

## **10. CLOSURE AND REHABILITATION**

### **10.1 EMBANKMENT PROFILE**

At the end of operation of the facility, the downstream faces of the external embankments will have a slope of 1V:3H, together with 5 m wide benches located at 10 m height intervals, yielding an overall slope profile of 1V:2.75H.

The adopted downstream profile will be inherently stable under both normal and seismic loading conditions, will provide a stable drainage system, and will allow for re-vegetation. The embankment should be rehabilitated soon after de-commissioning.

### **10.2 TAILINGS PROFILE**

During the operation, deposition will occur from the embankment and the low point on the tailings surface will occur approximately at the decant tower. During the final stage of operation deposition will also be carried out along the northern end of the embankment in order to deposit towards the location of the final spillway. Rehabilitation of the tailings surface will commence upon termination of deposition into the facility.

### **10.3 REHABILITATION**

The geochemical studies have concluded that the tailings are non-acid forming. In addition, the concentrations of enriched elements in the tailings are below ecological or health based levels. Regardless, the control of seepage through the facility by means of appropriate closure design will be one of the objectives of the rehabilitation process.

In addition to this objective, other aspects of the rehabilitation programme will be re-vegetation, erosion control and stormwater management. Establishing a surface cover of verdant vegetation will reduce the potential for adverse environmental impact such as dust generation and rainfall erosion, as well as improving aesthetics. To this end rehabilitation trials will be undertaken during the life of the storage to determine the most efficient method to provide an effective cap and rehabilitate the surface of the facility. The results of these trials will be used to design the most suitable cover which will be placed over the tailings surface.

After removal of the pond around the decant, drying of the tailings to a sufficient degree to facilitate trafficking is expected to take number of months. Prior to commencing the earthworks component of the rehabilitation, the tailings delivery and distribution pipework will be removed from the facility, and the top sections of the decant towers

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will be removed so that the towers are below the access causeway level. Finally, the decant towers will be backfilled and sealed.

It is anticipated that a low permeability layer will be required to minimise rainfall infiltration into the tailings. In addition, a layer of mine waste approximately 0.3 m to 0.5 m thick will be placed to act as a capillary break, and this will be followed by a layer of waste and topsoil, also 0.5 m thick, to act as a growth medium. The finished surface will be shallow ripped and seeded with shrubs and grasses.

The underdrainage system will need to continue to operate for some time after completion of capping and re-vegetation to drain excess water from the tailings deposit. The quantity of water recovered from the underdrainage system will reduce with time and experience with similar facilities suggests that water recovery may continue for a period of up to a year following closure. During this time, water from the underdrainage system will be pumped to the process ponds. After the flow ceases, the underdrainage pump will be removed and the underdrainage riser backfilled and sealed as part of the rehabilitation process.

The final profile of the tailings surface will be re-shaped to slope gently from the embankment towards the final spillway and the low spot on the tailings surface will be adjacent to the spillway. The closure spillway will be constructed along the southern perimeter of the facility to allow rainfall runoff from the surface of the rehabilitated facility to flow into the surrounding natural drainage system.

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## 11. CONSTRUCTION QUANTITIES

On the basis of the design presented, operating parameters supplied and/or assumed, construction quantities for the life of the facility has been determined. The quantities given are deemed accurate enough for an overall accuracy of  $\pm 10\%$  -  $15\%$ .

Life of operation quantities for construction of the tailings storage facility are presented in Table 11.1. The following assumptions were made:

- Zone A will be obtained from the TSF basin.
- Zone C will be obtained from mine waste (box cut) for Stage 1 and from mine waste or local borrow for Stages 2 and 3.
- Zone D will be obtained from the TSF basin or from mine waste (box cut).
- Zone E will be selected material from mine waste (box cut).
- Zone F will be imported from a local borrow source.
- Zone G will be selected material from mine waste (box cut).

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Table 11.1: Tailings Storage Facility, Bill of Quantities – Final Design

ITEM	Description	Unit	Stage 1	Stage 2	Final	Total
			Qty	Qty	Qty	Qty
<b>1</b>	<b>Preliminary and General</b>					
1.1	Mobilise to site (nominal 10%)	item	1	1	1	3
1.2	De-mobilise from site (nominal 10%)	item	1	1	1	3
1.3	Fixed costs while established on site (nominal 10%)	weeks	12	6	6	24
<b>1</b>	<b>TOTAL FOR PRELIMINARY AND GENERAL</b>					
<b>2</b>	<b>Foundation Preparation</b>					
2.1	Clear and grub embankment footprint	sq m	16,800	3,700	3,100	23,600
2.2	Strip topsoil (300 mm nominal) from embankment foundation and stockpile	sq m	16,800	3,700	3,100	23,600
2.3	Excavate embankment cut-off trench, haul, place and spread excavated material in Zone C	ccm	4,000	3,200	2,800	10,000
2.4	Scarify, moisture condition and compact embankment foundation (includes cut-off trench plus 5 m)	sq m	16,800	3,700	3,100	23,600
2.5	Win from borrow area, spread, moisture condition and compact Zone A material in cut-off trench	ccm	4,000	3,200	2,800	10,000
<b>2</b>	<b>TOTAL FOR FOUNDATION PREPARATION</b>					
<b>3</b>	<b>General Embankment Earthworks</b>					
3.1	Win from borrow, load, haul, spread, moisture condition and compact Zone A material in embankment zones	ccm	7,000	7,500	7,500	22,000
3.2	Win from mine waste, load, haul, spread, moisture condition and compact Zone C material in embankment	ccm	100,000	-	-	100,000
3.3	Win from borrow, load, haul, spread, moisture condition and compact Zone C material in embankment	ccm	-	14,800	35,000	49,800
3.4	Supply and install 160 mm dia class 400 slotted CPT draincoil, with tees, bends and joints inclusive for toe drain	m	190	150	130	470
3.5	Win from local borrow, haul and place Zone F sand material to upstream toe drain	ccm	70	50	50	170
3.7	Supply and install geotextile (Bidim A14 or equivalent) to upstream toe drain	sq m	570	450	390	1,410
3.8	Supply and install 1.5 mm (60 mil) smooth HDPE geomembrane to embankment upstream face	sq m	4,100	-	-	4,100
3.9	Win from local borrow, load, haul, place and compact wearing course onto embankment crest (150mm thick)	ccm	200	400	500	1,100
3.10	Win from local borrow, load, haul, and place Zone D fill as crest safety berm	ccm	100	200	300	600
<b>3</b>	<b>TOTAL FOR GENERAL EMBANKMENT EARTHWORKS</b>					
<b>4</b>	<b>Spillway</b>					
4.1	Clear vegetation from spillway area	Ha	0.15	-	-	0.15
4.2	Grub spillway area	Ha	0.15	-	-	0.15
4.3	Strip topsoil (300 mm) from spillway foundation, load and haul to stockpile	ccm	300.0	-	-	300.0
4.4	Excavate emergency spillway, load, haul and place excavated material to embankment Zone C	ccm	500	500	500	1,500
<b>4</b>	<b>TOTAL FOR SPILLWAY</b>					
<b>5</b>	<b>Basin Construction</b>					
5.1	Clear and grub basin area	sq m	36,100	16,900	18,900	71,900
5.2	Re-shape basin area along valley spine (nominal allowance)	ccm	14,800	35,000	-	49,800
5.3	Strip topsoil (160 mm nominal) and stockpile	sq m	36,100	16,900	18,900	71,900
5.4	Scarify, moisture condition and compact basin in situ materials to form basin liner	sq m	36,100	16,900	18,900	71,900
5.5	Prepare basin subgrade for HDPE lined area	sq m	25,100	5,400	5,900	36,400
5.6	Supply and install 1.5 mm HDPE liner to basin underdrainage area	sq m	25,100	5,400	5,900	36,400
<b>5</b>	<b>TOTAL FOR BASIN CONSTRUCTION</b>					
<b>6</b>	<b>Underdrainage System</b>					
6.1	Proof roll basin area	Ha	3.6	1.7	1.9	7.2
6.2	Excavate main collector drain to Zone C or designated local stockpile	ccm	-	-	-	-
6.3	Excavate surface drain for branch drains to Zone C or designated local stockpile	ccm	-	-	-	-
6.4	Win from borrow import, load, haul, place and spread Zone F drainage sand for main collector drain	ccm	800	200	-	800
6.5	Win from borrow import, load, haul, place and spread Zone F drainage sand for branch drains	ccm	70	10	-	80
6.6	Win from borrow import, load, haul, place and spread Zone F drainage sand for finger drains	ccm	1,600	540	270	2,410
6.7	Win from local borrow, haul and place Zone E erosion protection layer (150 mm thick) to main collector drain	ccm	390	100	-	490
6.9	Win from borrow, haul and place Zone E erosion protection layer to branch underdrains	ccm	-	-	-	-
6.10	Supply and install 1.5 mm (60 mil) textured HDPE geomembrane to decant tower	sq m	600	600	-	1,200
6.11	Supply and install geotextile (Bidim A14 or equivalent) to main collector drain	sq m	4,500	1,200	-	5,700
6.12	Supply and install geotextile (Bidim A14 or equivalent) to branch drains	sq m	900	100	-	1,000
6.13	Supply and install geotextile (Bidim A14 or equivalent) to finger drains	sq m	3,200	1,100	600	4,900
6.14	Supply and install 160 mm class 400 CPT draincoil to main collector drain, with filter sock, tees, bends and joints (includes connector pipes)	m	1,280	320	-	1,600
6.15	Supply and install 100 mm class 400 CPT draincoil to branch drains, with tees, bends and joints	m	330	40	-	370
6.16	Supply and install 100 mm class 400 CPT draincoil to finger drains, with tees, bends and joints	m	1,600	540	270	2,410
<b>6</b>	<b>TOTAL FOR UNDERDRAINAGE SYSTEM</b>					
<b>7</b>	<b>Underdrainage Collection Sump</b>					
7.1	Excavate underdrainage collection sump, place and spread to Zone C or designated spoil stockpile	ccm	360	-	-	360
7.2	Win from borrow import, load, haul, place and compact Zone A soil liner for underdrainage sump	ccm	90	-	-	90
7.3	Win from borrow import, load, haul and place clean gravel backfill to underdrainage sump	ccm	260	-	-	260
7.4	Supply and install geotextile (Bidim A24 or equivalent) to underdrainage sump	sq m	580	-	-	580
7.5	Supply and install 450 mm dia HDPE (SDR 11) riser pipe (slotted) including all fittings	m	7	-	-	7
7.6	Supply and install 450 mm dia HDPE (SDR 11) riser pipe (un-slotted) including all fittings	m	65	-	-	65
7.7	Supply and install solid end cap to riser pipe	No	1	-	-	1
7.8	Excavate riser pipe channel	ccm	10	-	-	10
7.9	Supply and install 1.5 mm (60 mil) smooth HDPE wear-sheets for underdrainage sump and riser pipe	sq m	780	-	-	780
7.10	Supply and install conveyor belt to support riser pipe	m	3	-	-	3
7.11	Supply and install cement stabiliser backfill to underdrainage riser excavation					
7.12	Install HDPE pipe boot (includes two steel band clamps)	No	1	-	-	1
<b>7</b>	<b>TOTAL FOR UNDERDRAINAGE SYSTEM</b>					
<b>8</b>	<b>Decant System</b>					
8.1	Spread, moisture condition and compact Zone A protective layer as base of decant causeway	ccm	180	360	0	540
8.2	Spread, moisture condition and compact Zone D decant access causeway fill	ccm	2,000	4,000	1,500	7,500
8.3	Win from stockpile, load, haul and place Zone G selected clean rockfill decant surround	ccm	550	1,100	700	2,350
8.4	Supply and install 25 MPa concrete base including formwork and reinforcement	ccm	6	6	0	12
8.5	Supply and install 1800 mm diameter slotted reinforced concrete decant tower	m	5	8	4	17
8.6	Supply and install geotextile (Bidim A24 or equivalent) to decant access causeway Zone D / rockfill transition	sq m	100	200	100	400
8.7	Win from stockpile, load, haul, place and compact wearing course to decant access causeway (150 mm thick)	ccm	30	30	30	90
8.8	Win from borrow, load, haul and place Zone D material as safety berm on decant access causeway	ccm	20	20	20	60
<b>8</b>	<b>TOTAL FOR DECANT SYSTEM</b>					
<b>9</b>	<b>Miscellaneous</b>					
9.1	Install and survey settlement pins	No.	10	10	5	25
9.2	Install piezometers	No.	5	5	5	15
9.3	Construct complete downstream monitoring bore stations	No.	5	-	-	5
9.4	Excavate diversion channel, load haul and place excavated soil to embankment Zone C	ccm	2,500	-	-	2,500
<b>9</b>	<b>TOTAL FOR MISCELLANEOUS</b>					

## 12. RISKS AND OPPORTUNITIES

### 12.1 GENERAL

The nature of the design process is such that the tailings storage facility design is based upon a series of design parameters and assumptions which may or may not be interconnected. In general terms the facility is designed for average operating conditions, with some allowance for extreme occurrences, as determined by design and government agency guidelines. The implications of variation of design assumptions versus operational experience are potential cost savings or cost increase, depending upon the actual circumstances. As such, the approach to the final design of the tailings storage facility has been to build in flexibility to the proposed construction and operation in order to allow for variation of these parameters during operation.

Monitoring and auditing of the facilities will be carried out on a regular basis in accordance with the guidelines presented in Section 8 in order to enable comparison of design assumptions with operational performance and to facilitate amendments to the operation and/or design of future stages of the facility construction.

Potential impacts to each facility arising from variation in design assumptions are discussed in the following sections.

### 12.2 TAILINGS STORAGE FACILITY

The design of the tailings storage facility is based upon the primary requirement to retain a defined mass of tailings material at a specific flow rate, and with particular physical and behavioural characteristics. The facility operates within the environment and climatic region of south-eastern New South Wales, and is required to meet defined operating standards in terms of seepage control and stability. The key operating functions and impacting design variables are as follows:

- Embankment raise timing and height – dependent on tailings throughput and achieved tailings density. Tailings density is dependent in part on climatic variables.
- Embankment stability – dependent on steady state phreatic surface and effectiveness of underdrainage and toe drains.
- Seepage rates – dependent on tailings consolidation and effectiveness of basin underdrainage.
- Supernatant recovery and volume of water pumped to process plant – dependent on tailings consolidation, rainfall runoff and evaporation.

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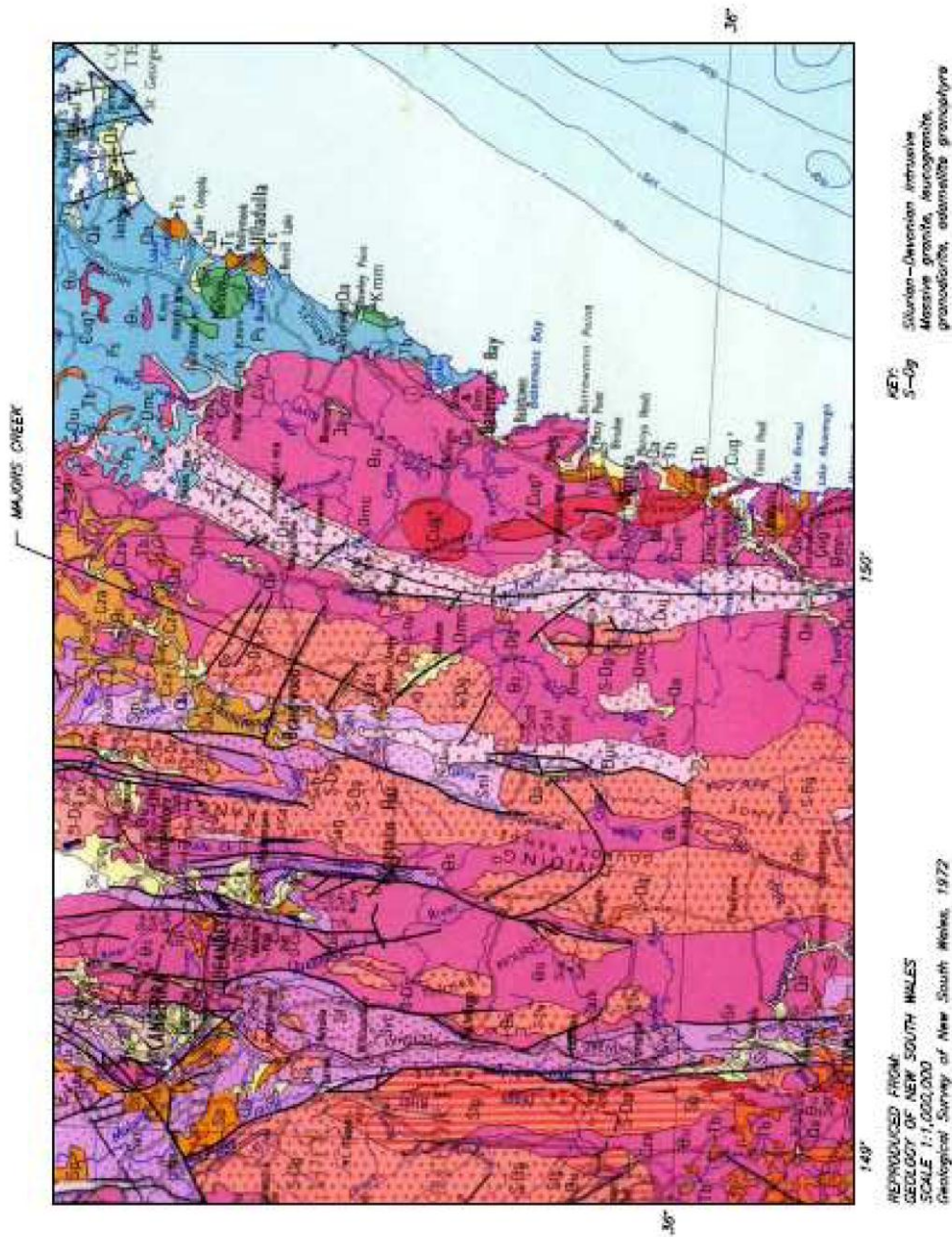


