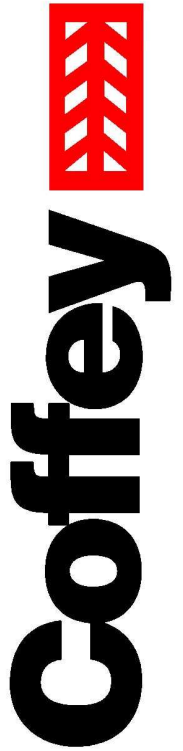


NSW DEPARTMENT OF COMMERCE
PRELIMINARY GEOTECHNICAL INVESTIGATION
AREAS B & D PRISON HOSPITAL
LONG BAY CORRECTIONAL COMPLEX, MALABAR

E12723.3-BC
4 July 2005



E12723.3-BC
4 July 2005

NSW Department of Commerce
Level 13, McKell Building
2-24 Rawson Place
SYDNEY NSW 2000

Attention: Mr Peter Shun

Dear Sir,

**RE: PRELIMINARY GEOTECHNICAL INVESTIGATION
AREAS B & D PRISON HOSPITAL
LONG BAY CORRECTIONAL COMPLEX, MALABAR**

Coffey Geosciences Pty Ltd (Coffey) is pleased to present the results of the geotechnical investigation undertaken for the proposed Prison Hospital and drainage works located in Areas B & D at Long Bay Correctional Facility, Malabar. If you wish to discuss any aspect of the report please contact Mr Peter Waddell or the undersigned on 9911 1000.

For and on behalf of

COFFEY GEOSCIENCES PTY LTD



MULIADI MERRY

Senior Geotechnical Engineer

Distribution:

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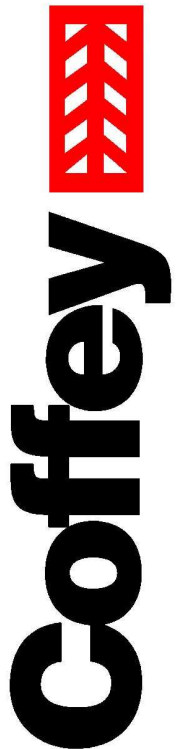


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Important Information About Your Coffey Report

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1. INTRODUCTION

This report details the results of the Stages 1 and 2 geotechnical investigations undertaken by Coffey Geosciences Pty Ltd (Coffey) for a proposed Prison Hospital development and drainage works in Areas B & D of the Long Bay Correctional Centre, Malabar.

The Stage 1 investigation work was undertaken for the NSW Department of Commerce generally in accordance with our proposal (Reference No: E12723/2-AE), dated 26 May 2004 while the Stage 2 investigation was carried out in general accordance with our proposal (Reference No: E12723/4-AE), dated 2 May 2005.

The current preliminary geotechnical investigation was undertaken concurrently with a Stage 2 Environmental Site Assessment, also by Coffey. For details of the environmental site assessment, reference should be made to the Stage 2 Environmental Site Assessment Report E12723.3- AS.

The objectives of the study were to provide information on subsurface conditions, a geotechnical model, discussion and recommendations on relevant geotechnical aspects including:

- A general description of the geology and geomorphology;
- Advice from local authorities on susceptibility to 100 year ARI flooding;
- Preliminary subsurface data;
- Suitability of excavated materials for re-use as fill for engineering and landscaping purposes;
- Preliminary recommendations on footing type, bearing capacities of foundation, earthworks and pavement design; and
- Advice on the need to line a stormwater detention basin.

Our overview of the relevant geotechnical investigation reports for Areas B and D that were prepared by others, is included in this report. Responses to Tasks 3, 4, 5, 6, 7 and 9 that were requested by the Contractors who are bidding for the construction works are also provided (Note that details of the tasks requested in presented in Section 6 of this report).

2. DETAILS OF PROPOSED DEVELOPMENT

It is understood that the new development will comprise:

- A new Long Bay Prison Hospital in Area B for the Department of Corrective Services (DCS). The hospital will comprise a mix of one and two storey buildings and yards set within a 6m high wall. The development will involve the relocation of the existing maintenance and storage facilities, which currently occupy the site of the proposed hospital.
- A new Forensic Hospital in Area D for the Corrections Health Services (CHS). The forensic hospital, like the prison hospital, will comprise a mix of one and two storey buildings and yards set within a 6m high wall. The development will also include a new building to house administration, stores and a pharmacy.
- A combined stormwater detention basin / playing field to be located in the southern part of Area D.
- Two stormwater detention basins in Area D.

- Construction of new roads and a car park.

At the time of the investigation and preparation of this report, the precise location and nature of the development had not been finalised. Area B covers an area of approximately 2.6Ha and Area D covers an area of approximately 7.9Ha. The location of these areas within the Long Bay Correctional Complex and a site locality plan is provided on Figure 1.

3. FIELD INVESTIGATIONS

3.1 Stage 1 Investigation

The Stage 1 field investigation was undertaken as a combined geotechnical and environmental assessment and comprised:

- Drilling of 17 boreholes (identified as CBH8, CBH25 to CBH32, CBH58 to CBH64 and CBH71) in Area B with the aid of a truck mounted drilling rig (see Figure 2);
- Collection of soil samples from eight hand auger holes (identified as HA14 to HA21) in Area D (see Figure 3);
- Drilling of 17 boreholes (identified as CBH9, CBH10, CBH12, CBH13 to CBH24, CBH65) in Area D with the aid of a truck mounted drilling rig; and
- Installation of groundwater wells into 5 of the boreholes (identified as CBH10/MW1, CBH11/MW2, CBH13/MW3, CBH14/MW4, CBH19/MW5) in Area D.

For the boreholes drilled using a truck mounted drill rig, Standard Penetration tests (SPT's) were undertaken at regular depth intervals to assess the engineering properties of the soils and to obtain samples. Disturbed auger samples were also obtained for laboratory testing purposes.

As part of the preliminary geotechnical investigation, coring of the bedrock was undertaken following the drilling, sampling and in-situ testing of the soil profile, in three of the boreholes. One of the geotechnical boreholes was located in Area B, with the other two located in Area D. The depth of the geotechnical boreholes ranged from 6m to 8.1m.

For the environmental assessment, the boreholes were generally taken 0.5m into natural soil or earlier V-bit refusal. Selected boreholes were extended to V-bit refusal to assess deeper subsurface condition and the presence of groundwater. These boreholes were also used in the geotechnical assessment. The depth of the environmental boreholes ranged from 0.1m to 6.5m.

The truck mounted drilling was undertaken between 25 October and 3 November 2004. The hand auger boreholes were drilled on 12 October 2004. The fieldwork was undertaken in the full time presence of a staff member from Coffey, who located the holes, nominated the sampling and testing and prepared field logs of the materials encountered. Ground surface levels at the test locations have been interpolated from survey data provided by NSW Department of Commerce.

Engineering borehole logs are included in Appendix A, along with Explanation Sheets defining the terms and symbols used in their preparation.

3.2 Stage 2 Investigation

The Stage 2 investigation was carried between 10 and 18 March 2005 and comprised the extension of four environmental borehole locations in Area B (refer to CBH76, CBH91, CBH94 and CBH122 on Figure 2) and

five locations in Area D (refer to CBH99, CBH100, CBH105, CBH108 and CBH109) to bedrock and the coring of the bedrock for approximately 2m to 3m. Geotechnical logging of the boreholes and rock cores obtained was subsequently carried out.

Drilling in soils was carried out using solid flight augers to the refusal depth of a tungsten carbide bit and then rock was cored using a triple tube core barrel.

Standard Penetration Tests were carried out at selected depth intervals as drilling progressed in soils for strength assessment and for obtaining samples for logging purposes.

Rock cores obtained from the borehole drilling were photographed and the photographs are with the borehole logs. Upon completion of the fieldwork the boreholes were backfilled with cuttings.

The field investigation was observed by a Coffey geotechnical engineer who located the boreholes by measuring from existing site features and logged the boreholes. Ground surface levels at the test locations have been interpolated from survey data provided by NSW Department of Commerce.

The engineering borehole logs of the boreholes are presented in Appendix A, along with Explanation Sheets defining the terms and symbols used in their preparation.

4. LABORATORY TESTING

Samples obtained during the Stage 1 investigation were taken to our NATA registered laboratory for testing. The following range of tests were undertaken:

- Soaked California bearing Ratio (CBR) – 2 tests
- Soil chemistry aggressivity (pH, sulphate and chloride) – 2 tests
- Soil pre-planting assessment suite of tests – 2 suites

The results of the laboratory testing are presented in Appendix C.

Representative rock core samples obtained during the borehole drilling in the Stage 2 Investigation were designated for Point Load Strength Index (PLI) testing for estimates of rock strength. The results of the PLI testing are presented in the borehole logs and in Appendix C.

5. OVERVIEW OF RELEVANT GEOTECHNICAL INFORMATION

Overview of the existing geotechnical investigation reports for Areas B and D that were prepared by others was carried out.

The relevant documents for Area B that were overviewed are as follows:

- Report dated 19 June 1987 on geotechnical investigation at Katingal area (Reference No: 5070JS);
- Report dated 25 August 1987 on further geotechnical investigations at the special purposes prison area (Reference No: 5212JS);
- Report dated January 1990 on geotechnical investigation at the industries building and warehouse area (Reference No: 89241);
- Report dated July 1986 on foundation investigation for Tower No. 10 (Reference No: L86016); and
- Report dated January 1981 on site investigation for Secure Oval (Reference No: 412 113).

The relevant documents for Area D that were overviewed include the following:

- Report dated 19 June 1987 on geotechnical investigation at Katingal area (Reference No: 5070JS);
- Report dated March 1986 on site investigation for the entrance roads and car park (Reference No: SSI/9508);
- Report dated 19 December 1991 on geotechnical investigations for the stormwater drainage works (Reference No: 01-GG982A);
- Report dated February 2002 on geotechnical investigation for Halfway House (Reference No: 02-GH22A);
- Report dated June 1981 on supplementary foundation investigation for the ward and administration blocks (Reference No: 31181);
- Report dated October 1979 on foundation investigation for the ward and administration blocks (Reference No: 39241-1); and
- Report dated December 1979 on supplementary foundation investigation for the ward and administration blocks.

Relevant geotechnical information from the reports was used in the preparation of this report as supplementary data to the Stages 1 and 2 investigations. Our overview results are presented in Section 7. Boreholes from previous investigations, by other consultants, were referenced in preparation of this report and copies of the engineering logs are provided in Appendix B.

6. TASKS REQUESTED BY PROPONENTS

The contractors who are bidding for the construction works at Areas B and B (referred to as “Proponents”) have requested a series of additional tasks to be carried out, as summarised below:

- Task 1. All environmental boreholes within the site boundary of the proposed Forensic Hospital to be assessed for geotechnical purposes as well.
- Task 2. A portion of samples to be analysed for PCBs in areas where TPH and PAHs have previously been found.
- Task 3. Provide recommendations on allowable bearing pressures of the soil layers above the bedrock and estimates of likely settlements.
- Task 4. Provide advice on depth and detail of engineered fill which may be used to bridge over filled ground and provide 100 kPa allowable bearing capacity for raft slabs / strip footings.
- Task 5. Provide CBR values in the areas of proposed roadways, together with recommendations on design parameters for rigid and flexible pavements.
- Task 6. Provide recommendations on design parameters for rigid and flexible retaining walls.
- Task 7. Provide recommendation on Earthquake Site Factor.
- Task 8. Provide recommendation on erosion potential soil parameters.
- Task 9. Carry out CBR tests at similar locations to those proposed for the boreholes and test pits over the whole site, including the site of the proposed Forensic Hospital.

Clarifications of Tasks 1 and 2 are provided as part of the Stage 2 Environmental Site Assessment (refer to our Report E12723.3- AS, dated 23 February 2005) and our responses to Tasks 3 to 9 (except Task 8) are provided in Sections 9.4 and 9.7 to 9.11. Response to Task 8 will be provided in an addendum letter upon the completion of the Emerson Class Number testing (which is still in progress at the time of the reporting).

7. RESULTS OF INVESTIGATION

7.1 Site Description

The site is located at Long Bay Correctional Complex, Malabar. Area B is located towards the south-eastern corner of the site. Area D is located near the south-western boundary of the site and includes the maximum security hospital and front of Long Bay Correctional Complex which faces Anzac Parade. These areas are identified in Figure 1.

The site generally slopes upwards towards the east, with Area D being the lowest lying part of the site. The level of the site at the site boundaries is typically consistent with the levels of surrounding land.

7.1.1 Area B

Area B is rectangular in shape with an area of approximately 2.6Ha. The main features of Area B at the time of the investigation are shown on Figure 2 and included:

- A workshop for maintenance of vehicles and plant in the centre of Area B.
- A plant nursery with a number of greenhouses in the south-eastern corner of Area B;
- A fenced area for the temporary storage of waste located between the maintenance stores and nursery;
- A number of concrete lined storage bays located towards the south-eastern corner of Area B for storage of dry material such as aggregate, sand, woodchip, bricks etc; and
- A number of other workshop / storage buildings.

7.1.2 Area D

Area D is irregular in shape with an area of approximately 7.9Ha. The main features of Area D at the time of the investigation are shown on Figure 3 and included:

- A playing field in the south eastern corner of Area D. This area is relatively flat and appears to be used as a soccer field;
- The Long Bay Correctional Centre Hospital, located to the north-west of the soccer field. The hospital comprises a number of hospital buildings, a paved recreation area and some landscaped areas. The area is surrounded by a wall; and
- The remainder of the Area D was vacant and grass covered. A vegetated fill stockpile is located to the immediate south-west of the hospital. The fill appeared to comprise soil and building rubble. A small pond is located to south-west of the stockpile. Some sandstone bedrock outcrops were observed near the pond.

7.2 Geological Setting

Reference to the Sydney 1:100,000 Geological Series Sheet 9130 (Edition 1) 1983, indicates that the site is underlain by Botany Basin Quaternary deposits and then Triassic Period Hawkesbury Sandstone. The quaternary deposits are described as comprising medium to fine grained sand with podsols. The Hawkesbury Sandstone generally comprises fine to coarse grained quartzose sandstone deposited in 1m to 3m thick beds and lenses. There are two dykes orientated west-northwest to east-southeast in the area. Previous geotechnical investigations carried out by others have identified the presence of the dykes within the site area.

7.3 Subsurface Conditions

For details of the conditions encountered in the boreholes, reference should be made to the engineering logs. Areas B and D are not adjacent and encountered differing subsurface conditions and are therefore discussed separately in the following sections.

7.3.1 Area B

Seventeen boreholes were carried out during the Stage 1 investigation within Area B, while another four cored boreholes were drilled during the Stage 2 investigation. Another twelve boreholes, from previous investigations by other consultants were also located within the area. Figure 2 shows the locations of the geotechnical tests carried out by Coffey and others in Area B.

In summary, the boreholes encountered pavement materials, fill, dune deposits, and minor depths of residual soils over sandstone bedrock. A summary of the Area B subsurface strata units encountered is provided in Table 1.

The top of sandstone levels is highest in the north-eastern corner at approximately RL42m AHD. The rock levels fall in a southerly direction to approximately RL39m AHD over a length of about 110m. A sharper drop-off in the rock levels was observed in a south-easterly direction which falls from RL42m AHD to RL39m AHD over a length of about 70m and in a south-westerly direction over a length of 50m. Inferred top of sandstone contours based on the borehole data are shown on Figure 2A. However, it should be noted that the sandstone bedrock surface could possibly include cliffs and benches.

TABLE 1: SUMMARY OF AREA B SUBSURFACE STRATA UNITS

Strata Unit	Description
Pavement Materials	A flexible pavement with total thickness was encountered in CBH8. No pavement materials were encountered in CBH25, CBH26, CBH32, CBH61 or CBH71. Concrete pavements 150mm to 200mm thick were encountered at the remaining borehole locations. Pavement was also observed at CBH76, CBH94 and CBH122. Ranges in thickness from 0.2m to 0.6m.
Fill	Sand, coke, building rubble, sandstone rubble, and gravely sand. Ranges in thickness from 0m to 2.1m.
Dune Deposits	Sand – fine to medium grained, loose to medium dense, becoming medium dense to dense near the soil / rock interface. Ranges in thickness from 0m to 2.4m.
Residual Soil	Silty clay – hard. Only encountered in two boreholes. Ranges in thickness from 0m to 1.5m.

TABLE 1: SUMMARY OF AREA B SUBSURFACE STRATA UNITS (CONTINUED)

Strata Unit	Description
Bedrock	<p>Sandstone was cored in CBH8, CBH76, CBH91, CBH94 and CBH122, and was inferred from V bit refusal in nine of the remaining sixteen current investigation boreholes.</p> <p>Encountered at depths ranging from 0.2m to 1.45m.</p> <p>The sandstone in CBH8 was low strength in the upper 1m and then high strength. The sandstone in the other cored boreholes (CBH76, CBH91, CBH94 and CBH122) was observed to be generally of medium strength with bands of low and high strength rock. In previous boreholes, the sandstone was highly to moderately weathered in the upper 0.4m to 2.5m of the strata unit.</p>

Our current investigation did not detect the presence of dykes on Area B; however, an igneous dyke was encountered during the previous site investigations carried out by other consultants at the special purposes prison area (refer to Report Reference No: 5212JS, dated 25 August 1987) and at Katingal Area (refer to Report Reference No: 5070S, dated 19 June 1987). The prison is located adjacent to Area B to the southwest and the Katingal area is located to the west. The dyke in the prison area was described to comprise medium to high plasticity, white clay, which extends to depths of up to 10.5m while the dyke in Katingal area was described to consist of medium plasticity, white to pale grey clay.

Groundwater was encountered within the boreholes during auger drilling at depths ranging from 0.8m to 2.4m below existing surface levels. The groundwater levels range from RL38.5 to RL41.5m AHD. Groundwater seepage was observed at RL38.9m AHD in CBH91.

7.3.2 Area D

Seventeen boreholes and eight hand auger holes were drilled within Area D during the Stage 1 investigation. Drilling of another five cored boreholes (refer to CBH99, CBH100, CBH105, CBH108 and CBH109 on Figure 3) was carried out during the Stage 2 investigation. Another eighty boreholes, from the previous investigations by other consultants were also located within the area. Figure 3 shows the locations of the geotechnical tests carried out by Coffey and others in Area D.

The results of the boreholes indicate the presence of an in-filled channel in the sandstone, commonly referred to as a Paleochannel. There are shallow depths to rock on either side of the Paleochannel with up to approximately 20m of fill, dune deposits and alluvium (along the southern area boundary) near the centre of the Paleochannel.

The sandstone levels range from approximately RL34m AHD away from the Paleochannel to approximately RL18m near the centre of the Paleochannel. Inferred top of sandstone contours are shown on Figure 3A.

A summary of the Area D subsurface strata units encountered is provided in Table 2.

TABLE 2: SUMMARY OF AREA D SUBSURFACE STRATA UNITS

Strata Unit	Description
Fill	<p>Clayey sand, building rubble, silty sand, sandstone rubble, ash, plastic, cloth, organics, grass, brick pats, furniture, sandstone boulders, concrete slabs, timber, bricks and metal pipes. Ranges in thickness from 0m to 7.5m.</p> <p>Described in parts as being very loose, uncompacted and with many voids. The greatest depth of fill was encountered along the alignment of the Paleochannel, and deepened towards the south.</p> <p>A layer of topsoil in the order of 0.1m thick was observed at CBH105 and CBH109.</p>
Dune Deposits	<p>Sand – fine to medium grained, loose to medium dense. Noted as being very loose above the water table, in parts.</p> <p>Contains some cemented sand layers, noted as being very dense, or described as “Waterloo rock”, “Coffee rock” or Indurated Sand.</p> <p>Ranges in thickness from 0m to 7.5m, with the greatest depths encountered along the Paleochannel alignment.</p>
Peat / Alluvium	<p>Sand, silty sand, sandy clay and silty clay with organics. With a variable consistency ranging from soft to hard and very loose to medium dense.</p> <p>With peat and clay layers.</p> <p>Ranges in thickness from 0m to 7.5m.</p>
Residual Soil	<p>Clay – stiff and very stiff, up to 3m thick. Only encountered at six locations in the current investigations and generally not differentiated from the alluvium on the previous investigation boreholes.</p>
Bedrock	<p>Sandstone was exposed on the surface in parts and was as deep as 20m or so near the alignment of the Paleochannel and the southern area boundary. The thickness of extremely to highly weathered sandstone varied from 0m in parts where exposed on the surface to greater than 5.5m.</p> <p>Medium to high strength, moderately weathered sandstone was noted in some of the cored boreholes below the upper extremely to highly weathered sandstone. Highly fractured to fragmented sandstone was observed in CBH99. Observations of the cores obtained from the Stage 2 investigation indicate that the strength of the sandstone is variable between the cored boreholes.</p> <p>Weaker and more fractured rock was observed in CBH99 and CBH105 within the Paleochannel while stronger rock was observed in the remaining cored boreholes.</p>

The current geotechnical investigations carried out by Coffey and the previous investigations by others have not detected any intrusion rock in Area D; however, this does not preclude the presence of dykes on the site area.

Groundwater within the boreholes during auger drilling in the current boreholes at depths ranging from 2.0m to 3.6m below existing surface levels, where encountered. The groundwater levels range from RL28.9m AHD to RL33.8m AHD. Groundwater seepage was observed at depths ranging from RL30m AHD in CBH99 to

RL31.7m AHD in CBH100.

8. LABORATORY TEST RESULTS

For details of the laboratory test results, reference should be made to the test results certificates provided in Appendix C. The results, excluding the pre-planting assessment are summarised in Table 3.

TABLE 3: SUMMARY OF LABORATORY TEST RESULTS

Borehole	Depth (m)	Sample Description	FMC (%)	SOMC (%)	SMDD (t/m ³)	CBR (%)	pH	Sulphate (% H ₂ O Sol)	Chloride
CBH8	0.6-1.6	(SP) Sand	5.6	11.0	1.81	20	-	-	-
CBH9	0.6-1.8	(SP) Sand	9.7	13.3	1.71	20	-	-	-
CBH8	1.0	(SP) Sand	-	-	-	-	5.5	0.002	0.001
CBH9	1.0	(SP) Sand	-	-	-	-	7.3	0.002	0.002

Note:

FMC Field Moisture Content

SOMC Optimum Moisture Content

SMDD Standard Maximum Dry Density

CBR California Bearing Ratio - 4 day soaked, 100%SMDD, SOMC, 4.5kg surcharge

pH Acidity

The pH results indicate that the soils are near neutral to slightly acidic. The sulphate and chloride test results indicate that the soil conditions are mild to non-aggressive to steel and concrete piles.

The results of the pre-planting assessment together with a summary and recommendations are provided on the report sheets by Sydney Environmental and Soil Laboratory, also included in Appendix C.

9. DISCUSSION AND RECOMMENDATIONS

9.1 Geological Model for Area B

Based on the results of the current preliminary investigation and the available boreholes from previous investigations, the geological model for Area B is presented in Table 4.

TABLE 4: GEOLOGICAL MODEL FOR AREA B

Strata Unit	Description	Approximate Unit Thickness (m)	Depth to Base of Unit (m)
1. Fill	Sand and rubble fill.	0 to 2.5	0 to 2.5
2. Dune Deposits	Sand – fine to medium grained, loose to medium dense, possible very loose and dense layers.	0 to 2.4	0 to 3

TABLE 4: GEOLOGICAL MODEL FOR AREA B (CONTINUED)

Strata Unit	Description	Approximate Unit Thickness (m)	Depth to Base of Unit (m)
3. Residual Soil	Clay – hard. Located in isolated pockets.	0 to 3	3.5
4. Bedrock	<p>Sandstone – variable degree of weathering and generally can be divided into two sub-units:</p> <ul style="list-style-type: none"> Unit 4A – Low to Medium Strength, Highly to Moderately Weathered, Highly Fractured Sandstone (Class IV Sandstone*) Unit 4B - Medium to High Strength, Moderately to Slightly Weathered, Slightly Fractured Sandstone (Class III Sandstone*) 	-	Drilled to depths of up to 6m

Note:

* Rock class assessed in accordance with Pells et al (1998) "Foundations on Sandstone and Shale in the Sydney Region" Aust. Geomech. Jnl. Dec 1998.

Groundwater level – at or near the soil / rock interface but subject to fluctuations in response to seasonal effects and periods of wet weather.

9.2 Geological Model for Area D

Based on the results of the current preliminary investigation and the available boreholes from previous investigations, the geological model for Area D is dominated by the presence of an in-filled Paleochannel.

During the formation of the Paleochannel, there may have been a series of water falls along the alignment and the sides of the Paleochannel could have been steep, with a series of ledges, overhangs and detached boulders. Coarse sand, cobbles and boulders may be expected at the base of former water falls. With a rise of water levels the water in the lower sections of the gorge would stagnate and vegetation would form and be deposited to form peat layers. Alluvium would also be deposited. Subsequently, wind blown dune deposits, which are present on the site, would have also been deposited over the peat and alluvial layers, further in-filling the Paleochannel. Finally, building rubble and general fill would have been placed to complete the filling of the Paleochannel and to provide a near level surface.

The geological model for Area D is provided in Table 5.

TABLE 5: GEOLOGICAL MODEL FOR AREA D

Strata Unit	Description	Approximate Unit Thickness (m)	Depth to Base of Unit (m)
1. Fill	Uncontrolled sand, building rubble and domestic waste. The greatest thickness would be expected along the alignment of the Paleochannel, which is shown on Figure 3A. Expected to be poorly compacted and highly variable in composition.	0 to 7.5	0 to 7.5

TABLE 5: GEOLOGICAL MODEL FOR AREA D (CONTINUED)

Strata Unit	Description	Approximate Unit Thickness (m)	Depth to Base of Unit (m)
2. Dune Deposits	Sand – fine to medium grained, loose to medium dense, possible very loose and dense layers.	0 to 7.5	0 to 12
3. Peat / Alluvium	Sand, silty sand, sandy clay and silty clay with organics. With a variable consistency ranging from soft to hard and very loose to medium dense. With peat and clay layers.	0 to 7.5	Up to >20
4. Residual Soil	Clay – stiff and very stiff.	0 to 0.9	Up to >20
5. Bedrock	<p>Sandstone – variable degree of weathering and strength and can be divided into three sub-units:</p> <ul style="list-style-type: none"> Unit 5A – Very Low to Low Strength, Highly Weathered, Fragmented to Highly Fractured Sandstone (Class V Sandstone*) Unit 5B – Low to Medium Strength, Moderately Weathered, Highly Fractured Sandstone (Class IV Sandstone*) Unit 5C – Medium to High Strength, Moderately to Slightly Weathered, Slightly Fractured Sandstone (Class III Sandstone*) 	-	Drilled to depths of up to 15.1m

Note:

* Rock class assessed in accordance with Pells et al (1998) "Foundations on Sandstone and Shale in the Sydney Region" Aust. Geomech. Jnl. Dec 1998.

Perched groundwater may be expected at or near the soil rock interface. The Paleochannel could act as a local drainage line and may be expected to fluctuate in response to seasonal effects and periods of wet weather. Within the Paleochannel water may be expected within 3m of the current ground surface.

9.3 Proposed Development

At the time of the investigation and preparation of this report, the final details and position of the developments were not known and hence preliminary comments are provided herein.

9.4 Earthworks

Recommendations related to treatment of existing fill provided in this section would not apply if a piled foundation were selected.

The fill and residual soil should be able to be excavated using hydraulic excavators and tracked loaders. Large hydraulic excavators with the assistance with rock breakers will be required to excavate the sandstone. If vibration sensitive structures are within close proximity to the excavations it may be necessary to restrict the use of rock breakers, or use rock saws or milling heads to limit vibrations.

The Proponents have requested that advice on depth and detail of engineered fill, which may be used to bridge over filled ground and is capable of supporting an allowable bearing pressure of 100kPa for shallow footings or rafts (refer to Task 4). Our response to this request is provided in the paragraph below. Should raft slabs / strip footings not be adopted then the recommendation provided below is not applicable. Furthermore, if the required bearing pressure is less than 100kPa then the thickness of compacted fill may be less than 2m.

Due to the variability and uncontrolled nature of the existing filling on the site, should shallow footings or rafts are considered, it is recommended that the fill below the proposed buildings in Area B be removed since the fill extends to relatively shallow depths. Fill, loose sediments and alluvium extend to considerable depth within the Paleochannel in Area D. It is not considered feasible to remove the entire fill from all locations. A compacted fill layer not less than 2m thick could be formed to bridge over filled ground in areas where fill exists at depth to support pavements and floor slabs. It may be difficult to compact the lower layers of fill, although we anticipate that these layers would eventually form a bridging layer, which would enable the placement and compaction of the subsequent fill layers in accordance with an engineering specification. The compaction specification should be met after placing not more than 0.5m thickness of fill. If excessive heaving occurs, a tensile geofabric may be required prior to placing the bridging material. In Area D, where present in shallow depths, the fill should be excavated and re-compacted or replaced.

The fill materials in Area B may generally be suitable for re-use as structural fill although unsuitable materials such as organics and oversize materials would require separation and removal prior to reuse. The use of Area B fill materials will also depend on its environmental classification.

The fill materials in Area D appear to contain a high proportion of unsuitable and oversize materials, which would require separation and removal prior to reuse. It is recommended that reference be made to the individual logs in areas where reuse of the fill is being considered. The use of Area D fill materials will also depend on its contamination classification.

Where natural soils are proposed to be exposed, subgrade preparation of natural soils for foundation or pavement construction should include the following:

- Strip all fill and stockpile suitable materials for re-use, if required. Proof roll the exposed surface of natural material. Excavate localised soft spots and replace with suitable fill, compacted to a minimum dry density ratio of 98% Standard Compaction at a moisture content within $\pm 2\%$ of Standard Optimum Moisture Content. If fill with low fines ($< 5\%$ silt and clay) is used it should be compacted to a minimum Density Index of 70%.
- Compact the exposed subgrade soils to a minimum dry density ratio of 98% Standard Compaction (or 70% Density Index).

In areas where fill is intended to be left in place (because removal of the fill is either not practicable or not desired by the proponent), other options could be considered such as partial removal of fill and replacement with compacted fill, modification of the in-situ soil or piled foundations. For the non-piling options consideration would need to be given to likely differential settlement.

Any new filling should be placed in layers (not exceeding 300 mm loose thickness) and compacted to a minimum dry density ratio of 98% Standard Compaction at a moisture content within $\pm 2\%$ of Optimum Moisture Content or 70% Density Index for low fines material.

9.5 Dewatering

Groundwater levels were observed in Area B at depths ranging from 0.8m to 2.4m below the existing site

grade while relatively deeper groundwater levels at depths ranging from 2m to 3.6m were observed in Area D. It is not known whether the proposed developments will involve any basement excavations. Dewatering may be able to be achieved using excavated sumps; however, the saturated sands may prove to be unstable and in relatively deep excavations, it may be necessary to use spears points instead of sumps and submersible pumps.

9.6 Slope Stability Issues

Permanent batter slope angles within controlled filling or natural sands should be no steeper than 2H:1V for batter heights up to 3m. Fill batters should be overfilled and trimmed back to profile. Soil batters should be vegetated (or covered by shotcrete, stone pitching or similar) to reduce the risk of erosion. Sandstone batters could be cut near vertical but should be inspected by a suitably qualified engineer to assess whether flatter angles or support measures such as shotcrete or rock bolts are required.

9.7 Retaining Walls

In Task 6, the Proponents have requested that recommendations on design parameters for rigid and flexible retaining walls be provided. Where retaining walls are required to be constructed, the structures could be designed assuming the preliminary geotechnical design parameters provided in Table 6.

TABLE 6: PRELIMINARY GEOTECHNICAL DESIGN PARAMETERS FOR RETAINING STRUCTURES

Strata Unit	Unit Weight, γ (kN/m ³)	Effective Cohesion, c' (kPa)	Friction Angle, ϕ' (degrees)	Elastic Modulus, E' (MPa)	Poisson's Ratio, ν	Active Earth Pressure Coefficient, K_a^1	'At Rest' Earth Pressure Coefficient ² , K_o
Compacted Fill and Residual Soils	20	0	32	20	0.35	0.3	0.5
Class V Sandstone*	22	30	35	100	0.3	0.27	0.5
Class IV Sandstone*	22	200	35	1,000	0.3	0.27 ³	- ³
Class III Sandstone*	24	500	35	4,000	0.2	0.27 ³	- ³

Note:

- * Rock class assessed in accordance with Pells et al (1998) "Foundations on Sandstone and Shale in the Sydney Region" Aust. Geomech. Jnl. Dec 1998.
- 1. Assume no wall friction.
- 2. Values provided assume a lateral movement of the wall of about 0.2% of the wall height is allowed to occur.
- 3. Values for better quality rock unit are dependent on the global effects of defects. Variability also occurs due to the in-situ stress environment and geometry of the excavation. For design parameters use should be made of the cohesion and friction angle parameters.

9.8 Foundations

9.8.1 Shallow Footings and Rafts

Our comments provided below serve as responses to Task 3 that was requested by the Proponents.

It is understood that the existing fill has not been placed in a controlled manner. Therefore, we consider that the fill will not be suitable as a bearing stratum to support the proposed building loads.

For lightly loaded structures, it may be possible to adopt shallow footings or a raft. However, with such foundations there is a greater potential for differential settlements, compared to rock foundations, which would require careful detailing of joints. To reduce the risk of adverse impacts from differential settlement it may be possible to adopt measures such as over-excavating the rock to a minimum of 0.6m below the footing level and replacing with compacted fill won from excavated sand or sandstone.

Shallow footings or edge and internal beams of a raft bearing on natural soils that are at least medium dense or compacted fill could be designed assuming an allowable bearing pressure of:

$$60B + 80D \text{ (kPa)}$$

Where:

- B is the footing width in metres and the minimum footing width is 0.4m
- D is the depth of embedment in metres and the minimum embedment is 0.3m.

An elastic modulus of 20MPa should be assumed for estimates of settlements in medium dense sand or compacted fill and a lower elastic modulus of 5MPa should be adopted for very loose to loose sand.

9.8.2 Footings to Rock

In areas of shallow sandstone, strip and pad footings or bored piles should be suitable to support structural loads. Open bored piles should be feasible, however, cohesionless sands could be encountered and/or seepage could occur and provision for measures such as temporary liners, dewatering and cleaning of open bored piers will be required. Alternatively, continuous flight auger (CFA) piles or driven piles could be adopted, which do not require cleaning and dewatering.

Within the vicinity of the Paleochannel in Area D, where compressible alluvium occurs at depth and/or uncontrolled fill is left in-situ, it is recommended that all floor slabs be suspended and that all loads be taken to the sandstone bedrock using either driven or CFA piles. Note that the depths to rock may be highly variable and possibly in excess of 20m in places. Consideration may be given to relocation of buildings proposed over the Paleochannel, particularly the deeper sections. Where piling foundation is not preferred then other options could be considered such as complete or partial removal of fill and replacement with compacted fill or modification of the in-situ soil. However, for the non-piling options consideration will need to be given to likely differential settlement.

Table 7 presents parameters for the preliminary design of footings bearing on controlled fill, residual soil and sandstone. Note that the parameters for controlled fill are applicable only when all underlying fill is first removed. If a shallow foundation system is to be adopted within variable foundation strata, consideration will need to be given to likely differential settlement. As a guide to likely settlements it could be assumed that footings founding in sands or fill (and not underlain by highly compressible soils), with a working load of 100kPa, will settle approximately 1% of the minimum footing dimension (eg approximately 10mm for a 1m wide footing) while footing founding on sandstone with working load of 100kPa would have negligible settlement.

TABLE 7: PRELIMINARY ALLOWABLE FOOTING DESIGN PARAMETERS

Geotechnical Unit	Rock Class	Allowable End Bearing Pressure (kPa) ⁽¹⁾	Allowable Shaft Adhesion (kPa) ⁽²⁾
Controlled Filling, Residual Sands	-	100	Nil
Sandstone	Class V	1,000	100
	Class IV	2,000	150
	Class III	3,500	350

Notes:

- (1) Allowable bearing pressures assume a minimum embedment of 0.3m into the relevant material. The recommended end bearing pressures should result in settlement of <1% of minimum footing dimension.
- (2) Shaft adhesion should only be assigned where piles are socketed at least 3 diameters into rock.

Where footings are designed to bear directly onto bedrock, the footing inspection and assessment requirements are provided in Table 8. Where the serviceability end-bearing pressure is greater than 2MPa, footing assessment should also include spoon testing (or cored boreholes) to assess whether defects below the base of the footing are within tolerable limits for the respective rock class.

TABLE 8: FOOTING INSPECTION AND ASSESSMENT REQUIREMENTS

Rock Unit	Testing Requirements
Class V and Class IV Sandstone Class III Sandstone with serviceability bearing pressures up to 2MPa	<ul style="list-style-type: none"> Visual inspection of pad footings Observation of piling and correlation with borehole information
Class III Sandstone with serviceability bearing pressures greater than 2MPa	<ul style="list-style-type: none"> Visual inspection; and Initial allowance for spoon testing 1/3 of all pad footings or coring of pier locations. May be reduced if consistent conditions are exposed in early testing or if rock below founding levels can be inspected in lift shaft excavations.

9.9 Aggression to Concrete and Steel

The results of the chemical testing on the samples from 1.0m depth in CBH8 and CBH9 and reference to Tables 6.1 and 6.3 of the Australian Standard AS 2159-1995 "Piling – Design and Installation" indicates the samples tested can be considered mild to non-aggressive to concrete and steel.

9.10 Pavements

This section serves to provide responses to Tasks 5 and 9 that were requested by the Proponents.

A subgrade CBR of 14% was recommended by others where the sands observed in the industries building and warehouse area forms the pavement subgrade (refer to Report Reference No: 89241, dated January 1990). The sands were described as fine to medium grained, brown to grey brown sand, classified as "SP" under the Unified Soil Classification System. An estimated CBR of 14% was provided for fine grained sands observed in the ward and administration blocks (refer to Report Reference No: 39241-1 dated October 1979), compacted to 100% Standard Maximum Dry Density Ratio (SMDD).

As part of the Stage 2 investigation, two sand samples from CBH8 in Area B and CBH9 in Area D were tested

for compaction and CBR. The sands observed in CBH8 and CBH9 were of fine to medium grained and grey brown in colour, containing minor low plasticity fines, also classified as “SP” under the USCS. The test results indicated that the sands have a soaked CBR value of 20%.

Our visual assessment of the available soil samples obtained from the Stages 1 and 2 investigations indicates that the natural sands are variable in nature. Variable particle sizes, colour and proportion of fines were observed in the sands. Due to the variability in the grain size and fines content in the sands, we would expect that the CBR of the sands to be variable. On the basis of our observations of the sand samples the available laboratory test results, we estimate that the CBR would range from 10% to 20%. Provided earthworks are undertaken as indicated in Section 9.4 of this report, pavements could be designed assuming a CBR value of 10%, subject to confirmation by a geotechnical engineer during construction.

9.11 Earthquake Loading Factors

This section is to respond to Task 7 that was requested by the Proponents.

Australian Standard AS 1170.4-1993, *Minimum Design Loads on Structures – Part 4: Earthquake Loads*, indicates the site lies within a region with a combined acceleration coefficient and site factor (a_S) of 0.08.

9.12 Soil Dispersion Potential

For assessment of soil dispersion potential, six disturbed samples from Area B and seven disturbed samples from Area D has been designated for Emerson Class Number testing. At the time of reporting, the testing is currently underway. An addendum letter will be issued to address Task 8 regarding soil dispersion potential.

9.13 Soil Pre-planting Assessment

For details of the results and recommendations for soil planting reference should be made to the soil pre-planting assessment results by Sydney Environmental and Soil Laboratory, which are provided in Appendix C. In summary the soils are suitable for planting provided treatment and additives as recommended on the test results sheets are followed.

10. LIMITATIONS AND FURTHER INVESTIGATION

The recommendations presented in this report are based on limited subsurface investigations. Ground conditions can change over relatively short distances and additional geotechnical advice may be required during construction to assess whether conditions are consistent with design assumptions. Coffey would be pleased to provide additional advice and construction stage services, if required.

