0.37 | 63.0 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Millbank Road-Worrigee Road
Saturday - Equivalent 120th HH
Existing
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Worrigee Road |  |  |  |  |  |  |  |  |  |  |
| L | 77 | 0.0 | 0.153 | 14.7 | LOS B | 0.6 | 4.2 | 0.45 | 0.88 | 46.6 |
| 2 T | 29 | 4.5 | 0.153 | 13.8 | LOS A | 0.6 | 4.2 | 0.45 | 1.00 | 44.5 |
| 3 R | 38 | 3.4 | 0.133 | 20.3 | LOS B | 0.4 | 2.8 | 0.64 | 1.00 | 42.5 |
| Approach | 144 | 1.8 | 0.153 | 16.0 | LOS B | 0.6 | 4.2 | 0.50 | 0.94 | 45.0 |
| East: Greenwell Point Road (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 20 | 6.7 | 0.136 | 10.4 | LOS A | 0.0 | 0.0 | 0.00 | 1.57 | 57.1 |
| 5 T | 241 | 1.1 | 0.136 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| $6 \quad \mathrm{R}$ | 16 | 0.0 | 0.014 | 11.2 | LOS A | 0.1 | 0.4 | 0.35 | 0.67 | 55.0 |
| Approach | 277 | 1.4 | 0.136 | 1.4 | NA | 0.1 | 0.4 | 0.02 | 0.15 | 76.3 |
| North: Millbank Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 8 | 16.7 | 0.018 | 14.4 | LOS A | 0.0 | 0.3 | 0.37 | 0.85 | 47.7 |
| 8 T | 24 | 11.1 | 0.116 | 20.0 | LOS B | 0.4 | 3.2 | 0.66 | 1.00 | 40.0 |
| 9 R | 16 | 0.0 | 0.116 | 20.4 | LOS B | 0.4 | 3.2 | 0.66 | 1.00 | 42.3 |
| Approach | 48 | 8.5 | 0.116 | 19.2 | LOS B | 0.4 | 3.2 | 0.61 | 0.97 | 42.1 |
| West: Greenwell Point Road (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 21 | 6.2 | 0.142 | 10.3 | LOS A | 0.0 | 0.0 | 0.00 | 1.56 | 57.1 |
| 11 T | 253 | 1.0 | 0.142 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| 12 R | 54 | 0.0 | 0.048 | 11.2 | LOS A | 0.2 | 1.3 | 0.35 | 0.68 | 55.0 |
| Approach | 327 | 1.2 | 0.142 | 2.5 | NA | 0.2 | 1.3 | 0.06 | 0.21 | 73.4 |
| All Vehicles | 797 | 1.8 | 0.153 | 5.6 | NA | 0.6 | 4.2 | 0.16 | 0.37 | 64.3 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Princes Highway-Kalandar Street
Friday AM (0800-0900) - Equivalent 120th HH
Existing
Signals - Fixed Time Cycle Time = 135 seconds (Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |
| 1 L | 3 | 0.0 | 1.049 | 103.7 | LOS F | 39.3 | 288.0 | 1.00 | 1.17 | 16.8 |
| 2 T | 962 | 5.6 | 1.049 | 111.7 | LOS F | 49.4 | 362.6 | 1.00 | 1.25 | 15.6 |
| 3 R | 38 | 0.0 | 0.459 | 81.6 | LOS F | 2.6 | 18.3 | 1.00 | 0.73 | 19.2 |
| Approach | 1003 | 5.3 | 1.049 | 110.6 | LOS F | 49.4 | 362.6 | 1.00 | 1.23 | 15.7 |
| East: Kalandar St (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 38 | 9.4 | 1.039 | 115.9 | LOS F | 51.4 | 370.2 | 1.00 | 1.23 | 10.7 |
| 5 T | 306 | 3.1 | 1.039 | 106.6 | LOS F | 51.4 | 370.2 | 1.00 | 1.23 | 10.1 |
| $6 \quad \mathrm{R}$ | 782 | 2.9 | 1.039 | 129.2 | LOS F | 56.2 | 403.3 | 1.00 | 1.21 | 9.7 |
| Approach | 1126 | 3.1 | 1.039 | 122.6 | LOS F | 56.2 | 403.3 | 1.00 | 1.22 | 9.8 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 259 | 7.3 | 0.261 | 9.2 | LOS A | 0.8 | 5.7 | 0.07 | 0.64 | 53.4 |
| 8 T | 898 | 7.3 | 0.734 | 40.8 | LOS C | 24.3 | 180.8 | 0.88 | 0.78 | 29.9 |
| 9 R | 157 | 11.3 | 1.042 | 112.3 | LOS F | 12.0 | 91.7 | 1.00 | 1.09 | 15.2 |
| Approach | 1314 | 7.8 | 1.042 | 43.1 | LOS D | 24.3 | 180.8 | 0.74 | 0.79 | 29.1 |
| West: Kalandar St (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 52 | 13.6 | 0.700 | 65.8 | LOS E | 9.0 | 66.1 | 0.95 | 0.90 | 19.0 |
| 11 T | 149 | 1.6 | 0.700 | 57.2 | LOS E | 12.8 | 92.9 | 0.97 | 0.86 | 18.0 |
| 12 R | 156 | 5.3 | 0.700 | 69.3 | LOS E | 12.8 | 92.9 | 1.00 | 0.85 | 17.9 |
| Approach | 357 | 4.9 | 0.700 | 63.7 | LOS E | 12.8 | 92.9 | 0.98 | 0.86 | 18.1 |
| All Vehicles | 3800 | 5.5 | 1.049 | 86.4 | LOS F | 56.2 | 403.3 | 0.91 | 1.04 | 16.8 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

| Movement Performance - Pedestrians |  |  |  |  |  |  |  |  |
| :---: | :---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Mov ID | Description | Demand <br> Flow <br> ped/h | Average <br> Delay <br> sec | Level of <br> Service | Average Back of Queue <br> Pedestrian <br> ped | Prop. <br> Distance <br> Queued | Effective <br> Stop Rate <br> per ped |  |
| P1 | Across S approach | 53 | 45.6 | LOS E | 0.2 | 0.2 | 0.82 | 0.82 |
| P3 | Across E approach | 53 | 37.0 | LOS D | 0.2 | 0.2 | 0.74 | 0.74 |
| P5 | Across N approach | 53 | 61.6 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 |
| All Pedestrians | 159 | 48.1 | LOS E |  |  | 0.84 | 0.84 |  |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

13S1231000 - West Culburra Subdivision
Princes Highway-Kalandar Street
Friday PM (1600-1700) - Equivalent 120th HH
Existing
Signals - Fixed Time Cycle Time $=135$ seconds (Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue <br> Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |
| 1 L | 12 | 0.0 | 1.038 | 100.6 | LOS F | 29.4 | 210.2 | 1.00 | 1.12 | 17.2 |
| 2 T | 785 | 2.6 | 1.038 | 107.4 | LOS F | 39.3 | 281.0 | 1.00 | 1.20 | 16.1 |
| 3 R | 109 | 1.4 | 1.004 | 120.5 | LOS F | 9.8 | 69.4 | 1.00 | 1.08 | 14.3 |
| Approach | 906 | 2.4 | 1.038 | 108.9 | LOS F | 39.3 | 281.0 | 1.00 | 1.18 | 15.8 |
| East: Kalandar St (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 99 | 1.5 | 1.065 | 132.7 | LOS F | 42.9 | 306.0 | 1.00 | 1.27 | 9.5 |
| 5 T | 245 | 2.4 | 1.065 | 123.7 | LOS F | 42.9 | 306.0 | 1.00 | 1.27 | 8.9 |
| $6 \quad \mathrm{R}$ | 567 | 1.8 | 1.065 | 151.3 | LOS F | 47.2 | 335.4 | 1.00 | 1.26 | 8.5 |
| Approach | 912 | 2.0 | 1.065 | 141.9 | LOS F | 47.2 | 335.4 | 1.00 | 1.26 | 8.7 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 815 | 1.1 | $1.000^{3}$ | 35.7 | LOS C | 18.5 | 130.6 | 0.21 | 0.77 | 32.5 |
| 8 T | 1318 | 3.1 | 1.100 | 162.7 | LOS F | 77.6 | 557.9 | 1.00 | 1.54 | 11.6 |
| 9 R | 138 | 5.4 | 0.587 | 44.5 | LOS D | 6.1 | 44.8 | 0.99 | 0.79 | 28.6 |
| Approach | 2272 | 2.5 | 1.100 | 109.9 | LOS F | 77.6 | 557.9 | 0.71 | 1.22 | 15.7 |
| West: Kalandar St (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 109 | 9.5 | $1.000^{3}$ | 56.8 | LOS E | 13.7 | 99.1 | 0.93 | 0.90 | 21.0 |
| 11 T | 305 | 0.0 | 1.101 | 113.6 | LOS F | 55.2 | 388.8 | 0.97 | 1.13 | 11.0 |
| 12 R | 303 | 1.0 | 1.101 | 185.3 | LOS F | 55.2 | 388.8 | 1.00 | 1.41 | 8.1 |
| Approach | 718 | 1.9 | 1.101 | 135.2 | LOS F | 55.2 | 388.8 | 0.97 | 1.22 | 10.2 |
| All Vehicles | 4807 | 2.3 | 1.101 | 119.6 | LOS F | 77.6 | 557.9 | 0.86 | 1.22 | 13.4 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.
$3 x=1.00$ due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

| Movement Performance - Pedestrians |  |  |  |  |  |  |  |  |
| :---: | :---: | ---: | :---: | :---: | :---: | ---: | ---: | ---: |
| Mov ID | Description | Demand <br> Flow <br> ped/h | Average <br> Delay <br> sec | Level of <br> Service | Average Back of Queue <br> Pedestrian <br> ped | Prop. <br> Distance <br> Queued | Effective <br> Stop Rate <br> per ped |  |
| P1 | Across S approach | 53 | 53.3 | LOS E | 0.2 | 0.2 | 0.89 | 0.89 |
| P3 | Across E approach | 53 | 38.5 | LOS D | 0.2 | 0.2 | 0.76 | 0.76 |
| P5 $\quad$ Across N approach | 53 | 53.3 | LOS E | 0.2 | 0.2 | 0.89 | 0.89 |  |
| All Pedestrians | 159 | 48.4 | LOS E |  |  | 0.84 | 0.84 |  |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

13S1231000 - West Culburra Subdivision
Princes Highway-Kalandar Street
Saturday - Equivalent 120th HH
Existing
Signals - Fixed Time Cycle Time $=135$ seconds (Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Princes Hwy (S) [ich per veh |  |  |  |  |  |  |  |  |  |  |
| 1 L | 5 | 25.0 | 0.872 | 74.6 | LOS F | 32.0 | 229.4 | 0.99 | 0.98 | 21.9 |
| 2 T | 960 | 2.6 | 0.872 | 60.0 | LOS E | 32.0 | 229.4 | 0.99 | 0.96 | 23.9 |
| 3 R | 116 | 4.5 | 0.966 | 104.3 | LOS F | 9.6 | 69.5 | 1.00 | 1.03 | 16.0 |
| Approach | 1081 | 2.9 | 0.966 | 64.8 | LOS E | 32.0 | 229.4 | 0.99 | 0.97 | 22.8 |
| East: Kalandar St (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 89 | 0.0 | 0.968 | 87.7 | LOS F | 35.3 | 248.4 | 1.00 | 1.15 | 13.4 |
| 5 T | 211 | 0.6 | 0.968 | 78.7 | LOS F | 35.3 | 248.4 | 1.00 | 1.15 | 12.6 |
| $6 \quad \mathrm{R}$ | 543 | 1.0 | 0.968 | 95.1 | LOS F | 35.3 | 248.4 | 1.00 | 1.09 | 12.5 |
| Approach | 843 | 0.8 | 0.968 | 90.3 | LOS F | 35.3 | 248.4 | 1.00 | 1.12 | 12.6 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 496 | 0.3 | 0.507 | 9.3 | LOS A | 2.0 | 14.2 | 0.08 | 0.65 | 53.1 |
| 8 T | 1386 | 1.0 | 0.961 | 65.3 | LOS E | 55.2 | 389.4 | 1.00 | 1.11 | 22.6 |
| $9 \quad \mathrm{R}$ | 91 | 11.6 | 0.482 | 42.7 | LOS D | 3.4 | 26.4 | 0.98 | 0.77 | 29.4 |
| Approach | 1973 | 1.3 | 0.961 | 50.2 | LOS D | 55.2 | 389.4 | 0.77 | 0.98 | 26.6 |
| West: Kalandar St (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 66 | 2.0 | 0.771 | 70.3 | LOS E | 10.8 | 76.1 | 0.96 | 0.96 | 18.0 |
| 11 T | 168 | 0.0 | 0.771 | 61.7 | LOS E | 15.0 | 105.2 | 0.98 | 0.92 | 17.1 |
| 12 R | 173 | 0.8 | 0.771 | 71.6 | LOS F | 15.0 | 105.2 | 1.00 | 0.88 | 17.5 |
| Approach | 407 | 0.6 | 0.771 | 67.3 | LOS E | 15.0 | 105.2 | 0.98 | 0.91 | 17.4 |
| All Vehicles | 4304 | 1.5 | 0.968 | 63.3 | LOS E | 55.2 | 389.4 | 0.89 | 1.00 | 21.5 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

| Movement Performance - Pedestrians |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back Pedestrian ped | Queue Distance m | Prop. Queued | Effective Stop Rate per ped |
| P1 | Across S approach | 53 | 53.3 | LOS E | 0.2 | 0.2 | 0.89 | 0.89 |
| P3 | Across E approach | 53 | 32.7 | LOS D | 0.1 | 0.1 | 0.70 | 0.70 |
| P5 | Across N approach | 53 | 61.6 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 |
| All Pedestrians |  | 159 | 49.2 | LOS E |  |  | 0.85 | 0.85 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