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FIGURES

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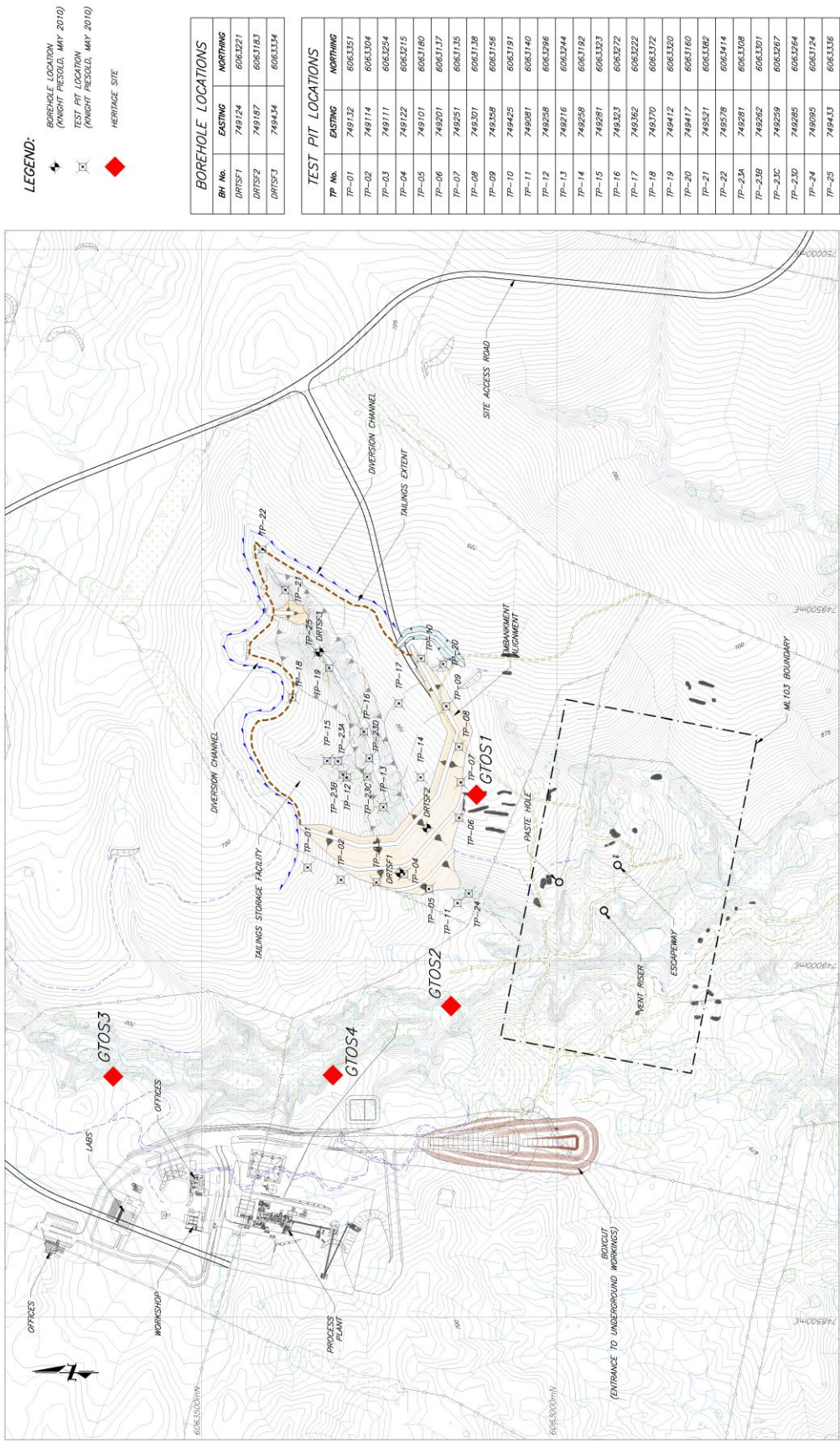


Ref: PE801-00139/05  
Figure 2.1

DARGUES REEF GOLD PROJECT  
FINAL DESIGN  
REGIONAL GEOLOGICAL MAP

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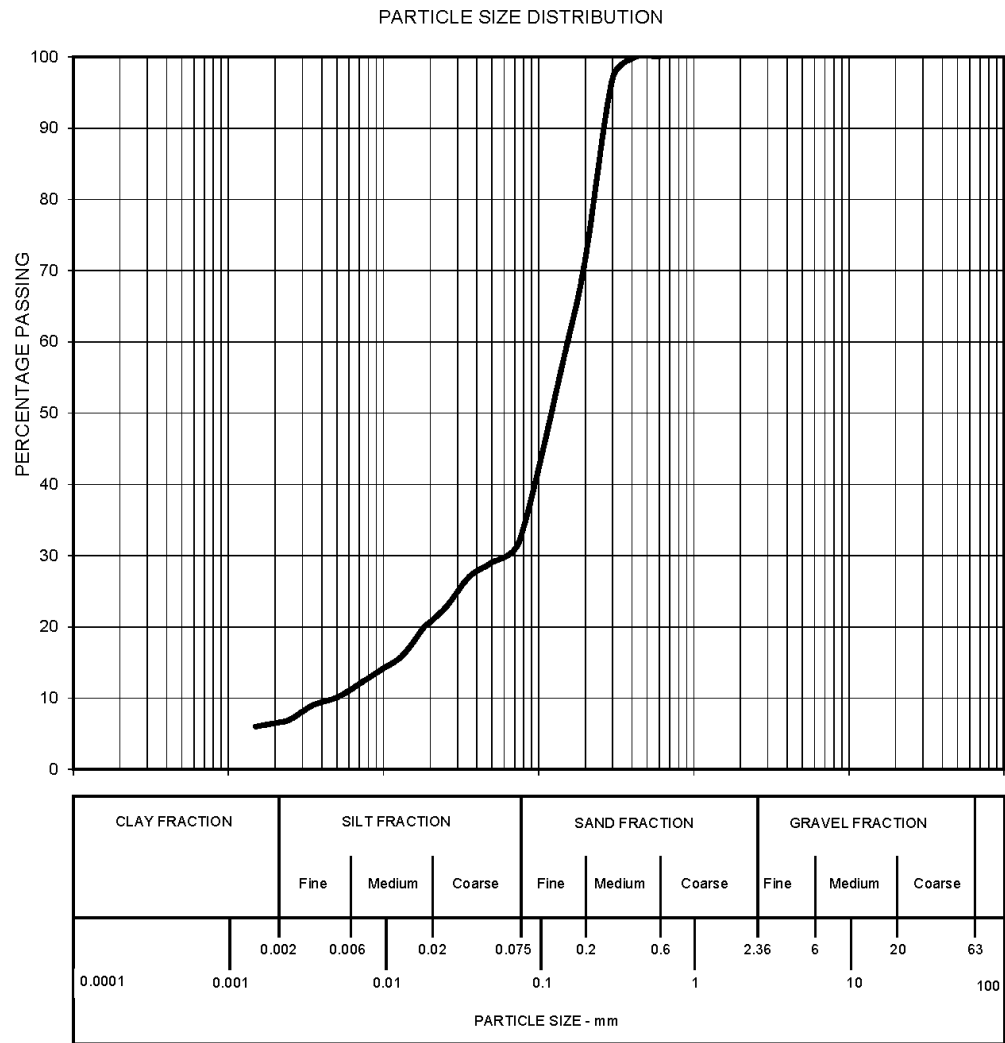


Ref: PE801-00139/05  
Figure 3.1  
Rev: 0

DARGUES REEF GOLD PROJECT  
FINAL DESIGN  
SITE INVESTIGATION LOCATIONS

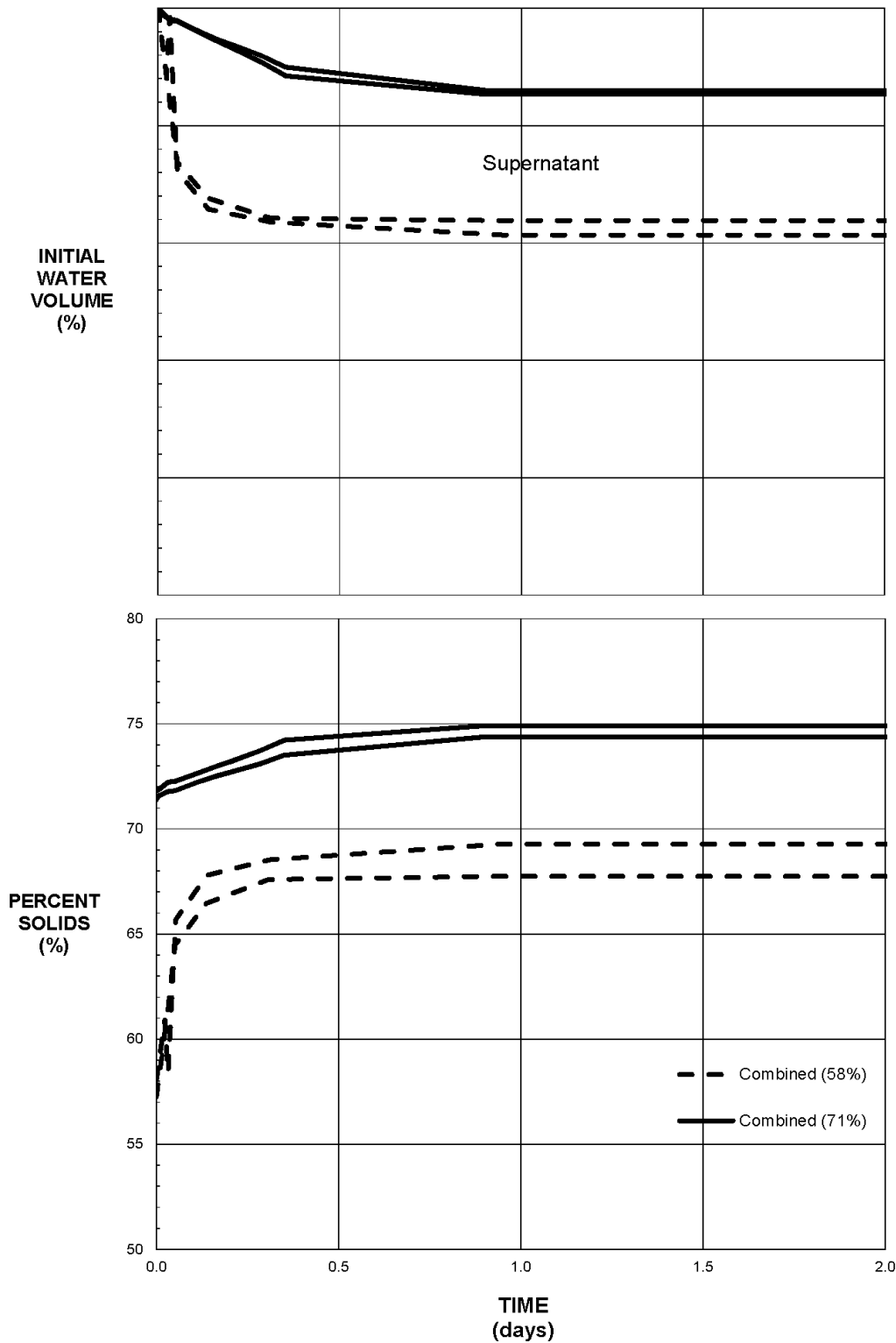
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PE801-00139 SUMMARY PLOTS.xlsm