The attached document entitled “Important Information about your Coffey Report” provides additional information on the uses and limitations of this report.

This report provides the results of a preliminary geotechnical investigation, which has been undertaken prior to the development location and details being finalised. Review of this report and further investigation is recommended once the final development details are known.



Complex subsurface geological conditions are present in Area D, related to the presence of a Paleochannel. Further investigation may be required depending on the proposed development, to assess piling conditions for driven or CFA piles, the quality of the sandstone, which is indicated to be highly variably, and to more accurately define the top of rock. Recommended additional investigation may include drilling and coring of the bedrock under proposed buildings, geophysical traverse lines to obtain continuous bedrock profiles along selected section lines.

For and on behalf of
COFFEY GEOSCIENCES PTY LTD



MULIADI MERRY
Senior Geotechnical Engineer

Information

Important information about your **Coffey** Report

As a client of Coffey you should know that site subsurface conditions cause more construction problems than any other factor. These notes have been prepared by Coffey to help you interpret and understand the limitations of your report.

Your report is based on project specific criteria

Your report has been developed on the basis of your unique project specific requirements as understood by Coffey and applies only to the site investigated. Project criteria typically include the general nature of the project; its size and configuration; the location of any structures on the site; other site improvements; the presence of underground utilities; and the additional risk imposed by scope-of-service limitations imposed by the client. Your report should not be used if there are any changes to the project without first asking Coffey to assess how factors that changed subsequent to the date of the report affect the report's recommendations. Coffey cannot accept responsibility for problems that may occur due to changed factors if they are not consulted.

Subsurface conditions can change

Subsurface conditions are created by natural processes and the activity of man. For example, water levels can vary with time, fill may be placed on a site and pollutants may migrate with time. Because a report is based on conditions which existed at the time of the subsurface exploration, decisions should not be based on a report whose adequacy may have been affected by time. Consult Coffey to be advised how time may have impacted on the project.

Interpretation of factual data

Site assessment identifies actual subsurface conditions only at those points where samples are taken and when they are taken. Data derived from literature and external data source review, sampling and subsequent laboratory testing are interpreted by geologists, engineers or scientists to provide an opinion about overall site conditions, their likely impact on the proposed development and recommended actions. Actual conditions may differ from those inferred to exist, because no professional, no matter how qualified, can reveal what is hidden by

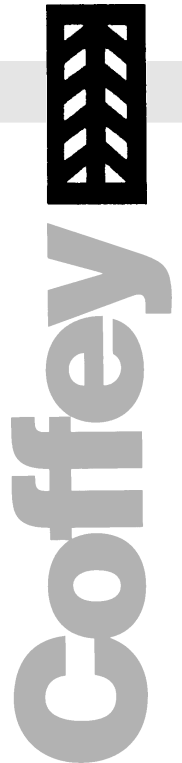
earth, rock and time. The actual interface between materials may be far more gradual or abrupt than assumed based on the facts obtained. Nothing can be done to change the actual site conditions which exist, but steps can be taken to reduce the impact of unexpected conditions. For this reason, owners should retain the services of Coffey through the development stage, to identify variances, conduct additional tests if required, and recommend solutions to problems encountered on site.

Your report will only give preliminary recommendations

Your report is based on the assumption that the site conditions as revealed through selective point sampling are indicative of actual conditions throughout an area. This assumption cannot be substantiated until project implementation has commenced and therefore your report recommendations can only be regarded as preliminary. Only Coffey, who prepared the report, is fully familiar with the background information needed to assess whether or not the report's recommendations are valid and whether or not changes should be considered as the project develops. If another party undertakes the implementation of the recommendations of this report there is a risk that the report will be misinterpreted and Coffey cannot be held responsible for such misinterpretation.

Your report is prepared for specific purposes and persons

To avoid misuse of the information contained in your report it is recommended that you confer with Coffey before passing your report on to another party who may not be familiar with the background and the purpose of the report. Your report should not be applied to any project other than that originally specified at the time the report was issued.



Important information about your **Coffey** Report



Interpretation by other design professionals

Costly problems can occur when other design professionals develop their plans based on misinterpretations of a report. To help avoid misinterpretations, retain Coffey to work with other project design professionals who are affected by the report. Have Coffey explain the report implications to design professionals affected by them and then review plans and specifications produced to see how they have incorporated the report findings.

Data should not be separated from the report*

The report as a whole presents the findings of the site assessment and the report should not be copied in part or altered in any way.

Logs, figures, drawings etc. are customarily included in our reports and are developed by scientists, engineers or geologists based on their interpretation of field logs (assembled by field personnel) and laboratory evaluation of field samples. These logs etc. should not under any circumstances be redrawn for inclusion in other documents or separated from the report in any way.

Geoenvironmental concerns are not at issue

Your report is not likely to relate any findings, conclusions, or recommendations about the potential for hazardous materials existing at the site unless specifically required to do so by the client. Specialist equipment, techniques, and personnel are used to perform a geoenvironmental assessment. Contamination can create major health, safety and environmental risks. If you have no information about the potential for your site to be contaminated or create an environmental hazard, you are advised to contact Coffey for information relating to geoenvironmental issues.

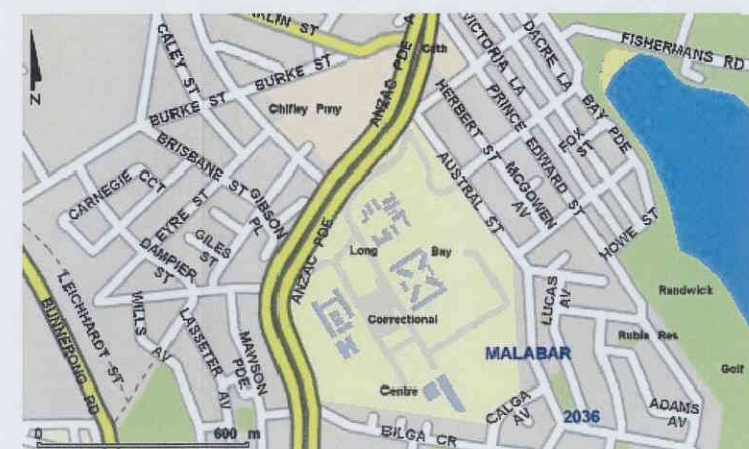
Rely on Coffey for additional assistance

Coffey is familiar with a variety of techniques and approaches that can be used to help reduce risks for all parties to a project, from design to construction. It is common that not all approaches will be necessarily dealt with in your site assessment report due to concepts proposed at that time. As the project progresses through design toward construction, speak with Coffey to develop alternative approaches to problems that may be of genuine benefit both in time and cost.

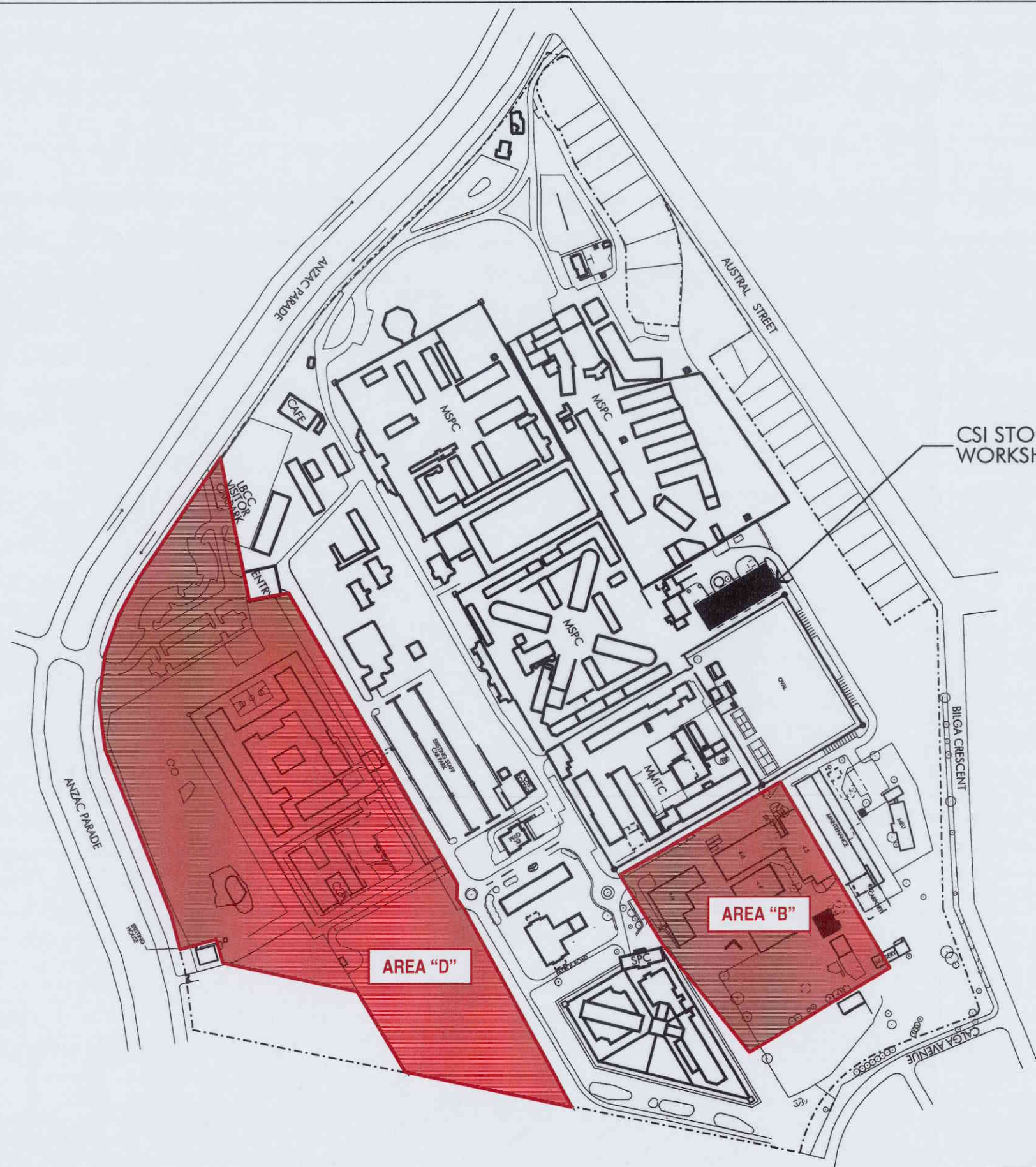
Responsibility

Reporting relies on interpretation of factual information based on judgement and opinion and has a level of uncertainty attached to it, which is far less exact than the design disciplines. This has often resulted in claims being lodged against consultants, which are unfounded. To help prevent this problem, a number of clauses have been developed for use in contracts, reports and other documents. Responsibility clauses do not transfer appropriate liabilities from Coffey to other parties but are included to identify where Coffey's responsibilities begin and end. Their use is intended to help all parties involved to recognise their individual responsibilities. Read all documents from Coffey closely and do not hesitate to ask any questions you may have.

** For further information on this aspect reference should be made to "Guidelines for the Provision of Geotechnical Information in Construction Contracts" published by the Institution of Engineers Australia, National Headquarters, Canberra, 1987.*



LOCATION PLAN



Coffey Geosciences Pty Ltd ACN 056 335 516		Geotechnical Resources Environmental Technical Project Management	
Drawn	DFD/SW	NSW DEPARTMENT OF COMMERCE PRELIMINARY GEOTECHNICAL INVESTIGATION LONG BAY CORRECTIONAL CENTRE SITE PLAN	FIGURE 1 Job no: E12723/3-BC
Approved	DFD		
Date	4/7/05		
Scale	1:4000		



LEGEND

- BOREHOLE LOCATIONS
- TOP OF SANDSTONE CONTOUR

PREVIOUS INVESTIGATIONS

- DPWS1 - REPORT 01-GG 82A (1-14)
- DPWS2 - REPORT 02 GH 22A
- JK1 - REPORT 5212JS & 5070JS 9-14, 101-149, 16 & 17
- JK2 - REPORT 5070JS 1-8, 15 & 19
- PWD1 - REPORT 89241
- PWD2 - REPORT L86016
- PWD3 - REPORT 89241
- PWD4 - REPORT 90202
- PWD5 - REPORT 89141
- WS1 - REPORT 81-M9 (12-14)
- WS2 - REPORT 81-M7
- DP - REPORT SSI/9508
- SP - REPORT 94-HS47
- GA1 - REPORT 39241 (1-24)
- GA2 - REPORT 31181 (25-35) AND TEST PITS (1-12)
- GA3 - REPORT 84627255 (1-11)

Coffey Geosciences Pty Ltd ACN 056 335 516

Geotechnical | Resources | Environmental | Technical | Project Management

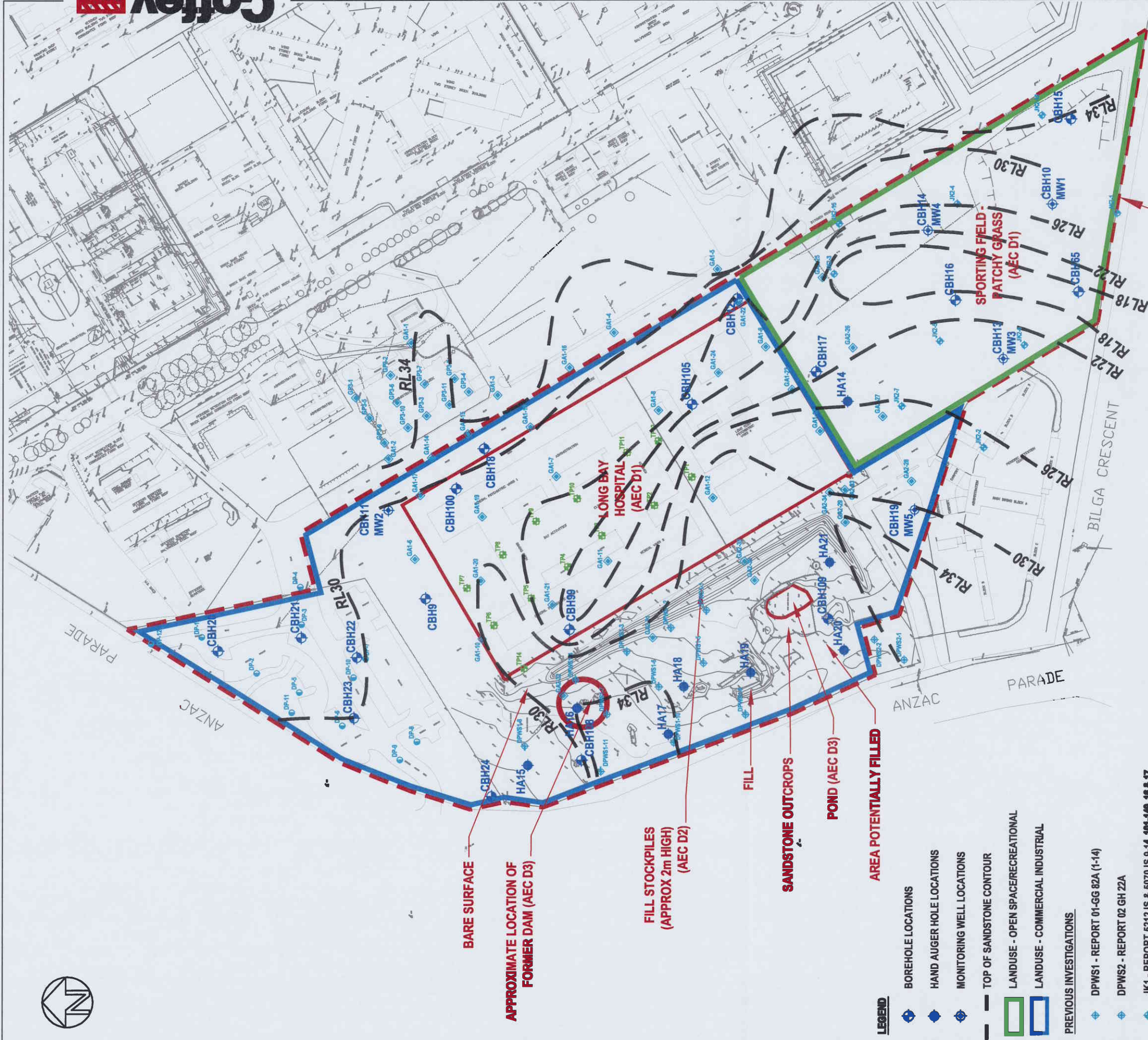
<div>100 0 50 Scale (metres)</div>	Revision	Description	Drawn	Approved	Date	Drawn	AJB/SW	DEPARTMENT OF COMMERCE PRELIMINARY GEOTECHNICAL INVESTIGATION LONG BAY CORRECTIONAL CENTRE AREA "B" INFERRED TOP OF SANDSTONE CONTOURS	FIGURE 2A Job no: E12723/3-BC
						Approved	DFD		
						Date	4/7/05		
						Scale	1:1000		



Scale (metres)

- ◆ GA2 - REPORT 31181 (25-35) AND TEST PITS (1-12)
- ◆ GA3 - REPORT 84627255 (1-11)

Job no: **E12723/3-BC**



AREA "D" BOUNDARY



Coffey Geosciences Pty Ltd			ACN 056 335 516		Geotechnical Resources Environmental Technical Project Management		
Drawn	AJB/ISW		DEPARTMENT OF COMMERCE PRELIMINARY GEOTECHNICAL INVESTIGATION LONG BAY CORRECTIONAL CENTRE AREA "D" INFERRED TOP OF SANDSTONE CONTOURS				
Approved	DFD						
Date	4/7/05						
Scale	1:2000						
					FIGURE 3A		
					Job no: E12723/3-BC		

APPENDIX A

ENGINEERING LOGS – AREA B

Soil Description

Explanation Sheet



Coffey

DEFINITION:

In engineering terms soil includes every type of uncemented or partially cemented inorganic or organic material found in the ground. In practice, if the material can be remoulded or disintegrated by hand in its field condition or in water it is described as a soil.

Other materials are described using rock description terms.

CLASSIFICATION SYMBOL & SOIL NAME

Soils are described in accordance with the Unified Soil Classification (USC) as shown in the table on the following page.

PARTICLE SIZE DESCRIPTIVE TERMS

NAME	SUBDIVISION	SIZE
Boulders		>200mm
Cobbles		63mm to 200mm
Gravel	coarse medium fine	20mm to 63mm 6mm to 20mm 2.36mm to 6mm
Sand	coarse medium fine	600 μ m to 2.36mm 200 μ m to 600 μ m 75 μ m to 200 μ m

MOISTURE CONDITION

- Dry** Looks and feels dry. Cohesive and cemented soils are hard, friable or powdery. Uncemented granular soils run freely through hands.
- Moist** Soil feels cool and darkened in colour. Cohesive soils can be moulded. Granular soils tend to cohere.
- Wet** As for moist but with free water forming on hands when handled.

CONSISTENCY OF COHESIVE SOILS

TERM	UNDRAINED STRENGTH su (kPa)	FIELD GUIDE
Very Soft	<12	A finger can be pushed well into the soil with little effort.
Soft	12 - 25	A finger can be pushed into the soil to about 25mm depth.
Firm	25 - 50	The soil can be indented about 5mm with the thumb, but not penetrated.
Stiff	50 - 100	The surface of the soil can be indented with the thumb, but not penetrated.
Very Stiff	100 - 200	The surface of the soil can be marked, but not indented with thumb pressure.
Hard	>200	The surface of the soil can be marked only with the thumbnail.
Friable	-	Crumbles or powders when scraped by thumb nail.

DENSITY OF GRANULAR SOILS

TERM	DENSITY INDEX (%)
Very Loose	Less than 15
Loose	15 - 35
Medium Dense	35 - 65
Dense	65 - 85
Very Dense	Greater than 85

MINOR COMPONENTS

TERM	ASSESSMENT GUIDE	PROPORTION OF MINOR COMPONENT IN:	
		Coarse grained	Fine grained
Trace of	Presence just detectable by feel or eye, but soil properties little or no different to general properties of primary component.	<5%	<15%
With some	Presence easily detected by feel or eye, soil properties little different to general properties of primary component.	5% - 12%	15% - 30%

SOIL STRUCTURE

ZONING		CEMENTING	
Layers	Continuous across exposure or sample	Weakly cemented	Easily broken up by hand in air or water
Lenses	Discontinuous layers of lenticular shape	Moderately cemented	Effort is required to break up the soil by hand in air or water
Pockets	Irregular inclusions of differential material		

GEOLOGICAL ORIGIN

WEATHERED IN PLACE SOILS

- Extremely weathered material Structure and fabric of parent rock visible
- Residual soil Structure and fabric of parent rock not visible

TRANSPORTED SOILS

- Aeolian soil Deposited by wind
- Alluvial soil Deposited by stream and rivers
- Colluvial soil Deposited on slopes (transported downslope by gravity)
- Fill Man made deposit. Fill may be significantly more variable between tested locations than naturally occurring soils.
- Lacustrine soil Deposited by lakes
- Marine soil Deposited in ocean basins, bays, beaches and estuaries



Explanation Sheet

SOIL CLASSIFICATION INCLUDING IDENTIFICATION AND DESCRIPTION

FIELD IDENTIFICATION PROCEDURES (Excluding particles larger than 60mm and basing fractions on estimated mass)					USC	PRIMARY NAME
COARSE GRAINED SOILS More than 50% of material less than 63mm is larger than 0.075mm	(A 0.075mm particle is about the smallest particle visible to the naked eye)	GRAVELS More than half of coarse fraction is larger than 2.0mm	CLEAN GRAVELS (Little or no fines)	Wide range in grain size and substantial amounts of all intermediate particle sizes.	GW	GRAVEL
				Predominantly one size or a range of sizes with more intermediate sizes missing.	GP	GRAVEL
			GRAVELS WITH FINES (Appreciable amount of fines)	Non-plastic fines (for identification procedures see ML below).	GM	SILTY GRAVEL
				Plastic fines (for identification procedures see CL below).	GC	CLAYEY GRAVEL
SANDS More than half of coarse fraction is smaller than 2.0mm		CLEAN SANDS (Little or no fines)	Wide range in grain sizes and substantial amounts of all intermediate sizes missing.	SW	SAND	
			Predominantly one size or a range of sizes with some intermediate sizes missing.	SP	SAND	
		SANDS WITH FINES (Appreciable amount of fines)	Non-plastic fines (for identification procedures see ML below).	SM	SILTY SAND	
			Plastic fines (for identification procedures see CL below).	SC	CLAYEY SAND	
FINE GRAINED SOILS More than 50% of material less than 63mm is smaller than 0.075mm		IDENTIFICATION PROCEDURES ON FRACTIONS <0.2mm				
	SILTS AND CLAYS Liquid limit less than 50	DRY STRENGTH	DILATANCY	TOUGHNESS		
		None to Low	Quick to slow	None	ML	SILT
		Medium to high	None	Medium	CL	CLAY
		Low to medium	Slow to very slow	Low	OL	ORGANIC SILT
	SILTS & CLAYS Liquid limit greater than 50	Low to medium	Slow to none	Low to medium	MH	SILT
		High	None	High	CH	CLAY
		Medium to high	None	Low to medium	OH	ORGANIC CLAY
HIGHLY ORGANIC SOILS		Readily identified by colour, odour, spongy feel and frequently by fibrous texture			Pt	PEAT
* Low plasticity - Liquid Limit W_L less than 35%. Medium plasticity - W_L between 35% and 50%.						

* Low plasticity - Liquid Limit W_L less than 35%. Medium plasticity - W_L between 35% and 50%.

COMMON DEFECTS IN SOIL

TERM	DEFINITION	DIAGRAM
PARTING	A surface or crack across which the soil has little or no tensile strength. Parallel or sub parallel to layering (eg bedding). May be open or closed.	
JOINT	A surface or crack across which the soil has little or no tensile strength but which is not parallel or sub parallel to layering. May be open or closed. The term 'fissure' may be used for irregular joints <0.2m in length	
SHEARED ZONE	Zone in clayey soil with roughly parallel near planar, curved or undulating boundaries containing closely spaced, smooth or slickensided, curved intersecting joints which divide the mass into lenticular or wedge shaped blocks.	
SHEARED SURFACE	A near planar curved or undulating, smooth, polished or slickensided surface in clayey soil. The polished or slickensided surface indicates that movement (in many cases very little) has occurred along the defect.	

TERM	DEFINITION	DIAGRAM
SOFTENED ZONE	A zone in clayey soil, usually adjacent to a defect in which the soil has a higher moisture content than elsewhere.	
TUBE	Tubular cavity. May occur singly or as one of a large number of separate or inter-connected tubes. Walls often coated with clay or strengthened by denser packing of grains. May contain organic matter.	
TUBE CAST	Roughly cylindrical elongated body of soil different from the soil mass in which it occurs. In some cases the soil which makes up the tube cast is cemented.	
INFILLED SEAM	Sheet or wall like body of soil substance or mass with roughly planar to irregular near parallel boundaries which cuts through a soil mass. Formed by infilling of open joints.	

Rock Description

Explanation Sheet



Coffey

AS1726-1993 – The descriptive terms used by Coffey are given below. They are broadly consistent with Australian Standard AS1726-1993.

DEFINITIONS:

Substance Rock substance, defect and mass are defined as follows:
Effectively homogeneous material, may be isotropic or anisotropic.
Defect Discontinuity or break in the continuity of a substance or substances.
Mass Any body of material which is not effectively homogeneous. It can consist of two or more substances without defects, or one or more substances with one or more defects.
In engineering terms rock substance is any naturally occurring aggregate of minerals and organic material which cannot be disintegrated or remoulded by hand in air or in water. Other material is described using soil descriptive terms.

SUBSTANCE DESCRIPTIVE TERMS:

ROCK NAME – Simple rock names are used rather than precise geological classification.

PARTICLE SIZE – Grain size terms for sandstone are:
Coarse grained 0.6mm to 2mm
Medium grained 0.2mm to 0.6mm
Fine grained 0.6mm (just visible) to 0.2mm

FABRIC – Terms for layering or penetrative fabric (eg. bedding, cleavage) are:

Massive No layering or penetrative fabric
Poorly developed Layering or fabric just visible. Little effect on properties.
Well developed Layering or fabric distinct. Rock breaks more easily parallel to layering or fabric.

ROCK SUBSTANCE STRENGTH TERMS

Term	Abbreviation	Point Load Index, I_{s50} (MPa)	Field Guide to Strength
------	--------------	-----------------------------------	-------------------------

Very Low	VL	Less than 0.1	Material crumbles under firm blows with sharp end of pick; can be peeled with knife; too hard to cut a triaxial sample by hand. Pieces up to 30mm thick can be broken by finger pressure.
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Low	L	0.1 to 0.3	Easily scored with a knife; indentations 1mm to 3mm show in the specimen with firm blows of the pick point; has dull sound under hammer. A piece of core 150mm long by 50mm diameter may be broken by hand. Sharp edges of core may be friable and break during handling.
------------	----------	------------	---

CLASSIFICATION OF WEATHERING PRODUCTS

Term	Abbreviation	Definition
Residual Soil	RS	Soil derived from the weathering of rock; the mass structure and substance fabric are no longer evident; there is a large change in volume but the soil has not been significantly transported.
Extremely Weathered	XW	Material is weathered to such an extent that it has soil properties, ie, it either disintegrates or can be remoulded, in water. Fabric of original rock still visible.
Distinctly Weathered	DW	Rock strength usually changed by weathering. The rock may be highly discoloured, usually by iron staining. Porosity may be increased by leaching, or may be decreased due to deposition of weathering products in pores.
Slightly Weathered	SW	Rock is slightly discoloured but shows little or no change of strength from fresh rock.
Fresh	FR	Rock shows no sign of decomposition or staining.

Medium	M	0.3 to 1	Readily scored with a knife; a piece of core 150mm long by 50mm diameter can be broken by hand with difficulty.
High	H	1 to 3	A piece of core 150mm long by 50mm diameter can not be broken by hand but can be broken by a pick with a single firm blow; rock rings under hammer.
Very High	VH	3 to 10	Hand specimen breaks with pick after more than one blow; rock rings under hammer.
Extremely High	EH	More than 10	Specimen requires many blows with geological pick to break through intact material; rock rings under hammer.

Notes:

1. In anisotropic rocks the field guide to strength applies to the strength perpendicular to the anisotropy. High strength anisotropic rocks may break readily parallel to the planar anisotropy.
2. The term extremely low is not used as a rock substance strength term. The term is used in AS1726-1993 but the field guide to strength makes it clear that it is a soil in engineering terms.
3. The unconfined compressive strength to isotropic rocks and anisotropic rocks which do not fail parallel to the planar anisotropy is typically 10 to 25 times the point load index. The ratio may vary for different rock types and lower strength rocks often have lower ratios than higher strength rocks.

Note: Where physical and chemical changes were caused by hot gases and liquids associated with igneous rocks the terms slightly altered (SA), distinctly altered (DA) and extremely altered (XA) may be used.



Rock Description Explanation Sheet

COMMON DEFECTS IN ROCK MASSES

Term	Definition	Diagram	Map Symbol	Graphic Log (Note 1)
Parting	A surface or crack across which the rock has little or no tensile strength. Parallel or sub parallel to layering (eg bedding) or a planar anisotropy in the rock substance (eg, cleavage). May be open or closed.			
Joint	A surface or crack across which the rock has little or no tensile strength but which is not parallel or sub parallel to layering or planar anisotropy in the rock substance. May be open or closed.			
Sheared Zone (Note 3)	Zone of rock substance with roughly parallel near planar, curved or undulating boundaries cut by closely spaced joints, sheared surfaces or other defects. Some of the defects are usually curved and intersect to divide the mass into lenticular or wedge shaped blocks.			
Sheared Surface (Note 3)	A near planar, curved or undulating surface which is usually smooth, polished or slickensided.			
Crushed Seam (Note 3)	Seam with roughly parallel almost planar boundaries, composed of disoriented, usually angular fragments of the host rock substance which may be more weathered than the host rock. The seam has soil properties.			
Infilled Seam	Seam of soil substance usually with distinct roughly parallel boundaries formed by the migration of soil into an open cavity or joint. Infilled seams less than 1mm thick may be described as veneer or coating on joint surface.			
Extremely Weathered Seam	Seam of soil substance, often with gradational boundaries. Formed by weathering of the rock substance in places.			

Notes on defects:

1. Borehole logs show the true dip of defects and face sketches and sections the apparent dip.
2. Partings and joints are not usually shown on the graphic log unless considered significant.
3. Sheared zones, sheared surfaces and crushed seams are faults in

geological terms.

DEFECT SHAPE TERMS

Planar	The defect does not vary in orientation
Curved	The defect has a gradual change in orientation
Undulating	The defect has a wavy surface
Stepped	The defect has one or more well defined steps
Irregular	The defect has many sharp changes in orientation

Note:

The assessment of defect shape is partly influenced by the scale of observation.

ROUGHNESS TERMS

Slickensided	Grooved or striated surface; usually polished
Polished	Shiny smooth surface
Smooth	Smooth to touch; few or no surface irregularities
Rough	Many small surface irregularities (amplitude generally less than 1mm); feels like fine to coarse sand paper
Very rough	Many large surface irregularities (amplitude generally more than 1mm); feels like, or coarser than, very coarse sand paper

COATING TERMS

Clean	No visible coating
Stained	No visible coating but surfaces are discoloured
Veneer	A visible coating of soil or mineral too thin to measure; may be patchy
Coating	A visible coating up to 1mm thick. Thicker soil material is described using appropriate defect terms (eg, infilled seam). Thicker rock strength material is usually described as a vein

BLOCK SHAPE TERMS

Blocky	Approximately equidimensional
Tabular	Thickness much less than length or width
Columnar	Height much greater than cross section

Engineering Log - Borehole

Client: **NSW Department Of Commerce**

Date started: **25.10.2004**

Principal: **NSW Department Of Corrective Services**

Date completed: **25.10.2004**Project: **Environmental/Geotechnical Investigation**

Logged by: **AD/SG**

Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**

Checked by: DFD

[illegible]

COFFEY



JOB NO

E12723/3

CBH 8

3.0 — 6.0 M



START
3.0

NO CORE
0-18m

3.30

4

5

END
6.0

E12723/1 CBH8



Engineering Log - Cored BoreholeSheet 2 of 2
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **25.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **25.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD/SG**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**Checked by: **DFD****Coffey**

drill model & mounting: P160 Truck				Easting:		slope: -90°		R.L. Surface: 41.9						
hole diameter: 100 mm Drilling fluid:				Northing:		bearing: -		datum: AHD						
drilling information				material substance				rock mass defects						
method	core-lift	water	RL	depth metres	graphic log core recovery	material rock type; grain characteristics, colour, structure, minor components	weathering alteration	estimated strength	Is ₍₅₀₎ MPa D- diam- etral A- axial	defect spacing mm	defect description type, inclination, planarity, roughness, coating, thickness			
								VL J M H VH EH		RD % 30 100 300 1000 3000	particular general			
			41	1										
			40	2										
			39	3		Continued from non-cored borehole								
						SANDSTONE: NO CORE:	HW							
			38	4		SANDSTONE: Medium grained, banded pale grey and orange brown, indistinct bedding at 20°.	HW MW		D A 0.17 0.27		SM, 10°, PL, RO, carbonaceous CO, 1mm.			
			37	5		SANDSTONE: Medium grained, pale grey banded grey and pale brown, distinctly cross bedding at 0° to 20°.			D A 1.51 2.07		SM, 0°, PL, sandy clay, 4mm.			
						Trace carbonaceous flecks.					PT, 0°-20°, PL, RO, clay VN.			
			36	6					D A 1.02 1.58		SM, 5°, PL, sandy clay, 3mm.			
						CBH8 terminated at 6m					SM, 10°, IR, carbonaceous, 4-2mm. PT, 0°, PL, carbonaceous CO, 1mm.			
			35	7										
			34	8										
method DT diatube AS auger screwing AD auger drilling RR roller/tricone CB claw or blade bit NMLC NMLC core NQ, HQ, PQ wireline core			core-lift casing used barrel withdrawn graphic log/core recovery core recovered - graphic symbols indicate material no core recovered			water 10/1/98 water level on date shown water inflow partial drill fluid loss complete drill fluid loss water pressure test result (lugeons) for depth interval shown			weathering FR fresh SW slightly weathered MW moderately weathered HW highly weathered XW extremely weathered DW distinctly weathered (covers MW and HW) strength VL very low L low M medium H high VH very high EH extremely high			defect type JT joint PT parting SM seam SZ sheared zone SS sheared surface CS crushed seam planarity PL planar CU curved UN undulating ST stepped IR irregular roughness VR very rough RO rough SO smooth SL slickensided coating CN clean SN stained VN veneer CO coating		

CORED BOREHOLE E12723.3.GPJ COFFEY.GDT 01.12.04

Form GEO 5.5 (Issue 3 Rev. 3)

Engineering Log - Borehole

Client: **NSW Department Of Commerce**
Principal: **NSW Department Of Corrective Services**
Project: **Environmental/Geotechnical Investigation**
Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**

drill model and mounting:		P160 Truck		Easting:		slope:		-90°		R.L. Surface:		40.8				
hole diameter:		100 mm		Northing		bearing:		-		datum:		AHD				
drilling information							material substance									
method	penetration			support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	pocket penetro- meter kPa	structure and additional observations	
	1	2	3													
V BIT				N		E					FILL: CLAYEY SAND: Fine to coarse grained, dark grey-black, low plasticity clay.	M			FILL	
					None observed	D	40.5	0.5								
						D	40.0									
						SPT 5,5,9 N*=14	39.5	1.0		SP	SAND: Fine to medium grained, pale grey-brown, low plasticity clay (<5%).		L			AEOLIAN DUNE OR BEACH DEPOSIT From 1-1.3m environmental sample.
						D		1.5		SP	SAND: Fine to medium grained, yellow-pale grey, low plasticity clay (<5%).		MD			From 2-2.2m environmental sample.
						D	39.0	2.0								
							38.5				Borehole CBH25 terminated at 2.25m					
							2.5									
							38.0									
							3.0									
							37.5									
							3.5									
							37.0									
							4.0									
method						support		notes, samples, tests				classification symbols and soil description			consistency/density index	
AS auger screwing*						M mud N nil		U ₆₀ undisturbed sample 50mm diameter				based on unified classification system			VS very soft	
AD auger drilling*						C casing		U ₆₃ undisturbed sample 63mm diameter							S soft	
RR roller/tricone						penetration		D disturbed sample							F firm	
W washbore						1 2 3 4		N standard penetration test (SPT)							St stiff	
CT cable tool								N* SPT - sample recovered							VSt very stiff	
HA hand auger								Nc SPT with solid cone							H hard	
DT diatube								V vane shear (kPa)							Fb friable	
B blank bit								P pressuremeter							VL very loose	
V V bit								Bs bulk sample							L loose	
T TC bit								E environmental sample							MD medium dense	
*bit shown by suffix e.g. ADT								R refusal							D dense	
															VD very dense	

Engineering Log - Borehole

Client: **NSW Department Of Commerce**
Principal: **NSW Department Of Corrective Services**
Project: **Environmental/Geotechnical Investigation**
Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**

drill model and mounting:		P160 Truck		Easting:		slope:		-90°		R.L. Surface:		39.9	
hole diameter:		100 mm		Northing		bearing:		-		datum:		AHD	
drilling information						material substance							
method	penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material	moisture condition	consistency/density index	pocket penetrometer	structure and additional observations
	1 2 3								soil type: plasticity or particle characteristics, colour, secondary and minor components.			100 200 300 400	
V BIT		N		E					FILL: CLAYEY SAND: Fine to coarse grained, dark grey, low plasticity clay.	M			FILL
		None observed		D	39.5	0.5							
				E	39.0	1.0		SP	SAND: Fine to medium grained, pale brown, low plasticity clay (<5%). Borehole CBH26 terminated at 1m	W	L		AEOLIAN DUNE OR BEACH DEPOSIT
					38.5	1.5							
					38.0	2.0							
					37.5	2.5							
					37.0	3.0							
					36.5	3.5							
					36.0	4.0							
method		support		notes, samples, tests		classification symbols and soil description		consistency/density index					
AS auger screwing*		M mud N nil		U ₅₀ undisturbed sample 50mm diameter		based on unified classification system		VS very soft					
AD auger drilling*		C casing		U ₆₃ undisturbed sample 63mm diameter				S soft					
RR roller/tricone		penetration		D disturbed sample				F firm					
W washbore		1 2 3 4		N standard penetration test (SPT)				St stiff					
CT cable tool		no resistance ranging to refusal		N* SPT - sample recovered				VSt very stiff					
HA hand auger		water		Nc SPT with solid cone				H hard					
DT diatube		10/1/98 water level on date shown		V vane shear (kPa)				Fb friable					
B blank bit		water inflow		P pressuremeter				VL very loose					
V V bit		water outflow		Bs bulk sample				L loose					
T TC bit				E environmental sample				MD medium dense					
*bit shown by suffix e.g. ADT				R refusal				D dense					
								VD very dense					

Engineering Log - BoreholeSheet 1 of 1
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **28.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **28.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**Checked by: **DFD**

drill model and mounting: P160 Truck		Easting:		slope: -90°		R.L. Surface: 41.5	
hole diameter: 100 mm		Northing		bearing: -		datum: AHD	
drilling information				material substance			
method	penetration 1 2 3	support	water	notes samples, tests, etc	RL	depth metres	structure and additional observations
DT		N					PAVEMENT MATERIAL
V BIT				E			FILL
				D	41.0	0.5	
				D	40.5	1.0	AEOLIAN DUNE OR BEACH DEPOSIT
				SPT 5,5,9 N*=14			From 1-1.3m environmental sample.
				D	40.0	1.5	
				E(D)			
					39.5	2.0	
					39.0	2.5	
					38.5	3.0	
					38.0	3.5	
					37.5	4.0	
method				support		notes, samples, tests	
AS auger screwing*				M mud		U ₅₀ undisturbed sample 50mm diameter	
AD auger drilling*				C casing		U ₆₃ undisturbed sample 63mm diameter	
RR roller/tricone						D disturbed sample	
W washbore						N standard penetration test (SPT)	
CT cable tool						N* SPT - sample recovered	
HA hand auger						Nc SPT with solid cone	
DT diatube						V vane shear (kPa)	
B blank bit						P pressuremeter	
V V bit						Bs bulk sample	
T TC bit						E environmental sample	
*bit shown by suffix						R refusal	
e.g. ADT							
				penetration 1 2 3 4			
				no resistance ranging to refusal			
				water			
				10/1/98 water level on date shown			
				water inflow			
				water outflow			
						classification symbols and soil description based on unified classification system	
						moisture	
						D dry	
						M moist	
						W wet	
						Wp plastic limit	
						WL liquid limit	
						consistency/density index	
						VS very soft	
						S soft	
						F firm	
						St stiff	
						VSt very stiff	
						H hard	
						Fb friable	
						VL very loose	
						L loose	
						MD medium dense	
						D dense	
						VD very dense	

