

**Cowman Stoddart Pty Ltd** PO Box 738 NOWRA NSW 2541

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1 May 2020

#### Attention: Mr. S Richardson

Dear Steve,

### Re: Flood Compliance Report for Proposed Amendment to Planning Modification 17, Shoalhaven Starches Expansion Project, 22, 24 and 171 Bolong Road, Bomaderry

This letter has been prepared by R W Dewar BSc, MEngSci, MIEAust CPEng Member No 477618 who has over 37 years of experience of floodplain management in NSW.

#### 1 Introduction

Shoalhaven Starches Pty Ltd intend to amend Planning Modification 17 as detailed below:

- 1. Amend the siting and footprint of the Product Dryer building (Gluten Dryer No. 8) from that which was approved under Modification 16.
- 2. Extend Specialty Products Building (SPB) to the north and to provide Bulk Chemical Storage to the south of the Product Dryer Building.
- 3. Part of the existing Stores and Maintenance Offices Buildings will be demolished.
- 4. The existing Maintenance workshop that will be retained will be altered internally to provide staff amenities and Plant Operational Control Rooms.
- 5. Changed internal truck movements associated with SPB will necessitate the removal of 48 carparks which are currently located adjacent to the existing Maintenance Store and Workshop. It is proposed to include the proposed Area 3 car park, north side of Bolong Road to the east of the fire pump house, in Modification 17.
- 6. Sifter room extension.

Plans of the plant amendments (refer Google map of plant below) are provided in Appendix A.



#### WMAwater Pty Ltd DIRECTORS

DIRECTORS M K Babister, RPEQ R W Dewar E J Askew F L N Ling, RPEQ SENIOR ASSOCIATES R Hardwick Jones M E Retallick

ASSOCIATES E M Harrison ABN 14 600 315 053 Level 2, 160 Clarence St, SYDNEY NSW 2000 Phone: 02 9299 2855 Fax: 02 9262 6208 Email: enquiry@wmawater.com.au Website: wmawater.com.au



All the sites for the amended works are inundated in the 1% Annual Exceedance Probability (AEP) flood event by floodwaters from the Shoalhaven River and this letter provides an assessment of the implications of these proposed amendments on flood levels, flows and velocities.

WMAwater (formerly known as Webb McKeown & Associates) undertook the 1990 Shoalhaven River Flood Study and subsequent 2008 Floodplain Risk Management Study and Plan for Shoalhaven City Council. We have also undertaken many similar type flood assessments for Shoalhaven Starches in the past and are therefore very familiar with flooding in the Shoalhaven River floodplain and the implications for flooding of further development in the northern floodplain and along Bolong Road.

### 2 Background

The proposal is to construct / amend existing approved plant as described in Appendix A. A detailed flood assessment for the existing approved works was completed in WMAwater's report of 30 April 2018 and a copy of that report is attached as Appendix B.

This present report therefore provides an assessment of the likely flood impacts of the proposed amendments in Appendix A compared to those provided in our report of 30 April 2018.

### 3 Compliance with Chapter G9: Development on Flood Prone Land (DCP2014)

The following sections describe compliance of the amendments with Chapter G9: Development on Flood Prone Land (DCP2014 Amended 1<sup>st</sup> July 2015). As the modifications will not involve fill, or subdivision of lands, compliance with these performance criteria has not been addressed.

PERFORMANCE CRITERIA	RESPONSE			
P1 Development or work on flood prone land will meet the following:				
The development will not increase the risk to life or	No additional workers will be on the site as a			
safety of persons during a flood event on the	result of the proposed works.			
development site and adjoining land.				
The development or work will not unduly restrict the	Refer Hydraulic Impact Assessment below.			
flow behaviour of floodwaters.				
The development or work will not unduly increase the	The proposed development is within existing			
level or flow of floodwaters or stormwater runoff on	built up industrial land with minimal			
land in the vicinity. The development or work will not	vegetation on the site. All runoff under			
exacerbate the adverse consequences of floodwaters	existing and future conditions will reach the			
flowing on the land with regard to erosion, siltation	ground in nearly identical locations and thus			
and destruction of vegetation.	the works will have no impact on erosion or			
	siltation.			
The structural characteristics of any building or work	A separate structural report will be provided.			
that are the subject of the application are capable of				
withstanding flooding in accordance with the				
requirements of the Council.				

## 3.1 Performance Criteria - General (Section 5.1 of DCP only)



PERFORMANCE CRITERIA	RESPONSE	
The development will not become unsafe during	A separate structural report on the potential	
floods or result in moving debris that potentially	failure of existing buildings and stored	
threatens the safety of people or the integrity of	equipment and product will be provided.	
structures.		
Potential damage due to inundation of proposed	Inundation of the site and the proposed plant	
buildings and structures is minimised.	and / or debris impact may cause damage to	
	electrical and other components feeding the	
	equipment as well as damage to the plant	
	itself. These issues will be considered in an	
	updated Shoalhaven Starches Flood Plan	
	taking into account the proposed works.	
The development will not obstruct escape routes for	The proposed works will not occupy escape	
both people and stock in the event of a flood.	routes or cause workers to become trapped.	
	Possible failure of lifts during a flood with	
	workers inside will be considered in an	
	updated Shoalhaven Starches Flood Plan.	
The development will not unduly increase dependency	The works will not increase the number of	
on emergency services.	workers from Shoalhaven Starches who may	
	be subject to flood risk as a result of the	
	proposed works.	
Interaction of flooding from all possible sources has	Refer Hydraulic Impact Assessment below.	
been taken into account in assessing the proposed		
development against risks to life and property		
resulting from any adverse hydraulic impacts.		
The development will not adversely affect the	The works will be constructed on land that is	
integrity of floodplains and floodways, including	partly designated as high hazard floodway in	
riparian vegetation, fluvial geomorphologic	the 1% AEP event. The site is industrial land	
environmental processes and water quality.	with limited existing vegetation and is beyond	
	the influence of normal fluvial geomorphic	
	processes. The works will have no impact on	
	water quality.	

#### 3.2 Hydraulic Impact Assessment

Detailed hydraulic assessment and modelling was undertaken as part of our of 30 April 2018 report (copy in Appendix B), thus the present assessment is only regarding whether the proposed amendments change the results from those provided in our 30 April 2018 report.

#### 3.2.1 Proposed Area 3 Car Park

The car park will not include any imported fill (apart from any surface treatment) and will rely upon regrading of the existing site. Thus the works will result in no change to design flood levels.

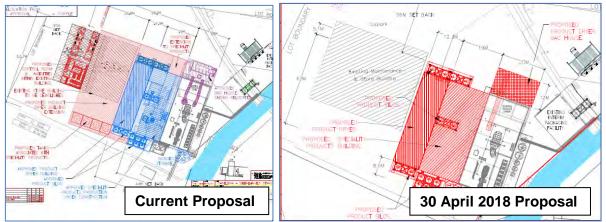


### 3.2.2 Sifter Room Extension

The sifter room extension is located above the 1% AEP flood level and thus the works will have no impact on the 1% AEP flood level.

#### 3.2.3 Modifications to Buildings

The figures below indicate the change in building outlines between those contained in our 30 April 2018 report and those currently proposed.



The hydraulic effects (change in flood levels, flows or velocities) of the proposed works were analysed using the TUFLOW hydraulic model established for the Shoalhaven Starches 2013 *Shoalhaven River Flood Study* and used in our 30 April 2018 assessment. This model was calibrated to match the historical flood level data for the 1974, 1975, 1978 and 1988 floods and used to provide updated design flood levels for the Shoalhaven River downstream of Nowra.

The modelling process was to compare the *Current Proposed* scenario peak flood levels in each grid cell to the *Existing* scenario adopted in our 30 April 2018 report as well as to the *30 April 2018 Proposed* scenario. The *Existing* scenario represents the existing floodplain including all proposed but un-built approved Shoalhaven Starches structures at the time of our 30 April 2018 report. The *30 April 2018 Proposed* scenario reflects the 30 April 2018 floodplain but including the works proposed in our 30 April 2018 report. The comparison between the scenarios is termed a flood impact map.

More frequent events, smaller than the 1% AEP, have not been modelled as the northern river bank of the Shoalhaven River is not overtopped to any significant extent until an event larger than the 5% AEP. Thus in these small more frequent events there would be nil impact on peak flood levels of the proposed amendments. Larger events than the 1% AEP will occur but these events are obviously extremely rare and are not used for flood related planning determinations by Councils except when their failure has potential catastrophic consequences (such as dam failure).



The results are provided as follows:

Figure 1: Existing topography;

Figure 2: 1% AEP flood levels for the *Existing* scenario;

level outside land owned by Shoalhaven Starches.

Figure 3: The 1% AEP flood impact map for the *Current Proposed* scenario versus the *Existing* scenario. The different colours reflect the change in peak water levels as a result of the proposed works. In summary the purple tones reflect a decrease in flood level whilst the blue/green/brown tones reflect an increase in peak level. The results indicate no increase in flood level outside land owned by Shoalhaven Starches. Downstream of the proposed works on Bolong Road there is a reduction in peak level of less than 0.1m. This occurs because the proposed works reduce slightly the amount of flood waters crossing through the site and thus flood levels are slightly lowered. Figure 4: This 1% AEP flood impact map indicates the difference between the *Current Proposed* scenario versus the *30 April 2018 Proposed* scenario. The different colours reflect the change in peak water levels. In summary the purple tones reflect a decrease in flood level whilst the blue/green/brown tones reflect an increase in peak level. The results indicate no change in flood level while the stere and the proposed works reflect the change in peak water levels. In summary the purple tones reflect a decrease in flood level whilst the blue/green/brown tones reflect an increase in peak level. The results indicate no change in flood

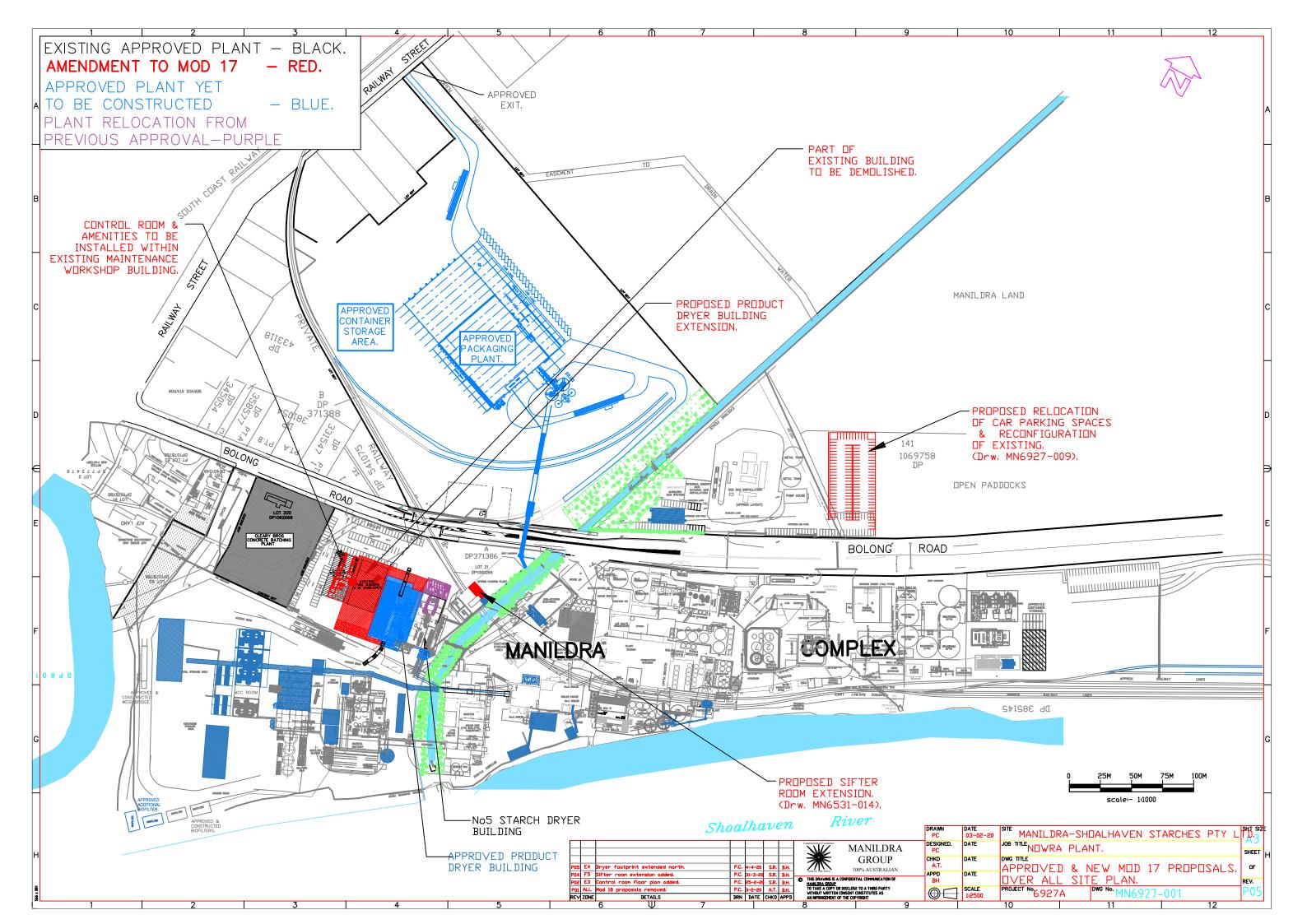
Should you have any questions or require further clarification regarding the above do not hesitate to contact the undersigned.

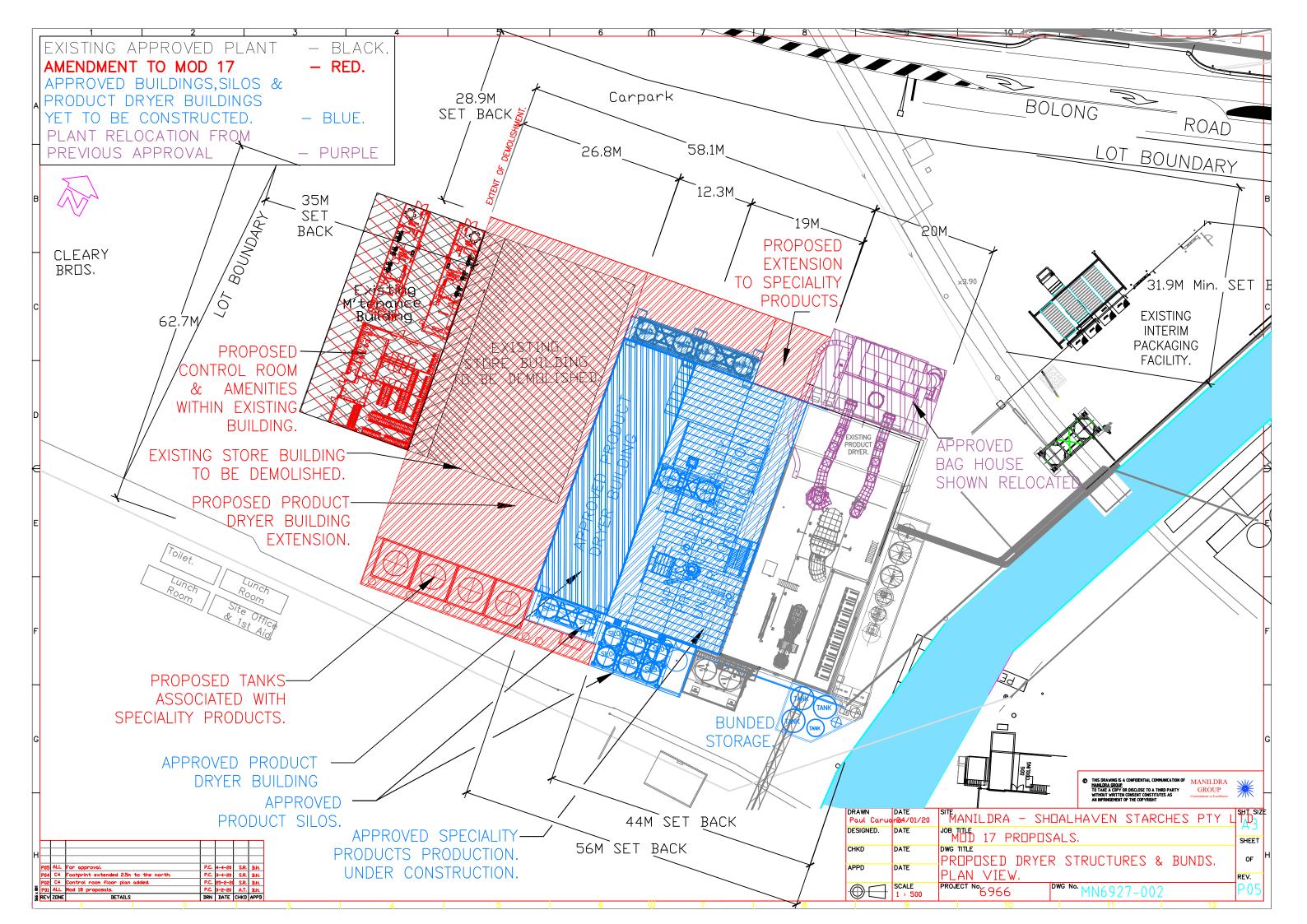
Yours Sincerely, **WMAwater** 

R W Dewar Director















**Cowman Stoddart Pty Ltd** PO Box 738 NOWRA NSW 2541

J:\Jobs\114044\Admin\SpecialtyPlantMar2018.docx 30 April 2018

Attention: Mr. S Richardson

Dear Steve,

#### Re: DCP2014 Chapter G9: Flood Compliance Report for Proposed Modification Application to MP06-0228, Shoalhaven Starches Expansion Project, Bolong Road, Proposed new Specialty Processing Facility and other works at 22, 24 and 171 Bolong Road, Bomaderry

This letter has been prepared by R W Dewar BSc, MEngSci, MIEAust CPEng Member No 477618 who has over 30 years of experience of floodplain management in NSW.

#### 1 Introduction

Shoalhaven Starches Pty Ltd intend to construct a number of works as listed below at 22, 24 and 171 Bolong Road (refer GoogleMaps aerial photograph below). These works are required to utilise grain that is currently approved to directly feed the fermentation process in the ethanol production process; to instead increase the amount of flour production and in effect increase the starch and gluten production. This change in production is necessary due to the reduced increase in expected demand for ethanol in petrol.

Detailed plans of the plant upgrade are provided in Appendix A.



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All the sites for the proposed works are inundated in the 1% Annual Exceedance Probability (AEP) flood event by floodwaters from the Shoalhaven River and this letter provides an assessment of the implications of this proposal on flood levels, flows and velocities.

WMAwater (formerly known as Webb McKeown & Associates) undertook the 1990 Shoalhaven River Flood Study and subsequent 2008 Floodplain Risk Management Study and Plan for Shoalhaven City Council. We have also undertaken many similar type flood assessments for Shoalhaven Starches in the past and are therefore very familiar with flooding in the Shoalhaven River floodplain and the implications for flooding of further development in the northern floodplain and along Bolong Road.

### 2 Description of Proposal

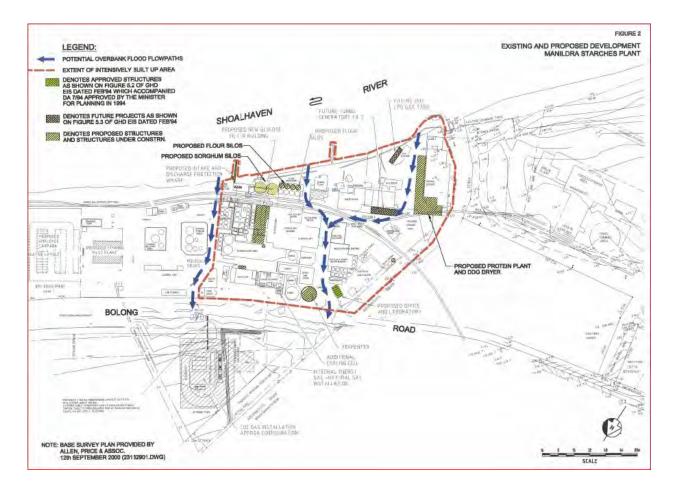
The proposal is to construct plant as described in Appendix A. Indicative ground levels at the sites adjacent to Abernethys Creek are around 3.5 to 4.5 mAHD (Figure 1) and the 1% AEP flood level is 5.7 mAHD according to the Flood Certificate (this provides the maximum levels for all sites) obtained on 5<sup>th</sup> April 2018 (attached as Appendix B). Figure 2 provides the 1% AEP peak flood depths based on the TUFLOW modelling as the Flood Certificate is based on prior flood study results (refer Section 3 of this letter for an explanation on the difference).