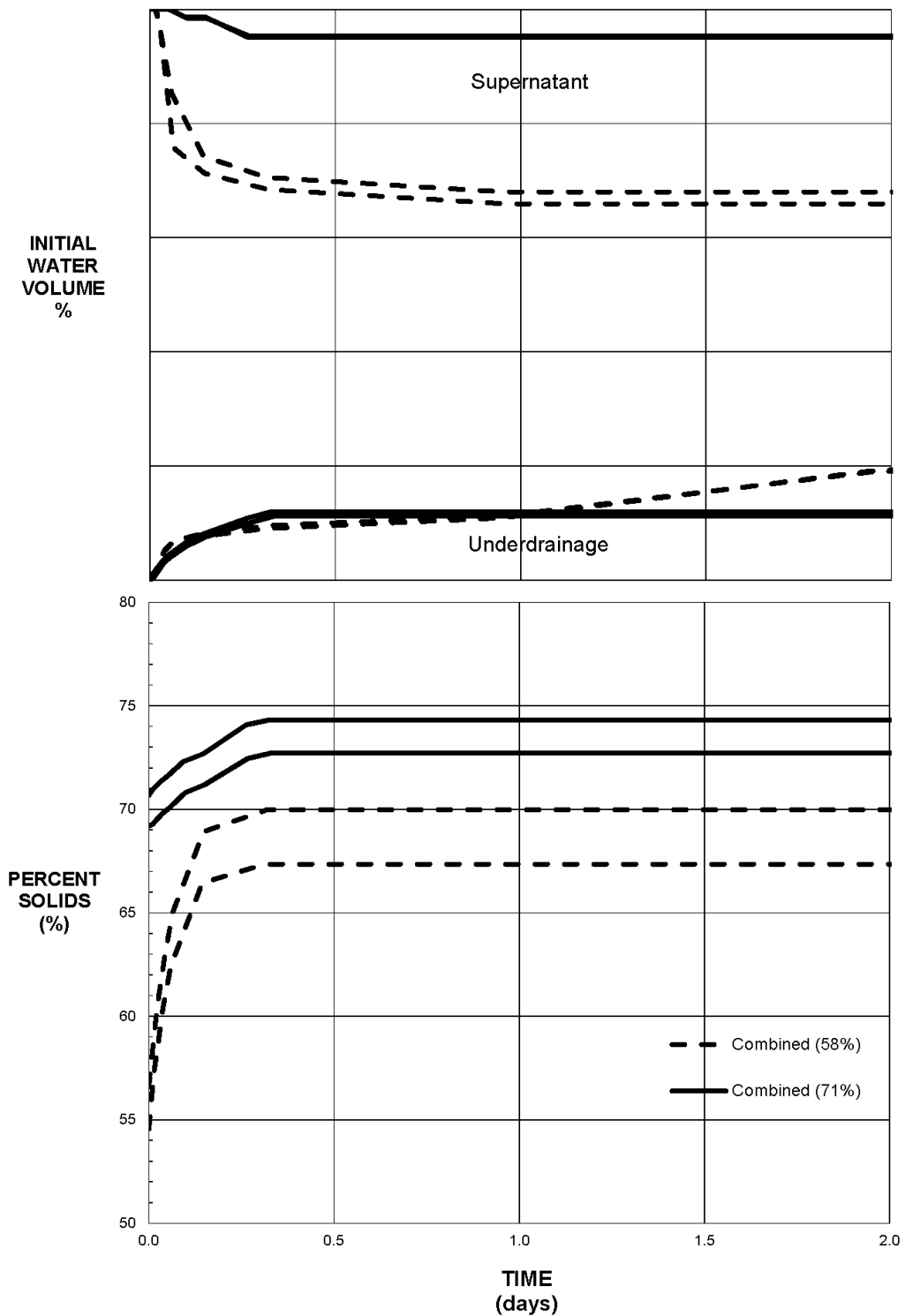


SYMBOL	SOIL DESCRIPTION	LIQUID LIMIT	PLASTICITY INDEX	SPECIFIC GRAVITY
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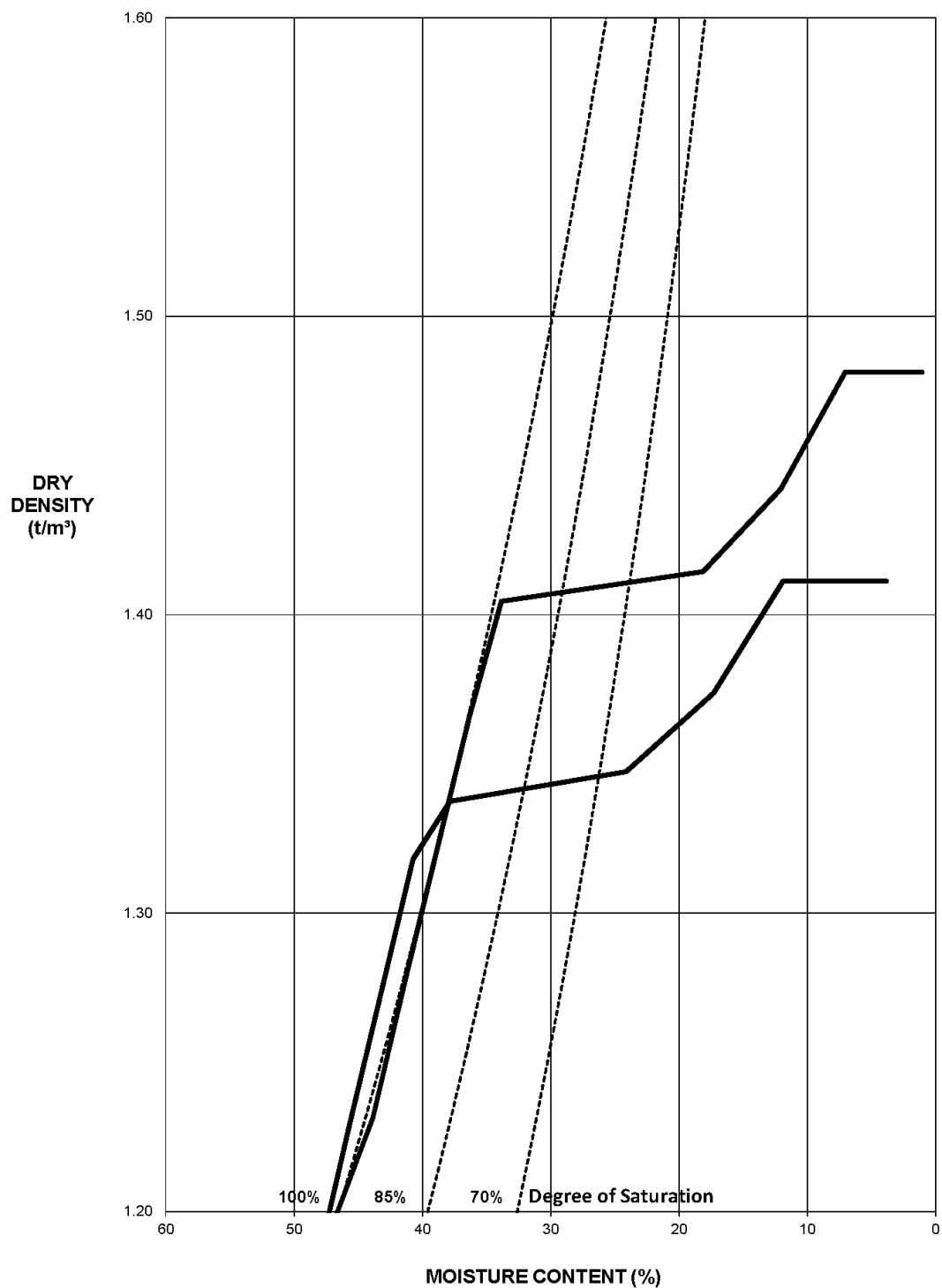
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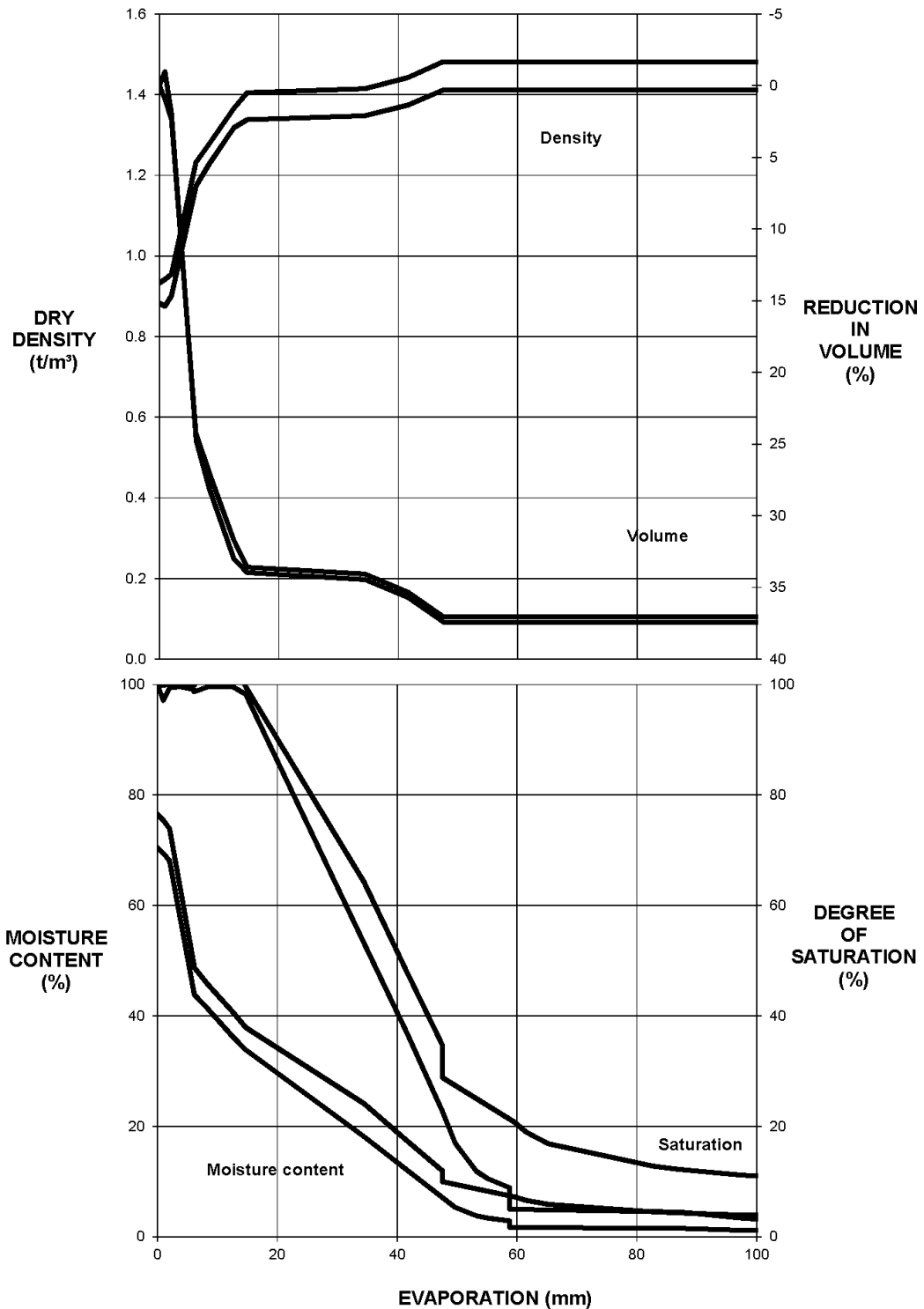


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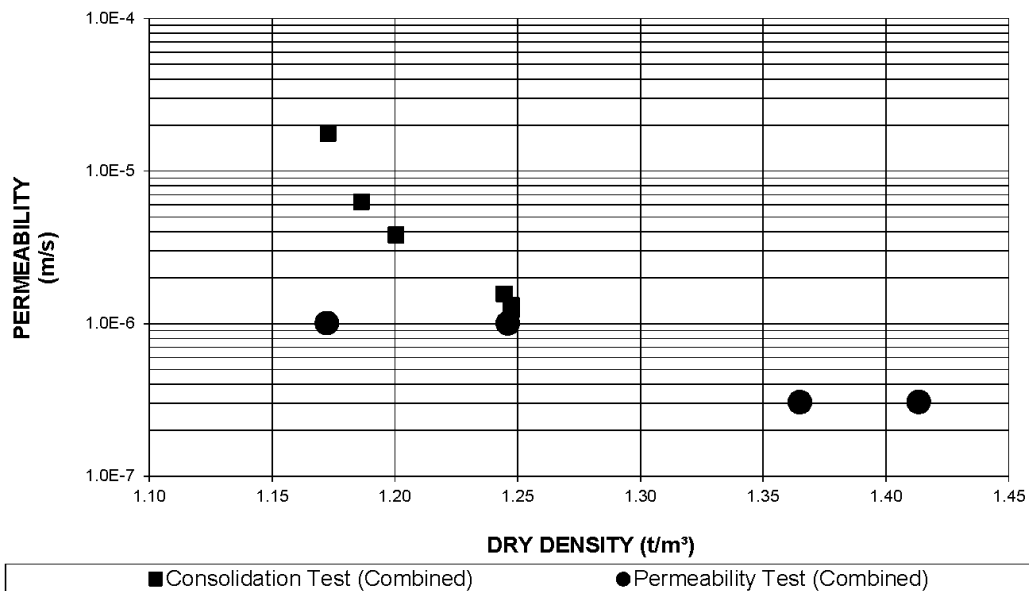
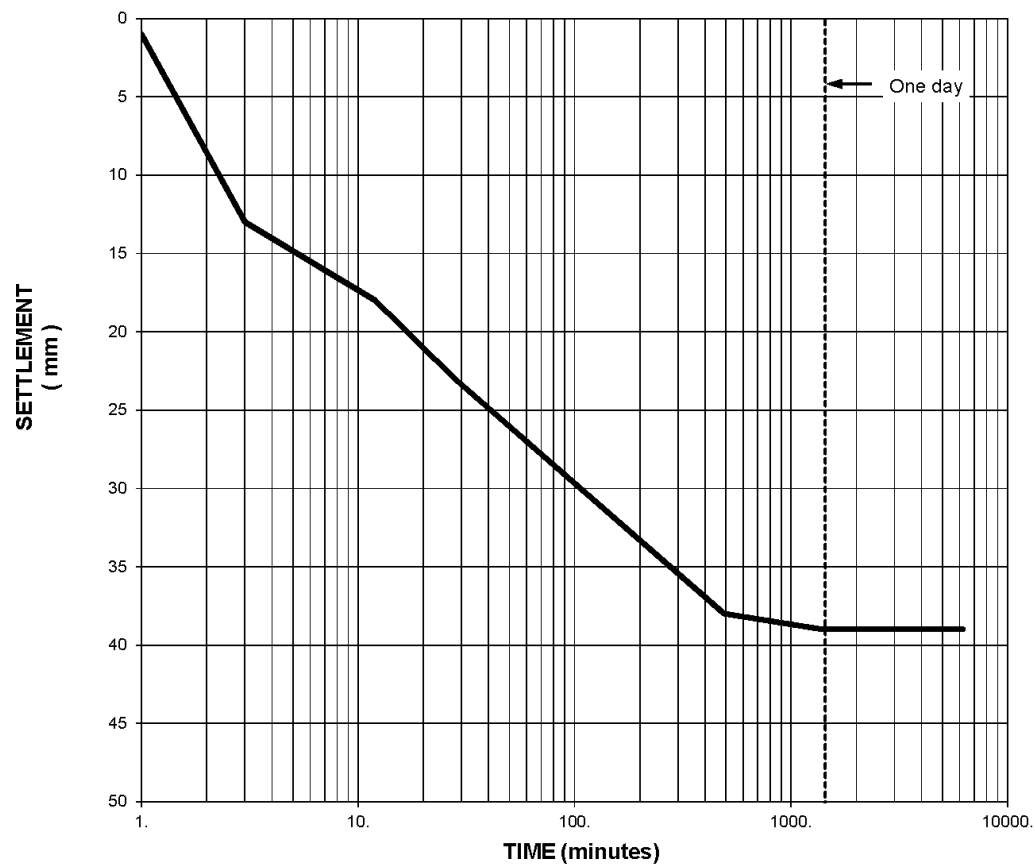




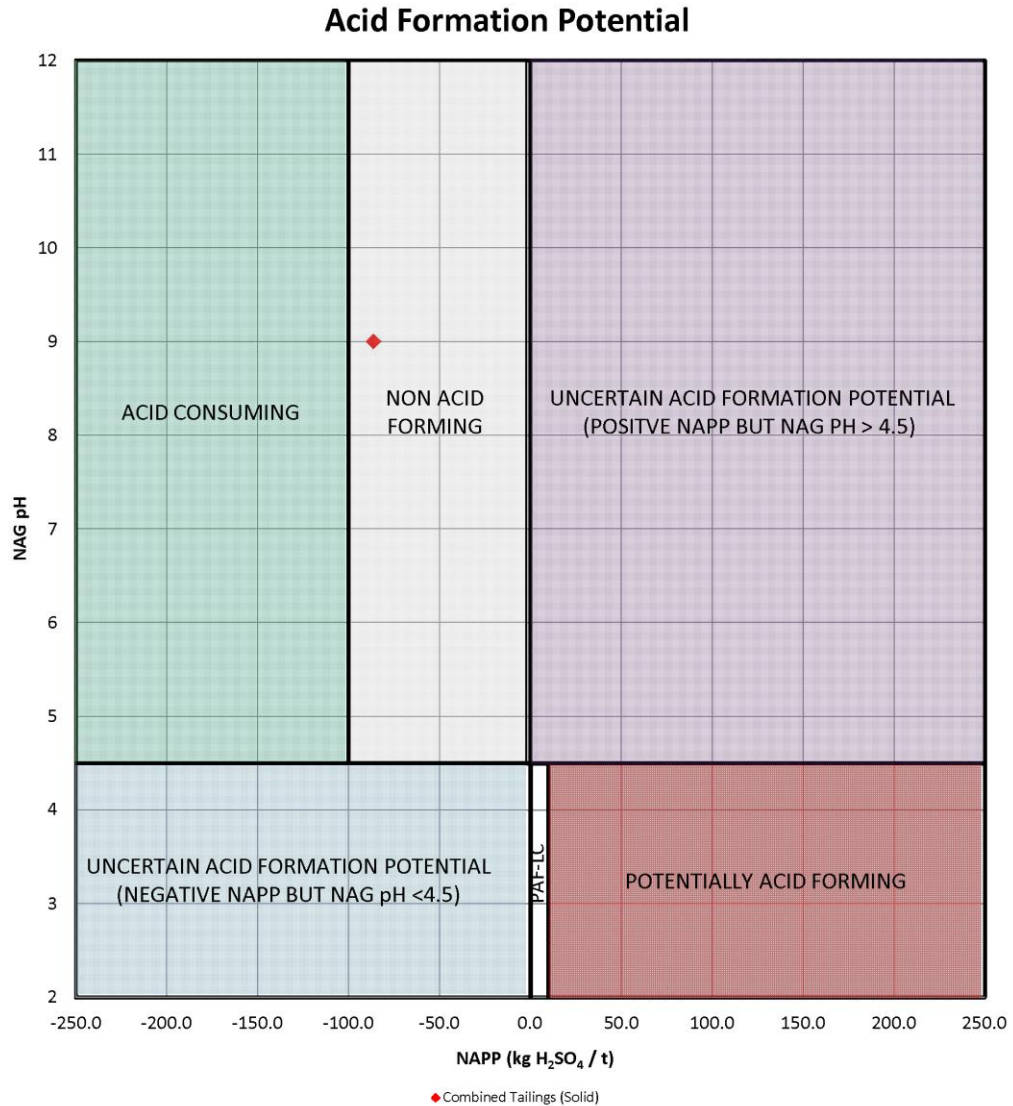
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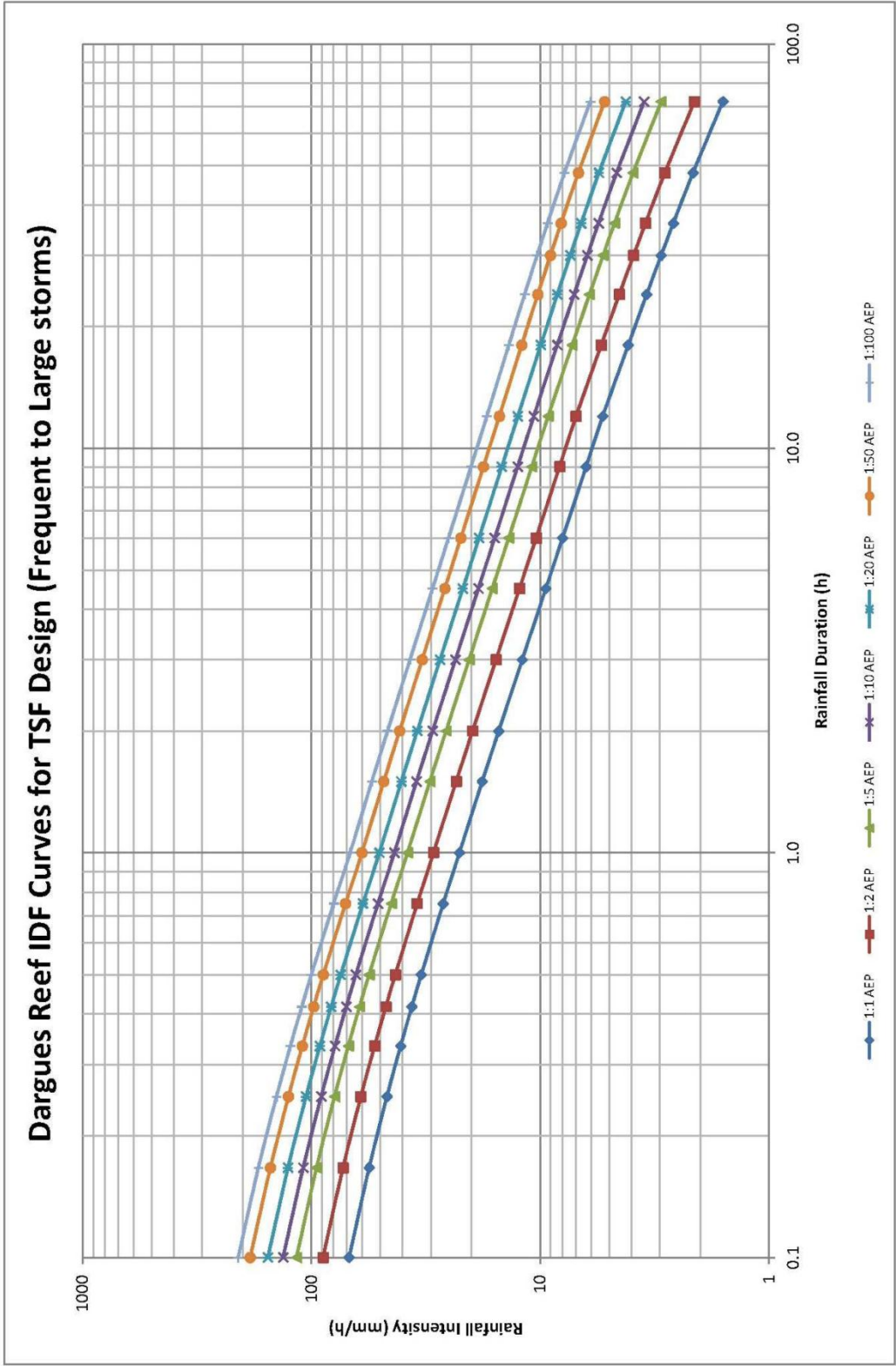
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PE801-00139 Tailings Solids Geochemistry Results - final Figure 4.7