BOREHOLE E12723.3.GPJ COFFEY.GDT 01.12.04

Form GEO 5.3 Issue 3 Rev.2

Engineering Log - Borehole

Client: **NSW Department Of Commerce**
Principal: **NSW Department Of Corrective Services**
Project: **Environmental/Geotechnical Investigation**
Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**

drill model and mounting:		P160 Truck		Easting:		slope:		-90°		R.L. Surface:		42.8																							
hole diameter:		100 mm		Northing		bearing:		-		datum:		AHD																							
drilling information						material substance																													
method		penetration		support		water		notes samples, tests, etc		RL		depth metres		graphic log		classification symbol		material		moisture condition		consistency/ density index		pocket penetro- meter kPa		structure and additional observations									
1 2 3																		soil type: plasticity or particle characteristics, colour, secondary and minor components.						100 200 300 400											
DT				N														CONCRETE: 150mm thick.								PAVEMENT MATERIALS									
V BIT								E		42.5								FILL: CLAYEY SAND: Fine to coarse grained, brown-orange brown, low plasticity clay. Traces of gravel fines (<5%).		M						FILL									
								D		0.5																									
										42.0						SP		SAND: Fine to medium grained, yellow and pale grey brown, low plasticity clay (<5%).				L				AEOLIAN DUNE OR BEACH DEPOSIT									
								SPT 5,5,7 N*=12		41.5																From 1-1.2m environmental sample.									
								D		1.5																									
										41.0																									
								D		2.0				CH				SILTY CLAY: Medium to high plasticity, pale grey-pale yellow, low liquid limit silt.				H				RESIDUAL									
										2.5																									
								SPT 25/100mm N*=R		40.0																From 2-2.6m environmental sample.									
								D		3.0																									
										39.5																									
								F		3.5				SM				SILTY CLAY: Medium to high plasticity clay, pale yellow to pale grey, low liquid limit silt. Borehole CBH28 terminated at 3.5m. Refusal on sandstone.				VSt													
										39.0																									
										4.0																									
method						support						notes, samples, tests						classification symbols and soil description						consistency/density index											
AS AD RR W CT HA DT B V T						auger screwing* auger drilling* roller/tricone washbore cable tool hand auger diatube blank bit V bit TC bit						M mud C casing penetration 1 2 3 4 no resistance ranging to refusal water 10/1/98 water level on date shown water inflow water outflow						U ₉₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal						based on unified classification system moisture D dry M moist W wet Wp plastic limit W _L liquid limit						VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense					
*bit shown by suffix e.g. ADT																																			

Engineering Log - Borehole

Client: **NSW Department Of Commerce**
Principal: **NSW Department Of Corrective Services**
Project: **Environmental/Geotechnical Investigation**
Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**

drill model and mounting: P160 Truck				Easting:		slope: -90°		R.L. Surface: 42.5							
hole diameter: 100 mm				Northing		bearing: -		datum: AHD							
drilling information				material substance											
method	penetration			support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	pocket penetro- meter kPa 100 200 300 400	structure and additional observations
	1	2	3												
DT				N							CONCRETE: 200mm thick.				PAVEMENT MATERIALS
V BIT						E					FILL: CLAYEY SAND: Fine to coarse grained, dark grey, low plasticity clay, waste building materials ie pieces crushed brick and gravel (<5%).	M			FILL
						D	42.0	0.5							
						D									
						D	41.5	1.0							
						SPT 5,7,10 N*=17									
						D				SP	SAND: Fine to medium grained, orange-brown and dark brown, low plasticity clay (<5%), mild hydrocarbon odour.	L			AEOLIAN DUNE OR BEACH DEPOSIT NB: Oil film on inner layer of bag. From 1-1.3m environmental sample.
							41.0	1.5			Borehole CBH29 terminated at 1.45m				
							40.5	2.0							
							40.0	2.5							
							39.5	3.0							
							39.0	3.5							
							38.5	4.0							
method				support		notes, samples, tests				classification symbols and soil description				consistency/density index	
AS AD RR W CT HA DT B V T *bit shown by suffix e.g. ADT				M mud C casing penetration 1 2 3 4 water 10/1/98 water level on date shown water inflow water outflow		N nil U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal				based on unified classification system moisture D dry M moist W wet Wp plastic limit W _L liquid limit				VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense	

Borehole No. **CBH30****Engineering Log - Borehole**Sheet 1 of 1
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **28.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **28.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**Checked by: **DFD**

drilling information				material substance							
method	penetration	support	notes	depth	graphic log	classification	material	moisture	consistency/	pocket	structure and
1 2 3			samples, tests, etc	metres		symbol	soil type: plasticity or particle characteristics, colour, secondary and minor components.	condition	density index	penetro- meter	additional observations
DT		N					CONCRETE: 150mm thick.	D			PAVEMENT MATERIAL
V BIT			E	41.0			FILL: CLAYEY SAND: Fine to coarse grained, dark grey-black, low plasticity clay.	M			FILL
			D	40.5							
			D	40.0		SP	SAND: Fine to medium grained, pale grey and dark grey, low plasticity clay (<5%).		MD		AEOLIAN DUNE OR BEACH DEPOSIT From 1-1.3m environmental sample.
			SPT 5,7,7 N*=14	40.0							
			D	39.5							
			E	2.0		SP	SAND: Fine to medium grained, pale grey and dark grey, low plasticity clay (<5%). Borehole CBH30 terminated at 2m	W			
				39.0							
				38.5							
				38.0							
				37.5							
				4.0							

method	support	notes, samples, tests	classification symbols and soil description	consistency/density index
AS AD RR W CT HA DT B V T *bit shown by suffix e.g. ADT	auger screwing* auger drilling* roller/tricone washbore cable tool hand auger diatube blank bit V bit TC bit	U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal	based on unified classification system moisture D dry M moist W wet Wp plastic limit WL liquid limit	VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense

BOREHOLE E12723.3.GPJ COFFEY.GDT 01.12.04

Form GEO 5.3 Issue 3 Rev.2

Borehole No. **CBH31****Engineering Log - Borehole**Sheet 1 of 1
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **28.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **28.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**Checked by: **DFD**

drilling information				material substance							
method	penetration	support	notes samples, tests, etc	depth metres	graphic log	classification symbol	material	moisture condition	consistency/density index	pocket penetrometer kPa	structure and additional observations
1	2	3		RL			soil type: plasticity or particle characteristics, colour, secondary and minor components.			100 200 300 400	
DT		N		42.5			CONCRETE: 200mm thick.				PAVEMENT MATERIAL
V BIT		None observed	E				FILL: CLAYEY SAND: Fine to coarse grained, brown, low plasticity clay.	M			FILL
			D	0.5							
			D	42.0		SP	SAND: Fine to medium grained, pale yellow-grey, low plasticity clay (<5%).		MD		AEOLIAN DUNE OR BEACH DEPOSIT
			D								
			E	1.0							
				41.5			Borehole CBH31 terminated at 1m. Refusal on sandstone.				
				1.5							
				41.0							
				2.0							
				40.5							
				2.5							
				40.0							
				3.0							
				39.5							
				3.5							
				39.0							
				4.0							

method	support	notes, samples, tests	classification symbols and soil description	consistency/density index
AS AD RR W CT HA DT B V T *bit shown by suffix e.g. ADT	M mud C casing penetration 1 2 3 4 no resistance ranging to refusal water 10/1/98 water level on date shown water inflow water outflow	U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal	based on unified classification system moisture D dry M moist W wet Wp plastic limit WL liquid limit	VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense

BOREHOLE E12723.3.GPJ COFFEY.GDT 01.12.04

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Engineering Log - Borehole

Sheet 1 of 1

Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **28.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **28.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**Checked by: **DFD**

drill model and mounting: P160 Truck Easting: slope: -90° R.L. Surface: 42.5
hole diameter: 100 mm Northing bearing: - datum: AHD

drilling information					material substance									
method	penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material	moisture condition	consistency/density index	pocket penetrometer	structure and additional observations	
TC Bit	1 2 3	N		E					FILL: CLAYEY SAND: Fine to coarse grained, pale yellow brown and grey, low plasticity clay (<5%).	M			FILL	
				D										
					42.0	0.5		SP	SAND: Fine to medium grained, pale grey-pale yellow, low plasticity clay (<5%).		MD		AEOLIAN DUNE OR BEACH DEPOSIT	
				D										
					41.5	1.0								
				E										
					41.0	1.5			Borehole CBH32 terminated at 1.2m. Refusal on sandstone.					
					40.5	2.0								
					40.0	2.5								
					39.5	3.0								
					39.0	3.5								
					38.5	4.0								

method	support	notes, samples, tests	classification symbols and soil description	consistency/density index
AS AD RR W CT HA DT B V T *bit shown by suffix e.g. ADT	auger screwing* auger drilling* roller/tricone washbore cable tool hand auger diatube blank bit V bit TC bit	M mud C casing penetration 1 2 3 4 no resistance ranging to refusal water 10/1/98 water level on date shown water inflow water outflow	undisturbed sample 50mm diameter undisturbed sample 63mm diameter disturbed sample standard penetration test (SPT) SPT - sample recovered SPT with solid cone vane shear (kPa) pressuremeter bulk sample environmental sample refusal	VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense
			moisture D dry M moist W wet Wp plastic limit WL liquid limit	

Borehole No. **CBH58****Engineering Log - Borehole**Sheet 1 of 1
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **1.11.2004**Principal: **NSW Department Of Corrective Services**Date completed: **1.11.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**Checked by: **DFD**


drill model and mounting: Mitsi Truck		Easting:		slope: -90°		R.L. Surface: 42.8													
hole diameter: 100 mm		Northing		bearing: -		datum: AHD													
drilling information				material substance															
method	penetration 1 2 3	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	pocket penetro- meter kPa 100 200 300 400	structure and additional observations						
DT		N	None observed						CONCRETE: 200mm thick.	D			PAVEMENT MATERIALS						
V BIT			None	E	42.5			SP	FILL: SAND: Fine to coarse grained, pale grey and pale brown, low plasticity clay (<5%). Traces of gravel fines (<5%). Borehole CBH58 terminated at 0.3m. Refusal at 0.3m on unknown fill. Maybe crushed concrete?	M			FILL						
						0.5													
						42.0													
						1.0													
						41.5													
						1.5													
						41.0													
						2.0													
						40.5													
						2.5													
						40.0													
						3.0													
						39.5													
						3.5													
						39.0													
						4.0													
method AS auger screwing* AD auger drilling* RR roller/tricone W washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT				support M mud N nil C casing penetration 1 2 3 4 no resistance ranging to refusal water 10/1/98 water level on date shown water inflow water outflow				notes, samples, tests U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal				classification symbols and soil description based on unified classification system moisture D dry M moist W wet Wp plastic limit WL liquid limit				consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense			

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Borehole No. **CBH59****Engineering Log - Borehole**Sheet 1 of 1
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **1.11.2004**Principal: **NSW Department Of Corrective Services**Date completed: **1.11.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**Checked by: **DFD****Coffey**

drill model and mounting: Mitsi Truck Easting: slope: -90° R.L. Surface: 42.8
 hole diameter: 100 mm Northing bearing: - datum: AHD





drilling information							material substance											
method	penetration			support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	pocket penetro- meter				structure and additional observations
	1	2	3											100 kPa	200 kPa	300 kPa	400 kPa	
DT				N	None observed						CONCRETE:						PAVEMENT MATERIALS	
							42.5	0.5			Borehole CBH59 terminated at 0.2m. TC bit refusal at 0.2m on sandstone.							
							42.0	1.0										
							41.5	1.5										
							41.0	2.0										
							40.5	2.5										
							40.0	3.0										
							39.5	3.5										
							39.0											
							4.0											

method AS auger screwing* AD auger drilling* RR roller/tricone W washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT		support M mud C casing penetration 1 2 3 4 no resistance ranging to refusal water 10/1/98 water level on date shown water inflow water outflow		notes, samples, tests U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal		classification symbols and soil description based on unified classification system moisture D dry M moist W wet Wp plastic limit W _L liquid limit		consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense	
--	--	--	--	--	--	---	--	---	--





Engineering Log - Borehole

Client: ***NSW Department Of Commerce***
Principal: ***NSW Department Of Corrective Services***
Project: ***Environmental/Geotechnical Investigation***

Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**

drill model and mounting:						Mitsi Truck	Easting:		slope:	-90°	R.L. Surface:		42.9						
hole diameter:						100 mm	Northing		bearing:	-	datum:		AHD						
drilling information							material substance												
method	penetration			support water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	pocket penetro- meter kPa 100 200 300 400	structure and additional observations					
	1	2	3																
DT				N						CONCRETE:					PAVEMENT MATERIALS				
				None observed						Borehole CBH60 terminated at 0.2m. TC bit refusal at 0.2m on sandstone.									
							42.5												
							0.5												
							42.0												
							1.0												
							41.5												
							1.5												
							41.0												
							2.0												
							40.5												
							2.5												
							40.0												
							3.0												
							39.5												
							3.5												
							39.0												
							4.0												
method AS auger screwing* AD auger drilling* RR roller/tricone W washbore CA cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT				support M mud N nil C casing penetration 1 2 3 4  no resistance ranging to refusal water  10/1/98 water level on date shown  water inflow  water outflow				notes, samples, tests U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal				classification symbols and soil description based on unified classification system moisture D dry M moist W wet Wp plastic limit WL liquid limit				consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense			

Borehole No. **CBH61****Engineering Log - Borehole**Sheet 1 of 1
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **1.11.2004**Principal: **NSW Department Of Corrective Services**Date completed: **1.11.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**Checked by: **DFD**

drill model and mounting: Mitsi Truck		Easting:		slope: -90°		R.L. Surface: 42.1			
hole diameter: 100 mm		Northing		bearing: -		datum: AHD			
drilling information				material substance					
method	penetration 1 2 3	support water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol		
TC Bit		N	E	42.0					
					0.5				
				41.5					
			E				SP		
				41.0	1.0				
					1.5				
				40.5					
					2.0				
				40.0					
					2.5				
				39.5					
					3.0				
				39.0					
					3.5				
				38.5					
					4.0				
method AS auger screwing* AD auger drilling* RR roller/tricone W washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT				support M mud C casing penetration 1 2 3 4  no resistance ranging to refusal water  10/1/98 water level on date shown  water inflow  water outflow		notes, samples, tests U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal		classification symbols and soil description based on unified classification system moisture D dry M moist W wet Wp plastic limit WL liquid limit	
consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense									

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Engineering Log - Borehole

Client: **NSW Department Of Commerce**
Principal: **NSW Department Of Corrective Services**
Project: **Environmental/Geotechnical Investigation**
Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**

[illegible]

Borehole No. **CBH63****Engineering Log - Borehole**Sheet 1 of 1
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **2.11.2004**Principal: **NSW Department Of Corrective Services**Date completed: **2.11.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**Checked by: **DFD**

drill model and mounting: Mitsi Truck		Easting:		slope: -90°		R.L. Surface: 42.7	
hole diameter: 100 mm		Northing		bearing: -		datum: AHD	
drilling information				material substance			
method	penetration 1 2 3	support	water	notes samples, tests, etc	RL	depth metres	material
DT		N	observed		42.5		CONCRETE:
V BIT		None observed					SANDSTONE: Borehole CBH63 terminated at 0.25m on sandstone (no sample collected).
						0.5	
						42.0	
						1.0	
						41.5	
						1.5	
						41.0	
						2.0	
						40.5	
						2.5	
						40.0	
						3.0	
						39.5	
						3.5	
						39.0	
						4.0	
method		support		notes, samples, tests		classification symbols and soil description	
AS auger screwing*		M mud N nil		U ₅₀ undisturbed sample 50mm diameter		moisture	
AD auger drilling*		C casing		U ₆₃ undisturbed sample 63mm diameter		D dry	
RR roller/tricone		penetration 1 2 3 4		D disturbed sample		M moist	
W washbore		no resistance ranging to refusal		N standard penetration test (SPT)		W wet	
CT cable tool		water		N* SPT - sample recovered		Wp plastic limit	
HA hand auger		10/1/98 water level on date shown		Nc SPT with solid cone		WL liquid limit	
DT diatube		water inflow		V vane shear (kPa)			
B blank bit		water outflow		P pressuremeter			
V V bit				Bs bulk sample			
T TC bit				E environmental sample			
*bit shown by suffix e.g. ADT				R refusal			
						consistency/density index	
						VS very soft	
						S soft	
						F firm	
						St stiff	
						VSt very stiff	
						H hard	
						Fb friable	
						VL very loose	
						L loose	
						MD medium dense	
						D dense	
						VD very dense	

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Borehole No. **CBH64****Engineering Log - Borehole**Sheet 1 of 1
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **2.11.2004**Principal: **NSW Department Of Corrective Services**Date completed: **2.11.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**Checked by: **DFD**

drill model and mounting: Mitsi Truck		Easting:		slope: -90°		R.L. Surface: 42.9							
hole diameter: 100 mm		Northing		bearing: -		datum: AHD							
drilling information				material substance									
method	penetration 1 2 3	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	pocket penetro- meter kPa 100 200 300 400	structure and additional observations
DT		N	None observed						CONCRETE:	D			PAVEMENT MATERIALS
V BIT			None observed	E	42.5				FILL: CLAYEY SAND: Fine to coarse grained, dark grey brown, low plasticity clay.	M			FILL
						0.5			Borehole CBH64 terminated at 0.4m. Refusal at 0.4m on sandstone.				
						42.0							
						1.0							
						41.5							
						1.5							
						41.0							
						2.0							
						40.5							
						2.5							
						40.0							
						3.0							
						39.5							
						3.5							
						39.0							
						4.0							
method AS auger screwing* AD auger drilling* RR roller/tricone W washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT		support M mud C casing penetration 1 2 3 4 no resistance ranging to refusal water 10/1/98 water level on date shown water inflow water outflow		notes, samples, tests U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal		classification symbols and soil description based on unified classification system moisture D dry M moist W wet Wp plastic limit W _L liquid limit		consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense					

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Borehole No. **CBH71****Engineering Log - Borehole**Sheet 1 of 1
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **3.11.2004**Principal: **NSW Department Of Corrective Services**Date completed: **3.11.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area B**Checked by: **DFD**

drill model and mounting:		Hand Auger		Easting:		slope: -90°		R.L. Surface:		40.1									
hole diameter:		100 mm		Northing		bearing: -		datum:		AHD									
drilling information						material substance													
method	penetration	support	water	notes samples, tests, etc	depth metres	graphic log	classification symbol	material	moisture condition	consistency/ density index	pocket penetro- meter kPa	structure and additional observations							
HA	1 2 3	N	None observed	E	40.0		SP	FILL: SANDY GRAVEL: Fine to medium grained, brown-pale grey, fine to coarse grained sand. SAND: Fine to medium grained, dark grey brown, low plasticity clay (<5%).	M	L		FILL AEOLIAN DUNE OR BEACH DEPOSIT							
				D															
				D	0.5		SP	SAND: Fine to medium grained, pale grey-pale brown, low plasticity clay (<5%).											
				E	39.5														
								Hand auger CBH71 terminated at 0.6m											
					1.0														
					39.0														
					1.5														
					38.5														
					2.0														
					38.0														
					2.5														
					37.5														
					3.0														
					37.0														
					3.5														
					36.5														
					4.0														
method AS auger screwing* AD auger drilling* RR roller/tricone W washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT				support M mud N nil C casing penetration 1 2 3 4  no resistance ranging to refusal water  10/1/98 water level on date shown  water inflow  water outflow				notes, samples, tests U ₆₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal				classification symbols and soil description based on unified classification system moisture D dry M moist W wet Wp plastic limit WL liquid limit				consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense			

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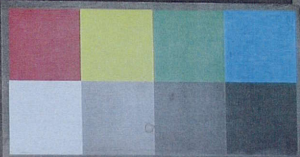
COFFEY




Job Number: E12723/4

CBH76

Depth: 0.79m to 3.95m



0	E12723/4 11-3-05	CBH76 0.79-3.95	START 0.95	B
1	B			B
2	B			B
3	B		F 3.95	



Borehole No. **CBH76**

Sheet 2 of 2
Office Job No.: **E12723/4**

Date started: **11.3.2005**Date completed: **11.3.2005**

Logged by: **SD/CW**

Checked by: **MM**

Engineering Log - Cored Borehole

Client: **Department of Commerce**

Principal: **Department of Corrective Services**

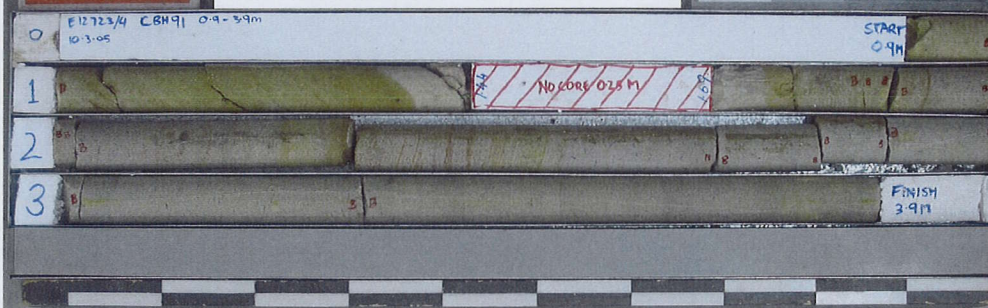
Project: **Environmental/Geotechnical Investigation**

Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area B.**

drill model & mounting: GEMCO TRUCK						Easting:		slope: -90°		R.L. Surface: 41.95									
hole diameter: 125 mm Drilling fluid:						Northing:		bearing:		datum: AHD									
drilling information						material substance						rock mass defects							
method	core-lift	water	RL	depth metres	graphic log core recovery	material rock type; grain characteristics, colour, structure, minor components	weathering alteration	estimated strength					Is ₆₀ MPa D- diam- etral A- axial		defect spacing mm		defect description type, inclination, planarity, roughness, coating, thickness		
								VL	L	M	H	VH	EH			RQD % 30 100 300 1000 3000		particular	general
	NONE OBSERVED		41	1	[dots]	Continued from non-cored borehole SANDSTONE: Coarse grained, pale grey and pale yellow brown, massive with some distinct laminations 0-20°.	MW/SW							D	A	0.77	0.78	JT, 20°, PL, RO, CN JT, 70°, PL, RO, CN	
			40	2	[dots]														
			39	3	[dots]	grading to medium grained, becoming grey, massive, trace of carbonaceous material.	FR							D	A	1.02	1.22	JT, 65°, IR, VRO, CN	
			38	4	[dots]	CBH76 terminated at 3.95m								D	A	0.76	0.85		
			37	5	[dots]														
			36	6	[dots]														
			35	7	[dots]														
			34	8	[dots]														
method DT diatube AS auger screwing AD auger drilling RR roller/tricone CB claw or blade bit NMLC NMLC core NQ, HQ, PQ wireline core			core-lift [casing symbol] casing used [barrel symbol] barrel withdrawn graphic log/core recovery [full bar] core recovered [dashed bar] - graphic symbols indicate material [empty bar] no core recovered			water [triangle up] 10/1/98 water level on date shown [triangle down] water inflow [inverted triangle] partial drill fluid loss [circle] complete drill fluid loss [vertical bar] water pressure test result (lugeons) for depth interval shown			weathering FR fresh SW slightly weathered MW moderately weathered HW highly weathered XW extremely weathered DW distinctly weathered (covers MW and HW) strength VL very low L low M medium H high VH very high EH extremely high			defect type JT joint PT parting SM seam SZ sheared zone SS sheared surface CS crushed seam planarity PL planar CU curved UN undulating ST stepped IR irregular			roughness VR very rough RO rough SO smooth SL slickensided coating CN clean SN stained VN veneer CO coating				



Job Number: E12723/4
CBH91
Depth: 0.9m to 3.9m



Borehole No. **CBH91****Engineering Log - Cored Borehole**Sheet 2 of 2
Office Job No.: **E12723/4**Client: **Department of Commerce**Date started: **10.3.2005**Principal: **Department of Corrective Services**Date completed: **10.3.2005**Project: **Environmental/Geotechnical Investigation**Logged by: **SD/SG**Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area B.**Checked by: **MM**

drill model & mounting: GEMCO TRUCK				Easting:		slope: -90°		R.L. Surface: 39.50			
hole diameter: 125 mm				Drilling fluid:		Northing:		bearing:			
drilling information				material substance				rock mass defects			
method	core-lift	water	RL	depth metres	graphic log core recovery	material rock type; grain characteristics, colour, structure, minor components	weathering alteration	estimated strength	Is ₍₅₀₎ MPa D- diam- etral A- axial	defect spacing mm	defect description type, inclination, planarity, roughness, coating, thickness
			39.0	0.5							
						Continued from non-cored borehole					
NMLC			38.5	1.0		SANDSTONE: Coarse grained, pale orange brown and pale grey, massive.	MW		D A 0.73 0.69		—JT, 40°, IR, RO, CN —JT, 50°, PL, RO, CN —JT, 50°, PL, RO, CN
			38.0	1.5		NO CORE: 1.44-1.69m.					
			37.5	2.0		SANDSTONE: Medium grained, pale grey banded pale orange brown, indistinctly laminated with some iron staining, 0-15 degrees, trace of carbonaceous flecks.	MW		D A 0.5 0.63		—SM, 0°, clayey sand, 30mm thick —PT, 5°, PL, RO, VN —PT, 5°, PL, RO, iron stained clay VN
			37.0	2.5		becoming pale grey and massive	SW				
			36.5	3.0					D A 0.44 0.59		
			36.0	3.5							
			35.5	4.0		CBH91 terminated at 3.9m			D A 0.21 0.13		

method	core-lift	water	weathering	defect type	roughness
DT diatube	10/1/98 water level on date shown	FR fresh	JT joint	VR very rough	
AS auger screwing		SW slightly weathered	PT parting	RO rough	
AD auger drilling		MW moderately weathered	SM seam	SO smooth	
RR roller/tricone		HW highly weathered	SZ sheared zone	SL slickensided	
CB claw or blade bit		XW extremely weathered	SS sheared surface		
NMLC NMLC core		DVW distinctly weathered (covers MW and HW)	CS crushed seam		
NQ, HQ, PQ wireline core					

planarity	coating
PL planar	CN clean
CU curved	SN stained
UN undulating	VN veneer
ST stepped	CO coating
IR irregular	

CORED BOREHOLE E12723.4 AREA B&D PHASE 2 UPDATED.GPJ COFFEY.GDT 27.06.05

Form GEO 5.5 Issue 3 Rev. 3

Borehole No. **CBH94****Engineering Log - Borehole**Sheet 1 of 1
Office Job No.: **E12723/4**Client: **Department of Commerce**Date started: **10.3.2005**Principal: **Department of Corrective Services**Date completed: **10.3.2005**Project: **Environmental/Geotechnical Investigation**Logged by: **SD**Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area B.**Checked by: **AJB**

drill model and mounting: MOLE 4WD		Easting:		slope: -90°		R.L. Surface: 40.50						
hole diameter: 125 mm		Northing		bearing:		datum: AHD						
drilling information				material substance								
method	penetration 1 2 3	support water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	pocket penetro- meter kPa 100 200 300 400	structure and additional observations
ADT		N						CONCRETE:				CONCRETE
		NONE OBSERVED			40		SM	SILTY SAND: Medium to coarse, black to dark grey, trace roots and organics.	M	VL		AEOLIAN SAND
			SPT 6.8, Hammer Bouncing N=3R		1		SP	SAND: Medium to coarse grained, pale brown to red-brown, poorly graded.		D-VD		
					39			Borehole CBH94 continued as cored hole				
					2							
					38							
					3							
					37							
					4							
					36							
					5							
					35							
					6							
					34							
					7							
					33							
					8							
method		support		notes, samples, tests		classification symbols and soil description based on unified classification system		consistency/density index				
AS auger screwing*		M mud N nil		U ₅₀ undisturbed sample 50mm diameter		moisture D dry M moist W wet Wp plastic limit W _L liquid limit		VS very soft				
AD auger drilling*		C casing		U ₆₃ undisturbed sample 63mm diameter				S soft				
RR roller/tricone		penetration 1 2 3 4		D disturbed sample				F firm				
W washbore		no resistance ranging to refusal		N standard penetration test (SPT)				St stiff				
CT cable tool		water		N+ SPT - sample recovered				VSt very stiff				
HA hand auger		10/1/98 water level on date shown		Nc SPT with solid cone				H hard				
DT diatube		water inflow		V vane shear (kPa)				Fb friable				
B blank bit		water outflow		P pressuremeter				VL very loose				
V V bit				Bs bulk sample				L loose				
T TC bit				E environmental sample				MD medium dense				
*bit shown by suffix e.g. ADT				R refusal				D dense				
								VD very dense				

BOREHOLE E12723.4 AREA B&D PHASE 2 UPDATED GPJ COFFEY GDT 30.06.05

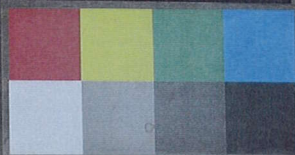
Form GEO 5.3 Issue 3 Rev.2



Job Number: E12723/4

CBH94

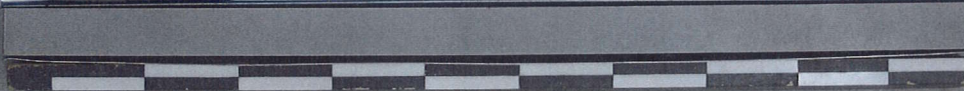
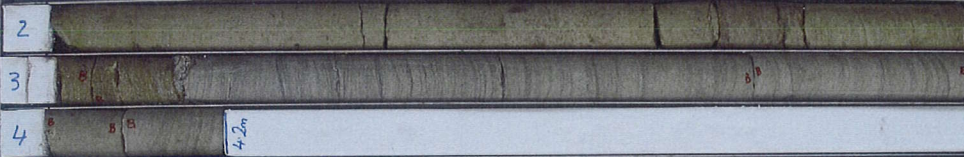
Depth: 1.2m to 4.2m



CBH94 17-4-19
E12723/4

START
1.2m

NO CORE 0.22m



Borehole No. **CBH94****Engineering Log - Cored Borehole**Sheet 1 of 1
Office Job No.: **E12723/4**Client: **Department of Commerce**Date started: **10.3.2005**Principal: **Department of Corrective Services**Date completed: **10.3.2005**Project: **Environmental/Geotechnical Investigation**Logged by: **SD**Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area B.**Checked by: **AJB****Coffey**

drill model & mounting: MOLE 4WD				Easting:		slope: -90°		R.L. Surface: 40.50						
hole diameter: 125 mm Drilling fluid:				Northing:		bearing:		datum: AHD						
drilling information				material substance				rock mass defects						
method	core-lift	water	RL	depth metres	graphic log core recovery	material rock type; grain characteristics, colour, structure, minor components	weathering alteration	estimated strength	Is ₍₅₀₎ MPa D- diam- etral A- axial	RQD %	defect spacing mm	defect description type, inclination, planarity, roughness, coating, thickness		
								VL L M H VH EH		30 100 300 1000 3000		particular general		
			40	1		Continued from non-cored borehole								
	NONE OBSERVED		39	2		NO CORE								
						SANDSTONE: Coarse grained, pale grey and pale yellow / brown, massive, with some dark grey siltstone clasts lenses.	MW		D A 0.32 0.75			PT, 15°, IR, VRO, Clay, VN JT, 40°, IR, VRO, Clay, VN PT, 10°, IR, VR, Clay VN XW, JM, 10°, IR, Siltstone PT, 15°, IR, RO, VN JT, 30°, IR, RO, VN XW, SM, 15-40°, IR, Siltstone		
			38	3		SANDSTONE: Coarse grained, pale grey with red / brown and orange / brown, indistinctly laminated 0-20° trace siltstone laminae.			D A 1.14 1.15	70		Closed JT, 15°, ST, Clay CO SM, 0°, PL, Sandy Clay, 5mm		
			37	4		SANDSTONE: Sandstone, medium grained, pale grey, distinctly laminated 0-20°, trace of carbonaceous laminae.	SW/Fr		D A 0.52 0.89					
						CBH94 terminated at 4.2m			D A 0.57 0.61					
			36	5										
			35	6										
			34	7										
			33	8										
method DT diatube AS auger screwing AD auger drilling RR roller/tricone CB claw or blade bit NMLC NMLC core NQ, HQ, PQ wireline core			core-lift casing used barrel withdrawn graphic log/core recovery core recovered graphic symbols indicate material no core recovered			water 10/1/98 water level on date shown water inflow partial drill fluid loss complete drill fluid loss water pressure test result (lugeons) for depth interval shown			weathering FR fresh SW slightly weathered MW moderately weathered HW highly weathered XW extremely weathered DW distinctly weathered (covers MW and HW) strength VL very low L low M medium H high VH very high EH extremely high			defect type JT joint PT parting SM seam SZ sheared zone SS sheared surface CS crushed seam planarity PL planar CU curved UN undulating ST stepped IR irregular roughness VR very rough RO rough SO smooth SL slickensided coating CN clean SN stained VN veneer CO coating		

CORED BOREHOLE E12723.4 AREA B&D PHASE 2 UPDATED.GPJ COFFEY.GDT 30.06.05

Form GEO 5.5 Issue 3 Rev. 3

Engineering Log - Borehole

Client: **Department of Commerce**

Principal: **Department of Corrective Services**

Project: **Environmental/Geotechnical Investigation**

Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area B.**

Date started: **11.3.2005**Date completed: **11.3.2005**

Logged by: **SG/SD**

Checked by: **MM**

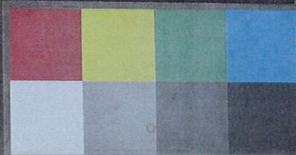
drilling information						material substance							
method	penetration 1 2 3	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	pocket penetro- meter KPa 100 200 300 400	structure and additional observations
ADT		N		E + DUPS					BASE COURSE: CONCRETE:				PAVEMENT
		NONE OBSERVED		SPT 2,3,4 N*=7	42	1		SP	SUB BASE COURSE: SAND: Medium to coarse grained, pale brown, quartz sand. SAND: Medium to coarse grained, yellow/brown, trace of silt. becoming pale brown with depth.	M	L		AEOLIAN SAND
									SANDSTONE: Soil strength, extremely weathered, medium to coarse grained, pale brown/grey and yellow.		VD		BEDROCK
					41	2			Borehole CBH122 continued as cored hole				
					40	3							
					39	4							
					38	5							
					37	6							
					36	7							
					35	8							
method AS auger screwing* AD auger drilling* RR roller/tricone W washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT				support M mud N nil C casing penetration no resistance ranging to refusal water 10/1/98 water level on date shown water inflow water outflow		notes, samples, tests U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal				classification symbols and soil description based on unified classification system moisture D dry M moist W wet Wp plastic limit WL liquid limit		consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense	



Job Number: E12723/4

CBH122

Depth: 1.35m to 4.35m



Borehole No. **CBH122****Engineering Log - Cored Borehole**Sheet 2 of 2
Office Job No.: **E12723/4**Client: **Department of Commerce**Date started: **11.3.2005**Principal: **Department of Corrective Services**Date completed: **11.3.2005**Project: **Environmental/Geotechnical Investigation**Logged by: **SG/SD**Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area B.**Checked by: **MM**

drill model & mounting: GEMCO TRUCK				Easting:		slope: -90°		R.L. Surface: 42.6					
hole diameter: 125 mm Drilling fluid:				Northing:		bearing:		datum: AHD					
drilling information				material substance				rock mass defects					
method	core-lift	water	RL	depth metres	graphic log core recovery	material	weathering alteration	estimated strength	IS ₍₉₀₎ MPa	D- diam- etral A- axial	RQD %	defect spacing mm	defect description
						rock type; grain characteristics, colour, structure, minor components		VL L M H VH EH			30 100 300 1000 3000		particular general
			42	1		Continued from non-cored borehole							
NMLC	NONE OBSERVED		41	2		NO CORE: 1.35-1.52m. SANDSTONE: Coarse grained, pale grey, massive.	SW			D A 0.39 0.3			—JT, 70°, PL, RO, CN. —PT, 0-15°, CU, RO, VN clay.
			40	3		becoming pale yellow with carbonaceous laminae	HW/MW SW/FR			D A 0.17 0.17	79		—PT, 10°, ST, RO, CN. —PT, 10°, IR, VRO, CO clay.
			39	4		SHALE: Dark grey SANDSTONE: Medium to coarse grained, banded pale grey and grey, distinctly laminated at 0 - 15 degrees, trace of carbonaceous laminae.				D A 1.11 0.94			JT, 90°, PL, RO, VN clay. PT, 0-15°, CU, RO, VN clay. PT, 0-15°, CU, RO, VN clay. JT, 70°, PL, RO, CN. PT, 0-15°, CU, RO, VN clay. PT, 0-15°, CU, RO, VN clay. PT, 0-15°, CU, RO, VN clay. PT, 0-15°, CU, RO, VN clay. PT, 0-15°, CU, RO, VN clay.
			38	5		CBH122 terminated at 4.35m				D A 0.63 0.65			PT, 0-15°, CU, RO, VN clay.
			37	6									
			36	7									
			35	8									

method DT diatube AS auger screwing AD auger drilling RR roller/tricone CB claw or blade bit NMLC NMLC core NQ, HQ, PQ wireline core	core-lift casing used barrel withdrawn graphic log/core recovery core recovered - graphic symbols indicate material no core recovered	water 10/1/98 water level on date shown water inflow partial drill fluid loss complete drill fluid loss water pressure test result (lugeons) for depth interval shown	weathering FR fresh SW slightly weathered MW moderately weathered HW highly weathered XW extremely weathered DWV distinctly weathered (covers MW and HW) strength VL very low L low M medium H high VH very high EH extremely high	defect type JT joint PT parting SM seam SZ sheared zone SS sheared surface CS crushed seam planarity PL planar CU curved UN undulating ST stepped IR irregular	roughness VR very rough RO rough SO smooth SL slickensided coating CN clean SN stained VN veneer CO coating
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CORED BOREHOLE E12723.4 AREA B&D PHASE 2 UPDATED.GPJ COFFEY.GDT 27.06.05

