

#### Memorandum

To: WINIM Developments on behalf of Catholic Education Diocese of Parramatta

From: The Transport Planning Partnership (TTPP)

Date: 2 August 2021

TTPP REF: 18173

## RE: WESTMEAD CATHOLIC COMMUNITY SIDRA MODELLING CALIBRATION AND VALIDATION

As requested, please find herein calibration and validation details of the SIDRA network modelling undertaken for the Westmead Catholic Community Education Precinct State Significant Development Application (SSD-10383).

#### **Transport and Accessibility Impact Assessment**

A traffic network model has been prepared to assess the impacts of the additional traffic generated by the proposed development. The modelling analysis was undertaken as part of a Transport and Accessibility Impact Assessment (TAIA) for the proposed development.

SIDRA Network 8 modelling software has been used for this assessment.

The intersections included in the SIDRA model are as follows and as shown in Figure 1.

- 1. Hawkesbury Road Alexandra Avenue.
- 2. Hawkesbury Road Railway Parade.
- 3. Hawkesbury Road Darcy Road.
- 4. Darcy Road UWS Car Park Westmead Hospital.
- 5. Darcy Road Site Access (proposed car park entry).
- 6. Darcy Road Site Access (Catherine McAuley) Westmead Hospital.
- 7. Darcy Road Site Access (Catherine McAuley).
- 8. Darcy Road Mons Road Institute Road.
- 9. Darcy Road Site Access (Mother Teresa).
- 10. Darcy Road Bridge Road Coles Car Park.
- 11. Alexandra Avenue Bridge Road.





Figure 1: Darcy Road Automatic Tube Count Survey Results

#### **Base Model Data**

The existing traffic models have been developed using SIDRA Intersection 8 modelling software and road network and intersection configuration and traffic volumes collected on the following periods:

- Wednesday 17 October 2018 7.30am-9.00am
- Wednesday 17 October 2018 2.30pm-5.00pm.

Traffic turning movement surveys in the abovementioned periods were accompanied by an automatic tube count on Darcy Road across a period of 24 hours per day for seven days. The automatic tube count data identified the road network peak period occur between 7.00am-8.00am and 3.00pm-4.00pm. The survey results are shown graphically in Figure 2 while a breakdown of the hourly vehicle trips is presented in Table 1.



Figure 2: Darcy Road Automatic Tube Count Survey Results



Table 1: Darcy Road Hourly Traffic Volumes

Starting Hour	Vehicles per Hour	Starting Hour	Vehicles per Hour
00:00	396	12:00	5466
01:00	257	13:00	5814
02:00	142	14:00	6525
03:00	169	15:00 (PM Peak)	7940
04:00	410	16:00	6725
05:00	1651	17:00	5896
06:00	4484	18:00	4464
07:00	7429	19:00	3311
08:00 (AM Peak)	8805	20:00	2712
09:00	5858	21:00	2439
10:00	5397	22:00	1745
11:00	5222	23:00	826



As identified in Figure 2 and Table 1, the surrounding road network peak periods occur between 8.00am-9.00am and 3.00pm-4.00pm.

Notably, traffic volumes in the identified peak hour (3pm-4pm) are 18% and 35% greater than the 4pm-5pm and 5pm-6pm typical commuter peak periods.

In the morning, traffic volumes in the identified peak hour (8am-9am) is 19% greater than the 7am-8am period.

Since automatic tube count data is recorded on a 60-minute basis, the above-mentioned peak hours have been reviewed against the turning movement survey data (which is recorded on a 15-minute basis). According to the turning movement survey data, the surrounding road network peak periods occur between 7.45am-8.45am and 3.00pm-4.00pm.

These peak periods correlate identically with the Darcy Road automatic tube count survey results, with the exception of a minor 15-minute period in the morning peak causing the peak hour to commence at 7.45am. Since turning movement surveys capture data more specifically on a 15-minute basis, the latter peak periods are taken as the local road network peak periods, namely:

AM peak hour: 7.45am-8.45am

• PM peak hour: 3.00pm-4.00pm.

#### Base Model Calibration and Validation

In accordance with industry-wide practice, SCATS History data and LX data has been used to develop, calibrate and validate the SIDRA base case model. This methodology has been applied in accordance with submissions received from TfNSW and peer review comments made by Bitzios.

The previous SIDRA modelling has been updated based on SCATS History data, .LX data, and Traffic Control Signal (TCS) plans for each signalised intersection. The model inputs include the following:

- Signal phasing arrangement.
- Phase timing.
- Phase frequency.
- Cycle times.
- Common control groups.
- Signal coordination and offsets.

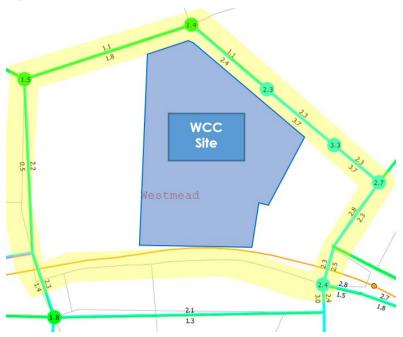
The model inputs for each of the above parameters is presented late in this report.



#### **Background Growth**

Traffic growth rates used in developing the future base models are based on the 2026 and 2036 Strategic Traffic Forecasting Model (STFM) plots received from Transport for NSW (TfNSW). STFM plots in the AM and PM peak periods (across 2-hours) are shown in Figure 4 to Figure 6.

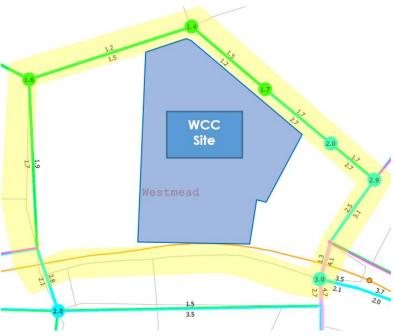
Figure 3: STFM Plots: 2026 AM Peak



2011TZ SYDNEY GMA STRATEGIC TRAFFIC FORECASTING MODEL Scenario 2026: 2026 SYDNEY TRAFFIC FORECASTING MODEL(LU2016V1.3)7-9AM(mf34)



Figure 4: STFM Plots: 2026 PM Peak



2011TZ SYDNEY GMA STRATEGIC TRAFFIC FORECASTING MODEL Scenario 20260: 2026 SYDNEY TRAFFIC FORECASTING MODEL(LU2016V1.3)4-6PM(mf54)

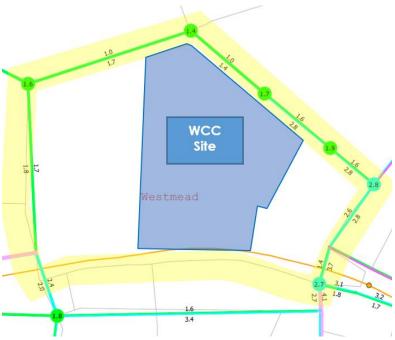
Figure 5: STFM Plots: 2036 AM Peak



2011TZ SYDNEY GMA STRATEGIC TRAFFIC FORECASTING MODEL Scenario 2036: 2036 SYDNEY TRAFFIC FORECASTING MODEL(LU2016V1.3)7-9AM(mf36)