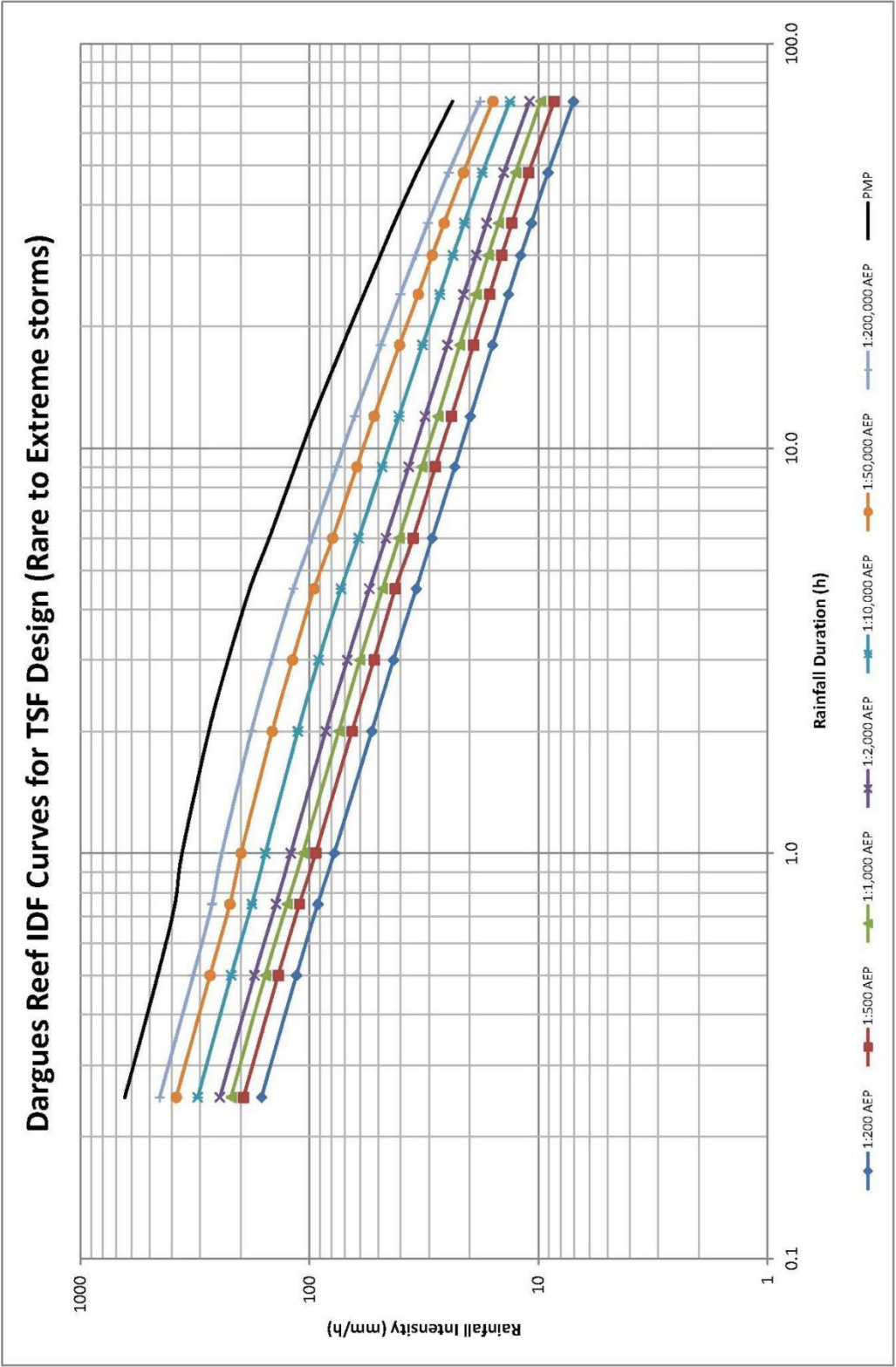
Acid Formation Potential	NAPP (kg H <sub>2</sub> SO <sub>4</sub> / t)	NAG pH
Potential Acid Forming (PAF)	>10	<4.5
Potential Acid Forming - Low Capacity (PAF - LC)	0 to 10	<4.5
Non Acid Forming (NAF)	Negative	≥4.5
Acid Consuming (AC)	Less than - 100	≥4.5
Uncertain	Positive	≥4.5
	Negative	<4.5



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Figure 5.1

DARGUES REEF GOLD PROJECT  
RAINFALL INTENSITY DURATION FREQUENCY DATA  
EQUivalent TO LARGE STORMS

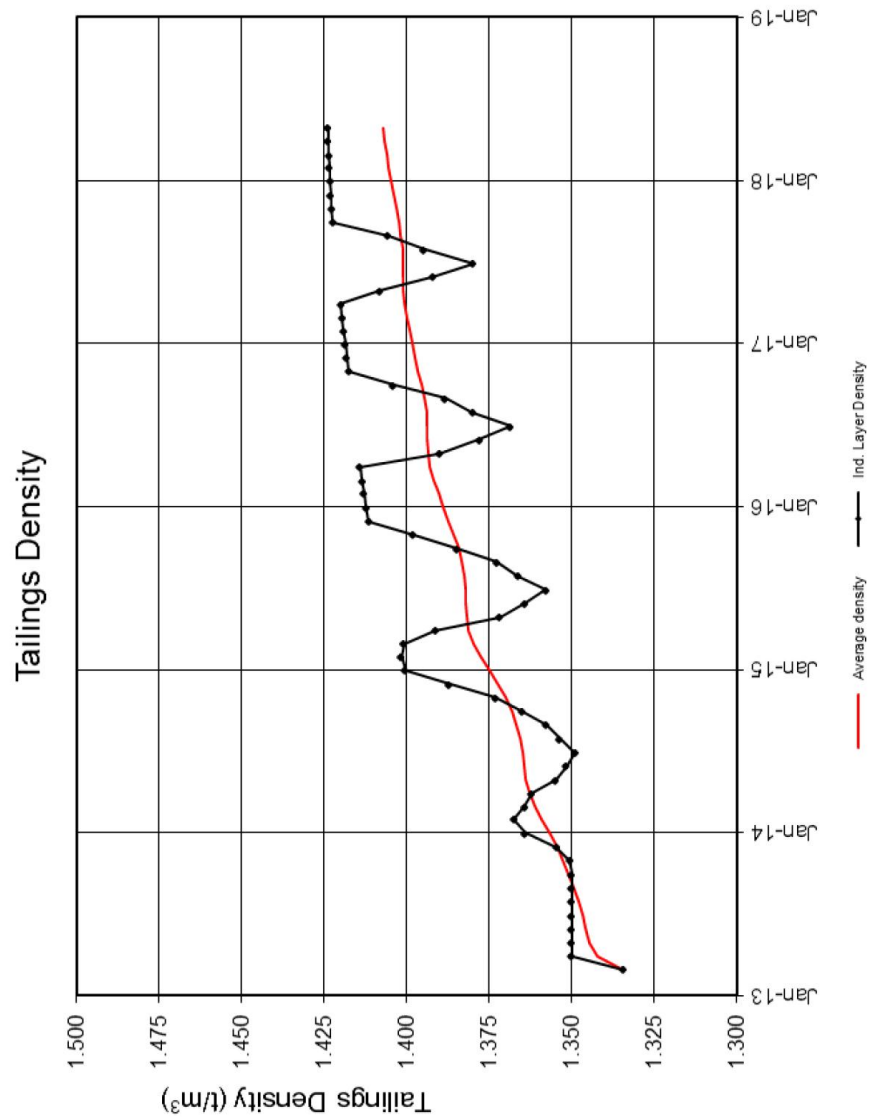
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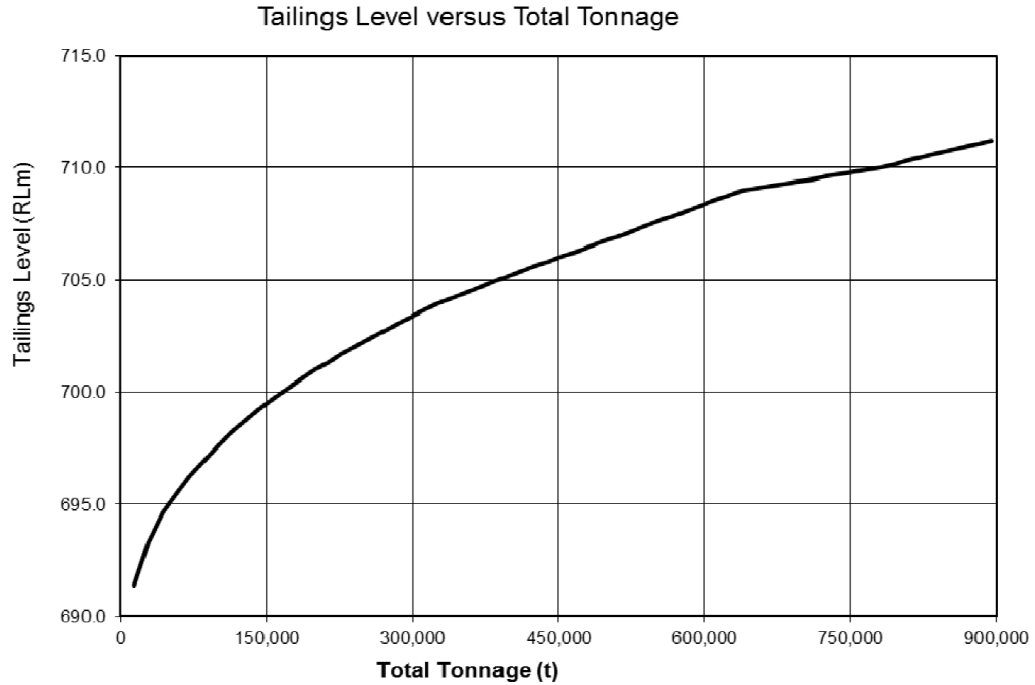
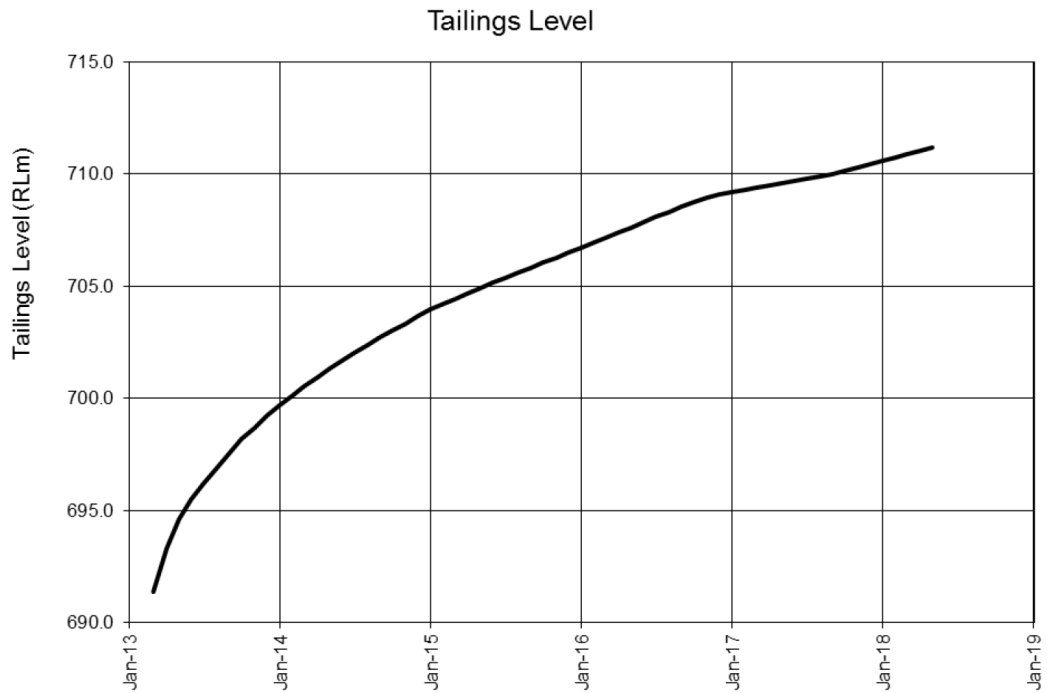


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Figure 5.2

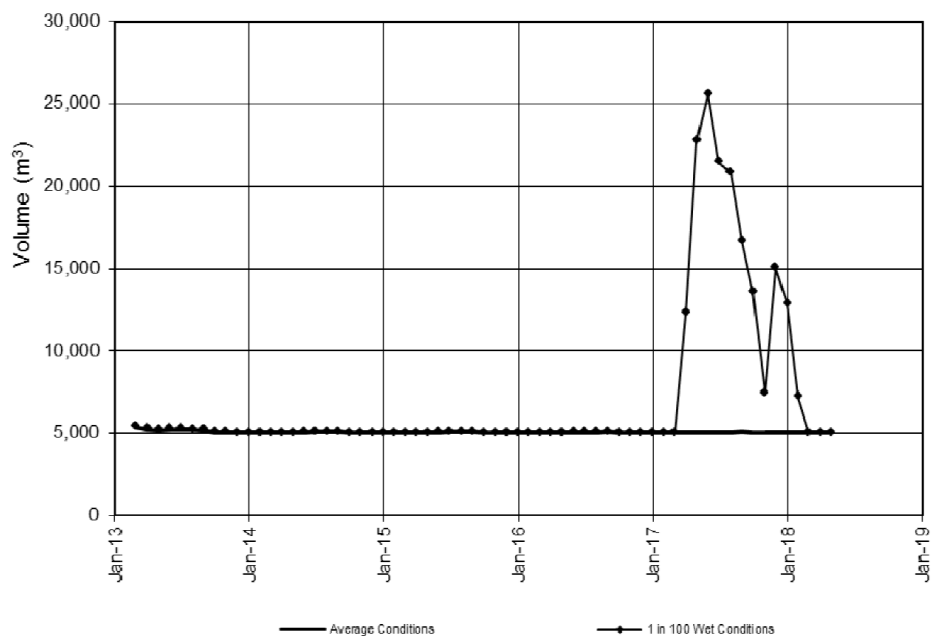
DARGUES REEF GOLD PROJECT  
RAINFALL INTENSITY DURATION FREQUENCY DATA  
BASED TO EXTREME STORMS

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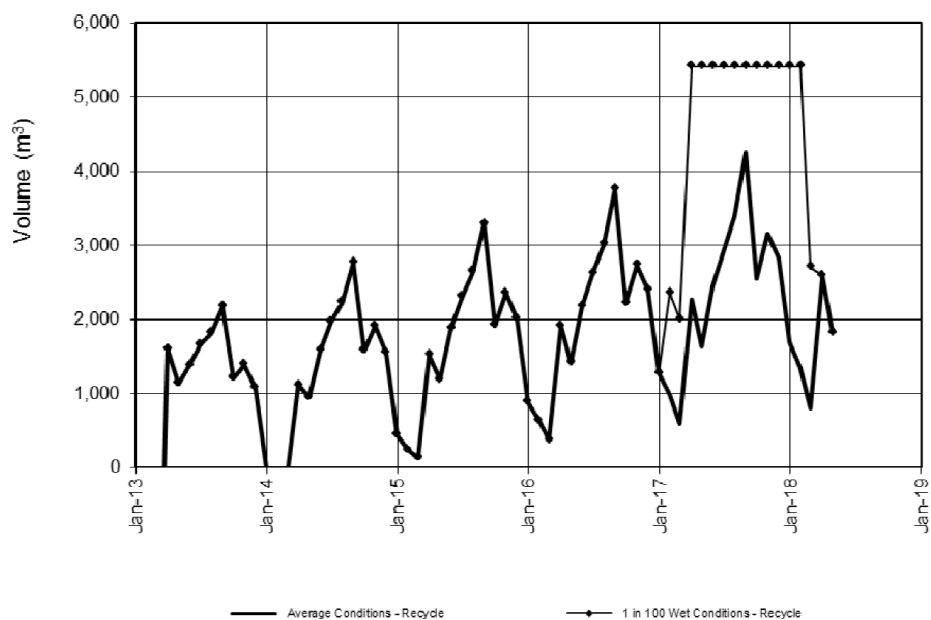