Form GEO 5.5 Issue 3 Rev. 3

APPENDIX A

ENGINEERING LOGS – AREA D

COFFEY



JOB NO E12723/3

CBH 9

5.10 — 8.09 M



E12723/3
CBH9

Sheet
5/10m

541

No Core
0.25m

546

6

8

No Core
0.18m

618m

7

8

80%
END



Engineering Log - Cored BoreholeSheet 2 of 2
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **25.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **25.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD/SG**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**Checked by: **DCD**

drill model & mounting: P160 Truck				Easting:		slope: -90°		R.L. Surface: 34.5				
hole diameter: 100 mm Drilling fluid:				Northing:		bearing: -		datum: AHD				
drilling information				material substance				rock mass defects				
method	core-lift	water	RL	depth metres	graphic log core recovery	material rock type; grain characteristics, colour, structure, minor components	weathering alteration	estimated strength VL L M H VH EH	IS ₍₅₀₎ MPa D- diam- etral A- axial	RQD % 30 100 300 1000 3000	defect spacing mm	defect description type, inclination, planarity, roughness, coating, thickness
												particular
			34	1								
			33	2								
			32	3								
			31	4								
			30	5								
						Continued from non-cored borehole						
			29	6		SANDSTONE: Medium grained, pale grey banded grey, indistinctly bedded. NO CORE:	MW					PT, 0°, IR, RO, sandy clay CO.
						SANDSTONE: Medium grained, pale grey banded grey, indistinctly bedded. NO CORE:	MW		D 0.64 A 0.7			SM, 0°-10°, IR, clayey sand, 20mm (XW).
			28	7		SANDSTONE: Medium grained, pale grey banded grey, indistinctly bedded.	MW					PT, 0°, IR, RO, VN.
						SANDSTONE: Medium grained, pale grey, indistinctly bedded to massive.			D 0.47 A 0.57			SM, 0°, PL, sandy clay, 10mm.
			27	8								SM, 0°, PL, sandy clay 20mm.
												JT, 70°, PL, VR, clayey sand CO (6.23-6.32m).
												Closed PT, 15°, PL.
												PT, 0°-20° PL, RO, sandy clay CO and VN.
			26	9		CBH9 terminated at 8.09m						





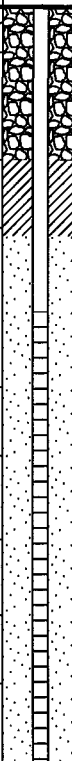


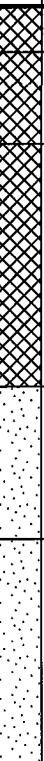
method DT diatube AS auger screwing AD auger drilling RR roller/tricone CB claw or blade bit NMLC NMLC core NQ, HQ, PQ wireline core	core-lift casing used barrel withdrawn graphic log/core recovery core recovered - graphic symbols indicate material no core recovered	water 10/1/98 water level on date shown water inflow partial drill fluid loss complete drill fluid loss water pressure test result (lugeons) for depth interval shown	weathering FR fresh SW slightly weathered MW moderately weathered HW highly weathered XW extremely weathered DW distinctly weathered (covers MW and HW) strength VL very low L low M medium H high VH very high EH extremely high	defect type JT joint PT parting SM seam SZ sheared zone SS sheared surface CS crushed seam planarity PL planar CU curved UN undulating ST stepped IR irregular roughness VR very rough RO rough SO smooth SL slickensided coating CN clean SN stained VN veneer CO coating
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CORED BOREHOLE E12723.3.GPJ COFFEY.GDT 01.12.04

Form GEO 5.5 Issue 3 Rev. 3


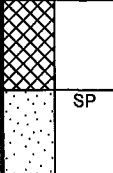




Engineering Log - Piezometer

Client: **NSW Department Of Commerce**
Principal: **NSW Department Of Corrective Services**
Project: **Environmental/Geotechnical Investigation**
Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**

drill model & mounting: P160 Truck										Easting:		slope: -90°		R.L. Surface: 35.7		
hole diameter: 100mm										Northing:		bearing: -		datum: AHD		
drilling information										material substance						
method	penetration			support	water	notes samples, tests, etc	well details	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	structure and additional observations	
	1	2	3													
V BIT				N		F						FILL: TOPSOIL: SILTY SAND: Fine to medium grained, dark grey-black, low liquid limit silt.	M		FILL	
						D										FILL: CLAYEY SAND: Fine to medium grained, dark grey and dark brown, low plasticity clay.
						D										FILL: CLAYEY SAND: Fine to medium grained, brown and grey and black and yellow, low plasticity clay.
						SPT 4,5,10 N*=15										
						D										
						SPT 4,4,9 N*=13										
						D										
						D										
						SPT 5,4,5 N*=9										
						D										
												Borehole CBH10 MWV1 terminated at 5m				





Engineering Log - Piezometer

Client: **NSW Department Of Commerce**
Principal: **NSW Department Of Corrective Services**
Project: **Environmental/Geotechnical Investigation**
Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**

drill model & mounting: P160 Truck				Easting:		slope: -90°		R.L. Surface: 35.7							
hole diameter: 100mm				Northing:		bearing: -		datum: AHD							
drilling information				material substance											
method	penetration 1 2 3			support	water	notes samples, tests, etc	well details	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	structure and additional observations
V BIT				N		E D D SPT 6,9,13 N*=22 D D SPT 3,4,6 N*=10 D D SPT 6,8,10 N*=18 D SPT 2,6,8 N*=14		35 34 33 32 31 30 29 28	1 2 3 4 5 6 7 8		SP SP SP SP SC	FILL: CLAYEY SAND: Fine to coarse grained, grey and brown, low plasticity clay, sandstone boulders up to 120mm (<5%). SAND: Fine to medium grained, pale yellow grey, low plasticity clay (<5%). SAND: Fine to medium grained, yellow and pale grey-white, low plasticity clay (<5%). SAND: Fine to medium grained, pale yellow grey, low plasticity clay (<5%). Sandy CLAY: Medium to high plasticity, grey, fine to medium sand. Borehole CBH11 MW2 terminated at 5.95m	M W M/W	 MD L MD St	FILL AEOLIAN DUNE OR BEACH DEPOSIT From 1-1.3m environmental sample. From 2.5-2.8m environmental sample.
method				support		notes, samples, tests		classification symbols and soil description		consistency/density Index					
AS auger screwing* AD auger drilling* RR roller/tricone W washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT				C casing N nil penetration 1 2 3 4  no resistance ranging to refusal water  10/1/98 water level on date shown  water inflow  water outflow		U ₅₀ undisturbed sample 50mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressure meter Bs bulk sample R refusal E environmental sample PID PID measurement WS water sample PZ piezometer		based on unified classification system moisture D dry M moist W wet Wp plastic limit W _L liquid limit		VS very soft S soft F firm St stiff VS _t very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense					

Engineering Log - Borehole

Client: **NSW Department Of Commerce**
Principal: **NSW Department Of Corrective Services**
Project: **Environmental/Geotechnical Investigation**
Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**

drill model and mounting:		P160 Truck		Easting:		slope:		-90°		R.L. Surface:		34.5	
hole diameter:		100 mm		Northing		bearing:		-		datum:		AHD	
drilling information										material substance			
method	penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	pocket penetro- meter kPa	structure and additional observations
	1 2 3												
VBIT		N		F					FILL: CLAYEY SAND: Fine to coarse grained, dark brown-black, low plasticity clay, waste building materials ie bricks, tiles, plastic.	M			FILL
				D		34		SP	SAND: Fine to medium grained, dark brown, low plasticity clay (<5%).		MD		AEOLIAN DUNE OR BEACH DEPOSIT
				D		1							
				SPT 5,8,10 N*=18		33							From 1-1.3m environmental sample.
				D		2		SP	SAND: Fine to medium grained, pale yellow grey, low plasticity clay (<5%).				
				D		32				W			From 2.5-2.6m environmental sample.
				SPT 25 for 100mm N*=R		3			V-bit refusal. Borehole CBH12 continued as cored hole				
						31							
						4							
						30							
						5							
						29							
						6							
						28							
						7							
						27							
						8							
method				support		notes, samples, tests		classification symbols and soil description		consistency/density index			
AS auger screwing* AD auger drilling* RR roller/tricone W washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT				M mud N nil C casing penetration 1 2 3 4  no resistance ranging to refusal water  10/1/98 water level on date shown  water inflow  water outflow		U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal		based on unified classification system moisture D dry M moist W wet Wp plastic limit W _L liquid limit		VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense			

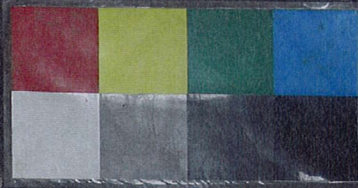
COFFEY



JOB NO E12723/3

CBH12

2.85—7.00 M



E12723/3

CBH12

2.85
START

3

327

NO CORE
1.85m

4

NO CORE.

5

512

6



Borehole No. **CBH12****Engineering Log - Cored Borehole**Sheet 2 of 2
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **26.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **26.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD/SG**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**Checked by: **DFD**

drilling information				material substance				rock mass defects				
method	core-lift	water	RL	depth metres	graphic log core recovery	material rock type; grain characteristics, colour, structure, minor components	weathering alteration	estimated strength	IS ₍₅₀₎ MPa D- diam- etral A- axial	RQD %	defect spacing mm	defect description type, inclination, planarity, roughness, coating, thickness
								VL L M H VH EH		30 100 300 1000 3000		particular
			34	1								
			33	2								
			32									
						Continued from non-cored borehole						
			31	3		SANDSTONE: Medium grained, pale brown.	XW		D A 0.04 0.04			PT, 0°, PL, RO.
				4		NO CORE:						
			30	5								
			29	6		SANDSTONE: Medium grained, pale brown.	XW					XW SM, IR, clayey sand, 40mm.
						SANDSTONE: Medium grained, pale grey, banded grey, indistinctly laminated 0°-20°.	MW		D A 0.42 0.68			XW SM, IR, clayey sand, 40mm. JT, 60°, PL, RO, CO. SM, IR, sandy clay 40mm.
			28	7					D A 0.44 0.28			PT, 10°, PL, RO, sandy clay CO. PT, 10°, IR, RO, carbonaceous VN. PT, 10°, PL, RO, carbonaceous CO. JT, 60°, IR, RO, CN (6.5-6.56m). JT, 60°, PL, RO, VN. JT, 30°, PL, RO, VN. JT, 60°, PL, RO, VN. PT, 15°, PL, RO, carbonaceous VN.
			27	8		CBH12 terminated at 7m						

method	core-lift	water	weathering	defect type	roughness
DT diatube	casing used	10/1/98 water level on date shown	FR fresh	JT joint	VR very rough
AS auger screwing	barrel withdrawn	water inflow	SW slightly weathered	PT parting	RO rough
AD auger drilling		partial drill fluid loss	MW moderately weathered	SM seam	SO smooth
RR roller/tricone		complete drill fluid loss	HW highly weathered	SZ sheared zone	SL slickensided
CB claw or blade bit			XW extremely weathered	SS sheared surface	
NMLC NMLC core			DW distinctly weathered (covers MW and HW)	CS crushed seam	
NQ, HQ, PQ wireline core					

graphic log/core recovery	strength	planarity	coating
core recovered	VL very low	PL planar	CN clean
- graphic symbols indicate material	L low	CU curved	SN stained
no core recovered	M medium	UN undulating	VN veneer
	H high	ST stepped	CO coating
	VH very high	IR irregular	
	EH extremely high		

CORED BOREHOLE E12723.3.GPJ COFFEY.GDT 01.12.04

Form GEO 5.5 Issue 3 Rev. 3

Engineering Log - Piezometer

Sheet 1 of 1

Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **26.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **26.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**Checked by: **DFD**

drill model & mounting: P160 Truck		Easting:		slope: -90°		R.L. Surface: 36.1	
hole diameter: 100mm		Northing:		bearing: -		datum: AHD	
drilling information				material substance			
method	penetration	support	water	notes samples, tests, etc	well details	RL	depth metres
V BIT	1 2 3	N		E		36	
				D			
				D			
				SPT 2,2,5 N*=7		35	1
				D			
				D		34	2
				D			
				SPT 25 for 100mm N*=R		33	3
				D			
				SPT 5,8,9 N*=17		32	4
				D			
				D		31	5
				SPT 3,4,6 N*=10		30	6
						29	7
							8
				Borehole CBH13 MW3 terminated at 5.95m			

method		support		notes, samples, tests		classification symbols and soil description		consistency/density index	
AS	auger screwing*	C	casing	U ₅₀	undisturbed sample 50mm diameter	based on unified classification system		VS	very soft
AD	auger drilling*	N	nil	D	disturbed sample			S	soft
RR	roller/tricone			N	standard penetration test (SPT)			F	firm
W	washbore			N*	SPT - sample recovered			St	stiff
CT	cable tool			Nc	SPT with solid cone			VSt	very stiff
HA	hand auger			V	vane shear (kPa)			H	hard
DT	diatube			P	pressure meter			Fb	friable
B	blank bit			Bs	bulk sample			VL	very loose
V	V bit			R	refusal			L	loose
T	TC bit			E	environmental sample			MD	medium dense
*bit shown by suffix				PID	PID measurement			D	dense
e.g. ADT				WS	water sample			VD	very dense
				PZ	piezometer				

penetration		water	
1	2	3	4
no resistance ranging to refusal		10/1/98 water level on date shown	
		water inflow	
		water outflow	

moisture	
D	dry
M	moist
W	wet
Wp	plastic limit
W _L	liquid limit

PIEZOMETER E12723.3.GPJ COFFEY.GDT 01.12.04

Form GEO 5.10 Issue 3 Rev.0

Engineering Log - Piezometer

Client: **NSW Department Of Commerce**
Principal: **NSW Department Of Corrective Services**
Project: **Environmental/Geotechnical Investigation**
Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**

drill model & mounting: P160 Truck										Easting:										slope: -90°										R.L. Surface: 34.5																																																																																																																																																																															
hole diameter:										Northing:										bearing: -										datum: AHD																																																																																																																																																																															
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Borehole No. **CBH15****Engineering Log - Borehole**Sheet 1 of 1
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **27.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **27.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**Checked by: **DFD**

drill model and mounting: P160 Truck Easting: slope: -90° R.L. Surface: 35.5
 hole diameter: 100 mm Northing bearing: - datum: AHD

drilling information						material substance									
method	penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material	moisture condition	consistency/ density index	pocket penetro- meter kPa	structure and additional observations		
V BIT	1 2 3	N		E					FILL: CLAYEY SAND: Fine to coarse grained, dark brown-black, low plasticity clay (<5%).	M			FILL		
				D	35.0	0.5									
				D	34.5	1.0		SP	SAND: Fine to medium grained, yellow brown and pale grey, low plasticity clay (<5%).		L		AEOLIAN DUNE OR BEACH DEPOSIT From 1-1.3m environmental sample.		
				SPT 2,3,2 N*=5											
				D	34.0	1.5									
					33.5	2.0			Borehole CBH15 terminated at 2m at v bit refusal.						
					33.0	2.5									
					32.5	3.0									
					32.0	3.5									
					31.5	4.0									

method	support	notes, samples, tests	classification symbols and soil description	consistency/density index
AS AD RR W CT HA DT B V T *bit shown by suffix e.g. ADT	auger screwing* auger drilling* roller/tricone washbore cable tool hand auger diatube blank bit V bit TC bit	M mud C casing penetration 1 2 3 4 no resistance ranging to refusal water 10/1/98 water level on date shown water inflow water outflow	U ₆₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal	VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense
			moisture D dry M moist W wet Wp plastic limit WL liquid limit	

BOREHOLE E12723.3.GPJ COFFEY.GDT 02.12.04

Form GEO.5.3 Issue 3 Rev.2

Borehole No. **CBH16****Engineering Log - Borehole**

Sheet 1 of 1

Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **27.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **27.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**Checked by: **DFD**

drill model and mounting: P160 Truck		Easting:		slope: -90°		R.L. Surface: 35.5	
hole diameter: 100 mm		Northing		bearing: -		datum: AHD	
drilling information				material substance			
method	penetration	support	water	notes samples, tests, etc	depth metres	graphic log	classification symbol
V BIT	1 2 3	N		E			
				D	35.0 0.5		
				SPT 4,5,7 N*=12	34.5 1.0		
				D	34.0 1.5		
				D	33.5 2.0		SP
				SPT 7,7,9 N*=16	33.0 2.5		
				D	32.5 3.0		
				D	32.0 3.5		SP
				E(D)	31.5 4.0		
Borehole CBH16 terminated at 3.9m							
method		support		notes, samples, tests		classification symbols and soil description based on unified classification system	
AS auger screwing*		M mud		U ₆₀ undisturbed sample 60mm diameter		moisture D dry M moist W wet W _p plastic limit W _L liquid limit	
AD auger drilling*		C casing		U ₆₃ undisturbed sample 63mm diameter			
RR roller/tricone				D disturbed sample		consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense	
W washbore				N standard penetration test (SPT)			
CT cable tool				N* SPT - sample recovered			
HA hand auger				Nc SPT with solid cone			
DT diatube				V vane shear (kPa)			
B blank bit				P pressuremeter			
V V bit				Bs bulk sample			
T TC bit				E environmental sample			
*bit shown by suffix e.g. ADT				R refusal			

BOREHOLE E12723.3.GPJ COFFEY.GDT 01.12.04

Form GEO 5.3 Issue 3 Rev 2

Engineering Log - BoreholeSheet 1 of 1
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **27.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **27.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**Checked by: **DFD**

drilling information				material substance									
method	penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	pocket penetro- meter kPa	structure and additional observations
1	2	3											
V BIT		N		E					FILL: CLAYEY SAND: Fine to coarse grained, dark grey and brown, low plasticity clay, waste building material ie crushed bricks and concrete, wire, steel (<5%).	M			FILL
				D	34	1							From 1-1.3m environmental sample.
				SPT 9,8,4 N*=12									
				D	33	2			FILL: CLAYEY SAND: Fine to coarse grained, mottled red brown, grey brown and black, low to medium plasticity clay.				
				D					FILL: CLAYEY SAND: Fine to coarse grained, mottled red, dark brown and orange brown, low to medium plasticity clay.				From 2.5-2.8m environmental sample.
				SPT 9,2,2 N*=4	32	3							
				D				SP	SAND: Fine to medium grained, grey-brown and dark grey, low to medium plasticity clay (<5%).		MD		AEOLIAN DUNE OR BEACH DEPOSIT
				D	31	4							From 4-4.3m environmental sample.
				SPT 6,6,9 N*=15									
				D	30	5		SP	SAND: Fine to medium grained, brown, low plasticity clay (<5%).		W		
					29	6							
					28	7			Borehole CBH17 terminated at 6.5m				
					27	8							

method	support	notes, samples, tests	classification symbols and soil description based on unified classification system	consistency/density index
AS auger screwing*	M mud	U ₅₀ undisturbed sample 50mm diameter		VS very soft
AD auger drilling*	C casing	U ₆₃ undisturbed sample 63mm diameter		S soft
RR roller/tricone		D disturbed sample		F firm
W washbore		N standard penetration test (SPT)		St stiff
CT cable tool		N* SPT - sample recovered		VS _t very stiff
HA hand auger		Nc SPT with solid cone		H hard
DT diatube		V vane shear (kPa)		Fb friable
B blank bit		P pressuremeter		VL very loose
V V bit		Bs bulk sample		L loose
T TC bit		E environmental sample		MD medium dense
*bit shown by suffix e.g. ADT		R refusal		D dense
				VD very dense

support	water
1 2 3 4	
no resistance ranging to refusal	
10/1/99 water level on date shown	
water inflow	
water outflow	

BOREHOLE E12723.3.GPJ COFFEY.GDT 01.12.04

Form GEO 5.3 Issue 3 Rev.2

Engineering Log - BoreholeSheet 1 of 1
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **27.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **27.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**Checked by: **DFD**

drill model and mounting: P160 Truck		Easting:		slope: -90°		R.L. Surface: 34.6	
hole diameter: 100 mm		Northing		bearing: -		datum: AHD	
drilling information				material substance			
method	penetration 1 2 3	support water	notes samples, tests, etc	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.
V BIT		N	E	34.5			FILL: CLAYEY SAND: Fine to coarse grained, grey pale brown, low plasticity clay.
			D	0.5			
			D	34.0			
			D	1.0			FILL: CLAYEY SAND: Fine to coarse grained, grey and pale brown with mottled red brown sandstone pieces up to 50mm, low plasticity clay.
			SPT 3,3,3 N*=6	33.5			
			D	1.5			
			D	33.0		SP	SAND: Fine to medium grained, grey brown and yellow, low plasticity clay (<5%).
			D	2.0			
			E	32.5			
				2.5			Borehole CBH18 terminated at 2.4m
				32.0			
				3.0			
				31.5			
				3.5			
				31.0			
				4.0			
method		support		notes, samples, tests		classification symbols and soil description	
AS auger screwing*		M mud N nil		U ₅₀ undisturbed sample 50mm diameter		based on unified classification system	
AD auger drilling*		C casing		U ₆₃ undisturbed sample 63mm diameter			
RR roller/tricone		penetration 1 2 3 4		D disturbed sample			
W washbore		no resistance ranging to refusal		N standard penetration test (SPT)			
CT cable tool				N* SPT - sample recovered			
HA hand auger				Nc SPT with solid cone			
DT diatube				V vane shear (kPa)			
B blank bit				P pressuremeter			
V V bit				Bs bulk sample			
T TC bit				E environmental sample			
*bit shown by suffix e.g. ADT				R refusal			
		water				moisture	
		10/1/98 water level on date shown				D dry	
		water inflow				M moist	
		water outflow				W wet	
						Wp plastic limit	
						W _L liquid limit	
						consistency/density index	
						VS very soft	
						S soft	
						F firm	
						St stiff	
						VSt very stiff	
						H hard	
						Fb friable	
						VL very loose	
						L loose	
						MD medium dense	
						D dense	
						VD very dense	

BOREHOLE E12723.3.GPJ COFFEY.GDT 01.12.04

Form GEO 5.3 Issue 3 Rev.2

Engineering Log - Piezometer

Sheet 1 of 2

Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **27.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **27.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**Checked by: **DFD**

drill model & mounting: P160 Truck		Easting:		slope: -90°		R.L. Surface: 35.4	
hole diameter: 100mm		Northing:		bearing: -		datum: AHD	
drilling information				material substance			
method	penetration	support	water	notes samples, tests, etc	well details	RL	depth metres
V BIT	1 2 3	N		E			
				D			0.5
				SPT 5.9,12 N*=21			1.0
				D			1.5
				D			2.0
				SPT 5.7,10 N*=17			2.5
				D			3.0
							3.5
							4.0
method AS auger screwing* AD auger drilling* RR roller/tricone W washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT				support C casing N nil penetration 1 2 3 4 no resistance ranging to refusal water 10/1/98 water level on date shown water inflow water outflow			
notes, samples, tests U ₅₀ undisturbed sample 50mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressure meter Bs bulk sample R refusal E environmental sample PID PID measurement WS water sample PZ piezometer				classification symbols and soil description based on unified classification system moisture D dry M moist W wet W _p plastic limit W _L liquid limit			
consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense							

PIEZOMETER E12723.3.GPJ COFFEY GDT 01.12.04

Form GEO 5.10 Issue 3 Rev.0

Engineering Log - PiezometerSheet 2 of 2
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **27.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **27.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**Checked by: **DFD****Coffey**

drill model & mounting: P160 Truck				Easting:		slope: -90°		R.L. Surface: 35.4							
hole diameter: 100mm				Northing:		bearing: -		datum: AHD							
drilling information					material substance										
method	penetration	support	water	notes samples, tests, etc	well details	RL	depth metres	graphic log	classification symbol	material	moisture condition	consistency/density index	structure and additional observations		
V BIT	1 2 3	N		SPT 4.9, 12 N*=21		31.0	4.5		SP	SAND: Fine to medium grained, pale grey brown and yellow brown, low plasticity clay (<5%). (continued)	W	MD	From 4-4.3m environmental sample.		
				D		30.5	5.0								
						30.0	5.5								
						29.5	6.0			Borehole CBH19 MW5 terminated at 5.5m					
						29.0	6.5								
						28.5	7.0								
						28.0	7.5								
						27.5	8.0								
method AS auger screwing* AD auger drilling* RR roller/tricone W washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT				support C casing N nil penetration 1 2 3 4 no resistance ranging to refusal water 10/1/98 water level on date shown water inflow water outflow				notes, samples, tests U ₅₀ undisturbed sample 50mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressure meter Bs bulk sample R refusal E environmental sample PID PID measurement WS water sample PZ piezometer				classification symbols and soil description based on unified classification system moisture D dry M moist W wet Wp plastic limit W _L liquid limit		consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense	

Engineering Log - BoreholeSheet 1 of 1
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **27.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **27.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**Checked by: **DED**

drilling information				material substance									
method	penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material	moisture condition	consistency/density index	pocket penetrometer kPa	structure and additional observations
1	2	3							soil type: plasticity or particle characteristics, colour, secondary and minor components.			100 200 300 400	
V BIT		N		E		35.0			FILL: CLAYEY SAND: Fine to coarse grained, grey brown and dark brown and black, low plasticity clay.	M			FILL
				D		0.5							
				D		34.5			FILL: SILTY SAND: Fine to coarse grained, dark brown-black, low liquid limit silt.				
				D		1.0							
				SPT 2,2,3 N*=5		34.0							From 1-1.3m environmental sample.
				D		1.5							
				D		33.5		SP	SAND: Fine to medium grained, pale yellow grey, low plasticity clay (<5%).		MD		AEOLIAN DUNE OR BEACH DEPOSIT
				D		2.0							
				D		33.0							
				D		2.5							
				SPT 4,7,9 N*=16		32.5							From 2.5-2.8m environmental sample.
				D		3.0							
				D		32.0							
						3.5			Borehole CBH20 terminated at 3.2m. Refusal at 3.2m on sandstone.				
						31.5							
						4.0							

method	support	notes, samples, tests	classification symbols and soil description	consistency/density index
AS AD RR W CT HA DT B V T *bit shown by suffix e.g. ADT	M mud C casing penetration 1 2 3 4 no resistance ranging to refusal water 10/1/98 water level on date shown water inflow water outflow	U ₆₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal	based on unified classification system moisture D dry M moist W wet Wp plastic limit WL liquid limit	VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense

BOREHOLE E12723.3.GPJ COFFEY.GDT 01.12.04

Form GEO 5.3 Issue 3 Rev.2

Engineering Log - BoreholeSheet 1 of 1
Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **27.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **27.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**Checked by: **DFD**

drilling information				material substance									
method	penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material	moisture condition	consistency/density index	pocket penetrometer	structure and additional observations
1	2	3							soil type: plasticity or particle characteristics, colour, secondary and minor components.			100 200 300 400	
V BIT		N		E	35.5				FILL: CLAYEY SAND: Fine to coarse grained, grey brown, low plasticity clay.	M			FILL
				D		0.5							
					35.0			SP	SAND: Fine to medium grained, grey and brown, low plasticity clay (<5%).		L		AEOLIAN DUNE OR BEACH DEPOSIT
				D		1.0							
				SPT 6,6,8 N*=14	34.5								From 1-1.3 environmental sample.
				D	34.0								
						2.0		SP	SAND: Fine to medium grained, yellow and pale grey, low plasticity clay (<5%).		MD		
				D	33.5								
						2.5							
				SPT 4,7,9 N*=16	33.0								From 2.5-2.8m environmental sample.
				D	32.5								
						3.0							
					32.0				Borehole CBH21 terminated at 3.25m. Refusal on sandstone at 3.25m.				
						3.5							
						4.0							

method	support	notes, samples, tests	classification symbols and soil description	consistency/density index
AS AD RR W CT HA DT B V T *bit shown by suffix e.g. ADT	M mud C casing penetration 1 2 3 4 no resistance ranging to refusal water 10/1/98 water level on date shown water inflow water outflow	U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal	based on unified classification system moisture D dry M moist W wet Wp plastic limit WL liquid limit	VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense

BOREHOLE E12723.3.GPJ COFFEY.GDT 01.12.04

Form GEO 5.3 Issue 3 Rev.2

Borehole No. **CBH23****Engineering Log - Borehole**

Sheet 1 of 1

Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **27.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **27.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**Checked by: **DSO**

drilling information				material substance								
method	penetration	support	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material	moisture condition	consistency/density index	pocket penetrometer kPa	structure and additional observations
1	2	3						soil type: plasticity or particle characteristics, colour, secondary and minor components.			100 200 300 400	
V BIT		N	E		32.5			FILL: CLAYEY SAND: Fine to coarse grained, orange brown, low plasticity clay.	M			FILL
			D		0.5							
			D		32.0							
			D		1.0			FILL: CLAYEY SAND: Fine to coarse grained, brown, low plasticity clay, waste building materials ie steel, plastics, concrete (<5%).				From 1-1.3m environmental sample.
			SPT 4,4,6 N*=10		31.5							
			D		1.5							
			D		31.0			FILL: CLAYEY SAND: Fine to coarse grained, dark brown and yellow, low plasticity clay, waste building materials ie conjute paperwork, wire, bricks (<5%).				
			D		2.0							
			SPT 19,18,25/100mm N*=R		30.0							
					3.0			Borehole CBH23 terminated at 2.7m. Refusal on sandstone.				
					29.5							
					3.5							
					29.0							
					4.0							

method	support	notes, samples, tests	classification symbols and soil description	consistency/density index
AS auger screwing*	M mud	U ₅₀ undisturbed sample 50mm diameter	based on unified classification system	VS very soft
AD auger drilling*	C casing	U ₆₃ undisturbed sample 63mm diameter		S soft
RR roller/tricone		D disturbed sample		F firm
W washbore		N standard penetration test (SPT)		St stiff
CT cable tool		N* SPT - sample recovered		VS _t very stiff
HA hand auger		Nc SPT with solid cone		H hard
DT diatube		V vane shear (kPa)		Fb friable
B blank bit		P pressuremeter		VL very loose
V V bit		Bs bulk sample		L loose
T TC bit		E environmental sample		MD medium dense
*bit shown by suffix		R refusal		D dense
e.g. ADT				VD very dense

BOREHOLE E12723.3.GPJ COFFEY.GDT 01.12.04

Form GEO 5.3 Issue 3 Rev.2

Engineering Log - Borehole

Client: **NSW Department Of Commerce**
Principal: **NSW Department Of Corrective Services**
Project: **Environmental/Geotechnical Investigation**
Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**

drill model and mounting:				P160 Truck		Easting:		slope:		-90°		R.L. Surface:		31.1	
hole diameter:				100 mm		Northing		bearing:		-		datum:		AHD	
drilling information						material substance									
method	penetration			support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	pocket penetro- meter kPa 100 200 300 400	structure and additional observations
	1	2	3												
V BIT			N		E	31.0			FILL: CLAYEY SAND: Fine to medium grained, yellow grey brown, low plasticity clay, waste building materials ie terracotta, bricks, wire, plastic, concrete.	M					FILL
					D	0.5									
						30.5									
					D	1.0									
						30.0									
					SPT 2,2,3 N*=5	1.5									
D	29.5	SC	SAND: Fine to medium grained, grey brown, low plasticity clay (<5%).	MD			AEOLIAN DUNE OR BEACH DEPOSIT								
D	29.0														
Borehole CBH24 terminated at 2.2m															
						2.5									
						28.5									
						3.0									
						28.0									
						3.5									
						27.5									
						4.0									
method		support				notes, samples, tests				classification symbols and soil description				consistency/density Index	
AS		M mud				U ₅₀ undisturbed sample 50mm diameter				VS				very soft	
AD		C casing				U ₆₃ undisturbed sample 63mm diameter				S				soft	
RR		penetration				D disturbed sample				F				firm	
W		1 2 3 4				N standard penetration test (SPT)				St				stiff	
CT						N* SPT - sample recovered				VSt				very stiff	
HA		water				Nc SPT with solid cone				H				hard	
DT		10/1/98 water level on date shown				V vane shear (kPa)				Fb				friable	
B						P pressuremeter				VL				very loose	
V						Bs bulk sample				L				loose	
T						E environmental sample				MD				medium dense	
*bit shown by suffix e.g. ADT						R refusal				D				dense	
										VD				very dense	

Engineering Log - Borehole

Client: **NSW Department Of Commerce**
Principal: **NSW Department Of Corrective Services**
Project: **Environmental/Geotechnical Investigation**
Borehole Location: **Long Bay Correctional Complex, Malabar, NSW, Area D**

drill model and mounting:		Mitsui Truck		Easting:		slope: -90°		R.L. Surface: 36.5				
hole diameter:		100 mm		Northing		bearing: -		datum: AHD				
drilling information				material substance								
method	penetration 1 2 3	support water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	pocket penetro- meter kPa 100 200 300 400	structure and additional observations
V BIT		N	E					FILL: CLAYEY SAND: Fine to coarse grained, brown to dark brown, low plasticity clay.	M			FILL
		None observed		36.0	0.5			FILL: CLAYEY SAND: Fine to coarse grained, pale grey/pale brown, low plasticity clay.				
			SPT 4,6,7 N*=13	35.5	1.0							
				35.0	1.5		SP	SAND: Fine to medium grained, mottled pale grey, orange, brown, low plasticity clay (<5%).		MD		AEOLIAN DUNE OR BEACH DEPOSIT
			E	34.0	2.5							
								Borehole CBH65 terminated at 2.5m				
				33.5	3.0							
				33.0	3.5							
				32.5	4.0							
method			support		notes, samples, tests			classification symbols and soil description based on unified classification system			consistency/density Index	
AS auger screwing*			M mud N nil		U ₅₀ undisturbed sample 50mm diameter			moisture D dry M moist W wet Wp plastic limit W _L liquid limit			VS very soft	
AD auger drilling*			C casing		U ₅₅ undisturbed sample 63mm diameter						S soft	
RR roller/tricone			penetration 1 2 3 4		D disturbed sample						F firm	
W washbore			no resistance ranging to refusal		standard penetration test (SPT)						St stiff	
CT cable tool			water		N* SPT - sample recovered						VS _t very stiff	
HA hand auger			10/1/98 water level on date shown		Nc SPT with solid cone						H hard	
DT diatube			water inflow		V vane shear (kPa)						Fb friable	
B blank bit			water outflow		P pressuremeter						VL very loose	
V V bit					Bs bulk sample						L loose	
T TC bit					E environmental sample						MD medium dense	
*bit shown by suffix e.g. ADT					R refusal						D dense	
											VD very dense	

Borehole No. **CBH99****Engineering Log - Borehole**

Sheet 1 of 3

Office Job No.: **E12723/4**Client: **Department of Commerce**Date started: **16.3.2005**Principal: **Department of Corrective Services**Date completed: **16.3.2005**Project: **Environmental/Geotechnical Investigation**Logged by: **CFW**Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area D.**Checked by: **MM**

drilling information				material substance									
method	penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	pocket penetro- meter kPa	structure and additional observations
1	2	3											
HA		N		E					FILL: SAND: Fine grained, yellow, with some silt and clay.	D			FILL
					34					M			
ADT						1							
				SPT 12,14,23 N*=37	33				Grading to medium grained, becoming dark grey, with some coarse white sandstone gravel.				
						2							
				SPT 12,14,20 N*=34	32								
						3			FILL: SANDY CLAY: Mottled orange/grey.				
								SM	SILTY SAND: Fine grained, grey/brown, with some medium to coarse grained sandstone gravel.	D-VD			AEOLIAN SAND
					31								
						4							
				SPT 12,22, Hammer Bouncing N*=R	30			SW	SAND: Fine to medium grained, yellow/brown.	W			
						5			with some clay.				
					29			MH	CLAYEY SILT: Low liquid limit, light grey/white, with some fine grained sand.	VSt			RESIDUAL SOILS
						6							
					28								
						7							
					27								
						8							

method		support		notes, samples, tests		classification symbols and soil description		consistency/density index	
AS	auger screwing*	M	mud	U ₅₀	undisturbed sample 50mm diameter	based on unified classification system		VS	very soft
AD	auger drilling*	C	casing	U ₆₃	undisturbed sample 63mm diameter			S	soft
RR	roller/tricone	penetration		D	disturbed sample	moisture		F	firm
W	washbore	1	2	N	standard penetration test (SPT)			St	stiff
CT	cable tool	3	4	N*	SPT - sample recovered	D	dry	VSt	very stiff
HA	hand auger	no resistance ranging to refusal		Nc	SPT with solid cone	M	moist	H	hard
DT	diatube	water		V	vane shear (kPa)	W	wet	Fb	friable
B	blank bit	10/1/98 water level on date shown		P	pressuremeter	Wp	plastic limit	VL	very loose
V	V bit	water inflow		Bs	bulk sample	WL	liquid limit	L	loose
T	TC bit	water outflow		E	environmental sample			MD	medium dense
*bit shown by suffix e.g. ADT				R	refusal			D	dense
								VD	very dense

BOREHOLE E12723/4 AREA B&D PHASE 2 UPDATED.GPJ COFFEY.GDT 27.06.05

Form GEO 5.3 Issue 3 Rev 2

Borehole No. **CBH99****Engineering Log - Borehole**Sheet 2 of 3
Office Job No.: **E12723/4**Client: **Department of Commerce**Date started: **16.3.2005**Principal: **Department of Corrective Services**Date completed: **16.3.2005**Project: **Environmental/Geotechnical Investigation**Logged by: **CFW**Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area D.**Checked by: **MM**

drill model and mounting: GEMCO TRUCK		Easting:		slope: -90°		R.L. Surface: 34.5	
hole diameter: 100 mm		Northing		bearing:		datum: AHD	
drilling information				material substance			
method	penetration 1 2 3	support water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol
ADT		N					MH
					26		CLAYEY SILT: Low liquid limit, light grey/white, with some fine grained sand. (continued)
					9		CH
					25		CLAY: High plasticity, grey, with some silt, trace of carbonaceous lenses.
			SPT 6,12,17 N*=29		10		
					24		
			SPT 10,10,15 N*=25		11		
					23		
					12		
			SPT				
			22/140mm N*=R		22		Borehole CBH99 continued as cored hole
					13		
					21		
					14		
					20		
					15		
					19		
					16		
method		support		notes, samples, tests		classification symbols and soil description based on unified classification system	
AS auger screwing*		M mud N nil		U ₅₀ undisturbed sample 50mm diameter		moisture D dry M moist W wet W _p plastic limit W _L liquid limit	
AD auger drilling*		C casing		U ₆₃ undisturbed sample 63mm diameter			
RR roller/tricone		penetration 1 2 3 4		D disturbed sample		consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense	
W washbore		no resistance ranging to refusal		N standard penetration test (SPT)			
CT cable tool		water		N* SPT - sample recovered			
HA hand auger		10/1/98 water level on date shown		Nc SPT with solid cone			
DT diatube		water inflow		V vane shear (kPa)			
B blank bit		water outflow		P pressuremeter			
V V bit				Bs bulk sample			
T TC bit				E environmental sample			
*bit shown by suffix e.g. ADT				R refusal			

BOREHOLE E12723.4 AREA B&D PHASE 2 UPDATED.GPJ COFFEY.GDT 27.06.05

Form GEO 5.3 Issue 3 Rev.2



Job Number: E12723/4

CBH99

Depth: 12.1m to 15.1m



Borehole No. **CBH99****Engineering Log - Cored Borehole**Sheet 3 of 3
Office Job No.: **E12723/4**Client: **Department of Commerce**Date started: **16.3.2005**Principal: **Department of Corrective Services**Date completed: **16.3.2005**Project: **Environmental/Geotechnical Investigation**Logged by: **CFW**Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area D.**Checked by: **MM**

drill model & mounting: GEMCO TRUCK				Easting:		slope: -90°		R.L. Surface: 34.5			
hole diameter: 100 mm				Drilling fluid:		Northing:		bearing:		datum: AHD	
drilling information				material substance				rock mass defects			
method	core-lift	water	RL	depth metres	graphic log core recovery	material rock type; grain characteristics, colour, structure, minor components	weathering alteration	estimated strength	Is ₅₀ MPa D- diam- A- axial	defect spacing mm	defect description type, inclination, planarity, roughness, coating, thickness
			26	9							
			25	10							
			24	11							
			23	12							
						Continued from non-cored borehole					
	NMLC		22	13		SANDSTONE/SILTSTONE: Fine grained, grey stained orange brown, indistinctly bedded, highly fractured.	HW		D 0.15 A 0.2	60	JT, IR, RO, 75° SN orange JT, IR, RO, 75° SN orange JT, IR, RO, 75° SN orange
			21	14		SANDSTONE/SILTSTONE: Fine grained, grey stained orange brown, indistinctly bedded, highly fractured.	HW		D 0.3 A 0.27	0	
			20	15		NO CORE: 14.6-15.1m.			D 0.22 A 0.17	0	
			19	16		CBH99 terminated at 15.1m					

method	core-lift	water	weathering	defect type	roughness
DT diatube	casings used	10/1/98 water level on date shown	FR fresh	JT joint	VR very rough
AS auger screwing	barrel withdrawn	water inflow	SW slightly weathered	PT parting	RO rough
AD auger drilling		partial drill fluid loss	MW moderately weathered	SM seam	SO smooth
RR roller/tricone		complete drill fluid loss	HW highly weathered	SZ sheared zone	SL slickensided
CB claw or blade bit			XW extremely weathered	SS sheared surface	
NMLC NMLC core	graphic log/core recovery		DW distinctly weathered (covers MW and HW)	CS crushed seam	
NQ, HQ, PQ wireline core	core recovered - graphic symbols indicate material				
	no core recovered				

strength	planarity	coating
VL very low	PL planar	CN clean
L low	CU curved	SN stained
M medium	UN undulating	VN veneer
H high	ST stepped	CO coating
VH very high	IR irregular	
EH extremely high		