Figure 6: STFM Plots: 2036 PM Peak



2011TZ SYDNEY GMA STRATEGIC TRAFFIC FORECASTING MODEL Scenario 20360: 2036 SYDNEY TRAFFIC FORECASTING MODEL(LU2016V1.3)4-6PM(mf56)

#### **Traffic Modelling Scenarios**

- Scenario 0: Existing conditions (base case) this scenario included baseline traffic with no development traffic.
- Scenario 1: 2023 Future conditions with background traffic growth (no development) –
  this scenario included the traffic growth rates applied to the baseline traffic to
  estimate the future traffic in Year 2023 without the development traffic.
- Scenario 2: 2023 Future conditions with background traffic growth and development this scenario included the Year 2023 future base traffic with the additional trips associated with the proposed development.
- Scenario 3: 2033 Future conditions with background traffic growth (no development) this scenario included the traffic growth rates applied to the baseline traffic to
  estimate the future traffic in Year 2033 without the development traffic.
- Scenario 4: 2033 Future conditions with background traffic growth and development this scenario included the Year 2033 future base traffic with the additional trips associated with the proposed development.

The modelled traffic values are presented in Figure 7 to Figure 11.



Figure 7: Traffic Volumes - Scenario 0

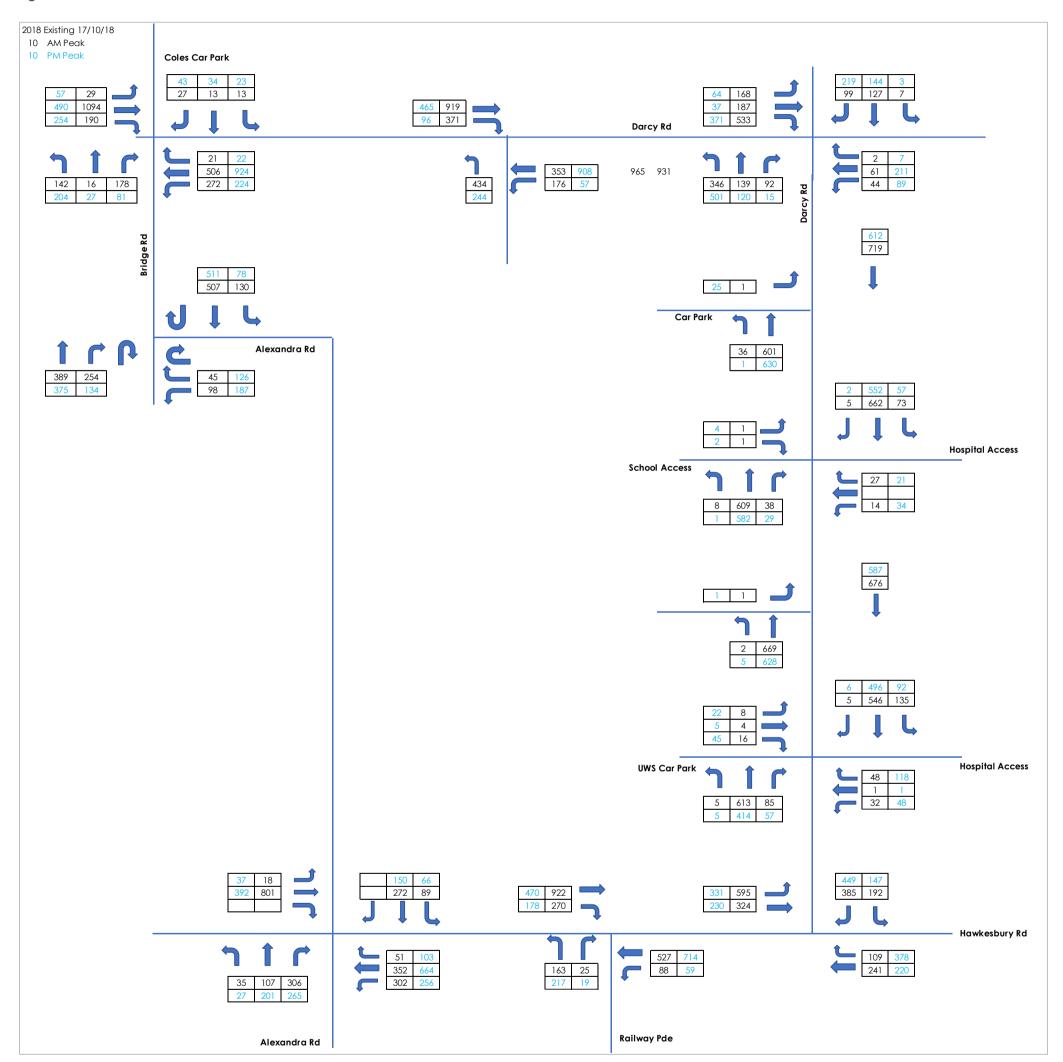




Figure 8: Traffic Volumes - Scenario 1

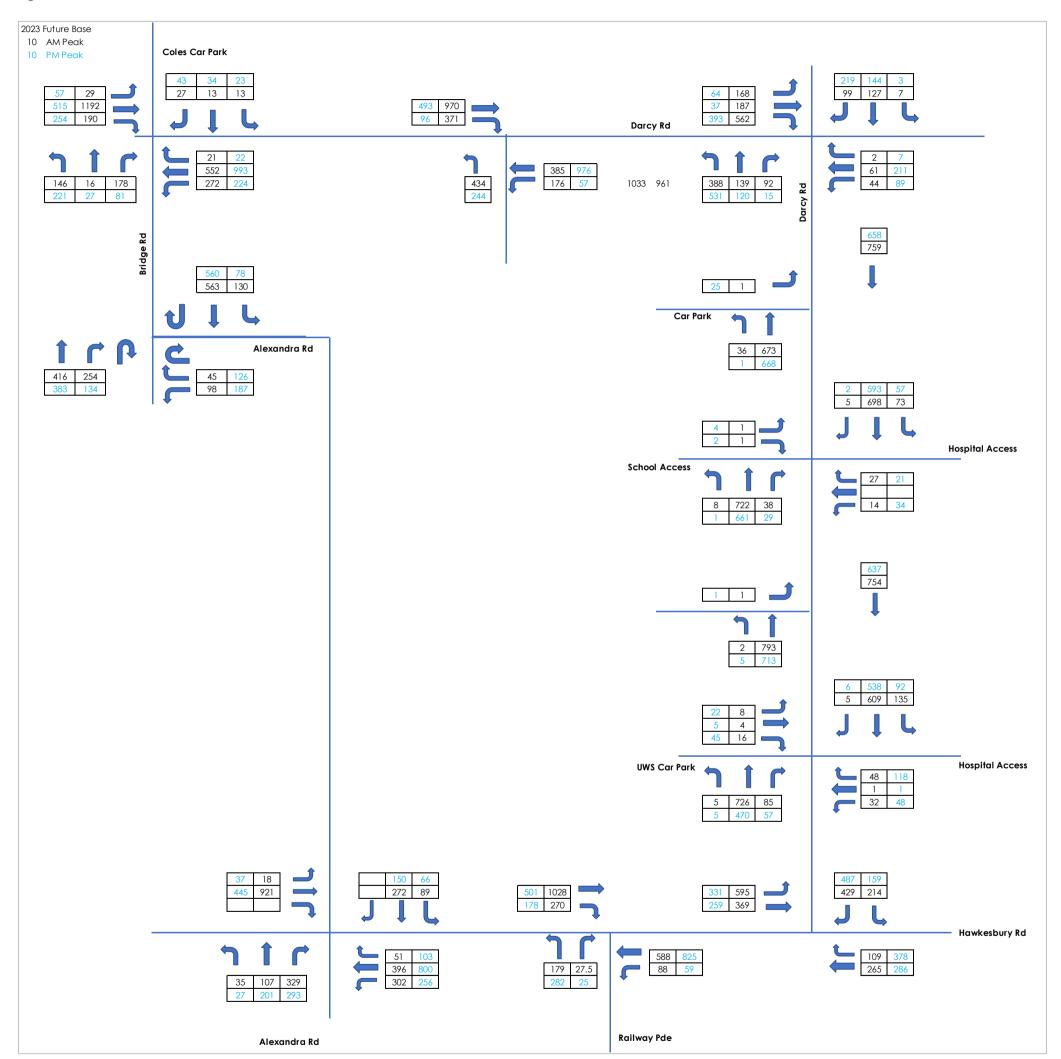




Figure 9: Traffic Volumes - Scenario 2

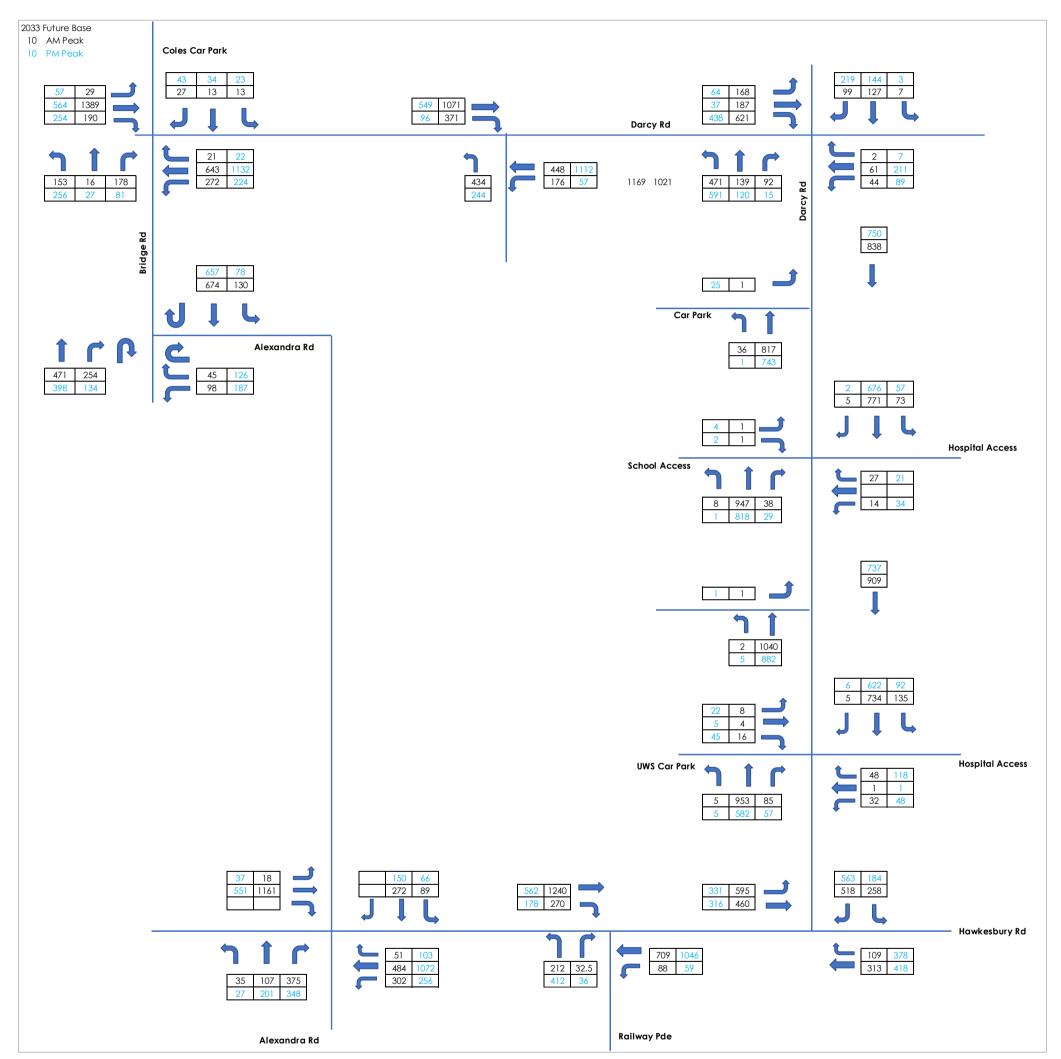




Figure 10: Traffic Volumes - Scenario 3

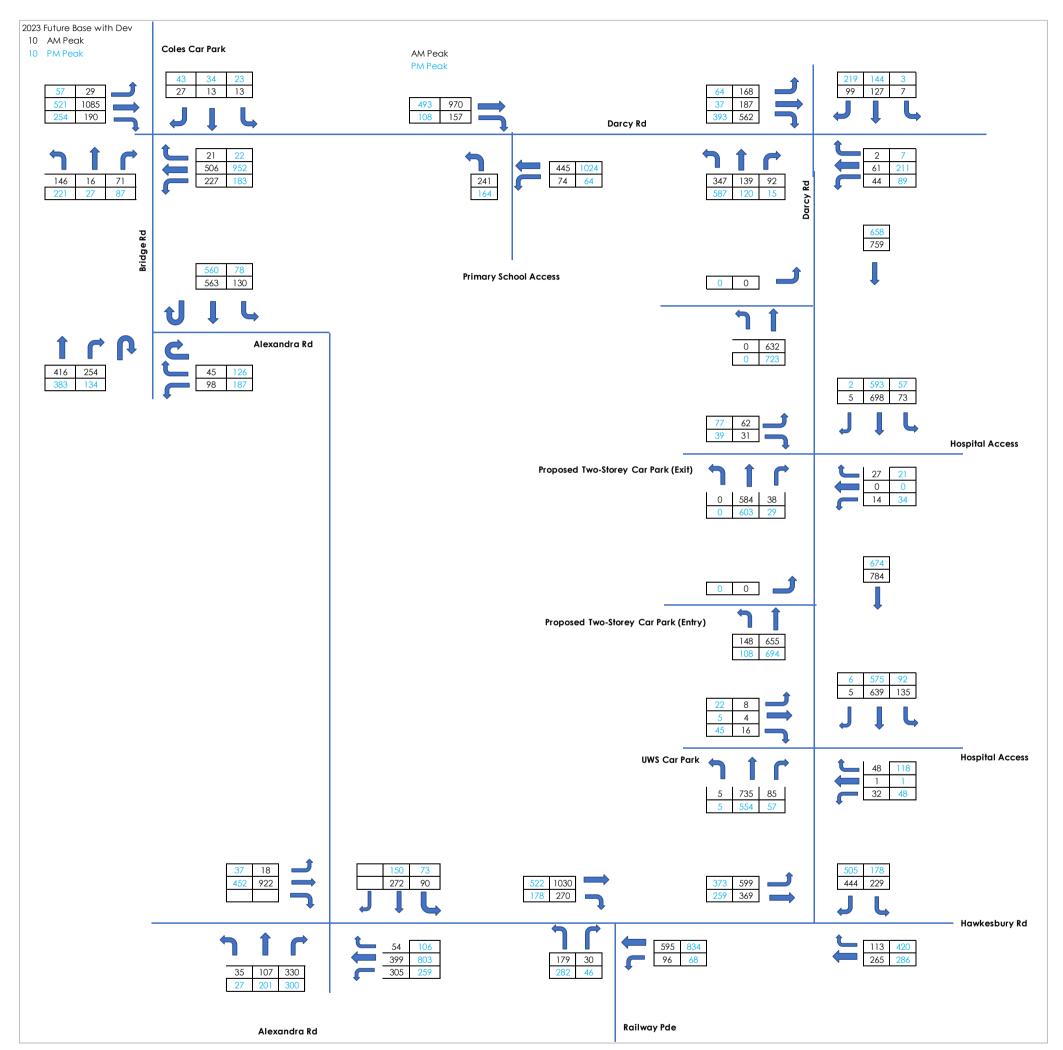
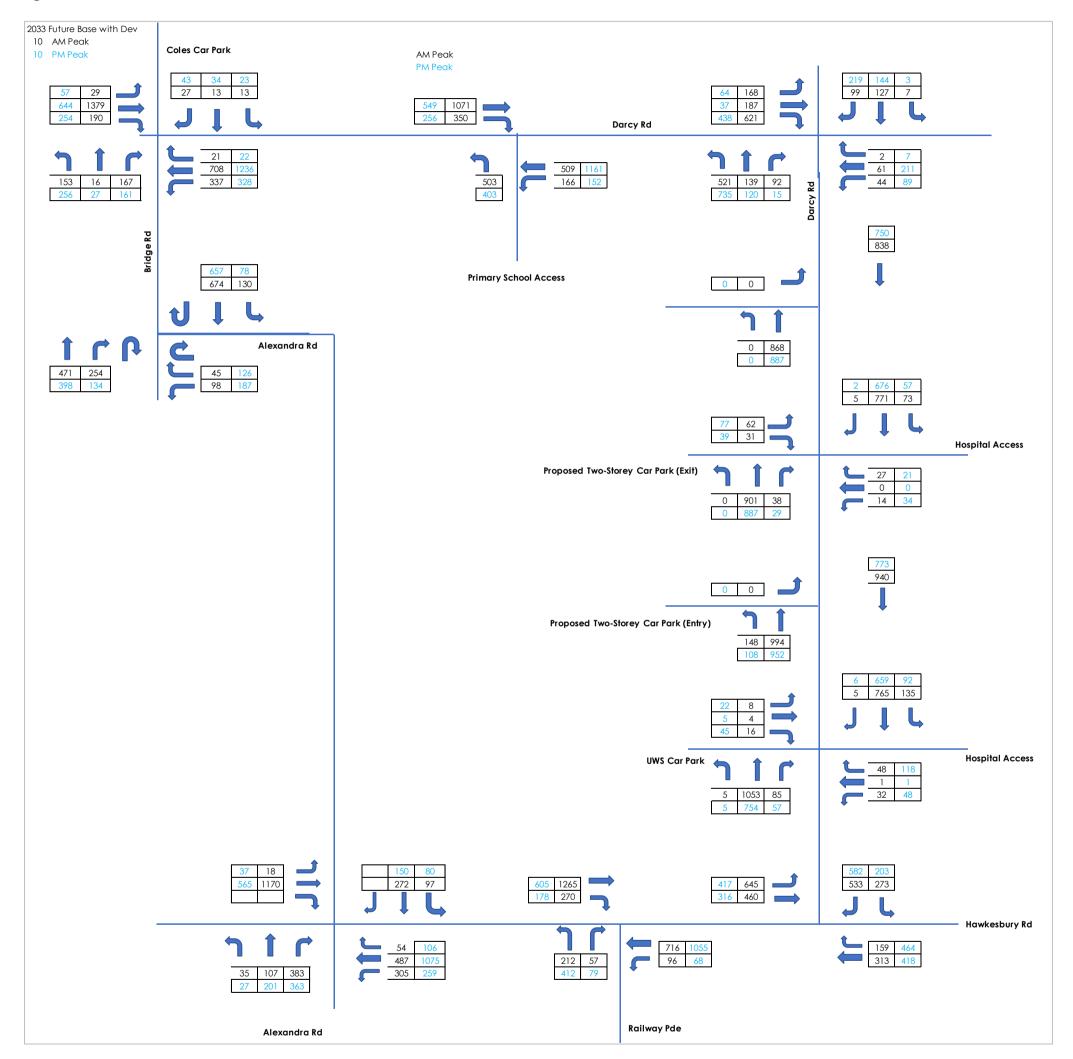




Figure 11: Scenario 4 Traffic Volumes





#### **SIDRA Modelling Existing Model Inputs and Assumptions**

#### **Intersection Layout Configuration**

All intersections have been configured based on site observations and Nearmap aerial imagery captured at the time of completing the modelling analysis. TCS plans for signalised intersections have been used as a basis in developing the intersection layout and phasing arrangement.

The intersection layout and lane configuration were checked in-line with on-site observations. This includes:

- Number of lanes on each approach.
- Traffic lane movement restrictions.
- Lane widths.
- Pedestrian crossings on intersection approaches
- Pedestrian protection (input into SIDRA in accordance with .LX data).

Two intersections have been modified since the traffic surveys were undertaken, namely:

- 1. Darcy Road Mons Road Institute Road which had undergone upgrades after the October 2018 traffic surveys were conducted.
- 2. Darcy Road Hawkesbury Road which had undergone changes due to the commencement of construction works as part of the Parramatta Light Rail project.

Changes at these intersections are described in further detail below.