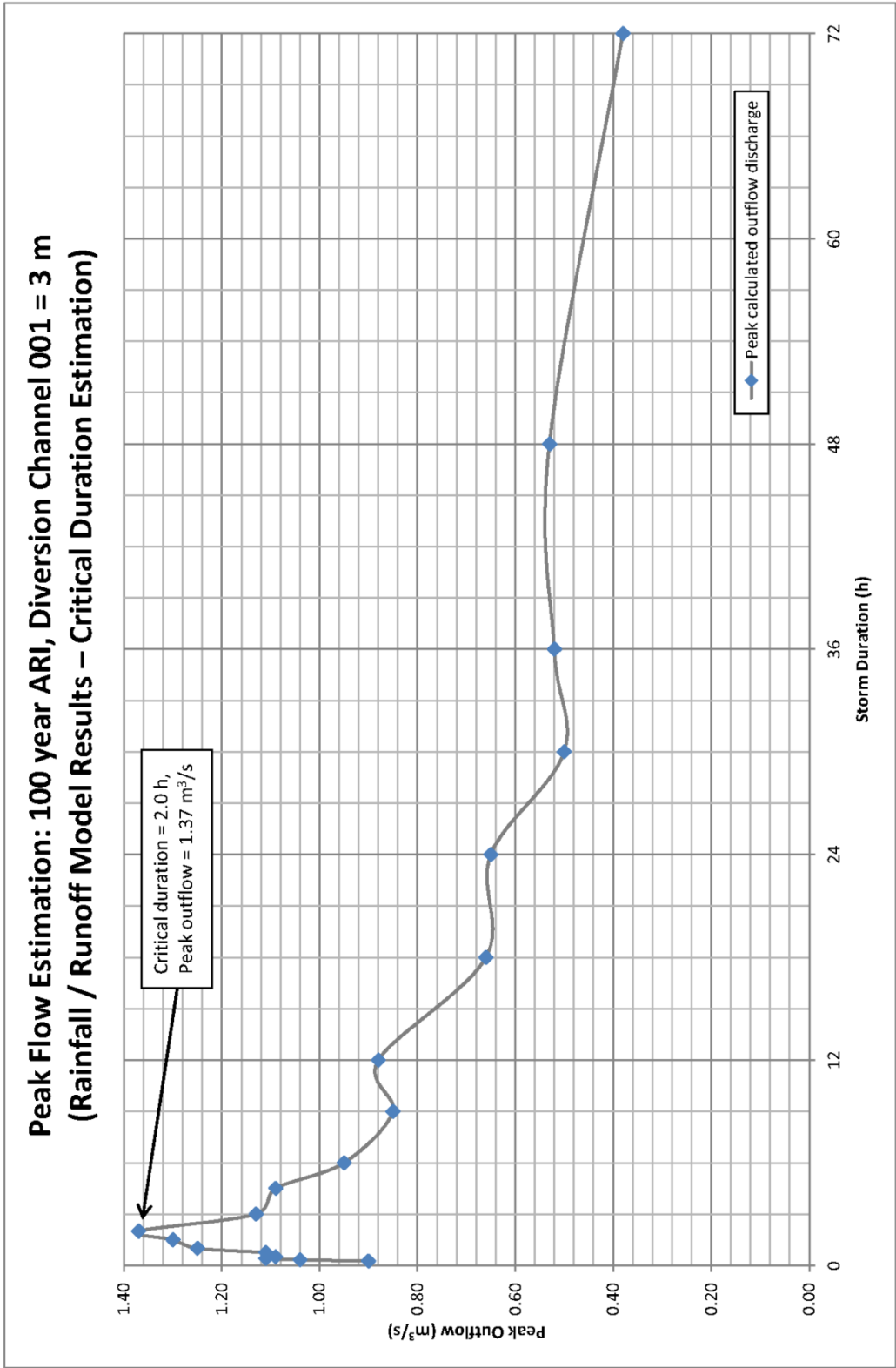
Pond Volumes



Recycle to Plant from TSF



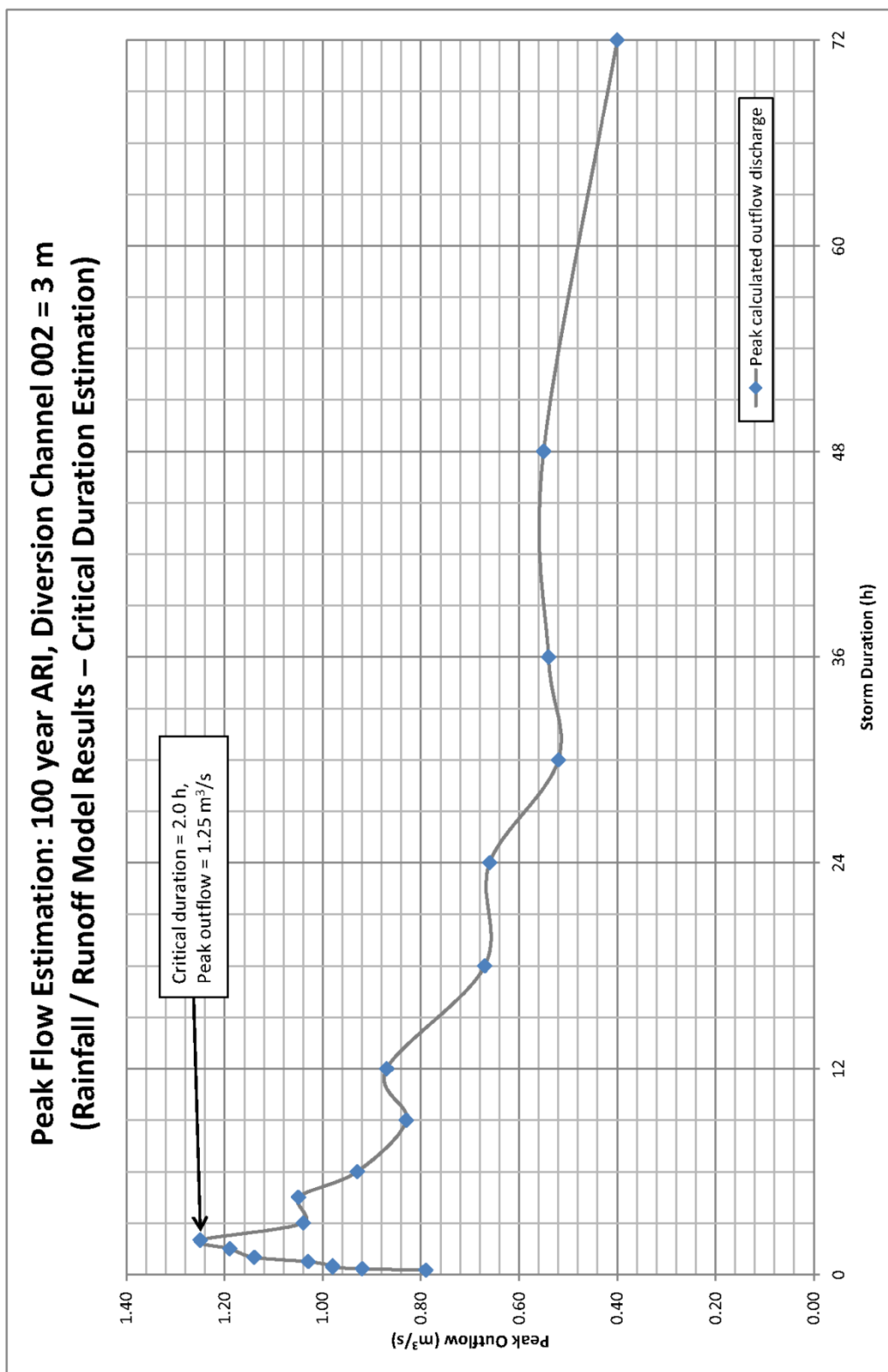




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Figure 5.6

DARGUES REEF GOLD PROJECT  
TSF DIVERSION CHANNEL 001  
PEAK RUNOFF AND CRITICAL DURATION RESULTS

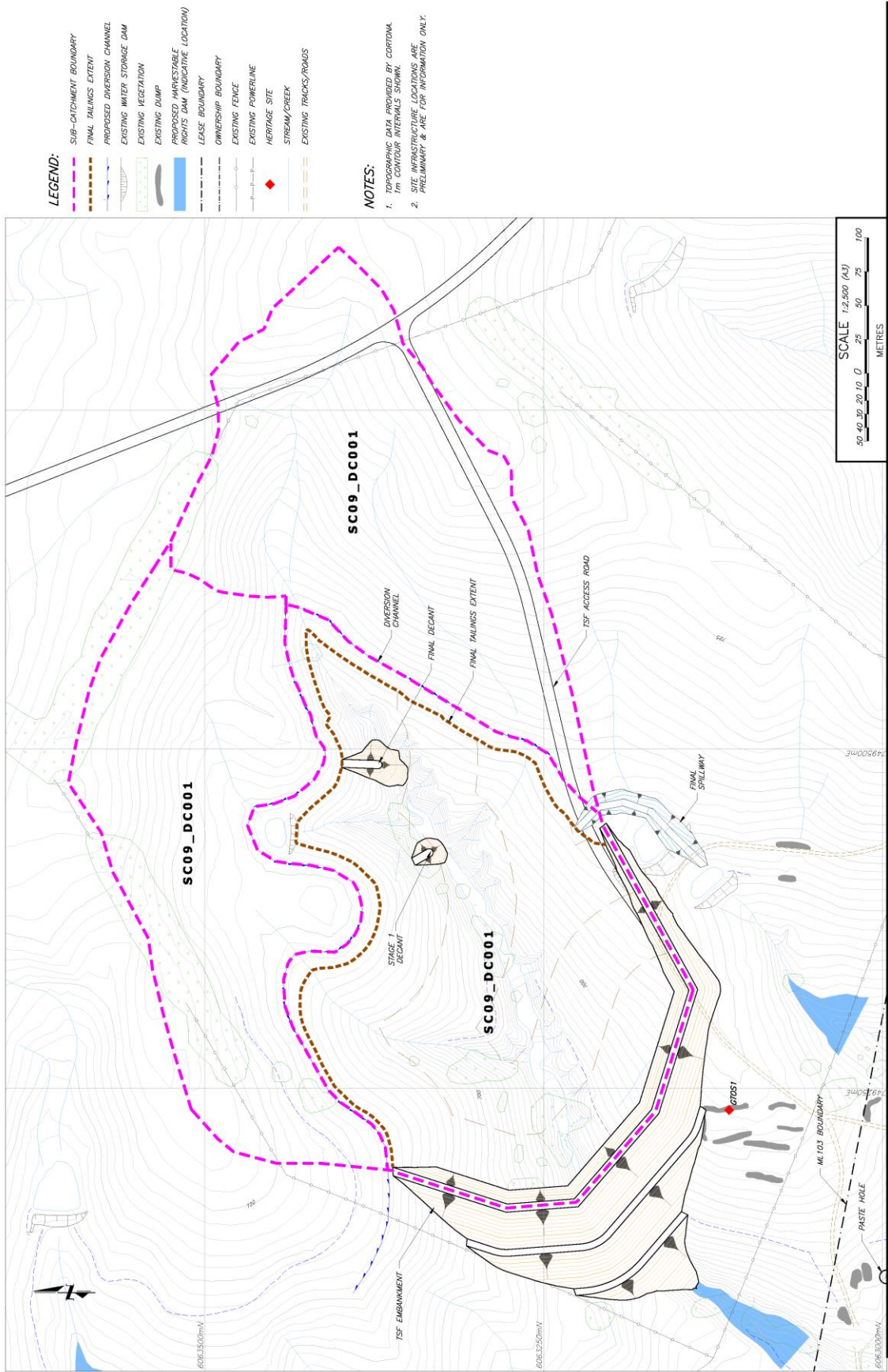
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DARGUES REEF GOLD PROJECT  
TSF DIVERSION CHANNEL 002  
PEAK RUNOFF AND CRITICAL DURATION RESULTS

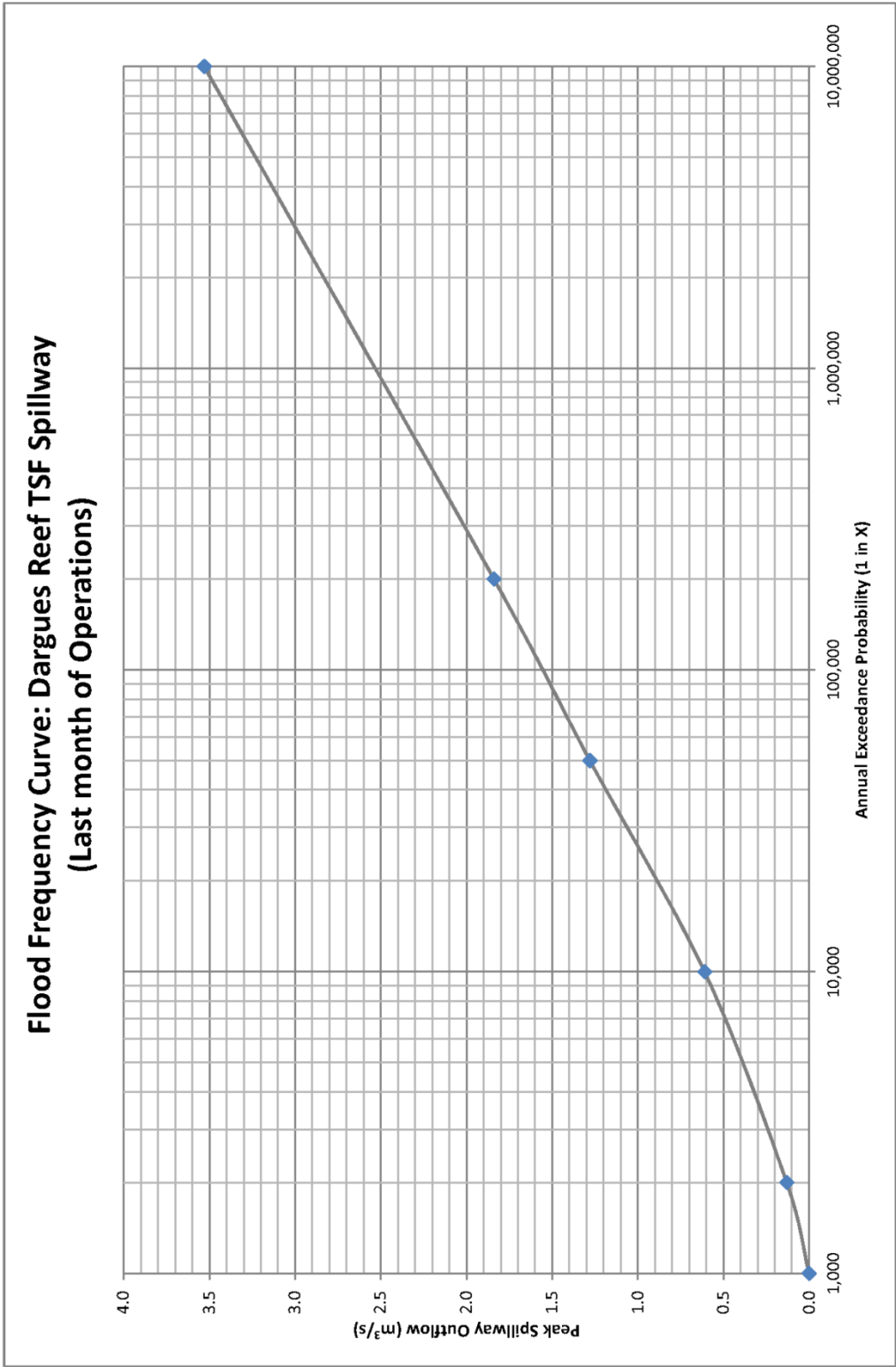
Ref: PE801-00139/05  
Figure 5.7



Ref: PE801-00139/05  
Figure 6.1  
Rev: 0

DARGUES REEF GOLD PROJECT  
SPILLWAY SIZING  
SUB-CATCHMENT DELINEATION MAP

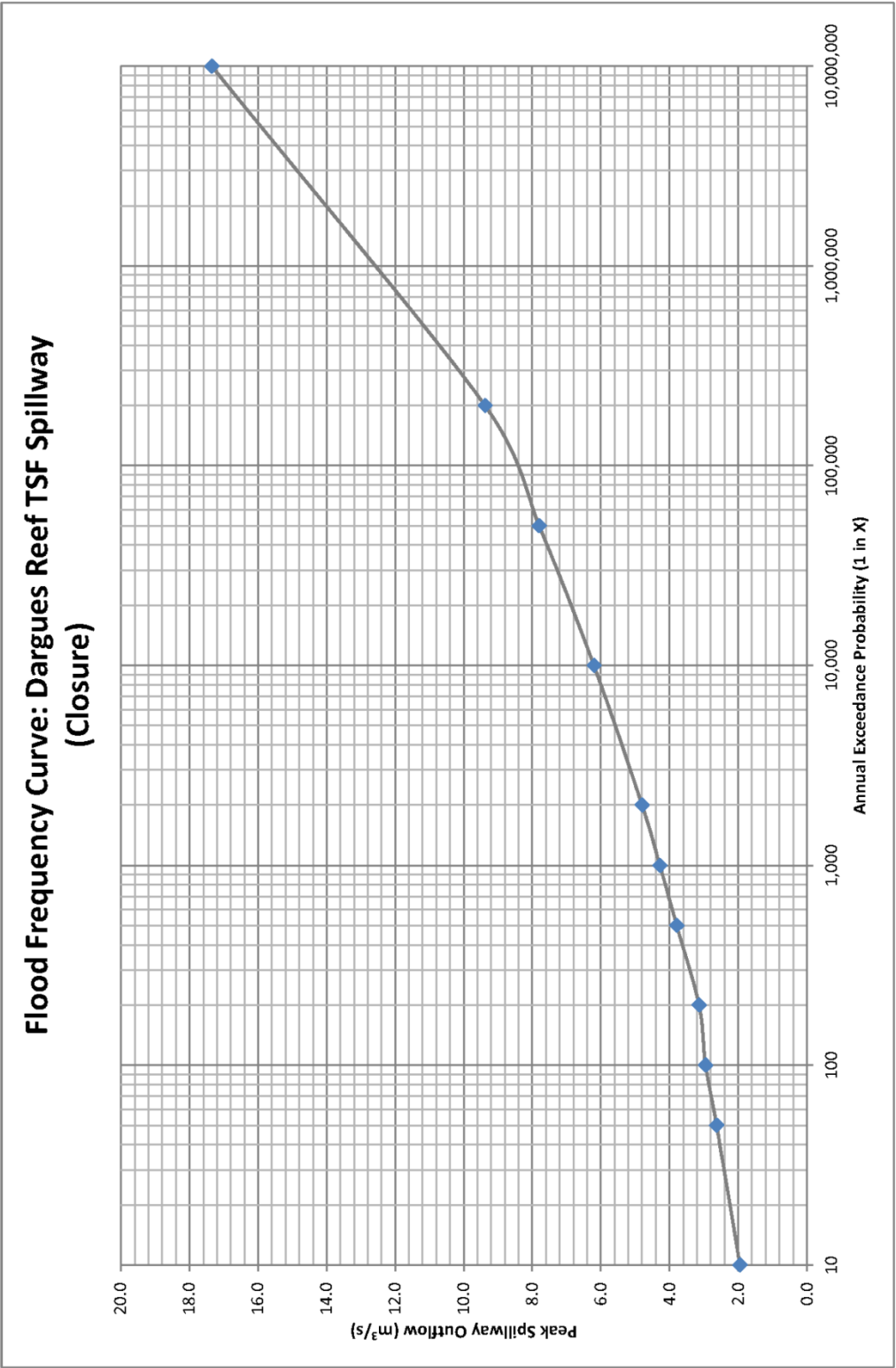
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Ref: PE801-00139/05  
Figure 6.2