The Flood Certificate also advises that in the 1% AEP event parts of the sites are described as High Hazard and Floodway and that the projected sea level rise estimates due to climate change will not increase the 1% AEP flood level at this site. It should be noted that the high hazard and floodway classifications (and all other flood related data) were taken from the hydraulic model established in the 1990 Shoalhaven River Flood Study. These are the maximum classifications for the site and the hazard will decrease towards Bolong Road as floodwaters dissipate into the northern floodplain.

The construction of any works on the floodplain will cause a loss of temporary floodplain storage and a loss of hydraulic conveyance. The resulting increase in flood levels will depend upon the magnitude of these losses. Given that parts of the proposed plant are on piers and / or raised above the 1% AEP flood level and the floodplain storage area of the Shoalhaven River floodplain is of the order of 100km<sup>2</sup>, the loss of temporary floodplain storage due to the works is generally too small to be accurately evaluated. The main issue from a flooding perspective is therefore the construction of plant as it will impede flow from the Shoalhaven River crossing the site to enter the northern floodplain (i.e. reduce the hydraulic conveyance through the site and potentially raise flood levels elsewhere).

Prior to construction of the Shoalhaven Starches plant at Bomaderry there would have been significant flow through the site during a flood, as there is across any river bank. However, since approximately 1960 the ongoing construction of the plant has effectively blocked the flow path through the site. This issue has been investigated in our October 2000 report titled "*Further Development within the Manildra starches Plant off Bolong Road, Bomaderry - Hydraulic Assessment*". The conclusions from that report are provided in Appendix C. In summary an agreement was reached that any future development within the intensively built-up area, as indicated on the Figure 2 below (taken from that report) would not require hydraulic modelling to quantify the hydraulic impacts and cumulative effects.





Flood impact assessment of the proposed plant (i.e will the works impact on flood levels) has therefore been separated into the following two categories (Sections 2.1 and 2.2) according to their site locations.

# 2.1 Proposed Plant within the Intensively Built Up Area as Described in the Figure above and in Appendix C

- a) pipeline from product dryer bag house;
- b) relocation of #7 boiler;
- c) main substation extension;
- d) #8 boiler and generator set;
- e) lime silos;
- f) flour mill C & flour mill A, B & C ventilation;
- g) modification to rail unloading.

# 2.2 Proposed Plant outside the Intensively Built Up Area as Described in the Figure above and in Appendix C

- a) product dryer, specialty product building and product dryer bag house;
- b) sifter room;
- c) indoor electrical sub station;
- d) relocation of 25 car parking spaces.



# 3 Compliance with Chapter G9: Development on Flood Prone Land (DCP2014)

The following sections describe compliance with Chapter G9: Development on Flood Prone Land (DCP2014 Amended 1<sup>st</sup> July 2015). As the works will not involve fill, or subdivision of lands, compliance with these performance criteria has not been addressed.

PERFORMANCE CRITERIA	RESPONSE				
P1 Development or work on flood prone land will meet the following:					
The development will not increase the risk to life or	Up to 15 additional workers from will be on				
safety of persons during a flood event on the	the site as a result of the proposed works.				
development site and adjoining land.	Thus the proposed development will increase				
	the number of workers from Shoalhaven				
	Starches who may be subject to flood risk.				
	Shoalhaven Starches already has a Flood Plan				
	to ensure safety of personnel, to minimise				
	flood damages and to assist with recovery.				
	This plan will be updated to ensure that it				
	accounts for the additional workers and new				
	plant on the site.				
The development or work will not unduly restrict the	Refer Hydraulic Impact Assessment below.				
flow behaviour of floodwaters.					
The development or work will not unduly increase the	The proposed development is within existing				
level or flow of floodwaters or stormwater runoff on	built up industrial land with minimal				
land in the vicinity. The development or work will not	vegetation on the site. All runoff under				
exacerbate the adverse consequences of floodwaters	existing and future conditions will reach the				
flowing on the land with regard to erosion, siltation	ground in nearly identical locations and thus				
and destruction of vegetation.	the works will have no impact on erosion or				
	siltation.				
The structural characteristics of any building or work	A separate structural report will be provided.				
that are the subject of the application are capable of					
withstanding flooding in accordance with the					
requirements of the Council.					
The development will not become unsafe during	A separate structural report on the potential				
floods or result in moving debris that potentially	failure of existing buildings and stored				
threatens the safety of people or the integrity of	equipment and product will be provided.				
structures.					
Potential damage due to inundation of proposed	Inundation of the site and the proposed plant				
buildings and structures is minimised.	and / or debris impact may cause damage to				
	electrical and other components feeding the				
	equipment as well as damage to the plant				
	itself. These issues will be considered in an				
	updated Shoalhaven Starches Flood Plan.				

#### 3.1 Performance Criteria - General (Section 5.1 of DCP only)



PERFORMANCE CRITERIA	RESPONSE	
The development will not obstruct escape routes for	The proposed works will not occupy escape	
both people and stock in the event of a flood.	routes or cause workers to become trapped.	
The development will not unduly increase dependency	The works will increase (by 15) the number of	
on emergency services.	workers from Shoalhaven Starches who may	
	be subject to flood risk. These issues will be	
	examined in an updated Shoalhaven Starches	
	Flood Plan.	
Interaction of flooding from all possible sources has	Refer Hydraulic Impact Assessment below.	
been taken into account in assessing the proposed		
development against risks to life and property		
resulting from any adverse hydraulic impacts.		
The development will not adversely affect the	The works will be constructed on land that is	
integrity of floodplains and floodways, including	partly designated as high hazard floodway in	
riparian vegetation, fluvial geomorphologic	the 1% AEP event (from flood certificate in	
environmental processes and water quality.	Appendix B). The site is industrial land with	
	limited existing vegetation and is beyond the	
	influence of normal fluvial geomorphic	
	processes. The works will have no impact on	
	water quality.	

#### 3.2 Hydraulic Impact Assessment

As noted above a large number of the proposed works are within the agreed upon *intensively built up area*. As such the loss of flow conveyance due to construction of the works in this area will be minimal as the existing access corridors (which become flow paths during a flood) through the site will always remain. For this reason no hydraulic impact assessment has been undertaken for the works described in Section 2.1.

Of the four work items located outside the agreed upon *intensively built up area* (Section 2.2) item d) the relocation of 25 car parking spaces will involve no buildings, minimal earth works and all cars will be removed prior overtopping of the river bank. Thus the hydraulic impact of this work is nil.

Hydraulic modelling of items a), b) and c) in Section 2.2 has been undertaken as described below.

As items b) - sifter room and c) - indoor electrical sub station in Section 2.2 are relatively small and generally "shielded" by existing plant to the south (the generally direction of flow from the Shoalhaven River is from south to north) the impacts of these structures will be small. However item a) - product dryer, specialty product building and product dryer bag house in Section 2.2 is much larger and restricts an existing flow path through the site. The March 2018 Nearmap aerial photograph shows a cleared area where the specialty building is to be located. However existing conditions have been assumed as at the August 2015 Nearmap aerial photograph



which shows the presence of an existing building on the west of the recently constructed product dryer building.



#### 3.2.1 Hydraulic Modelling

Hydraulic or flood modelling typically involves the setting up and calibration of two computer models. A hydrologic model that converts the rainfall to runoff and a hydraulic model that includes inflow from the hydrologic model, as well as ocean boundaries, which determines peak flood levels and velocities based on hydraulic formulae. Both models are calibrated to historical data, including historical flood levels and river flow gaugings, to ensure that they can replicate the historical events and are then used to determine design flood events. These are events that have a known probability of occurrence, such as the 1% Annual Exceedance Probability (AEP) event.

The CELLS model of the Shoalhaven River (established as part of the 1990 *Lower Shoalhaven River Flood Study*) represented the channel and floodplain as a series of interconnected cells, termed either river or floodplain cells. The river cells were connected by cross sections and the floodplain cells connected by weirs. Approximately 100 cells were used in the Shoalhaven River model with some cells over 4km<sup>2</sup> in area. The CELLS model is termed a one dimensional (1D) branched model in that it cannot account for flow in other than the one direction but has "branches" which allow flow to extend across the floodplain. The model used both field survey for weirs as well as bathymetric survey for the river cross sections at approximately 1 to 2 kilometre spacing.

The CELLS model is an unsteady flow model in that it modelled the full flood event (rising and falling water levels) and not just the peak and included ocean tidal hydrographs at both entrances, namely the Shoalhaven Heads and Crookhaven River, and some six flow hydrographs from the WBNM hydrologic model. Council's Flood Certificates are based on results from the CELLS model.



Since 1990 there have been significant advancements in the field of hydraulic modelling, though in hydrologic modelling there has been significantly less advancements and the WBNM model used previously is still used today.

The main advancements in hydraulic modelling are through the use of more complex computer software (TUFLOW) that allows the river and floodplain to be discretised into a grid. This is typically 15m by 15m on large rivers and up to 2m by 2m on small urban catchments. These models are termed 2 Dimensional (2D) in that they determine the flow direction between grid cells producing vector velocities. These models are thus able to more accurately define the topography and in turn can more accurately represent the hydraulic effects of even a small development on a large floodplain. The use of TUFLOW allows more accurate definition of all hydraulic parameters (hazard, hydraulic classification, peak velocities and depths etc.) on the site. Thus rather than a single value provided from the CELLS model (1990 *Lower Shoalhaven River Flood Study*) TUFLOW is able to demonstrate that hazard, velocity and other parameters will change as flow crosses over the northern bank and enters the northern floodplain.

#### 3.2.2 Hydraulic Modelling Process

The hydraulic effects (change in flood levels, flows or velocities) of the proposed works (items a), b) and c) in Section 2.2) at the Shoalhaven Starches plant site at Bomaderry were analysed using the TUFLOW hydraulic model established for the Shoalhaven Starches 2013 *Shoalhaven River Flood Study.* This model was calibrated to match the historical flood level data for the 1974, 1975, 1978 and 1988 floods and used to provide updated design flood levels for the Shoalhaven River downstream of Nowra.

The modelling process was to compare the peak flood levels in each grid cell for the *Existing* and *Proposed* scenarios. The *Existing* scenario represents the existing floodplain including all proposed but un-built approved Shoalhaven Starches structures. The *Proposed* scenario reflects the existing floodplain but including the proposed plant (items a), b) and c) in Section 2.2 as described in Appendix A). The comparison between the *Existing* and *Proposed* scenarios is termed a flood impact map.

More frequent events, smaller than the 1% AEP, have not been modelled as the northern river bank of the Shoalhaven River is not overtopped to any significant extent until an event larger than the 5% AEP. Thus in these small more frequent events there would be nil impact on peak flood levels of the proposed works. Larger events than the 1% AEP will occur but these events are obviously extremely rare and are not used for flood related planning determinations by Councils except when their failure has potential catastrophic consequences (such as dam failure).

#### 3.2.3 Hydraulic Modelling Results

The flood impact maps for the 1% AEP, 0.5% AEP and PMF / Extreme events are provided as Figures 3 to 5. The different colours reflect the change in peak water levels as a result of the proposed works. In summary the purple tones reflect a decrease in flood level whilst the blue/green/brown tones reflect an increase in peak level.



The proposed works do slightly decrease the amount of floodwaters from entering the northern floodplain across the river bank. Thus immediately upstream and to the east of the proposed works there is an increase in peak level in the 1% and 0.5% AEP events but minimal impact in the PMF / Extreme event. The increase in level in the 1% and 0.5% AEP events is largely within the confines of land owned by Shoalhaven Starches.

Downstream of the proposed works on Bolong Road there is a reduction in peak level of less than 0.1m. This occurs because the proposed works reduce slightly the amount of flood waters crossing through the site and thus flood levels are slightly lowered.

Should you have any questions or require further clarification regarding the above do not hesitate to contact the undersigned.

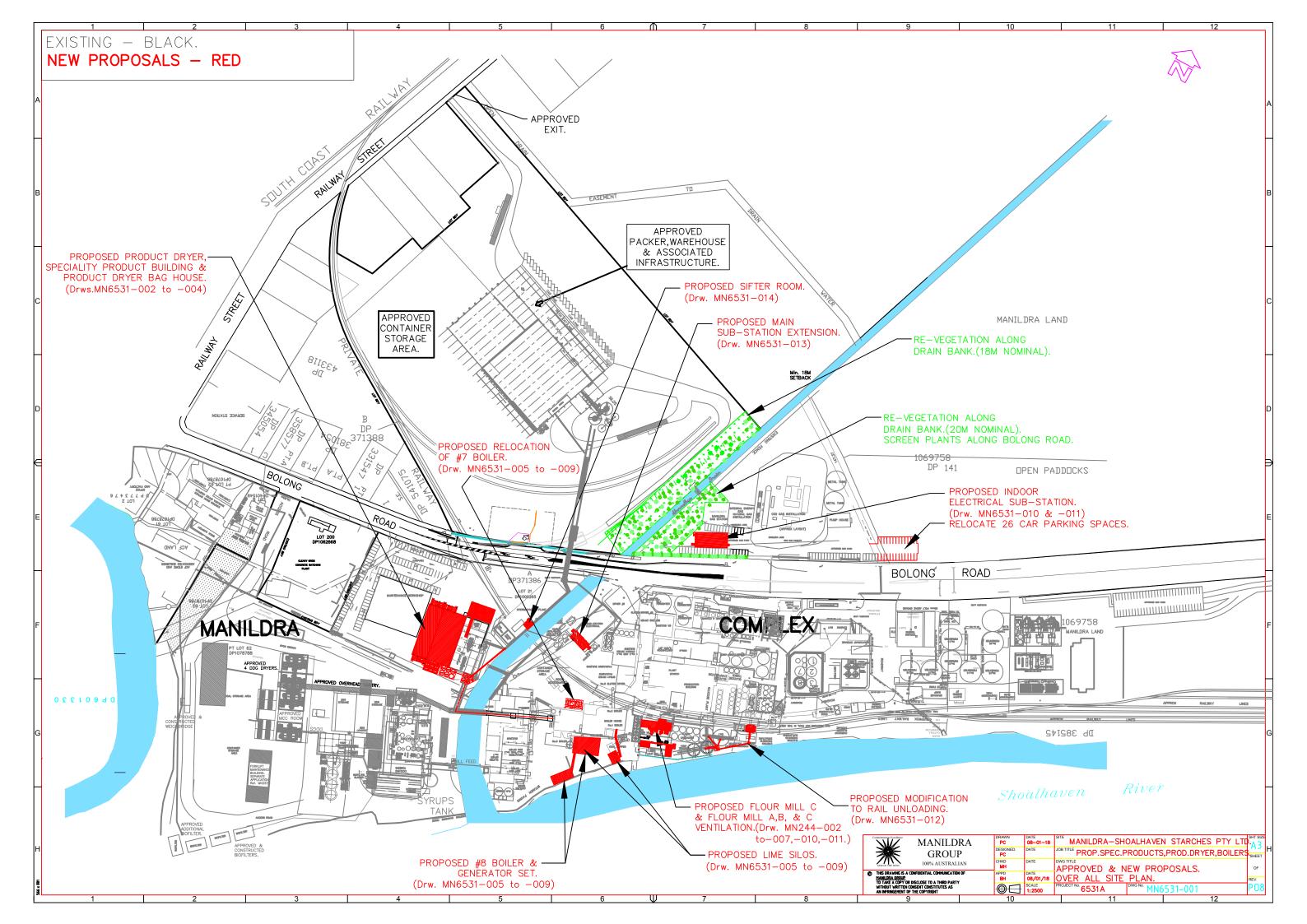
Yours Sincerely, WMAwater

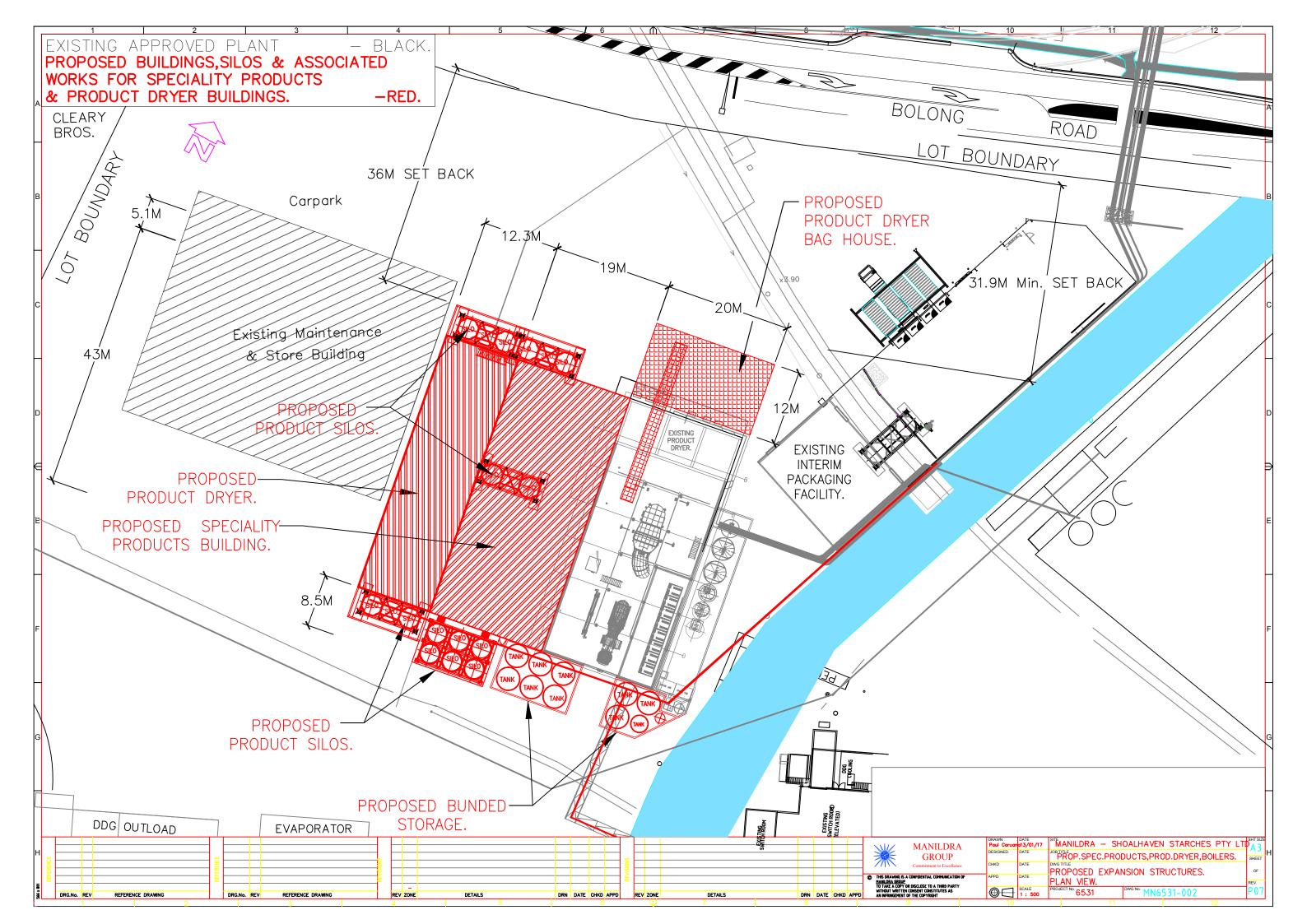
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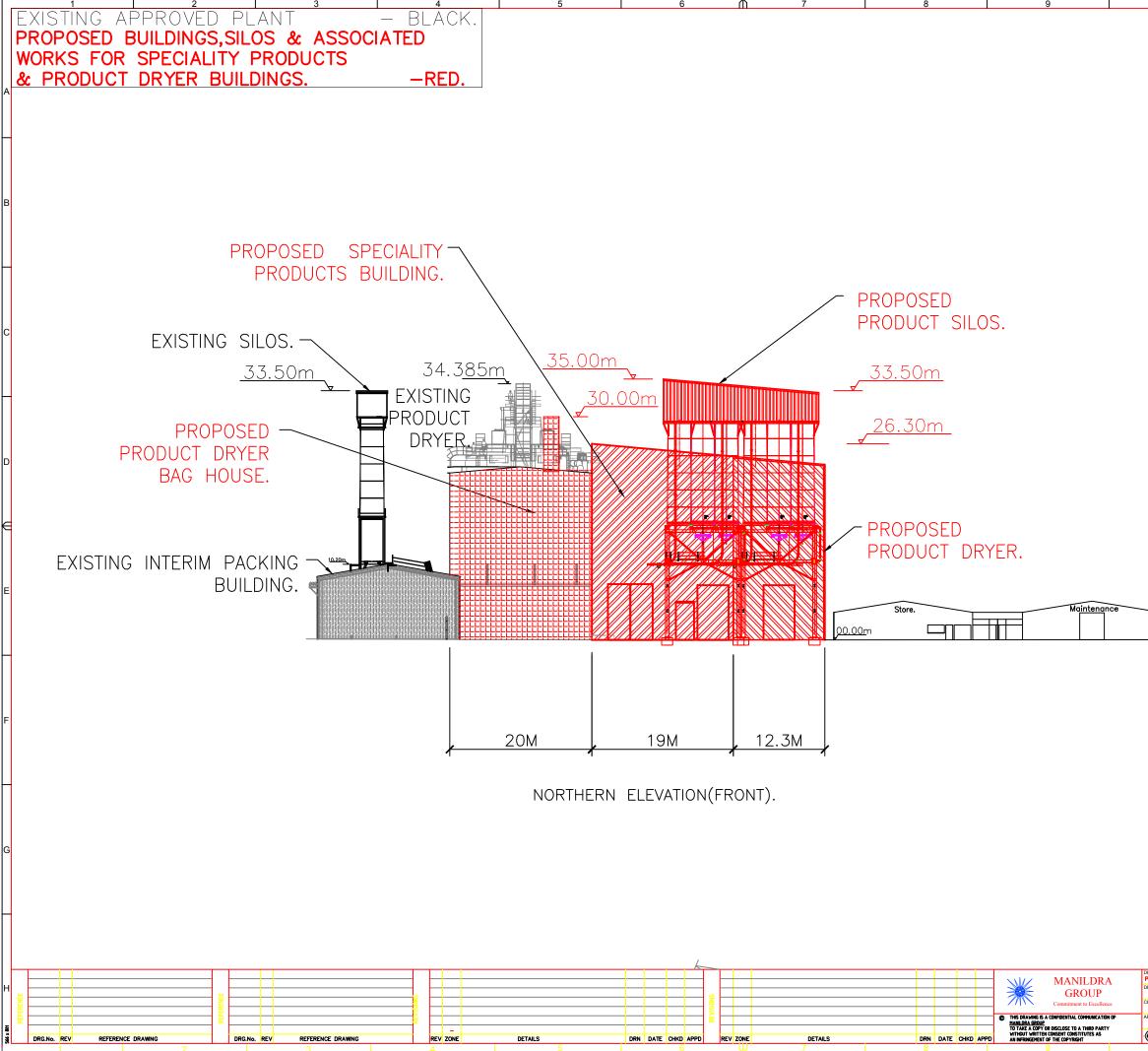
R W Dewar Director



