ANNEXURE 12

Stormwater Assessment

prepared by Allen Price and Scarratts

> 22, 24, 171 and 220 Bolong Road, Bomaderry

STORMWATER MANAGEMENT REPORT



Allen Price & Scarratts Pty Ltd Land & Development Consultants 75 Plunkett Street NOWRA NSW 2541

Client: Shoalhaven Starches P/L
Project: Stormwater Management Plan

Project No: N27563 SCC Ref: FOR MOD 16 Date: 07 June 2018

DEVELOPMENT DRAINAGE DESIGN REPORT at

MANILDRA VARIOUS SITES
BOLONG ROAD,
BOMADERRY

This report has been prepared to support a modification to development approval (MOD 16) in accordance with Shoalhaven City Council's (SCC) D5 Engineering Standards document and DCP2014 for storm events from the 1:10 ARI to 1:100 ARI.

Calculations have been prepared by a qualified practicing engineer using DRAINS version 2018.05.

There are multiple areas proposed for development under this modification. Appendix A ref N27563-401 numbers each site to be addressed and are summarised as follows:

1. Product Dryer

This is currently hardstand area, therefore no pre/post flow considerations are required. Stormwater is proposed to sheet from roof to existing capture from ground (no downpipes) to match existing discharge pattern as outlined in Drawing ref MN6531-016 (Appendix B).

2. Sifter Room

The proposed sifter room raises the existing roof area (MN6531-014), therefore no pre/post flow considerations are required. Stormwater will continue to discharge unchanged from existing condition per pattern in drawing ref MN6531-016 (Appendix B).

3. Main Sub-Station Extension

The proposed Main Sub-Station extension raises the existing roof area with minor extensions (MN6531-013), therefore no pre/post flow considerations are required. Stormwater will continue to discharge unchanged from existing condition per pattern in drawing ref MN6531-016 (Appendix B).

4. Proposed Indoor Electrical Sub-Station

The indoor electrical substation (MN6531-010 & -011) is proposed over existing hardstand area, therefore no pre/post flow considerations are required. Stormwater is to be captured and discharged to existing nearby drainage infrastructure. See drawing N27563-402 (Appendix A) and Appendix C for calculations and layout.

5. Relocation of Parking Spaces

Proposed relocation of 26 parking spaces displaced by indoor electrical sub-station (4) is to sheet overland to existing swale to be dispersed over Adjoining bunded paddocks. Discharge is controlled by earth bund wall per prior plan N27259-101 to 103 as such no pre/post flow calculations are required.

6. & 7. Regularisation

Areas for MOD 3 & 12 Regularisation are proposed to have minor adjustment in location with no change in stormwater discharge location or patterns. Stormwater discharge pattern will remain as per MN6531-016 (Appendix B).

8. Rail Unloading Modification

The proposed rail unloading modification has no roof structure and is constructed over existing hardstand area (MN6531-012). Therefore no pre/post flow considerations are required. Stormwater discharge unchanged from existing pattern as per MN6531-016 (Appendix B)

9. Flour Mill C & Flour Mill A, B, C Ventilation

Proposed Flour Mill C structure (MN244-002 to -007, -010, -011) will be contained within the existing flour mill buildings and therefore stormwater analysis is not applicable. The ventilation structures also provide no increase in hardstand area or other increase in capture areas and therefore no pre/post flow considerations are required. Stormwater discharge patterns will remain as per MN6531-016 (Appendix B).

10. Lime Silos

Proposed Lime Silos are to be constructed over existing hardstand area (MN6531-005 to -009) and therefore no pre/post flow considerations are required. Stormwater is to sheet from roof structures to existing ground capture points and discharge per existing condition per MN6531-016 (Appendix B).