DARGUES REEF GOLD PROJECT  
TSF SPILLWAY FLOOD FREQUENCY CURVE  
LAST MONTH OF OPERATIONS

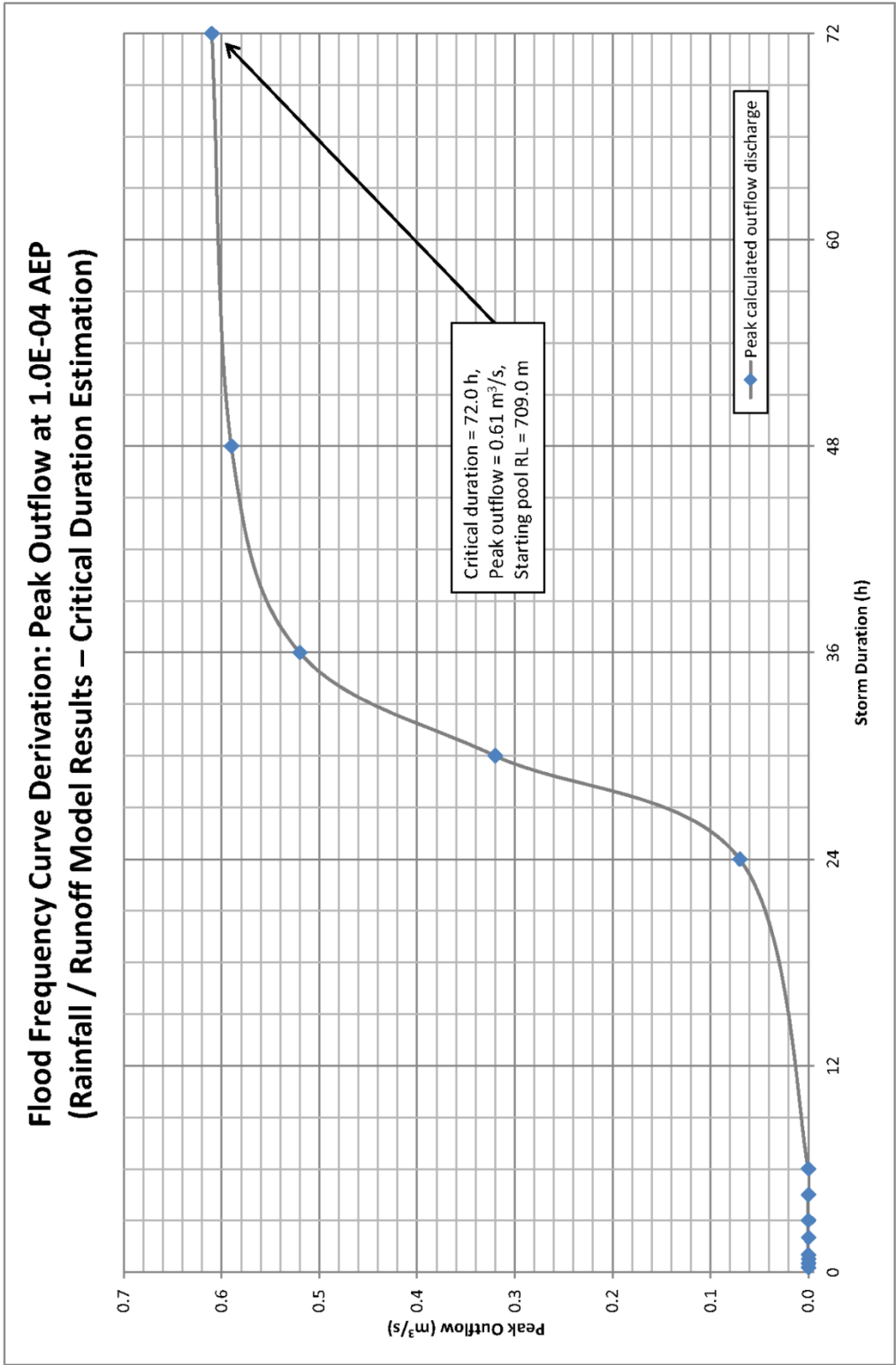
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Ref: PE801-00139/05  
Figure 6.3

DARGUES REEF GOLD PROJECT  
TSF SPILLWAY FLOOD FREQUENCY CURVE  
CLOSURE

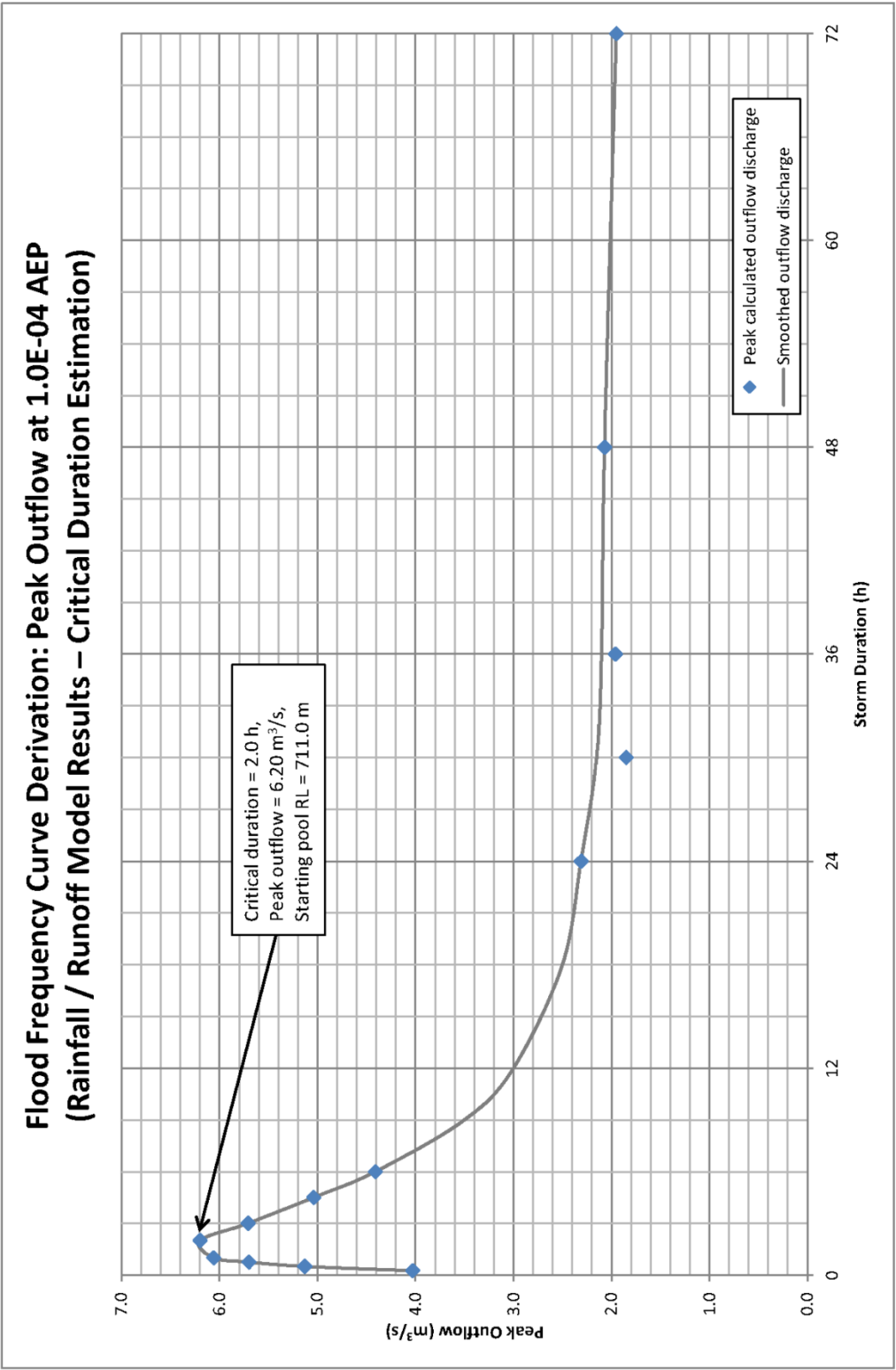
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Ref: PE801-00139/05  
Figure 6.4

DARGUES REEF GOLD PROJECT  
TSF SPILLWAY CRITICAL DURATION AT 1:10,000 AEP  
LAST MONTH OF OPERATIONS

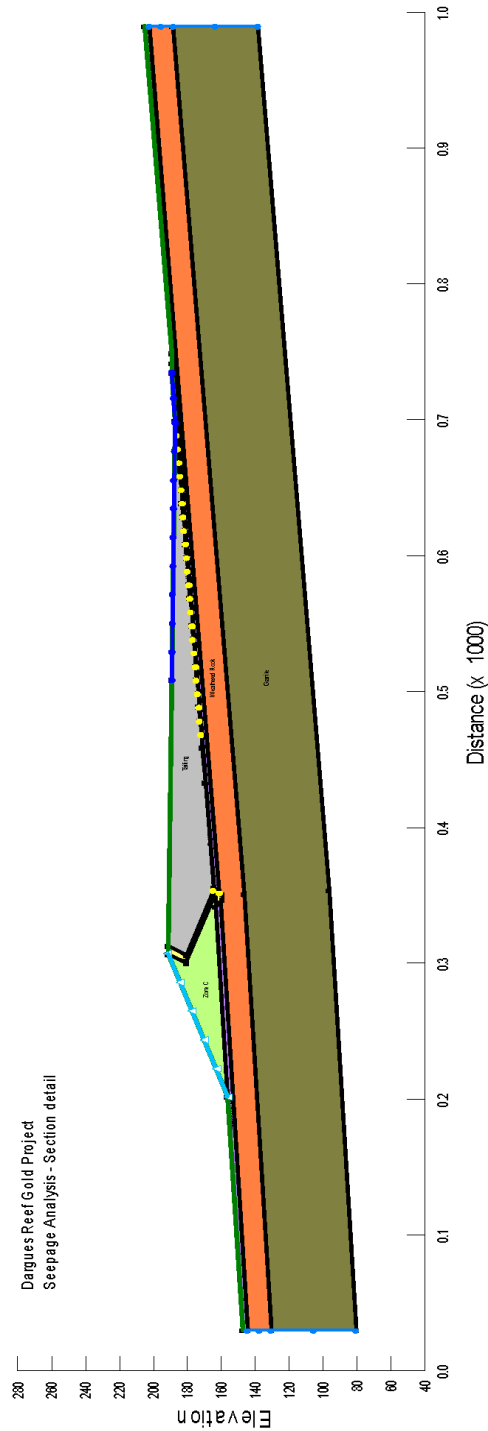
**Knight Piésold**  
CONSULTING



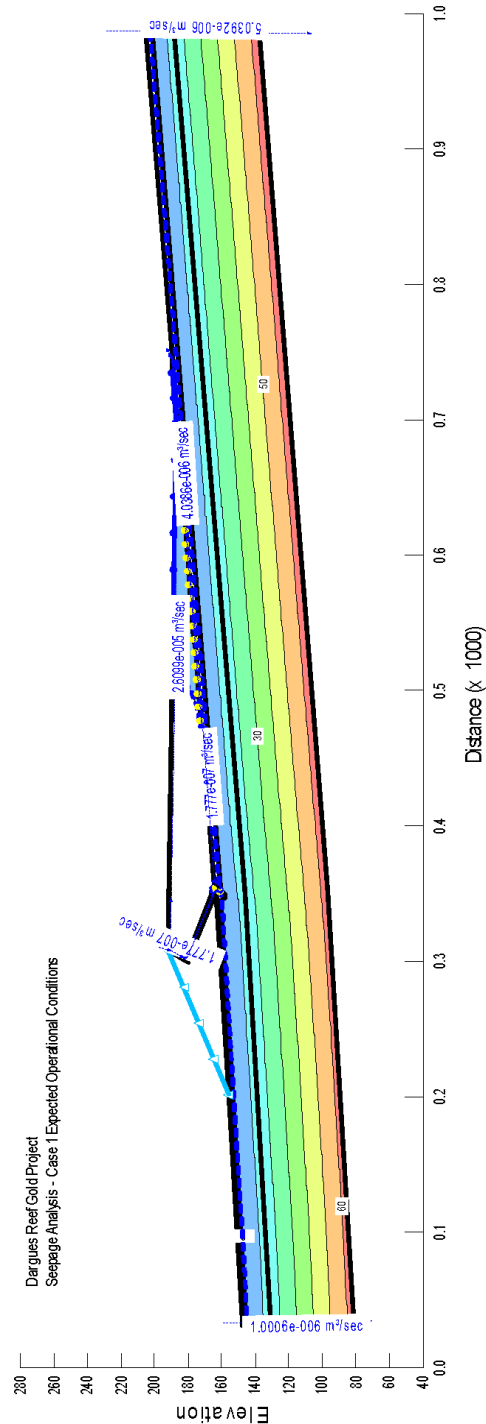
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Figure 6.5

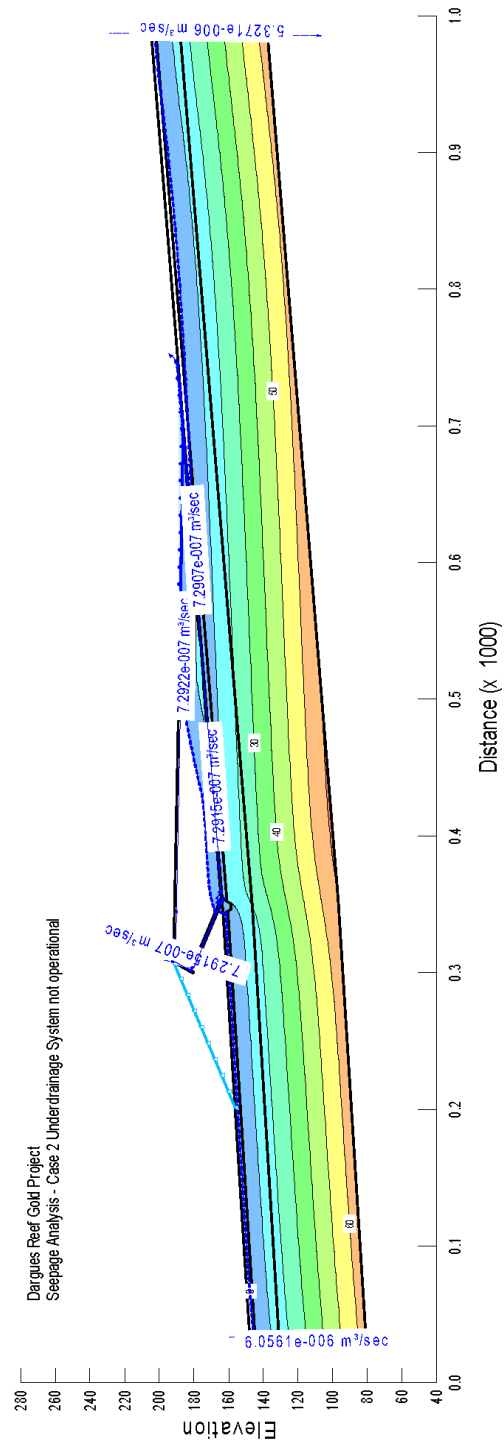
DARGUES REEF GOLD PROJECT  
TSF SPILLWAY CRITICAL DURATION AT 1:10,000 AEP  
CLOSURE