CORED BOREHOLE E12723.4 AREA B&D PHASE 2 UPDATED GPJ COFFEY GDT 27.06.05

Form GEO 5.5 Issue 3 Rev. 3

Coffey

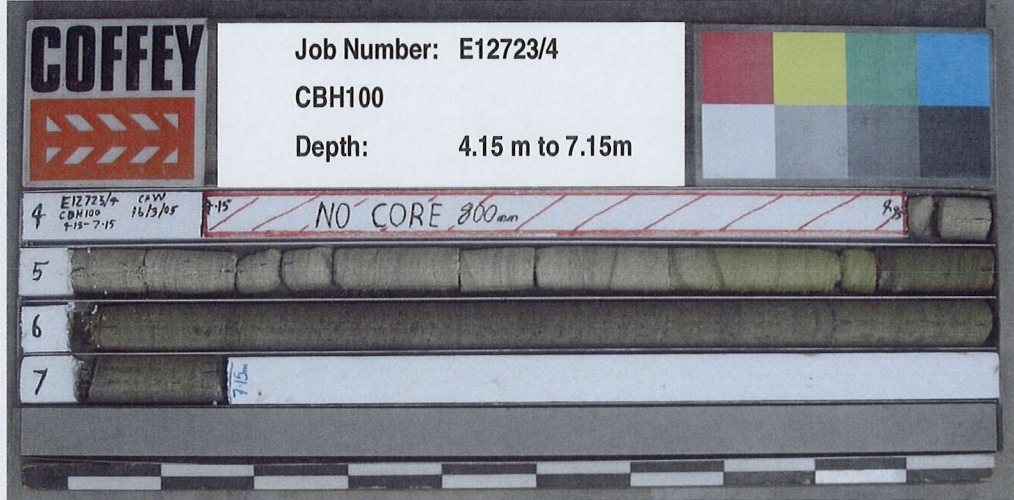
Engineering Log - BoreholeSheet 1 of 2
Office Job No.: **E12723/4**Client: **Department of Commerce**Date started: **16.3.2005**Principal: **Department of Corrective Services**Date completed: **16.3.2005**Project: **Environmental/Geotechnical Investigation**Logged by: **CFW**Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area D.**Checked by: **MM**

drilling information				material substance								
method	penetration	support	water	notes samples, tests, etc	depth metres	graphic log	classification symbol	material	moisture condition	consistency/density index	pocket penetrometer kPa	structure and additional observations
1	2	3			RL			soil type: plasticity or particle characteristics, colour, secondary and minor components.			100 200 300 400	
HA		N		E	34			FILL: SANDY SILT: Brown, with some angular/subangular medium grained gravel.	D			FILL 100mm thick topsoil and grass at surface.
					1			FILL: CLAYEY SAND: Fine to medium grained, yellow, with some coarse grained, extremely weathered sandstone gravel.	M			30mm black plastic pipe discovered at 0.3m during hand augering.
ADT				SPT 7,8,8 N*=16	33							ALS D6, DUB D6
				SPT 13,20,18 N*=38	2		SW	SAND: Fine to medium grained, dark grey, mottled yellow, with some organic material.		D		AEOLIAN SAND
					3			becoming light grey.	W	L		
				SPT 4,4,5 N*=9	31		SC	CLAYEY SAND: Fine to medium grained, clay is of medium plasticity.				RESIDUAL SOIL
					4			Borehole CBH100 continued as cored hole				
					5							
					6							
					7							
					8							

method		support		notes, samples, tests		classification symbols and soil description based on unified classification system		consistency/density index	
AS	auger screwing*	M	mud	U ₅₀	undisturbed sample 50mm diameter			VS	very soft
AD	auger drilling*	C	casing	U ₆₃	undisturbed sample 63mm diameter			S	soft
RR	rotter/tricone			D	disturbed sample			F	firm
W	washbore			N	standard penetration test (SPT)			St	stiff
CT	cable tool			N*	SPT - sample recovered			VSt	very stiff
HA	hand auger			Nc	SPT with solid cone			H	hard
DT	diatube			V	vane shear (kPa)			Fb	friable
B	blank bit			P	pressuremeter			VL	very loose
V	V bit			Bs	bulk sample			L	loose
T	TC bit			E	environmental sample			MD	medium dense
*bit shown by suffix e.g. ADT				R	refusal			D	dense
								VD	very dense

BOREHOLE E12723.4 AREA B&D PHASE 2 UPDATED GPJ COFFEY.GDT 27.06.05

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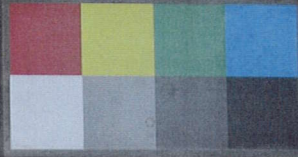


COFFEY

Job Number: E12723/4

CBH100

Depth: 4.15 m to 7.15m



4 E12723/4
CBH100
7-15-7-15
CPW
16/13/05

7-15

NO CORE 800 mm

7-15

5

6

7

7/15



Engineering Log - Cored BoreholeSheet 2 of 2
Office Job No.: **E12723/4**Client: **Department of Commerce**Date started: **16.3.2005**Principal: **Department of Corrective Services**Date completed: **16.3.2005**Project: **Environmental/Geotechnical Investigation**Logged by: **CFW**Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area D.**Checked by: **MM**





drilling model & mounting: GEMCO TRUCK				Easting:		slope: -90°		R.L. Surface: 34.5			
hole diameter: 100 mm				Drilling fluid:		Northing:		bearing:		datum: AHD	
drilling information				material substance				rock mass defects			
method	core-lift	water	RL	depth metres	graphic log core recovery	material rock type; grain characteristics, colour, structure, minor components	weathering alteration	estimated strength VL L M H VH EH	Is ₍₅₀₎ MPa D- diam- etral A- axial	defect spacing mm RD % 30 100 300 1000 3000	defect description type, inclination, planarity, roughness, coating, thickness
											particular
			34	1							
			33	2							
			32	3							
			31	4							
						Continued from non-cored borehole					
NMLC			30	5		NO CORE: 4.1-4.95m.					
			29	6		SANDSTONE: Coarse grained, pale grey, some distinct cross bedding, 0-30°.	MW		D A 0.22 0.19		JT, 40°, PL, RO, CN. JT, 0°, PL, RO, CN.
			28	7		Grading to medium to coarse grained, becoming grey, indistinct bedded at 0-20°, trace of carbonaceous material.			D A 0.44 0.39		XW SM, 5°, PL, Clayey Sand, 10mm.
			27	8		CBH100 terminated at 7.15m			D A 0.71 0.93		PT, 0-20°, PL, RO, VN.

method	core-lift	water	weathering	defect type	roughness
DT diatube	□ casing used	10/1/98 water level on date shown	FR fresh	JT joint	VR very rough
AS auger screwing	□ barrel withdrawn	▶ water inflow	SW slightly weathered	PT parting	RO rough
AD auger drilling		▶ partial drill fluid loss	MW moderately weathered	SM seam	SO smooth
RR roller/tricone		▶ complete drill fluid loss	HW highly weathered	SZ sheared zone	SL slickensided
CB claw or blade bit			XW extremely weathered	SS sheared surface	
NMLC NMLC core			DW distinctly weathered (covers MW and HW)	CS crushed seam	
NQ, HQ, PQ wireline core					
	graphic log/core recovery		strength	planarity	coating
	□ core recovered		VL very low	PL planar	CN clean
	□ - graphic symbols indicate material		L low	CU curved	SN stained
	□ no core recovered		M medium	UN undulating	VN veneer
			H high	ST stepped	CO coating
			VH very high	IR irregular	
			EH extremely high		

CORED BOREHOLE E12723.4 AREA B&D PHASE 2 UPDATED.GPJ COFFEY.GDT 27.06.05

Form GEO 5.5 Issue 3 Rev. 3

Engineering Log - BoreholeSheet 1 of 3
Office Job No.: **E12723/4**Client: **Department of Commerce**Date started: **18.3.2005**Principal: **Department of Corrective Services**Date completed: **18.3.2005**Project: **Environmental/Geotechnical Investigation**Logged by: **CFW**Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area D.**Checked by: **MM**

drill model and mounting: MOLE TRUCK		Easting:		slope: -90°		R.L. Surface: 35.1					
hole diameter: 100 mm		Northing		bearing:		datum: AHD					
drilling information				material substance							
method	penetration 1 2 3	support water	notes samples, tests, etc	RL metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	pocket penetro- meter kPa 100 200 300 400	structure and additional observations
HA		N	E	35			TOPSOIL: Dark brown, with some roots.	M			FILL
ADT							FILL: SAND: Fine to medium grained, yellow/grey, with some subrounded coarse grained, highly weathered sandstone gravel.				Topsoil and grass at surface.
			SPT 7,7,5 N*=12	34	1		FILL: SILTY SAND: Medium grained, brown/grey, with some subrounded, medium to coarse grained sandstone gravel, trace of subangular, fine and medium grained brown gravel.				Tree roots at 1m.
				33	2		FILL: SAND: Fine grained, grey, with some medium grained, moderately weathered white sandstone gravel and medium plasticity clay.				
			SPT 1,1,1 N*=2	32	3	MH	SANDY SILT: Medium to high plasticity, dark brown, sand medium to fine grained sand, with some roots and high plasticity clay.	W	VS-S		ALLUVIUM?
				31	4	SP	SAND: Fine to medium grained, grey.		MD		AEOLIAN SAND
			SPT 7,7,5 N*=12	30	5						
				29	6						
				28	7						
					8						
method AS auger screwing* AD auger drilling* RR roller/tricone W washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT		support M mud C casing penetration 1 2 3 4  water  10/1/98 water level on date shown  water inflow  water outflow		notes, samples, tests U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal		classification symbols and soil description based on unified classification system moisture D dry M moist W wet W _p plastic limit W _L liquid limit		consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense			

BOREHOLE E12723/4 AREA B&D PHASE 2 UPDATED.GPJ COFFEY.GDT 27.06.05

Form GEO 5.3 Issue 3 Rev 2

Engineering Log - Borehole

Client: **Department of Commerce**

Date started: 18.3.2005

Principal: **Department of Corrective Services**

Date completed: **18.3.2005**

Project: **Environmental/Geotechnical Investigation**

Logged by: **CFW**

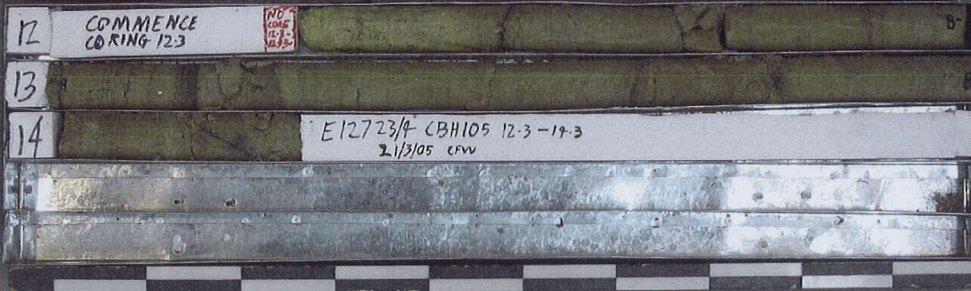
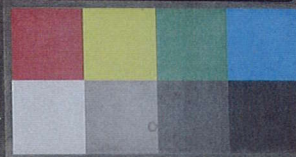
Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area D.**

Checked by: **MM**

drill model and mounting:		MOLE TRUCK		Easting:		slope:		-90°		R.L. Surface:		35.1															
hole diameter:		100 mm		Northing		bearing:				datum:		AHD															
drilling information				material substance																							
method		penetration		support		water		notes samples, tests, etc		RL		depth metres		graphic log		classification symbol		material		moisture condition		consistency/ density index		pocket penetro- meter KPa		structure and additional observations	
1 2 3		N								27		9		SP		SAND: Fine to medium grained, grey. (continued)		W		MD				AEOLIAN SAND			
RR										26		10		CL/CH		SANDY CLAY: Medium plasticity, white, fine to medium grained to coarse quartz sand.				VSt				RESIDUAL SOIL			
										25		11															
										24		12															
										23		13															
										22		14															
										21		15															
										20		16															
										19		17															
										18		18															
										17		19															
										16		20															
										15		21															
										14		22															
										13		23															
										12		24															
										11		25															
										10		26															
										9		27															
										8		28															
										7		29															
										6		30															
										5		31															
										4		32															
										3		33															
										2		34															
										1		35															
										0		36															
										-1		37															



Job Number: E12723/4
CBH105
Depth: 12.3m to 14.3m



Engineering Log - Cored Borehole

Client: **Department of Commerce**

Date started: **18.3.2005**

Principal: **Department of Corrective Services**

Date completed: **18.3.2005**

Project: **Environmental/Geotechnical Investigation**

Logged by: **CFW**

Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area D.**

Checked by: **MM**

drill model & mounting: MOLE TRUCK						Easting:		slope: -90°		R.L. Surface: 35.1							
hole diameter: 100 mm Drilling fluid:						Northing:		bearing:		datum: AHD							
drilling information						material substance						rock mass defects					
method	core-lift	water	RL	depth metres	graphic log core recovery	material rock type; grain characteristics, colour, structure, minor components	weathering alteration	estimated strength					Is ₅₀ MPa D- diam- etral A- axial	ROD %	defect spacing mm	defect description	
								VL	L	M	H	VH				EH	particular
			27														
			26	9													
			25	10													
			24	11													
			23	12													
						Continued from non-cored borehole											
NMLC			22	13		NO CORE: 12.3-12.33m. SANDSTONE: Fine grained, pale grey/brown, indistinctly bedded at 5°, occasional thin carbonaceous laminations.	XW HW					D A 0.05 0.04				PT, PL, SO, 30°. SM, 40mm gravel infill.	
			21	14		Grading to medium grained, pale grey/brown, cross bedded at 15 degrees.						D A 0.13 0.05				PT, PL, RO, CN. PT, PL, RO, CN. PT, PL, RO, CN. Extremely Weathered Seam, 20mm.	
						CBH105 terminated at 14.3m						D A 0.09 0.11				PT, PL, SO, CN. JT, PL, SO, VN clay. PT, PL, SO, CN.	
			20	15													
				16													
method DT diatube AS auger screwing AD auger drilling RR roller/tricone CB claw or blade bit NMLC NMLC core NQ, HQ, PQ wireline core			core-lift casing used barrel withdrawn graphic log/core recovery core recovered - graphic symbols indicate material no core recovered			water 10/1/98 water level on date shown water inflow partial drill fluid loss complete drill fluid loss water pressure test result (lugeons) for depth interval shown			weathering FR fresh SW slightly weathered MW moderately weathered HW highly weathered XW extremely weathered DW distinctly weathered (covers MWV and HW) strength VL very low L low M medium H high VH very high EH extremely high				defect type JT joint PT parting SM seam SZ sheared zone SS sheared surface CS crushed seam planarity PL planar CU curved UN undulating ST stepped IR irregular roughness VR very rough RO rough SO smooth SL slickensided coating CN clean SN stained VN veneer CO coating				

Engineering Log - BoreholeSheet 1 of 2
Office Job No.: **E12723/4**Client: **Department of Commerce**Date started: **14.3.2005**Principal: **Department of Corrective Services**Date completed: **14.3.2005**Project: **Environmental/Geotechnical Investigation**Logged by: **CFW**Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area D.**Checked by: **MM**

drill model and mounting: MOLE TRUCK		Easting:		slope: -90°		R.L. Surface: 32.1	
hole diameter: 100 mm		Northing		bearing:		datum: AHD	
drilling information				material substance			
method	penetration 1 2 3	support water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol
ADT		N		32			
		NONE OBSERVED					
				31	1		SC
				30	2		
Borehole CBH108 continued as cored hole							
				29	3		
				28	4		
				27	5		
				26	6		
				25	7		
					8		
method		support		notes, samples, tests		classification symbols and soil description	
AS auger screwing*		M mud N nil		U ₅₀ undisturbed sample 50mm diameter		VS very soft	
AD auger drilling*		C casing		U ₆₃ undisturbed sample 63mm diameter		S soft	
RR roller/tricone				D disturbed sample		F firm	
W washbore				N standard penetration test (SPT)		St stiff	
CT cable tool				N* SPT - sample recovered		VSt very stiff	
HA hand auger				Nc SPT with solid cone		H hard	
DT diatube				V vane shear (kPa)		Fb friable	
B blank bit				P pressuremeter		VL very loose	
V V bit				Bs bulk sample		L loose	
T TC bit				E environmental sample		MD medium dense	
*bit shown by suffix				R refusal		D dense	
e.g. ADT						VD very dense	

BOREHOLE E12723.4 AREA B&D PHASE 2 UPDATED.GPJ COFFEY.GDT 30.06.05

Form GEO 5.3 Issue 3 Rev.2



Job Number: E12723/4
CBH108
Depth: 2.3m to 5.3m



E12723/4 CBH 108

230 NO CORE
0.16 2.16

3

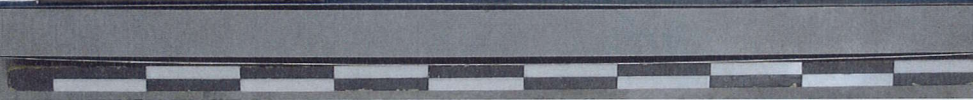
B-8

4

B-8

5

5.30
END



Engineering Log - Cored Borehole

Sheet 2 of 2

Office Job No.: **E12723/4**Client: **Department of Commerce**Date started: **14.3.2005**Principal: **Department of Corrective Services**Date completed: **14.3.2005**Project: **Environmental/Geotechnical Investigation**Logged by: **CFW**Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area D.**Checked by: **MM****Coffey**

drill model & mounting: MOLE TRUCK		Easting:		slope: -90°		R.L. Surface: 32.1							
hole diameter: 100 mm		Drilling fluid:		Northing:		bearing: datum: AHD							
drilling information			material substance			rock mass defects							
method	core-lift	water	RL	depth metres	graphic log core recovery	material	weathering alteration	estimated strength	Is ₍₅₀₎ MPa	D- diam- etral A- axial	RQD %	defect spacing mm	defect description
						rock type; grain characteristics, colour, structure, minor components		VL L M H VH EH			30 100 300 1000 3000		type, inclination, planarity, roughness, coating, thickness
			32										particular
				1									general
				2									
						Continued from non-cored borehole							
						NO CORE: 2.3-2.46m.							
				3		SANDSTONE: Medium grained, pale grey, indistinctly laminated, 0-20°, occasional fine carbonaceous laminations.	SW						
				4									
				5		Grading to medium to coarse grained, grey/pale grey, distinctly laminated at 0-20°, trace of quartz nodules up to 5mm.	FR						
				6		CBH108 terminated at 5.3m							
				7									
				8									

method DT diatube AS auger screwing AD auger drilling RR roller/tricone CB claw or blade bit NMLC NMLC core NQ, HQ, PQ wireline core	core-lift casing used barrel withdrawn graphic log/core recovery core recovered indicate material no core recovered	water 10/1/98 water level on date shown water inflow partial drill fluid loss complete drill fluid loss water pressure test result (lugesons) for depth interval shown	weathering FR fresh SW slightly weathered MW moderately weathered HW highly weathered XW extremely weathered DW distinctly weathered (covers MW and HW) strength VL very low L low M medium H high VH very high EH extremely high	defect type JT joint PT parting SM seam SZ sheared zone SS sheared surface CS crushed seam planarity PL planar CU curved UN undulating ST stepped IR irregular	roughness VR very rough RO rough SO smooth SL slickensided coating CN clean SN stained VN veneer CO coating
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CORED BOREHOLE E12723.4 AREA B&D PHASE 2 UPDATED.GPJ COFFEY.GDT 30.06.05

Form GEO 5.5 Issue 3 Rev. 3

Borehole No. **CBH109****Engineering Log - Borehole**Sheet 1 of 2
Office Job No.: **E12723/4**Client: **Department of Commerce**Date started: **14.3.2005**Principal: **Department of Corrective Services**Date completed: **14.3.2005**Project: **Environmental/Geotechnical Investigation**Logged by: **CFW**Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area D.**Checked by: **MM**

drill model and mounting: MOLE TRUCK		Easting:		slope: -90°		R.L. Surface: 33.7	
hole diameter: 100 mm		Northing		bearing:		datum: AHD	
drilling information				material substance			
method	penetration	support	notes samples, tests, etc	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.
ADT	1 2 3	N		33		SW SW	TOPSOIL: SAND: Medium grained, dark grey, with some roots. SAND: Medium grained, grey.
		NONE OBSERVED		1			Borehole CBH109 continued as cored hole
				32			
				2			
				31			
				3			
				30			
				4			
				29			
				5			
				28			
				6			
				27			
				7			
				26			
				8			
method		support		notes, samples, tests		classification symbols and soil description	
AS auger screwing*		M mud N nil		U ₅₀ undisturbed sample 50mm diameter		VS very soft	
AD auger drilling*		C casing		U ₆₃ undisturbed sample 63mm diameter		S soft	
RR roller/tricone		penetration		D disturbed sample		F firm	
W washbore		1 2 3 4		N standard penetration test (SPT)		St stiff	
CT cable tool		no resistance ranging to refusal		N* SPT - sample recovered		VSt very stiff	
HA hand auger		water		Nc SPT with solid cone		H hard	
DT diatube		10/1/98 water level on date shown		V vane shear (kPa)		Fb friable	
B blank bit		water inflow		P pressuremeter		VL very loose	
V V bit		water outflow		Bs bulk sample		L loose	
T TC bit				E environmental sample		MD medium dense	
*bit shown by suffix				R refusal		D dense	
e.g. ADT						VD very dense	
						moisture	
						D dry	
						M moist	
						W wet	
						Wp plastic limit	
						W _L liquid limit	

BOREHOLE E12723.4 AREA B&D PHASE 2 UPDATED.GPJ COFFEY.GDT 30.06.05

Form GEO 5.3 Issue 3 Rev.2



Job Number: E12723/4
CBH109
Depth: 0.78m to 3.8m



E12723/4 CBH109

1 CORE STARTED 0.78m

2 NO CORE 0.15m

3 NO CORE 0.1m

CBH109 TERMINATED AT 3.8m

Borehole No. **CBH109**

Sheet 2 of 2

Office Job No.: **E12723/4****Engineering Log - Cored Borehole**Client: **Department of Commerce**Date started: **14.3.2005**Principal: **Department of Corrective Services**Date completed: **14.3.2005**Project: **Environmental/Geotechnical Investigation**Logged by: **CFW**Borehole Location: **Long Bay Correctional Facility, Malabar, NSW, Area D.**Checked by: **MM**

drilling information				material substance				rock mass defects				
method	core-lift	water	RL	depth metres	material	weathering alteration	estimated strength	Is ₅₀₀ MPa	D- diam- etral A- axial	RQD %	defect spacing mm	defect description
					rock type; grain characteristics, colour, structure, minor components		VL L M H VH EH			30 100 300 1000 3000		particular
			33		Continued from non-cored borehole							
NMLC				1	SANDSTONE: Coarse grained, pale grey and pale orange-brown, distinctly laminated 0-60°.	MW		D A	0.07 0.12			
	NONE OBSERVED		32	2	NO CORE: 1.63-1.73m. SANDSTONE: Coarse grained, pale grey and pale orange-brown, distinct laminated 0-60°.	MW		D A	0.2 0.16			JT, 60°, PL, RO, CN.
			31	3	NO CORE: 2.13-2.28m. SANDSTONE: Medium grained, grey and pale grey, indistinct laminations 0-20°, trace of carbonaceous material and quartz nodules (5mm).	SW		D A	0.77 0.68	82		SM, 0°, PL, Sandy Clay, 40mm. PT, 0°, PL, RO, CN.
			30	4	CBH109 terminated at 3.8m			D A	0.23 0.62			PT, 15°, CU, RO, CN. PT, 0°, PL, RO, CN
			29	5								
			28	6								
			27	7								
			26	8								

method	core-lift	water	weathering	defect type	roughness
DT diatube	□ casing used	▽ 10/1/98 water level on date shown	FR fresh	JT joint	VR very rough
AS auger screwing	□ barrel withdrawn	▶ water inflow	SW slightly weathered	PT parting	RO rough
AD auger drilling		▶ partial drill fluid loss	MW moderately weathered	SM seam	SO smooth
RR roller/tricone		▶ complete drill fluid loss	HW highly weathered	SZ sheared zone	SL slickensided
CB claw or blade bit			XW extremely weathered	SS sheared surface	
NMLC NMLC core			DW distinctly weathered (covers MW and HW)	CS crushed seam	
NQ, HQ, PQ wireline core					

graphic log/core recovery	strength	planarity	coating
□ core recovered	VL very low	PL planar	CN clean
□ - graphic symbols indicate material	L low	CU curved	SN stained
□ no core recovered	M medium	UN undulating	VN veneer
	H high	ST stepped	CO coating
	VH very high	IR irregular	
	EH extremely high		

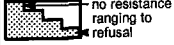



CORED BOREHOLE E12723.4 AREA B&D PHASE 2 UPDATED GPJ COFFEY.GDT 27.06.05

Form GEO 5.5 Issue 3 Rev. 3

Borehole No. **HA15****Engineering Log - Borehole**

Sheet 1 of 1

Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **12.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **12.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Facility, Malabar NSW, Area D**Checked by: **DD**

drill model and mounting: Hand Auger		Easting:		slope: -90°		R.L. Surface: 31.8			
hole diameter: 100 mm		Northing		bearing:		datum: AHD			
drilling information				material substance					
method	penetration 1 2 3	support water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol		
HA		N	E						
		None observed							
				31.5			SP		
			E		0.5				
Borehole HA15 terminated at 0.5m									
				31.0					
				1.0					
				30.5					
				1.5					
				30.0					
				2.0					
method AS auger screwing* AD auger drilling* RR roller/tricone CT washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT				support M mud N nil C casing penetration 1 2 3 4  water  10/1/98 water level on date shown  water inflow  water outflow		notes, samples, tests U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal		classification symbols and soil description based on unified classification system moisture D dry M moist W wet Wp plastic limit W _L liquid limit	consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense

BOREHOLE HAND AUGERS - SITE D.GPJ COFFEY.GDT 01.12.04





Form GEO 5.3 Issue 3 Rev.2

Coffey

Borehole No. **HA16**

Sheet 1 of 1

Office Job No.: **E12723/03**Date started: **12.10.2004**Date completed: **12.10.2004**Logged by: **AD**Checked by: **DFD****Engineering Log - Borehole**Client: **NSW Department Of Commerce**Principal: **NSW Department Of Corrective Services**Project: **Environmental/Geotechnical Investigation**Borehole Location: **Long Bay Correctional Facility, Malabar NSW, Area D****Coffey**





drill model and mounting:		Hand Auger		Easting:		slope: -90°		R.L. Surface: 35.2											
hole diameter:		100 mm		Northing		bearing:		datum: AHD											
drilling information				material substance															
method	penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	pocket penetro- meter kPa	structure and additional observations						
HA	1 2 3	N		E				SP	SILTY SAND: Fine to medium grained, dark brown, low liquid limit silt.	M	L	100 200 300 400	AEOLIAN DUNE OR BEACH DEPOSIT						
			None observed		35.0														
				E		0.5													
									Borehole HA16 terminated at 0.5m. Hand Auger refusal on Sandstone Bedrock.										
					34.5														
						1.0													
					34.0														
						1.5													
					33.5														
						2.0													
method AS auger screwing* AD auger drilling* RR roller/tricone W washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT				support M mud N nil C casing penetration 1 2 3 4  no resistance ranging to refusal water  10/1/98 water level on date shown  water inflow  water outflow				notes, samples, tests U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal				classification symbols and soil description based on unified classification system moisture D dry M moist W wet W _p plastic limit W _L liquid limit				consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense			

BOREHOLE HAND AUGERS - SITE D.G.P.J. COFFEY.GDT_01.12.04

Borehole No. **HA17****Engineering Log - Borehole**

Sheet 1 of 1

Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **12.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **12.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Facility, Malabar NSW, Area D**Checked by: **DFD**

drill model and mounting: Hand Auger		Easting:		slope: -90°		R.L. Surface: 32.8								
hole diameter: 100 mm		Northing		bearing:		datum: AHD								
drilling information				material substance										
method	penetration 1 2 3	support water	notes samples, tests, etc	RL	depth metres	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	structure and additional observations							
HA		N	E			SAND: Fine to medium grained, pale grey brown, low plasticity clay (<5%).	AEOLIAN DUNE OR BEACH DEPOSIT							
		None observed		32.5										
			E	0.5										
				32.0		Borehole HA17 terminated at 0.5m. Hand Auger refusal on Sandstone Bedrock.								
				1.0										
				31.5										
				1.5										
				31.0										
				2.0										
method AS auger screwing* AD auger drilling* RR roller/tricone W washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT			support M mud N nil C casing penetration 1 2 3 4  no resistance ranging to refusal water  10/1/98 water level on date shown  water inflow  water outflow			notes, samples, tests U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal			classification symbols and soil description based on unified classification system moisture D dry M moist W wet Wp plastic limit WL liquid limit			consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense		

BOREHOLE HAND AUGERS - SITE D.GPJ COFFEY.GDT 01.12.04

Form GEO 5.3 Issue 3 Rev.2

Coffey

Borehole No. **HA18**
 Sheet 1 of 1
 Office Job No.: **E12723/03**
 Date started: **12.10.2004**
 Date completed: **12.10.2004**
 Logged by: **AD**
 Checked by: **DFD**

Engineering Log - Borehole

Client: **NSW Department Of Commerce**
 Principal: **NSW Department Of Corrective Services**
 Project: **Environmental/Geotechnical Investigation**
 Borehole Location: **Long Bay Correctional Facility, Malabar NSW, Area D**



drill model and mounting: Hand Auger Easting: slope: -90° R.L. Surface: 33.7
 hole diameter: 100 mm Northing bearing: datum: AHD

drilling information					material substance									
method	penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material	moisture condition	consistency/ density index	pocket penetro- meter kPa	structure and additional observations	
1	2	3							soil type: plasticity or particle characteristics, colour, secondary and minor components.			100 200 300 400		
HA		N	None observed	E				SP	SAND: Medium to coarse grained, pale grey brown, low plasticity clay (<5%).	M	L		AEOLIAN DUNE OR BEACH DEPOSIT	
					33.5									
				E										
						0.5								
					33.0									
						1.0								
					32.5									
						1.5								
					32.0									
						2.0								
									Borehole HA18 terminated at 0.3m. Hand Auger refusal on Sandstone Bedrock.					

method AS auger screwing* AD auger drilling* RR roller/tricone W washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT	support M mud N nil C casing penetration 1 2 3 4 no resistance ranging to refusal water 10/1/98 water level on date shown water inflow water outflow	notes, samples, tests U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal	classification symbols and soil description based on unified classification system moisture D dry M moist W wet Wp plastic limit W _L liquid limit	consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense
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Borehole No. **HA19**

Sheet 1 of 1

Office Job No.: **E12723/03**Date started: **12.10.2004**Date completed: **12.10.2004**Logged by: **AD**Checked by: **DFD****Engineering Log - Borehole**Client: **NSW Department Of Commerce**Principal: **NSW Department Of Corrective Services**Project: **Environmental/Geotechnical Investigation**Borehole Location: **Long Bay Correctional Facility, Malabar NSW, Area D**

drill model and mounting: Hand Auger Easting: slope: -90° R.L. Surface: 34.3
 hole diameter: 100 mm Northing bearing: datum: AHD

drilling information					material substance									
method	penetration	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material	moisture condition	consistency/density index	pocket penetrometer kPa	structure and additional observations	
1	2	3							soil type: plasticity or particle characteristics, colour, secondary and minor components.			100 200 300 400		
HA		N		E				SP	SAND: Fine to coarse grained, dark grey brown, low plasticity clay (<5%).	M			AEOLIAN DUNE OR BEACH DEPOSIT	
			None observed		34.0			SP	SAND: Fine to coarse grained, pale grey yellow brown, low plasticity clay (<5%).		L			
				E	0.5									
									Borehole HA19 terminated at 0.5m. Hand Auger refusal on Sandstone Bedrock.					
					33.5									
						1.0								
					33.0									
						1.5								
					32.5									
						2.0								

method AS auger screwing* AD auger drilling* RR roller/tricone W washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT	support M mud N nil C casing penetration 1 2 3 4 water 10/1/98 water level on date shown water inflow water outflow	notes, samples, tests U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal	classification symbols and soil description based on unified classification system moisture D dry M moist W wet W _p plastic limit W _L liquid limit	consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense
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BOREHOLE HAND AUGERS - SITE D.GPJ COFFEY.GDT 01.12.04

Form GEO.5.3 Issue 3 Rev.2

Coffey





Borehole No. **HA20****Engineering Log - Borehole**

Sheet 1 of 1

Office Job No.: **E12723/03**Client: **NSW Department Of Commerce**Date started: **12.10.2004**Principal: **NSW Department Of Corrective Services**Date completed: **12.10.2004**Project: **Environmental/Geotechnical Investigation**Logged by: **AD**Borehole Location: **Long Bay Correctional Facility, Malabar NSW, Area D**Checked by: **DC D**

drill model and mounting: Hand Auger Easting: slope: -90° R.L. Surface: 33.0
 hole diameter: 100 mm Northing bearing: datum: AHD

drilling information						material substance									
method	penetration 1 2 3	support	water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	pocket penetro- meter kPa 100 200 300 400	structure and additional observations		
HA		N	None observed	E				SP	SAND: Fine to coarse grained, grey brown, low plasticity clay (<5%).	M	L		AEOLIAN DUNE OR BEACH DEPOSIT		
					32.5	0.5			Borehole HA20 terminated at 0.1m. Hand Auger refusal on Sandstone Bedrock.						
					32.0	1.0									
					31.5	1.5									
					31.0	2.0									

method AS auger screwing* AD auger drilling* RR roller/tricone W washbore CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT	support M mud N nil C casing penetration 1 2 3 4  no resistance ranging to refusal water  10/1/98 water level on date shown  water inflow  water outflow	notes, samples, tests U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal	classification symbols and soil description based on unified classification system moisture D dry M moist W wet Wp plastic limit W _L liquid limit	consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense
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BOREHOLE HAND AUGERS - SITE D.G.P.J. COFFEY GDT 01.12.04





Form GEO 5.3 Issue 3 Rev.2

Coffey

Borehole No. **HA21**

Sheet 1 of 1

Office Job No.: **E12723/03**Date started: **12.10.2004**Date completed: **12.10.2004**Logged by: **AD**Checked by: **DFD****Engineering Log - Borehole**Client: **NSW Department Of Commerce**Principal: **NSW Department Of Corrective Services**Project: **Environmental/Geotechnical Investigation**Borehole Location: **Long Bay Correctional Facility, Malabar NSW, Area D****Coffey**

drill model and mounting: Hand Auger		Easting:		slope: -90°		R.L. Surface: 34.6			
hole diameter: 100 mm		Northing		bearing:		datum: AHD			
drilling information				material substance					
method	penetration 1 2 3	support water	notes samples, tests, etc	RL	depth metres	graphic log	classification symbol		
HA		N	E	34.5			SP		
		None observed	E						
Borehole HA21 terminated at 0.2m. Hand Auger refusal on Sandstone Bedrock.									
<div style="display: flex; justify-content: space-between;"> 0.5 34.0 </div> <div style="display: flex; justify-content: space-between;"> 1.0 33.5 </div> <div style="display: flex; justify-content: space-between;"> 1.5 33.0 </div> <div style="display: flex; justify-content: space-between;"> 2.0 </div>									
method AS auger screwing* AD auger drilling* RR roller/tricone CT cable tool HA hand auger DT diatube B blank bit V V bit T TC bit *bit shown by suffix e.g. ADT		support M mud N nil C casing penetration 1 2 3 4  no resistance ranging to refusal water  10/1/98 water level on date shown  water inflow  water outflow		notes, samples, tests U ₅₀ undisturbed sample 50mm diameter U ₆₃ undisturbed sample 63mm diameter D disturbed sample N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone V vane shear (kPa) P pressuremeter Bs bulk sample E environmental sample R refusal		classification symbols and soil description based on unified classification system moisture D dry M moist W wet W _p plastic limit W _L liquid limit		consistency/density index VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense	

E12723.3-BC
4 July 2005

APPENDIX B – AREA B

OTHER CONSULTANTS ENGINEERING LOGS



PwDI-1



GEOTECHNICAL CENTRE

BOREHOLE NO. 1

PROJECT: LONG BAY GOAL

DATE: 4.1.90

LOCATION: PROPOSED WAREHOUSE

SURFACE RL:

CONTRACTOR: GEOTECH CENTRE DRILLER: G. KELLY

RIG TYPE: GEMCO 210B

DEPTH (m)	WATER	BIT	SAMPLE OR TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grain size, moisture, remarks
0				FILL		FILL SAND Dark Brown, Loose & Damp .8
1		YEE	SPT 3, 3, 4 N = 7	SP (v)		SAND (Medium Grained) Brown, Loose & Damp
2			D			Grey Brown, Loose & Saturated END at 2.3
4.1.90						Vee Bit Refusal on Extremely Weathered Sandstone
3						
4						
5						
6						
7						
8						
9						
10						

SAMPLE OR TEST

U.....undisturbed

D.....disturbed

SPT.....standard penetration test

CPT.....cone penetration test

v : visual

l : laboratory

WATER

Water table

Water inflow

DRILLING SUPERVISOR: O. FERGUSON

PROJECT ENGINEER: D. SARIAN

SHEET 50 OF 50 SHEETS LONG BAY 1

SCALE 1:50



GEOTECHNICAL CENTRE

BOREHOLE NO. 2

PROJECT: LONG BAY GOAL

DATE: 4.1.90

LOCATION: PROPOSED WAREHOUSE

SURFACE RL:

CONTRACTOR: GEOTECH CENTRE DRILLER: G.KELLY

RIG TYPE: GEMCO 210B

DEPTH (m)	WATER	BIT	SAMPLE OR TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grainsize, moisture, remarks
0				FILL		FILL SAND & COKE (19mm), SOME RUBBLE Dark Brown, Loose & Damp .8
1		YEE	SPT 3, 3, 4 N = 7 D	SP (v)		SAND (Medium Grained) Brown, Loose & Damp
2	4.1.90					Grey Brown, Loose & Saturated END at 2.1
						Vee Bit Refusal on Extremely Weathered Sandstone.
3						
4						
5						
6						
7						
8						
9						
10						

SAMPLE OR TEST

U.....undisturbed

D.....disturbed

SPT....standard penetration test

CPT....cone penetration test

V : visual
1 : laboratory

WATER

Water table

Water inflow

DRILLING SUPERVISOR O.FERGUSON

PROJECT ENGINEER: D.SARIAN

SHEET 50 OF 50 SHEETS LONG BAY2

SCALE 1:50



GEOTECHNICAL CENTRE

BOREHOLE NO. 3

PROJECT: LONG BAY GOAL

DATE: 4.1.90

LOCATION: PROPOSED WAREHOUSE

SURFACE RL:

CONTRACTOR: GEOTECH CENTRE DRILLER: G. KELLY

RIG TYPE: GEMCO 210B

DEPTH (m)	WATER	BIT	SAMPLE OR TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grainsize, moisture, remarks
0				FILL		FILL SAND & COKE (19mm), SOME BUILDING RUBBLE. Dark Brown, Loose & Dry. .8
1		VEE	SPT 4,4,4 N = 8 D	SP (v)		SAND (Medium Grained) Brown, Loose & Damp Grey Brown, Loose & Saturated
2	4.1.90					END at 2.0 Vee Bit Refusal on Extremely Weathered Sandstone
3						
4						
5						
6						
7						
8						
9						
10						

SAMPLE OR TEST

U undisturbed

D disturbed

SPT . . . standard penetration test

CPT . . . cone penetration test

v : visual
l : laboratory

WATER

Water table

Water inflow

DRILLING SUPERVISOR: D. FERGUSON

PROJECT ENGINEER: D. SARIAN

SHEET 50 OF 50 SHEETS LONG BAY 3

SCALE 1:50



GEOTECHNICAL CENTRE

BOREHOLE NO. 4

PROJECT: LONG BAY GOAL

DATE: 4.1.90

LOCATION: PROPOSED WAREHOUSE

SURFACE RL:

CONTRACTOR: GEOTECH CENTRE DRILLER: G.KELLY

RIG TYPE: GEMCO 210B

DEPTH (m)	WATER	BIT	SAMPLE OR TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grainsize, moisture, remarks
0				FILL		FILL Extremely Weathered Sandstone .5
		VEE		SP (v)		SAND, Light Brown, Loose & Damp .8
1			SPT 4.4.4 N = 8	SP (v)		SAND (Medium Grained) Grey Brown, Loose & Damp
			D			Loose & Saturated END at 1.7
2	4.1.90					Vee Bit Refusal on Extremely Weathered Sandstone
3						
4						
5						
6						
7						
8						
9						
10						

SAMPLE OR TEST

U undisturbed

D disturbed

SPT standard penetration test

CPT cone penetration test

v : visual

l : laboratory

WATER

Water table

Water inflow

DRILLING SUPERVISOR: D. FERGUSON

PROJECT ENGINEER: D. SARIAN

SHEET OF SHEETS LONG BAY 4

SCALE 1:50



GEOTECHNICAL CENTRE

BOREHOLE NO. 5

PROJECT: LONG BAY GOAL

DATE: 4.1.90

LOCATION: PROPOSED WAREHOUSE

SURFACE RL:

CONTRACTOR: GEOTECH CENTRE DRILLER: G.KELLY

RIG TYPE: GEMCO 210B

DEPTH (m)	WATER	BIT	SAMPLE OR TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grain size, moisture, remarks
0						VEE BIT REFUSAL ON MODERATELY WEATHERED SANDSTONE
1						
2						
3						
4						
5						
6						
7						
8						
9						
10						

SAMPLE OR TEST

U.....undisturbed

D.....disturbed

SPT....standard penetration test

CPT....cone penetration test

V : visual

L : laboratory

WATER

✓ Water table

✗ Water inflow

DRILLING SUPERVISOR: O. FERGUSON

PROJECT ENGINEER: D. SARIAN

SHEET 50 OF 50 SHEETS LONG BAY 5

SCALE 1:50

PUBLIC WORKS DEPARTMENT
GEOMECHANICS LABORATORY

PWD2-1
BOREHOLE NO. 1

PROJECT : MALABAR COMPLEX OF PRISONS

DATE : 21. 7. 86

LOCATION : SECURITY TOWER No. 10

SURFACE RL. :

CONTRACTOR : GROUND TEST P.L. DRILLER : BRIAN LAMB RIG TYPE : PIONEER P 160

DEPTH (m)	WATER	BIT	SAMPLE OR TEST	SOIL GROUP	GRAPHIC LOG	PROFILE DESCRIPTION type, colour, consistency, grainsize, moisture, rem.
0				1A		REINFORCED CONCRETE (RECENTLY POURED)
1				1B		REINFORCED CONCRETE
2				2		BRICKWORK CONSTRUCTION
3				3		HAWKESBURY SANDSTONE, MED. GRAINED, MASSIVE, WHITE IN COLOUR
4						END BORE AT 3.8

<p>SAMPLE OR TEST</p> <p>U undisturbed</p> <p>D disturbed</p> <p>SPT standard penetration test</p> <p>CPT cone penetration test</p>	<p>WATER</p> <p>↘ Water table</p> <p>↘ Water inflow</p>	<p>DRILLING SUPERVISOR : S. CELOTTO</p> <p>PROJECT ENGINEER : S. CELOTTO</p> <p>SHEET 1 OF 1</p> <p>SCALE 1:50</p>
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PUBLIC WORKS DEPARTMENT
GEOMECHANICS LABORATORY

BOREHOLE NO. 2

PROJECT : MALABAR COMPLEX OF PRISONS

DATE : 21.7.86

LOCATION : SECURITY TOWER No. 10

SURFACE AL. :

CONTRACTOR : GROUND TEST P. L. DRILLER : BRIAN LAMB RIG TYPE : PIONEER P 160

DEPTH (m)	WATER	BIT	SAMPLE OR TEST	SOIL GROUP	GRAPHIC LOG	PROFILE DESCRIPTION	
						type, colour, consistency, grainsize, moisture, remarks	
0				1A		REINFORCED CONCRETE (RECENTLY POURED)	0.
1				1B		REINFORCED CONCRETE	1.
2				2		BRICKWORK CONSTRUCTION	2
3				3		HAWKESBURY SANDSTONE, MED. GRAINED, MASSIVE, WHITE IN COLOUR	
4						END BORE AT 3.9	

SAMPLE OR TEST

U undisturbed

D disturbed

SPT standard penetration test

CPT cone penetration test

WATER

Water table

Water inflow

DRILLING SUPERVISOR : S. CELOTTO

PROJECT ENGINEER : S. CELOTTO

SHEET 1 OF 1

SCALE : 50

JKI-14

JEFFERY AND KATAUSKAS PTY. LTD.