13S1231000 - West Culburra Subdivision
Coonamia Road- Currarong Road-Forest Road
Friday AM (0800-0900) - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| East: Currarong Road |  |  |  |  |  |  |  |  |  |  |
| 5 T | 13 | 0.0 | 0.041 | 0.9 | LOS A | 0.2 | 1.2 | 0.31 | 0.00 | 79.6 |
| 6 R | 40 | 0.0 | 0.041 | 13.3 | LOS A | 0.2 | 1.2 | 0.31 | 0.77 | 68.3 |
| Approach | 53 | 0.0 | 0.041 | 10.3 | NA | 0.2 | 1.2 | 0.31 | 0.59 | 70.8 |
| North: Coonamia Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 7 | 0.0 | 0.012 | 13.1 | LOS A | 0.0 | 0.2 | 0.22 | 0.68 | 67.6 |
| 9 R | 67 | 1.8 | 0.096 | 14.1 | LOS A | 0.4 | 2.7 | 0.35 | 0.73 | 66.8 |
| Approach | 75 | 1.6 | 0.096 | 14.0 | LOS A | 0.4 | 2.7 | 0.34 | 0.73 | 66.9 |
| West: Forest Road |  |  |  |  |  |  |  |  |  |  |
| 10 L | 197 | 1.8 | 0.117 | 12.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.79 | 69.1 |
| 11 T | 19 | 6.3 | 0.117 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 100.0 |
| Approach | 216 | 2.2 | 0.117 | 11.6 | NA | 0.0 | 0.0 | 0.00 | 0.72 | 71.1 |
| All Vehicles | 343 | 1.7 | 0.117 | 11.9 | NA | 0.4 | 2.7 | 0.12 | 0.70 | 70.1 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Coonamia Road- Currarong Road-Forest Road
Friday PM (1600-1700) - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| East: Currarong Road |  |  |  |  |  |  |  |  |  |  |
| 5 T | 15 | 0.0 | 0.020 | 0.6 | LOS A | 0.1 | 0.7 | 0.27 | 0.00 | 82.9 |
| 6 R | 15 | 0.0 | 0.020 | 13.1 | LOS A | 0.1 | 0.7 | 0.27 | 0.87 | 69.4 |
| Approach | 29 | 0.0 | 0.020 | 6.8 | NA | 0.1 | 0.7 | 0.27 | 0.43 | 75.7 |
| North: Coonamia Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 37 | 0.0 | 0.058 | 13.0 | LOS A | 0.1 | 0.8 | 0.20 | 0.69 | 67.7 |
| 9 R | 185 | 4.0 | 0.249 | 14.2 | LOS A | 1.1 | 8.3 | 0.35 | 0.73 | 67.1 |
| Approach | 222 | 3.3 | 0.249 | 14.0 | LOS A | 1.1 | 8.3 | 0.33 | 0.73 | 67.2 |
| West: Forest Road |  |  |  |  |  |  |  |  |  |  |
| 10 L | 137 | 5.4 | 0.094 | 13.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.84 | 69.1 |
| 11 T | 34 | 4.3 | 0.094 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 100.0 |
| Approach | 171 | 5.2 | 0.094 | 10.5 | NA | 0.0 | 0.0 | 0.00 | 0.67 | 73.7 |
| All Vehicles | 422 | 3.9 | 0.249 | 12.1 | NA | 1.1 | 8.3 | 0.19 | 0.68 | 70.3 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Coonamia Road- Currarong Road-Forest Road
Saturday - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue <br> Distance <br> m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| East: Currarong Road |  |  |  |  |  |  |  |  |  |  |
| 5 T | 11 | 0.0 | 0.027 | 0.6 | LOS A | 0.1 | 0.8 | 0.27 | 0.00 | 82.0 |
| $6 \quad \mathrm{R}$ | 26 | 0.0 | 0.027 | 13.1 | LOS A | 0.1 | 0.8 | 0.27 | 0.78 | 68.6 |
| Approach | 37 | 0.0 | 0.027 | 9.5 | NA | 0.1 | 0.8 | 0.27 | 0.56 | 72.0 |
| North: Coonamia Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 29 | 0.0 | 0.046 | 13.0 | LOS A | 0.1 | 0.7 | 0.19 | 0.69 | 67.8 |
| 9 R | 151 | 1.8 | 0.202 | 13.9 | LOS A | 0.9 | 6.3 | 0.34 | 0.73 | 67.2 |
| Approach | 180 | 1.5 | 0.202 | 13.7 | LOS A | 0.9 | 6.3 | 0.31 | 0.72 | 67.3 |
| West: Forest Road |  |  |  |  |  |  |  |  |  |  |
| 10 L | 152 | 0.9 | 0.092 | 12.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.80 | 69.1 |
| 11 T | 19 | 0.0 | 0.092 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 100.0 |
| Approach | 171 | 0.8 | 0.092 | 11.2 | NA | 0.0 | 0.0 | 0.00 | 0.71 | 71.6 |
| All Vehicles | 387 | 1.0 | 0.202 | 12.2 | NA | 0.9 | 6.3 | 0.17 | 0.70 | 69.6 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Kalandar Street-Kinghorne Street-Albatross Road
Friday AM (0800-0900) - Equivalent 120th HH
Existing
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue <br> Distance | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Kinghorne Sttreet |  |  |  |  |  |  |  |  |  |  |
| 1 L | 31 | 7.7 | 0.711 | 19.1 | LOS B | 9.6 | 69.3 | 0.97 | 1.10 | 40.2 |
| 2 T | 483 | 3.4 | 0.711 | 17.7 | LOS B | 9.6 | 69.3 | 0.97 | 1.09 | 40.3 |
| 3 R | 59 | 0.0 | 0.711 | 22.5 | LOS B | 9.6 | 69.3 | 0.97 | 1.10 | 38.4 |
| Approach | 573 | 3.3 | 0.711 | 18.2 | LOS B | 9.6 | 69.3 | 0.97 | 1.09 | 40.1 |
| East: Kalandar Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 315 | 7.5 | 0.422 | 7.9 | LOS A | 2.7 | 19.5 | 0.45 | 0.59 | 45.2 |
| 6 R | 156 | 2.3 | 0.422 | 12.6 | LOS A | 2.7 | 19.5 | 0.45 | 0.78 | 42.0 |
| Approach | 471 | 5.8 | 0.422 | 9.4 | LOS A | 2.7 | 19.5 | 0.45 | 0.65 | 44.0 |
| North: Kinghorne Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 127 | 3.7 | 0.327 | 8.8 | LOS A | 2.2 | 15.9 | 0.57 | 0.66 | 47.2 |
| 8 T | 31 | 0.0 | 0.327 | 7.8 | LOS A | 2.2 | 15.9 | 0.57 | 0.62 | 47.2 |
| 9 R | 176 | 3.4 | 0.327 | 11.9 | LOS A | 2.2 | 15.9 | 0.57 | 0.74 | 45.5 |
| Approach | 334 | 3.2 | 0.327 | 10.4 | LOS A | 2.2 | 15.9 | 0.57 | 0.70 | 46.2 |
| South West: Albatross Road |  |  |  |  |  |  |  |  |  |  |
| 30 L | 261 | 4.5 | 0.729 | 19.6 | LOS B | 9.3 | 68.4 | 1.00 | 1.18 | 38.6 |
| 32 R | 217 | 8.1 | 0.729 | 23.8 | LOS B | 9.3 | 68.4 | 1.00 | 1.18 | 37.3 |
| Approach | 478 | 6.2 | 0.729 | 21.5 | LOS B | 9.3 | 68.4 | 1.00 | 1.18 | 38.0 |
| All Vehicles | 1855 | 4.6 | 0.729 | 15.4 | LOS B | 9.6 | 69.3 | 0.77 | 0.93 | 41.3 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Kalandar Street-Kinghorne Street-Albatross Road
Friday AM (1600-1700) - Equivalent 120th HH
Existing
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Kinghorne Sttreet |  |  |  |  |  |  |  |  |  |  |
| 1 L | 34 | 0.0 | 0.576 | 15.2 | LOS B | 5.6 | 39.4 | 0.90 | 0.99 | 42.9 |
| 2 T | 326 | 0.0 | 0.576 | 13.9 | LOS A | 5.6 | 39.4 | 0.90 | 0.97 | 43.1 |
| 3 R | 77 | 0.0 | 0.576 | 18.8 | LOS B | 5.6 | 39.4 | 0.90 | 1.01 | 40.8 |
| Approach | 437 | 0.0 | 0.576 | 14.9 | LOS B | 5.6 | 39.4 | 0.90 | 0.98 | 42.6 |
| East: Kalandar Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 278 | 0.0 | 0.436 | 8.9 | LOS A | 2.8 | 19.8 | 0.61 | 0.71 | 44.0 |
| 6 R | 115 | 0.0 | 0.436 | 13.8 | LOS A | 2.8 | 19.8 | 0.61 | 0.84 | 40.9 |
| Approach | 393 | 0.0 | 0.436 | 10.3 | LOS A | 2.8 | 19.8 | 0.61 | 0.74 | 43.0 |
| North: Kinghorne Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 315 | 0.0 | 0.766 | 15.0 | LOS B | 10.8 | 75.8 | 0.96 | 1.01 | 42.2 |
| 8 T | 80 | 0.0 | 0.766 | 14.2 | LOS A | 10.8 | 75.8 | 0.96 | 1.01 | 42.3 |
| 9 R | 304 | 0.0 | 0.766 | 18.2 | LOS B | 10.8 | 75.8 | 0.96 | 1.02 | 40.7 |
| Approach | 699 | 0.0 | 0.766 | 16.3 | LOS B | 10.8 | 75.8 | 0.96 | 1.02 | 41.6 |
| South West: Albatross Road |  |  |  |  |  |  |  |  |  |  |
| 30 L | 300 | 0.0 | 0.774 | 16.7 | LOS B | 11.2 | 78.4 | 1.00 | 1.12 | 40.4 |
| 32 R | 341 | 0.0 | 0.774 | 20.9 | LOS B | 11.2 | 78.4 | 1.00 | 1.12 | 39.0 |
| Approach | 641 | 0.0 | 0.774 | 18.9 | LOS B | 11.2 | 78.4 | 1.00 | 1.12 | 39.6 |
| All Vehicles | 2169 | 0.0 | 0.774 | 15.7 | LOS B | 11.2 | 78.4 | 0.90 | 0.99 | 41.3 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Project: P:\12S1200-1299\12S1231000 - West Cullburra MWT\ModellingISIDRA\130226sid-12S1231000 West

13S1231000 - West Culburra Subdivision
Kalandar Street-Kinghorne Street-Albatross Road
Saturday - Equivalent 120th HH
Existing
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Kinghorne Sttreet |  |  |  |  |  |  |  |  |  |  |
| 1 L | 21 | 12.5 | 0.225 | 10.5 | LOS A | 1.4 | 9.9 | 0.59 | 0.74 | 47.5 |
| 2 T | 162 | 1.6 | 0.225 | 8.8 | LOS A | 1.4 | 9.9 | 0.59 | 0.66 | 47.5 |
| 3 R | 29 | 0.0 | 0.225 | 13.6 | LOS A | 1.4 | 9.9 | 0.59 | 0.81 | 44.7 |
| Approach | 213 | 2.5 | 0.225 | 9.6 | LOS A | 1.4 | 9.9 | 0.59 | 0.69 | 47.1 |
| East: Kalandar Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 199 | 6.0 | 0.262 | 7.5 | LOS A | 1.4 | 10.1 | 0.37 | 0.56 | 45.8 |
| $6 \quad \mathrm{R}$ | 96 | 1.4 | 0.262 | 12.2 | LOS A | 1.4 | 10.1 | 0.37 | 0.77 | 42.3 |
| Approach | 295 | 4.5 | 0.262 | 9.0 | LOS A | 1.4 | 10.1 | 0.37 | 0.63 | 44.5 |
| North: Kinghorne Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 152 | 0.8 | 0.310 | 8.5 | LOS A | 2.0 | 14.0 | 0.52 | 0.64 | 47.4 |
| 8 T | 21 | 0.0 | 0.310 | 7.7 | LOS A | 2.0 | 14.0 | 0.52 | 0.60 | 47.5 |
| 9 R | 157 | 2.6 | 0.310 | 11.8 | LOS A | 2.0 | 14.0 | 0.52 | 0.73 | 45.6 |
| Approach | 329 | 1.6 | 0.310 | 10.0 | LOS A | 2.0 | 14.0 | 0.52 | 0.68 | 46.6 |
| South West: Albatross Road |  |  |  |  |  |  |  |  |  |  |
| 30 L | 161 | 1.4 | 0.373 | 8.0 | LOS A | 2.5 | 18.0 | 0.56 | 0.62 | 47.4 |
| 32 R | 234 | 1.7 | 0.373 | 12.2 | LOS A | 2.5 | 18.0 | 0.56 | 0.75 | 45.6 |
| Approach | 395 | 1.6 | 0.373 | 10.5 | LOS A | 2.5 | 18.0 | 0.56 | 0.70 | 46.3 |
| All Vehicles | 1232 | 2.4 | 0.373 | 9.8 | LOS A | 2.5 | 18.0 | 0.51 | 0.68 | 46.2 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Princes Highway-Forest Road
Friday AM (0800-0900) - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back o Vehicles veh | Queue <br> Distance <br> m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |
| 2 T | 1413 | 3.1 | 0.739 | 0.4 | X | X | X | X | 0.00 | 98.7 |
| 3 R | 27 | 13.0 | 0.043 | 17.2 | LOS B | 0.2 | 1.2 | 0.56 | 0.81 | 56.9 |
| Approach | 1440 | 3.3 | 0.739 | 0.7 | NA | 0.2 | 1.2 | 0.01 | 0.02 | 97.7 |
| South East: Forest Road (Median RT) |  |  |  |  |  |  |  |  |  |  |
| 23 R | 102 | 2.3 | 0.056 | 8.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 53.3 |
| Approach | 102 | 2.3 | 0.056 | 8.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 53.3 |
| East: Forest Road |  |  |  |  |  |  |  |  |  |  |
| 4 L | 36 | 3.3 | 0.078 | 13.1 | LOS A | 0.2 | 1.5 | 0.53 | 0.80 | 51.4 |
| $6 \quad \mathrm{R}$ | 102 | 2.3 | 0.213 | 14.6 | LOS B | 0.7 | 5.3 | 0.59 | 0.88 | 49.9 |
| Approach | 138 | 2.6 | 0.213 | 14.2 | LOS A | 0.7 | 5.3 | 0.58 | 0.86 | 50.3 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 57 | 8.3 | 0.032 | 13.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.76 | 63.3 |
| 8 T | 521 | 16.0 | 0.295 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 100.0 |
| Approach | 578 | 15.2 | 0.295 | 1.3 | NA | 0.0 | 0.0 | 0.00 | 0.07 | 95.8 |
| All Vehicles | 2258 | 6.3 | 0.739 | 2.0 | NA | 0.7 | 5.3 | 0.04 | 0.11 | 90.1 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Princes Highway-Forest Road
Friday PM (1600-1700) - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue <br> Distance <br> m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |
| 2 T | 640 | 6.0 | 0.341 | 0.1 | X | X | X | X | 0.00 | 99.8 |
| 3 R | 51 | 2.9 | 0.357 | 44.3 | LOS D | 1.2 | 8.5 | 0.93 | 1.01 | 33.0 |
| Approach | 691 | 5.8 | 0.357 | 3.3 | NA | 1.2 | 8.5 | 0.07 | 0.07 | 89.5 |
| South East: Forest Road (Median RT) |  |  |  |  |  |  |  |  |  |  |
| 23 R | 66 | 2.2 | 0.036 | 8.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 53.3 |
| Approach | 66 | 2.2 | 0.036 | 8.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 53.3 |
| East: Forest Road |  |  |  |  |  |  |  |  |  |  |
| 4 L | 49 | 0.0 | 0.448 | 51.2 | LOS D | 1.5 | 10.2 | 0.95 | 1.04 | 30.2 |
| $6 \quad \mathrm{R}$ | 66 | 2.2 | 0.654 | 66.4 | LOS E | 2.4 | 16.9 | 0.96 | 1.10 | 26.0 |
| Approach | 116 | 1.3 | 0.654 | 59.9 | LOS E | 2.4 | 16.9 | 0.96 | 1.07 | 27.6 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 148 | 2.0 | 0.081 | 12.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.75 | 63.3 |
| 8 T | 1475 | 2.0 | 0.766 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 100.0 |
| Approach | 1623 | 2.0 | 0.766 | 1.2 | NA | 0.0 | 0.0 | 0.00 | 0.07 | 96.1 |
| All Vehicles | 2496 | 3.0 | 0.766 | 4.7 | NA | 2.4 | 16.9 | 0.06 | 0.13 | 83.7 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Princes Highway-Forest Road
Saturday - Equivalent 120th HH
Existing
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \mathrm{HV} \\ \% \end{gathered}$ | Deg. <br> v/c | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |
| 2 T | 812 | 1.7 | 0.421 | 0.1 | X | X | X | X | 0.00 | 99.7 |
| 3 R | 40 | 0.0 | 0.142 | 25.1 | LOS B | 0.5 | 3.3 | 0.83 | 0.96 | 46.4 |
| Approach | 852 | 1.6 | 0.421 | 1.3 | NA | 0.5 | 3.3 | 0.04 | 0.04 | 95.7 |
| South East: Forest Road (Median RT) |  |  |  |  |  |  |  |  |  |  |
| 23 R | 88 | 0.0 | 0.048 | 8.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 53.3 |
| Approach | 88 | 0.0 | 0.048 | 8.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 53.3 |
| East: Forest Road |  |  |  |  |  |  |  |  |  |  |
| 4 L | 45 | 0.0 | 0.202 | 25.0 | LOS B | 0.7 | 4.6 | 0.85 | 0.96 | 42.0 |
| 6 R | 88 | 3.0 | 0.470 | 33.6 | LOS C | 1.8 | 12.6 | 0.89 | 1.05 | 37.3 |
| Approach | 134 | 2.0 | 0.470 | 30.7 | LOS C | 1.8 | 12.6 | 0.88 | 1.02 | 38.8 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 125 | 0.0 | 0.067 | 12.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.75 | 63.3 |
| 8 T | 1153 | 1.7 | 0.598 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 100.0 |
| Approach | 1278 | 1.6 | 0.598 | 1.2 | NA | 0.0 | 0.0 | 0.00 | 0.07 | 95.8 |
| All Vehicles | 2352 | 1.5 | 0.598 | 3.2 | NA | 1.8 | 12.6 | 0.06 | 0.14 | 87.0 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Princes Highway-Moss Street
Friday AM (0800-0900) - Equivalent 120th HH
Existing
Signals - Fixed Time Cycle Time $=135$ seconds (Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |
| 1 L | 3 | 0.0 | 0.947 | 91.5 | LOS F | 22.5 | 167.8 | 1.00 | 1.06 | 17.9 |
| 2 T | 904 | 7.6 | 1.025 | 105.5 | LOS F | 29.3 | 218.8 | 1.00 | 1.16 | 16.3 |
| 3 R | 158 | 3.7 | 0.906 | 88.8 | LOS F | 12.0 | 86.5 | 1.00 | 0.97 | 17.5 |
| Approach | 1065 | 7.0 | 1.025 | 103.0 | LOS F | 29.3 | 218.8 | 1.00 | 1.13 | 16.4 |
| East: Moss St (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 34 | 3.4 | 0.377 | 50.0 | LOS D | 3.9 | 28.5 | 0.82 | 0.77 | 24.8 |
| 5 T | 227 | 4.1 | 1.011 | 97.9 | LOS F | 42.5 | 308.5 | 0.97 | 1.13 | 14.4 |
| $6 \quad \mathrm{R}$ | 257 | 4.6 | 1.011 | 118.6 | LOS F | 42.5 | 308.5 | 1.00 | 1.25 | 14.3 |
| Approach | 518 | 4.3 | 1.011 | 105.0 | LOS F | 42.5 | 308.5 | 0.97 | 1.17 | 14.8 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 187 | 5.0 | $1.000^{3}$ | 55.0 | LOS D | 29.0 | 213.5 | 0.98 | 0.93 | 25.5 |
| 8 T | 1450 | 6.8 | 1.005 | 85.3 | LOS F | 52.3 | 387.5 | 1.00 | 1.16 | 18.9 |
| 9 R | 356 | 3.7 | 1.022 | 99.5 | LOS F | 27.1 | 195.7 | 1.00 | 1.10 | 16.0 |
| Approach | 1994 | 6.1 | 1.022 | 85.0 | LOS F | 52.3 | 387.5 | 0.99 | 1.13 | 18.7 |
| West: Moss St (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 126 | 10.3 | 0.257 | 21.6 | LOS B | 3.1 | 23.3 | 0.66 | 0.75 | 25.0 |
| 11 T | 131 | 3.6 | 0.526 | 54.0 | LOS D | 10.8 | 79.6 | 0.95 | 0.79 | 10.1 |
| 12 R | 49 | 11.9 | 0.526 | 61.8 | LOS E | 10.8 | 79.6 | 0.95 | 0.82 | 12.7 |
| Approach | 306 | 7.7 | 0.526 | 41.9 | LOS C | 10.8 | 79.6 | 0.83 | 0.78 | 14.6 |
| All Vehicles | 3883 | 6.2 | 1.025 | 89.2 | LOS F | 52.3 | 387.5 | 0.98 | 1.11 | 17.2 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.
$3 x=1.00$ due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

| Movement Performance - Pedestrians |  |  |  |  |  |  |  |  |
| :---: | :---: | ---: | :---: | :---: | :---: | ---: | ---: | ---: | ---: |
| Mov ID | Description | Demand <br> Flow <br> ped/h | Average <br> Delay <br> sec | Level of <br> Service | Average Back of Queue <br> Pedestrian <br> ped | Prop. <br> Distance <br> Queued | Effective <br> Stop Rate <br> per ped |  |
| P1 | Across S approach | 53 | 55.1 | LOS E | 0.2 | 0.2 | 0.90 | 0.90 |
| P3 | Across E approach | 53 | 40.1 | LOS E | 0.2 | 0.2 | 0.77 | 0.77 |
| P5 | Across N approach | 53 | 61.6 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 |
| P7 | Across W approach | 53 | 56.0 | LOS E | 0.2 | 0.2 | 0.91 | 0.91 |
| All Pedestrians | 212 | 53.2 | LOS E |  |  | 0.89 | 0.89 |  |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