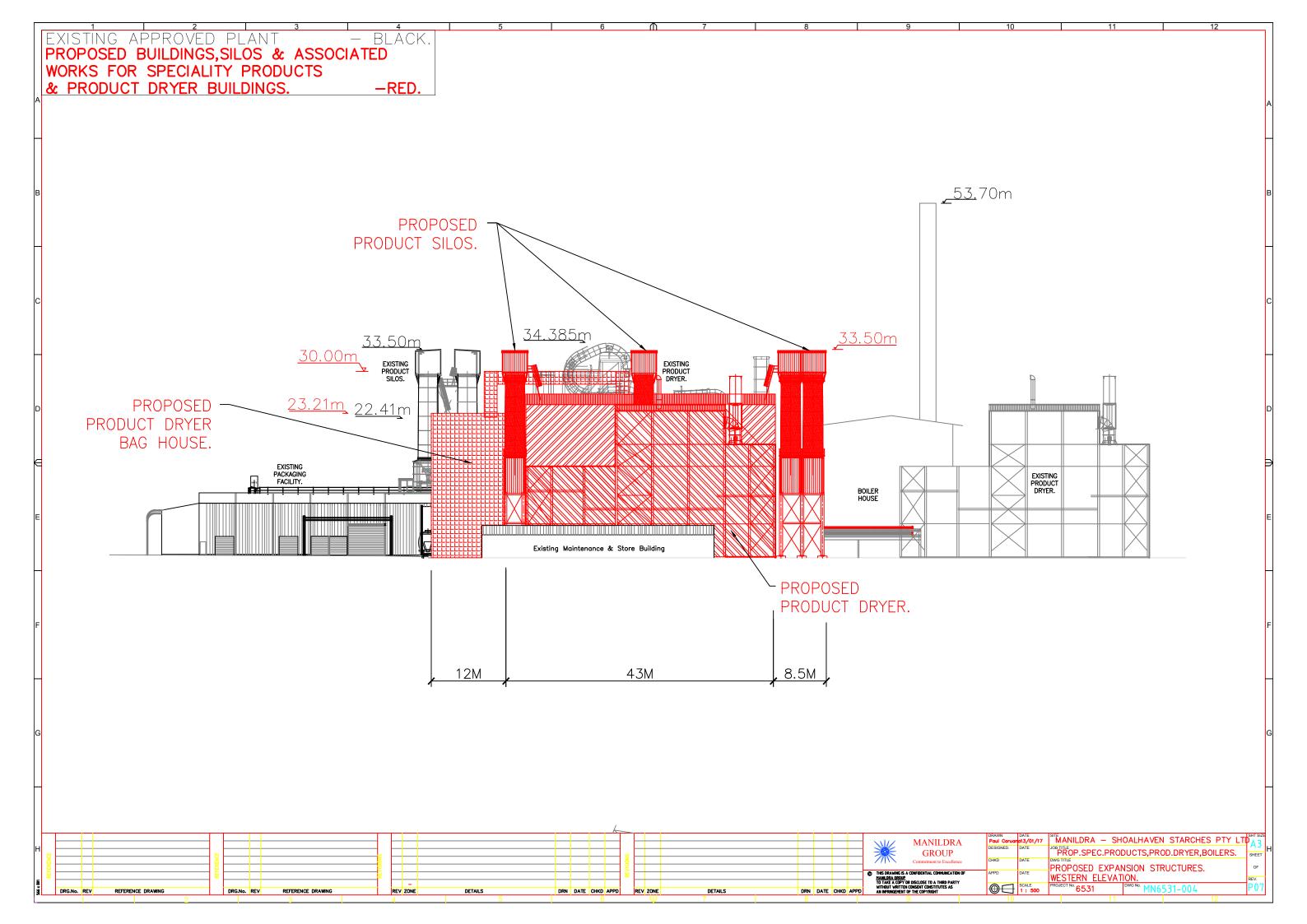


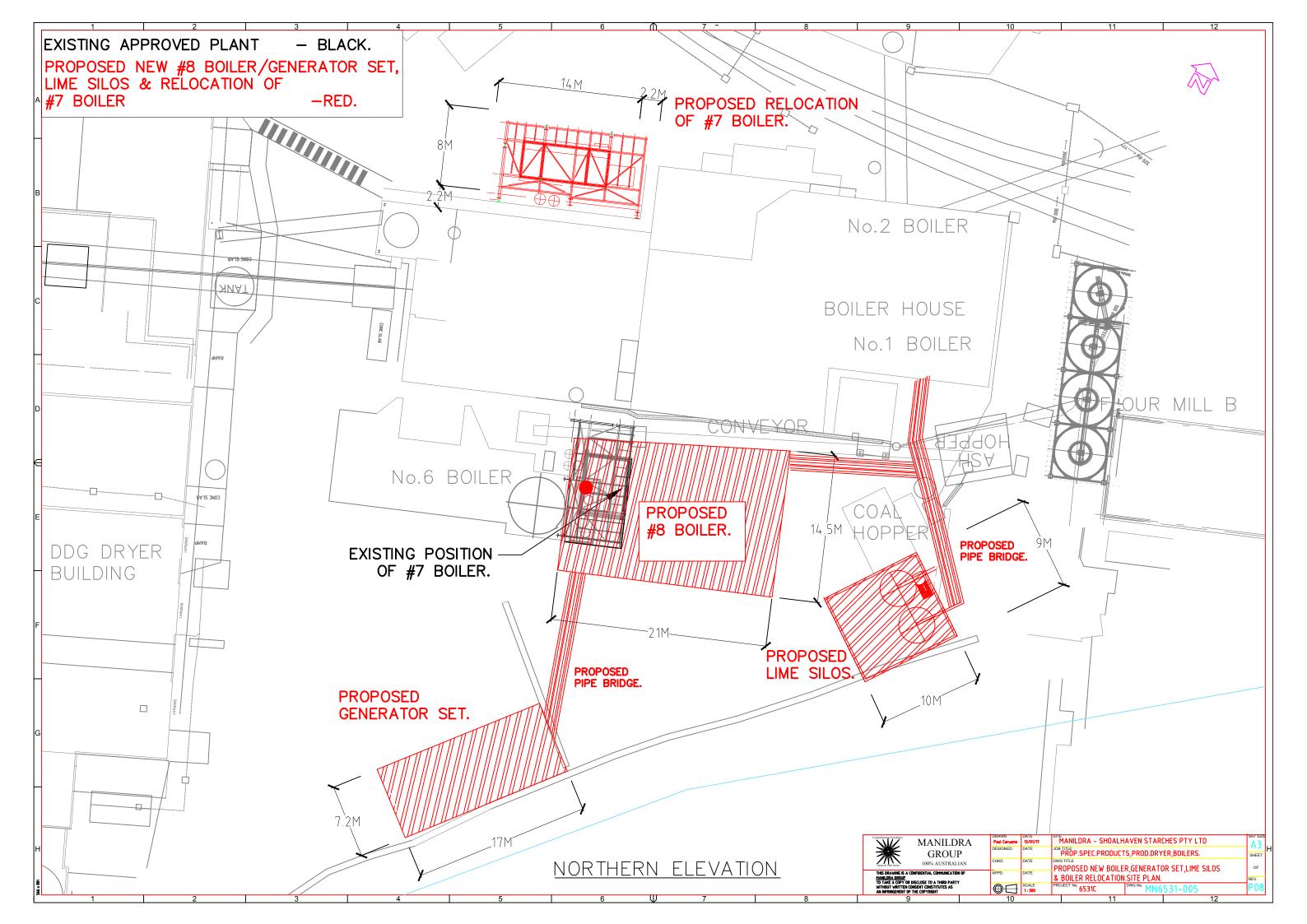


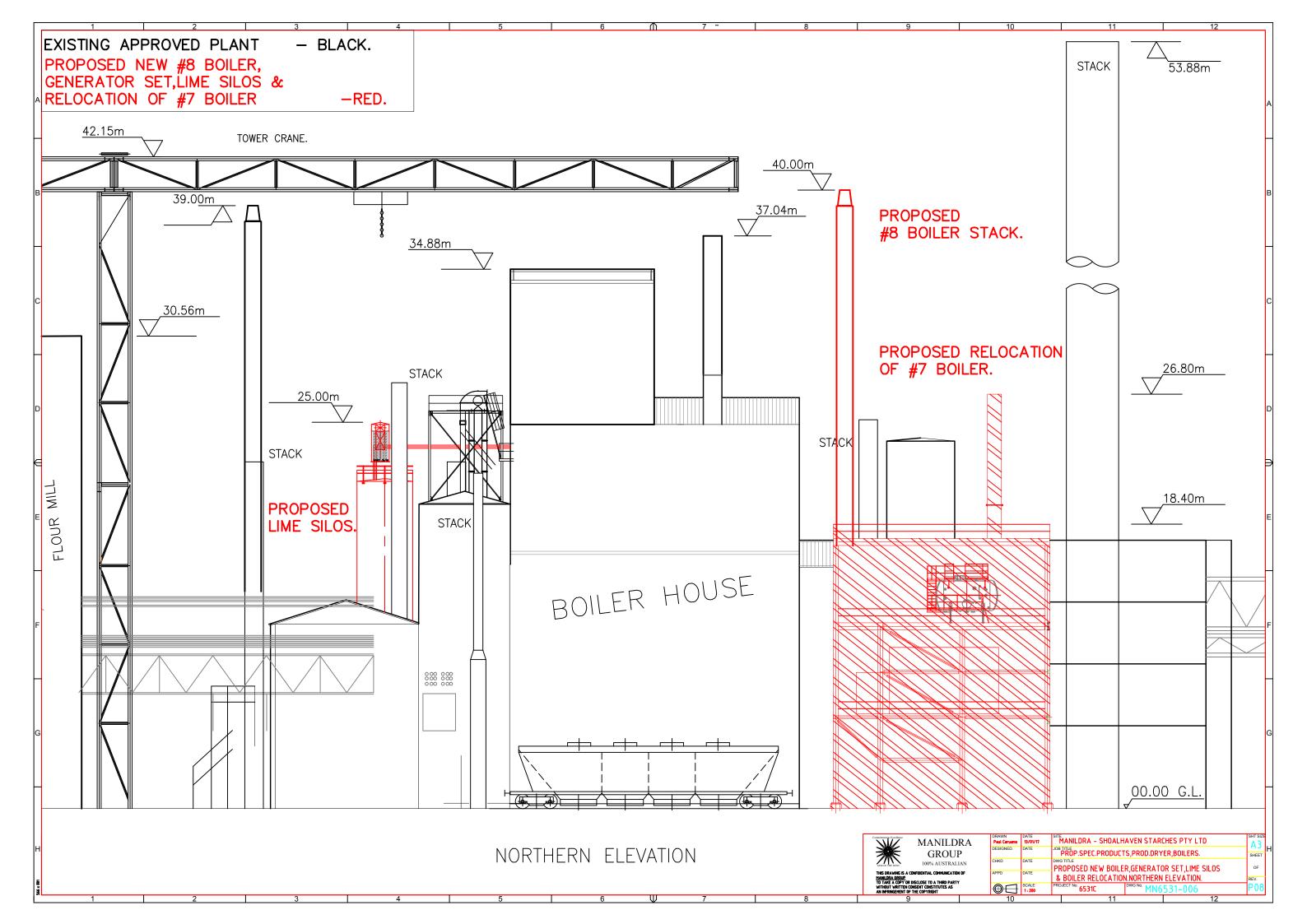


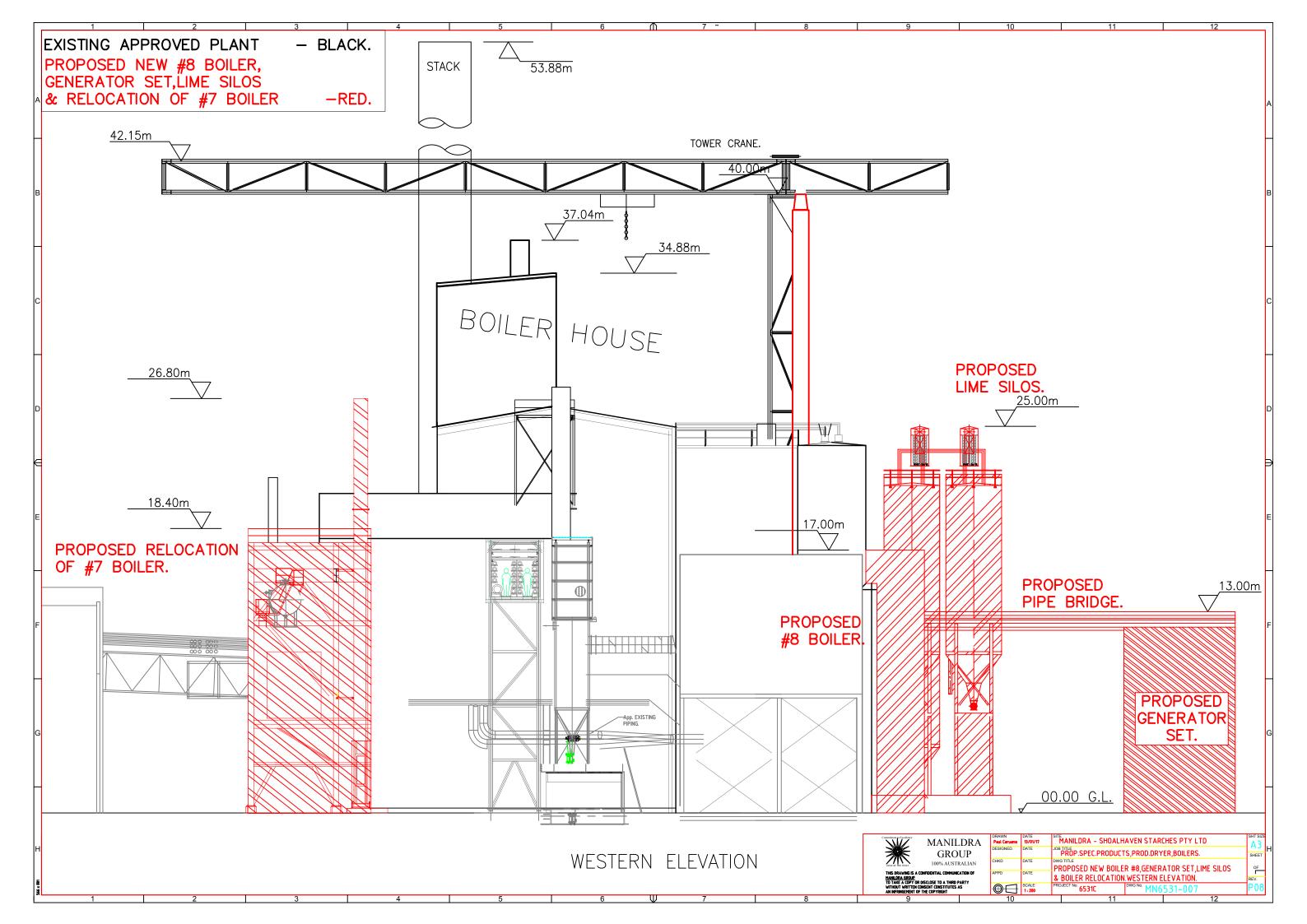


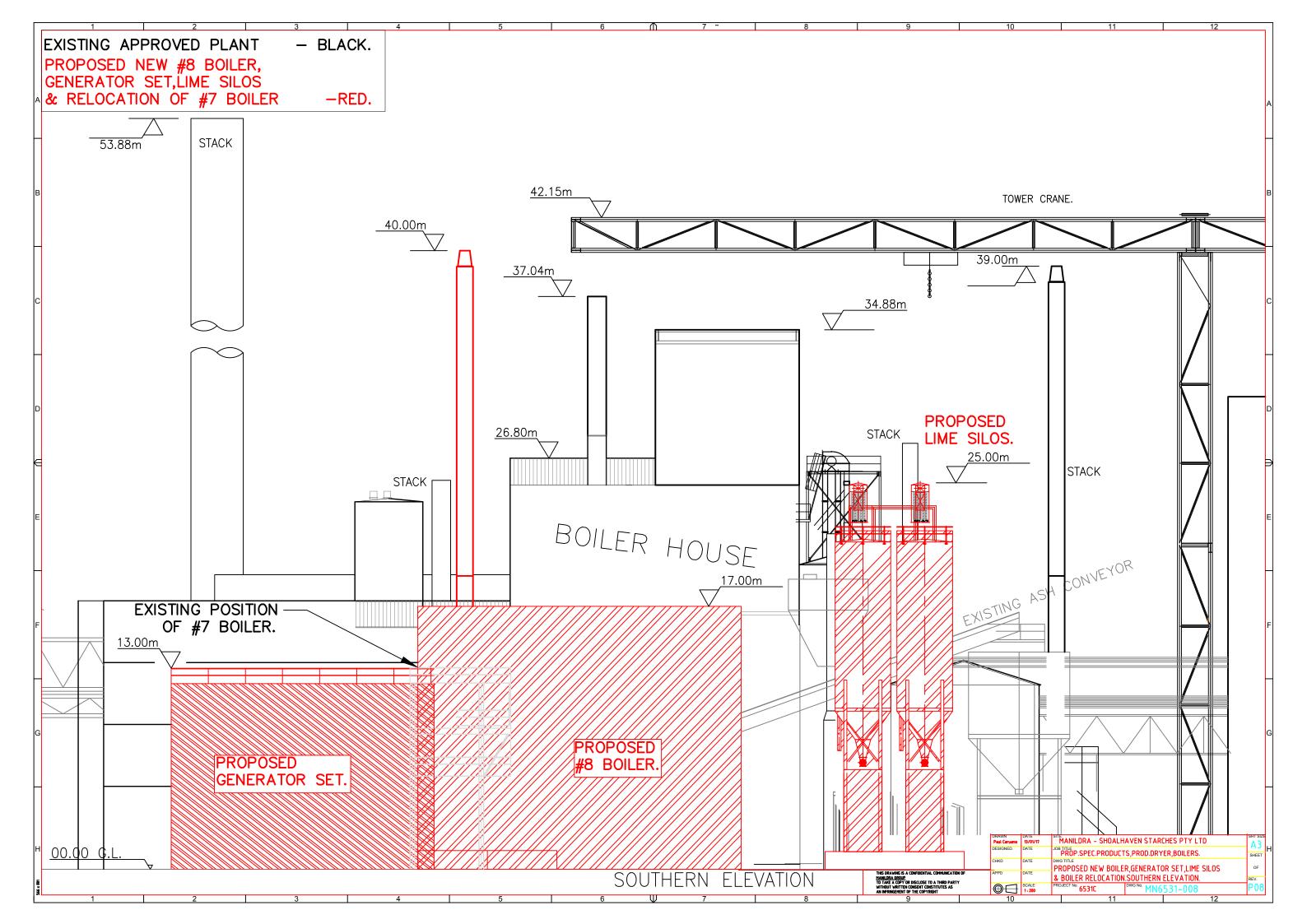
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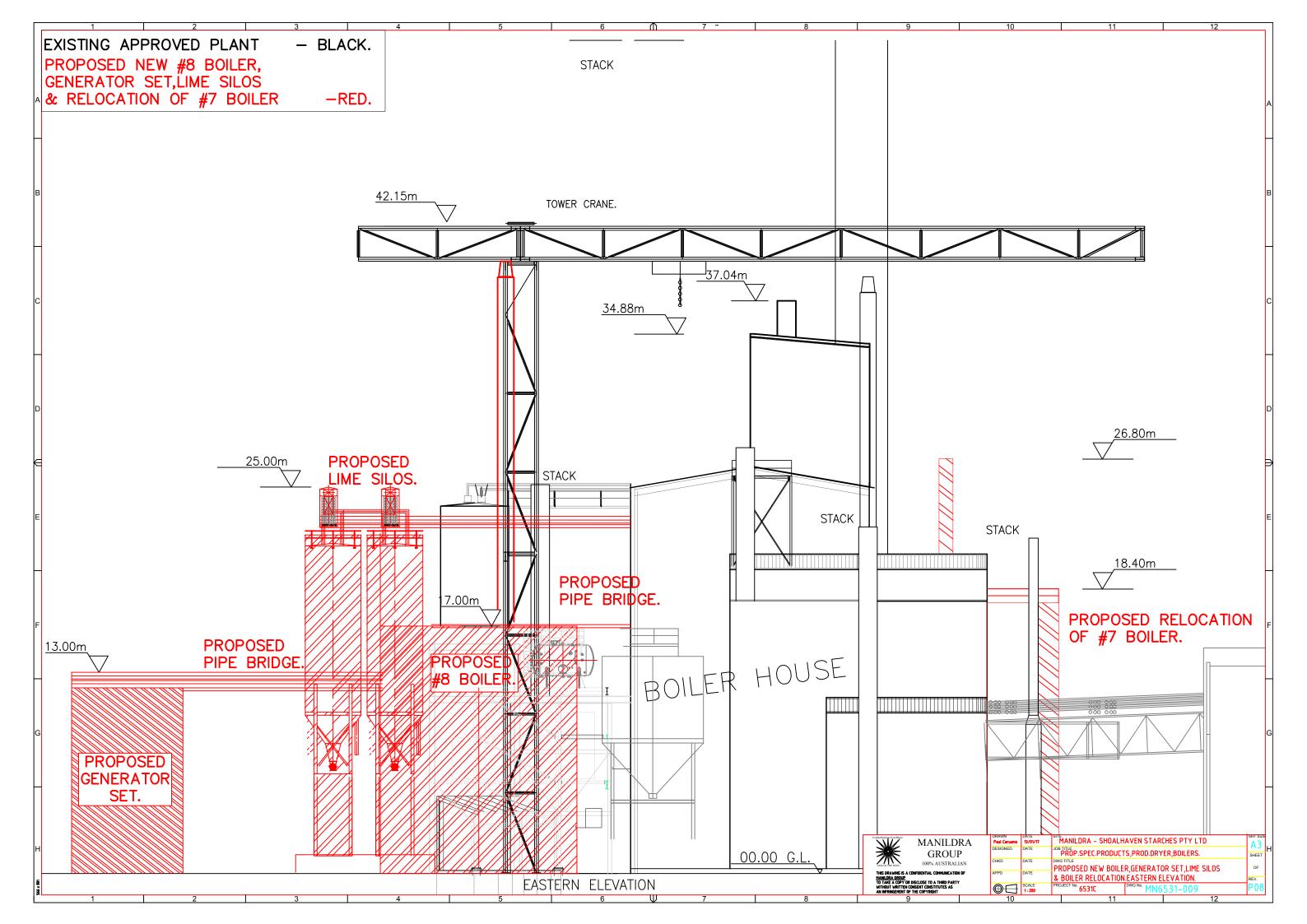