**Knight Piésold**  
CONSULTING



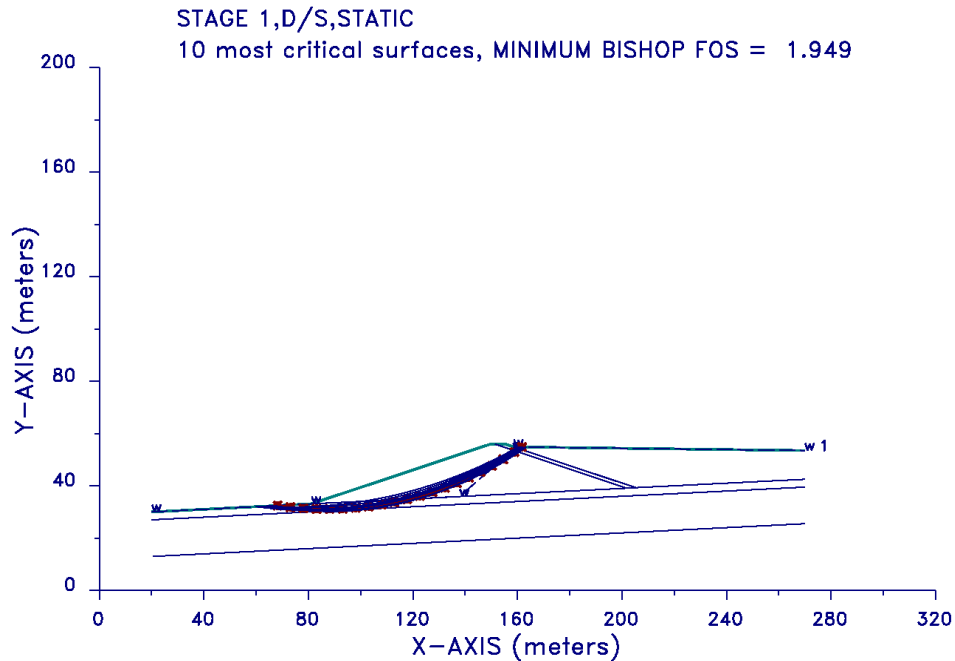






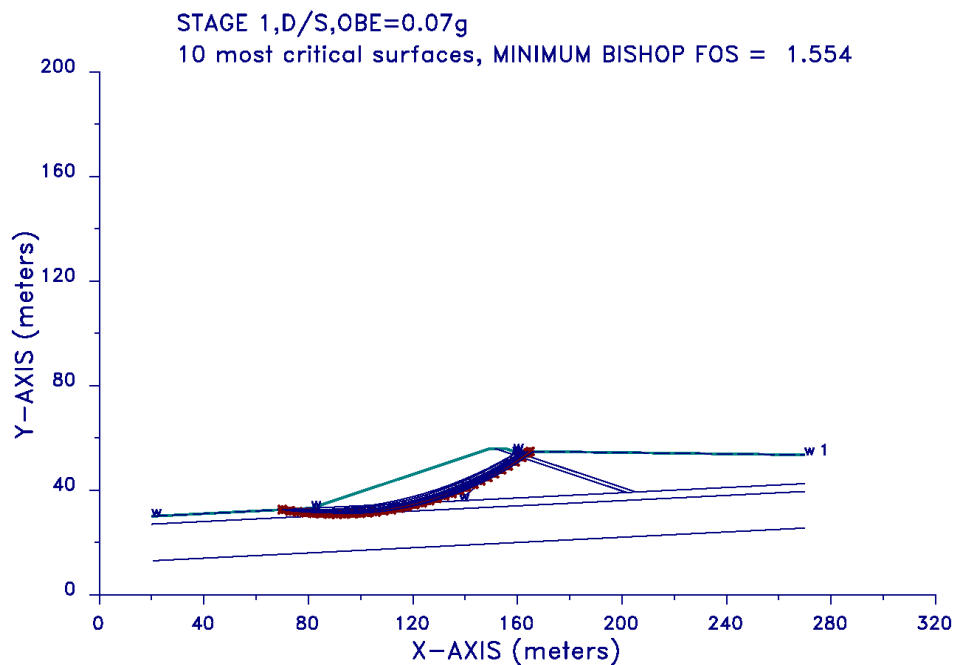
STARTER, HIGH POND - STATIC

S1DSS 10-08-10 17:23

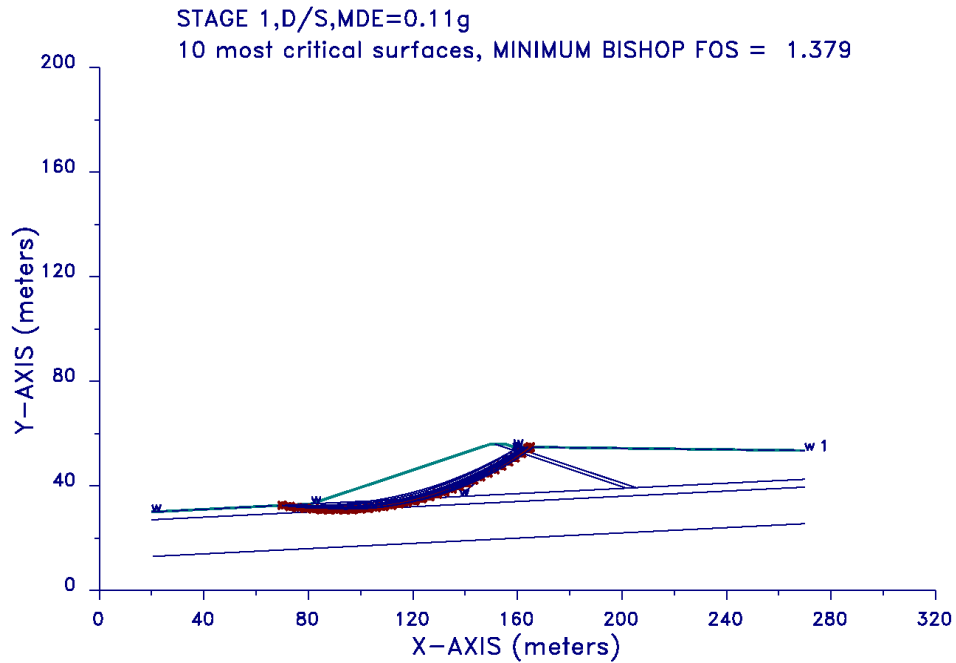


STARTER, HIGH POND - OPERATING BASIS EARTHQUAKE

S1DSO 11-23-11 15:19

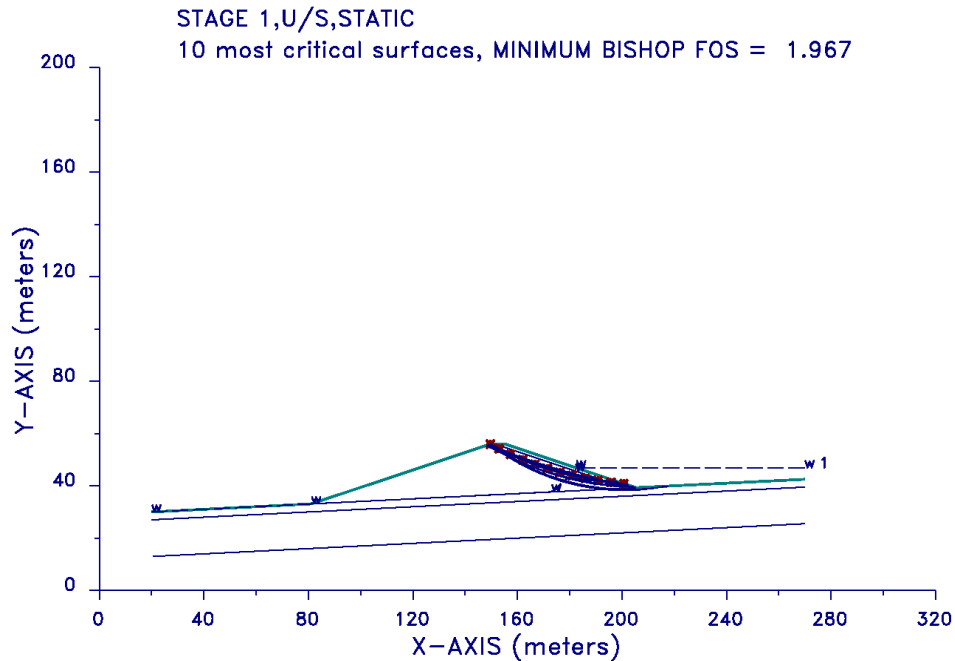


**STARTER, HIGH POND - MAXIMUM DESIGN EARTHQUAKE**  
 S1DSM 11-23-11 11:18



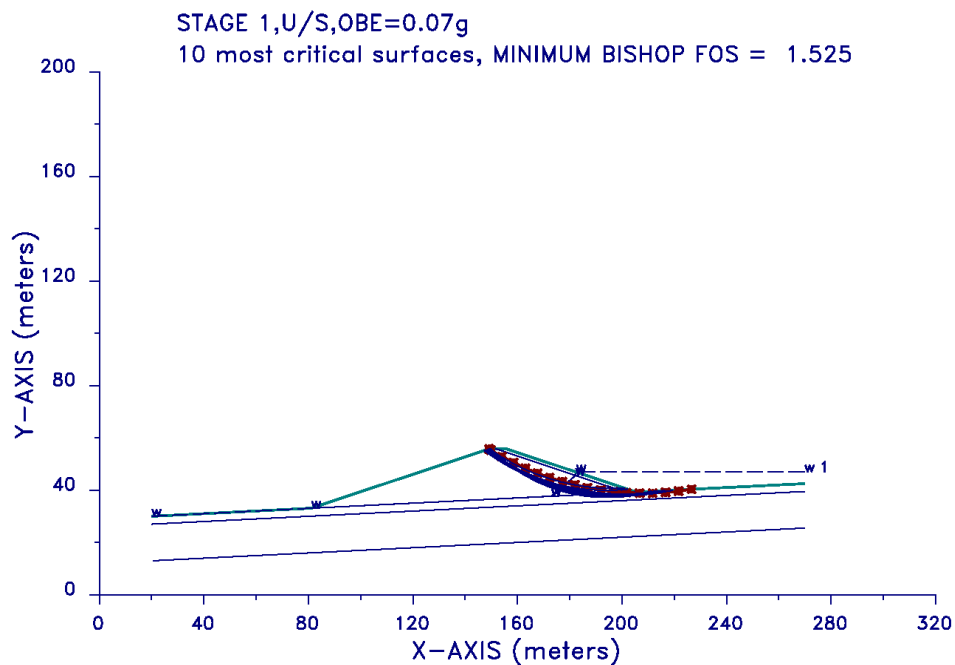
STARTER, INTERMEDIATE POND - STATIC

S1U5S 10-08-10 17:41



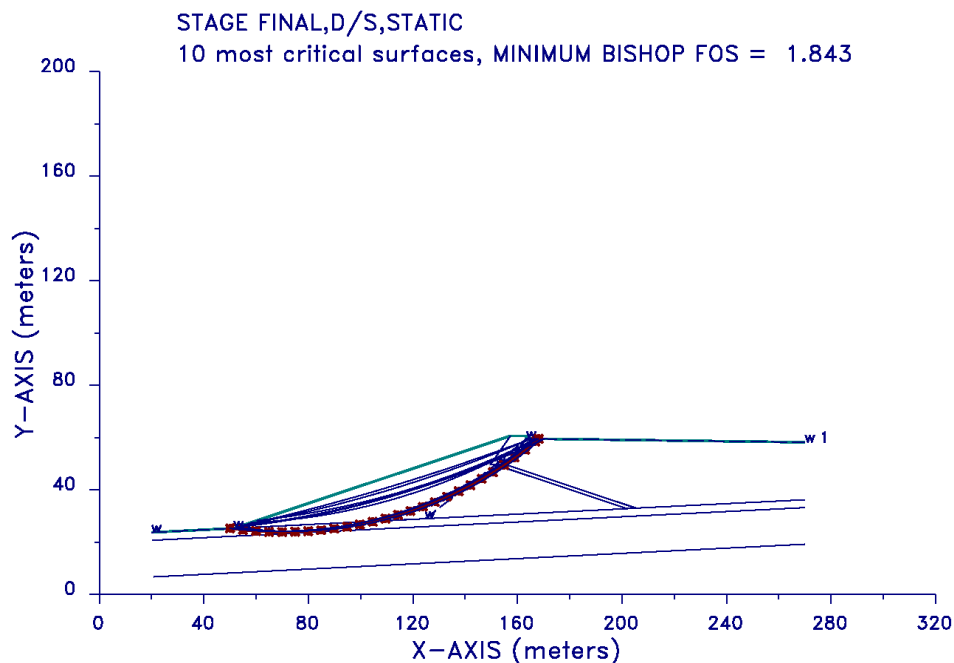
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S1U5O 11-23-11 12:03



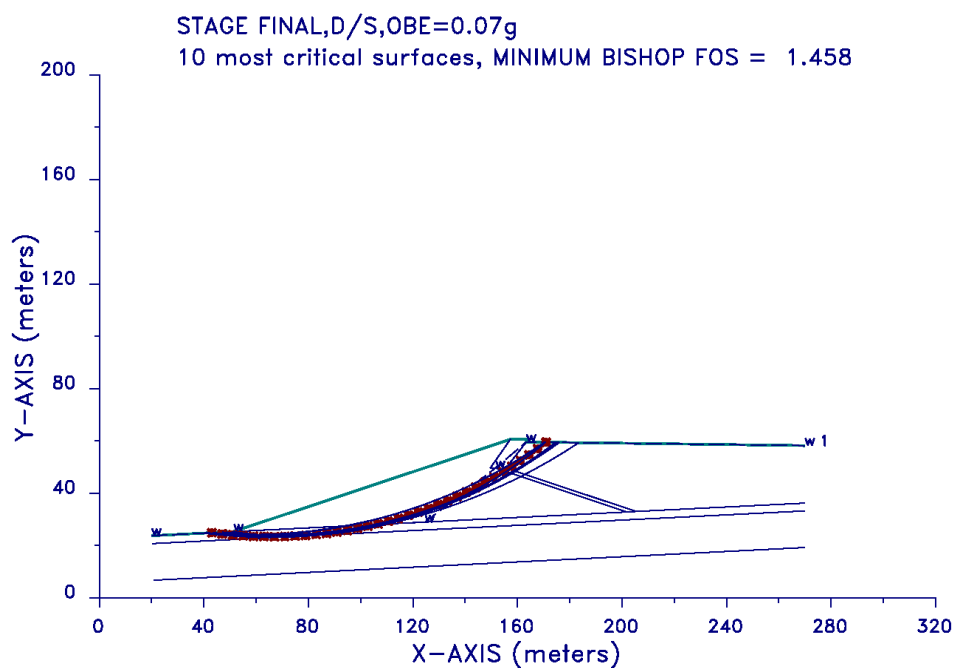
**STAGE FINAL, HIGH POND - STATIC**

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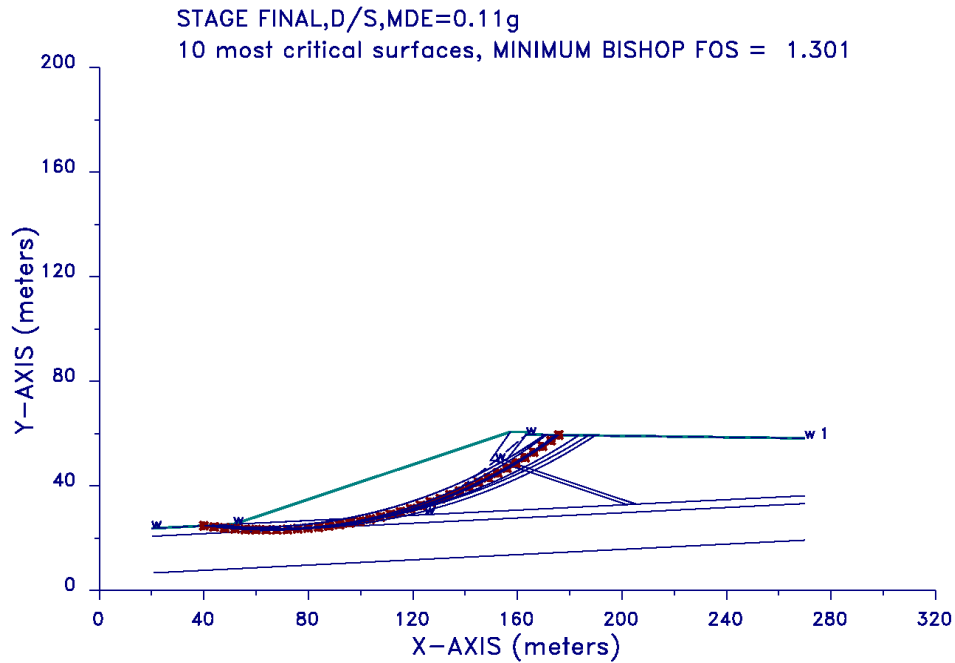