13S1231000 - West Culburra Subdivision
Princes Highway-Moss Street
Friday PM (1600-1700) - Equivalent 120th HH
Existing
Signals - Fixed Time Cycle Time $=135$ seconds (Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |
| 1 L | 11 | 0.0 | 1.144 | 205.9 | LOS F | 42.0 | 301.3 | 1.00 | 1.42 | 9.0 |
| 2 T | 1117 | 3.1 | 1.237 | 260.8 | LOS F | 58.8 | 422.6 | 1.00 | 1.63 | 7.8 |
| 3 R | 115 | 0.0 | 0.927 | 93.6 | LOS F | 8.9 | 62.2 | 1.00 | 0.99 | 16.8 |
| Approach | 1242 | 2.8 | 1.237 | 244.9 | LOS F | 58.8 | 422.6 | 1.00 | 1.57 | 8.2 |
| East: Moss St (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 34 | 4.3 | 0.369 | 56.2 | LOS D | 3.9 | 28.1 | 0.87 | 0.77 | 23.2 |
| 5 T | 149 | 0.0 | 0.991 | 88.2 | LOS F | 31.0 | 219.2 | 0.97 | 1.06 | 15.4 |
| $6 \quad \mathrm{R}$ | 232 | 1.9 | 0.991 | 109.3 | LOS F | 31.0 | 219.2 | 1.00 | 1.20 | 15.1 |
| Approach | 415 | 1.4 | 0.991 | 97.4 | LOS F | 31.0 | 219.2 | 0.98 | 1.11 | 15.7 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 199 | 3.7 | $1.000^{3}$ | 62.5 | LOS E | 29.6 | 213.5 | 1.00 | 0.94 | 23.3 |
| 8 T | 1465 | 3.5 | 1.166 | 190.1 | LOS F | 80.2 | 578.3 | 1.00 | 1.56 | 10.2 |
| 9 R | 262 | 0.7 | 1.128 | 179.4 | LOS F | 27.7 | 195.1 | 1.00 | 1.26 | 9.9 |
| Approach | 1926 | 2.9 | 1.166 | 175.4 | LOS F | 80.2 | 578.3 | 1.00 | 1.46 | 10.7 |
| West: Moss St (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 169 | 1.2 | 0.330 | 19.1 | LOS B | 3.6 | 25.2 | 0.61 | 0.75 | 26.6 |
| 11 T | 245 | 0.0 | 1.237 | 288.0 | LOS F | 113.0 | 795.9 | 1.00 | 1.79 | 2.4 |
| 12 R | 135 | 1.1 | 1.237 | 295.5 | LOS F | 113.0 | 795.9 | 1.00 | 1.79 | 3.1 |
| Approach | 886 | 0.8 | 1.237 | 240.2 | LOS F | 113.0 | 795.9 | 0.88 | 1.47 | 3.8 |
| All Vehicles | 4469 | 2.3 | 1.237 | 199.9 | LOS F | 113.0 | 795.9 | 0.98 | 1.35 | 9.2 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.
$3 x=1.00$ due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

| Movement Performance - Pedestrians |  |  |  |  |  |  |  |  |
| :---: | :---: | ---: | :---: | :---: | :---: | ---: | ---: | ---: | ---: |
| Mov ID | Description | Demand <br> Flow <br> ped/h | Average <br> Delay <br> sec | Level of <br> Service | Average Back of Queue <br> Pedestrian <br> ped | Prop. <br> Distance <br> Queued | Effective <br> Stop Rate <br> per ped |  |
| P1 | Across S approach | 53 | 61.6 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 |
| P3 | Across E approach | 53 | 44.0 | LOS E | 0.2 | 0.2 | 0.81 | 0.81 |
| P5 | Across N approach | 53 | 47.3 | LOS E | 0.2 | 0.2 | 0.84 | 0.84 |
| P7 | Across W approach | 53 | 56.0 | LOS E | 0.2 | 0.2 | 0.91 | 0.91 |
| All Pedestrians | 212 | 52.2 | LOS E |  |  | 0.88 | 0.88 |  |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

13S1231000 - West Culburra Subdivision
Princes Highway-Moss Street
Saturday - Equivalent 120th HH
Existing
Signals - Fixed Time Cycle Time $=135$ seconds (Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue <br> Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |
| 1 L | 12 | 0.0 | 0.820 | 66.0 | LOS E | 23.8 | 170.1 | 0.98 | 0.93 | 23.0 |
| 2 T | 1167 | 2.7 | 0.887 | 60.8 | LOS E | 28.0 | 200.4 | 0.99 | 0.96 | 23.7 |
| 3 R | 104 | 0.0 | 0.541 | 72.5 | LOS F | 6.7 | 47.1 | 1.00 | 0.79 | 20.2 |
| Approach | 1283 | 2.4 | 0.887 | 61.8 | LOS E | 28.0 | 200.4 | 0.99 | 0.94 | 23.4 |
| East: Moss St (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 32 | 0.0 | 0.228 | 52.5 | LOS D | 2.4 | 17.0 | 0.83 | 0.75 | 24.0 |
| 5 T | 80 | 0.0 | 0.612 | 53.2 | LOS D | 13.0 | 91.7 | 0.95 | 0.78 | 20.8 |
| $6 \quad \mathrm{R}$ | 146 | 1.8 | 0.612 | 62.5 | LOS E | 13.0 | 91.7 | 0.97 | 0.83 | 21.8 |
| Approach | 258 | 1.0 | 0.612 | 58.4 | LOS E | 13.0 | 91.7 | 0.95 | 0.80 | 21.8 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 115 | 2.3 | 0.757 | 48.5 | LOS D | 20.7 | 147.2 | 0.82 | 0.96 | 27.7 |
| 8 T | 1303 | 1.5 | 0.757 | 40.2 | LOS C | 27.2 | 192.6 | 0.88 | 0.78 | 30.1 |
| 9 R | 258 | 1.0 | 0.858 | 49.2 | LOS D | 12.0 | 85.0 | 1.00 | 0.91 | 26.1 |
| Approach | 1676 | 1.5 | 0.858 | 42.1 | LOS C | 27.2 | 192.6 | 0.89 | 0.81 | 29.3 |
| West: Moss St (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 311 | 0.0 | 0.590 | 23.7 | LOS B | 8.0 | 55.9 | 0.77 | 0.80 | 23.5 |
| 11 T | 154 | 0.0 | 0.700 | 55.8 | LOS D | 16.1 | 113.0 | 0.99 | 0.84 | 9.8 |
| 12 R | 103 | 1.3 | 0.700 | 63.2 | LOS E | 16.1 | 113.0 | 0.99 | 0.85 | 12.3 |
| Approach | 567 | 0.2 | 0.700 | 39.6 | LOS C | 16.1 | 113.0 | 0.87 | 0.82 | 15.8 |
| All Vehicles | 3784 | 1.6 | 0.887 | 49.5 | LOS D | 28.0 | 200.4 | 0.93 | 0.86 | 24.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

Movement Performance - Pedestrians

| Mov ID | Description | Demand <br> Flow <br> ped/h | Average <br> Delay <br> sec | Level of <br> Service |  | Average Back of Queue <br> Pedestrian <br> ped | Prop. <br> Distance <br> Queued | Effective <br> Stop Rate <br> per ped |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | ---: | ---: | ---: |
| P1 | Across S approach | 53 | 61.6 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 |
| P3 | Across E approach | 53 | 36.3 | LOS D | 0.1 | 0.1 | 0.73 | 0.73 |
| P5 | Across N approach | 53 | 60.7 | LOS F | 0.2 | 0.2 | 0.95 | 0.95 |
| P7 | Across W approach | 53 | 47.3 | LOS E | 0.2 | 0.2 | 0.84 | 0.84 |
| All Pedestrians | 212 | 51.5 | LOS E |  |  | 0.87 | 0.87 |  |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

13S1231000 - West Culburra Subdivision
Culburra Road-Coonamia Road
Friday AM (0800-0900) - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles <br> veh | Queue <br> Distance <br> m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Coonamia Rd |  |  |  |  |  |  |  |  |  |  |
| 1 L | 165 | 0.0 | 0.234 | 14.4 | LOS A | 0.8 | 5.6 | 0.46 | 0.79 | 61.2 |
| 3 R | 85 | 6.5 | 0.175 | 17.6 | LOS B | 0.7 | 5.1 | 0.55 | 0.86 | 57.3 |
| Approach | 251 | 2.2 | 0.234 | 15.5 | LOS B | 0.8 | 5.6 | 0.49 | 0.81 | 59.8 |
| East: Culburra Rd (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 58 | 2.9 | 0.032 | 11.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.74 | 63.3 |
| 5 T | 328 | 3.7 | 0.172 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 386 | 3.6 | 0.172 | 1.7 | NA | 0.0 | 0.0 | 0.00 | 0.11 | 76.6 |
| West: Culburra Rd (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 123 | 12.3 | 0.068 | 0.0 | X | X | X | X | 0.00 | 80.0 |
| 12 R | 33 | 3.6 | 0.033 | 13.1 | LOS A | 0.1 | 0.9 | 0.42 | 0.72 | 61.2 |
| Approach | 156 | 10.5 | 0.068 | 2.8 | NA | 0.1 | 0.9 | 0.09 | 0.15 | 74.7 |
| All Vehicles | 793 | 4.5 | 0.234 | 6.3 | NA | 0.8 | 5.6 | 0.17 | 0.34 | 70.2 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Culburra Road-Coonamia Road
Friday PM (1600-1700) - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Coonamia Rd |  |  |  |  |  |  |  |  |  |  |
| 1 L | 77 | 7.7 | 0.107 | 13.7 | LOS A | 0.3 | 2.2 | 0.30 | 0.71 | 62.7 |
| 3 R | 99 | 2.0 | 0.181 | 16.3 | LOS B | 0.7 | 5.2 | 0.52 | 0.82 | 58.6 |
| Approach | 176 | 4.5 | 0.181 | 15.2 | LOS B | 0.7 | 5.2 | 0.42 | 0.77 | 60.4 |
| East: Culburra Rd (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 89 | 2.0 | 0.049 | 11.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.74 | 63.3 |
| 5 T | 139 | 2.6 | 0.072 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 228 | 2.4 | 0.072 | 4.5 | NA | 0.0 | 0.0 | 0.00 | 0.29 | 72.0 |
| West: Culburra Rd (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 371 | 1.1 | 0.191 | 0.0 | X | X | X | X | 0.00 | 79.9 |
| 12 R | 152 | 3.9 | 0.131 | 12.5 | LOS A | 0.6 | 4.0 | 0.34 | 0.71 | 61.6 |
| Approach | 522 | 1.9 | 0.191 | 3.6 | NA | 0.6 | 4.0 | 0.10 | 0.21 | 73.1 |
| All Vehicles | 926 | 2.5 | 0.191 | 6.0 | NA | 0.7 | 5.2 | 0.14 | 0.33 | 70.1 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Culburra Road-Coonamia Road
Saturday - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Coonamia Rd |  |  |  |  |  |  |  |  |  |  |
| 1 L | 104 | 1.3 | 0.144 | 13.8 | LOS A | 0.4 | 3.1 | 0.38 | 0.74 | 62.2 |
| 3 R | 89 | 0.0 | 0.177 | 16.9 | LOS B | 0.7 | 4.9 | 0.54 | 0.85 | 57.6 |
| Approach | 194 | 0.7 | 0.177 | 15.2 | LOS B | 0.7 | 4.9 | 0.45 | 0.79 | 60.0 |
| East: Culburra Rd (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 106 | 0.0 | 0.057 | 11.3 | LOS A | 0.0 | 0.0 | 0.00 | 0.73 | 63.3 |
| 5 T | 225 | 0.8 | 0.116 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 332 | 0.5 | 0.116 | 3.6 | NA | 0.0 | 0.0 | 0.00 | 0.24 | 73.3 |
| West: Culburra Rd (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 227 | 0.8 | 0.117 | 0.0 | X | X | X | X | 0.00 | 80.0 |
| 12 R | 104 | 2.5 | 0.100 | 12.9 | LOS A | 0.4 | 2.9 | 0.41 | 0.73 | 61.2 |
| Approach | 332 | 1.3 | 0.117 | 4.1 | NA | 0.4 | 2.9 | 0.13 | 0.23 | 72.4 |
| All Vehicles | 857 | 0.9 | 0.177 | 6.4 | NA | 0.7 | 4.9 | 0.15 | 0.36 | 69.6 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

Culburra Road-Mayfield Road
Friday AM (0800-0900) - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Culburra Road (S) 0 en er |  |  |  |  |  |  |  |  |  |  |
| 1 L | 2 | 0.0 | 0.251 | 10.1 | LOS A | 0.0 | 0.0 | 0.00 | 1.73 | 57.1 |
| 2 T | 486 | 0.0 | 0.251 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 488 | 0.0 | 0.251 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 79.9 |
| North: Culburra Road (N) |  |  |  |  |  |  |  |  |  |  |
| 8 T | 154 | 0.0 | 0.080 | 2.7 | LOS A | 0.7 | 5.0 | 0.58 | 0.00 | 63.0 |
| 9 R | 1 | 0.0 | 0.080 | 12.8 | LOS A | 0.7 | 5.0 | 0.58 | 1.31 | 60.3 |
| Approach | 155 | 0.0 | 0.080 | 2.7 | NA | 0.7 | 5.0 | 0.58 | 0.01 | 63.0 |
| West: Mayfield Road |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 0.0 | 0.008 | 16.3 | LOS B | 0.0 | 0.2 | 0.61 | 0.70 | 44.3 |
| 12 R | 2 | 0.0 | 0.008 | 16.3 | LOS B | 0.0 | 0.2 | 0.61 | 0.79 | 44.5 |
| Approach | 3 | 0.0 | 0.008 | 16.3 | LOS B | 0.0 | 0.2 | 0.61 | 0.76 | 44.4 |
| All Vehicles | 646 | 0.0 | 0.251 | 0.8 | NA | 0.7 | 5.0 | 0.14 | 0.01 | 74.8 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

Processed: Thursday, 28 February 2013 2:07:18 PM
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SIDRA
INTERSECTION

Culburra Road-Mayfield Road
Friday PM (1600-1700) - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queue <br> Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Culburra Road (S) |  |  |  |  |  |  |  |  |  |  |
| 1 L | 1 | 0.0 | 0.115 | 10.1 | LOS A | 0.0 | 0.0 | 0.00 | 1.73 | 57.1 |
| 2 T | 217 | 4.6 | 0.115 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 218 | 4.6 | 0.115 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 79.9 |
| North: Culburra Road (N) |  |  |  |  |  |  |  |  |  |  |
| 8 T | 502 | 1.8 | 0.261 | 1.2 | LOS A | 2.1 | 14.9 | 0.44 | 0.00 | 66.4 |
| 9 R | 1 | 0.0 | 0.261 | 11.4 | LOS A | 2.1 | 14.9 | 0.44 | 1.41 | 60.4 |
| Approach | 503 | 1.8 | 0.261 | 1.2 | NA | 2.1 | 14.9 | 0.44 | 0.00 | 66.4 |
| West: Mayfield Road |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 0.0 | 0.030 | 56.4 | LOS D | 0.1 | 0.9 | 0.80 | 0.60 | 25.6 |
| 12 R | 1 | 100.0 | 0.030 | 61.8 | LOS E | 0.1 | 0.9 | 0.80 | 0.95 | 28.1 |
| Approach | 2 | 50.0 | 0.030 | 59.1 | LOS E | 0.1 | 0.9 | 0.80 | 0.78 | 27.0 |
| All Vehicles | 723 | 2.8 | 0.261 | 1.0 | NA | 2.1 | 14.9 | 0.31 | 0.01 | 69.6 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