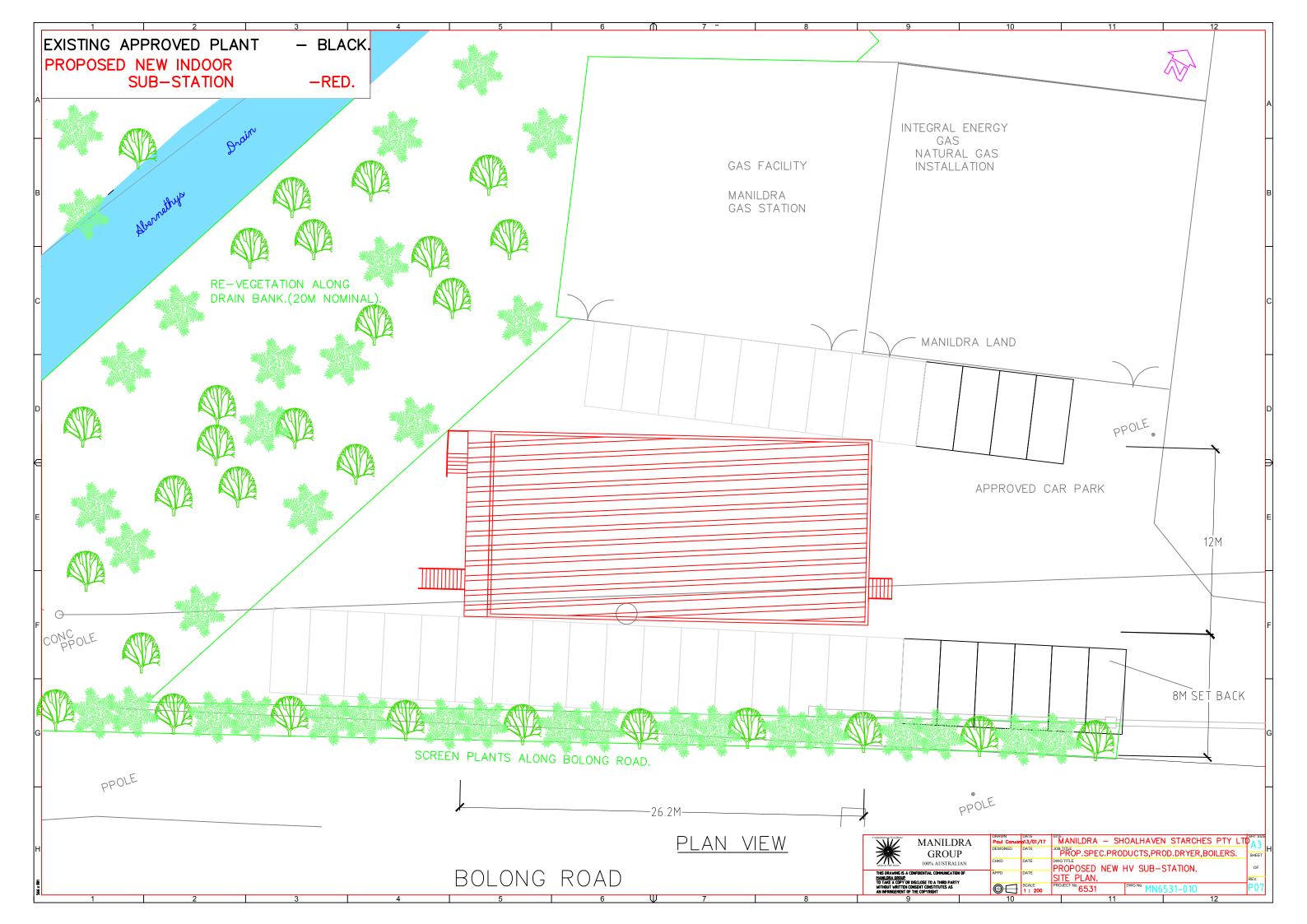


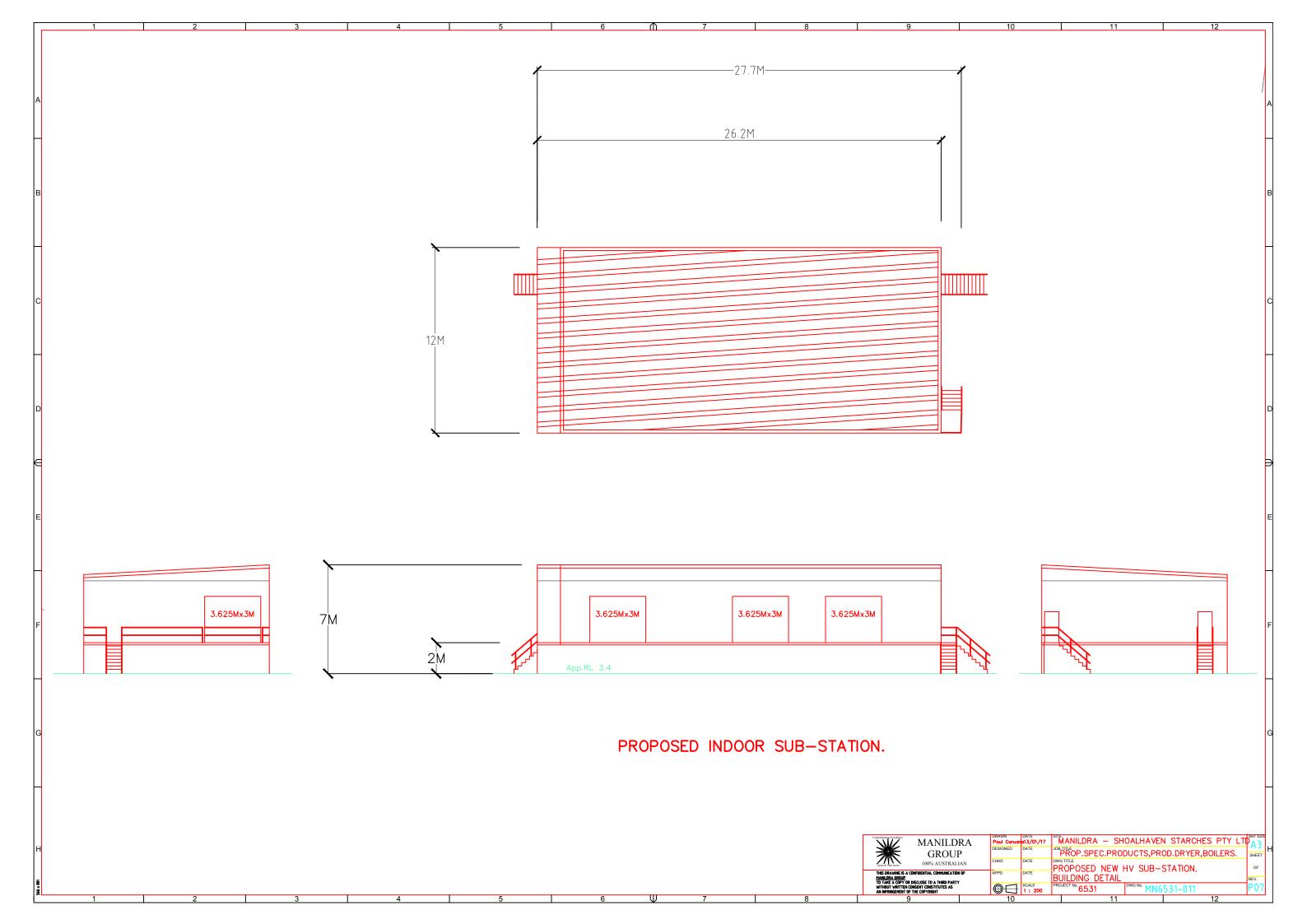


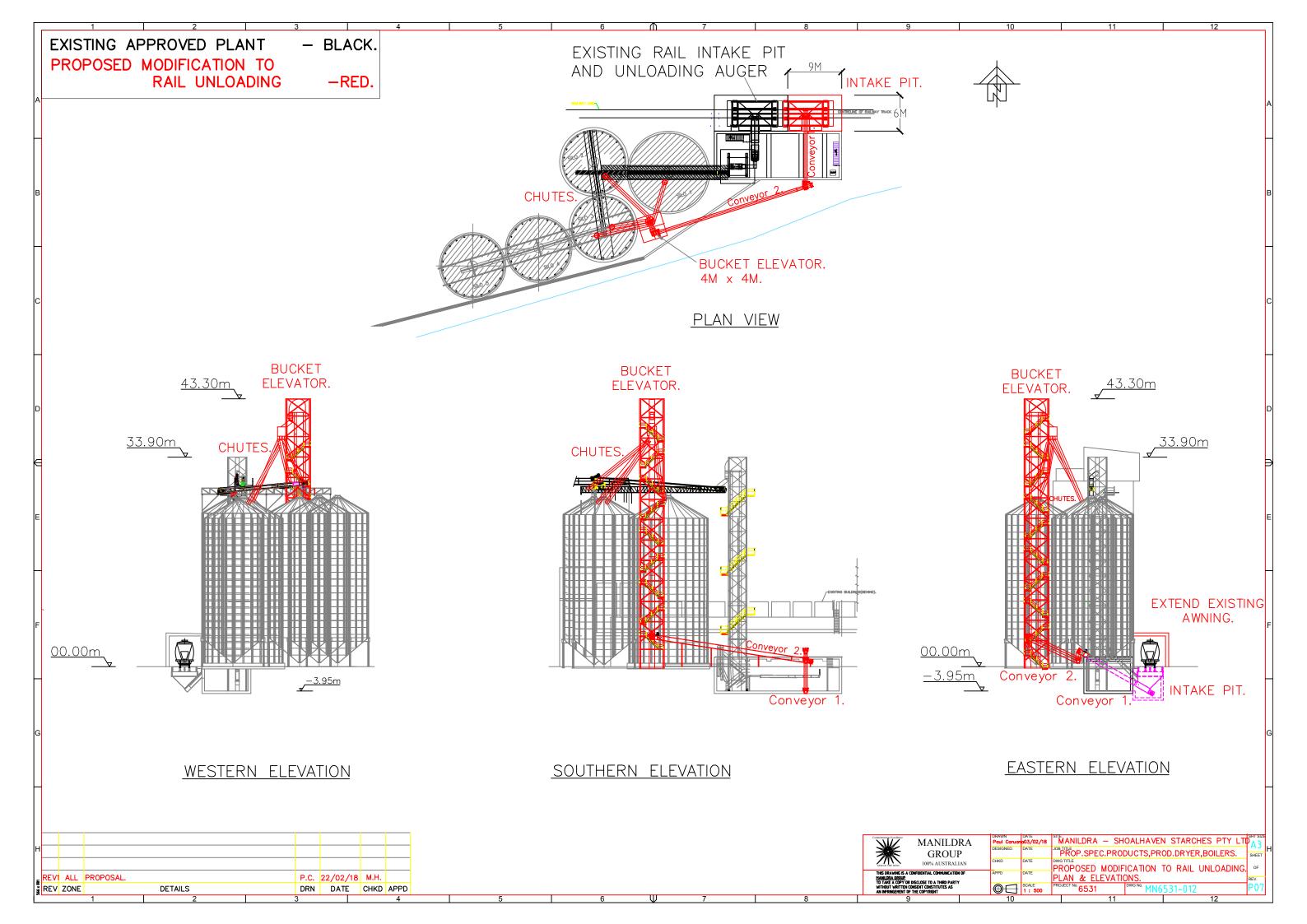


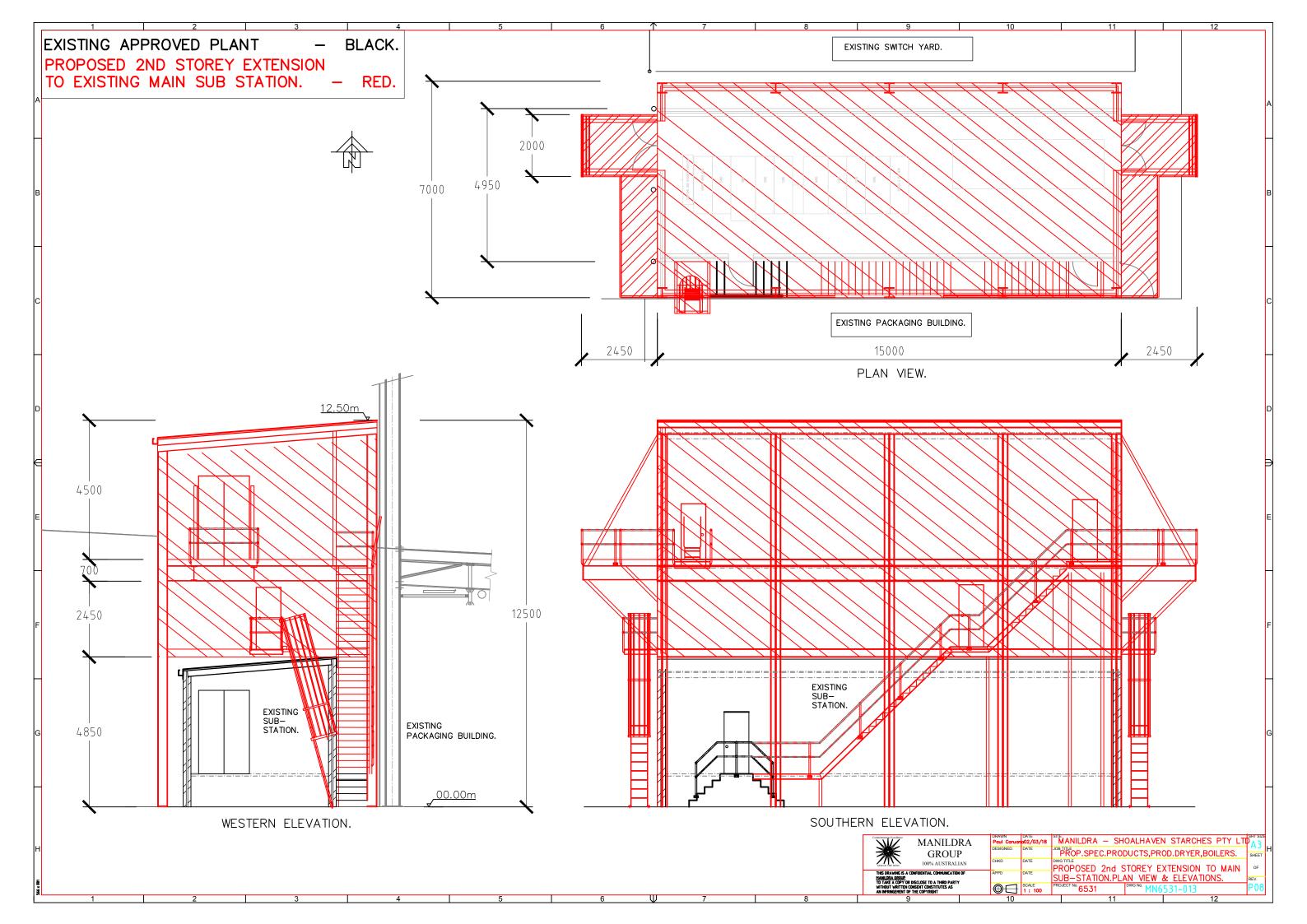


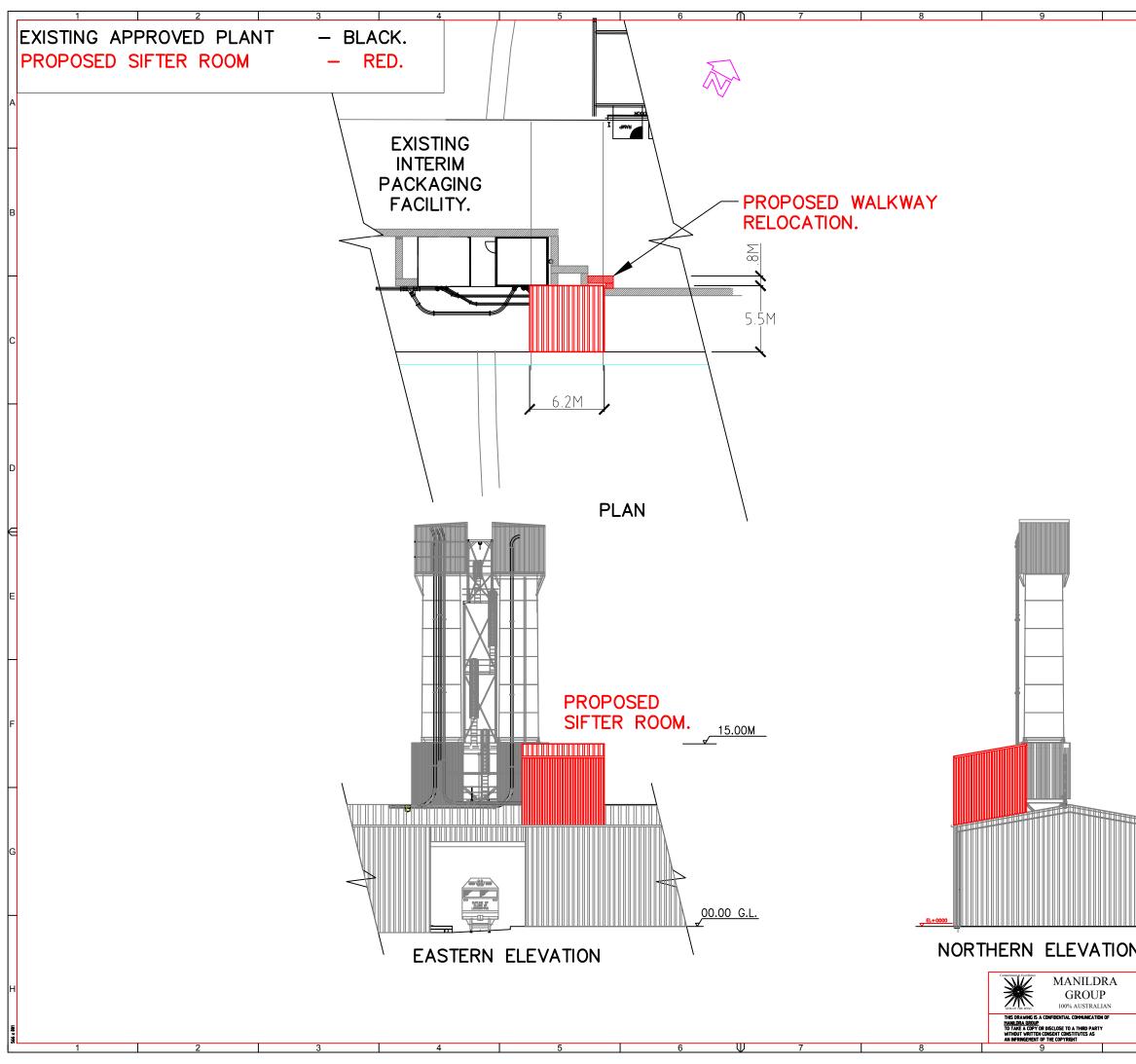




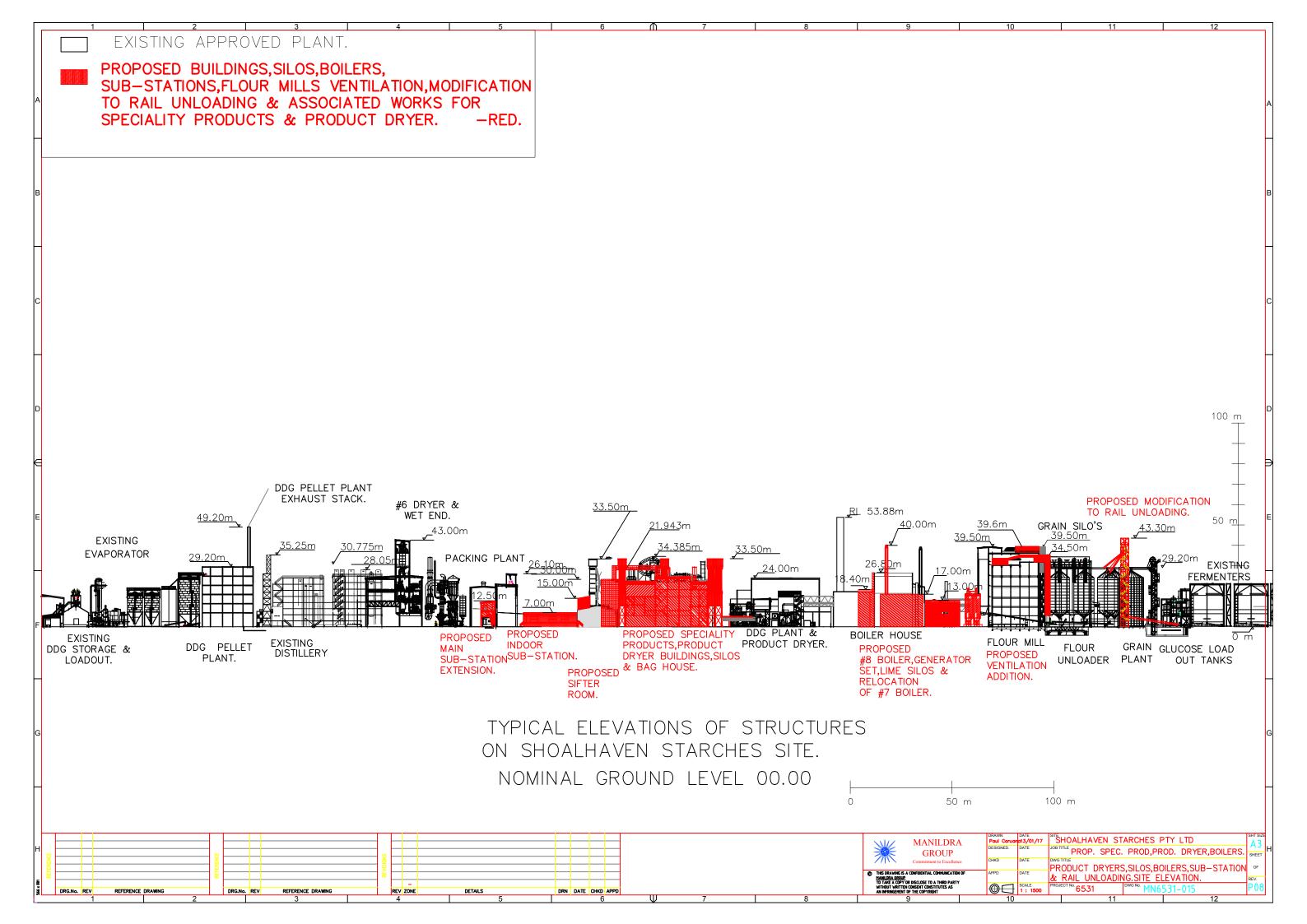