#### 1. Darcy Road – Institute Road – Mons Road

After the October 2018 traffic surveys were conducted as part of this Project, the intersection of Darcy Rd – Institute Rd – Mons Rd had undergone upgrades. The upgrades included a left turn slip lane on the Darcy Road west approach and installation of a "No Right Turn" restriction on the Darcy Road south-east approach. As a result of the upgrades, amendments by TfNSW were made to the signal phasing arrangement and intersection cycle time.

From Nearmap aerial imagery, the intersection upgrades were carried out at the end of 2018 into early 2019. Figure 12 shows the intersection arrangement prior to the upgrades (which the existing modelling scenario has been based upon) while Figure 13 shows the current intersection arrangement with the upgrades (which future modelling scenarios have been based upon).

Therefore, the base case modelling for this intersection has been carried out based on the geometric layout, signal phasing, and cycle time prior to the intersection upgrade. Signal data has been extracted from the SCATS History data and .LX data which have been obtained for the same day which the traffic surveys were undertaken.



Figure 12: Darcy Road – Mons Road – Institute Road – Previous Configuration



Figure 13: Darcy Road – Mons Road – Institute Road – Current Configuration





#### 2. Darcy Road – Hawkesbury Road

After the October 2018 traffic surveys were conducted as part of this Project, construction works as part of the Parramatta Light Rail project had commenced at the intersection of Darcy Rd – Hawkesbury Rd had. Construction works resulted in the following changes:

- North leg: Three lanes including a dedicated bus lane reduced to one lane.
- South leg: Three lanes including a dedicated bus lane reduced to one lane.
- West leg: Three lanes reduced to two lanes.

Figure 14 shows the intersection arrangement prior to PLR construction (existing modelling scenario has been based upon) while Figure 15 shows the current intersection arrangement under construction works.

The intersection configuration is expected to change throughout the PLR construction phase in order to accommodate road works, as required.

However, future modelling scenarios for the Proposal in years 2023 and 2033 have been based upon the ultimate intersection configuration with PLR. Information of the ultimate intersection layout, signal phasing and phase timings has been obtained from the PLR Project Team via TfNSW.

TTPP has sought this information under a confidentiality agreement with TfNSW, and thus, the specific details cannot be provided in this public documentation. Notwithstanding, the details have been input into the future SIDRA models of which TfNSW is undertaking its review.



Figure 14: Darcy Road – Hawkesbury Road – Previous Configuration



Figure 15: Darcy Road – Hawkesbury Road – Current Configuration





#### TCS Plans, SCATS History Data and .LX Data

Traffic Signal Control (TCS) plans, SCATS History data, and .LX data have been obtained from TfNSW for the same day which the traffic surveys were undertaken. This information includes the traffic signal phasing arrangement, phase timing and frequency, cycle timing, common control groups, and signal coordination and offsets.

Within the surrounding road network there are two sets of signal coordination, namely:

- Along Darcy Road and Hawkesbury Road, between Catherine McAuley Hospital Access and Alexandra Road intersections, and
- Along Darcy Road between Bridge Road and Mons Road-Institute Road intersections.

The two sets of coordinated signals and the reference site for the corresponding coordinated signal are illustrated in Figure 16.



Figure 16: Sets of Coordinated Signals



SIDRA Network modelling software is limited to the application of one set of signal coordination per network only. Given that the Darcy-Hawksbury set of signal coordination forms the larger group of coordinated signals within the surrounding network, this was the adopted signal coordination in the model. Signal offsets which have been adopted in the model are presented in Table 2.

Table 2: Darcy-Hawksbury Signal Coordination and Offset

TCS No.	Intersection	Signal Coordination With	Offset								
AM Peak											
1571	Hawkesbury Rd – Alexandra Ave and Hawkesbury Rd – Railway Pde	Reference	e site (no offset)								
1631	Hawkesbury Rd – Darcy Rd TCS 1571 0 s offset										
3281	Darcy Road – UWS Car Park – Westmead Hospital	TCS 1571	0 s offset from ref. site								
3282	Darcy Road – Catherine McAuley – Westmead Hospital	TCS 1571	0 s offset from ref. site								
	PM Peak										
1571	Hawkesbury Rd – Alexandra Ave and Hawkesbury Rd – Railway Pde	Reference	e site (no offset)								
1631	Hawkesbury Rd – Darcy Rd	TCS 1571	-5 s offset from ref. site								
3281	Darcy Road – UWS Car Park – Westmead Hospital	TC\$ 1571	-5 s offset from ref. site								
3282	Darcy Road – Catherine McAuley– Westmead Hospital	TCS 1571	-5 s offset from ref. site								

Signal phasing arrangements, phase timing, cycle times, and phase frequency information which has been extracted from the SCATS History data and applied in the model is presented in Table 3.

In accordance with comments provided by TfNSW, the maximum cycle time at each signalised intersection has been adopted in the existing and future modelling scenarios (which are based on maximum phase times). Excerpts from TfNSW's submissions addressed to DPIE (dated 29 March 2021) addressing this point are provided as follows:

- Base and future intersections do not use the current intersections maximum cycle length but have adopted the cycle length operating at the time of inspection.
- Existing Maximum Phase Splits were not used.

It is noted that the initial method of model calibration and validation was based upon data collected from site observations which included queue length data. Whilst this methodology was accepted in practice, it was commented by TfNSW (and Bitzios) that SCATS History data and .LX data be obtained for the revised model calibration and validation. Therefore, the revised modelling has been prepared in accordance with this advice. This methodology has been verbally acknowledged and agreed by TfNSW in the most recent meetings with the Planning Delivery Unit (PDU), DPIE, and TfNSW held on 13 July and 26 July 2021.



It is noted that the Hawkesbury Road – Alexandra Avenue and Hawkesbury – Railway Parade intersections operate under a common control group (CCG) as indicated by the TCS plan.

TCS plans, showing the signal phasing arrangements in detail, are given in Attachment One.

SCATS History data and .LX data have been provided in Appendix B of the updated TAIA.