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SIDRA
INTERSECTION

Culburra Road-Mayfield Road
Saturday - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Culburra Road (S) 0 der |  |  |  |  |  |  |  |  |  |  |
| L | 3 | 0.0 | 0.173 | 10.1 | LOS A | 0.0 | 0.0 | 0.00 | 1.72 | 57.1 |
| 2 T | 333 | 0.5 | 0.173 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 336 | 0.5 | 0.173 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.02 | 79.7 |
| North: Culburra Road ( N ) |  |  |  |  |  |  |  |  |  |  |
| 8 T | 309 | 0.5 | 0.160 | 1.8 | LOS A | 1.3 | 9.2 | 0.51 | 0.00 | 64.7 |
| 9 R | 1 | 0.0 | 0.160 | 11.9 | LOS A | 1.3 | 9.2 | 0.51 | 1.36 | 60.9 |
| Approach | 311 | 0.5 | 0.160 | 1.8 | NA | 1.3 | 9.2 | 0.51 | 0.00 | 64.6 |
| West: Mayfield Road |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 0.0 | 0.012 | 16.6 | LOS B | 0.0 | 0.3 | 0.59 | 0.65 | 44.0 |
| 12 R | 3 | 0.0 | 0.012 | 16.6 | LOS B | 0.0 | 0.3 | 0.59 | 0.80 | 44.2 |
| Approach | 4 | 0.0 | 0.012 | 16.6 | LOS B | 0.0 | 0.3 | 0.59 | 0.76 | 44.1 |
| All Vehicles | 651 | 0.5 | 0.173 | 1.0 | NA | 1.3 | 9.2 | 0.25 | 0.02 | 71.4 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

Processed: Thursday, 28 February 2013 2:15:42 PM

SIDRA
INTERSECTION

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Pyree Lane
Friday AM (0800-0900) - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Pyreen Ln |  |  |  |  |  |  |  |  |  |  |
| 1 L | 479 | 1.9 | 0.261 | 11.2 | X | X | X | X | 0.69 | 58.8 |
| 3 R | 14 | 8.3 | 0.020 | 13.0 | LOS A | 0.1 | 0.6 | 0.34 | 0.69 | 56.9 |
| Approach | 493 | 2.1 | 0.261 | 11.2 | LOS A | 0.1 | 0.6 | 0.01 | 0.69 | 58.8 |
| East: Greenwell Pt Rd (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 20 | 66.7 | 0.094 | 14.9 | LOS B | 0.0 | 0.0 | 0.00 | 1.42 | 58.9 |
| 5 T | 147 | 5.6 | 0.094 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 167 | 12.9 | 0.094 | 1.8 | NA | 0.0 | 0.0 | 0.00 | 0.17 | 76.8 |
| West: Greenwell Pt Rd (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 63 | 5.6 | 0.034 | 0.0 | X | X | X | X | 0.00 | 80.0 |
| 12 R | 138 | 7.4 | 0.306 | 18.3 | LOS B | 1.4 | 10.6 | 0.61 | 0.92 | 50.5 |
| Approach | 201 | 6.9 | 0.306 | 12.5 | LOS A | 1.4 | 10.6 | 0.42 | 0.63 | 57.2 |
| All Vehicles | 861 | 5.3 | 0.306 | 9.7 | NA | 1.4 | 10.6 | 0.10 | 0.57 | 61.2 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Pyree Lane
Friday PM (1600-1700) - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Pyreen Ln |  |  |  |  |  |  |  |  |  |  |
| 1 L | 179 | 5.8 | 0.100 | 11.3 | X | X | X | X | 0.69 | 58.9 |
| 3 R | 32 | 0.0 | 0.039 | 11.5 | LOS A | 0.2 | 1.1 | 0.23 | 0.68 | 58.0 |
| Approach | 211 | 4.9 | 0.100 | 11.4 | LOS A | 0.2 | 1.1 | 0.03 | 0.69 | 58.7 |
| East: Greenwell Pt Rd (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 26 | 0.0 | 0.050 | 10.9 | LOS A | 0.0 | 0.0 | 0.00 | 1.07 | 58.9 |
| 5 T | 68 | 4.3 | 0.050 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 95 | 3.1 | 0.050 | 3.0 | NA | 0.0 | 0.0 | 0.00 | 0.30 | 72.9 |
| West: Greenwell Pt Rd (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 173 | 1.7 | 0.090 | 0.0 | X | X | X | X | 0.00 | 80.0 |
| 12 R | 481 | 1.5 | 0.701 | 18.6 | LOS B | 9.6 | 68.2 | 0.69 | 0.93 | 49.8 |
| Approach | 654 | 1.6 | 0.701 | 13.7 | LOS A | 9.6 | 68.2 | 0.51 | 0.68 | 55.4 |
| All Vehicles | 959 | 2.5 | 0.701 | 12.1 | NA | 9.6 | 68.2 | 0.36 | 0.65 | 57.5 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Pyree Lane
Friday AM (0800-0900) - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Pyreen Ln |  |  |  |  |  |  |  |  |  |  |
| 1 L | 274 | 1.2 | 0.149 | 11.1 | X | X | X | X | 0.69 | 58.9 |
| 3 R | 55 | 0.0 | 0.080 | 12.7 | LOS A | 0.3 | 2.2 | 0.37 | 0.72 | 56.5 |
| Approach | 328 | 1.0 | 0.149 | 11.4 | LOS A | 0.3 | 2.2 | 0.06 | 0.69 | 58.5 |
| East: Greenwell Pt Rd (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 169 | 2.7 | 0.146 | 11.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.87 | 58.9 |
| 5 T | 100 | 3.9 | 0.146 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 269 | 3.2 | 0.146 | 7.0 | NA | 0.0 | 0.0 | 0.00 | 0.55 | 65.4 |
| West: Greenwell Pt Rd (W) |  |  |  |  |  |  |  |  |  |  |
| 11 T | 227 | 1.2 | 0.117 | 0.0 | X | X | X | X | 0.00 | 80.0 |
| 12 R | 146 | 1.2 | 0.307 | 17.4 | LOS B | 1.4 | 10.2 | 0.60 | 0.92 | 51.0 |
| Approach | 374 | 1.2 | 0.307 | 6.8 | LOS A | 1.4 | 10.2 | 0.24 | 0.36 | 65.6 |
| All Vehicles | 972 | 1.7 | 0.307 | 8.4 | NA | 1.4 | 10.2 | 0.11 | 0.52 | 62.9 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Jindy Andy Lane
Friday AM (0800-0900) - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \mathrm{HV} \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| North East: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |
| 25 T | 442 | 4.8 | 0.234 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| 26 R | 191 | 0.7 | 0.283 | 10.8 | LOS A | 1.3 | 9.1 | 0.44 | 0.72 | 46.2 |
| Approach | 633 | 3.6 | 0.283 | 3.2 | NA | 1.3 | 9.1 | 0.13 | 0.22 | 66.7 |
| North West: Jindy Andy Lane |  |  |  |  |  |  |  |  |  |  |
| 27 L | 45 | 3.0 | 0.045 | 11.9 | LOS A | 0.2 | 1.2 | 0.28 | 0.68 | 57.3 |
| 29 R | 12 | 30.0 | 0.084 | 37.7 | LOS C | 0.3 | 2.4 | 0.82 | 0.96 | 36.9 |
| Approach | 57 | 8.5 | 0.084 | 17.1 | LOS B | 0.3 | 2.4 | 0.39 | 0.74 | 51.5 |
| South West: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |
| 30 L | 8 | 42.9 | 0.091 | 13.5 | LOS A | 0.0 | 0.0 | 0.00 | 1.47 | 58.9 |
| 31 T | 157 | 8.7 | 0.091 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 165 | 10.4 | 0.091 | 0.7 | NA | 0.0 | 0.0 | 0.00 | 0.07 | 78.6 |
| All Vehicles | 855 | 5.2 | 0.283 | 3.7 | NA | 1.3 | 9.1 | 0.12 | 0.22 | 67.4 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Jindy Andy Lane
Friday AM (1600-1700) - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| North East: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |
| 25 T | 193 | 5.9 | 0.103 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| 26 R | 67 | 2.4 | 0.192 | 17.1 | LOS B | 0.7 | 5.1 | 0.66 | 0.89 | 40.8 |
| Approach | 260 | 5.0 | 0.192 | 4.4 | NA | 0.7 | 5.1 | 0.17 | 0.23 | 65.3 |
| North West: Jindy Andy Lane |  |  |  |  |  |  |  |  |  |  |
| 27 L | 185 | 1.9 | 0.273 | 14.8 | LOS B | 1.1 | 7.9 | 0.55 | 0.89 | 54.0 |
| 29 R | 3 | 0.0 | 0.011 | 21.5 | LOS B | 0.0 | 0.3 | 0.70 | 0.83 | 46.9 |
| Approach | 188 | 1.8 | 0.273 | 14.9 | LOS B | 1.1 | 7.9 | 0.55 | 0.88 | 53.9 |
| South West: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |
| 30 L | 14 | 11.1 | 0.249 | 11.6 | LOS A | 0.0 | 0.0 | 0.00 | 1.37 | 58.9 |
| 31 T | 464 | 1.9 | 0.249 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 478 | 2.1 | 0.249 | 0.3 | NA | 0.0 | 0.0 | 0.00 | 0.04 | 79.2 |
| All Vehicles | 926 | 2.9 | 0.273 | 4.4 | NA | 1.1 | 7.9 | 0.16 | 0.26 | 68.7 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Jindy Andy Lane
Saturday - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| North East: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |
| 25 T | 287 | 3.1 | 0.150 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| 26 R | 109 | 1.4 | 0.218 | 13.3 | LOS A | 0.9 | 6.2 | 0.56 | 0.83 | 43.9 |
| Approach | 397 | 2.6 | 0.218 | 3.7 | NA | 0.9 | 6.2 | 0.15 | 0.23 | 66.4 |
| North West: Jindy Andy Lane |  |  |  |  |  |  |  |  |  |  |
| 27 L | 113 | 0.0 | 0.134 | 12.8 | LOS A | 0.5 | 3.5 | 0.42 | 0.75 | 56.3 |
| 29 R | 9 | 0.0 | 0.033 | 21.3 | LOS B | 0.1 | 0.8 | 0.69 | 0.89 | 47.1 |
| Approach | 122 | 0.0 | 0.134 | 13.4 | LOS A | 0.5 | 3.5 | 0.44 | 0.76 | 55.5 |
| South West: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |
| 30 L | 5 | 0.0 | 0.166 | 10.9 | LOS A | 0.0 | 0.0 | 0.00 | 1.33 | 58.9 |
| 31 T | 313 | 3.2 | 0.166 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 318 | 3.1 | 0.166 | 0.2 | NA | 0.0 | 0.0 | 0.00 | 0.02 | 79.5 |
| All Vehicles | 837 | 2.4 | 0.218 | 3.8 | NA | 0.9 | 6.2 | 0.14 | 0.23 | 68.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Mayfield Road
Friday AM (0800-0900) - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South East: Mayfield Road |  |  |  |  |  |  |  |  |  |  |
| 21 L | 12 | 0.0 | 0.020 | 12.3 | LOS A | 0.1 | 0.5 | 0.49 | 0.72 | 47.9 |
| 23 R | 1 | 0.0 | 0.020 | 12.3 | LOS A | 0.1 | 0.5 | 0.49 | 0.81 | 48.0 |
| Approach | 13 | 0.0 | 0.020 | 12.3 | LOS A | 0.1 | 0.5 | 0.49 | 0.73 | 47.9 |
| North East: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |
| 24 L | 3 | 0.0 | 0.237 | 10.1 | LOS A | 0.0 | 0.0 | 0.00 | 1.72 | 57.1 |
| 25 T | 448 | 3.8 | 0.237 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 452 | 3.7 | 0.237 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 79.8 |
| South West: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |
| 31 T | 162 | 9.2 | 0.103 | 6.4 | LOS A | 1.5 | 11.3 | 0.75 | 0.00 | 59.0 |
| 32 R | 6 | 0.0 | 0.103 | 16.6 | LOS B | 1.5 | 11.3 | 0.75 | 1.17 | 56.2 |
| Approach | 168 | 8.8 | 0.103 | 6.8 | NA | 1.5 | 11.3 | 0.75 | 0.04 | 58.9 |
| All Vehicles | 633 | 5.0 | 0.237 | 2.1 | NA | 1.5 | 11.3 | 0.21 | 0.03 | 72.1 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Mayfield Road
Friday PM (1600-1700) - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South East: Mayfield Road |  |  |  |  |  |  |  |  |  |  |
| 21 L | 6 | 0.0 | 0.042 | 18.8 | LOS B | 0.1 | 1.1 | 0.55 | 0.63 | 42.4 |
| 23 R | 6 | 25.0 | 0.042 | 20.2 | LOS B | 0.1 | 1.1 | 0.55 | 0.88 | 42.6 |
| Approach | 13 | 12.5 | 0.042 | 19.5 | LOS B | 0.1 | 1.1 | 0.55 | 0.76 | 42.5 |
| North East: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |
| 24 L | 1 | 0.0 | 0.106 | 10.1 | LOS A | 0.0 | 0.0 | 0.00 | 1.73 | 57.1 |
| 25 T | 199 | 5.7 | 0.106 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 200 | 5.7 | 0.106 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.01 | 79.9 |
| South West: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |
| 31 T | 478 | 1.4 | 0.270 | 2.8 | LOS A | 3.6 | 25.3 | 0.62 | 0.00 | 61.8 |
| 32 R | 15 | 0.0 | 0.270 | 13.0 | LOS A | 3.6 | 25.3 | 0.62 | 1.19 | 60.3 |
| Approach | 493 | 1.4 | 0.270 | 3.2 | NA | 3.6 | 25.3 | 0.62 | 0.04 | 61.8 |
| All Vehicles | 705 | 2.8 | 0.270 | 2.6 | NA | 3.6 | 25.3 | 0.44 | 0.04 | 65.5 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Mayfield Road
Saturday - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South East: Mayfield Road |  |  |  |  |  |  |  |  |  |  |
| 21 L | 8 | 0.0 | 0.013 | 11.5 | LOS A | 0.0 | 0.3 | 0.41 | 0.66 | 48.7 |
| 23 R | 1 | 0.0 | 0.013 | 11.5 | LOS A | 0.0 | 0.3 | 0.41 | 0.79 | 48.8 |
| Approach | 9 | 0.0 | 0.013 | 11.5 | LOS A | 0.0 | 0.3 | 0.41 | 0.67 | 48.7 |
| North East: Greenwell Point Road (NE) |  |  |  |  |  |  |  |  |  |  |
| 24 L | 4 | 66.7 | 0.157 | 13.0 | LOS A | 0.0 | 0.0 | 0.00 | 2.25 | 57.1 |
| 25 T | 297 | 1.0 | 0.157 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| Approach | 301 | 1.9 | 0.157 | 0.2 | NA | 0.0 | 0.0 | 0.00 | 0.03 | 79.6 |
| South West: Greenwell Point Road (SW) |  |  |  |  |  |  |  |  |  |  |
| 31 T | 314 | 1.8 | 0.183 | 4.1 | LOS A | 2.5 | 17.7 | 0.69 | 0.00 | 60.3 |
| 32 R | 12 | 0.0 | 0.183 | 14.3 | LOS A | 2.5 | 17.7 | 0.69 | 1.17 | 59.0 |
| Approach | 325 | 1.8 | 0.183 | 4.5 | NA | 2.5 | 17.7 | 0.69 | 0.04 | 60.2 |
| All Vehicles | 636 | 1.8 | 0.183 | 2.5 | NA | 2.5 | 17.7 | 0.36 | 0.05 | 67.8 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000-West Culburra Subdivision
Greenwell Point Road-Millbank Road-Worrigee Road
Friday AM (0800-0900) - Equivalent 120th HH
Future - Full Site Development
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue <br> Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Worrigee Road |  |  |  |  |  |  |  |  |  |  |  |
| 1 | L | 69 | 5.1 | 0.448 | 23.0 | LOS B | 2.4 | 17.5 | 0.71 | 1.12 | 40.8 |
| 2 | T | 118 | 3.0 | 0.448 | 21.6 | LOS B | 2.4 | 17.5 | 0.71 | 1.10 | 38.7 |
| 3 | R | 23 | 0.0 | 0.084 | 23.0 | LOS B | 0.3 | 2.0 | 0.71 | 1.00 | 40.5 |
| Approa |  | 211 | 3.4 | 0.448 | 22.2 | LOS B | 2.4 | 17.5 | 0.71 | 1.09 | 39.6 |
| East: Greenwell Point Road (E) |  |  |  |  |  |  |  |  |  |  |  |
| 4 | L | 14 | 0.0 | 0.227 | 10.1 | LOS A | 0.0 | 0.0 | 0.00 | 1.65 | 57.1 |
| 5 | T | 421 | 3.0 | 0.227 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| 6 | R | 37 | 7.4 | 0.033 | 11.3 | LOS A | 0.1 | 0.9 | 0.32 | 0.67 | 55.2 |
| Approa |  | 472 | 3.2 | 0.227 | 1.2 | NA | 0.1 | 0.9 | 0.02 | 0.10 | 76.8 |
| North: Millbank Road |  |  |  |  |  |  |  |  |  |  |  |
| 7 | L | 16 | 16.7 | 0.032 | 13.9 | LOS A | 0.1 | 0.5 | 0.31 | 0.86 | 48.1 |
| 8 | T | 26 | 13.6 | 0.148 | 24.6 | LOS B | 0.5 | 4.0 | 0.75 | 1.00 | 37.1 |
| 9 | R | 12 | 0.0 | 0.148 | 24.8 | LOS B | 0.5 | 4.0 | 0.75 | 1.00 | 39.4 |
| Approa |  | 54 | 11.6 | 0.148 | 21.5 | LOS B | 0.5 | 4.0 | 0.62 | 0.96 | 40.5 |
| West: Greenwell Point Road (W) |  |  |  |  |  |  |  |  |  |  |  |
| 10 | L | 47 | 7.5 | 0.114 | 10.4 | LOS A | 0.0 | 0.0 | 0.00 | 1.27 | 57.1 |
| 11 | T | 160 | 9.1 | 0.114 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| 12 | R | 61 | 5.8 | 0.069 | 12.5 | LOS A | 0.3 | 1.9 | 0.47 | 0.75 | 54.0 |
| Approach |  | 268 | 8.1 | 0.114 | 4.7 | NA | 0.3 | 1.9 | 0.11 | 0.40 | 68.6 |
| All Vehicles |  | 1004 | 5.0 | 0.448 | 7.6 | NA | 2.4 | 17.5 | 0.22 | 0.43 | 60.7 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Millbank Road-Worrigee Road
Friday PM (1600-1700) - Equivalent 120th HH
Future - Full Site Development
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Worrigee Road 0 |  |  |  |  |  |  |  |  |  |  |
| 1 L | 58 | 0.0 | 0.218 | 17.8 | LOS B | 0.8 | 5.8 | 0.49 | 0.84 | 44.0 |
| 2 T | 47 | 0.0 | 0.218 | 16.6 | LOS B | 0.8 | 5.8 | 0.49 | 1.00 | 42.0 |
| 3 R | 23 | 5.3 | 0.110 | 28.0 | LOS B | 0.4 | 2.7 | 0.79 | 1.00 | 37.7 |
| Approach | 128 | 0.9 | 0.218 | 19.2 | LOS B | 0.8 | 5.8 | 0.55 | 0.93 | 42.0 |
| East: Greenwell Point Road (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 23 | 7.1 | 0.107 | 10.4 | LOS A | 0.0 | 0.0 | 0.00 | 1.47 | 57.1 |
| 5 T | 177 | 5.5 | 0.107 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| 6 R | 16 | 0.0 | 0.019 | 12.4 | LOS A | 0.1 | 0.5 | 0.48 | 0.72 | 53.7 |
| Approach | 216 | 5.2 | 0.107 | 2.0 | NA | 0.1 | 0.5 | 0.04 | 0.21 | 74.7 |
| North: Millbank Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 25 | 0.0 | 0.050 | 14.6 | LOS B | 0.1 | 0.9 | 0.49 | 0.91 | 46.7 |
| 8 T | 54 | 2.8 | 0.270 | 25.5 | LOS B | 1.1 | 7.6 | 0.79 | 1.03 | 36.2 |
| 9 R | 18 | 0.0 | 0.270 | 26.3 | LOS B | 1.1 | 7.6 | 0.79 | 1.03 | 38.5 |
| Approach | 97 | 1.5 | 0.270 | 22.8 | LOS B | 1.1 | 7.6 | 0.71 | 1.00 | 39.1 |
| West: Greenwell Point Road (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 17 | 0.0 | 0.251 | 10.1 | LOS A | 0.0 | 0.0 | 0.00 | 1.64 | 57.1 |
| 11 T | 469 | 0.7 | 0.251 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| 12 R | 115 | 1.3 | 0.097 | 11.0 | LOS A | 0.4 | 2.8 | 0.32 | 0.68 | 55.2 |
| Approach | 601 | 0.8 | 0.251 | 2.4 | NA | 0.4 | 2.8 | 0.06 | 0.18 | 73.6 |
| All Vehicles | 1042 | 1.8 | 0.270 | 6.3 | NA | 1.1 | 7.6 | 0.18 | 0.35 | 63.2 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Greenwell Point Road-Millbank Road-Worrigee Road
Saturday - Equivalent 120th HH
Future - Full Site Development
Stop (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Worrigee Road 77 le |  |  |  |  |  |  |  |  |  |  |
| 1 L | 77 | 0.0 | 0.163 | 15.2 | LOS B | 0.6 | 4.4 | 0.48 | 0.89 | 46.2 |
| 2 T | 29 | 4.5 | 0.163 | 14.2 | LOS A | 0.6 | 4.4 | 0.48 | 1.00 | 44.2 |
| 3 R | 40 | 3.4 | 0.150 | 21.7 | LOS B | 0.5 | 3.3 | 0.68 | 1.00 | 41.6 |
| Approach | 146 | 1.9 | 0.163 | 16.7 | LOS B | 0.6 | 4.4 | 0.53 | 0.94 | 44.5 |
| East: Greenwell Point Road (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 22 | 6.7 | 0.152 | 10.4 | LOS A | 0.0 | 0.0 | 0.00 | 1.57 | 57.1 |
| 5 T | 269 | 1.1 | 0.152 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| 6 R | 19 | 0.0 | 0.018 | 11.3 | LOS A | 0.1 | 0.5 | 0.37 | 0.67 | 55.0 |
| Approach | 311 | 1.4 | 0.152 | 1.4 | NA | 0.1 | 0.5 | 0.02 | 0.15 | 76.2 |
| North: Millbank Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 12 | 16.7 | 0.025 | 14.6 | LOS B | 0.1 | 0.4 | 0.38 | 0.86 | 47.6 |
| 8 T | 24 | 11.1 | 0.127 | 21.3 | LOS B | 0.5 | 3.4 | 0.69 | 1.00 | 39.2 |
| 9 R | 16 | 0.0 | 0.127 | 21.6 | LOS B | 0.5 | 3.4 | 0.69 | 1.00 | 41.5 |
| Approach | 52 | 9.0 | 0.127 | 19.9 | LOS B | 0.5 | 3.4 | 0.62 | 0.97 | 41.7 |
| West: Greenwell Point Road (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 21 | 6.2 | 0.152 | 10.3 | LOS A | 0.0 | 0.0 | 0.00 | 1.57 | 57.1 |
| 11 T | 272 | 1.0 | 0.152 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 80.0 |
| 12 R | 54 | 0.0 | 0.050 | 11.3 | LOS A | 0.2 | 1.4 | 0.37 | 0.69 | 54.9 |
| Approach | 346 | 1.2 | 0.152 | 2.4 | NA | 0.2 | 1.4 | 0.06 | 0.20 | 73.7 |
| All Vehicles | 855 | 1.9 | 0.163 | 5.6 | NA | 0.6 | 4.4 | 0.16 | 0.36 | 64.5 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Princes Highway-Kalandar Street
Friday AM (0800-0900) - Equivalent 120th HH
Future - Full Site Development
Signals - Fixed Time Cycle Time = 135 seconds (Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |
| 1 L | 3 | 0.0 | 1.082 | 128.4 | LOS F | 44.0 | 322.3 | 1.00 | 1.22 | 14.1 |
| 2 T | 962 | 5.6 | 1.082 | 136.7 | LOS F | 53.8 | 394.6 | 1.00 | 1.32 | 13.4 |
| 3 R | 41 | 0.0 | 0.497 | 81.8 | LOS F | 2.8 | 19.9 | 1.00 | 0.73 | 19.2 |
| Approach | 1006 | 5.3 | 1.082 | 134.5 | LOS F | 53.8 | 394.6 | 1.00 | 1.30 | 13.5 |
| East: Kalandar St (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 41 | 9.4 | 1.069 | 138.2 | LOS F | 59.5 | 428.7 | 1.00 | 1.30 | 9.2 |
| 5 T | 324 | 3.1 | 1.069 | 128.9 | LOS F | 59.5 | 428.7 | 1.00 | 1.30 | 8.6 |
| 6 R | 823 | 2.9 | 1.069 | 151.5 | LOS F | 64.2 | 460.7 | 1.00 | 1.27 | 8.5 |
| Approach | 1188 | 3.1 | 1.069 | 144.9 | LOS F | 64.2 | 460.7 | 1.00 | 1.28 | 8.6 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 266 | 7.3 | 0.268 | 9.2 | LOS A | 0.8 | 5.9 | 0.07 | 0.64 | 53.4 |
| 8 T | 898 | 7.3 | 0.751 | 42.2 | LOS C | 24.8 | 184.6 | 0.90 | 0.79 | 29.4 |
| 9 R | 157 | 11.3 | 1.042 | 112.3 | LOS F | 12.0 | 92.4 | 1.00 | 1.09 | 15.2 |
| Approach | 1321 | 7.8 | 1.042 | 43.9 | LOS D | 24.8 | 184.6 | 0.74 | 0.80 | 28.9 |
| West: Kalandar St (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 52 | 13.6 | 0.708 | 66.0 | LOS E | 9.2 | 67.2 | 0.95 | 0.91 | 19.0 |
| 11 T | 154 | 1.6 | 0.708 | 57.5 | LOS E | 13.0 | 94.3 | 0.97 | 0.86 | 17.9 |
| 12 R | 156 | 5.3 | 0.708 | 69.6 | LOS E | 13.0 | 94.3 | 1.00 | 0.85 | 17.9 |
| Approach | 361 | 4.9 | 0.708 | 63.9 | LOS E | 13.0 | 94.3 | 0.98 | 0.86 | 18.1 |
| All Vehicles | 3877 | 5.4 | 1.082 | 100.2 | LOS F | 64.2 | 460.7 | 0.91 | 1.08 | 15.0 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