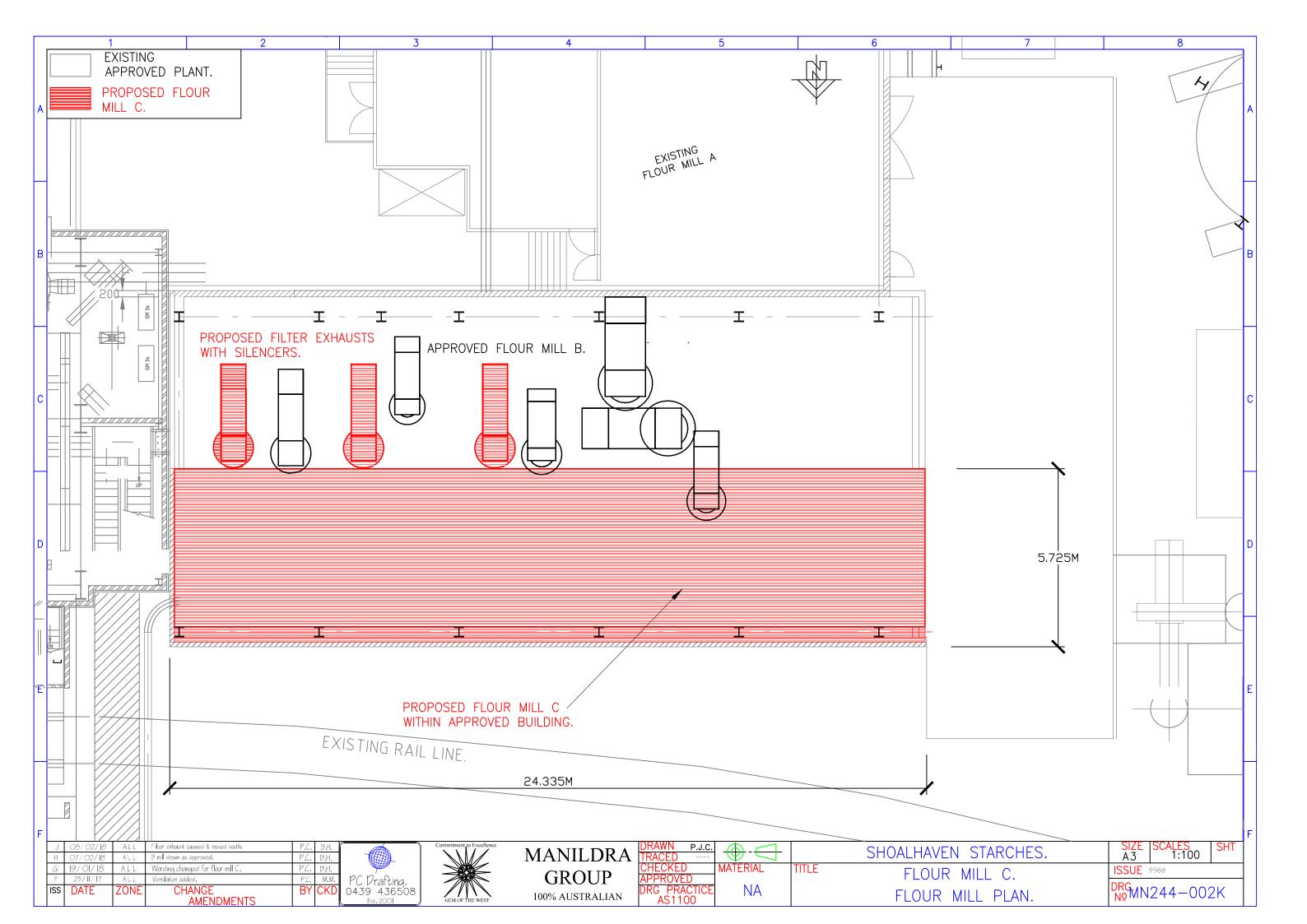


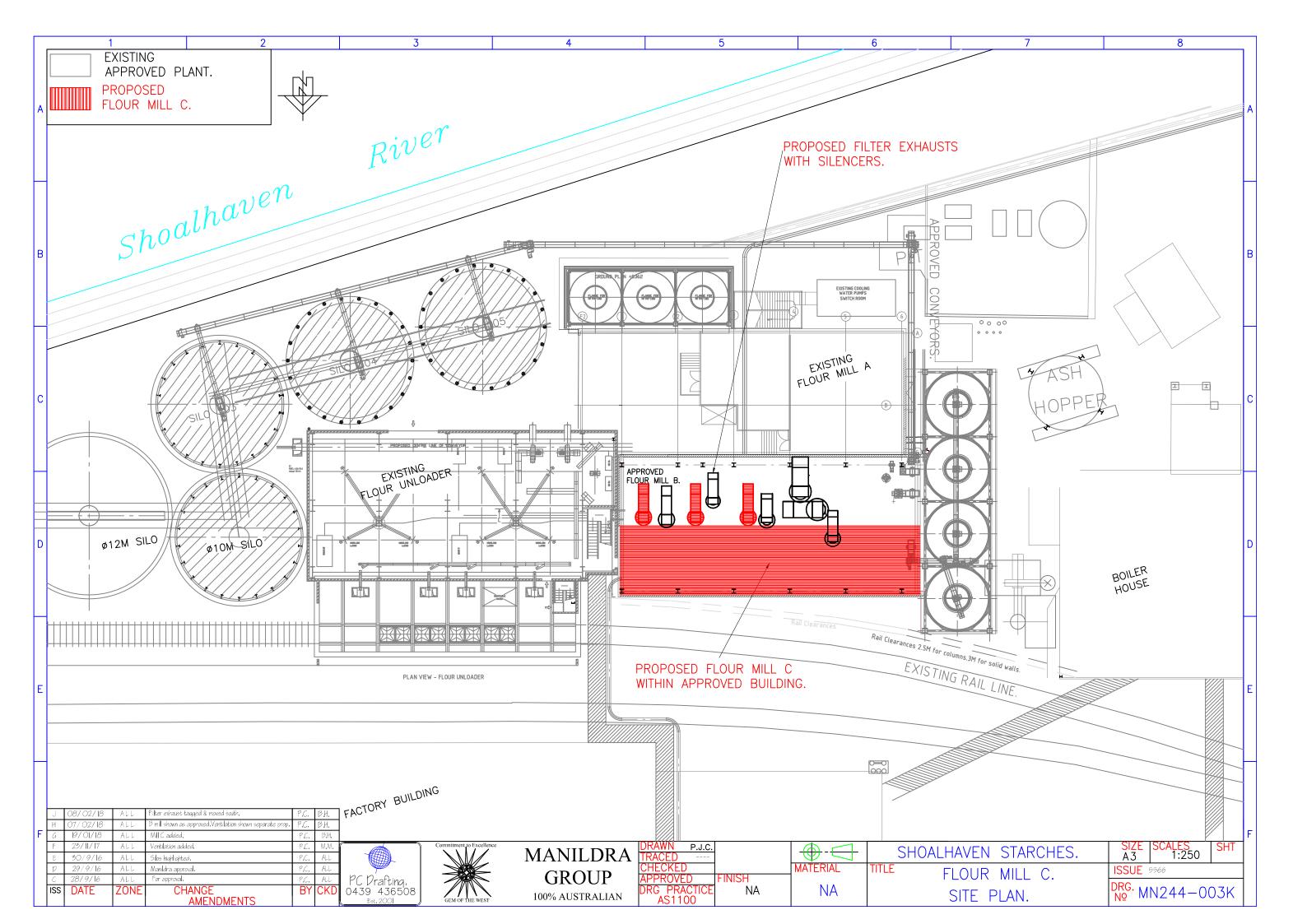


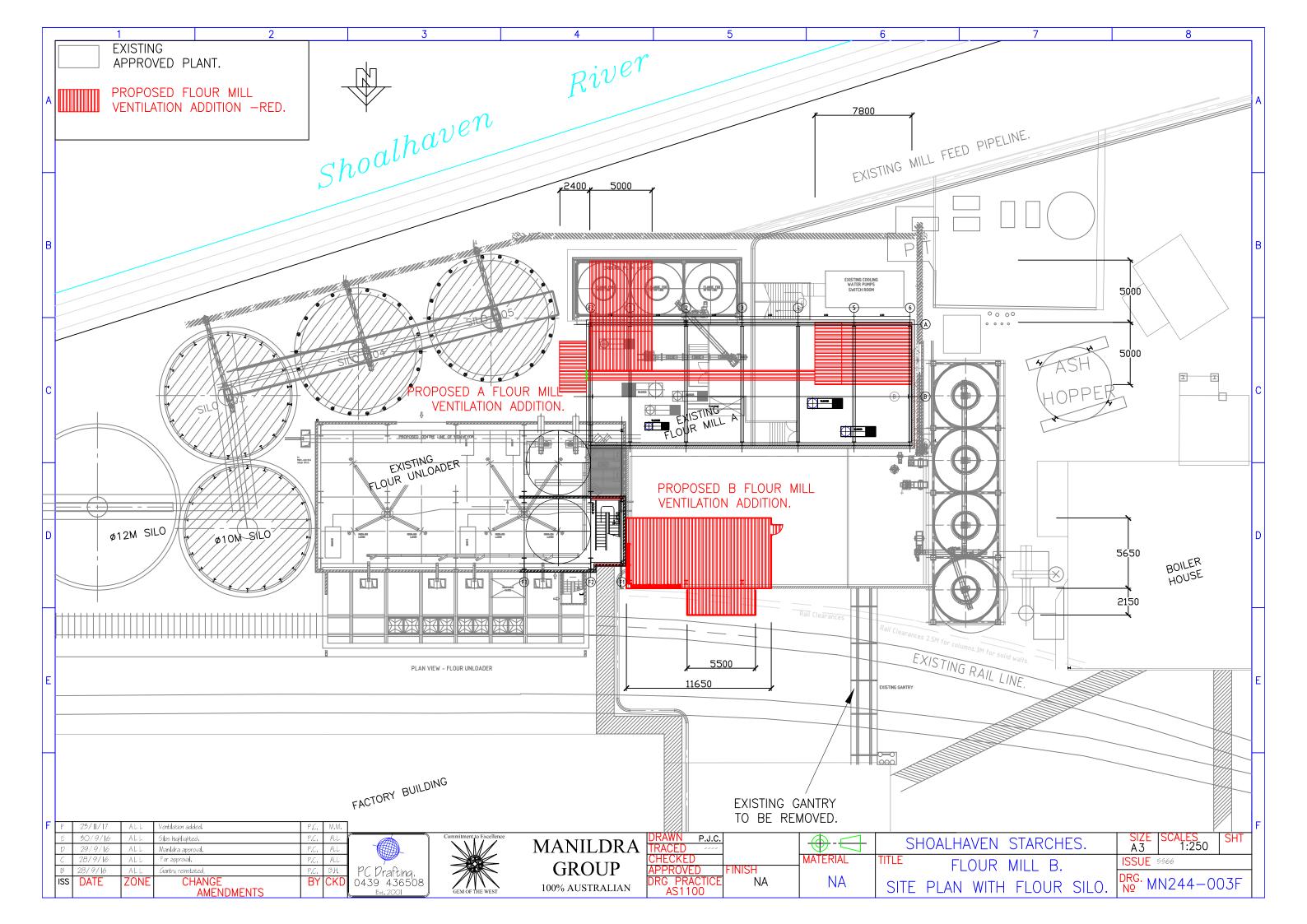


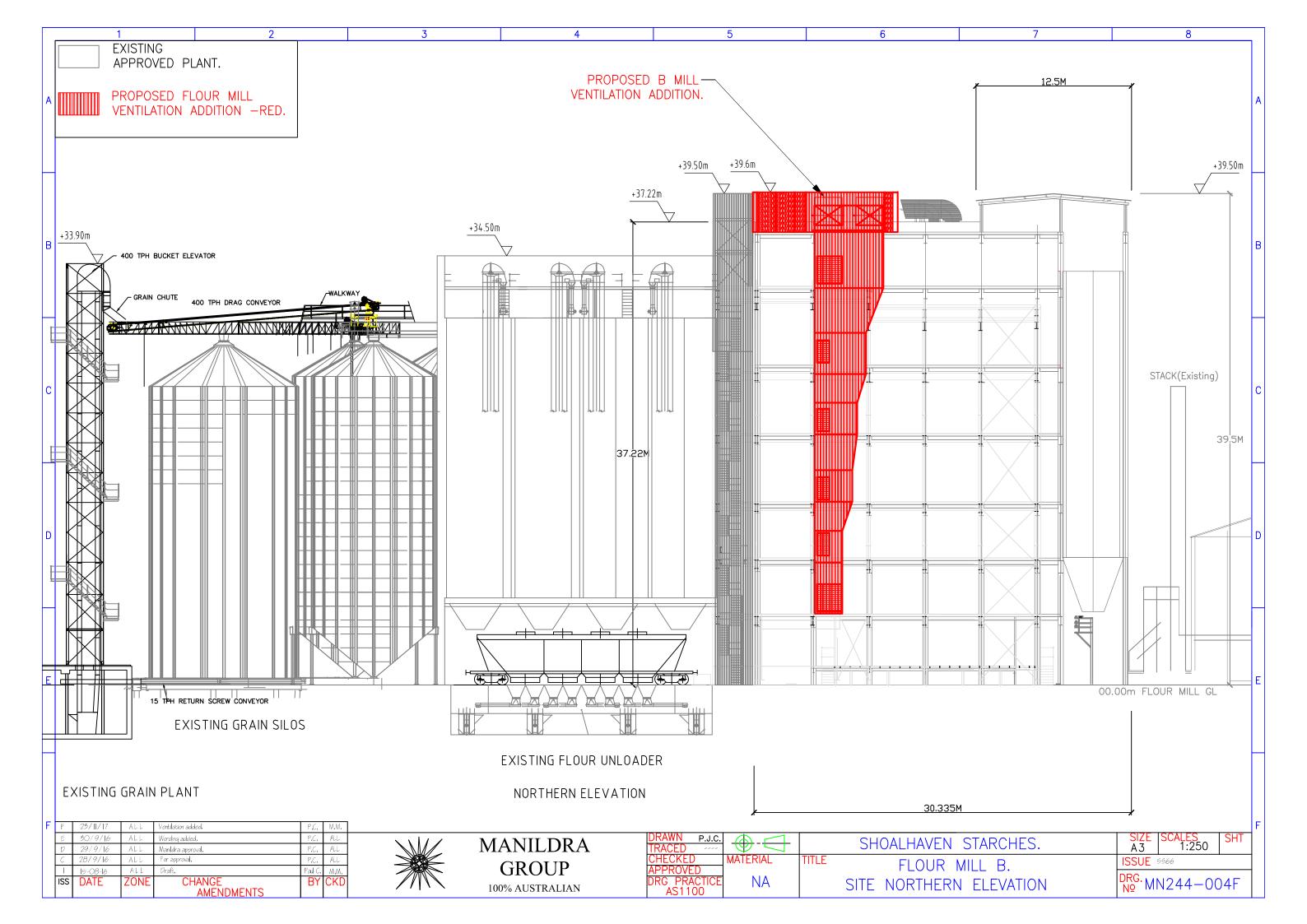
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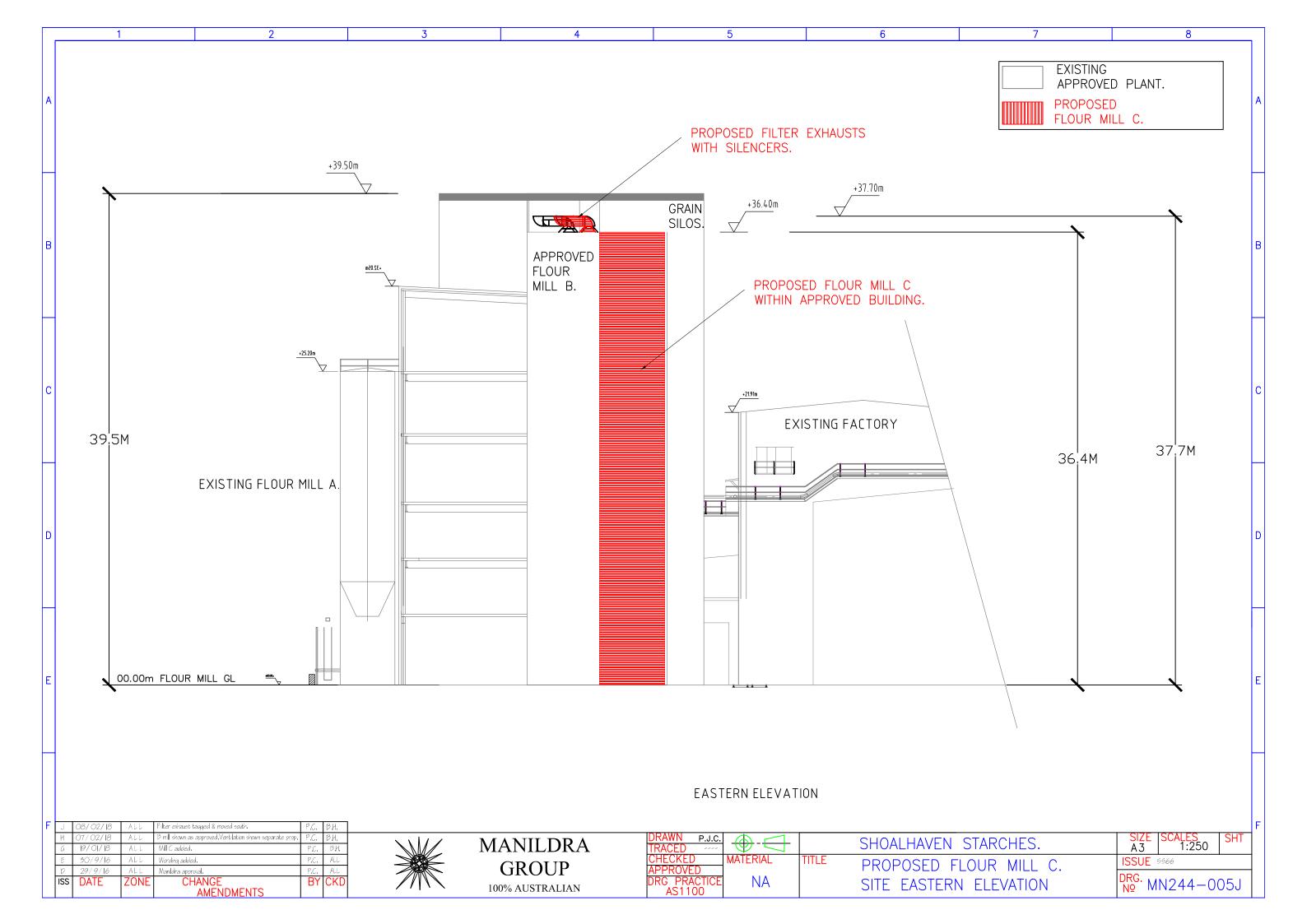