11. #8 Boiler and Generator Set

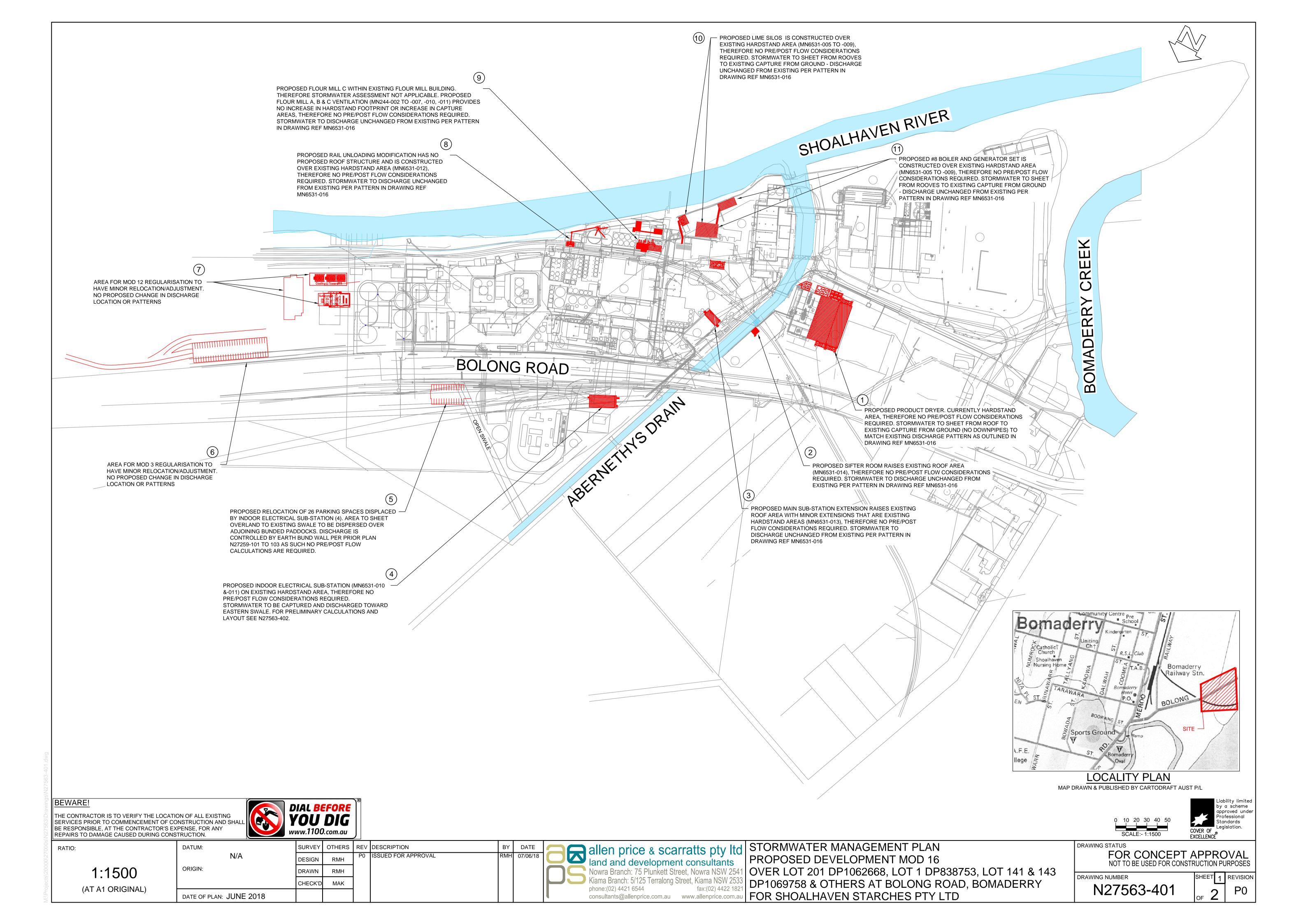
Proposed #8 Boiler and Generator set are to be constructed over existing hardstand area (MN6531-005 to -009) and therefore no pre/post flow considerations are required. Stormwater is to sheet from roof structures to existing ground capture points and discharge per existing condition per MN6531-016 (Appendix B).