| Movement Performance - Pedestrians |  |  |  |  |  |  |  |  |
| :---: | :---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Mov ID | Description | Demand <br> Flow <br> ped/h | Average <br> Delay <br> sec | Level of <br> Service | Average Back of Queue <br> Pedestrian <br> ped | Prop. <br> Distance <br> Queued | Effective <br> Stop Rate <br> per ped |  |
| P1 | Across S approach | 53 | 44.8 | LOS E | 0.2 | 0.2 | 0.81 | 0.81 |
| P3 | Across E approach | 53 | 37.8 | LOS D | 0.2 | 0.2 | 0.75 | 0.75 |
| P5 | Across N approach | 53 | 61.6 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 |
| All Pedestrians | 159 | 48.1 | LOS E |  |  | 0.84 | 0.84 |  |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

13S1231000 - West Culburra Subdivision
Princes Highway-Kalandar Street
Friday PM (1600-1700) - Equivalent 120th HH
Future - Full Site Development
Signals - Fixed Time Cycle Time = 125 seconds (Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |
| 1 L | 12 | 0.0 | 1.081 | 130.4 | LOS F | 32.8 | 234.8 | 1.00 | 1.20 | 13.9 |
| 2 T | 785 | 2.6 | 1.081 | 136.2 | LOS F | 41.9 | 300.0 | 1.00 | 1.30 | 13.4 |
| 3 R | 114 | 1.4 | 1.104 | 185.8 | LOS F | 12.7 | 89.6 | 1.00 | 1.22 | 10.0 |
| Approach | 911 | 2.4 | 1.104 | 142.3 | LOS F | 41.9 | 300.0 | 1.00 | 1.29 | 12.9 |
| East: Kalandar St (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 102 | 1.5 | 1.104 | 159.8 | LOS F | 46.8 | 333.6 | 1.00 | 1.39 | 8.1 |
| 5 T | 248 | 2.4 | 1.104 | 150.8 | LOS F | 46.8 | 333.6 | 1.00 | 1.39 | 7.6 |
| $6 \quad \mathrm{R}$ | 573 | 1.8 | 1.104 | 177.5 | LOS F | 49.9 | 354.4 | 1.00 | 1.37 | 7.4 |
| Approach | 923 | 2.0 | 1.104 | 168.4 | LOS F | 49.9 | 354.4 | 1.00 | 1.38 | 7.5 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 867 | 1.1 | $1.000^{3}$ | 34.2 | LOS C | 18.5 | 130.6 | 0.22 | 0.78 | 33.2 |
| 8 T | 1303 | 3.1 | 1.109 | 167.1 | LOS F | 75.1 | 539.5 | 1.00 | 1.60 | 11.4 |
| 9 R | 138 | 5.4 | 0.612 | 42.7 | LOS D | 5.8 | 42.4 | 0.99 | 0.79 | 29.3 |
| Approach | 2308 | 2.5 | 1.109 | 109.7 | LOS F | 75.1 | 539.5 | 0.71 | 1.24 | 15.7 |
| West: Kalandar St (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 109 | 9.5 | $1.000^{3}$ | 54.2 | LOS D | 13.7 | 99.2 | 0.94 | 0.90 | 21.7 |
| 11 T | 322 | 0.0 | 1.094 | 104.0 | LOS F | 52.0 | 366.2 | 0.97 | 1.13 | 11.8 |
| 12 R | 303 | 1.0 | 1.094 | 174.9 | LOS F | 52.0 | 366.2 | 1.00 | 1.44 | 8.6 |
| Approach | 735 | 1.8 | 1.094 | 125.8 | LOS F | 52.0 | 366.2 | 0.98 | 1.22 | 10.8 |
| All Vehicles | 4877 | 2.3 | 1.109 | 129.3 | LOS F | 75.1 | 539.5 | 0.86 | 1.27 | 12.6 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.
$3 x=1.00$ due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

| Movement Performance - Pedestrians |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back Pedestrian ped | Queue <br> Distance m | Prop. Queued | Effective Stop Rate per ped |
| P1 | Across S approach | 53 | 51.1 | LOS E | 0.2 | 0.2 | 0.90 | 0.90 |
| P3 | Across E approach | 53 | 36.9 | LOS D | 0.1 | 0.1 | 0.77 | 0.77 |
| P5 | Across N approach | 53 | 50.2 | LOS E | 0.2 | 0.2 | 0.90 | 0.90 |
| All Pedestrians |  | 159 | 46.0 | LOS E |  |  | 0.86 | 0.86 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

13S1231000 - West Culburra Subdivision
Princes Highway-Kalandar Street
Friday PM (1600-1700) - Equivalent 120th HH
Future - Full Site Development
Signals - Fixed Time Cycle Time = 125 seconds (Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |
| 1 L | 12 | 0.0 | 1.081 | 130.4 | LOS F | 32.8 | 234.8 | 1.00 | 1.20 | 13.9 |
| 2 T | 785 | 2.6 | 1.081 | 136.2 | LOS F | 41.9 | 300.0 | 1.00 | 1.30 | 13.4 |
| 3 R | 114 | 1.4 | 1.104 | 185.8 | LOS F | 12.7 | 89.6 | 1.00 | 1.22 | 10.0 |
| Approach | 911 | 2.4 | 1.104 | 142.3 | LOS F | 41.9 | 300.0 | 1.00 | 1.29 | 12.9 |
| East: Kalandar St (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 102 | 1.5 | 1.104 | 159.8 | LOS F | 46.8 | 333.6 | 1.00 | 1.39 | 8.1 |
| 5 T | 248 | 2.4 | 1.104 | 150.8 | LOS F | 46.8 | 333.6 | 1.00 | 1.39 | 7.6 |
| $6 \quad \mathrm{R}$ | 573 | 1.8 | 1.104 | 177.5 | LOS F | 49.9 | 354.4 | 1.00 | 1.37 | 7.4 |
| Approach | 923 | 2.0 | 1.104 | 168.4 | LOS F | 49.9 | 354.4 | 1.00 | 1.38 | 7.5 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 867 | 1.1 | $1.000^{3}$ | 34.2 | LOS C | 18.5 | 130.6 | 0.22 | 0.78 | 33.2 |
| 8 T | 1303 | 3.1 | 1.109 | 167.1 | LOS F | 75.1 | 539.5 | 1.00 | 1.60 | 11.4 |
| 9 R | 138 | 5.4 | 0.612 | 42.7 | LOS D | 5.8 | 42.4 | 0.99 | 0.79 | 29.3 |
| Approach | 2308 | 2.5 | 1.109 | 109.7 | LOS F | 75.1 | 539.5 | 0.71 | 1.24 | 15.7 |
| West: Kalandar St (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 109 | 9.5 | $1.000^{3}$ | 54.2 | LOS D | 13.7 | 99.2 | 0.94 | 0.90 | 21.7 |
| 11 T | 322 | 0.0 | 1.094 | 104.0 | LOS F | 52.0 | 366.2 | 0.97 | 1.13 | 11.8 |
| 12 R | 303 | 1.0 | 1.094 | 174.9 | LOS F | 52.0 | 366.2 | 1.00 | 1.44 | 8.6 |
| Approach | 735 | 1.8 | 1.094 | 125.8 | LOS F | 52.0 | 366.2 | 0.98 | 1.22 | 10.8 |
| All Vehicles | 4877 | 2.3 | 1.109 | 129.3 | LOS F | 75.1 | 539.5 | 0.86 | 1.27 | 12.6 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.
$3 x=1.00$ due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

| Movement Performance - Pedestrians |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back Pedestrian ped | Queue <br> Distance m | Prop. Queued | Effective Stop Rate per ped |
| P1 | Across S approach | 53 | 51.1 | LOS E | 0.2 | 0.2 | 0.90 | 0.90 |
| P3 | Across E approach | 53 | 36.9 | LOS D | 0.1 | 0.1 | 0.77 | 0.77 |
| P5 | Across N approach | 53 | 50.2 | LOS E | 0.2 | 0.2 | 0.90 | 0.90 |
| All Pedestrians |  | 159 | 46.0 | LOS E |  |  | 0.86 | 0.86 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