Table 3: Signalised Intersection Phase and Cycle Times

TCS No.	Modelled Signalised	Phase Times and	d Cycle Times (s)	Phase Frequency		
ICS NO.	Intersection	AM Peak	PM Peak	AM Peak	PM Peak	
1571	Hawkesbury Road – Alexandra Avenue and Hawkesbury Road – Railway Parade *Intersections operate under a CCG	A = 56 B = 15 C = 18 D = 28 E = 30 Cycle time = 147	A = 50 B = 29 C = 19 D = 26 E = 33 Cycle time = 157	A = 92% B = 35% C = 96% D = 100% E = 96%	A = 93% B = 19% C = 96% D = 100% E = 93%	
1631	Hawkesbury Road – Darcy Road	A = 49 B = 28 C = 32 D = 17 E = 19 Cycle time = 145	A = 34 B = 26 C = 43 D = 17 E = 22 Cycle time = 145	A = 96% B = 92% C = 100% D = 58% E = 96%	A = 100% B = 92% C = 100% D = 54% E = 100%	
3281	Darcy Road – UWS Car Park – Westmead Hospital	A = 82 D = 18 E = 24 G = 22 Cycle time = 145	A = 76 D = 18 E = 24 G = 18 Cycle time = 136	A = 96% D = 100% E = 88% G = 96%	A = 93% D = 100% E = 85% G = 78%	
3282	Darcy Road – Catherine McAuley Access – Westmead Hospital Access	A = 94 D = 31 E = 14 Cycle time = 139	A = 111 B = 26 E = 13 Cycle time = 150	A = 100% D = 96% E = 96%	A = 92% D = 100% E = 64%	
2393	Darcy Road – Mons Road – Institute Road	A = 49 B = 21 C = 14 D = 18 E = 36 F = 19	A = 45 B = 17 C = 15 D = 19 E = 33 F = 20 G = 12 Cycle time = 161	A = 92% B = 96% C = 76% D = 92% E = 100% F = 24%	A = 100% B = 92% C = 63% D = 100% E = 100% F = 75% G = 54%	
1630	Darcy Road – Bridge Road- Coles Car Park	A = 111 D = 26 E = 13 Cycle time = 140	A = 77 D = 31 E = 24 Cycle time = 132	A = 96% D = 100% E = 96%	A = 93% D = 100% E = 96%	



#### **Gap Acceptance**

Gap acceptance for turning movements at all intersections have been maintained in accordance with the SIDRA default values which are in-line with the recommended values in Appendix E of the RMS Traffic Modelling Guidelines as shown in Figure 17.

Figure 17: Recommended Gap Acceptance Parameters

Recommended values of gap acceptance parameters: Based on AUSTROADS (2002, 2005) Guides

Type of movement .	AUSTROADS	(2002, 2005)	Default or recommended values and ranges for use in SIDRA INTERSECTION		
•	Critical gap (seconds)	Follow-up headway (seconds)	Critical gap (seconds)	Follow-up headway (seconds)	
Left turn (1)	5	2 - 3	(3 - 6)	(2.0 - 3.5)	
1-lane opposing			4.5	2.5	
2-lane (or more) opposing			5.0	3.0	
Through movement crossing one	-way road				
2-lane one-way	4	. 2	4.5 (4 - 5)	2.5 (2 - 3)	
3-lane one-way	6	3	5.5 (5 - 6)	3.0 (2.5 - 3.5)	
4-lane one-way	8	4	6.0 (5 - 8)	3.5 (3 - 4)	
Through movement crossing two	-way road				
2-lane two-way	5	3	5.0 (4.5 - 5.5)	3.0 (2.5 - 3.5)	
4-lane two-way	8	5	6.5 (5 - 8)	3.5 (3 - 5)	
6-lane two-way	8	5	7.5 (7 - 8)	4.5 (4 - 5)	
Right turn from major road (2)					
Across 1 lane	4	2	4.0 (3.5 - 4.5)	2.0 (2 - 3)	
Across 2 lanes	5	3	4.5 (4 - 5)	2.5 (2 - 3)	
Across 3 lanes	6	4	5.5 (5 - 6)	3.5 (3 - 4)	
Right turn from minor road (3)					
One-way	3	3	Use Left turn	values above	
2-lane two-way	5	3	5.5 (5 - 6)	3.5 (3 - 4)	
4-lane two-way	8	5	7.0 (6 - 8)	4.0 (3 - 5)	
6-lane two-way	8	5	8.0 (7 - 9)	5.0 (4 - 6)	
Merge from acceleration lane	3	2	3.0 (2.5 - 3.5)	2.0 (1.5 - 2.5)	

Notes (1) to (3) below are not included in the AUSTROADS Guides.

- (1) This is considered to apply to left-turn movements from minor road, as well as slip-lane left-turn movements from minor road.
- (2) This case is relevant to two-way major road conditions with one direction of the major road opposing (1-lane, 2-lane or 3-lane).
- (3) The conditions specified (one-way, 2-lane two-way, 4-lane two-way, 6-lane two-way) are relevant to the opposing movement lanes on the major road.

Source: RMS Traffic Modelling Guidelines



#### **Capacity Adjustment**

Capacity adjustment inputs have been maintained as the SIDRA default values, with the exception of the Mother Teresa site access which has been adjusted in accordance with the site conditions as observed at the time of the traffic surveys.

Notably, the Kiss and Drop facility, which is accessed via this driveway, has recently been upgraded to improve the site conditions (as explained in Sections 6.7 and 8.4 of the updated TAIA report). Therefore, the future operating conditions of this intersection in reality would be expected to be better than the conditions shown by the modelling.

#### Extra Bunchina

Site observations and traffic survey footage captured vehicles arriving and departing intersections in platoons as a result of multiple traffic signals along Darcy Road and Hawkesbury Road being located closely together.

As such, an extra bunching factor has been applied to intersection approaches adjacent to signalised intersections along Darcy Road and Hawkesbury Road. Extra bunching factors have been applied in accordance with Table 5.2.1 of the SIDRA Intersection 8 User Guide which has been reproduced in Figure 18 below. Distances to upstream signals are based on measurements taken from Nearmap aerial imagery.

Figure 18: Extra Bunching Factors

Distance to upstream signals (m)	< 100	100-200	200-400	400-600	600-800	> 800
(ft)	< 350	350-700	700-1300	1300-2000	2000-2600	> 2600
Extra bunching (%)	25	20	15	10	5	0

Source: SIDRA Intersection 8 User Guide

#### **SIDRA Modelling Results**

SIDRA modelling results for existing and future scenarios and mitigation measures have been presented and discussed in detail in Chapter 8 of the updated TAIA report.



#### **Summary**

The Transport Planning Partnership (TTPP) has prepared an updated Transport and Accessibility Impact Assessment in response to comments provided by the agencies throughout the Response to Submissions stage of the SSD Application.

As requested by DPIE, the calibration and validation details of the SIDRA network modelling have been provided in this report.