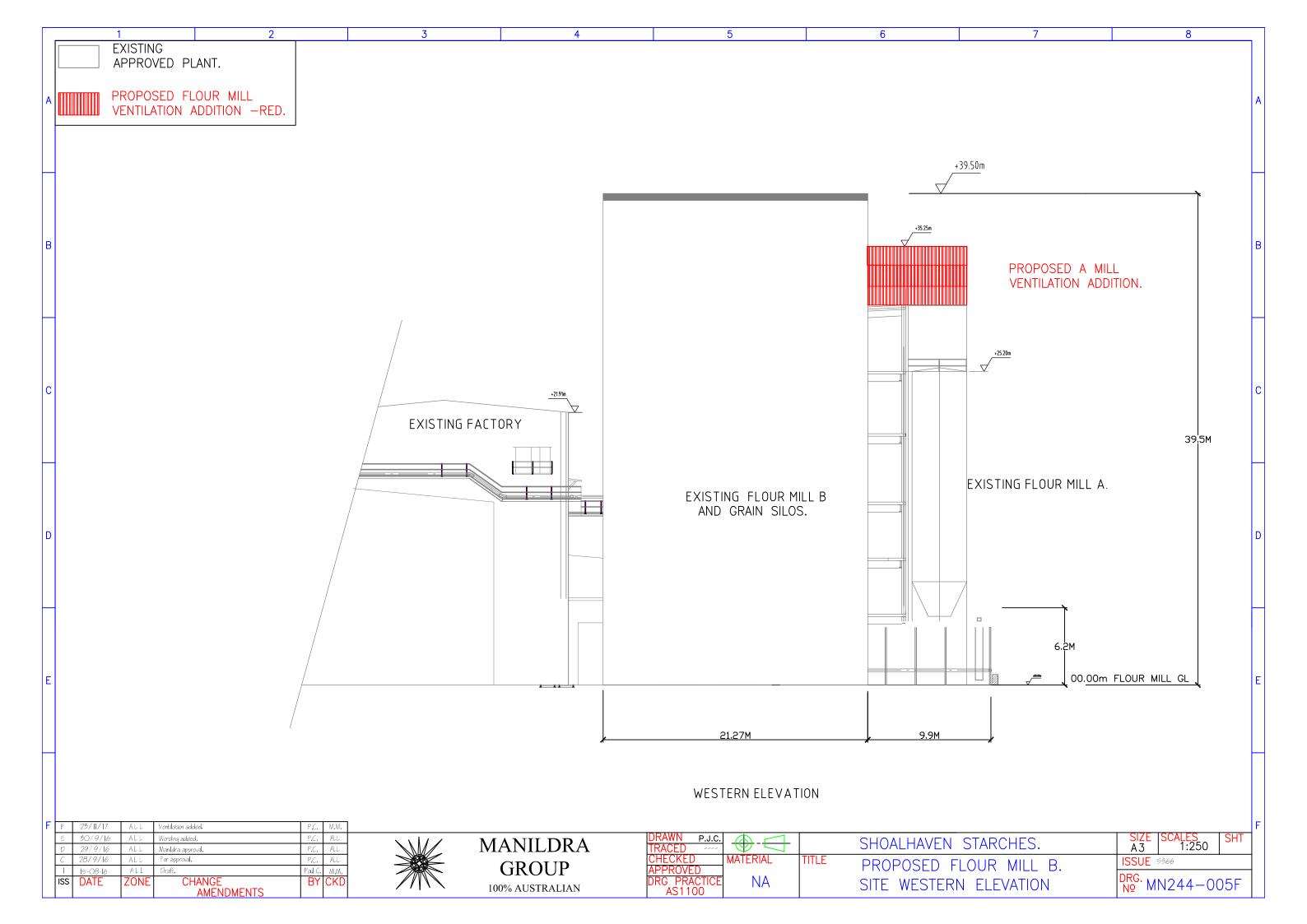


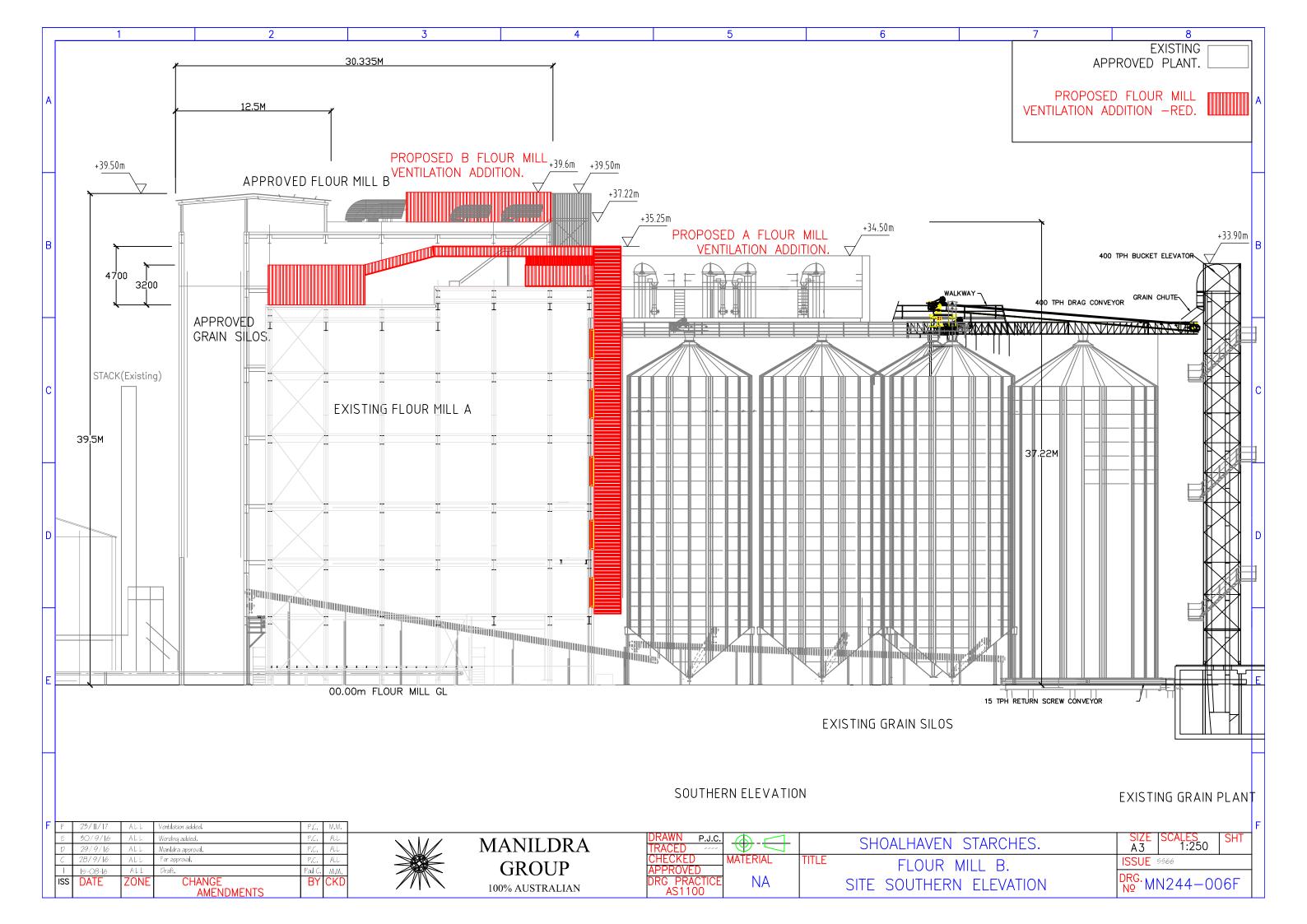


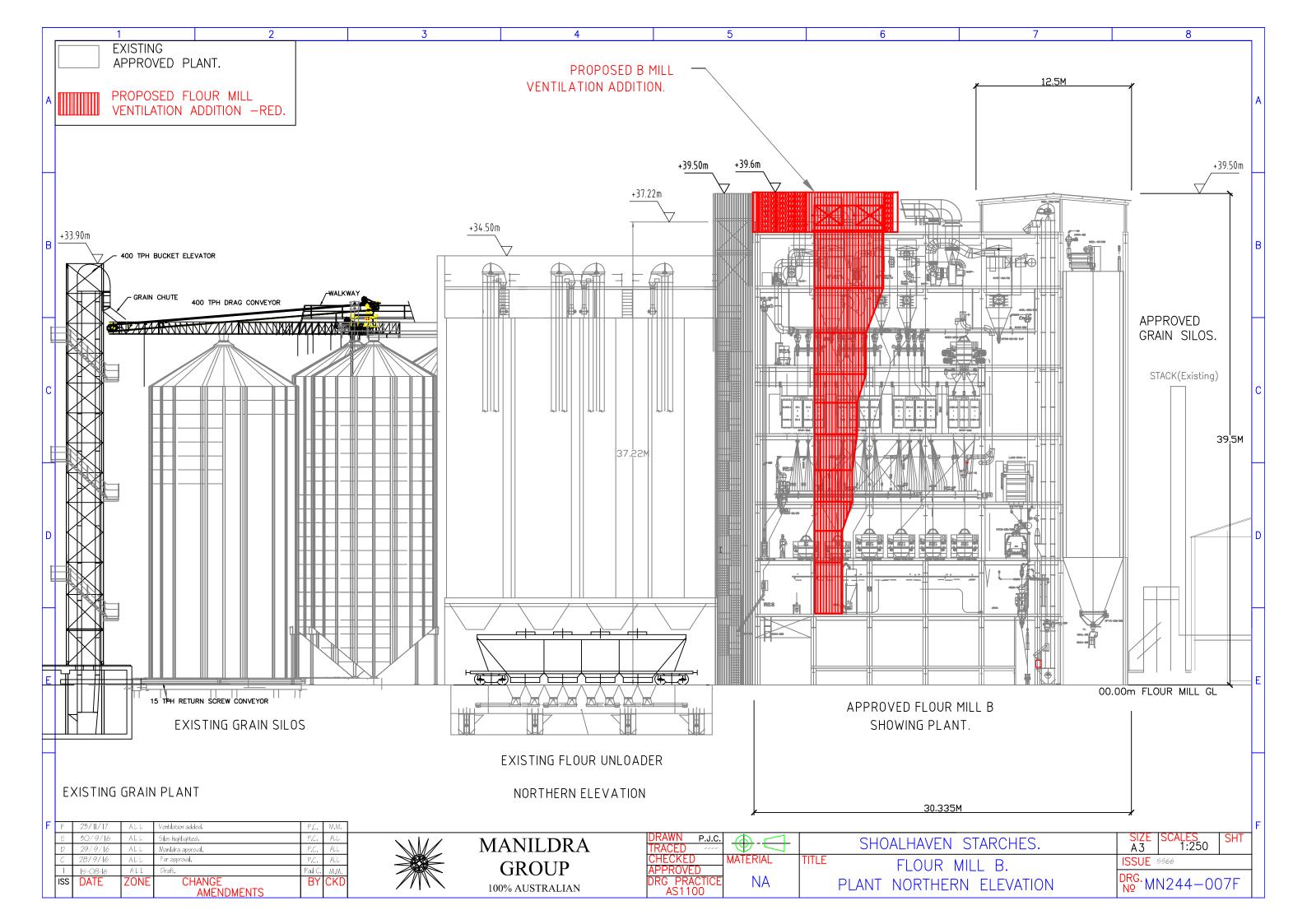


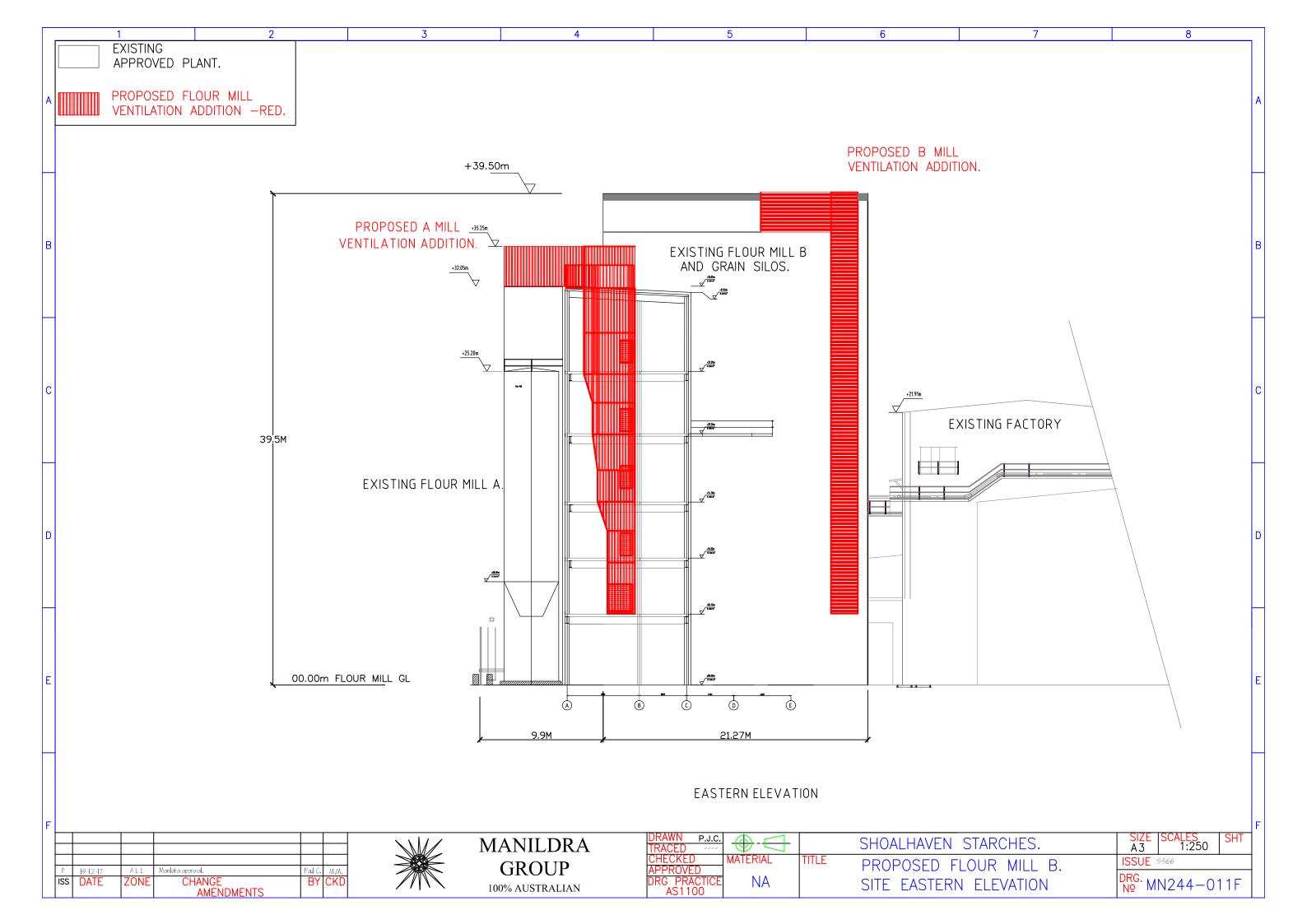


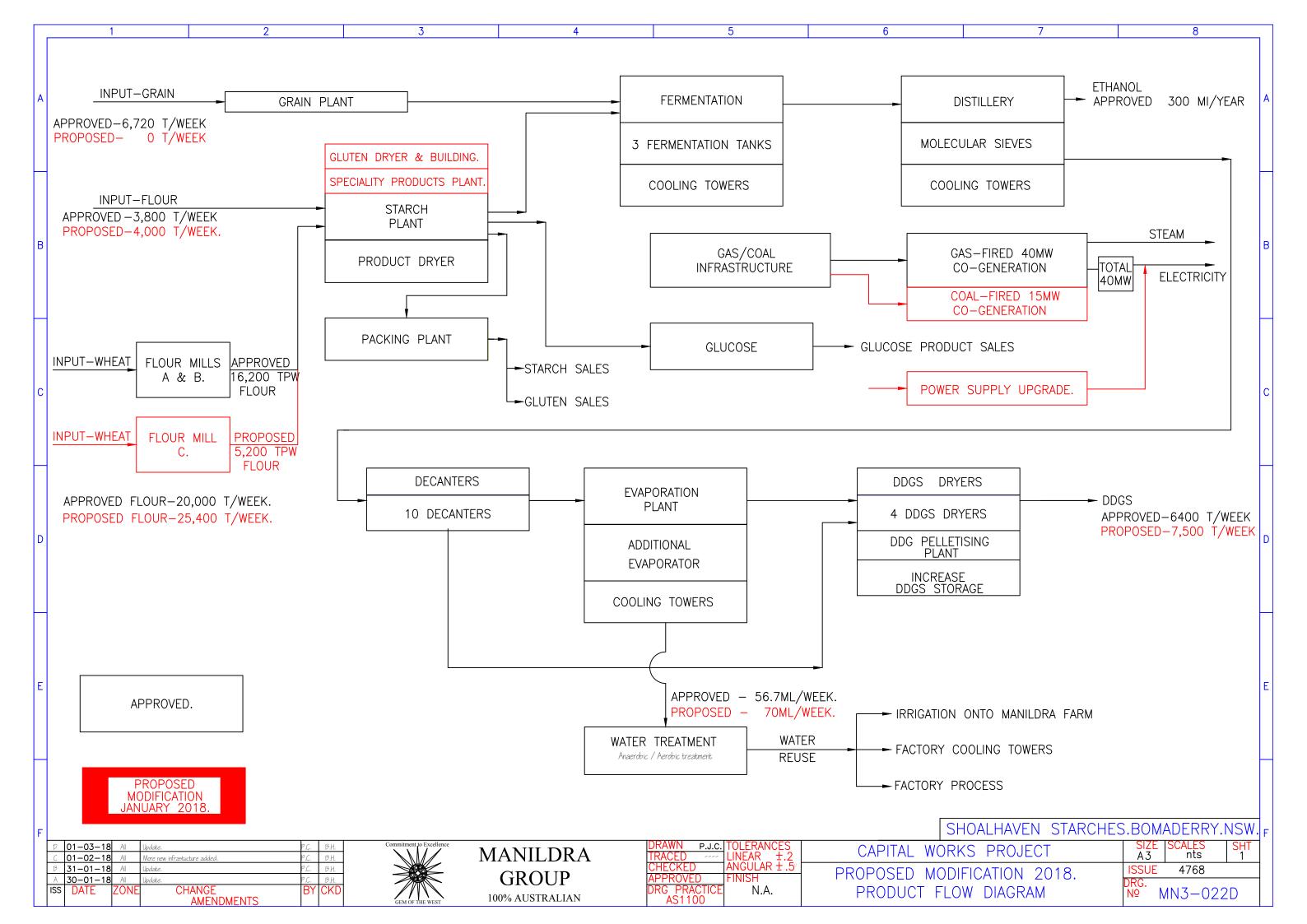




















Bridge Rd, Nowra NSW 2541 **02 4429 3111** Deering St, Ulladulla NSW 2539 **02 4429 8999** 

Address all correspondence to The General Manager, PO Box 42, Nowra NSW 2541 Australia DX5323 Nowra Fax 02 4422 1816

COUNCIL REFERENCE: 28112E (D18/102788) CONTACT PERSON: Kate Britton DATE: 5 April 2018

Stephen Richardson

Thank you for your recent inquiry in relation to flood data held by Shoalhaven City Council.

Please find below the original details of your inquiry, some general information on flooding as well as the requested property specific Flood Certificate.

Details of Inquiry:

Name of Inquirer	Stephen Richardson Date Requested: 14 Mar 2018
Reason for Enquiry	New Construction
Contact Details	Phone: 44236198 Email: steve@cowmanstoddart.com.au Postal:
Preferred Response	Email
Notes	
Survey Detail	Not Provided
Flood Safety Tip	Causeways can kill! Never drive through flood waters! Wait and be safe!
Flood Safety Tip General Flood Information	

# FLOOD CERTIFICATE

According to the Lower Shoalhaven River Floodplain Risk Management Plan – Climate Change Assessment (2011) the following properties are affected by the 1% AEP flood event:

Bolong Rd, BOMADERRY - Lot 141 DP 1069758 160 Bolong Rd, BOMADERRY - Lot 1 DP 838753 171 Bolong Rd, BOMADERRY - Lot 241 DP 1130535 Bolong Rd, BOMADERRY - Lot B DP 334511 Bolong Rd, BOMADERRY - Lot A DP 42232 22 Bolong Rd, BOMADERRY - Lot 21 DP 1000265 Bolong Rd, BOMADERRY - Lot B DP 376494 Bolong Rd, BOMADERRY - Lot 1 DP 385146 24 Bolong Rd, BOMADERRY - Lot 201 DP 1062668

These properties have been treated as a whole and levels provided are the highest for each event across the entire area.

### **FLOOD INFORMATION**

Year	Existing	Projected 2050	Projected 2100
Flood Planning Level	Not applicable	6.2m AHD	6.2m AHD

Hazard Category	High	High	High
Hydraulic Category	Floodway	Floodway	Floodway

Probable Maximum Flood Level	7.9m AHD	7.9m AHD	7.9m AHD
1% AEP Flood Level	5.7m AHD	5.7m AHD	5.7m AHD
2% AEP Flood Level	5.2m AHD	5.2m AHD	5.2m AHD
5% AEP Flood Level	5.0m AHD	5.0m AHD	5.0m AHD
10% AEP Flood Level	4.5m AHD	4.5m AHD	4.5m AHD

 Velocity (1% AEP flood event)
 1.0m/s\*
 0.9m/s\*
 0.9m/s\*

Minimal velocity information is available for this property therefore the provided velocity is approximate only

## SITE SPECIFIC CONSIDERATIONS

- Current NSW Government legislation requires climate change to be considered as part of this Floodplain Risk Management Study and Plan. Climate change related information evolves with time and it is expected that existing flood behaviour and levels may change in the future.
- 2. All applications for buildings, and the like, must take into account the projected 2050 flood information. All subdivision and other long-term planning must take into account the projected 2100 flood information.
- 3. Information provided in this flood certificate uses previous State Government sea level rise benchmarks (400mm and 900mm for the 2050 and 2100 horizon's respectively).

On Tuesday 10th February 2015 Council's Policy & Resources Committee resolved to no longer use State Government benchmarks and to "Establish a sea level rise benchmarks for planning purposes based on a 2030 horizon 100 mm, a 2050 horizon of 230 mm and 360 mm horizon for 2100". The new benchmarks will be incorporated into the flood information in future. Until studies incorporating the new benchmarks are undertaken Council will continue to use the best available information.

4. Not all of the property is categorised high hazard floodway. Part of the property is categorised high hazard flood storage. For more specific information regarding the different hazard and hydraulic categorisations on this property please contact Council's Natural Resource and Floodplain Unit on (02) 44293392.



1AEP 2050 1m contours



**1AEP 2050 hazard and hydraulic categories** (Red-High hazard floodway; Yellow- High hazard storage; Green – Low hazard storage; Brown – Flood planning extent)

# STANDARD CONSIDERATIONS

#### **Properties below the Flood Planning Level:**

Council considers the land in question to be below the flood planning level and therefore subject to flood related development controls. The conditions as set out below will reduce flood risk in flood events up to the Flood Planning Level, however the property may still be subject to flooding at higher levels during rare flood events.

#### Development controls apply to flood affected properties.

**Development conditions will vary depending on flood hazard, hydraulic category as well as the type of development that is proposed.** Please refer to the following documents for information on Council's flood related development controls and the NSW State Government's Floodprone Land Policy.

- Shoalhaven Development Control Plan Chapter 9: Development on Flood Prone Land <u>http://dcp2014.shoalhaven.nsw.gov.au/main-category/whole-document</u>
- NSW Floodplain Development Manual 2005: <u>http://www.environment.nsw.gov.au/floodplains/manual.htm</u>

#### DISCLAIMER

Your enquiry relating to the likelihood of the land specified in the application being flooded has been referred to the Council's Floodplain Engineer.

In responding to your application the Council seeks to bring to your attention the fact that pursuant to s.733 of the Local Government Act a council does not incur liability in respect of the giving of any advice furnished in good faith by the Council relating to the likelihood of any land being flooded or the nature or extent of any such flooding.

The Council does not have a legal obligation to provide advice to you and to the extent that this reply is giving advice, the Council provides that advice in good faith with the intention of preserving, so far as is legally possible, the Council's immunity from liability pursuant to s.733 of the Local Government Act.

While all reasonable care has been taken to ensure the accuracy of the information given in this reply, its purpose is to provide a general indication of flood risk in the area. Flood lines shown on Council maps indicate the approximate extent of flooding only in relation to the abovementioned land.

The information provided may contain errors or omissions and the accuracy may not suit the purposes of all users. A site survey and further investigation are strongly recommended before commencement of any project based on this data.

The information given is the most current information at the time of the request. It is to be noted, however, that flood information is constantly reviewed and updated and as such, the information contained in this regard is current only on the day of issue.

Before acting upon the information provided in this reply, the Council urges you to obtain separate and independent advice as Council, in giving this information, does not intend it to be relied upon in such a fashion as to impose liability upon the Council.

Should you not be prepared to accept the information contained in this reply upon that basis then you should immediately notify Council.