13S1231000 - West Culburra Subdivision
Coonamia Road- Currarong Road-Forest Road
Friday AM (0800-0900) - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{array}{r} \text { HV } \\ \% \end{array}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| East: Currarong Road |  |  |  |  |  |  |  |  |  |  |
| 5 T | 13 | 0.0 | 0.041 | 0.9 | LOS A | 0.2 | 1.2 | 0.32 | 0.00 | 79.0 |
| 6 R | 40 | 0.0 | 0.041 | 13.3 | LOS A | 0.2 | 1.2 | 0.32 | 0.77 | 68.3 |
| Approach | 53 | 0.0 | 0.041 | 10.4 | NA | 0.2 | 1.2 | 0.32 | 0.59 | 70.6 |
| North: Coonamia Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 7 | 0.0 | 0.012 | 13.1 | LOS A | 0.0 | 0.2 | 0.22 | 0.68 | 67.6 |
| 9 R | 85 | 1.8 | 0.123 | 14.3 | LOS A | 0.5 | 3.5 | 0.37 | 0.74 | 66.6 |
| Approach | 93 | 1.6 | 0.123 | 14.2 | LOS A | 0.5 | 3.5 | 0.36 | 0.73 | 66.7 |
| West: Forest Road |  |  |  |  |  |  |  |  |  |  |
| 10 L | 209 | 1.8 | 0.124 | 12.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.79 | 69.1 |
| 11 T | 19 | 6.3 | 0.124 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 100.0 |
| Approach | 228 | 2.2 | 0.124 | 11.7 | NA | 0.0 | 0.0 | 0.00 | 0.72 | 71.0 |
| All Vehicles | 374 | 1.7 | 0.124 | 12.1 | NA | 0.5 | 3.5 | 0.13 | 0.71 | 69.8 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Coonamia Road- Currarong Road-Forest Road
Friday PM (1600-1700) - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| East: Currarong Road |  |  |  |  |  |  |  |  |  |  |
| 5 T | 15 | 0.0 | 0.020 | 0.7 | LOS A | 0.1 | 0.7 | 0.30 | 0.00 | 81.8 |
| 6 R | 15 | 0.0 | 0.020 | 13.2 | LOS A | 0.1 | 0.7 | 0.30 | 0.86 | 69.4 |
| Approach | 29 | 0.0 | 0.020 | 7.0 | NA | 0.1 | 0.7 | 0.30 | 0.43 | 75.2 |
| North: Coonamia Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 37 | 0.0 | 0.059 | 13.1 | LOS A | 0.1 | 0.9 | 0.22 | 0.69 | 67.6 |
| 9 R | 200 | 4.0 | 0.274 | 14.4 | LOS A | 1.3 | 9.3 | 0.38 | 0.74 | 66.8 |
| Approach | 237 | 3.4 | 0.274 | 14.2 | LOS A | 1.3 | 9.3 | 0.35 | 0.73 | 66.9 |
| West: Forest Road |  |  |  |  |  |  |  |  |  |  |
| 10 L | 161 | 5.4 | 0.108 | 13.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.83 | 69.1 |
| 11 T | 34 | 4.3 | 0.108 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 100.0 |
| Approach | 195 | 5.2 | 0.108 | 10.8 | NA | 0.0 | 0.0 | 0.00 | 0.69 | 73.1 |
| All Vehicles | 461 | 4.0 | 0.274 | 12.3 | NA | 1.3 | 9.3 | 0.20 | 0.69 | 69.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Coonamia Road- Currarong Road-Forest Road
Saturday - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| East: Currarong Road |  |  |  |  |  |  |  |  |  |  |
| 5 T | 11 | 0.0 | 0.027 | 0.7 | LOS A | 0.1 | 0.8 | 0.29 | 0.00 | 80.9 |
| 6 R | 26 | 0.0 | 0.027 | 13.2 | LOS A | 0.1 | 0.8 | 0.29 | 0.78 | 68.6 |
| Approach | 37 | 0.0 | 0.027 | 9.6 | NA | 0.1 | 0.8 | 0.29 | 0.56 | 71.7 |
| North: Coonamia Road |  |  |  |  |  |  |  |  |  |  |
| 7 L | 29 | 0.0 | 0.047 | 13.0 | LOS A | 0.1 | 0.7 | 0.21 | 0.69 | 67.7 |
| 9 R | 174 | 1.8 | 0.238 | 14.1 | LOS A | 1.1 | 7.6 | 0.36 | 0.74 | 66.9 |
| Approach | 203 | 1.5 | 0.238 | 13.9 | LOS A | 1.1 | 7.6 | 0.34 | 0.73 | 67.0 |
| West: Forest Road |  |  |  |  |  |  |  |  |  |  |
| 10 L | 175 | 0.9 | 0.104 | 12.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.79 | 69.1 |
| 11 T | 19 | 0.0 | 0.104 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 100.0 |
| Approach | 194 | 0.8 | 0.104 | 11.4 | NA | 0.0 | 0.0 | 0.00 | 0.72 | 71.3 |
| All Vehicles | 434 | 1.1 | 0.238 | 12.4 | NA | 1.1 | 7.6 | 0.18 | 0.71 | 69.3 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.
$13 S 1231000$ - West Culburra Subdivision
Kalandar Street-Kinghorne Street-Albatross Road
Friday AM (0800-0900) - Equivalent 120th HH
Future - Full Site Development
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Kinghorne Sttreet |  |  |  |  |  |  |  |  |  |  |
| 1 L | 31 | 7.7 | 0.722 | 20.0 | LOS B | 10.1 | 72.6 | 0.98 | 1.13 | 39.5 |
| 2 T | 483 | 3.4 | 0.722 | 18.5 | LOS B | 10.1 | 72.6 | 0.98 | 1.12 | 39.7 |
| 3 R | 59 | 0.0 | 0.722 | 23.3 | LOS B | 10.1 | 72.6 | 0.98 | 1.12 | 37.9 |
| Approach | 573 | 3.3 | 0.722 | 19.1 | LOS B | 10.1 | 72.6 | 0.98 | 1.12 | 39.5 |
| East: Kalandar Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 327 | 7.5 | 0.437 | 7.9 | LOS A | 2.8 | 20.6 | 0.46 | 0.59 | 45.1 |
| 6 R | 161 | 2.3 | 0.437 | 12.6 | LOS A | 2.8 | 20.6 | 0.46 | 0.78 | 42.0 |
| Approach | 488 | 5.8 | 0.437 | 9.4 | LOS A | 2.8 | 20.6 | 0.46 | 0.65 | 44.0 |
| North: Kinghorne Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 129 | 3.7 | 0.329 | 8.8 | LOS A | 2.2 | 16.0 | 0.58 | 0.66 | 47.1 |
| 8 T | 31 | 0.0 | 0.329 | 7.9 | LOS A | 2.2 | 16.0 | 0.58 | 0.62 | 47.1 |
| 9 R | 176 | 3.4 | 0.329 | 12.0 | LOS A | 2.2 | 16.0 | 0.58 | 0.74 | 45.5 |
| Approach | 336 | 3.2 | 0.329 | 10.4 | LOS A | 2.2 | 16.0 | 0.58 | 0.70 | 46.2 |
| South West: Albatross Road |  |  |  |  |  |  |  |  |  |  |
| 30 L | 261 | 4.5 | 0.738 | 20.1 | LOS B | 9.6 | 70.6 | 1.00 | 1.19 | 38.2 |
| 32 R | 219 | 8.1 | 0.738 | 24.4 | LOS B | 9.6 | 70.6 | 1.00 | 1.20 | 37.0 |
| Approach | 480 | 6.2 | 0.738 | 22.1 | LOS B | 9.6 | 70.6 | 1.00 | 1.19 | 37.6 |
| All Vehicles | 1877 | 4.7 | 0.738 | 15.8 | LOS B | 10.1 | 72.6 | 0.78 | 0.94 | 40.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Project: P:\12S1200-1299\12S1231000 - West Cullburra MWT\ModellingISIDRA\130226sid-12S1231000 West

13S1231000 - West Culburra Subdivision
Kalandar Street-Kinghorne Street-Albatross Road
Friday AM (1600-1700) - Equivalent 120th HH
Future - Full Site Development
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles <br> veh | Queue <br> Distance <br> m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Kinghorne Sttreet |  |  |  |  |  |  |  |  |  |  |
| 1 L | 34 | 0.0 | 0.581 | 15.4 | LOS B | 5.7 | 40.2 | 0.90 | 1.00 | 42.7 |
| 2 T | 326 | 0.0 | 0.581 | 14.1 | LOS A | 5.7 | 40.2 | 0.90 | 0.98 | 42.9 |
| 3 R | 79 | 0.0 | 0.581 | 19.0 | LOS B | 5.7 | 40.2 | 0.90 | 1.02 | 40.7 |
| Approach | 439 | 0.0 | 0.581 | 15.1 | LOS B | 5.7 | 40.2 | 0.90 | 0.99 | 42.5 |
| East: Kalandar Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 280 | 0.0 | 0.440 | 8.9 | LOS A | 2.9 | 20.1 | 0.62 | 0.71 | 43.9 |
| 6 R | 116 | 0.0 | 0.440 | 13.8 | LOS A | 2.9 | 20.1 | 0.62 | 0.84 | 40.9 |
| Approach | 396 | 0.0 | 0.440 | 10.3 | LOS A | 2.9 | 20.1 | 0.62 | 0.75 | 43.0 |
| North: Kinghorne Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 322 | 0.0 | 0.782 | 15.7 | LOS B | 11.5 | 80.8 | 0.98 | 1.04 | 41.6 |
| 8 T | 80 | 0.0 | 0.782 | 14.9 | LOS B | 11.5 | 80.8 | 0.98 | 1.04 | 41.7 |
| 9 R | 304 | 0.0 | 0.782 | 18.9 | LOS B | 11.5 | 80.8 | 0.98 | 1.05 | 40.2 |
| Approach | 706 | 0.0 | 0.782 | 17.0 | LOS B | 11.5 | 80.8 | 0.98 | 1.05 | 41.0 |
| South West: Albatross Road |  |  |  |  |  |  |  |  |  |  |
| 30 L | 300 | 0.0 | 0.786 | 17.3 | LOS B | 11.7 | 82.1 | 1.00 | 1.13 | 40.0 |
| 32 R | 348 | 0.0 | 0.786 | 21.5 | LOS B | 11.7 | 82.1 | 1.00 | 1.13 | 38.6 |
| Approach | 648 | 0.0 | 0.786 | 19.5 | LOS B | 11.7 | 82.1 | 1.00 | 1.13 | 39.2 |
| All Vehicles | 2189 | 0.0 | 0.786 | 16.2 | LOS B | 11.7 | 82.1 | 0.91 | 1.01 | 41.0 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Project: P:\12S1200-1299\12S1231000 - West Cullburra MWT\ModellingISIDRA\130226sid-12S1231000 West

13S1231000 - West Culburra Subdivision
Kalandar Street-Kinghorne Street-Albatross Road
Saturday - Equivalent 120th HH
Future - Full Site Development
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Kinghorne Sttreet |  |  |  |  |  |  |  |  |  |  |
| 1 L | 21 | 12.5 | 0.226 | 10.6 | LOS A | 1.4 | 10.0 | 0.60 | 0.74 | 47.5 |
| 2 T | 162 | 1.6 | 0.226 | 8.8 | LOS A | 1.4 | 10.0 | 0.60 | 0.66 | 47.5 |
| 3 R | 29 | 0.0 | 0.226 | 13.7 | LOS A | 1.4 | 10.0 | 0.60 | 0.81 | 44.7 |
| Approach | 213 | 2.5 | 0.226 | 9.7 | LOS A | 1.4 | 10.0 | 0.60 | 0.69 | 47.1 |
| East: Kalandar Street |  |  |  |  |  |  |  |  |  |  |
| 4 L | 204 | 6.0 | 0.268 | 7.5 | LOS A | 1.4 | 10.4 | 0.37 | 0.56 | 45.7 |
| 6 R | 98 | 1.4 | 0.268 | 12.2 | LOS A | 1.4 | 10.4 | 0.37 | 0.77 | 42.3 |
| Approach | 302 | 4.5 | 0.268 | 9.0 | LOS A | 1.4 | 10.4 | 0.37 | 0.63 | 44.5 |
| North: Kinghorne Street |  |  |  |  |  |  |  |  |  |  |
| 7 L | 154 | 0.8 | 0.313 | 8.6 | LOS A | 2.0 | 14.2 | 0.53 | 0.65 | 47.4 |
| 8 T | 21 | 0.0 | 0.313 | 7.7 | LOS A | 2.0 | 14.2 | 0.53 | 0.60 | 47.5 |
| 9 R | 157 | 2.6 | 0.313 | 11.8 | LOS A | 2.0 | 14.2 | 0.53 | 0.74 | 45.6 |
| Approach | 332 | 1.6 | 0.313 | 10.0 | LOS A | 2.0 | 14.2 | 0.53 | 0.69 | 46.5 |
| South West: Albatross Road |  |  |  |  |  |  |  |  |  |  |
| 30 L | 161 | 1.4 | 0.377 | 8.0 | LOS A | 2.6 | 18.2 | 0.57 | 0.62 | 47.4 |
| 32 R | 237 | 1.7 | 0.377 | 12.2 | LOS A | 2.6 | 18.2 | 0.57 | 0.75 | 45.6 |
| Approach | 398 | 1.6 | 0.377 | 10.5 | LOS A | 2.6 | 18.2 | 0.57 | 0.70 | 46.3 |
| All Vehicles | 1244 | 2.5 | 0.377 | 9.9 | LOS A | 2.6 | 18.2 | 0.51 | 0.68 | 46.1 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Project: P:\12S1200-1299\12S1231000 - West Cullburra MWT\ModellingISIDRA\130226sid-12S1231000 West

13S1231000 - West Culburra Subdivision
Princes Highway-Forest Road
Friday AM (0800-0900) - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |
| 2 T | 1413 | 3.1 | 0.739 | 0.4 | X | X | X | X | 0.00 | 98.7 |
| 3 R | 37 | 13.0 | 0.058 | 17.3 | LOS B | 0.2 | 1.7 | 0.56 | 0.83 | 56.7 |
| Approach | 1449 | 3.4 | 0.739 | 0.8 | NA | 0.2 | 1.7 | 0.01 | 0.02 | 97.3 |
| South East: Forest Road (Median RT) |  |  |  |  |  |  |  |  |  |  |
| 23 R | 107 | 2.3 | 0.059 | 8.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 53.3 |
| Approach | 107 | 2.3 | 0.059 | 8.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 53.3 |
| East: Forest Road |  |  |  |  |  |  |  |  |  |  |
| 4 L | 36 | 3.3 | 0.078 | 13.1 | LOS A | 0.2 | 1.5 | 0.54 | 0.80 | 51.4 |
| 6 R | 107 | 2.3 | 0.227 | 14.9 | LOS B | 0.8 | 5.9 | 0.60 | 0.89 | 49.6 |
| Approach | 143 | 2.6 | 0.227 | 14.5 | LOS A | 0.8 | 5.9 | 0.59 | 0.87 | 50.1 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 60 | 8.3 | 0.034 | 13.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.76 | 63.3 |
| 8 T | 521 | 16.0 | 0.295 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 100.0 |
| Approach | 581 | 15.2 | 0.295 | 1.3 | NA | 0.0 | 0.0 | 0.00 | 0.08 | 95.6 |
| All Vehicles | 2281 | 6.3 | 0.739 | 2.1 | NA | 0.8 | 5.9 | 0.05 | 0.12 | 89.6 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Princes Highway-Forest Road
Friday PM (1600-1700) - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |
| 2 T | 640 | 6.0 | 0.341 | 0.1 | X | X | X | X | 0.00 | 99.8 |
| 3 R | 63 | 2.9 | 0.457 | 48.6 | LOS D | 1.6 | 11.4 | 0.95 | 1.03 | 30.9 |
| Approach | 703 | 5.7 | 0.457 | 4.4 | NA | 1.6 | 11.4 | 0.08 | 0.09 | 86.4 |
| South East: Forest Road (Median RT) |  |  |  |  |  |  |  |  |  |  |
| 23 R | 71 | 2.2 | 0.039 | 8.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 53.3 |
| Approach | 71 | 2.2 | 0.039 | 8.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 53.3 |
| East: Forest Road |  |  |  |  |  |  |  |  |  |  |
| 4 L | 60 | 0.0 | 0.551 | 57.0 | LOS E | 1.9 | 13.2 | 0.96 | 1.06 | 28.4 |
| 6 R | 71 | 2.2 | 0.721 | 75.2 | LOS F | 2.7 | 19.6 | 0.97 | 1.14 | 24.0 |
| Approach | 131 | 1.2 | 0.721 | 66.8 | LOS E | 2.7 | 19.6 | 0.96 | 1.10 | 25.9 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 160 | 2.0 | 0.087 | 12.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.75 | 63.3 |
| 8 T | 1475 | 2.0 | 0.766 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 100.0 |
| Approach | 1635 | 2.0 | 0.766 | 1.2 | NA | 0.0 | 0.0 | 0.00 | 0.07 | 95.8 |
| All Vehicles | 2539 | 3.0 | 0.766 | 5.7 | NA | 2.7 | 19.6 | 0.07 | 0.15 | 81.0 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Princes Highway-Forest Road
Saturday - Equivalent 120th HH
Future - Full Site Development
Giveway / Yield (Two-Way)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |
| 2 T | 812 | 1.7 | 0.421 | 0.1 | X | X | X | X | 0.00 | 99.7 |
| 3 R | 52 | 0.0 | 0.187 | 25.9 | LOS B | 0.6 | 4.4 | 0.84 | 0.97 | 45.6 |
| Approach | 863 | 1.6 | 0.421 | 1.6 | NA | 0.6 | 4.4 | 0.05 | 0.06 | 94.5 |
| South East: Forest Road (Median RT) |  |  |  |  |  |  |  |  |  |  |
| 23 R | 98 | 0.0 | 0.053 | 8.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 53.3 |
| Approach | 98 | 0.0 | 0.053 | 8.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.61 | 53.3 |
| East: Forest Road |  |  |  |  |  |  |  |  |  |  |
| 4 L | 62 | 0.0 | 0.280 | 26.6 | LOS B | 1.0 | 6.7 | 0.86 | 0.99 | 41.0 |
| $6 \quad \mathrm{R}$ | 98 | 3.0 | 0.536 | 36.3 | LOS C | 2.1 | 15.0 | 0.91 | 1.07 | 36.0 |
| Approach | 160 | 1.8 | 0.536 | 32.5 | LOS C | 2.1 | 15.0 | 0.89 | 1.04 | 37.8 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 137 | 0.0 | 0.074 | 12.5 | LOS A | 0.0 | 0.0 | 0.00 | 0.75 | 63.3 |
| 8 T | 1153 | 1.7 | 0.598 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 100.0 |
| Approach | 1289 | 1.5 | 0.598 | 1.3 | NA | 0.0 | 0.0 | 0.00 | 0.08 | 95.4 |
| All Vehicles | 2411 | 1.5 | 0.598 | 3.8 | NA | 2.1 | 15.0 | 0.08 | 0.16 | 84.9 |