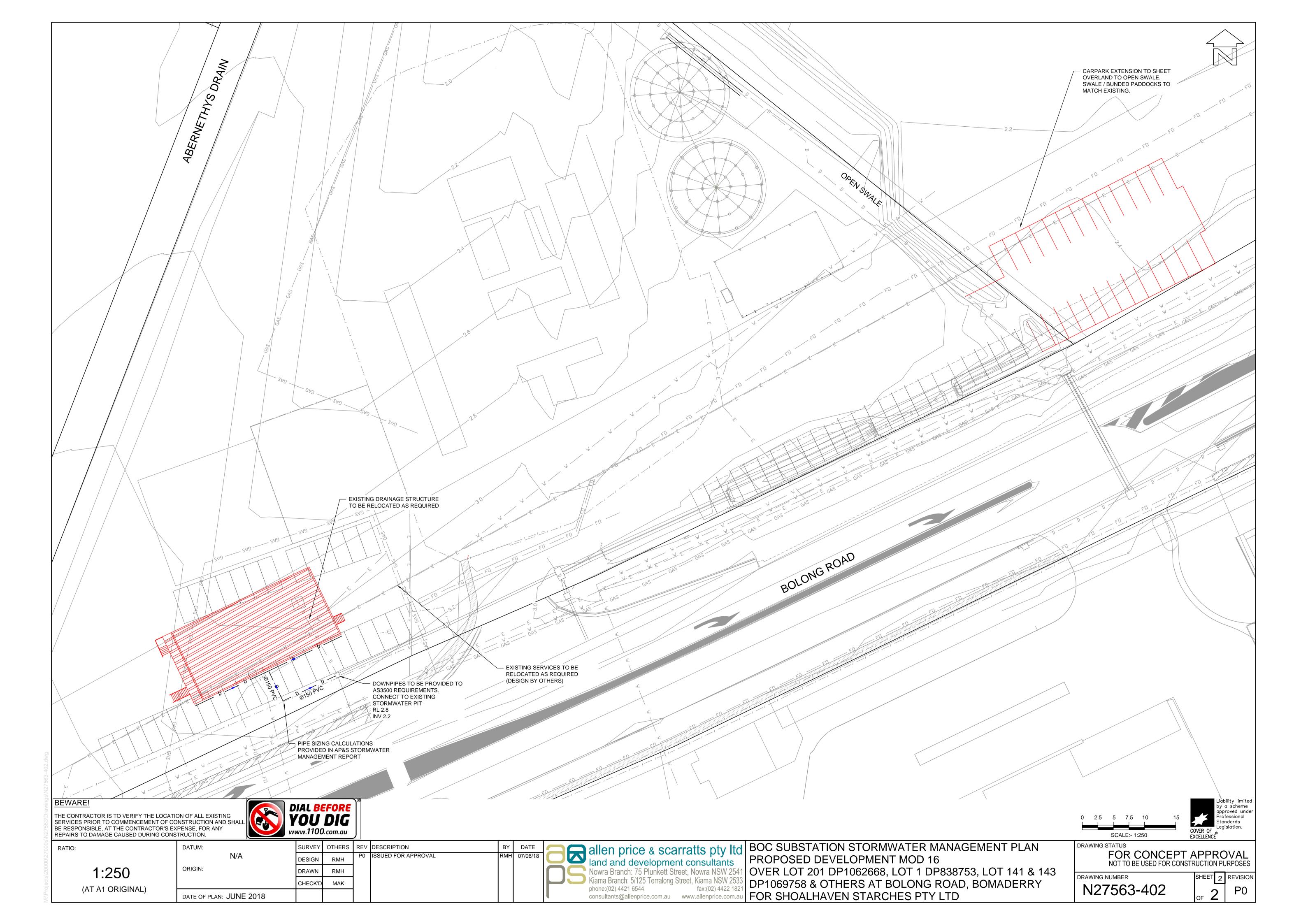
As per the above summary, further consideration and calculations are required only for the Indoor Electrical Sub-Station (4).

It is found that 150mm diameter PVC pipes laid at minimum 0.4% grade will be sufficient to convey roofwater to the nearby drainage infrastructure. As there is no requirement for pre/post flow considerations there is no additional requirement for detention structures.