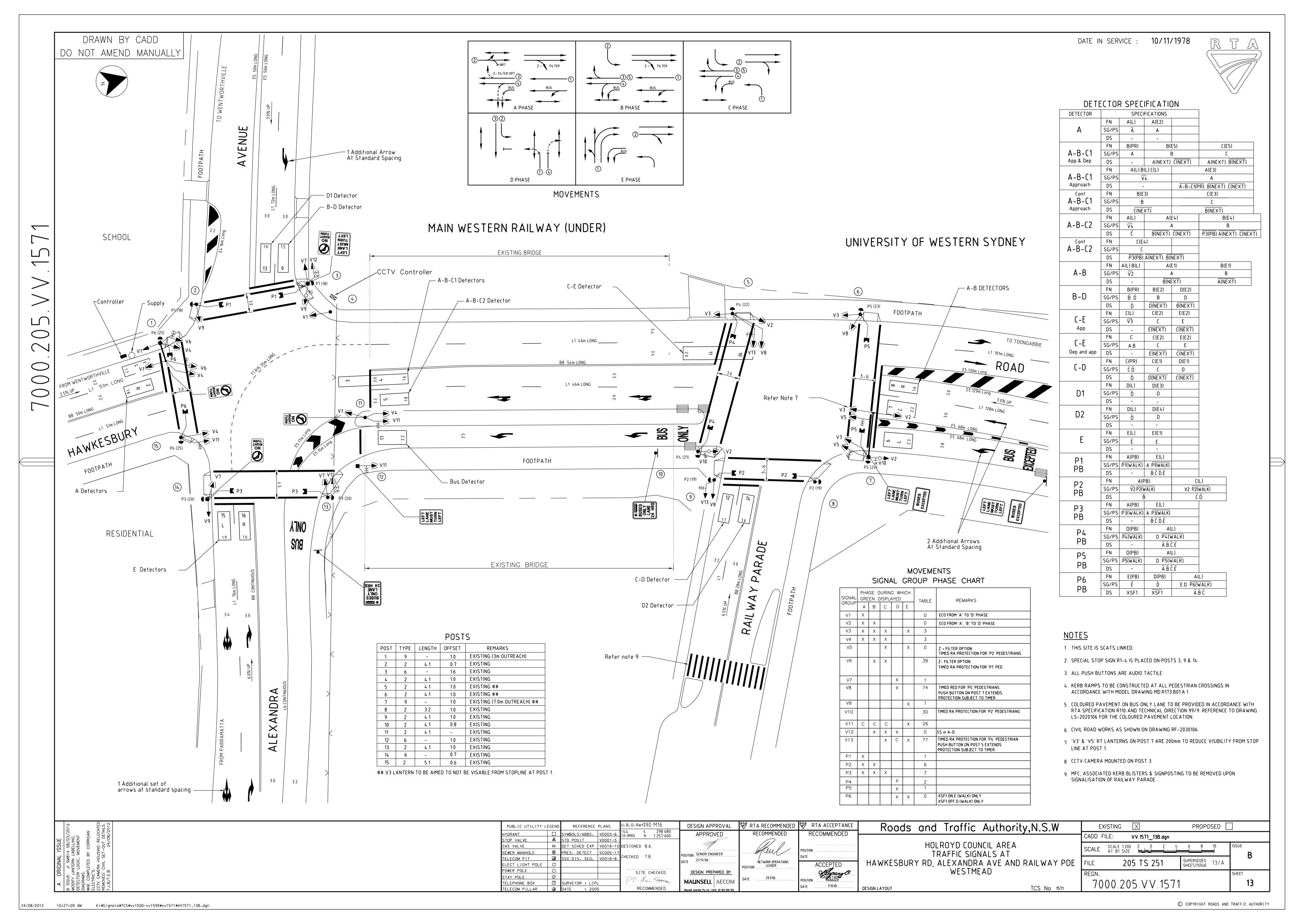
We trust the above is to your satisfaction. Should you have any queries regarding the above or require further information, please do not hesitate to contact the undersigned on 8437 7800.

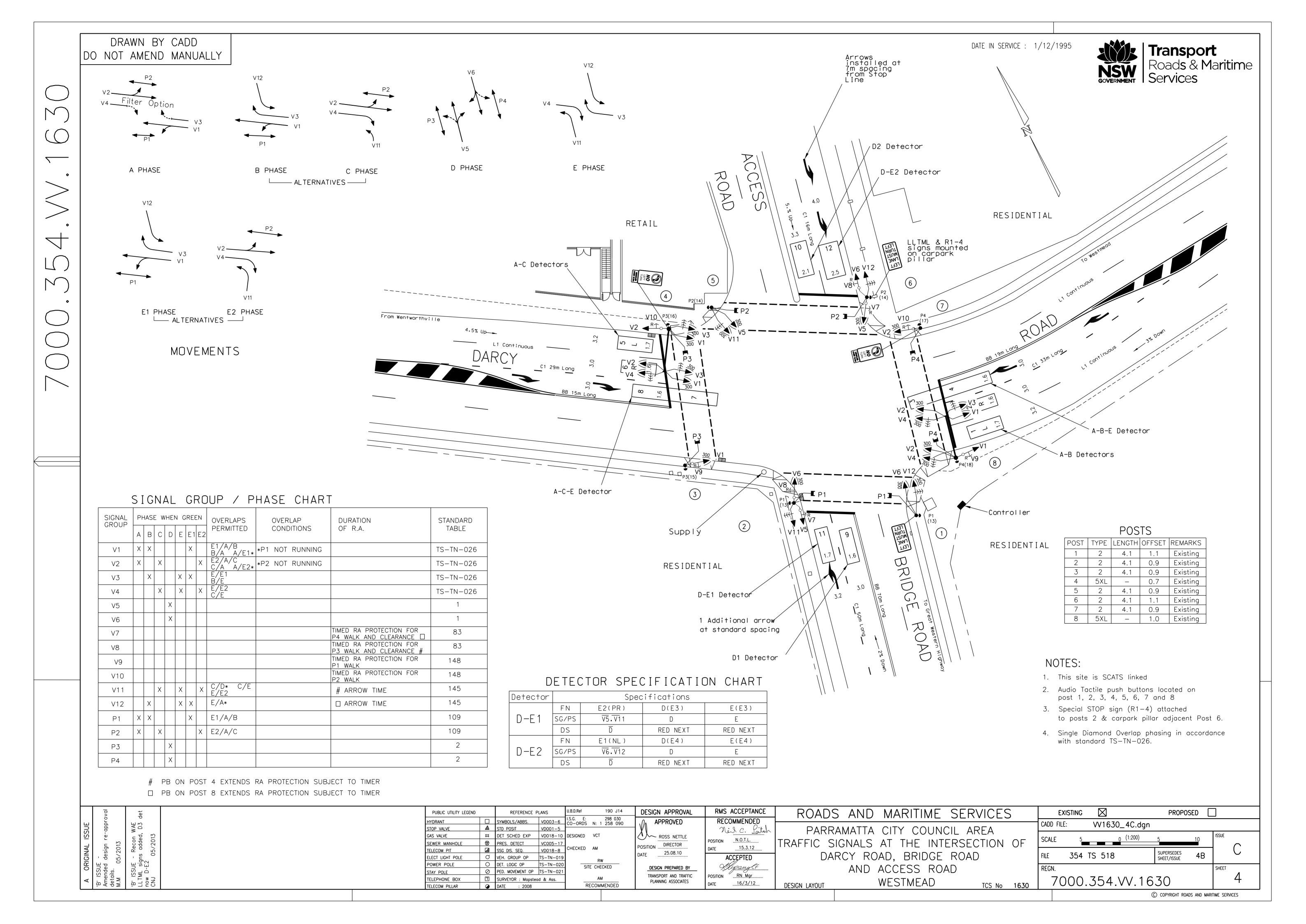
Yours sincerely,

Ken Hollyoak Director



# Attachment One: Traffic Control Signal (TCS) Plans





16.05.06

POSITION .