Borehole No.







14

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT
 Project: PROPOSED NEW BUILDING
 Location: KATINGAL AREA, LONG BAY GAOL. N.S.W.

Job No. 5070 JS
 Date: 28.5.87

Method: SPIRAL AUGER
 HYDRAPOWER RIG
 R.L. Surface: 39.65m
 Datum: SITE

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
DRY ON COMPLETION						FILL: ashes and sand				
					SM	SILTY SAND: fine to medium grained, dark brown, trace clay		(L)		
AFTER 5 hours	D.S.	N>>11 4,11/75mm HAMMER BOUNCING	1			SANDSTONE: medium grained, light grey, extremely weak then highly weathered, very weak				ESTIMATED 'V' BIT REFUSAL
	D.S.		2			as above, but orange brown, highly weathered, extremely weak with weak iron cemented bands.				MODERATE 'TC' BIT RESISTANCE
	D.S.		3			as above, but light grey, moderately weathered, weak to medium strong.				MODERATE RESISTANCE WITH LOW BAND
	D.S.		3.5m							MODERATE TO HIGH RESISTANCE
						END OF BOREHOLE				
			4							
			5							
			6							
			7							

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT Project: PROPOSED NEW BUILDING Location: KATINGAL AREA, LONG BAY GAOL. N.S.W.										
Job No. 5070 JS		Method: SPIRAL AUGER		R.L. Surface: 42.95m						
Date: 28.5.87		HYDRA POWER RIG		Datum: SITE						
Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/Rel. Density	Hand Penetrometer Readings kPa.	Remarks
DRY ON COMPLETION						FILL: gravelly sand, fine grained, dark grey some silt with wire netting, concrete fragments, nylon sheeting	M			POORLY COMPACTED
			1		SP	SAND: fine to medium grained, light brown then orange brown, some silt	M	L		
	D.S.	N >> 13 2, 5, 8/30mm				SANDSTONE: medium grained, light orange brown, highly weathered, weak				ESTIMATED 'V' BIT REFUSAL MODERATE TO HIGH 'TC' BIT RESISTANCE
	D.S.		2			very weak bond, grey brown				MODERATE 'TC' BIT RESISTANCE
			3			as above, but light grey, moderately weathered, medium strong.				MODERATE TO HIGH 'TC' BIT RESISTANCE
	D.S.		4.0			END OF BOREHOLE				
			5							
			6							
			7							

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT
 Project: PROPOSED NEW BUILDING
 Location: KATINGAL AREA, LONG BAY GAOL. N.S.W.

Job No. 5070 JS
 Date: 28.5.87

Method: SPIRAL AUGER R.L. Surface: 40.19m
 HYDRAPOWER RIG Datum: SITE

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
						FILL: gravelly sand				
					SP	SAND: fine to medium grained, orange brown some silt	M	MD		
	D.S.	N > 16 9,7,9/25mm	1							ESTIMATED 'V' BIT REFUSAL
		HAMMER BOUNCING				SANDSTONE: medium grained, light grey, highly weathered, weak				MODERATE 'TC' BIT RESISTANCE
			2			as above, but moderately weathered, medium strong some iron staining				HIGH 'TC' BIT RESISTANCE
						as above, but mainly light grey, moderately weathered, weak				MODERATE TO HIGH 'TC' BIT RESISTANCE
	D.S.		3.03m							
			3			END OF BOREHOLE				
			4							
			5							
			6							
			7							

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT
 Project: PROPOSED SPECIAL PURPOSE PRISON
 Location: LONG BAY GAOL, MALABAR, N.S.W.

Job No. 5212 JS
 Date: 28.7.87

Method: SPIRAL AUGER
 JACRO RIG

R.L. Surface: 40.5m APPROX.
 Datum: STANDARD

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/Rel. Density	Hand Penetrometer Readings kPa.	Remarks
						FILL: silty sand, fine to medium grained, brownish grey with some gravels	M			APPEARS POORLY COMPACTED
AFTER 5 hours	D.S.	N = 12 3, 4, 8	1		SM	SILTY SAND: fine to medium grained, brown with some clay	M W	MD		
	D.S.	N > 7 7/20mm	2		CL	SANDY CLAY: low plasticity, yellow brown and light grey, fine to medium grained sand	MC > PL			ESTIMATED 'V' BIT REFUSAL
	D.S.	AND BOUNCING	2.5m			SANDSTONE: fine to medium grained, whitish brown and light grey, highly weathered, weak.				HIGH 'T' BIT RESISTANCE
			3			END OF BOREHOLE				
			4							
			5							
			6							
			7							

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT
 Project: PROPOSED SPECIAL PURPOSE PRISON
 Location: LONG BAY GAOL, MALABAR, N.S.W.

Job No. 5212 JS
 Date: 28.7.87

Method: SPIRAL AUGER
 JACRO RIG.

R.L. Surface: 38.1 metres APPA
 Datum: STANDARD

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
DRY ON COMPLETION						FILL: silty sand, fine to medium grained, grey with some gravels.	M			APPEARS POORLY COMPACTED
		N = 12 12 / 50mm								ESTIMATED 'V' BIT REFUSAL
	D.S.	AND BOUNCING	1			SANDSTONE: fine to medium grained, brownish white and light grey, highly weathered, very weak				HIGH 'TC' BIT RESISTANCE
	D.S.		1.2m			END OF BOREHOLE				
			2							
			3							
			4							
			5							
			6							
			7							

E12723.3-BC
4 July 2005

APPENDIX B – AREA D

OTHER CONSULTANTS ENGINEERING LOGS



TEST PIT REPORT

DP-1

CLIENT MORRISON WHITTEN & NICEY PTY.LTD. DATE 6/3/86

PIT No. 1

SITE LONG BAY HOSPITAL

CONTRACT No. SSI/9508

LOCATION ANZAC PARADE, LONG BAY

SURFACE LEVEL APPROX 35.5

Depth (metres)	Description of Strata	Sampling and In-situ Testing			
		Sampling		Dynamic Penetrometer	
		Type	Depth	Depth	Blows/150mm.
S.L.	FILLING - dense medium grained grey sand with sandstone, road metal and ash	D	0.30		
0.50	FILLING - dense medium grained gravel sized slag with reddish brown medium grained sand and rootlets	B	0.50		
		D	0.55		
0.60					
	SAND - dense medium grained light grey sand	D	1.00		
1.40					
1.50	CEMENTED SAND - dense dark brown medium grained cemented sand	D	1.50		
	SAND - dense light yellowish brown, medium grained sand	D	2.00		
		D	2.50		
2.70		D	2.70		
TEST PIT DISCONTINUED AT 2.70 METRES.					

EQUIPMENT Backhoe

TECHNICIAN

Willey

SAMPLE TYPE

PIT SIZE 3 m x 0.5 m

D Disturbed Sample

WATER LEVEL No free ground water observed

B Bulk Sample

REMARKS

U Undisturbed Sample



D.J. Douglas & Partners

TEST PIT REPORT

DP-2

CLIENT MORRISON WHITTEN & NICEY PTY. LTD DATE 6/3/86

PIT No. 2

SITE LONG BAY HOSPITAL

CONTRACT No. SSI/9508

LOCATION ANZAC PARADE, LONG BAY

SURFACE LEVEL Approx. 34.5

Depth (metres)	Description of Strata	Sampling and In-situ Testing			
		Sampling		Dynamic Penetrometer	
		Type	Depth	Depth	Blows/150mm.
S.L.	TOPSOIL - dark brown silty sand with rootlets				
0.25		2B	0.50		
		D	1.00		
		D	1.50		
	SAND - medium dense very light brown, medium grained sand, grading to dense with depth	D	2.00		
2.40	SANDSTONE - medium strong very light brown medium grained sandstone	D	2.40		
	<u>REFUSAL AT 2.40 METRES.</u>				

EQUIPMENT Backhoe

TECHNICIAN Willey

SAMPLE TYPE

PIT SIZE 5 m x 3 m

D Disturbed Sample

WATER LEVEL No free ground water observed

B Bulk Sample

REMARKS

U Undisturbed Sample



D.J. Douglas & Partners

TEST PIT REPORT

DP-3

CLIENT MORRISON WHITTEN & NICEY PTY. LTD DATE 6/3/86

PIT No. 3

SITE LONG BAY HOSPITAL

CONTRACT No. SS1/9508

LOCATION ANZAC PARADE, LONG BAY

SURFACE LEVEL Approx. 35.6

Depth (metres)	Description of Strata	Sampling and In-situ Testing			
		Sampling		Dynamic Penetrometer	
		Type	Depth	Depth	Blows/150mm.
S.L.	TOPSOIL - dark grey silty medium grained sand with rootlets				
0.40		2B	0.50		
	SAND - dense light grey, medium grained sand	D	1.00		
1.40					
1.50	SAND - dense dark brown medium grained weakly cemented sand	D	1.50		
	SAND - dense yellowish brown medium grained sand	D	2.00		
2.50		D	2.50		
	TEST PIT DISCONTINUED AT 2.50 METRES, DUE TO EXCESSIVE CAVING.				

EQUIPMENT Backhoe TECHNICIAN Willey

PIT SIZE 3 m x 0.5 m

WATER LEVEL No free ground water observed

REMARKS

SAMPLE TYPE

D Disturbed Sample

B Bulk Sample

U Undisturbed Sample



D.J. Douglas & Partners

TEST PIT REPORT

DP-4

CLIENT MORRISON WHITTEN & NICEY PTY. LTD. DATE 6/3/86

PIT No. 4

SITE LONG BAY HOSPITAL

CONTRACT No. SSI/9508

LOCATION ANZAC PARADE, LONG BAY

SURFACE LEVEL Approx. 36.0

Depth (metres)	Description of Strata	Sampling and In-situ Testing			
		Sampling		Dynamic Penetrometer	
		Type	Depth	Depth	Blows/150mm.
S.L.	FILLING - dense dark brown silty medium grained sand with rubble, scrap metal and concrete				
0.70		2B	0.70		
	SAND - very dense light grey medium grained sand	D	1.00		
1.60		D	1.50		
1.75	SAND - dense dark brown weakly cemented medium grained sand				
		D	2.00		
	SAND - very dense yellowish brown medium grained sand				
2.50		D	2.50		
TEST PIT DISCONTINUED AT 2.50 METRES.					
DUE TO EXCESSIVE CAVING.					

EQUIPMENT Backhoe

TECHNICIAN Willey

SAMPLE TYPE

PIT SIZE 3 m x 1 m

D Disturbed Sample

WATER LEVEL No free ground water observed

B Bulk Sample

REMARKS

U Undisturbed Sample



D.J. Douglas & Partners

TEST PIT REPORT

DP-5

CLIENT MORRISON WHITTEN & NICEY PTY. LTD. DATE 6/3/86

PIT No. 5

SITE LONG BAY HOSPITAL

CONTRACT No. SSI/9508

LOCATION ANZAC PARADE, LONG BAY

SURFACE LEVEL Approx. 33.5

Depth (metres)	Description of Strata	Sampling and In-situ Testing			
		Sampling		Dynamic Penetrometer	
		Type	Depth	Depth	Blows/150mm.
S.L.	FILLING - medium dense dark grey silty sand with sandstone concrete bricks and scrap metal				
2.60					
	REFUSAL AT 2.60 METRES.				

EQUIPMENT Backhoe

TECHNICIAN Willey

SAMPLE TYPE

PIT SIZE 3 m x 1 m

D Disturbed Sample

WATER LEVEL No free ground water during digging.

B Bulk Sample

REMARKS Free ground water observed at 2.50 m
after $\frac{1}{2}$ hour.

U Undisturbed Sample



D.J. Douglas & Partners

TEST PIT REPORT

DP-6

CLIENT MORRISON WHITTEN & NICEY PTY. LTD. DATE 6/3/86

PIT No. 6

SITE LONG BAY HOSPITAL

CONTRACT No. SSI/9508

LOCATION ANZAC PARADE, LONG BAY

SURFACE LEVEL Approx. 32.5

Depth (metres)	Description of Strata	Sampling and In-situ Testing			
		Sampling		Dynamic Penetrometer	
		Type	Depth	Depth	Blows/150mm.
S.L.		B	0.50		
		D	1.00		
	FILLING - dense dark grey silty sand with bricks, concrete, scrap metal, cloth and plastic	D	1.50		
		D	2.00		
2.60	SANDSTONE - medium strong very light brown sandstone	D	2.50		
	<u>REFUSAL AT 2.60 METRES.</u>				

EQUIPMENT Backhoe

TECHNICIAN Willey

SAMPLE TYPE

PIT SIZE 3 m x 1 m

D Disturbed Sample

WATER LEVEL Free ground water observed at 2.50 m.

B Bulk Sample

REMARKS Water level at 2.50 m after 1 hour.

U Undisturbed Sample



D.J. Douglas & Partners

TEST PIT REPORT

DP-7

CLIENT MORRISON WHITTEN & NICEY PTY. LTD. DATE 6/3/86

PIT No. 7

SITE LONG BAY HOSPITAL

CONTRACT No. SSI/9508

LOCATION ANZAC PARADE, LONG BAY

SURFACE LEVEL Approx.

Depth (metres)	Description of Strata	Sampling and In-situ Testing			
		Sampling		Dynamic Penetrometer	
		Type	Depth	Depth	Blows/150mm.
S.L.	FILLING - dark brown silty sand with sandstone, road metal				
0.50					
	SAND - light yellowish brown medium grained sand				
1.50					
	TEST PIT DISCONTINUED AT 1.50 METRES.				

EQUIPMENT Backhoe

TECHNICIAN Willey

SAMPLE TYPE

PIT SIZE 3 m x 1 m

D Disturbed Sample

WATER LEVEL No free ground water observed

B Bulk Sample

REMARKS

U Undisturbed Sample



D.J. Douglas & Partners


TEST PIT REPORT

DP-8

CLIENT MORRISON WHITTEN & NICEY PTY. LTD. DATE 6/3/86 PIT No. 8
 SITE LONG BAY HOSPITAL CONTRACT No. SSI/9508
 LOCATION ANZAC PARADE, LONG BAY SURFACE LEVEL Approx. 31.5

Depth (metres)	Description of Strata	Sampling and In-situ Testing			
		Sampling		Dynamic Penetrometer	
		Type	Depth	Depth	Blows/150mm.
S.L.					
		B	0.50		
		D	1.00		
	FILLING - dark brown silty sand with scrap metal, bricks, bottles, sandstone, plastic	D	1.50		
		D	2.00		
2.40	SAND - light grey medium grained sand	D	2.50		
2.60					
	TEST PIT DISCONTINUED AT 2.60 METRES, DUE TO EXCESSIVE CAVING.				

EQUIPMENT Backhoe TECHNICIAN Willey SAMPLE TYPE
 PIT SIZE 3 m x 1 m D Disturbed Sample
 WATER LEVEL Free ground water observed at 2.10 m. B Bulk Sample
 REMARKS Water level at 2.10 m after 1 hour 50min U Undisturbed Sample

 D.J. Douglas & Partners

LOCATION DEPTH (m)	PENETRATION RESISTANCE BLOWS / 150 mm.											
	DP -1	DP -2	DP -3	DP -4	DP -5	DP -6	DP -8	DP -9	DP -10	DP -11	DP -12	
0.00 - 0.15	4	1	4	10	3	8	10/20mm	4	4	1	4	
0.15 - 0.30	6	3	14	9	4	5		8	7	4	2	
0.30 - 0.45	6	5	14	12	6	6		8	7	4	2	
0.45 - 0.60	6	3	13	20	3	7		11	9	3	1	
0.60 - 0.75	11	4	11	10	3	7		12	20/80	3	2	
0.75 - 0.90	6	4	8	10	3	10		12		3	2	
0.90 - 1.05	7	4	7	18	1	11		13/50		8	2	
1.05 - 1.20	4	4	6	17	2	11				7	2	
1.20 - 1.35	7	4	9	14	3	9				4	3	
1.35 - 1.50	5	5	12	10	3	8				4	5	
1.50 - 1.65	5	6	11	9/30mm	8	6				4	7	
1.65 - 1.80	5	6	9		7	5				4	4	
1.80 - 1.95												
1.95 - 2.10												
2.10 - 2.25												
2.25 - 2.40												
2.40 - 2.55												
2.55 - 2.70												
2.70 - 2.85												
2.85 - 3.00												
3.15 - 3.30												
3.30 - 3.45												
3.45 - 3.60												
3.60 - 3.75												
3.75 - 3.90												
3.90 - 4.05												
4.05 - 4.20												
4.20 - 4.35												

AUSTRALIAN STANDARD AS 1289 ~~6302~~ F3.3

RESULTS OF DYNAMIC PENETROMETER TESTING

SITE ANZAC PARADE, LONG BAY

CLIENT MORRISON WHITTEN AND NICEY PTY. LTD.

REPORT No. 9508

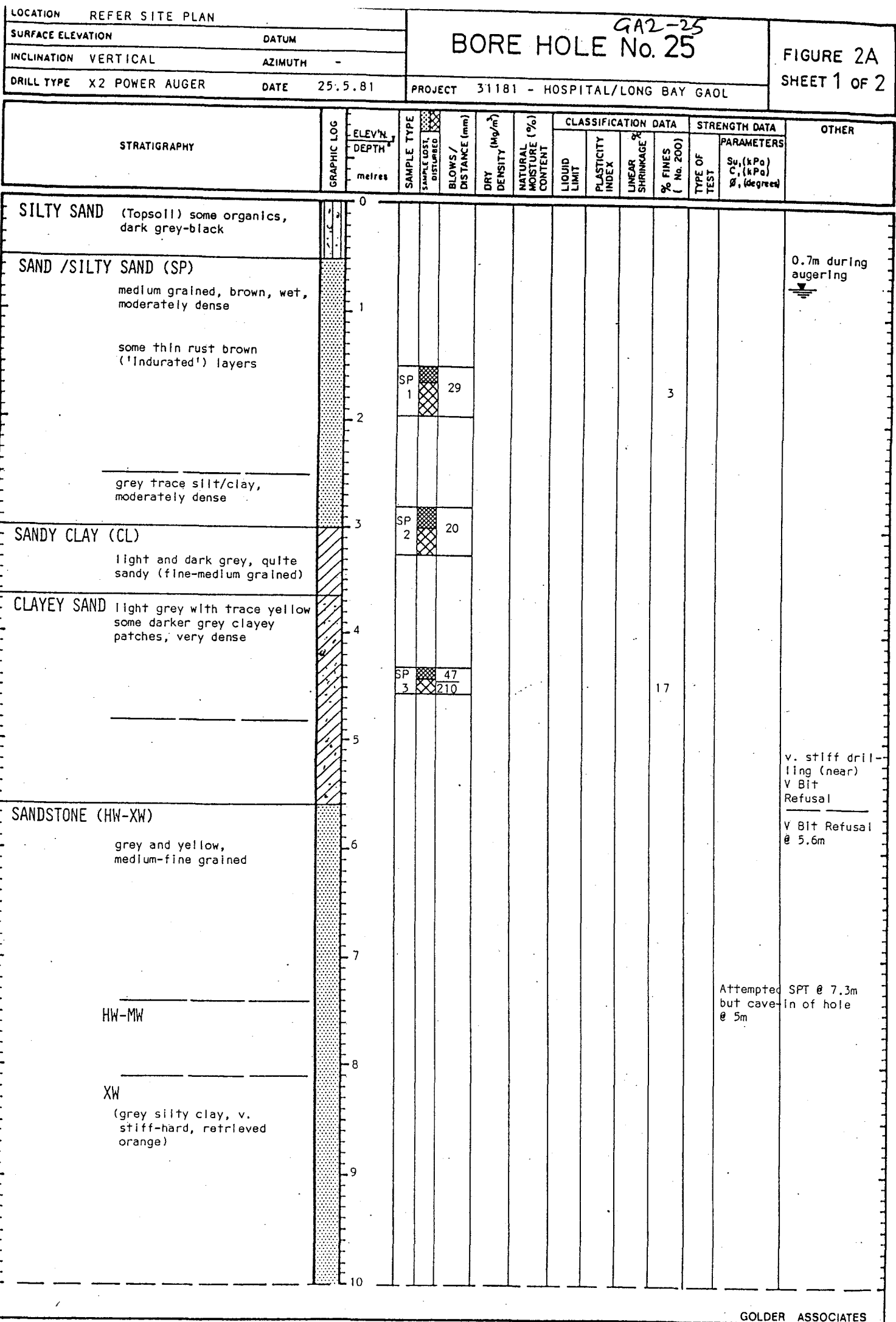
DATE 6/3/86



This Laboratory is registered by the National Association of Testing Authorities, Australia. The test(s) reported herein have been performed in accordance with its terms of registration.

GROUND TEST PTY LIMITED

A subsidiary of D.J. Douglas & Partners Pty. Ltd.



SURFACE ELEVATION	DATUM
INCLINATION VERTICAL	AZIMUTH -
DRILL TYPE X2 POWER AUGER	DATE 25.5.81

GAZ-26
BORE HOLE No. 26

PROJECT 31181 - HOSPITAL/LONG BAY GAOL

FIGURE 2C
SHEET 1 OF 2

STRATIGRAPHY	GRAPHIC LOG	ELEV'N. DEPTH metres	SAMPLE TYPE SAMPLE LOST DISTURBED	BLOWS/ DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE %	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su (kPa) C _u (kPa) σ _v (degrees)	
SANDY PEAT black-v. dark brown, organic peat, silt and some sand, smelly, wet, v. soft		0	AS 1			42	Organic content				=	6 %	
SAND (SP) brown, medium grained, very loose, wet		1	SP 2	4									0.8m water table during augering
SANDY CLAY grey and darker grey, soft, wet		2											
SANDY CLAY (CH) grey, very moist, quite silty, stiff		3	SP 3	13			27	13	6		* pp	150-200kPa	
CH/CL, some fine sand, firm-stiff		4											
CL		5	SP 4	31									
SAND (SP) siliceous, grey and dark grey, medium grained, moderately dense		6											
very dense, grading to XW sandstone		7											
CLAY (CL) silty clay XW sandstone some sandy layers		8											
		9											
thin layers 0.2-0.3 peaty brown, indurated sand		10											

* pp - Pocket Penetrometer test, measures UCS value

GOLDER ASSOCIATES

SURFACE ELEVATION

DATUM

INCLINATION

VERTICAL

AZIMUTH

DRILL TYPE

X2 POWER AUGER

DATE

25.5.81

PROJECT

31181 - HOSPITAL/LONG BAY GAOL

GAZ-26
BORE HOLE No. 26

FIGURE 2D

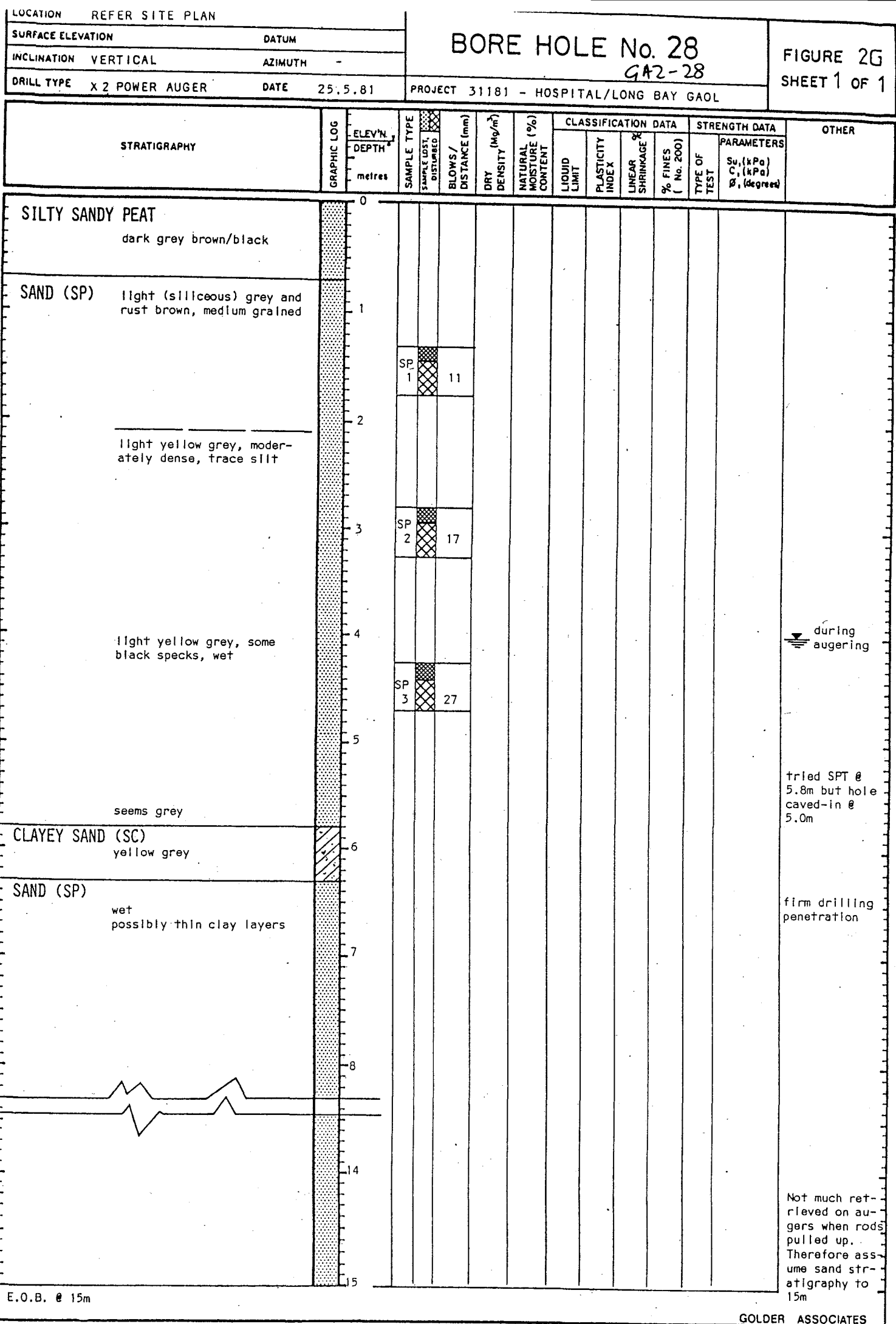
SHEET 2 OF 2

STRATIGRAPHY	GRAPHIC LOG ELEV'N. DEPTH metres	SAMPLE TYPE SAMPLE LOST, DISTURBED	BLOWS/ DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
						LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
SILTY CLAY (CL) grey, some sandy layers	10											
SANDSTONE (XW) grey, medium grained some ironstone	11											
(HW-MW) grey and yellow	12											
	13											
	14											V Bit Ref- usal @ 13.5m
END OF BORING @ 15m	15											Very stiff drilling @ 14.5m

GOLDER ASSOCIATES

SURFACE ELEVATION		DATUM		BORE HOLE No. 27 GA2-27				FIGURE 2F SHEET 2 OF 2	
INCLINATION VERTICAL		AZIMUTH -							
DRILL TYPE X2 POWER AUGER		DATE 25.5.81		PROJECT 31181 - HOSPITAL/LONG BAY GAOL					

STRATIGRAPHY	GRAPHIC LOG	ELEV'N DEPTH metres	SAMPLE TYPE	SAMPLE USE: DISTURBED	BLOWS/ DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE CONTENT (%)	CLASSIFICATION DATA				STRENGTH DATA		OTHER
								LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
SANDY CLAY (CL) grey, red brown and yellow banded		10												
		11												
		12												
		13												Very slow drilling @ 13m
CLAYEY SAND (SC) grey/yellow		14												V Bit Ref. used @ 14.1m
		15												Augered to 15m with TC Bit
END OF BORING @ 15m														



LOCATION REFER SITE PLAN		<div style="display: flex; justify-content: space-between;"> <div> BORE HOLE No. 29 GA2-29 </div> <div> FIGURE 2H SHEET 1 OF 1 </div> </div>									
SURFACE ELEVATION	DATUM										
INCLINATION VERTICAL	AZIMUTH -										
DRILL TYPE X2 POWER AUGER	DATE 26.5.81	PROJECT 31181 - HOSPITAL/LONG BAY GAOL									

STRATIGRAPHY	GRAPHIC LOG	ELEV'N. DEPTH metres	SAMPLE TYPE SAMPLE LOST, DISTURBED	BLOWS / DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%)	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE %	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
SILTY PEATY SAND black-v.dk. brown, highly organic		0											
SAND (SP) medium-fine grained, dark grey													
SANDSTONE (HW-MW) light grey/white and yellow/brown banded, medium grained, mostly friable		1											
END OF BOREHOLE @ 1.5m GROUNDWATER NOT ENCOUNTERED		2											V Bit Refusal @ 0.7m Slow auger progress - jerky TC Bit Refusal @ 1.5m

LOCATION REFER SITE PLAN		<div style="display: flex; justify-content: space-between;"> <div> BORE HOLE No. 30 GA2-30 </div> <div> FIGURE SHEET OF </div> </div>									
SURFACE ELEVATION	DATUM										
INCLINATION VERTICAL	AZIMUTH -										
DRILL TYPE X2 POWER AUGER	DATE 26.5.81	PROJECT 31181 - HOSPITAL/LONG BAY GAOL									

STRATIGRAPHY	GRAPHIC LOG	ELEV'N. DEPTH metres	SAMPLE TYPE SAMPLE LOST, DISTURBED	BLOWS / DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%)	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE %	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
TOPSOIL silty sand, dk. grey/black, rootlets, organics, v.moist/wet		0											
SANDSTONE (MW-HW) light grey/white													
SANDSTONE (MW-HW) some thin brown bands		1											
END OF BOREHOLE @ 1.5m GROUNDWATER NOT ENCOUNTERED		2											V Bit Refusal @ 0.6m TC Bit Refusal @ 1.5m

LOCATION REFER SITE PLAN		<div style="display: flex; justify-content: space-between;"> <div> BORE HOLE No. 31 GA2-31 </div> <div> FIGURE SHEET OF </div> </div>									
SURFACE ELEVATION	DATUM										
INCLINATION VERTICAL	AZIMUTH -										
DRILL TYPE X2 POWER AUGER	DATE 26.5.81	PROJECT 31181 - HOSPITAL/LONG BAY GAOL									

STRATIGRAPHY	GRAPHIC LOG	ELEV'N. DEPTH metres	SAMPLE TYPE SAMPLE LOST, DISTURBED	BLOWS / DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%)	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE %	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
TOPSOIL silty sand, black/brown, highly organic		0											
SILTY SAND (SM) dark grey													
SANDSTONE (MW-HW) light grey/white/yellow, medium grained		1											
END OF BOREHOLE @ 1.5m GROUNDWATER NOT ENCOUNTERED		2											V Bit Refusal @ 0.9m TC Bit Refusal @ 1.5m

SURFACE ELEVATION	DATUM
INCLINATION VERTICAL	AZIMUTH
DRILL TYPE X 2 POWER AUGER	DATE 26.5.81

BORE HOLE No. 34

GA2-34

PROJECT 31181 - HOSPITAL/LONG BAY GAOL

FIGURE 2K
SHEET 1 OF 1

STRATIGRAPHY	GRAPHIC LOG	ELEV'N. DEPTH metres	SAMPLE TYPE	SAMPLE LOSS DISTURBED	BLOWS/ DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
								LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE (%)	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
FILL sand, sandstone gravels and cobbles and silt, grey brown and white mixed		0												
SAND (SP) grey brown light grey, medium grained, moist, mod. dense		1												
		2												during augering
		3												V Bit Ref- usal @ 3.1m
SANDSTONE (HW-MW) light grey/white, friable		4												Stiff going, some softer patches
END OF BOREHOLE @ 4.8m		5												TC Bit Ref- usal @ 4.8m

LOCATION		REFER SITE PLAN		BORE HOLE No. 35 GA2-35		FIGURE 2L SHEET 1 OF 1			
SURFACE ELEVATION		DATUM							
INCLINATION		VERTICAL						AZIMUTH	
DRILL TYPE		X2 POWER AUGER		DATE		26.5.81		PROJECT 31181 - HOSPITAL/LONG BAY GAOL	

STRATIGRAPHY	GRAPHIC LOG	ELEV'N. DEPTH metres	SAMPLE TYPE	SAMPLE LOSS DISTURBED	BLOWS / DISTANCE (mm)	DRY DENSITY (Mg/m³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
								LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
FILL silty sand, some sandstone cobbles, light grey/dark brown		0												
SAND (SP) grey brown														
dark rust brown (Indurated)														
orange brown		1												V Bit Ref- usal @ 1.0m
SANDSTONE (MW) yellow/brown and grey														TC Bit Ref- usal @ 1.3m
END OF BOREHOLE @ 1.3m		1.3												
		2												

LOCATION REFER SITE PLAN			TEST PIT 1 TP1										FIGURE 3A							
SURFACE ELEV.		DATUM		SAMPLE DATA			MOIST. DENSITY		CLASSIFICATION DATA					TEST RESULTS REMARKS						
METHOD BACKHOE - JCB		DATE 25.5.81		TYPE		BLOWS PER 0.3 METRES		DRY DENSITY Mg/m ³		NATURAL MOISTURE CONT. (%)		LIQUID LIMIT			PLASTICITY INDEX		LINEAR SHRINKAGE percent		% FINES (< No. 200)	
GROUNDWATER 1.6m		DATE 25.5.81		LOG		ELEV'N DEPTH metres														
FILL - approximately: 80% silt, sand with some grass, rootlets and other organics 20% bricks, building rubble etc. (black-dark grey, smelly)																				<div style="text-align: center;"> 1.6m Pit tended to cave-in during excavation </div>
SILTY SAND (SP) brown, fine to medium grained seems moderately dense																				
SANDY/SILTY CLAY (CL) grey with brown mottling seems firm-stiff																				
DEPTH OF PIT: 3.9m																				No backhoe refusal