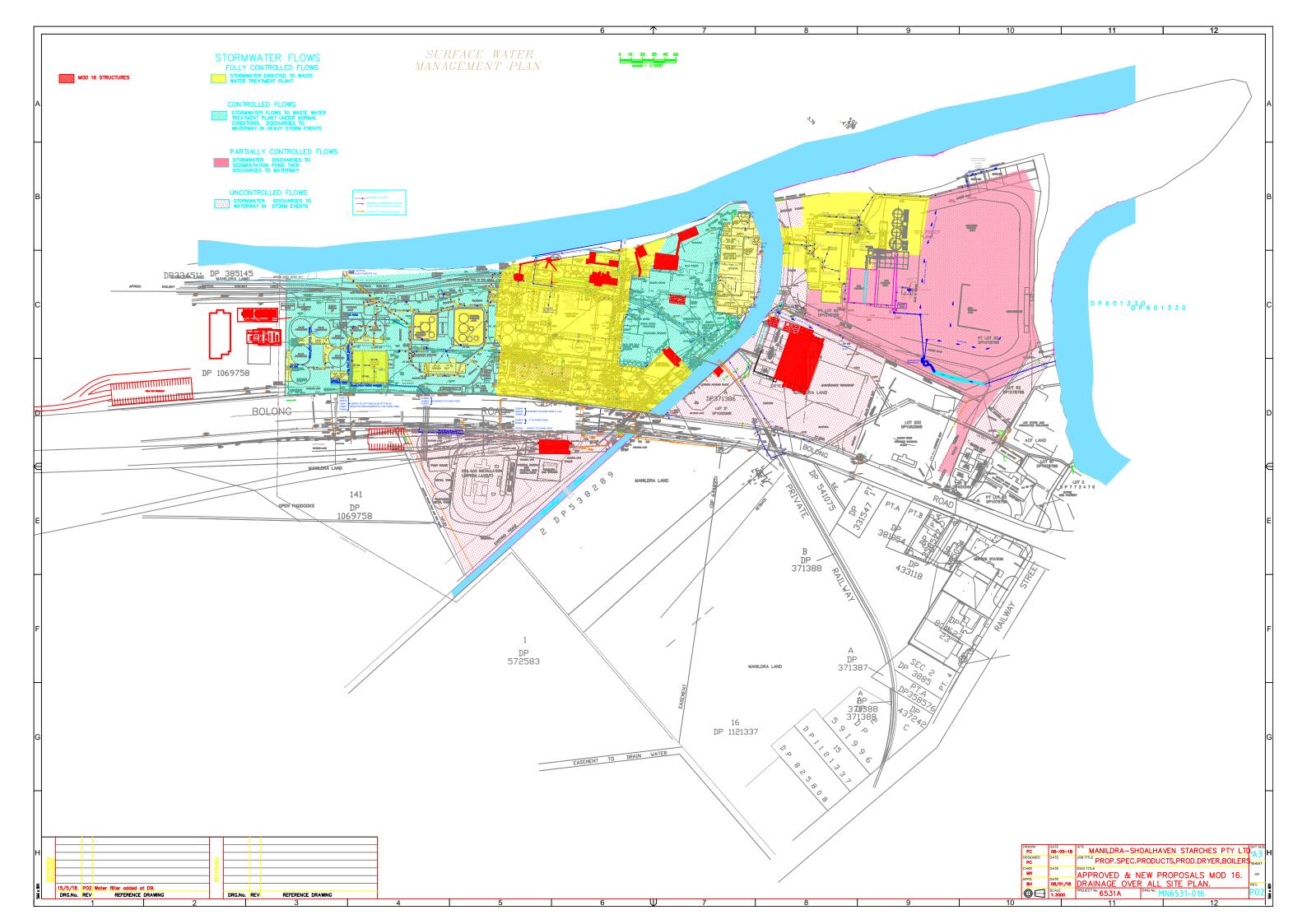
Appendix A to C show the calculations verifying that stormwater will be managed from the site such that there is no increase in discharge for up to the 1:100 ARI storm event.

APPENDIX A - Stormwater Management Plans ref N27563-401 to 402





APPENDIX B: MN6531-016 – MOD16 Drainage Over All Site plan



Appendix C - Stormwater Calculations - Rational Method and Pipe Size Calculations

allen price & scarratts land and development consultants **Allen Price & Scarratts**

Land & Development Consultants

75 Plunkett Street **NOWRA NSW 2541**

Client: **Shoalhaven Starches**

Project: **MOD 16 Application - Stormwater Management**

N27563 **Project No:** SCC Ref: N/A 07-Jun-18 Date:

Note: Bold cells only are inputs

Discharge

Block Size = **313** m2 % of area drained = 100% Area to drain = **313** m2 ARI = 10 years 5 mins tc =

C =**0.9** (from Table 5.1 DCP100 - 100% impervious) I = **194** mm/hr (from DCP100 IFD Table for Nowra)

Q =15.2 L/s (Q = CIA/360)

Pipe size to cater for design flow using Colebrook-White Method

Nominal Diameter = 150 mm

Actual ID = 0.151 m from table below (Hardies Design Manual, 1987 - page 2-51 for sewer pipe which is smaller ID than stormwater pipe)

0.003 mm for uPVC with chemically cemented joints (Hardies Design manual, 1987 - page 4-78) k =

1.14E-06 Water at 15 degrees C

V = 0.84 m/s $N_R =$ 1.12E+05 f = 0.01757

Sf = 0.0042 m/m ie at a grade of: 0.4 %

or 1 in: 236.6

Assume minimum grade (from site inspection) = 1 %

Therefore, DN 150 mm pipe at minimum grade of 0.4 % is adequate to cater for design flows of 15.2 L/s