#### GLOSSARY

**AEP (Annual Exceedance Probability)** means the chance of a flood of a given or larger size occurring in any one year, usually expressed as a percentage – for example a 1% AEP flood event has a 1% chance of occurring in any one calendar year.

**AHD (Australian Height Datum)** is a common national surface level datum corresponding approximately to mean sea level.

**Flood fringe** is the part of the floodplain remaining after the floodway and flood storage areas have be defined.

**Flood planning area** is any land identified as being flood affected in the 1% AEP flood event plus freeboard.

**Flood planning level (FPL)** is the 1% AEP flood level plus freeboard. The FPL is used for planning purposes, as determined in floodplain risk management studies and incorporated in floodplain risk management plans.

**Flood prone land** means any land susceptible to flooding up to the probable maximum flood event (that is, land within the floodplain) as identified in an adopted Council flood study or floodplain risk management study and plan.

**Flood storage** areas are those parts of the floodplain that are important for the temporary storage of floodwaters during the passage of a flood.

**Flood study** is a technical investigation of flood behaviour. It defines the nature of flood risk by establishing the extent, level and velocity of floodwaters. The study also provides information on the distribution of flood flows across various sections of the flood plain for the full range of flood events up to and including the PMF.

**Floodplain risk management plan** is a plan developed in accordance with the principles and guidelines contained in the NSW Government Floodplain Management Manual. Usually includes both written and diagrammatic information describing how particular areas of flood prone land are to be used and managed to achieve defined objectives.

**Floodplain risk management study** is a study that identifies and compares various risk management options. This includes an assessment of their social, economic, ecological and cultural impacts, together with opportunities to maintain and enhance river and floodplain environments.

**Floodway** means those parts of the floodplain where a significant discharge of water occurs during floods. They are often aligned with natural defined channels. Floodway's are areas that, even if only partially blocked, would cause a significant redistribution of flood flow, or a significant increase in flood levels.

**Freeboard** is currently 0.5m for all catchments in the Shoalhaven. Freeboard is a factor of safety used to set the FPL (i.e. FPL = 1% AEP flood level plus freeboard (0.5m)). Freeboard takes into account uncertainties in flood modelling and climate change predictions, local factors that cannot be included in the flood model or wave action caused by wind, boats or vehicles driving through flood waters.

**Hazard category** represents the risk or danger to personal safety, evacuation movements and buildings and structures within the Flood Planning Area during the 1% AEP flood. There are only two possible hazard categories – high or low.

**Hydraulic category** describes the function of a specific part of the Flood Planning Area in conveying flood waters during a 1% AEP flood. There are three possible hydraulic categories – floodway, flood storage or flood fringe.

**Probable maximum flood (PMF)** is the largest flood that could conceivably occur at a particular location, usually estimated from probable maximum precipitation. Generally, it is not physically or economically possible to provide complete protection against this event. The PMF defines the extent of flood prone land, that is, the floodplain.

**Provisional** is used for hazard categories that have been determined in a flood study. Hazard categories are provisional until the floodplain risk management study and plan has been completed and adopted by Council, as this document considers additions risks, not considered during the flood study.

Further Development within the Manildra Starches Plant off Bolong Road, Bomaderry

#### 3. CONCLUSIONS

#### 3.1 Proposed Development

For the reasons detailed above, and as agreed in consultation with Council and DLWC, hydraulic modelling of the proposed development has not been undertaken. There is a need however, to consider (amongst other things) the flood hazard and structural assessment (with regard to velocity of floodwaters and impact by flood debris) of the proposed development. In quantifying the flood hazard, some important issues for consideration include:

- damage to the plant, including as a result of flood debris or structural failure,
- damage to the plant due to the possible buoyancy of equipment,
- malfunction of the plant (or any services on which the plant relies for operation) as a result
  of inundation and the associated risk of such malfunction to other users of the floodplain,
  access and evacuation.

#### 3.2 Future Development

In consultation with Council and the DLWC, it is agreed that any future development of the Manildra Starches Plant within the intensively built-up area, as defined on Figures 2 and 4, will not require hydraulic modelling to quantify the hydraulic impacts and cumulative effects. The hydraulic impacts and cumulative effects of such developments are considered to be insignificant given the intensive development already present. As mentioned in previous sections, the only opportunity for floodwaters to pass through the Intensively built-up area of the site is through the timited number of gaps or openings between the plant and associated buildings. Although these gaps or openings may be relocated to accommodate any future development, the movement of overland floodwaters will never be completely blocked, as gaps or openings similar to those which currently exist will always be maintained for trafficability requirements.

Any proposed future development is not exempt from flood hazard and structural assessment as outlined in Section 3.1

#### 3.3 Future Development on the Northern Floodplain

This study has identified that there is no need for hydraulic modelling of the proposed, or any future proposed development within the existing intensively built-up area of the Manildra Starches Plant (shown on Figures 2 and 4). However during the course of this investigation, and in consultation with Council and the DLWC, it should be noted that any further development upon the northerm floodplain (outside the built-up area shown on Figures 2 and 4), and in particular any development adjacent to the river bank, has the potential to increase the cumulative impact on flood levels and velocities.

The main areas of concern on the northern river bank are the unrestricted low lying areas between high ground and the existing developments, termed flowpaths or floodways. The bank is relatively

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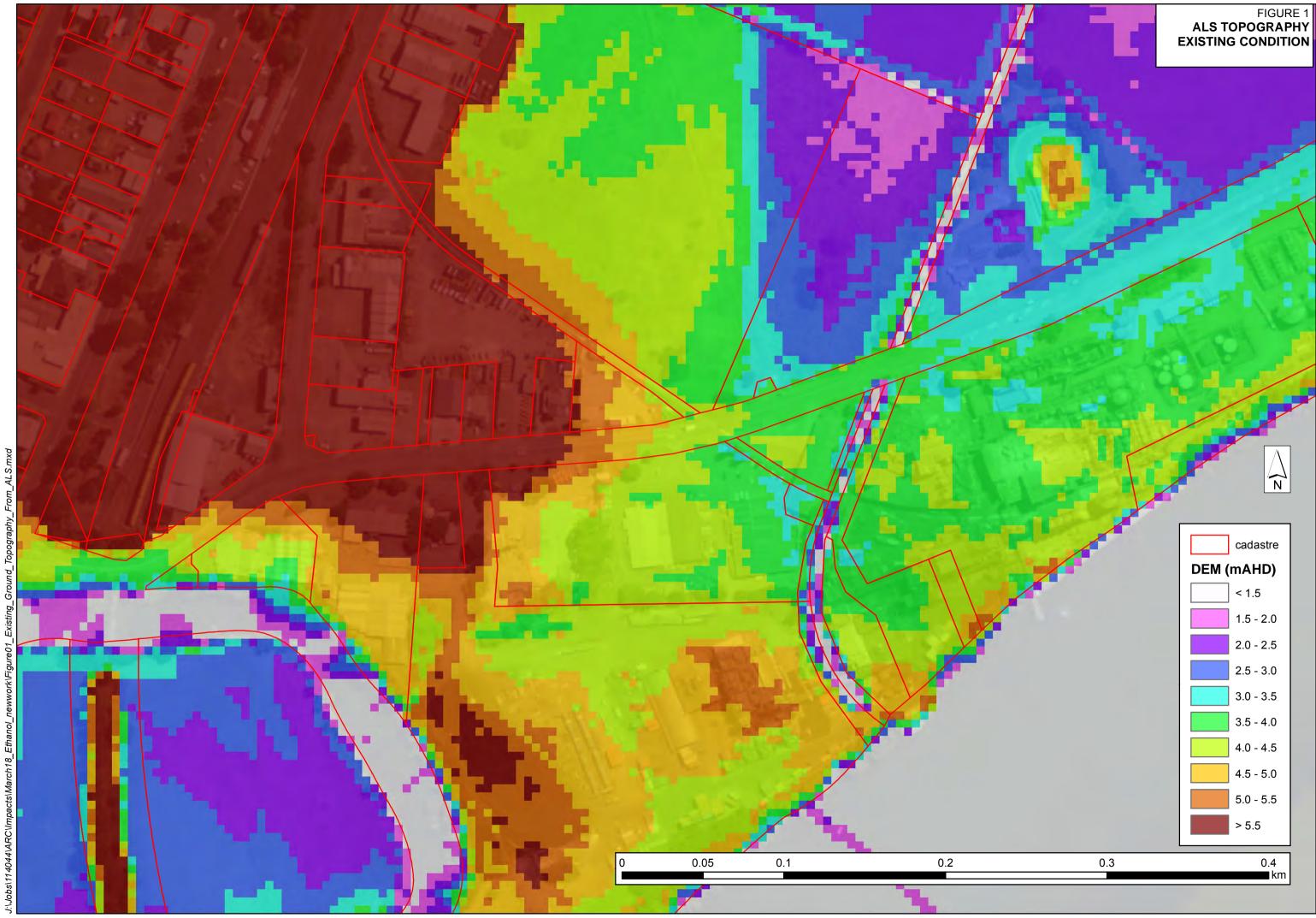
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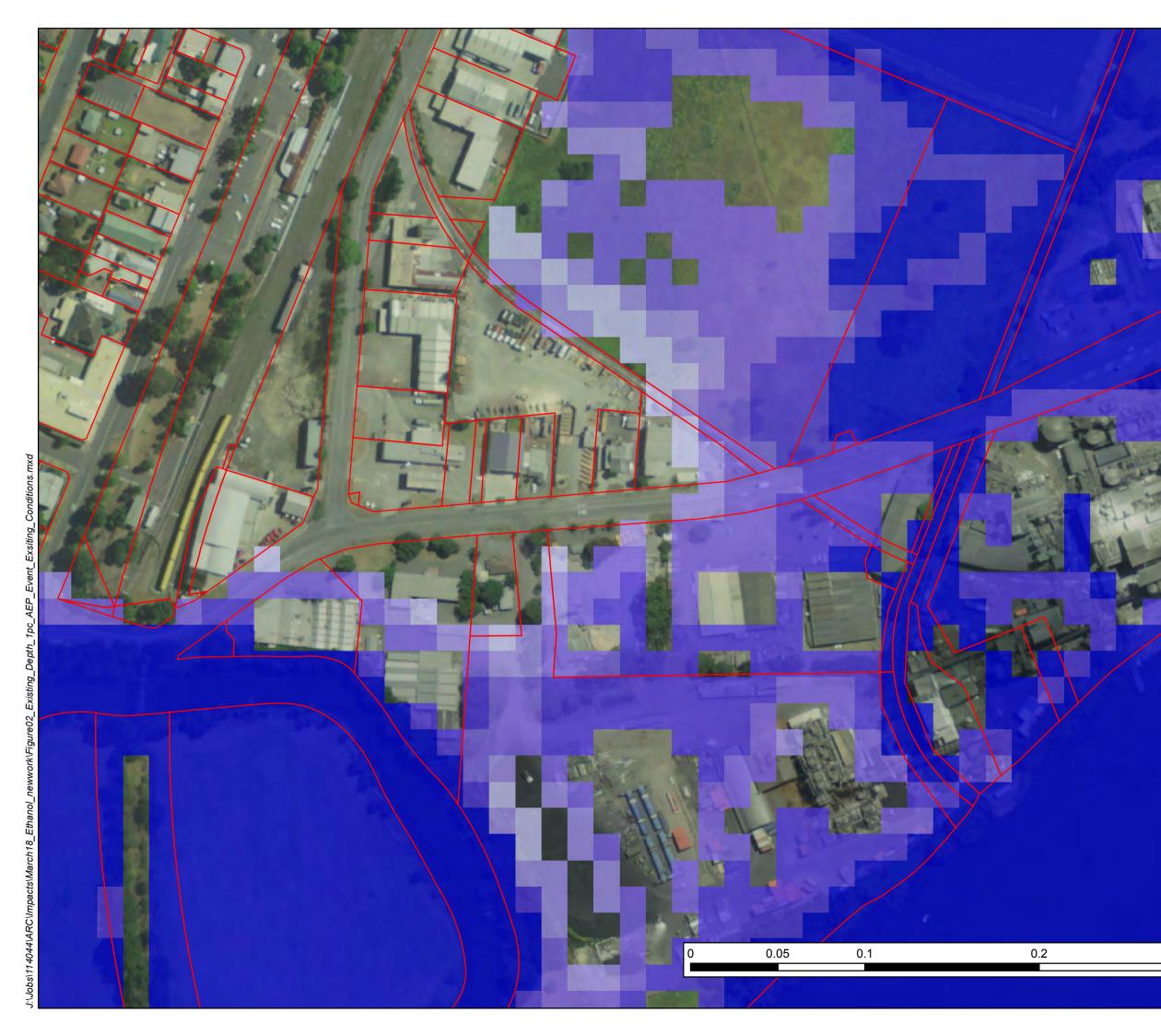
Figure 1: Existing Ground Topography from ALS