LOCATION REFER SITE PLAN			TEST PIT 2 TP2																	
SURFACE ELEV.		DATUM		SAMPLE DATA			MOIST. DENSITY		CLASSIFICATION DATA					TEST RESULTS REMARKS						
METHOD BACKHOE - JCB		DATE 25.5.81		TYPE		BLOWS PER 0.3 METRES		DRY DENSITY Mg/m ³		NATURAL MOISTURE CONT. (%)		LIQUID LIMIT			PLASTICITY INDEX		LINEAR SHRINKAGE percent		% FINES (< No. 200)	
GROUNDWATER 2.3m approx.		DATE 25.5.81		LOG		ELEV'N DEPTH metres														
FILL - approximately: 50% bricks, brickbats, tiles, parts of brickwalls, etc. 20% timber (bed frames, cupboards etc). 10% metal - plumbing pipes, springs, etc. 20% sand, dark grey brown (Fill difficult to excavate, mainly due to pipes, uncompacted, some voids evident)																				<div style="text-align: center;"> 2.3m during excavation </div>
<div style="text-align: center;"> wet sandier </div>																				
BAND (SP) some silt, grey, seems moderately dense																				
DEPTH OF PIT: 4.0m																				No backhoe refusal

LOCATION REFER SITE PLAN				TEST PIT 3 TP3										FIGURE 3B									
SURFACE ELEV.		DATUM		METHOD BACKHOE - JCB		DATE 25.5.81		SAMPLE DATA		MOIST.DENSITY		CLASSIFICATION DATA						TEST RESULTS REMARKS					
GROUNDWATER Not Encountered				DATE 25.5.81				TYPE		BLOWS PER 0-3 METRES		DRY DENSITY Mg/m ³		NATURAL MOISTURE CONT.(%)		LIQUID LIMIT			PLASTICITY INDEX		LINEAR SHRINKAGE, Percent		% FINES (< No.200)
STRATIGRAPHY				LOG	ELEV'N DEPTH metres	TYPE	SAMPLE LOST, DISTURBED	BLOWS PER 0-3 METRES	DRY DENSITY Mg/m ³	NATURAL MOISTURE CONT.(%)	LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE, Percent	% FINES (< No.200)									
FILL - approximately: 70% sand and silt, yellow grey with some dark grey brown (topsoil and indurated sand) 20% sandstone cobbles and boulders, concrete slabs to 1m wide 10% timber and bits of metal, eg pipe rubble (Pit slightly smelly, Fill seems loose except on surface)					1																		
SANDSTONE - XW-MW light grey (MW)					3										Backhoe refusal @ 3.1m								

DEPTH OF PIT: 3.1m

LOCATION - REFER SITE PLAN				TEST PIT 4 TP4																			
SURFACE ELEV.		DATUM		METHOD BACKHOE - JCB		DATE 25.5.81		SAMPLE DATA		MOIST.DENSITY		CLASSIFICATION DATA						TEST RESULTS REMARKS					
GROUNDWATER Not Encountered				DATE 25.5.81				TYPE		BLOWS PER 0-3 METRES		DRY DENSITY Mg/m ³		NATURAL MOISTURE CONT.(%)		LIQUID LIMIT			PLASTICITY INDEX		LINEAR SHRINKAGE, Percent		% FINES (< No.200)
STRATIGRAPHY				LOG	ELEV'N DEPTH metres	TYPE	SAMPLE LOST, DISTURBED	BLOWS PER 0-3 METRES	DRY DENSITY Mg/m ³	NATURAL MOISTURE CONT.(%)	LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE, Percent	% FINES (< No.200)									
FILL - approximately: 70% sand and silt, very dark grey/black 25% bricks, brickbats and concrete slabs to 0.5m wide 5% timber, cloth rags, organics, etc. (Pit slightly smelly, fill seems loose except on surface)					1																		
SANDSTONE (HW-MW) grey (MW)					3										Backhoe refusal @ 2.4m								

DEPTH OF PIT: 2.4m

LOCATION REFER SITE PLAN				TEST PIT 5 TP5										FIGURE 3C									
SURFACE ELEV.		DATUM		SAMPLE DATA			MOIST. DENSITY		CLASSIFICATION DATA					TEST RESULTS REMARKS									
METHOD BACKHOE - JCB		DATE 25.5.81		BLOWS PER 0-3 METRES		DRY DENSITY Mg/m ³		NATURAL MOISTURE CONT. (%)		LIQUID LIMIT		PLASTICITY INDEX				LINEAR SHRINKAGE Percent		% FINES (< No. 200)					
GROUNDWATER 1.4m		DATE 25.5.81																					
STRATIGRAPHY			LOG	ELEV'N DEPTH metres	TYPE		SAMPLE LOST, DISTURBED		BLOWS PER 0-3 METRES		DRY DENSITY Mg/m ³		NATURAL MOISTURE CONT. (%)		LIQUID LIMIT		PLASTICITY INDEX		LINEAR SHRINKAGE Percent		% FINES (< No. 200)		
FILL - approximately: 50% silty sand 50% bricks and sandstone boulders/cobbles				1																			
approximately: 80% bricks, sandstone boulders and concrete slabs to 0.7m wide wet 20% timber, metal and some organic matter (Pit quite smelly)				2																			
Possibly sandstone below 2.7m depth			1	3																			

Caveing in of pit @ 1.4m
During excavation

Backhoe refusal
Not certain due to collapse of pit from 1.4m

DEPTH OF PIT: 2.7m

LOCATION REFER SITE PLAN				TEST PIT 6 TP6																		
SURFACE ELEV.		DATUM		SAMPLE DATA			MOIST. DENSITY		CLASSIFICATION DATA					TEST RESULTS REMARKS								
METHOD BACKHOE - JCB		DATE 25.5.81		BLOWS PER 0-3 METRES		DRY DENSITY Mg/m ³		NATURAL MOISTURE CONT. (%)		LIQUID LIMIT		PLASTICITY INDEX				LINEAR SHRINKAGE Percent		% FINES (< No. 200)				
GROUNDWATER Not Encountered		DATE 25.5.81																				
STRATIGRAPHY			LOG	ELEV'N DEPTH metres	TYPE		SAMPLE LOST, DISTURBED		BLOWS PER 0-3 METRES		DRY DENSITY Mg/m ³		NATURAL MOISTURE CONT. (%)		LIQUID LIMIT		PLASTICITY INDEX		LINEAR SHRINKAGE Percent		% FINES (< No. 200)	
FILL - approximately: 80% sand - yellow, grey and dark brown with some black (indurated) layers 20% granular fill eg sandstone boulders to 0.3m wide, brickbats				1																		
lot of rags and other smelly organic matter				2																		
SAND (SP) yellow grey																						
SANDSTONE (MW)				3																		

Backhoe refusal @ 2.3m

DEPTH OF PIT: 2.3m

LOCATION REFER SITE PLAN				TEST PIT 7 TP7										FIGURE 30			
SURFACE ELEV.		DATUM		SAMPLE DATA			MOIST. DENSITY		CLASSIFICATION DATA					TEST RESULTS REMARKS			
METHOD BACKHOE - JCB		DATE 25.5.81		BLOWS PER 0-3 METRES		DRY DENSITY Mg/m ³		NATURAL MOISTURE CONT. (%)		LIQUID LIMIT		PLASTICITY INDEX				LINEAR SHRINKAGE percent	
GROUNDWATER 2.1m approx. DATE 25.5.81				LOG		ELEV'N DEPTH metres											
STRATIGRAPHY																	
FILL - approximately: 90% sand - grey and yellow, some black/grey (organic topsoil) layers moist, trace bricks etc. 10% organics (black) and other granular material, eg bricks sandier seems natural lime sand						1											
						2											
						3											
No backhoe refusal																	

DEPTH OF PIT 3.9m

LOCATION REFER SITE PLAN				TEST PIT 8 TP8										TEST RESULTS REMARKS			
SURFACE ELEV.		DATUM		SAMPLE DATA			MOIST. DENSITY		CLASSIFICATION DATA								
METHOD BACKHOE - JCB		DATE 25.5.81		BLOWS PER 0-3 METRES		DRY DENSITY Mg/m ³		NATURAL MOISTURE CONT. (%)		LIQUID LIMIT		PLASTICITY INDEX		LINEAR SHRINKAGE percent		% FINES (< No. 200)	
GROUNDWATER 2.7m approx. DATE 25.5.81				LOG		ELEV'N DEPTH metres											
STRATIGRAPHY																	
FILL - approximately 80%-90% sand - grey and yellow with some dark grey/black layers seems moderately dense 10%-20% granular fill, eg timber, sandstone cobbles, rags, and metal rubble, smelly						1											
						2											
						3											
SANDSTONE (XW-HW) yellow and white, medium grained Possibly large floater though																	
Backhoe refusal @ 3.8m																	

DEPTH OF PIT: 3.8m

SURFACE ELEV. DATUM

METHOD BACKHOE - JCB

DATE 25.5.81

GROUNDWATER 3.0m

DATE 25.5.81

TEST PIT 9 TP9

FIGURE 3E

METHOD	BACKHOE - JCB	DATE	25.5.81	SAMPLE DATA			MOIST.DENSITY		CLASSIFICATION DATA					TEST RESULTS REMARKS
GROUNDWATER 3.0m		DATE 25.5.81		TYPE	SAMPLE LOST, DISTURBED	BLOWS PER 0-3 METRES	DRY DENSITY Mg/m ³	NATURAL MOISTURE CONT.(%)	LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE, Percent	% FINES (< No.200)		
STRATIGRAPHY		LOG	ELEV'N DEPTH metres											
FILL - approximately 90% sand - organe brown, fine grained 10% granular rubble, eg timber, bricks, sandstone cobbles, etc.													Sides of pit vertical below 2.0m During augering Sloppy black material retrieved below 3.0m No backhoe refusal (stiff digging too)	
a lot of rags, timber and other smelly organics @ 1.9m														
SANDY CLAY/CLAYEY SAND brown (possibly fill)														
peaty zone, many roots, dark brown @ 3.0m														

DEPTH OF PIT: 4.0m

LOCATION REFER SITE PLAN

SURFACE ELEV. DATUM


METHOD BACKHOE - JCB

DATE 25.5.81

GROUNDWATER 3.1m

DATE 25.5.81

TEST PIT 10 TP10

LOCATION REFER SITE PLAN			TEST PIT 10 TPIo										
SURFACE ELEV. DATUM													
METHOD BACKHOE - JCB		DATE 25.5.81		SAMPLE DATA		MOIST.DENSITY		CLASSIFICATION DATA					TEST RESULTS REMARKS
GROUNDWATER 3.1m		DATE 25.5.81		TYPE	SAMPLE LOST DISTURBED	BLOWS PER 0-3 METRES	DRY DENSITY Mg/m ³	NATURAL MOISTURE CONT.(%)	LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE percent	% FINES (\leq No.200)	
STRATIGRAPHY		LOG	ELEV'N DEPTH metres										
FILL - approximately: 80% sand - mostly dark grey-black, medium grained 20% bricks, timber, concrete blocks to 1 1/2m wide, bits of reinforcing wire etc.				1									
_____				2									
clayier zone				3									
SAND (SP) dark brown, some silt, indurated sand, seems original topsoil 3.0 to 3.2m													
													Sides of pit near vertical below 2.5m
													 3.1m
													No backhoe refusal

DEPTH OF PIT 4.0m

LOCATION REFER SITE PLAN			TEST PIT 11 TP11										FIGURE 3F	
SURFACE ELEV.		DATUM		SAMPLE DATA			MOIST. DENSITY		CLASSIFICATION DATA					TEST RESULTS REMARKS
METHOD BACKHOE - JCB		DATE 25.5.81		BLOWS PER 0-3 METRES			DRY DENSITY Mg/m ³		NATURAL MOISTURE CONTENT (%)					
GROUNDWATER 3.2m		DATE 25.5.81												
STRATIGRAPHY			LOG	ELEV'N DEPTH metres	TYPE	SAMPLE LOST DISTURBED	BLOWS PER 0-3 METRES	DRY DENSITY Mg/m ³	NATURAL MOISTURE CONT. (%)	LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE Percent	% FINES (< No. 200)	
FILL - approximately: 70% sand - dark grey/black 30% building rubble, eg timber, tyres, many steel or iron water pipes, concrete slabs to 1/2m wide, bricks, corrugated iron, rags and other miscellaneous fill (smelly)				1										
				2										
				3										
Seems original topsoil (black peaty sand) from 4.0 to 4.2m DEPTH OF PIT: 4.2m				4										

3.2m
Mainly black sloop
retrieved below 3.3m

No backhoe refusal

LOCATION REFER SITE PLAN			TEST PIT 12 TP12										FIGURE 3F	
SURFACE ELEV.		DATUM		SAMPLE DATA			MOIST. DENSITY		CLASSIFICATION DATA					TEST RESULTS REMARKS
METHOD BACKHOE - JCB		DATE 25.5.81		BLOWS PER 0-3 METRES			DRY DENSITY Mg/m ³		NATURAL MOISTURE CONTENT (%)					
GROUNDWATER Not Encountered		DATE 25.5.81												
STRATIGRAPHY			LOG	ELEV'N DEPTH metres	TYPE	SAMPLE LOST DISTURBED	BLOWS PER 0-3 METRES	DRY DENSITY Mg/m ³	NATURAL MOISTURE CONT. (%)	LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE Percent	% FINES (< No. 200)	
FILL - approximately: 70% sand - dark grey brown 30% metal pipes (similar to type used in scaffolding), wire screens, sheet iron				1										
				2										
				3										

Backhoe unable to dig
thru metal pipe and
sheeting iron tangle

DEPTH OF PIT: 2.0m



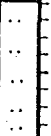
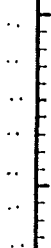
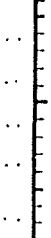
LOCATION REFER SITE PLAN				TEST PIT 13 TP13										FIGURE 3G		
SURFACE ELEV.		DATUM		METHOD BACKHOE - JCB		DATE 25.5.81		SAMPLE DATA		MOIST.DENSITY		CLASSIFICATION DATA				TEST RESULTS REMARKS
GROUNDWATER Not Encountered		DATE 25.5.81														
STRATIGRAPHY				LOG	ELEV'N DEPTH metres	TYPE	SAMPLE LOG DISTURBED	BLOWS PER 0.3 METRES	DRY DENSITY Mg/m ³	NATURAL MOISTURE CONT.(%)	LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE percent	% FINES (\leq No.200)	Backhoe refusal @ 0.7m	
TOPSOIL silt and sand, dark grey brown/ black, organic																
SILTY SAND (SM) dark grey some rust brown indurated layers																
SANDSTONE (HW-MW)					1											
					2											
					3											

DEPTH OF PIT: 0.7m


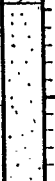
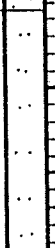

LOCATION REFER SITE PLAN				TEST PIT 14 TP14												
SURFACE ELEV.		DATUM		METHOD BACKHOE - JCB		DATE 25.5.81		SAMPLE DATA		MOIST.DENSITY		CLASSIFICATION DATA				TEST RESULTS REMARKS
GROUNDWATER 3.4m		DATE 25.5.81														
STRATIGRAPHY				LOG	ELEV'N DEPTH metres	TYPE	SAMPLE LOG DISTURBED	BLOWS PER 0.3 METRES	DRY DENSITY Mg/m ³	NATURAL MOISTURE CONT.(%)	LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE percent	% FINES (\leq No.200)	3.4m No backhoe refusal	
FILL - approximately: 70% sand (silty), very dark grey- black and dark brown layered, moist																
20% sandstone cobbles, boulders and similar granular fill																
10% organics (topsoil, black, grass, smelly vegetation etc)					1											
					2											
					3											

DEPTH OF PIT: 3.6m






LOCATION Refer Site Plan		BORE HOLE No. 1 GA1-1		FIGURE 2A SHEET 1 OF 1
SURFACE ELEVATION 41.2	DATUM Std.			
INCLINATION Vertical	AZIMUTH -			
DRILL TYPE Gemco	DATE 8.10.79	PROJECT Long Bay		

STRATIGRAPHY	GRAPHIC LOG	ELEV'N. DEPTH metres	SAMPLE TYPE SAMPLE LOG, DISTURBED	BLOWS / 300 DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
FILL SILTY SAND, grey slightly organic Some bricks, concrete pieces Loose		0											TC Bit auger
		1	SP 1	9									
SILTY SAND (SP) Dark grey & brown moist organic		2	AS 2										
SANDSTONE MW-SW Medium grained, light grey													
Grey with occas. thin brown and orange streaking		3	NMLC - RC 1				Joints moderately to widely spaced @ 20° to horiz., clean.						NMLC rock coring
		4											
Grey with thin darker grey streaking		5	NMLC - RC 2				-10mm sandy clay filled joints						
END OF BORING @ 5.8m GROUNDWATER NOT ENCOUNTERED		6											

LOCATION Refer Site Plan			BORE HOLE No. 2 GA1-2		FIGURE 2B SHEET 1 OF 1		
SURFACE ELEVATION 37.1		DATUM Std.					
INCLINATION Vertical		AZIMUTH -					
DRILL TYPE Gemco		DATE 8.10.79		PROJECT Long Bay			

STRATIGRAPHY	GRAPHIC LOG	ELEV'N DEPTH metres	SAMPLE TYPE	SAMPLE LOST, DISTURBED	BLOWS / 300 DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
								LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE (%)	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
FILL (SAND) Dark grey, slightly organic Light grey, some darker grey loose		0												
		1	SP	1	7									
			AS	2										
SILTY SAND Dark brown, slightly organic (SP) fine grained, yellow brown moist - orange-brown grey brown		2	AS	3										
			AS	4										
			SP	5	6/150	Refusal 22				2				80% passing 0.425mm
SANDSTONE MW Light grey Becoming MW		3												
END OF BOREHOLE @ 4.0m GROUNDWATER NOT ENCOUNTERED		4												
		5												

LOCATION Refer Site Plan		BORE HOLE No. 3 GAI-3		FIGURE 2C	
SURFACE ELEVATION 37.1	DATUM Std.			SHEET 1 OF 1	
INCLINATION Vertical	AZIMUTH -				
DRILL TYPE Gemco	DATE 8.10.79	PROJECT Long Bay			

STRATIGRAPHY	GRAPHIC LOG	ELEV'N. DEPTH metres	SAMPLE TYPE SAMPLE LOST DISTURBED	BLOWS / 300 DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE %	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degree)	
SILTY SAND (SP) grey, slightly organic Orange-brown fine grained		0											TC auger
			SP 1	15/100									
SANDSTONE HW-MW yellow & orange, medium grain		1											
softer		2											
MW grey		3											
v. hard pieces		4											

LOCATION Refer Site Plan		BORE HOLE No. 4 GA1-4		FIGURE 2D	
SURFACE ELEVATION 35.2				SHEET 1 OF 1	
INCLINATION Vertical		DATUM Std.		AZIMUTH -	
DRILL TYPE Gemco		DATE 8.10.79		PROJECT Long Bay	

STRATIGRAPHY	GRAPHIC LOG	ELEV'N DEPTH metres	SAMPLE TYPE	SAMPLE DIST. DISTURBED	BLOWS / 300 DISTANCE (mm)	DRY DENSITY (Mg/m³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
								LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE %	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
SILTY SAND (SP) fine grained, dark grey slightly organic (topsoil)		0												TC Auger
		1	SP	1	7									
SANDSTONE MM light grey, some yellow		2												12.10.79
		3												
END OF BOREHOLE @ 3.0m		4												Water level 1.83m 8.00am 9.10.79

LOCATION	Refer Site Plan			BORE HOLE No. 5 941-5	FIGURE 2E SHEET 1 OF 1
SURFACE ELEVATION	34.9	DATUM	Std.		
INCLINATION	Vertical	AZIMUTH	-		
DRILL TYPE	Gemco	DATE	8.10.79	PROJECT	Long Bay

STRATIGRAPHY	GRAPHIC LOG	ELEV'N. DEPTH metres	SAMPLE TYPE SAMPLE LOST DISTURBED	BLOWS / DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE (%)	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) Cc, (kPa) φ, (degrees)	

SILTY SAND dark grey, fine grained, slightly organic		0												TC Bit Auger
SANDSTONE MW-SW grey & orange & brown banded		0.43												NMLC rock coring
		1												12.10.79
		2												
BORING ENDED @ 2.0m		3												Water level 0.83m 8.00am 9.10.79

LOCATION Refer Site Plan		BORE HOLE No. 6 GAI-6		FIGURE 2F	
SURFACE ELEVATION 34.7	DATUM Std.			SHEET 1 OF 1	
INCLINATION Vertical	AZIMUTH -				
DRILL TYPE Gemco	DATE 9.10.79	PROJECT Long Bay			

STRATIGRAPHY	GRAPHIC LOG	ELEV'N. DEPTH metres	SAMPLE TYPE SAMPLE LOST DISTURBED	BLOWS / 300 DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE CONTENT (%)	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
SILTY SAND (SP) V. dark grey-black & rust-brown fine grained		0											V-bit auger
loose		1	SP 1	5		4				3		92% passing 0.524mm	
Orange & rust brown (waterloo rock) weakly cemented		2											
loose v.wet			SP 2	7						1.4		93% passing 0.425mm	
SAND (SP) pale yellow brown, fine grained		3											
moderately dense			SP 3	16									
SILT (OH) organic, black		4											
SILTY CLAY (CL) grey		5	AS 4										
becoming sandy													
SILTY SAND (SP) medium grained light brown		6											
coarse grained dense			SP 5	19/250		'Bouncing' Refusal							
SANDY CLAY (CL) grey CLAYEY SAND		7											
SILT (OH) stiff, dark brown-black, organic		8	AS 6			35						organic content 13%	
			SP 7			'Bouncing' refusal of SPT							TC Auger
SANDSTONE HW-XW		9											
		10	AS 8										

END OF BORING @ 10.0m
GROUNDWATER NOT MEASURED

LOCATION Refer Site Plan				BORE HOLE No. 8 GA1-8										FIGURE 2H SHEET 1 OF 1	
SURFACE ELEVATION 32.8		DATUM Std.													
INCLINATION Vertical		AZIMUTH -		PROJECT Long Bay											
DRILL TYPE Gemco		DATE 7.10.79													
STRATIGRAPHY	GRAPHIC LOG	ELEV'N	SAMPLE TYPE	BLOWS / 300 DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE CONTENT (%)	CLASSIFICATION DATA				STRENGTH DATA		OTHER		
		DEPTH					SAMPLE LOST DISTURBED	LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE %	% FINES (No. 200)	TYPE OF TEST		PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
		metres													
SILTY SAND (SP) V. dark grey-black, organic.		0												V Bit augered	
SANDY SILT (OL) black, organic			AS 1											organic content 6.5%	
loose		1												12.10.79	
v. wet			SP 2	6		56								organic content 9%	
SILTY SAND (SP) grey,		2													
CLAYEY SAND		3	AS 3											Tricon roller	
SANDY CLAY (CL) grey v. moist firm			SP 4	7		16	29	17		54	PP	qu, 100		89% passing 0.425mm	
CLAYEY SAND (SC) dense		4	SP 5	29						3				82% passing 0.425mm	
SANDSTONE XW grey, occas. darker grey streaking		5													
HW		6	NMLC1 - RC1			Joints widely spaced									
		7	NMLC - RC2												
		8													
MW		9	NMLC - RC3			Joints moderately widely spaced									
END OF BORING @ 9.7m		10													

LOCATION	Refer Site Plan		<div style="text-align: center;"> BORE HOLE No. 9 <i>GAI-9</i> </div>	<div style="text-align: center;"> FIGURE 2I SHEET 1 OF 1 </div>	
SURFACE ELEVATION	32.8	DATUM			Std.
INCLINATION	Vertical	AZIMUTH			-
DRILL TYPE	Gemco	DATE			8.10.79
PROJECT			Long Bay		

STRATIGRAPHY	GRAPHIC LOG	ELEV'N. DEPTH metres	SAMPLE TYPE SAMPLE LOST DISTURBED	BLOWS / DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%)	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE %	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
FILL Silty Sand, dark grey slightly organic Grey - light brown		0											TC Bit auger
SANDY SILT (OL) v. dark grey-black organic moist-wet loose		1	SP 1	3									12.10.79
(brown-black) Highly organic			AS 2										
SAND (SP) medium - coarse grain light brown-grey		2	SP 3	10									
CLAYEY SAND (SC) grey medium-fine grain loose		3											
moderately dense		4	SP 4	13									
moderately dense		5	SP 5	25									
light grey brown medium grain		6											
		7		29									solid cone
SANDY CLAY (CL), bands of stiff grey clay grey firm-stiff		8	AS 6										
		9											
SANDSTONE HW-XW		10											

Water 0.85m
10.9.79



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STRATIGRAPHY	GRAPHIC LOG	ELEV'N. DEPTH metres	SAMPLE TYPE SAMPLE LOST, DISTURBED	BLOWS/ DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE CONTENT (%)	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE (%)	% FINES (No. 200)	TYPE OF TEST	PARAMETERS	
												S _u , (kPa) C _u , (kPa) φ, (degrees)	

GOLDER ASSOCIATES

LOCATION Refer Site Plan		BORE HOLE No. 11 GAI-11	FIGURE 2L SHEET 1 OF 1
SURFACE ELEVATION	DATUM		
INCLINATION Vertical	AZIMUTH		
DRILL TYPE Gemco	DATE 9.10.79	PROJECT Long Bay	

STRATIGRAPHY	GRAPHIC LOG	ELEV'N DEPTH metres	SAMPLE TYPE SAMPLE NO. DISTURBED	BLOWS / DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE (%)	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su (kPa) C _u (kPa) φ (degrees)	

FILL Silty sand, & concrete blocks & bricks		0																	
		1																	
Unable to penetrate concrete or steel		2																	

LOCATION Refer Site Plan		BORE HOLE No. 12 GAI-12		FIGURE 2M SHEET 1 OF 1	
SURFACE ELEVATION 33.5	DATUM Std.				
INCLINATION Vertical		AZIMUTH			
DRILL TYPE Gemco	DATE 9.10.79	PROJECT Long Bay			

STRATIGRAPHY	GRAPHIC LOG	ELEV'N DEPTH metres	SAMPLE TYPE SAMPLE DIST. DISTURBED	BLOWS / DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degree)	
SILTY SAND (SP) grey-dark grey sl. organic		0											V Bit Auger
SAND ('Waterloo Rock') dark brown-rust brown, weakly cemented		1											
SAND (SP) v. wet fine grained yellow-brown		2											12.10.79
SANDSTONE HW-XW		3											TC Bit auger
		3.8											
END OF BORING @ 3.8m		4											
		5											

LOCATION Refer Site Plan		BORE HOLE No. 13 G41-13		FIGURE 2N SHEET 1 OF 1
SURFACE ELEVATION 33.8	DATUM Std.			
INCLINATION Vertical		AZIMUTH		
DRILL TYPE Gemco	DATE 8.10.79	PROJECT Long Bay		

STRATIGRAPHY	GRAPHIC LOG	ELEV'N, DEPTH metres	SAMPLE TYPE	SAMPLE LOSS, DISTURBED	BLOWS / DISTANCE (mm)	DRY DENSITY (Mg/m³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
								LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
SILTY SAND (SP) grey & darker grey slightly organic Fine grained		0												V Bit Auger
		1	SP 1	18										
INDURATED SAND brown-black ('Waterloo' Rock') weakly cemented			NMLC - RC 1											12.10.79
SAND (SP) orange brown med. grained		2												
SILTY CLAY (CL) grey, organic inclusions		3												
SANDY CLAY (CL) grey brown soft-firm (moist)		4												
													V Bit Refusal	
SANDSTONE XW Grey, some grey streaking @ 100		5	NMLC - RC 2				Joints moderately widely spaced horizontal							NMLC rock coring
		6					Joints closely spaced							
HW														
END OF BORING @ 6.51m		7												Water level 1.35m 8.00am 9.10.79

LOCATION Refer Site Plan		SURFACE ELEVATION 36.6		DATUM Std.	BORE HOLE No. 14 GAI-14	FIGURE 2A SHEET 1 OF 1
INCLINATION Vertical		AZIMUTH				
DRILL TYPE Gemco 210B		DATE 6.12.79		PROJECT Long Bay Hospital		

STRATIGRAPHY	GRAPHIC LOG	ELEV'N DEPTH metres	SAMPLE TYPE SAMPLE LOST, DISTURBED	BLOWS / DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE %	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
FILL Silty sand, bricks, and concrete rubble		0											
SAND (SP)													
Grey		1											
Rust brown													
Yellow - brown		2	SP 1	14/300									
Orange brown Fine-med grained		3											
END OF BORING @ 3.2m GROUNDWATER NOT ENCOUNTERED		4											

LOCATION Refer Site Plan		SURFACE ELEVATION 36.2		DATUM Std.	BORE HOLE No. 15 GAI-15	FIGURE SHEET OF
INCLINATION Vertical		AZIMUTH				
DRILL TYPE Gemco 210B		DATE 6.12.79		PROJECT Long Bay Hospital		

STRATIGRAPHY	GRAPHIC LOG	ELEV'N DEPTH metres	SAMPLE TYPE SAMPLE LOST, DISTURBED	BLOWS / DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE %	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
FILL Silty Sand, loose dark grey-black		0											
SAND (SP)													
loose - med.dense fine grained grey		1											
Rust brown ('Waterloo Rock')													
		2	SP 1	9/300									
END OF BORING @ 2.2m GROUNDWATER NOT ENCOUNTERED DURING AUGERING		3											

LOCATION		Refer Site Plan		BORE HOLE No. 16 GA1-16				FIGURE 28 SHEET 1 OF 1	
SURFACE ELEVATION		35.1							
INCLINATION		Vertical		AZIMUTH					
DRILL TYPE		Gemco 210B		DATE		6.12.79		PROJECT Long Bay Hospital	

STRATIGRAPHY	GRAPHIC LOG	ELEV'N DEPTH metres	SAMPLE TYPE SAMPLE NO. DISTURBED	BLOWS/ DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
SAND (SP) Loose - mod. dense fine grained Orange brown		0											
		1											
		2	SP 1	12/300									
			'Bouncing' Refusal 'V' Bit Refusal @ 2.2m										
END OF BORING @ 2.2m GROUNDWATER NOT ENCOUNTERED													




LOCATION		Refer Site Plan		BORE HOLE No. 17 GA1-17				FIGURE SHEET OF	
SURFACE ELEVATION		35.6							
INCLINATION		Vertical		AZIMUTH					
DRILL TYPE		Gemco 210B		DATE		6.12.79		PROJECT Long Bay Hospital	

STRATIGRAPHY	GRAPHIC LOG	ELEV'N DEPTH metres	SAMPLE TYPE SAMPLE NO. DISTURBED	BLOWS/ DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
FILL Silty Sand dark grey black		0											V Bit augered
		1											
Gray Occas. bit of refuse		2	SP 1	14/300									
		3											
SAND (SP) Mod. dense Grey-brown Fine grained		2											
		3											
moist-wet		3	SP 2	22/300									
		4											
		5											
		6											
			'V' Bit Refusal @ 5.8m										
END OF BORING @ 5.8m													



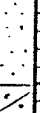
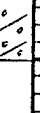
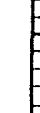
during augering
 v. slow drilling


GOLDER ASSOCIATES

LOCATION		Refer Site Plan		BORE HOLE No. 18 GA1-18										FIGURE 2C SHEET 1 OF 1					
SURFACE ELEVATION		34.5														DATUM		Std.	
INCLINATION		Vertical														AZIMUTH			
DRILL TYPE		Gemco 210B		DATE		6.12.79		PROJECT										Long Bay Hospital	
STRATIGRAPHY				GRAPHIC LOG	ELEV'N DEPTH metres	SAMPLE TYPE SAMPLE LOST, DISTURBED	BLOWS / DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER			
										LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)				
FILL Silty Sand dark grey Silty Sand, bricks, concrete blocks, metal & plastic waste					0											TC Bit Augered			
					1														
SAND (SP) loose fine grained yellow grey sloppy orange brown					2	SP 1	10/3-0									During Drilling			
					3	SP 2		"Bouncing"	Refusal				Refusal of 'V' Bit						
END OF BORING @ 3.1m																			

LOCATION		Refer Site Plan		BORE HOLE No. 19 GA1-19										FIGURE SHEET OF					
SURFACE ELEVATION		34.0 approx.														DATUM		Std	
INCLINATION		Vertical														AZIMUTH			
DRILL TYPE		Gemco 210B		DATE		7.12.79		PROJECT Long Bay Hospital											
STRATIGRAPHY				GRAPHIC LOG	ELEV'N DEPTH		SAMPLE TYPE 	BLOWS / DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER		
					metres						LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE (%)	% FINES (No. 200)	TYPE OF TEST	PARAMETERS			
FILL Sand, some bricks loose					0	SP 1	8/300												
											SAND (SP) fine grained Gray brown wet	2	SP 2	14/300					
END OF BORING @ 4.0m				4	'V' Bit Refusal @ 3.8m 'TC' Bit Refusal @ 4.0m														
During Drilling 																			
GOLDER ASSOCIATES																			

LOCATION	Refer Site Plan			BORE HOLE No. 20 GA1-20	FIGURE 2D SHEET 1 OF 1
SURFACE ELEVATION	33.7 approx.	DATUM	Std		
INCLINATION	Vertical	AZIMUTH			
DRILL TYPE	Gemco 210B	DATE	7.12.79		
PROJECT				Long Bay Hospital	

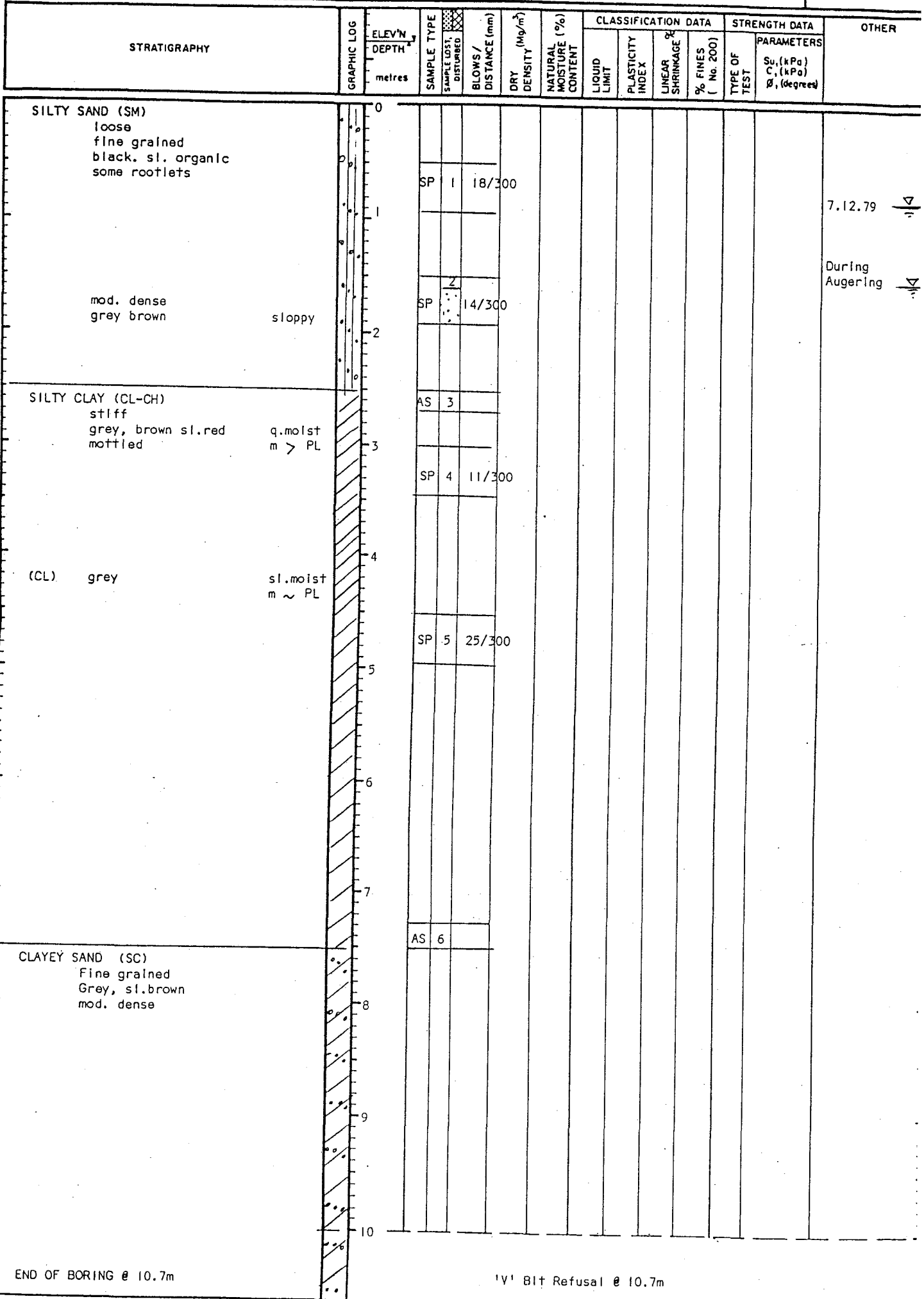
STRATIGRAPHY	GRAPHIC LOG	ELEV'N DEPTH metres	SAMPLE TYPE SAMPLE NO. DISTURBED	BLOWS / DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE %	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
FILL Sand, ash some bricks, timber and other building waste Organic, black - dk. grey		0											
		1											
		2	SP 1	8/300									
sloppy		3											
SAND (SP) fine grey		3	SP 2	5									
SANDY CLAY (CL) grey		3											
END OF BORING @ 3.5m		3	AS 3										
		4											

During Drilling 

LOCATION	Refer Site Plan		<h1 style="text-align: center;">BORE HOLE No. 21</h1> <p style="text-align: center;">GA1-21</p>	FIGURE 2E SHEET 1 OF 1	
SURFACE ELEVATION	33.2 approx.	DATUM			Std.
INCLINATION	Vertical	AZIMUTH			
DRILL TYPE	Gemco 210B	DATE			7.12.79
PROJECT			Long Bay Hospital		

STRATIGRAPHY	GRAPHIC LOG	ELEV'N DEPTH metres	SAMPLE TYPE SAMPLE LOST DISTURBED	BLOWS / DISTANCE (mm)	DRY DENSITY (Mg/m ³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE %	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su, (kPa) C, (kPa) φ, (degrees)	
FILL Sand, gravel some bricks, ash, and other building rubble dark grey		0											TC Bit augered thru. fill
		1											
SAND/CLAYEY SAND (SC) fine-med grained	sloppy	2		SP 1 4/300									During Augering V Bit augered
		3											
SILTY CLAY (CL-CH) v. stiff	sl.moist m sl PL			SP 2 30/300									
Grey, thin black band		4											
		5											
SANDY CLAY (CL) v. stiff hard grey				SP 3 38/300									
		6											
		7											v.stiff drilling
		8		AS 4									
Grey & darker grey sl organic													
END OF BORING @ 8.4m													'V' Bit Refusal @ 8.4m
		9											

LOCATION Refer Site Plan		BORE HOLE No. 23 G41-23		FIGURE 26 SHEET 1 OF 1
SURFACE ELEVATION 33.2	DATUM Std			
INCLINATION Vertical	AZIMUTH			
DRILL TYPE Gemco 210B	DATE 5.12.79	PROJECT Long Bay Hospital		



END OF BORING @ 10.7m

'V' Bit Refusal @ 10.7m

LOCATION	Refer Site Plan			BORE HOLE No. 24 GAI-24	FIGURE 2H SHEET 1 OF 1
SURFACE ELEVATION	33.0	DATUM	Std		
INCLINATION	Vertical	AZIMUTH			
DRILL TYPE	Gemco 210B	DATE	5.12.79	PROJECT	Long Bay Hospital

STRATIGRAPHY	GRAPHIC LOG	ELEV'n DEPTH metres	SAMPLE TYPE SAMPLE LOG DISTURBED	BLOWS/ DISTANCE (mm)	DRY DENSITY (Mg/m³)	NATURAL MOISTURE (%) CONTENT	CLASSIFICATION DATA				STRENGTH DATA		OTHER
							LIQUID LIMIT	PLASTICITY INDEX	LINEAR SHRINKAGE %	% FINES (No. 200)	TYPE OF TEST	PARAMETERS Su (kPa) C (kPa) φ (degrees)	
SILTY SAND (SM-SP) Fine grained v. loose Dark brown-black. sl. organic grey, brown sloppy		0											
			SP 1	1/300									
		1											7.12.79
SANDY CLAY (CL) stiff Grey, fine sandy grained moist m > PL		2											
			SP 2	7/150									During Augering
		3											
CLAYEY SAND (SC) mod. dense fine grained dark grey-sl. bluish grey		4											
			AS 4										
		5											Stiffer Drilling
SANDSTONE (XW-HW)		6											
		7											
		8											
		9											'V' Bit Refusal @ 8.9m
		10											'TC' Bit Refusal @ 10.5m

DPWSI-1

NSW DEPARTMENT
OF PUBLIC WORKS
AND SERVICES

GEOTECHNICAL & ENVIRONMENTAL

BOREHOLE

BH1

PROJECT: LONG BAY CORRECTIONAL CENTRE

DATE: 1/11/01

LOCATION: AREA 1

SURFACE RL:

CONTRACTOR: SAXON

DRILLER: P. CLOSE

RIG TYPE: EXPLORER MKI

DEPTH (m)	WATER	BIT	SAMPLE OF TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grainsize, moisture, remarks
0						
				FILL		FILL - SILTY SAND with clayey silt lumps, some organics and grass roots; uncontrolled (very loose); dry.
				ROCK		SANDSTONE; medium to coarse grained; moderately weathered; weak; light grey and yellow-brown.
						END at 0.60
1						
2						
3						
4						

SAMPLE OR TEST

U.....Undisturbed
D.....Disturbed
SPT.....Standard Penetration Test
CPT.....Cone Penetration Test

v : visual
l : laboratory

WATER

Water Table
 Water Inflow

DRILLING SUPERVISOR: C. KARWAJ
PROJECT COORDINATOR: C. KARWAJ
SHEET 1 OF 1 SHEETS
SCALE 1 : 20

NSW DEPARTMENT
OF PUBLIC WORKS
AND SERVICES

GEOTECHNICAL & ENVIRONMENTAL

BOREHOLE

BH2

PROJECT: LONG BAY CORRECTIONAL CENTRE

DATE: 1/11/01

LOCATION: AREA 1

SURFACE RL:

CONTRACTOR: SAXON

DRILLER: P. CLOSE

RIG TYPE: EXPLORER MKI

DEPTH (m)	WATER	BIT	SAMPLE OR TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grain size, moisture, remarks
0	No Groundwater Encountered	TC		FILL		FILL - SILTY SAND with some gravel-sized sandstone fragments mantled by a thin layer of silty topsoil with grass roots;
				FILL		light grey-brown; uncontrolled (very loose); dry. 0.30
				SM (v)		becoming dark grey to black; contains coke fragments and ash. 0.60
1				ROCK		SILTY SAND; dark brown; loose; moist. 0.75
						SANDSTONE; highly weathered; very weak to weak; light grey, yellow, orange and brown. END at 1.10
						NOTE: Slow, hard drilling for a TC bit below 1m - rock ground down to a silt powder. No refusal.
2						
3						
4						

SAMPLE OR TEST

U.....Undisturbed
D.....Disturbed
SPT....Standard Penetration Test
CPT....Cone Penetration Test

v : visual
l : laboratory

WATER

➤ Water Table
➤ Water Inflow

DRILLING SUPERVISOR: C. KARWAJ
PROJECT COORDINATOR: C. KARWAJ
SHEET 1 OF 1 SHEETS
SCALE 1 : 20

NSW DEPARTMENT
OF PUBLIC WORKS
AND SERVICES

GEOTECHNICAL & ENVIRONMENTAL

BOREHOLE

BH3

PROJECT: LONG BAY CORRECTIONAL CENTRE

DATE: 1/11/01

LOCATION: AREA 1

SURFACE RL:

CONTRACTOR: SAXON

DRILLER: P. CLOSE

RIG TYPE: EXPLORER MKI

DEPTH (m)	WATER	BIT	SAMPLE OR TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grain size, moisture, remarks
0						
1						
2						
3						
4						

FILL - SILTY SAND with some gravel and grass roots; contains occasional fragments of wood, plastic, charcoal/slag; coarse gravel/cobbles in pockets (0.5-0.7mm) 1.2m - 1.3m; grey-brown; uncontrolled (very loose); dry to moist below 0.9m, becoming wet at 1.9m.