X: Not applicable for Continuous movement.
Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
SIDRA Standard Delay Model used.

13S1231000 - West Culburra Subdivision
Princes Highway-Moss Street
Friday AM (0800-0900) - Equivalent 120th HH
Future - Full Site Development
Signals - Fixed Time Cycle Time $=135$ seconds (Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{aligned} & \text { HV } \\ & \% \end{aligned}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back of Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Princes Hwy (S) mill |  |  |  |  |  |  |  |  |  |  |
| 1 L | 5 | 0.0 | 0.974 | 100.6 | LOS F | 24.6 | 183.4 | 1.00 | 1.11 | 16.6 |
| 2 T | 928 | 7.6 | 1.054 | 122.4 | LOS F | 32.7 | 243.9 | 1.00 | 1.22 | 14.6 |
| 3 R | 158 | 3.7 | 0.982 | 109.7 | LOS F | 13.6 | 98.2 | 1.00 | 1.06 | 14.9 |
| Approach | 1092 | 7.0 | 1.054 | 120.4 | LOS F | 32.7 | 243.9 | 1.00 | 1.20 | 14.6 |
| East: Moss St (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 37 | 3.4 | 0.382 | 48.3 | LOS D | 4.0 | 28.9 | 0.80 | 0.77 | 25.3 |
| 5 T | 243 | 4.1 | 1.025 | 105.2 | LOS F | 47.6 | 345.5 | 0.97 | 1.17 | 13.7 |
| $6 \quad \mathrm{R}$ | 275 | 4.6 | 1.025 | 126.7 | LOS F | 47.6 | 345.5 | 1.00 | 1.28 | 13.6 |
| Approach | 555 | 4.3 | 1.025 | 112.1 | LOS F | 47.6 | 345.5 | 0.97 | 1.20 | 14.1 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 193 | 5.0 | $1.000^{3}$ | 55.8 | LOS D | 29.0 | 213.4 | 0.99 | 0.92 | 25.2 |
| 8 T | 1479 | 6.8 | 1.066 | 119.8 | LOS F | 63.7 | 471.6 | 1.00 | 1.32 | 14.7 |
| 9 R | 335 | 3.7 | 1.040 | 111.7 | LOS F | 27.1 | 195.8 | 1.00 | 1.13 | 14.7 |
| Approach | 2006 | 6.1 | 1.066 | 112.3 | LOS F | 63.7 | 471.6 | 1.00 | 1.25 | 15.3 |
| West: Moss St (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 126 | 10.3 | 0.268 | 22.5 | LOS B | 3.2 | 24.4 | 0.67 | 0.75 | 24.5 |
| 11 T | 134 | 3.6 | 0.535 | 54.1 | LOS D | 11.0 | 81.1 | 0.96 | 0.79 | 10.1 |
| 12 R | 49 | 11.9 | 0.535 | 61.9 | LOS E | 11.0 | 81.1 | 0.96 | 0.82 | 12.6 |
| Approach | 309 | 7.7 | 0.535 | 42.5 | LOS C | 11.0 | 81.1 | 0.84 | 0.78 | 14.5 |
| All Vehicles | 3962 | 6.2 | 1.066 | 109.1 | LOS F | 63.7 | 471.6 | 0.98 | 1.19 | 14.9 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.
$3 x=1.00$ due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

| Movement Performance - Pedestrians |  |  |  |  |  |  |  |  |
| :---: | :---: | ---: | :---: | :---: | :---: | ---: | ---: | ---: | ---: |
| Mov ID | Description | Demand <br> Flow <br> ped/h | Average <br> Delay <br> sec | Level of <br> Service | Average Back of Queue <br> Pedestrian <br> ped | Prop. <br> Distance <br> Queued | Effective <br> Stop Rate <br> per ped |  |
| P1 | Across S approach | 53 | 53.3 | LOS E | 0.2 | 0.2 | 0.89 | 0.89 |
| P3 | Across E approach | 53 | 40.8 | LOS E | 0.2 | 0.2 | 0.78 | 0.78 |
| P5 | Across N approach | 53 | 61.6 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 |
| P7 | Across W approach | 53 | 56.0 | LOS E | 0.2 | 0.2 | 0.91 | 0.91 |
| All Pedestrians | 212 | 53.0 | LOS E |  |  | 0.88 | 0.88 |  |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

13S1231000 - West Culburra Subdivision
Princes Highway-Moss Street
Friday PM (1600-1700) - Equivalent 120th HH
Future - Full Site Development
Signals - Fixed Time Cycle Time = 135 seconds (Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Princes Hwy (S) |  |  |  |  |  |  |  |  |  |  |
| L | 13 | 0.0 | 1.149 | 210.3 | LOS F | 42.7 | 306.5 | 1.00 | 1.43 | 8.8 |
| 2 T | 1120 | 3.1 | 1.243 | 265.7 | LOS F | 59.6 | 428.5 | 1.00 | 1.64 | 7.7 |
| 3 R | 115 | 0.0 | 1.191 | 266.0 | LOS F | 16.2 | 113.5 | 1.00 | 1.31 | 7.0 |
| Approach | 1247 | 2.8 | 1.243 | 265.2 | LOS F | 59.6 | 428.5 | 1.00 | 1.61 | 7.6 |
| East: Moss St (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 37 | 4.3 | 0.378 | 56.3 | LOS D | 4.0 | 28.7 | 0.87 | 0.77 | 23.2 |
| 5 T | 153 | 0.0 | 1.013 | 98.5 | LOS F | 33.6 | 237.5 | 0.97 | 1.11 | 14.3 |
| 6 R | 235 | 1.9 | 1.013 | 121.7 | LOS F | 33.6 | 237.5 | 1.00 | 1.25 | 14.0 |
| Approach | 424 | 1.4 | 1.013 | 107.7 | LOS F | 33.6 | 237.5 | 0.98 | 1.16 | 14.6 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 214 | 3.7 | $1.000^{3}$ | 62.2 | LOS E | 29.6 | 213.6 | 1.00 | 0.95 | 23.3 |
| 8 T | 1525 | 3.5 | 1.238 | 244.3 | LOS F | 97.0 | 699.1 | 1.00 | 1.74 | 8.2 |
| 9 R | 240 | 0.7 | 1.167 | 209.9 | LOS F | 27.8 | 195.8 | 1.00 | 1.29 | 8.7 |
| Approach | 1978 | 3.0 | 1.238 | 220.4 | LOS F | 97.0 | 699.1 | 1.00 | 1.60 | 8.8 |
| West: Moss St (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 160 | 1.2 | 0.330 | 19.1 | LOS B | 3.4 | 24.2 | 0.61 | 0.75 | 26.5 |
| 11 T | 259 | 0.0 | 1.216 | 268.9 | LOS F | 112.8 | 794.8 | 1.00 | 1.74 | 2.5 |
| 12 R | 135 | 1.1 | 1.216 | 276.3 | LOS F | 112.8 | 794.8 | 1.00 | 1.74 | 3.3 |
| Approach | 900 | 0.8 | 1.216 | 227.9 | LOS F | 112.8 | 794.8 | 0.89 | 1.45 | 4.0 |
| All Vehicles | 4549 | 2.4 | 1.243 | 223.2 | LOS F | 112.8 | 794.8 | 0.98 | 1.42 | 8.2 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.
$3 x=1.00$ due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Movement Performance - Pedestrians

| Mov ID | Description | Demand <br> Flow <br> ped/h | Average <br> Delay <br> sec | Level of <br> Service | Average Back of Queue <br> Pedestrian | Prop. <br> Distance | Effective <br> Queued <br> Stop Rate <br> per ped |  |
| :---: | :---: | :---: | :---: | :---: | :---: | ---: | ---: | ---: |
| P1 | Across S approach | 53 | 61.6 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 |
| P3 | Across E approach | 53 | 44.0 | LOS E | 0.2 | 0.2 | 0.81 | 0.81 |
| P5 | Across N approach | 53 | 45.6 | LOS E | 0.2 | 0.2 | 0.82 | 0.82 |
| P7 | Across W approach | 53 | 56.0 | LOS E | 0.2 | 0.2 | 0.91 | 0.91 |
| All Pedestrians | 212 | 51.8 | LOS E |  |  | 0.87 | 0.87 |  |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

13S1231000 - West Culburra Subdivision
Princes Highway-Moss Street
Saturday - Equivalent 120th HH
Future - Full Site Development
Signals - Fixed Time Cycle Time = 135 seconds (Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Princes Hwy (S) 0 |  |  |  |  |  |  |  |  |  |  |
| 1 L | 14 | 0.0 | 0.807 | 64.6 | LOS E | 23.7 | 169.6 | 0.97 | 0.93 | 23.3 |
| 2 T | 1183 | 2.7 | 0.873 | 58.7 | LOS E | 27.8 | 198.7 | 0.99 | 0.94 | 24.2 |
| 3 R | 104 | 0.0 | 0.583 | 73.8 | LOS F | 6.8 | 47.7 | 1.00 | 0.79 | 20.0 |
| Approach | 1301 | 2.4 | 0.873 | 60.0 | LOS E | 27.8 | 198.7 | 0.99 | 0.93 | 23.8 |
| East: Moss St (E) |  |  |  |  |  |  |  |  |  |  |
| 4 L | 35 | 0.0 | 0.243 | 52.6 | LOS D | 2.6 | 18.1 | 0.83 | 0.75 | 24.0 |
| 5 T | 85 | 0.0 | 0.651 | 53.7 | LOS D | 13.9 | 98.5 | 0.96 | 0.79 | 20.7 |
| $6 \quad \mathrm{R}$ | 155 | 1.8 | 0.651 | 63.0 | LOS E | 13.9 | 98.5 | 0.98 | 0.83 | 21.7 |
| Approach | 275 | 1.0 | 0.651 | 58.8 | LOS E | 13.9 | 98.5 | 0.96 | 0.81 | 21.7 |
| North: Princes Hwy (N) |  |  |  |  |  |  |  |  |  |  |
| 7 L | 121 | 2.3 | 0.737 | 45.2 | LOS D | 19.6 | 139.6 | 0.79 | 0.95 | 28.8 |
| 8 T | 1313 | 1.5 | 0.737 | 37.7 | LOS C | 26.8 | 189.8 | 0.85 | 0.76 | 31.1 |
| $9 \quad \mathrm{R}$ | 258 | 1.0 | 0.858 | 49.0 | LOS D | 11.9 | 84.2 | 1.00 | 0.91 | 26.2 |
| Approach | 1692 | 1.5 | 0.858 | 40.0 | LOS C | 26.8 | 189.8 | 0.87 | 0.80 | 30.2 |
| West: Moss St (W) |  |  |  |  |  |  |  |  |  |  |
| 10 L | 311 | 0.0 | 0.609 | 24.2 | LOS B | 8.2 | 57.2 | 0.78 | 0.80 | 23.3 |
| 11 T | 163 | 0.0 | 0.755 | 58.6 | LOS E | 17.3 | 121.4 | 1.00 | 0.89 | 9.5 |
| 12 R | 103 | 1.3 | 0.755 | 66.1 | LOS E | 17.3 | 121.4 | 1.00 | 0.89 | 11.9 |
| Approach | 577 | 0.2 | 0.755 | 41.4 | LOS C | 17.3 | 121.4 | 0.88 | 0.84 | 15.3 |
| All Vehicles | 3844 | 1.6 | 0.873 | 48.3 | LOS D | 27.8 | 198.7 | 0.92 | 0.85 | 25.3 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
SIDRA Standard Delay Model used.

| Movement Performance - Pedestrians |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back Pedestrian ped | Queue <br> Distance m | Prop. Queued | Effective Stop Rate per ped |
| P1 | Across S approach | 53 | 61.6 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 |
| P3 | Across E approach | 53 | 34.8 | LOS D | 0.1 | 0.1 | 0.72 | 0.72 |
| P5 | Across N approach | 53 | 61.6 | LOS F | 0.2 | 0.2 | 0.96 | 0.96 |
| P7 | Across W approach | 53 | 46.5 | LOS E | 0.2 | 0.2 | 0.83 | 0.83 |
| All Pedestrians |  | 212 | 51.1 | LOS E |  |  | 0.86 | 0.86 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)
Pedestrian movement LOS values are based on average delay per pedestrian movement.
Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

12S1231000 West Culburra Subdivision
Culburra Road - Collector Road-East
Friday AM (0800-0900) - Equivalent 120th HH
Future - Full Site Development
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | f Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| East: Culburra Road - East Approach |  |  |  |  |  |  |  |  |  |  |
| 5 T | 277 | 2.5 | 0.212 | 2.5 | LOS A | 1.3 | 9.3 | 0.02 | 0.25 | 47.1 |
| $6 \quad \mathrm{R}$ | 98 | 1.0 | 0.212 | 8.8 | LOS A | 1.3 | 9.3 | 0.02 | 0.90 | 41.8 |
| Approach | 375 | 2.1 | 0.212 | 4.1 | LOS A | 1.3 | 9.3 | 0.02 | 0.42 | 45.5 |
| North: East Access - North Approach |  |  |  |  |  |  |  |  |  |  |
| 7 L | 311 | 1.0 | 0.254 | 4.8 | LOS A | 1.5 | 10.5 | 0.42 | 0.49 | 43.6 |
| 9 R | 1 | 1.0 | 0.254 | 9.9 | LOS A | 1.5 | 10.5 | 0.42 | 0.76 | 41.3 |
| Approach | 312 | 1.0 | 0.254 | 4.8 | LOS A | 1.5 | 10.5 | 0.42 | 0.49 | 43.6 |
| West: Culburra Road - West Approach |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 1.0 | 0.155 | 4.1 | LOS A | 0.8 | 5.6 | 0.24 | 0.44 | 44.8 |
| 11 T | 213 | 2.5 | 0.155 | 2.9 | LOS A | 0.8 | 5.6 | 0.24 | 0.32 | 45.6 |
| Approach | 214 | 2.5 | 0.155 | 2.9 | LOS A | 0.8 | 5.6 | 0.24 | 0.32 | 45.6 |
| All Vehicles | 900 | 1.8 | 0.254 | 4.1 | LOS A | 1.5 | 10.5 | 0.21 | 0.42 | 44.8 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Processed: Wednesday, 13 March 2013 2:09:41 PM SIDRA INTERSECTION 5.1.13.2093