**STAGE FINAL, HIGH POND - OPERATING BASIS EARTHQUAKE**

SFDSO 11-23-11 14:28

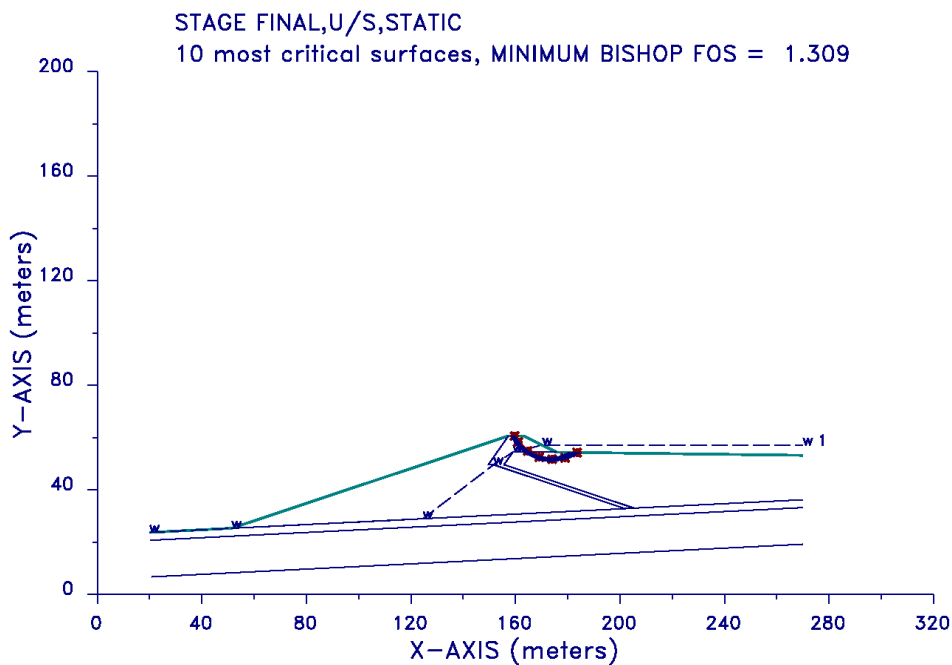


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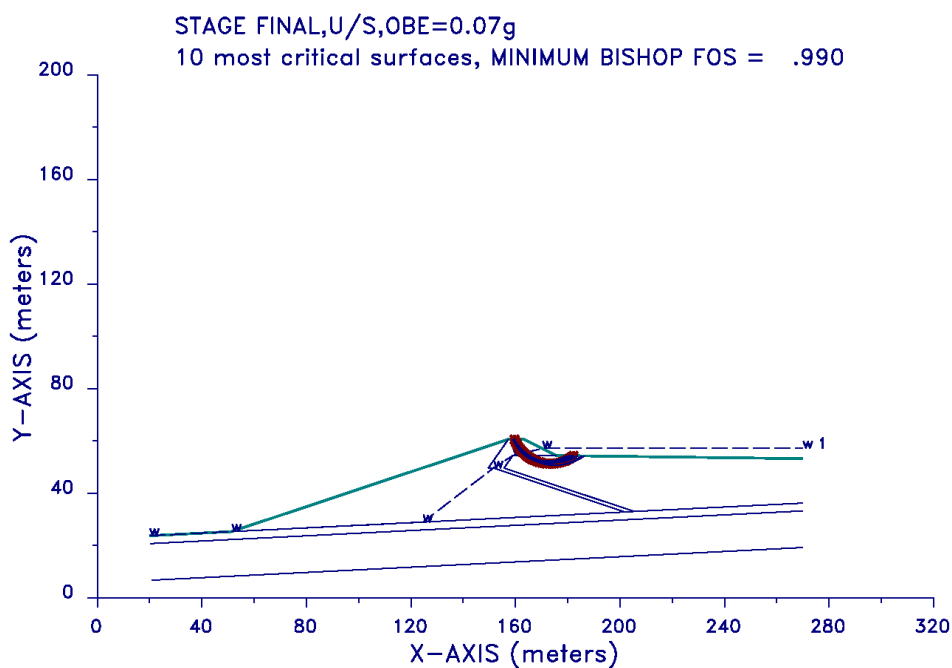
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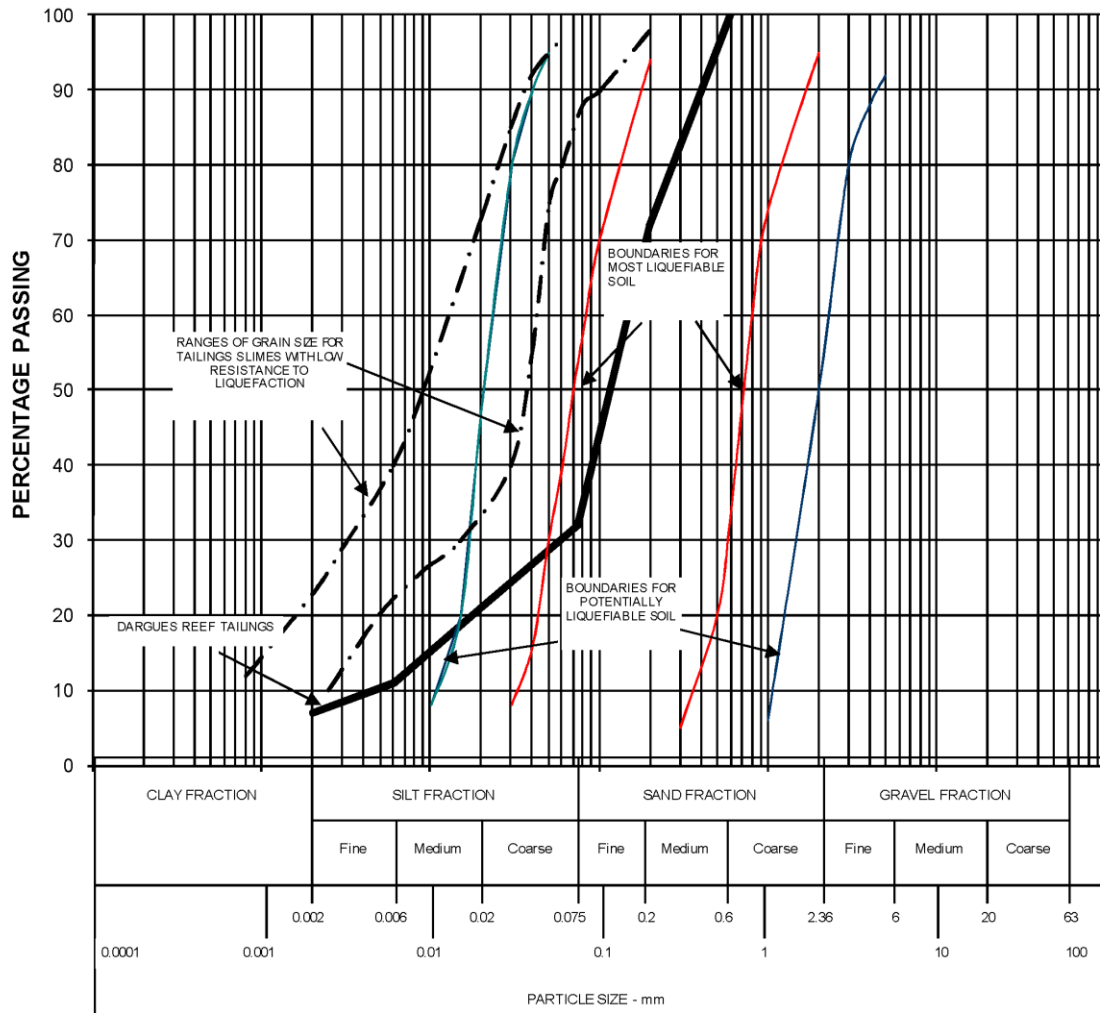


**STAGE FINAL, INTERMEDIATE POND - OPERATING BASIS EARTHQUAKE**

SFUSS 11-23-11 15:00







DRAWINGS

***Knight Piésold***  
**CONSULTING**

DARGUES REEF GOLD PROJECT  
TAILINGS STORAGE FACILITY

FINAL DESIGN  
DECEMBER, 2011

Prepared for:

CORTONA RESOURCES  
LIMITED

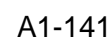
Prepared by:

***Knight Piésold***  
CONSULTING

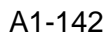
A.B.N. 67 001 040 419  
A.C.N. 001 040 419


Knight Piésold Pty. Ltd.  
Level 1, 184 Adelaide Terrace  
EAST PERTH, WA 6004  
AUSTRALIA



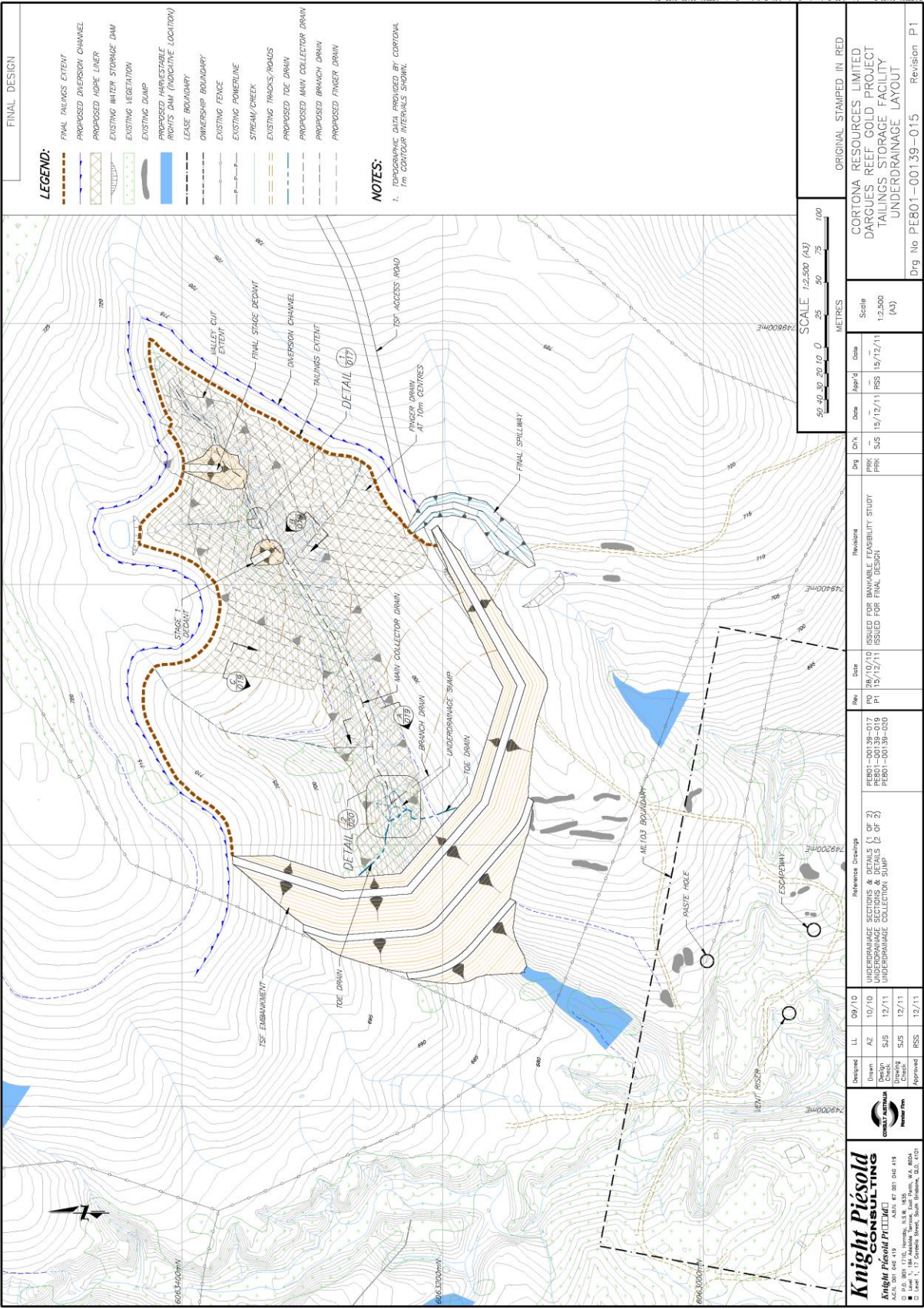






 <b>Knight Piésold CONSULTING</b> <i>Knight Piésold PILLING</i> 1700 DOW STREET, SUITE 400 FALLS CHURCH, VA 22044 TEL: 703.441.4000 FAX: 703.441.4001 WWW.KPIESOLD.COM	Designed LL 10/10	Drawn PRK 10/10	Design Check SJS 12/11	Drawing Check SJS 12/11	Approved RSS 12/11	Reference Drawings GENERAL ARRANGEMENT	Rev P1	Date 15/12/11	Revisions ISSUED FOR BANAABLE FEASIBILITY STUDY ISSUED FOR FINAL DESIGN	Dwg PRK SJS	Chk - -	Date - -	Scale AS SHOWN (A3)	CORTONA RESOURCES LIMITED DARQUES REEF GOLD PROJECT TALINGS STORAGE FACILITY EMBANKMENT SECTIONS & DETAILS	Dwg No PE801-001:39-012 Revision P1







**NOTES:**

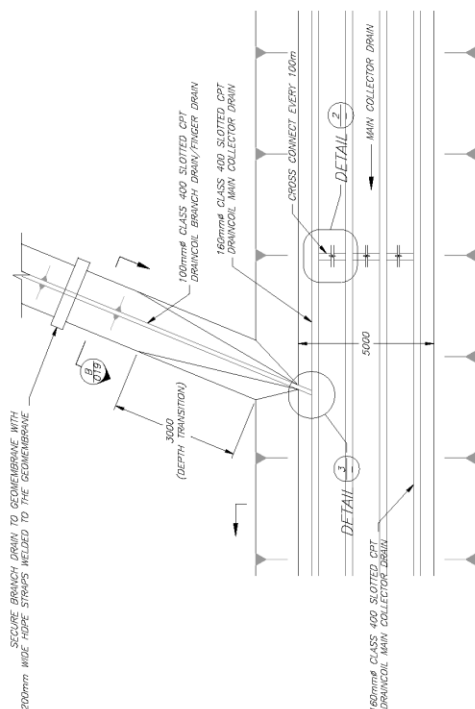
- 1. DEFENDING ON THE CONSTRUCTION METHOD APPROVED BY CONTRACTOR. THE GROUNDWATER ALONG THE MAIN GREY BED COULD BE PLACED AT THE BEGINNING OR END OF INSTALLATION. THE GROUNDWATER ALONG THE MAIN GREY BED COULD BE PLACED AT THE BEGINNING OR END OF INSTALLATION. THE SLOPES SHALL BE ANCHORED, SUBSEQUENT CONNECTION AT THIS POINT SHALL BE MADE BY CUTTING THE CONNECTION AT THE END POINTS OF THE ANCHOR, AND THEN CONTIGUOUSLY WELDING THE PANELS.
- 2. ALL GEOTEXTILE USED TO WARP DRAINAGE SHALL BE CONTINUOUSLY JOINED, SEWN OR THERMALLY BONDED (LESTER) AT THE OVERLAP AS DIRECTED BY THE ENGINEER.
- 3. GEOTEXTILE FILTER SOCK FOR CPT DRAINCOLL SHALL BE BOWD A14 NON-MIXED GEOTEXTILE OF APPROVED EQUIVALENT.
- 4. A NORMAL BEDDING LAYER OF 300mm THICKNESS MAY BE REQUIRED ALONG THE MAIN GREY BED ALIGNMENT TO ENABLE THE DRAINAGE TO BE ADDED ON THE SITE AS DIRECTED BY THE ENGINEER.
- 5. CONTAMINATED GRADING FILL SHALL BE PLACED IF REQUIRED TO ALLOW DRAINAGE BY GRAVITY AND/OR TO FORM MAIN CHANNEL.

## BRANCH DRAINS ON GEOMEMBRANE CONSTRUCTION SEQUENCE

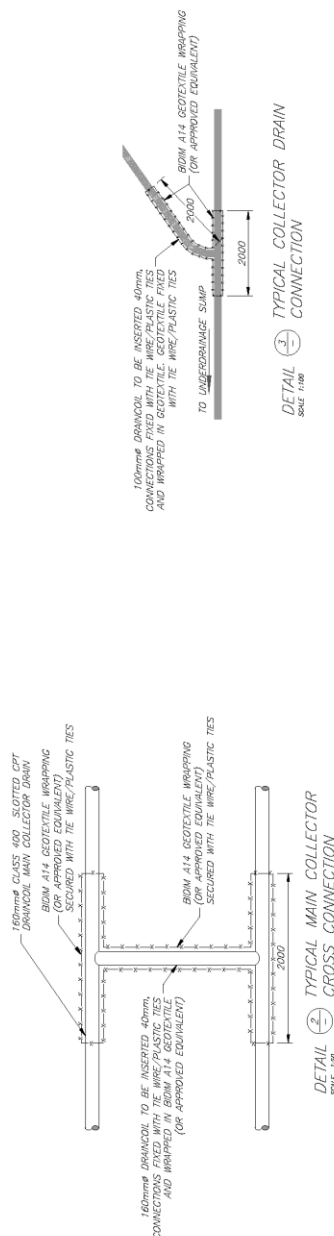
2. PLACE GEOTEXTILE.
3. PLACE, CUT AND CONNECT DRAINCOIL.
4. PLACE ZONE F SAND.
5. WRAP THE SAND WITH GEOTEXTILE TO FORM DRAIN.
6. SECURE BRANCH DRAINS TO HDPE GEOMEMBRANE LINER USING STRIPS WELDED TO THE BASIN LINER.
7. REMOVE ALL TEMPORARY WORKS.
8. INSPECT ADJACENT GEOMEMBRANE AND REPAIR IF DAMAGED.


### MAIN COLLECTOR DRAINS ON GEOMEMBRANE CONSTRUCTION SEQUENCE

7. PLACE GEOTEXTILE
8. PLACE DRAINCOIL WITH GEOTEXTILE FILTER SOCK. CUT AND CROSS
9. CONNECT DRAINCOIL EVERY 100m AND TIE IN WITH BRANCH DRAINS.
10. PLACE ZONE F SAND.
11. WRAP THE SAND WITH GEOTEXTILE TO FORM DRAIN.
12. REMOVE ALL TEMPORARY WORKS.
13. INSPECT ADJACENT GEOMEMBRANE AND REPAIR IF DAMAGED.



DETAIL  $\left(\frac{1}{0.15}\right)$  TYPICAL COLLECTOR DRAIN JUNCTION SCHEMATIC



<div><b>Knight Piesold CONSULTING</b></div> <div><b>Knight Piesold Pte Ltd</b> 100 Robinson Road, #14-01, Robinson Centre Singapore 068906 Tel: 6733 1111, Fax: 6733 1112 Email: info@knightpiesold.com Website: www.knightpiesold.com</div>		Described LL 09/10		Drawn PRK 10/10 Design SJS 12/11 Checked SJS 12/11 Drawing RSS 12/11 Approval		Reference Drawings  UNDERDRAINAGE LAYOUT UNDERDRAINAGE SECTIONS & DETAILS (2 OF 2)		Rev PD 09/10/10 PER01-00136-015 PER01-00135-019		Revisions  ISSUED FOR AVAILABLE FEASIBILITY STUDY ISSUED FOR FINAL DESIGN		Drg PRK PRK SJS SJS SJS SJS		Date 09/10/10 15/12/11 15/12/11 15/12/11 15/12/11 15/12/11		Scale AS SHOWN (AS) (1 OF 2) (2 OF 2)	
CORTONA RESOURCES LIMITED DARGUES REEF GOLD PROJECT TAILINGS STORAGE FACILITY UNDERDRAINAGE SECTIONS & DETAILS (1 OF 2)														Drg No E801-00139-017 Revision P1			