DESIGN LAYOUT

DATE

MAUNSELL AECOM

Dilama,

RECOMMENDED

TAY POLE

LEPHONE BOX

SURVEYOR: LCPL

7000.354.VV.2393

TCS No **2393** 

DRAWN BY CADD

—One additional arrow at standard spacing

√B-E Detector

GAS VALVE

TELECOM PIT

POWER POLE

STAY POLE

SEWER MANHOLE

ELECT LIGHT POLE

TELEPHONE BOX

TELECOM PILLAR

PRES DETECT

SSG DIS SEQ.
CABLE INST

CABLE CHART

# DET SCHED EXP

SURVEYOR: LCPL

DATE: 2005

VC005-17

VD018-8

SHEET 8

SHEET 9

VD018-10 DESIGNED R.W.

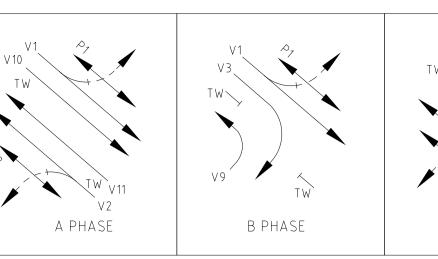
CHECKED T.B.

SITE CHECKED

PP Lee Door

/—A-B Detectors

DO NOT AMEND MANUALLY



NETWORK OPERATIONS

16.6.06

POSITION LEADER

ACCEPTED

16.6.06

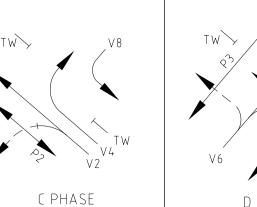
DESIGN LAYOUT

Stanget ROAD NETWORK POSITION MANAGER

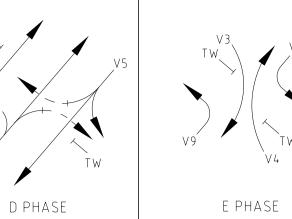
POSITION PROJECT MANAGER

DESIGN PREPARED BY

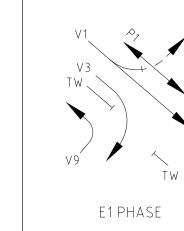
MAUNSELL AECOM

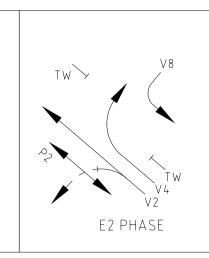


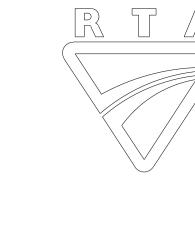
— D1 Detectors (PRESENCE TIMED)



MOVEMENTS







### POSTS

POST	TYPE	LENGTH	OFFSET	REMARKS
1	2	4.1	1.0	EXISTING
2	6	-	1.0	EXISTING
3	2	4.1	0.7	EXISTING
4	4MA	_	1.9	EXISTING
5	2	4.1	_	EXISTING
6	2	4.1	1.0	EXISTING
7	2	4.1	1.0	EXISTING
8	6	_	1.0	EXISTING
9	2	4.1	1.2	EXISTING
10	4MA	_	1.1	EXISTING
11	2	4.1	_	EXISTING
12	2	4.1	1.0	EXISTING

## NOTES

- THIS SITE IS SCATS LINKED.
- 2. SPECIAL STOP SIGN R1-4 IS PLACED ON POST 3 & 9.
- 3. AUDIO TACTILE PUSH BUTTONS ARE PROVIDED ON POSTS 3, 4,5, 8, 9 & 10.
- 4. SINGLE DIAMOND OVERLAP IN ACCORDANCE WITH STANDARD DRAWING VD018-5.
- 5. COLOURED PAVEMENT ON T-WAY IS IN ACCORDANCE WITH SPECIFICATION R110 AND AND TECHNICAL DIRECTION No 99/9
- 6. PROVISION MADE IN SIGNAL DUCTS AND CABLING FOR FUTURE PEDESTRIAN CROSSING OVER DARCY ROAD SOUTH

## SIGNAL GROUP/PHASE CHART

	IGNAL ROUP	WH		ASE I GR		N DI	SPL	AY E2	OVERLAPS PERMITTED	OVERLAPS CONDITIONS	DURATION OF RA
	V1	X	X				X		E1/A/B; A/E1 *, B-A	*P1NOT RUNNING	
	V 2	X		X				X	E2/A/C; A/E2*; C-A	*P2 NOT RUNNING	
	V3		X			X	X		E/E1, B/E		
	V 4			$\times$		X		$\times$	E/E2, C/E		
	V5				X						
	V 6				X						
	V7										▲ ARROW TIME
	V8			$\times$		X		$\times$	C/E, E/E2, C/D		
	V 9		X			X	X		B/E, E/E1, B/D*	* P3 NOT DEMANDED	▲ ARROW TIME
	V10	$\times$									
	V11	$\times$									
\	P1	X	X				X		E1/A/B		
	P2	$\times$		$\times$				X	E2/A/C		
	Р3				X						

▲ PUSH BUTTON ON POST 8 EXTENDS RED ARROW SUBJECT TO TIMER.

VV 3282<u>7</u>B.dgn

SCALE 1:200

354.TS.544

EXISTING

CADD FILE:

SCALE

FILE

TCS No 3282

PARRAMATTA CITY COUNCIL AREA

TRAFFIC SIGNALS AT

DARCY ROAD, HIGH SCHOOL ENTRANCE

AND NORTH WEST T-WAY WESTMEAD

WESTMEAD

Sin Toninhous Day Bus 1 Detector  Bus 1 Detector	V1 6 P3(15) 7 P1(12) P3 V1	HOSPITAL ENTRAN	NCE  RESERVANTE OS SETONIEN  OS
P2(13) P2(13) P2(13) P2(23)	TI 30R 30m LONG - III 32R - 34m LONG - III 32R - 34	TO TO TO THE SERVICE OF THE SERVICE	ON THE CONTRACT OF THE CONTRAC
D2 Detector (PRESENCE TIMED)  HIGH SCHOOLS	P2 V5 V8 P2(13)  1  Controller  Undergound Supply  A-C Detectors	PATH 11 CONTROLLS  TO A THE TOTAL CONTROLLS	SIGNAL GROUP
	PUBLIC UTILITY LEGEND  REFERENCE PLANS  U.B.D.Ref. 190 L 15  HYDRANT  SYMBOLS/ABBS. VD003-6  CO-ORDS. N: 1 258 045	DESIGN APPROVAL  APPROVED  RECOMMENDED  CONFIRMATION	Roads and Traffic Authority, N.S.W

ISSUE

SHEET

PROPOSED \_\_\_

SUPERSEDES SHEET/ISSUE