- Figure 2: Peak Flood Depth 1% AEP Event Existing Conditions
- Figure 3: Peak Flood Level Impact 1% AEP Proposed Scenario
- Figure 4: Peak Flood Level Impact 0.5% AEP Proposed Scenario
- Figure 5: Peak Flood Level Impact Extreme Event Proposed Scenario



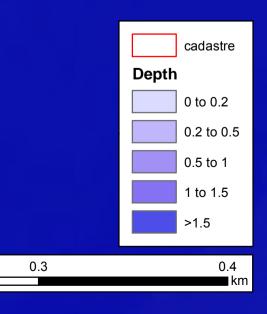






#### FIGURE 2 PEAK FLOOD DEPTH 1% AEP EVENT EXISTING CONDITION





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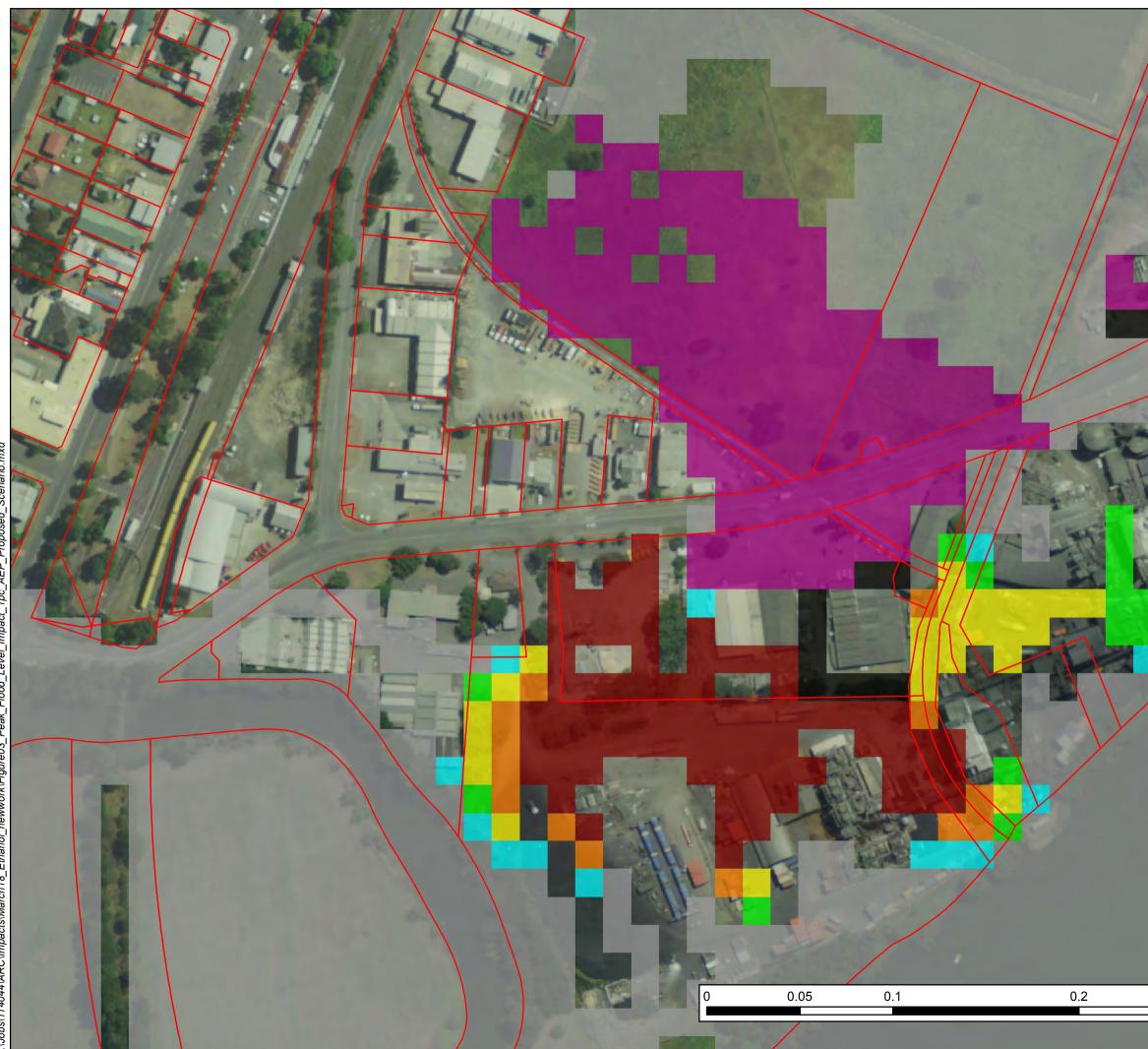


FIGURE 3 PEAK FLOOD LEVEL IMPACT 1% AEP EVENT PROPOSED DEVELOPMENT



	cadastreMGA	
Impact (m)		
	-0.12 to -0.05	
	-0.05 to -0.01	
	-0.01 to 0.01	
	0.01 to 0.02	
	0.02 to 0.03	
	0.03 to 0.05	
	0.05 to 0.07	
	0.07 to 0.09	
	No Longer Flooded	
	Newly Flooded	

0.3

0.4

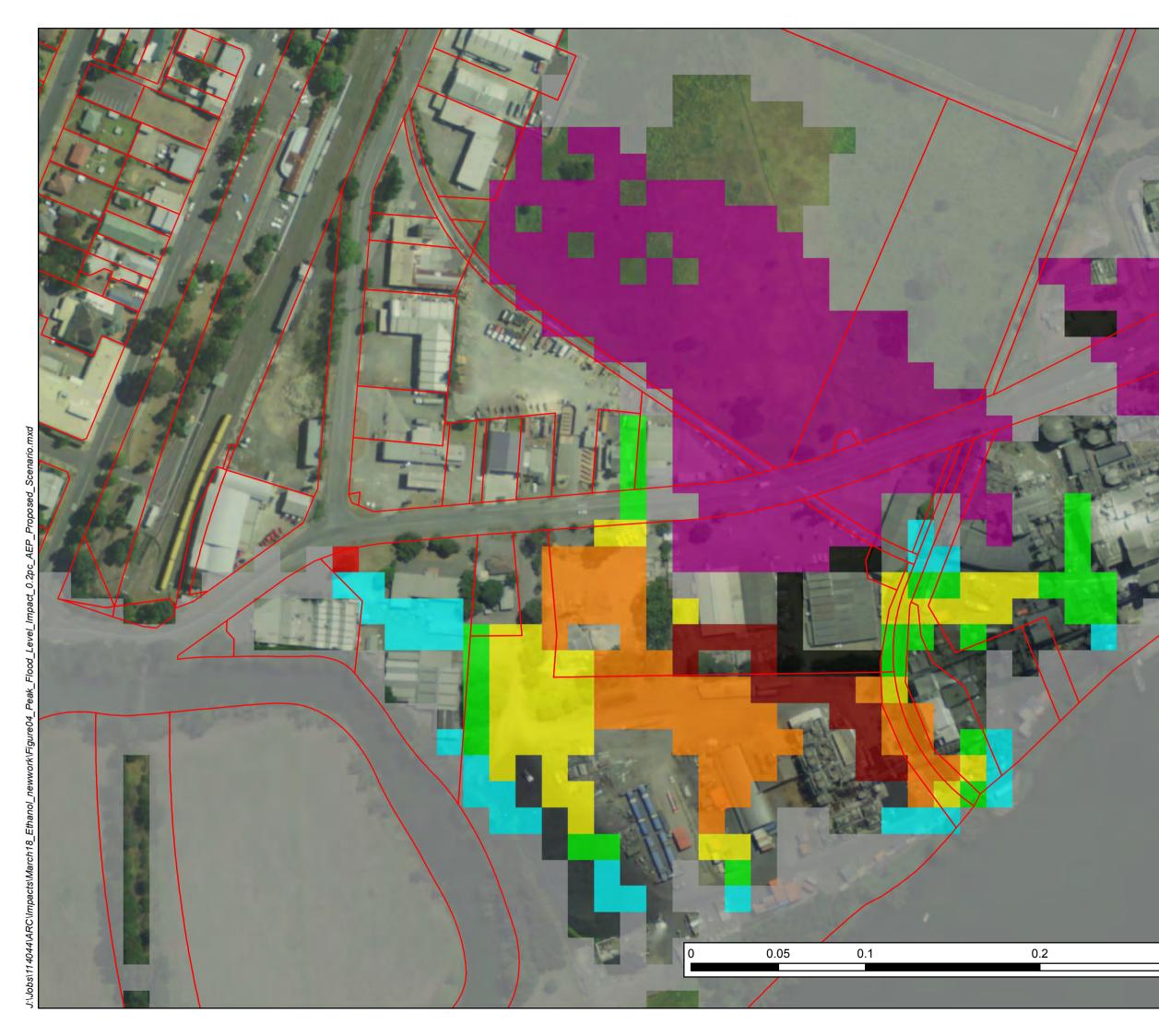


FIGURE 4 PEAK FLOOD LEVEL IMPACT 0.5% AEP EVENT PROPOSED DEVELOPMENT



	cadastreMGA	
Impact (m)		
	<-0.05	
	-0.05 to -0.01	
	-0.01 to 0.01	
	0.01 to 0.02	
	0.02 to 0.03	
	0.03 to 0.05	
	0.05 to 0.07	
	>0.07	
	No Longer Flooded	
	Newly Flooded	
	0.4	

0.3

0.4 ∎ km



#### FIGURE 5 PEAK FLOOD LEVEL IMPACT EXTREME EVENT PROPOSED DEVELOPMENT



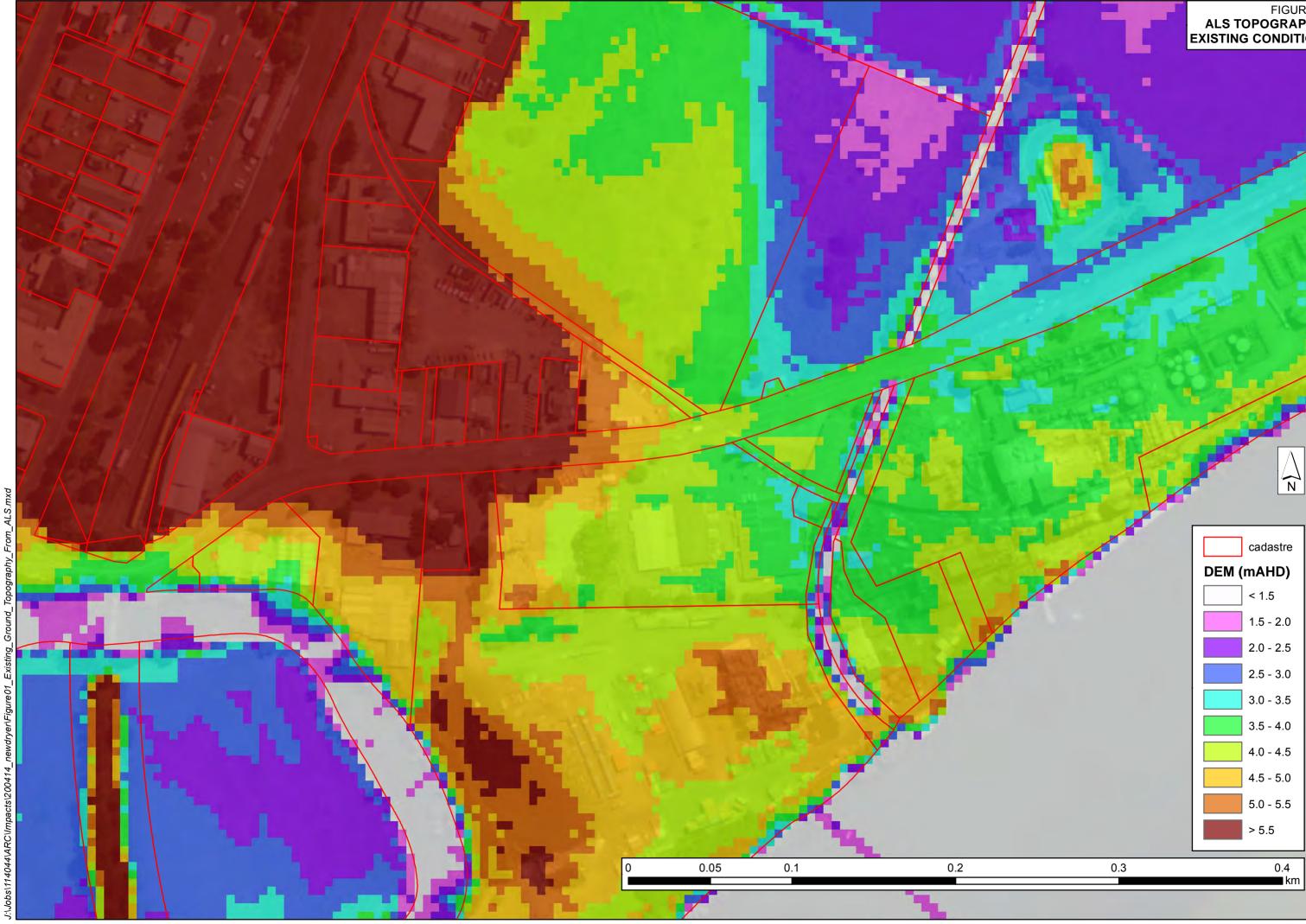
	cadastreMGA	
Impact (m)		
	< -0.05	
	-0.05 to -0.01	
	-0.01 to 0.01	
	0.01 to 0.02	
	0.02 to 0.03	
	0.03 to 0.05	
	0.05 to 0.07	
	>0.07	
	No Longer Flooded	
	Newly Flooded	
	0.4	

0.3

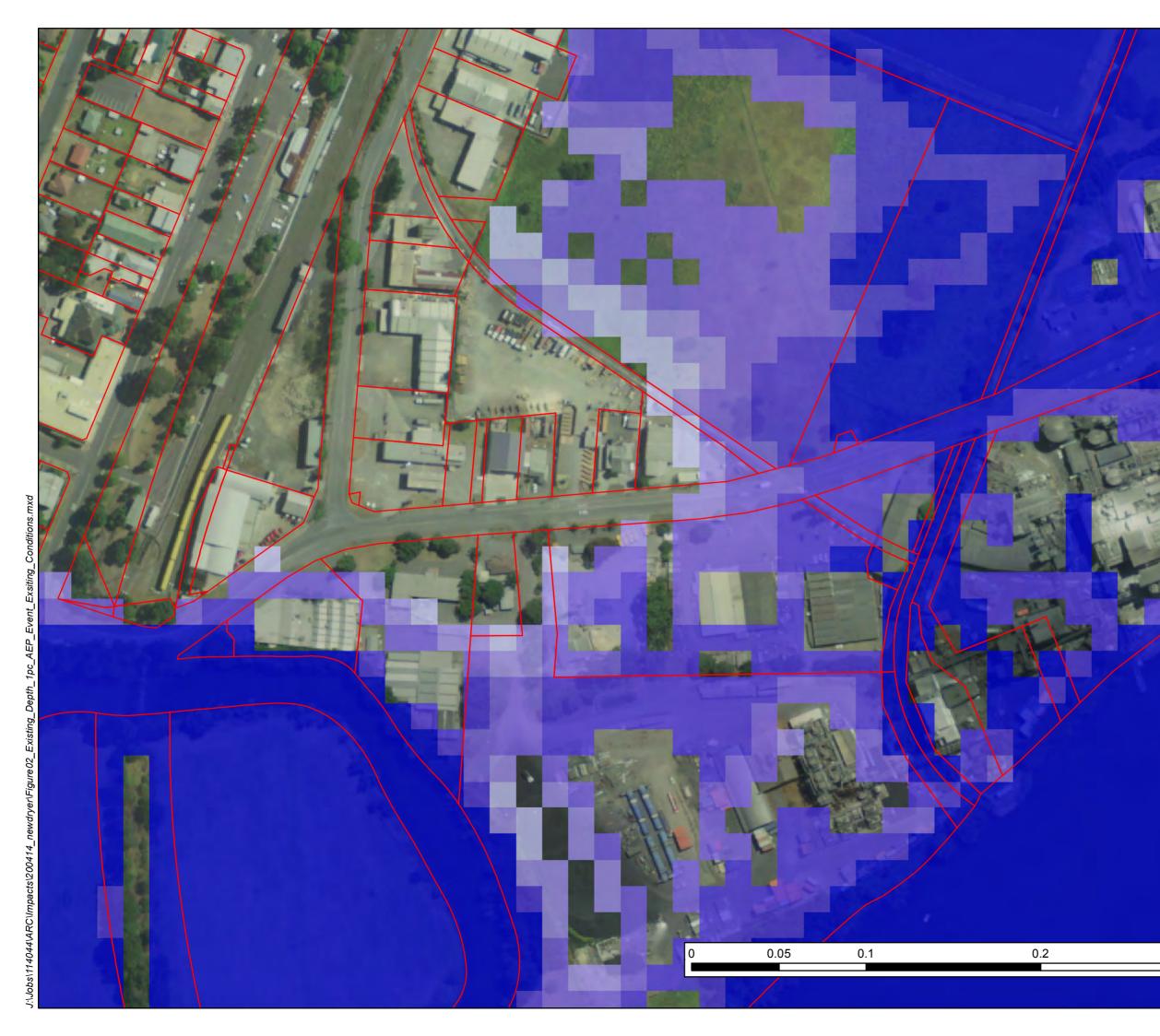
0.4 ■ km







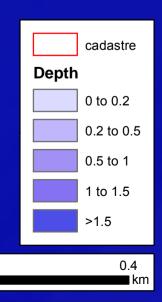
# FIGURE 1 ALS TOPOGRAPHY EXISTING CONDITION



#### FIGURE 2 PEAK FLOOD DEPTH 1% AEP EVENT EXISTING CONDITION

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0.3