SIDRA
INTERSECTION

12S1231000 West Culburra Subdivision
Culburra Road - Collector Road-East
Friday PM (1600-1700) - Equivalent 120th HH
Future - Full Site Development
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | f Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| East: Culburra Road - East Approach |  |  |  |  |  |  |  |  |  |  |
| 5 T | 193 | 2.5 | 0.285 | 2.5 | LOS A | 2.0 | 13.9 | 0.02 | 0.23 | 47.0 |
| $6 \quad \mathrm{R}$ | 312 | 1.0 | 0.285 | 8.8 | LOS A | 2.0 | 13.9 | 0.02 | 0.76 | 41.8 |
| Approach | 504 | 1.6 | 0.285 | 6.4 | LOS A | 2.0 | 13.9 | 0.02 | 0.56 | 43.5 |
| North: East Access - North Approach |  |  |  |  |  |  |  |  |  |  |
| 7 L | 104 | 1.0 | 0.099 | 5.3 | LOS A | 0.6 | 3.9 | 0.51 | 0.54 | 43.1 |
| 9 R | 1 | 1.0 | 0.099 | 10.4 | LOS A | 0.6 | 3.9 | 0.51 | 0.76 | 41.1 |
| Approach | 105 | 1.0 | 0.099 | 5.4 | LOS A | 0.6 | 3.9 | 0.51 | 0.54 | 43.1 |
| West: Culburra Road - West Approach |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 1.0 | 0.319 | 5.4 | LOS A | 1.9 | 13.5 | 0.51 | 0.58 | 43.7 |
| 11 T | 365 | 2.5 | 0.319 | 4.3 | LOS A | 1.9 | 13.5 | 0.51 | 0.47 | 43.9 |
| Approach | 366 | 2.5 | 0.319 | 4.3 | LOS A | 1.9 | 13.5 | 0.51 | 0.47 | 43.9 |
| All Vehicles | 976 | 1.9 | 0.319 | 5.5 | LOS A | 2.0 | 13.9 | 0.26 | 0.52 | 43.6 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Processed: Wednesday, 13 March 2013 2:09:42 PM SIDRA INTERSECTION 5.1.13.2093

SIDRA
INTERSECTION

12S1231000 West Culburra Subdivision
Culburra Road - Collector Road-East
Saturday - Equivalent 120th HH
Future - Full Site Development
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back Vehicles veh | f Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| East: Culburra Road - East Approach |  |  |  |  |  |  |  |  |  |  |
| 5 T | 251 | 2.5 | 0.255 | 2.5 | LOS A | 1.7 | 11.8 | 0.02 | 0.24 | 47.0 |
| $6 \quad \mathrm{R}$ | 201 | 1.0 | 0.255 | 8.8 | LOS A | 1.7 | 11.8 | 0.02 | 0.82 | 41.8 |
| Approach | 452 | 1.8 | 0.255 | 5.3 | LOS A | 1.7 | 11.8 | 0.02 | 0.50 | 44.4 |
| North: East Access - North Approach |  |  |  |  |  |  |  |  |  |  |
| 7 L | 201 | 1.0 | 0.171 | 4.8 | LOS A | 1.0 | 6.8 | 0.43 | 0.49 | 43.6 |
| 9 R | 1 | 1.0 | 0.171 | 9.9 | LOS A | 1.0 | 6.8 | 0.43 | 0.75 | 41.3 |
| Approach | 202 | 1.0 | 0.171 | 4.8 | LOS A | 1.0 | 6.8 | 0.43 | 0.49 | 43.5 |
| West: Culburra Road - West Approach |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 1.0 | 0.194 | 4.6 | LOS A | 1.0 | 7.4 | 0.37 | 0.50 | 44.3 |
| 11 T | 240 | 2.5 | 0.194 | 3.5 | LOS A | 1.0 | 7.4 | 0.37 | 0.38 | 44.8 |
| Approach | 241 | 2.5 | 0.194 | 3.5 | LOS A | 1.0 | 7.4 | 0.37 | 0.38 | 44.8 |
| All Vehicles | 895 | 1.8 | 0.255 | 4.7 | LOS A | 1.7 | 11.8 | 0.21 | 0.47 | 44.3 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Processed: Wednesday, 13 March 2013 2:09:42 PM SIDRA INTERSECTION 5.1.13.2093

SIDRA
INTERSECTION

12S1231000 West Culburra Subdivision
Culburra Road - Collector Road-East - Golf Course
Friday AM (0800-0900) - Equivalent 120th HH
Future - Full Site Development
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue <br> Distance <br> m | Prop. Queued | Effective Stop Rate per veh | Average Speed km/h |
| South: Golf Course Access (S) |  |  |  |  |  |  |  |  |  |  |
| L | 4 | 0.0 | 0.009 | 7.1 | LOS A | 0.0 | 0.3 | 0.45 | 0.50 | 48.7 |
| 2 T | 1 | 0.0 | 0.009 | 6.0 | LOS A | 0.0 | 0.3 | 0.45 | 0.44 | 49.1 |
| 3 R | 4 | 0.0 | 0.009 | 12.7 | LOS A | 0.0 | 0.3 | 0.45 | 0.68 | 45.5 |
| Approach | 9 | 0.0 | 0.009 | 9.5 | LOS A | 0.0 | 0.3 | 0.45 | 0.57 | 47.2 |
| East: Culburra Road - East Approach |  |  |  |  |  |  |  |  |  |  |
| 4 L | 15 | 0.0 | 0.237 | 5.7 | LOS A | 1.4 | 10.0 | 0.10 | 0.48 | 51.4 |
| 5 T | 277 | 2.5 | 0.237 | 2.6 | LOS A | 1.4 | 10.0 | 0.10 | 0.25 | 46.5 |
| 6 R | 98 | 1.0 | 0.237 | 8.9 | LOS A | 1.4 | 10.0 | 0.10 | 0.84 | 41.7 |
| Approach | 389 | 2.0 | 0.237 | 4.3 | LOS A | 1.4 | 10.0 | 0.10 | 0.41 | 45.3 |
| North: East Access - North Approach |  |  |  |  |  |  |  |  |  |  |
| 7 L | 311 | 1.0 | 0.259 | 4.9 | LOS A | 1.5 | 10.8 | 0.44 | 0.50 | 43.5 |
| 8 T | 1 | 0.0 | 0.259 | 5.7 | LOS A | 1.5 | 10.8 | 0.44 | 0.50 | 49.5 |
| 9 R | 1 | 1.0 | 0.259 | 10.0 | LOS A | 1.5 | 10.8 | 0.44 | 0.76 | 41.2 |
| Approach | 313 | 1.0 | 0.259 | 4.9 | LOS A | 1.5 | 10.8 | 0.44 | 0.50 | 43.5 |
| West: Culburra Road - West Approach |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 1.0 | 0.166 | 4.1 | LOS A | 0.9 | 6.2 | 0.26 | 0.44 | 44.7 |
| 11 T | 213 | 2.5 | 0.166 | 3.0 | LOS A | 0.9 | 6.2 | 0.26 | 0.32 | 45.4 |
| 12 R | 15 | 0.0 | 0.166 | 11.6 | LOS A | 0.9 | 6.2 | 0.26 | 0.85 | 46.6 |
| Approach | 228 | 2.3 | 0.166 | 3.5 | LOS A | 0.9 | 6.2 | 0.26 | 0.36 | 45.5 |
| All Vehicles | 940 | 1.7 | 0.259 | 4.3 | LOS A | 1.5 | 10.8 | 0.25 | 0.43 | 44.8 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Processed: Wednesday, 13 March 2013 2:09:42 PM SIDRA INTERSECTION 5.1.13.2093 Project: P:\12S1200-1299\12S1231000 - West Cullburra MWT\Modelling\SIDRA\130313sid12S1231000 CulburraCollector Rd East.sip 8000056, GTA CONSULTANTS, ENTERPRISE

12S1231000 West Culburra Subdivision
Culburra Road - Collector Road-East - Golf Course
Friday PM (1600-1700) - Equivalent 120th HH
Future - Full Site Development
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \text { HV } \\ \% \end{gathered}$ | Deg. Satn v/c | Average Delay sec | Level of Service | 95\% Back <br> Vehicles <br> veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Golf Course Access (S) |  |  |  |  |  |  |  |  |  |  |
| 1 L | 23 | 0.0 | 0.047 | 7.9 | LOS A | 0.2 | 1.7 | 0.53 | 0.58 | 48.1 |
| 2 T | 1 | 0.0 | 0.047 | 6.8 | LOS A | 0.2 | 1.7 | 0.53 | 0.53 | 48.3 |
| 3 R | 23 | 0.0 | 0.047 | 13.5 | LOS A | 0.2 | 1.7 | 0.53 | 0.73 | 44.9 |
| Approach | 47 | 0.0 | 0.047 | 10.6 | LOS A | 0.2 | 1.7 | 0.53 | 0.65 | 46.4 |
| East: Culburra Road - East Approach |  |  |  |  |  |  |  |  |  |  |
| 4 L | 6 | 0.0 | 0.298 | 5.6 | LOS A | 2.0 | 14.2 | 0.07 | 0.45 | 51.5 |
| 5 T | 193 | 2.5 | 0.298 | 2.5 | LOS A | 2.0 | 14.2 | 0.07 | 0.23 | 46.6 |
| 6 R | 312 | 1.0 | 0.298 | 8.8 | LOS A | 2.0 | 14.2 | 0.07 | 0.73 | 41.7 |
| Approach | 511 | 1.6 | 0.298 | 6.4 | LOS A | 2.0 | 14.2 | 0.07 | 0.54 | 43.4 |
| North: East Access - North Approach |  |  |  |  |  |  |  |  |  |  |
| 7 L | 104 | 1.0 | 0.103 | 5.5 | LOS A | 0.6 | 4.1 | 0.53 | 0.55 | 43.0 |
| 8 T | 1 | 0.0 | 0.103 | 6.3 | LOS A | 0.6 | 4.1 | 0.53 | 0.55 | 48.7 |
| 9 R | 1 | 1.0 | 0.103 | 10.6 | LOS A | 0.6 | 4.1 | 0.53 | 0.76 | 41.0 |
| Approach | 106 | 1.0 | 0.103 | 5.5 | LOS A | 0.6 | 4.1 | 0.53 | 0.55 | 43.0 |
| West: Culburra Road - West Approach |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 1.0 | 0.331 | 5.6 | LOS A | 2.0 | 14.3 | 0.53 | 0.60 | 43.5 |
| 11 T | 365 | 2.5 | 0.331 | 4.4 | LOS A | 2.0 | 14.3 | 0.53 | 0.49 | 43.7 |
| 12 R | 6 | 0.0 | 0.331 | 13.1 | LOS A | 2.0 | 14.3 | 0.53 | 0.87 | 46.0 |
| Approach | 373 | 2.5 | 0.331 | 4.6 | LOS A | 2.0 | 14.3 | 0.53 | 0.50 | 43.8 |
| All Vehicles | 1037 | 1.7 | 0.331 | 5.9 | LOS A | 2.0 | 14.3 | 0.31 | 0.53 | 43.6 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Processed: Wednesday, 13 March 2013 4:25:01 PM
SIDRA INTERSECTION 5.1.13.2093
Project: P:\12S1200-1299\12S1231000 - West Cullburra MWT\ModellingISIDRA\130313sid12S1231000 Culburra-
Collector Rd East.sip
8000056, GTA CONSULTANTS, ENTERPRISE

12S1231000 West Culburra Subdivision
Culburra Road - Collector Road-East - Golf Course
Saturday - Equivalent 120th HH
Future - Full Site Development
Roundabout

| Movement Performance - Vehicles |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov ID Turn | Demand Flow veh/h | $\begin{gathered} \mathrm{HV} \\ \% \end{gathered}$ | Deg. <br> v/c | Average Delay sec | Level of Service | 95\% Back of <br> Vehicles veh | Queue Distance m | Prop. Queued | Effective Stop Rate per veh | Average Speed $\mathrm{km} / \mathrm{h}$ |
| South: Golf Course Access (S) |  |  |  |  |  |  |  |  |  |  |
| 1 L | 18 | 0.0 | 0.036 | 7.6 | LOS A | 0.2 | 1.3 | 0.51 | 0.56 | 48.3 |
| 2 T | 1 | 0.0 | 0.036 | 6.4 | LOS A | 0.2 | 1.3 | 0.51 | 0.50 | 48.6 |
| 3 R | 18 | 0.0 | 0.036 | 13.2 | LOS A | 0.2 | 1.3 | 0.51 | 0.72 | 45.2 |
| Approach | 37 | 0.0 | 0.036 | 10.2 | LOS A | 0.2 | 1.3 | 0.51 | 0.63 | 46.7 |
| East: Culburra Road - East Approach |  |  |  |  |  |  |  |  |  |  |
| 4 L | 18 | 0.0 | 0.286 | 5.7 | LOS A | 1.9 | 13.3 | 0.12 | 0.46 | 51.2 |
| 5 T | 251 | 2.5 | 0.286 | 2.6 | LOS A | 1.9 | 13.3 | 0.12 | 0.24 | 46.3 |
| 6 R | 201 | 1.0 | 0.286 | 8.9 | LOS A | 1.9 | 13.3 | 0.12 | 0.77 | 41.6 |
| Approach | 469 | 1.8 | 0.286 | 5.4 | LOS A | 1.9 | 13.3 | 0.12 | 0.48 | 44.2 |
| North: East Access - North Approach |  |  |  |  |  |  |  |  |  |  |
| 7 L | 201 | 1.0 | 0.177 | 5.0 | LOS A | 1.0 | 7.1 | 0.46 | 0.51 | 43.4 |
| 8 T | 1 | 0.0 | 0.177 | 5.8 | LOS A | 1.0 | 7.1 | 0.46 | 0.51 | 49.3 |
| 9 R | 1 | 1.0 | 0.177 | 10.1 | LOS A | 1.0 | 7.1 | 0.46 | 0.76 | 41.2 |
| Approach | 203 | 1.0 | 0.177 | 5.0 | LOS A | 1.0 | 7.1 | 0.46 | 0.52 | 43.4 |
| West: Culburra Road - West Approach |  |  |  |  |  |  |  |  |  |  |
| 10 L | 1 | 1.0 | 0.212 | 4.7 | LOS A | 1.2 | 8.3 | 0.40 | 0.51 | 44.1 |
| 11 T | 240 | 2.5 | 0.212 | 3.6 | LOS A | 1.2 | 8.3 | 0.40 | 0.40 | 44.5 |
| 12 R | 18 | 0.0 | 0.212 | 12.3 | LOS A | 1.2 | 8.3 | 0.40 | 0.84 | 46.4 |
| Approach | 259 | 2.3 | 0.212 | 4.2 | LOS A | 1.2 | 8.3 | 0.40 | 0.43 | 44.6 |
| All Vehicles | 968 | 1.7 | 0.286 | 5.2 | LOS A | 1.9 | 13.3 | 0.28 | 0.48 | 44.3 |

Level of Service (LOS) Method: Delay (RTA NSW).
Vehicle movement LOS values are based on average delay per movement
Intersection and Approach LOS values are based on average delay for all vehicle movements.
Roundabout Capacity Model: SIDRA Standard.
SIDRA Standard Delay Model used.

Processed: Wednesday, 13 March 2013 4:26:11 PM
SIDRA INTERSECTION 5.1.13.2093
Project: P:\12S1200-1299\12S1231000 - West Cullburra MWT\ModellingISIDRA\130313sid12S1231000 Culburra-
Collector Rd East.sip
8000056, GTA CONSULTANTS, ENTERPRISE

## AppendixE

## SIDRA INTERSEC TION Layouts




Culburra Road (S)






Appendix F

## Appendix F

Preliminary Intersection Concept Design


Appendix G

## Appendix G

## Bus Operator Correspondence



## Ken Hollyoak <br> Associate Director <br> GTA Consultants <br> PO Box 5254 <br> West Chatswood NSW 1515

## Re: Proposed Residential Development - Culburra

As a follow up from our initial discussion relating to your proposed development of 900 home sites in Culburra, the following information is provided.

Kennedys Bus \& Coach is the contracted provider for Transport for NSW in supplying school \& route services to Culburra / Orient Point. Additional patronage from your development would be welcomed and approval to include this development within our normal operations can be easily arranged. If the Development Proposal is approved we would be happy to extend our current service arrangements. There would be no additional cost to the government undercurrent contract arrangements.

It is important that you be aware in the design of this residential housing estate, that development enables bus stops to be located within 400 metre from access points / dwellings. The geometry of the road must be designed in order to allow for a minimum 12.5 metre to 13.5 metre bus to circulate through the proposed development.

Consideration also needs to be had relating to provision of Disability Access for low floor wheelchair buses which will be compulsory on all route services by 2020.

We look forward to providing services to this proposed new development. If I can be of any further assistance please contact me on the number below.

Yours sincerely

12th September 2072

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[^0]:    1 Based on the peak hour traffic counts undertaken by Skyhigh in May 2012 and assuming a peak-to-daily ratio of $8 \%$ for arterial roads and $10 \%$ for local roads.

[^1]:    2 Program used under license from Akcelik \& Associates Pty Ltd.

[^2]:    Fri AM Peak Conversion Factor for AADT Analysis: 0.94 (Converts consultant's 2012 survey data to AADT values for LOS C analysis) Fri PM Peak Conversion Factor for AADT Analysis: Sat MD Peak Conversion Factor for AADT Analysis: 0.95 (Converts consultant's 2012 survey data to AADT values for LOS C analysis) 1.14 (Converts consultant's 2012 survey data to AADT values for LOS C analysis)