FILL

TC

SP/SM
(v)

SAND with some silt; fine to medium grained; dark grey-brown; medium dense; wet. (possibly fill)

ROCK

SANDSTONE; extremely weathered to highly weathered; extremely weak; light grey with some yellow, orange and brown.

END at 3.80

NOTE: Firm to hard drilling for a TC bit below 3.7m. No refusal.

SAMPLE OR TEST

U.....Undisturbed
D.....Disturbed
SPT.....Standard Penetration Test
CPT.....Cone Penetration Test

v : visual
l : laboratory

WATER

Water Table
Water Inflow

DRILLING SUPERVISOR: C. KARWAJ
PROJECT COORDINATOR: C. KARWAJ
SHEET 1 OF 1 SHEETS
SCALE 1 : 20

NSW DEPARTMENT
OF PUBLIC WORKS
AND SERVICES

GEOTECHNICAL & ENVIRONMENTAL

BOREHOLE

BH4

PROJECT: LONG BAY CORRECTIONAL CENTRE

DATE: 1/11/01

LOCATION: AREA 1

SURFACE RL:

CONTRACTOR: SAXON

DRILLER: P. CLOSE

RIG TYPE: EXPLORER MKI

DEPTH (m)	WATER	BIT	SAMPLE OF TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grain size, moisture, remarks
0						
1		TC		FILL		FILL - SILTY SAND with occasional gravel and grass roots; rare pieces of plastic and glass, coarse gravel to cobble-sized sandstone fragments at 0.4m and 0.6m to 0.85m; dark grey-brown; uncontrolled (very loose); dry to 0.6m then moist.
1.60				FILL		FILL - cobbles and boulders in sandy matrix; grey-brown; wet.
2						NOTE: TC bit refusal at 1.8m depth - interpreted to be on a boulder within fill.
3						
4						

SAMPLE OR TEST

U.....Undisturbed
D.....Disturbed
SPT.....Standard Penetration Test
CPT.....Cone Penetration Test

v : visual
l : laboratory

WATER

Water Table
Water Inflow

DRILLING SUPERVISOR: C. KARWAJ
PROJECT COORDINATOR: C. KARWAJ
SHEET 1 OF 1 SHEETS
SCALE 1 : 20

NSW DEPARTMENT
OF PUBLIC WORKS
AND SERVICES

GEOTECHNICAL & ENVIRONMENTAL

BOREHOLE

BH5

PROJECT: LONG BAY CORRECTIONAL CENTRE

DATE: 1/11/01

LOCATION: AREA 1

SURFACE RL:

CONTRACTOR: SAXON

DRILLER: P. CLOSE

RIG TYPE: EXPLORER MKI

DEPTH (m)	WATER	BIT	SAMPLE OR TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grain size, moisture, remarks
0						
				FILL		FILL - SILTY SAND with grass roots; pockets of clayey silt; grey-brown and brown; uncontrolled (very loose); dry
				ROCK		SANDSTONE; highly weathered; very weak; light grey, yellow and orange-brown.
						END at 0.60
						NOTE: TC bit refusal at 0.6m depth.
1						
2						
3						
4						

SAMPLE OR TEST

U.....Undisturbed
 D.....Disturbed
 SPT.....Standard Penetration Test
 CPT.....Cone Penetration Test

v : visual
 l : laboratory

WATER

Water Table
 Water Inflow

DRILLING SUPERVISOR: C. KARWAJ
 PROJECT COORDINATOR: C. KARWAJ
 SHEET 1 OF 1 SHEETS
 SCALE 1 : 20

NSW DEPARTMENT
OF PUBLIC WORKS
AND SERVICES

GEOTECHNICAL & ENVIRONMENTAL

BOREHOLE

BH6

PROJECT: LONG BAY CORRECTIONAL CENTRE

DATE: 1/11/01




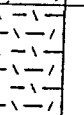
LOCATION: AREA 1

SURFACE RL:

CONTRACTOR: SAXON

DRILLER: P. CLOSE

RIG TYPE: EXPLORER MKI

RIG TYPE: EXPLORER MKI						
DEPTH (m)	WATER	BIT	SAMPLE OR TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grainsize, moisture, remarks
0	No Groundwater Encountered	TC	D	FILL		FILL - SILTY SAND with blue metal gravel; trace of organics; grey-brown; uncontrolled (very loose); dry.
0.40				FILL		FILL - SAND with traces of silt; occasional coarse gravel-sized fragments at 0.75m and 1.4m; dark grey-brown; uncontrolled (very loose); moist to very moist with depth.
1.60				SM (v)		SILTY SAND with traces of gravel; fine to medium grained; brown; loose; wet.
1.80				ROCK		SANDSTONE; highly weathered to moderately weathered; very weak; light grey.
2						NOTE: TC bit refusal at 2.1m. No groundwater in the borehole at completion of drilling; however, the sand below 1.6m is in a wet state. Some seepage likely if long term monitoring was undertaken.
3						END at 2.10

SAMPLE OR TEST

U.....Undisturbed
D.....Disturbed
SPT.....Standard Penetration Test
CPT.....Cone Penetration Test

v : visual
l : laboratory

WATER

➤ Water Table
➤ Water Inflow

DRILLING SUPERVISOR: C. KARWAJ
PROJECT COORDINATOR: C. KARWAJ
SHEET 1 OF 1 SHEETS
SCALE 1 : 20

NSW DEPARTMENT
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GEOTECHNICAL & ENVIRONMENTAL

BOREHOLE

BH7

PROJECT: LONG BAY CORRECTIONAL CENTRE

DATE: 1/11/01

LOCATION: AREA 1

SURFACE RL:

CONTRACTOR: SAXON

DRILLER: P. CLOSE

RIG TYPE: EXPLORER MKI

DEPTH (m)	WATER	BIT	SAMPLE OF TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grain size, moisture, remarks
0	No Groundwater Encountered	TC		FILL		FILL - SILTY SAND with traces of clay and gravel; dark grey-brown; very loose; moist.
0.35				FILL		FILL - gravel and cobbles in a silty sand and sand matrix; gravel is coke/slag, blue metal and sandstone fragments; contains traces of clay 0.7m to 0.9m; grey-brown to 1.0m then black; uncontrolled (very loose to loose); moist to very moist with wet pockets below 0.7m.
1.40				ROCK		SANDSTONE; moderately weathered; medium strong; light grey. END at 1.60
2						NOTE: TC bit refusal at 1.6m. No groundwater in the borehole at completion of drilling; however, the fill below 0.7m is in a wet state.
3						
4						

SAMPLE OR TEST

U.....Undisturbed
D.....Disturbed
SPT.....Standard Penetration Test
CPT.....Cone Penetration Test

v : visual
l : laboratory

WATER

Water Table
 Water Inflow

DRILLING SUPERVISOR: C. KARWAJ
PROJECT COORDINATOR: C. KARWAJ
SHEET 1 OF 1 SHEETS
SCALE 1 : 20

NSW DEPARTMENT
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AND SERVICES

GEOTECHNICAL & ENVIRONMENTAL

BOREHOLE

BH8

PROJECT: LONG BAY CORRECTIONAL CENTRE

LOCATION: AREA 1

CONTRACTOR: SAXON

DRILLER: P. CLOSE

DATE: 1/11/01

SURFACE RL:

RIG TYPE: EXPLORER MKI

DEPTH (m)	WATER	BIT	SAMPLE OF TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grainsize, moisture, remarks
0				FILL		FILL - SAND with silt and gravel; light grey-brown to brown; uncontrolled (very loose); dry.
0.70				FILL		FILL - gravel and cobbles in sandy matrix; grey-brown; medium dense; moist with wet pocket at 0.8m.
1.10				FILL		FILL - SILTY SAND to CLAYEY SILTY SAND; black; uncontrolled (loose); very moist to 1.5m then wet.
2.10				SP/SM (v)		SAND with some silt; medium grained; grey-brown; loose; wet.
2.30				CL/SC (v)		SANDY CLAYEY SILT/CLAYEY SILTY SAND with some roots; dark chocolate brown; firm; moist to very moist.
2.60						INDURATED SAND (coffee rock); dark chocolate brown; dense; moist. Strong H2S odour.
3.10				SC/CI (v)		CLAYEY SAND/SANDY CLAY; light grey; stiff; very moist.
3.35				ROCK		SANDSTONE; highly weathered; extremely weak; light grey and yellow-brown.
END at 3.60						

SAMPLE OR TEST

U.....Undisturbed
D.....Disturbed
SPT.....Standard Penetration Test
CPT.....Cone Penetration Test

v : visual
l : laboratory

WATER

Water Table
 Water Inflow

DRILLING SUPERVISOR: C. KARWAJ
PROJECT COORDINATOR: C. KARWAJ
SHEET 1 OF 1 SHEETS
SCALE 1 :20

NSW DEPARTMENT
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AND SERVICES

GEOTECHNICAL & ENVIRONMENTAL

BOREHOLE

BH9

PROJECT: LONG BAY CORRECTIONAL CENTRE

DATE: 1/11/01

LOCATION: AREA 1

SURFACE RL:

CONTRACTOR: SAXON

DRILLER: P. CLOSE

RIG TYPE: EXPLORER MKI

DEPTH (m)	WATER	BIT	SAMPLE OR TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grain size, moisture, remarks
0				SP/SM (v)		SAND with some silt and grass roots; fine to medium grained; mid grey; loose; dry.
				SP (v)		becoming light grey-brown; medium dense and moist. 0.40
			D	SM (v)		SILTY SAND; dark grey-brown; loose; very moist to wet. 0.60
1				SC (v)		CLAYEY SILTY SAND; dark chocolate-brown; medium dense; very moist. 0.85
						INDURATED SAND (coffee rock); dark chocolate-brown to black; dense; moist. 1.10
2			TC			
			SPT 8/50, N = R			
3				ROCK		SANDSTONE; extremely weathered; extremely weak; light grey; moist to very moist. 2.50
4						END at 3.45

SAMPLE OR TEST

U.....Undisturbed
D.....Disturbed
SPT.....Standard Penetration Test
CPT.....Cone Penetration Test

v : visual
l : laboratory

WATER

Water Table
 Water Inflow

DRILLING SUPERVISOR: C. KARWAJ
PROJECT COORDINATOR: C. KARWAJ
SHEET 1 OF 1 SHEETS
SCALE 1 : 20



NSW DEPARTMENT
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AND SERVICES

GEOTECHNICAL & ENVIRONMENTAL

BOREHOLE

BH10

PROJECT: LONG BAY CORRECTIONAL CENTRE

DATE: 1/11/01

LOCATION: AREA 1

SURFACE RL:

CONTRACTOR: SAXON

DRILLER: P. CLOSE

RIG TYPE: EXPLORER MKI

DEPTH (m)	WATER	BIT	SAMPLE or TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grainsize, moisture, remarks
0	No Groundwater Encountered	TC		SP/SM (v)		SAND with some silt, fine gravel and grass roots; mid grey; loose; dry.
				SM (v)		SILTY SAND with some gravel and traces of organics; dark grey-brown; loose to medium dense; very moist. 0.40
				ROCK		SANDSTONE; highly weathered; very weak; light grey and yellow-brown. 0.60
1						
2						
3						
4						END at 1.10

SAMPLE OR TEST

U.....Undisturbed
D.....Disturbed
SPT....Standard Penetration Test
CPT....Cone Penetration Test

v : visual
l : laboratory

WATER

Water Table
Water Inflow

DRILLING SUPERVISOR: C. KARWAJ
PROJECT COORDINATOR: C. KARWAJ
SHEET 1 OF 1 SHEETS
SCALE 1 : 20



NSW DEPARTMENT
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GEOTECHNICAL & ENVIRONMENTAL

BOREHOLE

BH11

PROJECT: LONG BAY CORRECTIONAL CENTRE

DATE: 1/11/01

LOCATION: AREA 1

SURFACE RL:

CONTRACTOR: SAXON

DRILLER: P. CLOSE

RIG TYPE: EXPLORER MKI

DEPTH (m)	WATER	BIT	SAMPLE or TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grain size, moisture, remarks
0				FILL		FILL - SAND with silt and gravel; mid to dark grey; uncontrolled (loose); dry.
	N.G.E	TC		ROCK		SANDSTONE; highly weathered to moderately weathered; very weak to weak; light grey and yellow-brown.
1						NOTE: TC bit refusal at 0.87m depth.
2						
3						
4						

SAMPLE OR TEST

U.....Undisturbed
D.....Disturbed
SPT.....Standard Penetration Test
CPT.....Cone Penetration Test

v : visual
l : laboratory

WATER

Water Table
Water Inflow

DRILLING SUPERVISOR: C. KARWAJ
PROJECT COORDINATOR: C. KARWAJ
SHEET 1 OF 1 SHEETS
SCALE 1 : 20

NSW DEPARTMENT
OF PUBLIC WORKS
AND SERVICES

GEOTECHNICAL & ENVIRONMENTAL

BOREHOLE

BH12

PROJECT: LONG BAY CORRECTIONAL CENTRE

DATE: 2/11/01

LOCATION: AREA 2

SURFACE RL:

CONTRACTOR: SAXON

DRILLER: P. CLOSE

RIG TYPE: EXPLORER MKI

DEPTH (m)	WATER	BIT	SAMPLE or TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grain size, moisture, remarks
0				FILL		FILL - SAND with silt, gravel and grass roots; gravel is sandstone fragments; yellow-brown and grey-brown; uncontrolled (loose); dry.
0.60						
1			SPT 3,4,4 N = 8			SAND with trace of silt; medium grained; golden yellow-brown; loose; moist to very moist with depth.
2	No Groundwater Encountered	TC		SP (v)		
3			SPT 3,5,5 N = 10			
4						- becoming medium dense to dense.

SAMPLE OR TEST

U.....Undisturbed
D.....Disturbed
SPT....Standard Penetration Test
CPT....Cone Penetration Test

v : visual
l : laboratory

WATER

➤ Water Table
➤ Water Inflow

DRILLING SUPERVISOR: C. KARWAJ
PROJECT COORDINATOR: C. KARWAJ
SHEET 1 OF 2 SHEETS
SCALE 1 : 20



NSW DEPARTMENT
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GEOTECHNICAL & ENVIRONMENTAL

BOREHOLE

BH1

PROJECT: LONG BAY CORRECTIONAL CENTRE

DATE: 25.01.02

LOCATION: HALFWAY HOUSE

SURFACE RL:

CONTRACTOR: SAXONS

DRILLER: P.CLOSE

RIG TYPE: EXPLORER MK.1

DEPTH (m)	WATER	BIT	SAMPLE OR TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grainsize, moisture, remarks
5						--- (continued from previous sheet) --- SAND with a trace of silt; medium grained; grey brown; medium dense; wet.
6				SP (v)		
7		Vee				
			D	SP (v)		- becomes interbedded with silty clay lenses.
8						
			D	SC (v)		ORGANIC (PEATY) CLAYEY SAND; black and dark grey; stiff; very moist.
9						END at 9.00
						NOTE: Vee bit refusal at 9.0m on what is inferred to be a moderately weathered sandstone.
10						

SAMPLE OR TEST

U.....Undisturbed

D.....Disturbed

SPT....Standard Penetration Test

CPT....Cone Penetration Test

v : visual
l : laboratory

WATER

➤ Water Table

➤ Water Inflow

DRILLING SUPERVISOR: M.ASHOVER

PROJECT COORDINATOR: C.KARWAJ

SHEET 2 OF 2 SHEETS

SCALE 1 :25

NSW DEPARTMENT
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GEOTECHNICAL & ENVIRONMENTAL

BOREHOLE

BH2

PROJECT: LONG BAY CORRECTIONAL CENTRE

DATE: 25.01.02

LOCATION: HALFWAY HOUSE

SURFACE RL:

CONTRACTOR: SAXONS

DRILLER: P.CLOSE

RIG TYPE: EXPLORER MK.1

DEPTH (m)	WATER	BIT	SAMPLE OR TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grainsize, moisture, remarks	
0				SP (v)		TOPSOIL - sand with traces of silt; dark grey to mid grey; very loose; moist.	0.25
			SPT 2,2,2 N = 4	SP (v)		SAND; medium grained; light brown, occasional dark brown lenses or pockets; very loose; just moist.	1.00
1			D	SM (v)		SILTY SAND with traces of clay and organics; anoxic odour; medium grained; very loose; wet.	1.40
	1.1m						
	1.4m		SPT 1,2,3 N = 5	SP (v)		SAND with traces of silt; traces of decomposed organics and roots; dark grey; loose; wet.	
2							
		Vee					
3			SPT 3,12,20 N = 32	SP (v)		SAND; fine to medium grained; light to mid brown; medium dense; wet.	3.00
4				SC (v)		SILTY SAND with clay; light grey; medium dense; very moist to wet.	3.90
5							

SAMPLE OR TEST

U.....Undisturbed

D.....Disturbed

SPT.....Standard Penetration Test

CPT.....Cone Penetration Test

v : visual
l : laboratory

WATER

Water Table

Water Inflow

DRILLING SUPERVISOR: M.ASHOVER

PROJECT COORDINATOR: C.KARWAJ

SHEET 1 OF 2 SHEETS

SCALE 1 :25

NSW DEPARTMENT
OF PUBLIC WORKS
AND SERVICES

GEOTECHNICAL & ENVIRONMENTAL

BOREHOLE

BH2

PROJECT: LONG BAY CORRECTIONAL CENTRE

DATE: 25.01.02

LOCATION: HALFWAY HOUSE

SURFACE RL:

CONTRACTOR: SAXONS

DRILLER: P.CLOSE

RIG TYPE: EXPLORER MK.1

DEPTH (m)	WATER	BIT	SAMPLE OR TEST	SOIL GROUP	GRAPHIC LOG	SOIL DESCRIPTION Soil type, colour, consistency, grain size, moisture, remarks
5				SC (v)		--- (continued from previous sheet) --- SILTY SAND with clay; light grey; medium dense; very moist to wet.
		Vee	D	SC/OH (v)		CLAYEY SAND with SILTY CLAY interbeds; black and dark grey banded; medium dense/stiff; very moist.
6				ROCK		SANDSTONE; highly weathered; very weak.
						NOTE: Vee bit refusal at 6.6m.
7						
8						
9						
10						

SAMPLE OR TEST

U.....Undisturbed
D.....Disturbed
SPT.....Standard Penetration Test
CPT.....Cone Penetration Test

v : visual
l : laboratory

WATER

Water Table
 Water Inflow

DRILLING SUPERVISOR: M.ASHOVER
PROJECT COORDINATOR: C.KARWAJ
SHEET 2 OF 2 SHEETS
SCALE 1 : 25

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT
 Project: PROPOSED NEW BUILDING
 Location: KATINGAL AREA, LONG BAY GADL. N.S.W.

Job No. 5070 JS
 Date: 20.5.87

Method: SPIRAL AUGER R.L. Surface: 37.12 m
 HYDRAPOWER RIG Datum: SITE

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
		NC = 14 10 25mm TEST ABANDONED- OBSTRUCTION IN HOLE	1			FILL: Gravelly sand, fine to medium grained, brown, some silt. Gravel of sandstone fragments. Occasional fragments of brick, concrete and timber to cobble? size	M			FILL APPEARS POOR TO MODERATE COMPACTION
		NC = 4 6 7 10	2							
			3							
			4							
		NC = 10 12 100mm TEST ABANDONED- OBSTRUCTION IN HOLE	5				M-W			NC: VALUE PROBABLY AFFECTED BY OBSTRUCTION IN HOLE
			6		SM	SILTY SAND: fine to medium grained, grey brown	W	(MD)		ALLUVIUM (HOLE COLLAPSED TO 5.2m WHEN AUGERS WITHDRAWN FROM 6.2m)
	D.S.		7							T 10mm T 12mm
		NC: ATTEMPTED BUT OBSTRUCTION IN HOLE AT 5.2m								

MOISTURE ON CPT CONE (TEST AT 4.7m)

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT
 Project: PROPOSED NEW BUILDING
 Location: KATINGAL AREA, LONG BAY GAOL. N.S.W.

Job No. 5070 JS
 Date: 20.5.87

Method: SPIRAL AUGER RIG
 HYDRAPOWER RIG
 R.L. Surface: 37.12 m
 Datum: SITE

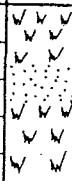
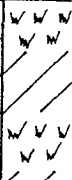
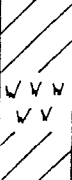

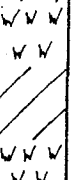
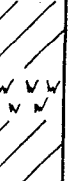
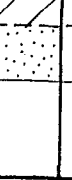

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
			7		SM	SILTY SAND: fine to medium grained, grey brown.	W	(MD)		60mm LOW-MODERATE DRILLING RESISTANCE
			8		CL/SC	SANDY CLAY/CLAYEY SAND: medium plasticity, fines, mainly fine grained sand, grey	MC=PL	(ST-VST)		55mm
	D.S.									60mm
										50mm
			9							100mm
										85mm
			10		SM	SILTY SAND: fine to medium grained, brown	W	(MD)		70mm
										25mm
			11							20mm
										15mm
			12							10mm
										30mm
			13		OL/SM	INTERBEDDED PEAT and SAND: Peat; fibrous and silty, dark brown, dense. Sand; silty, fine to medium grained, grey	M	H/D		MODERATE DRILLING RESISTANCE
			14							

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT
 Project: PROPOSED NEW BUILDING
 Location: KATINGAL AREA, LONG BAY GAOL. N.S.W.

Job No. 5070 JS
 Date: 20.5.87

Method: SPIRAL AUGER
 HYDRAPOWER RIG
 R.L. Surface: 37.12m
 Datum: SITE

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
	D.S.				OL / SM	INTERBEDDED PEAT AND SAND as above, peat predominates	M	H/D		MODERATE DRILLING RESISTANCE
			15			INTERBEDDED PEAT and CLAY: Peat; dark brown, silty. Clay; low plasticity, grey bedding < 100mm	MC PL	V. ST		VERY LOW DRILLING RESISTANCE 5mm
			16							0mm
			17							20mm
			18							40mm
			19							10mm
			20			----- Becomes mainly clay				30mm
	D.S.		20.5m			----- Possibly ----- SAND AT BASE OF HOLE				A FEW MODERATE BANDS 60mm
						END OF BOREHOLE				

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT
 Project: PROPOSED NEW BUILDING
 Location: KATINGAL AREA, LONG BAY GAOL. N.S.W.

Job No. 5070 JS
 Date: 20.5.87

Method: SPIRAL AUGER R.L. Surface: 35.55 m
 HYDRAPOWER RIG Datum: SITE

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
						FILL: topsoil (100mm) over brown fine to medium grained, sand	M	V.L		FILL
			1		SP	SAND: medium grained, light brown, trace silt	M	V.L		DUNE SAND/ ALLUVIUM
	D.S.	Nc = <div>1 2 1</div>	2							
			3					M D		
		Nc = <div>4 5 6 6 5</div>	4			as above, grey brown	M-W W			
			5					D		
		Nc = 30	6							
			7.5m							

HOLE COLLAPSED ON 28.5.87

END OF BOREHOLE

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT

Project: PROPOSED NEW BUILDING

Location: KATINGAL AREA, LONG BAY GAOL. N.S.W.

Job No. 5070 JS

Date: 20.5.87

Method: SPIRAL AUGER
HYDRAPOWER RIG

R.L. Surface:

33.26 m

Datum:

SITE

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/Rel. Density	Hand Penetrometer Readings kPa.	Remarks
28.5.87					SM	SILTY SAND: fine to medium grained, dark brown, peaty	M	(V.L)		ALLUVIUM
	D.S.	Nc = 4	1				W	(L)		
		6						MD		
		8	2							
		8			CL	CLAY: medium plasticity, banded light grey and orange brown	MC>PL	(st)		MODERATE DRILLING RESISTANCE
	D.S.		3							
		N = 17			SP/CL	SAND: fine to medium grained, light brown, some silt, interbedded with clay, medium plasticity, light grey and orange brown	W/MC>PL	MD/st		LOW RESISTANCE
		3, 7, 10	4			Some peaty sand around 4.4m				
						clay band, as above				MODERATE RESISTANCE
			5			mainly sand, as above				LOW RESISTANCE
	D.S.					clay band, as above				MODERATE RESISTANCE
		Nc = 5	6							
		11								
		13								
			7							LOW RESISTANCE

BOREHOLE LOG

Client: *PUBLIC WORKS DEPARTMENT*
 Project: *PROPOSED NEW BUILDING*
 Location: *KATINGAL AREA, LONG BAY GAOL. N.S.W.*

Job No. *5070 JS*
 Date: *20.5.87*

Method: *SPIRAL AUGER* R.L. Surface: *33.26m*
HYDRAPOWER RIG Datum: *SITE*





Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
			7		SM	SILTY SAND: fine to medium grained, grey brown	W	(MD)		ALLUVIUM RELATIVE DENSITY ASSESSED ONLY LOW DRILLING RESISTANCE
			8							
			9							
			10							
			11		CL	----- Probably ----- clay band	MC > PL	(St)		MODERATE RESISTANCE
			12		SC	----- Probably ----- CLAYEY SAND: fine to medium grained, grey brown	M	(MD)		MODERATE RESISTANCE WITH LOW BANDS
			13							
			14							

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT
 Project: PROPOSED NEW BUILDING
 Location: KATINGAL AREA, LONG BAY GAOL. N.S.W.

Job No. 5070 JS
 Date: 20.5.87

Method: SPIRAL AUGER R.L. Surface: 33.26m
 HYDRAPOWER RIG Datum: SITE

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
			15		SC/SP	CLAYEY SAND/ SAND: with some clay, fine to medium grained, light grey	M	(MD)		ALLUVIUM (RELATIVE DENSITIES ASSESSED ONLY)
			16							
			17							
			17.8m			as above, but with a few brown organic bands				
			18			END OF BOREHOLE				AUGER REFUSAL
			19							
			20							

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT
 Project: PROPOSED NEW BUILDING
 Location: KATINGAL AREA, LONG BAY GAOL. N.S.W.

Job No. 5070 JS
 Date: 20.5.87

Method: SPIRAL AUGER
 HYDRAPOWER RIG

R.L. Surface: 33.88m
 Datum: SITE

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
ON S.P.T RODS 22.5.87						TOP SOIL: peaty sand, dark brown				ALLUVIUM
			1		SP	SAND: fine to medium grained, brown some silt	M	L		
							W			
	D.S.	Nc = 4 6 3 9	2		SM	SILTY SAND: fine to medium grained, dark brown organic		MD		
	D.S.	N = 8 2, 3, 5	3		CL	CLAY: low plasticity, light grey and orange brown, trace sand	MC > PL	St.	140-280	
			4		SM	SILTY SAND: fine to medium grained, brown	W	MD		
	D.S.	Nc = 5 9 13	5							
			6		CL	CLAY: low plasticity, light grey, trace to some fine sand.	MC > PL	St		
			7							

T 60mm

T 20mm

T 10mm

T 55mm

A

2/2


BOREHOLE LOG

Client: *PUBLIC WORKS DEPARTMENT*
 Project: *PROPOSED NEW BUILDING*
 Location: *KATINGAL AREA, LONG BAY GAOL. N.S.W.*

Job No. *5070 JS*
 Date: *20.5.87*

Method: *SPIRAL AUGER*
HYDRAPOWER RIG

R.L. Surface: *33.88m*
 Datum: *SITE*

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
	D.S.		7 5m		CL	CLAY: as above	MC & PL	(54)		
			8 9 10 11 12 13			END OF BOREHOLE				

Borehole No.

5

BOREHOLE LOG

[illegible]

BOREHOLE LOG

Client: *PUBLIC WORKS DEPARTMENT*
 Project: *PROPOSED NEW BUILDING*
 Location: *KATINGAL AREA, LONG BAY GAOL. N.S.W.*

Job No. *5070 JS*
 Date: *22.5.87*

Method: *SPIRAL AUGER*
HYDRAPOWER RIG

R.L. Surface: *35.83m*
 Datum: *SITE*

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
						<i>TOPSOIL: silty sand, fine to medium grained, dark grey</i>	<i>M</i>	<i>L</i>		
					<i>SP</i>	<i>SAND: fine to medium grained, white</i>	<i>M</i>	<i>L</i>		
			<i>1</i>			<i>Grading to</i>				
					<i>SM</i>	<i>SILTY SAND: fine to medium grained, dark brown and yellow brown</i>				
						<i>Grading to</i>				
	<i>D.S.</i>	<i>N=5</i>			<i>SP</i>	<i>SAND: fine to medium grained, yellow</i>				
		<i>2,2,3</i>	<i>2</i>							
			<i>3</i>							
	<i>D.S.</i>	<i>N=9</i>								
		<i>3,5,4</i>								
<i>22.5.87</i>										
					<i>CL/SC</i>	<i>CLAYEY SAND/SANDY CLAY: fine to medium grained, medium plasticity, light grey</i>	<i>W</i>	<i>V.S.T to H/MD</i>		
<i>ON RODS AFTER TEST AT 4.5m</i>			<i>4</i>							
	<i>D.S.</i>	<i>N=17</i>								
		<i>5,8,9</i>	<i>5</i>						<i>380</i> <i>410</i> <i>340</i> <i>350</i> <i>580</i>	
										<i>40mm</i>
										<i>20mm</i>
	<i>D.S.</i>		<i>6.0m</i>							<i>10mm</i>
			<i>6</i>							
						<i>END OF BOREHOLE</i>				

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT
 Project: PROPOSED NEW BUILDING
 Location: KATINGAL AREA, LONG BAY GAOL. N.S.W.

Job No. 5070 JS
 Date: 21.5.87

Method: SPIRAL AUGER R.L. Surface: 35.68m
 HYDRAPOWER RIG Datum: SITE

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
						FILL: silty sand, fine to medium grained with gravel dark grey brown	M			POORLY COMPACTED
					SP	SAND: fine to medium grained, white	M	L		
	D.S.	N=6 3, 3, 3	1		SM	Grading to SILTY SAND: fine to medium grained with weakly cemented iron indurated zones, dark brown				
					SP	Grading to SAND: fine to medium grained, yellow brown				
			2							
	D.S.	N=8 2, 4, 4								
			3							
	D.S.	N=3 1, 2, 1	4			as above, becoming		VL		
			5							
	D.S.	N=17 6, 7, 10			SC	Grading to CLAYEY SAND: fine to medium grained, light grey with interbedded layers of sand, fine to medium grained, yellow brown and CLAY medium to high plasticity, yellow brown		MD		
			6							
			7					L		

T 60mm

T 90mm

BOREHOLE LOG

Client: *PUBLIC WORKS DEPARTMENT*
 Project: *PROPOSED NEW BUILDING*
 Location: *KATINGAL AREA, LONG BAY GAOL. N.S.W.*

Job No. *5070 JS*
 Date: *21.5.87*

Method: *SPIRAL AUGER* R.L. Surface: *35.68m*
HYDRAPOWER RIG Datum: *SITE*


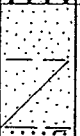
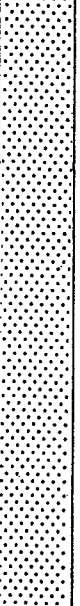
Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
			7		SC	CLAYEY SAND: fine to medium grained, no interbedded clay or sand layers evident	W	L-MD		T 50mm
			8							T 70mm
										T 125mm
			9.0							T 30mm
			9			END OF BOREHOLE				
			10							
			11							
			12							
			13							

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT
 Project: PROPOSED NEW BUILDING
 Location: KATINGAL AREA, LONG BAY GAOL. N.S.W.

Job No. 5070 JS
 Date: 22.5.87

Method: SPIRAL AUGER R.L. Surface: 34.95m
 HYDRAPOWER RIG Datum: SITE

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
						FILL: silty sand, fine to medium grained with gravel	M			POORLY COMPACTED
	D.S.	N > 16 3, 5, 11/150mm	1		SP SC	SAND: fine to medium grained, light grey Grading to CLAYEY SAND: fine to medium grained grey and light grey	M			ESTIMATED 'V' BIT REFUSAL
	D.S.	HAMMER BOUNCING				Grading to SANDSTONE: fine to medium grained, extremely weathered, extremely weak, with iron indurated silty sand zones orange brown				LOW 'TC' BIT RESISTANCE WITH OCCASIONAL MODERATE BANDS.
			2			SANDSTONE: fine to medium grained, highly weathered, weak yellow brown becoming light grey				MODERATE 'TC' BIT RESISTANCE
	D.S.		3							
ON COMPLETION			4			as above, but moderately weathered medium strong, light grey				MODERATE TO HIGH 'TC' BIT RESISTANCE
	D.S.		4.5m							
			5			END OF BOREHOLE				
			6							
			7							

JEFFERY AND KATAUSKAS PTY. LTD.

JK2-15

Borehole No.


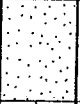
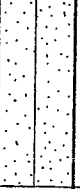







15

1/2

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT
Project: PROPOSED NEW BUILDING
Location: KATINGAL AREA, LONG BAY GAOL. N.S.W.

Job No. 5070 JS Method: SPIRAL AUGER R.L. Surface: 34.53m
Date: 28.5.87 HYDRAPOWER RIG Datum: SITE

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
						FILL: gravelly sand, fine to medium grained, dark brown	D-M			POORLY COMPACTED
					SP	SAND: fine to medium grained, light grey brown	M	L		ALLUVIUM
			1		SM	SILTY SAND: fine to medium grained, dark brown	M	MD-D		
AFTER 4 hours	D.S.	N >> 22 10, 13, 9, 130 mm								
			2		SP/SC	SAND: fine to medium grained, brown, some silt, some clayey bands	W			
			3							
	D.S.	N = 28 6, 12, 16								
					SC	CLAYEY SAND: fine to medium grained, light brown				
			4							
			5							
			6							
			7							

NE = 20/75 mm
HAMMER BOUNCING

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT
 Project: PROPOSED NEW BUILDING
 Location: KATINGAL AREA, LONG BAY GAOL. N.S.W.

Job No. 5070 JS
 Date: 28.5.87

Method: SPIRAL AUGER R.L. Surface: 34.53m
 HYDRAPOWER RIG Datum: SITE





Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
			7		SC	CLAYEY SAND: as above				
			8			SANDSTONE: medium grained, light grey brown, highly weathered, very weak to weak.				ESTIMATED 'V'-BIT REFUSAL
			9.0m							MODERATE TO HIGH 'TC' BIT RESISTANCE
			9			END OF BOREHOLE				LOW 'TC' BIT RESISTANCE
			10							
			11							
			12							
			13							

BOREHOLE LOG

Client: PUBLIC WORKS DEPARTMENT
 Project: PROPOSED NEW BUILDING
 Location: KATINGAL AREA, LONG BAY GAOL. N.S.W.

Job No. 5070 JS
 Date: 28.5.87

Method: SPIRAL AUGER R.L. Surface: 36.20m
 HYDRAPOWER RIG Datum: SITE

Groundwater record	Samples	FIELD TESTS	Depth (m.)	Graphic Log	Unified Classification	DESCRIPTION	Moisture Condition	Consistency/ Rel. Density	Hand Penetrometer Readings kPa.	Remarks
			1			FILL: sand, medium grained, some silt with some brick and sandstone fragments	M			
		SPT ATTEMPTED REFUSAL ON SANDSTONE	2		SP	SAND: medium grained, light brown				(ESTIMATED) 'V' BIT REFUSAL
	D.S.		3			SANDSTONE: medium grained, light brown, moderately weathered, weak to medium strong as above, but weak				MODERATE TO HIGH 'TC' BIT RESISTANCE
	D.S.		4			as above, but medium strong, light grey				MODERATE TO HIGH 'TC' BIT RESISTANCE
			4.5m			END OF BOREHOLE				
			5							
			6							
			7							

APPENDIX C

LABORATORY TEST RESULTS

california bearing ratio test results

client : **DEPARTMENT OF COMMERCE**job no : **E12723/3**

principal :

laboratory : **SYDNEY**project : **ENVIRONMENTAL & GEOTECHNICAL INVESTIGATION**report date : **November 23, 2004**location : **Longbat Correctional Complex - Malabar, NSW**

test report :

test procedure : **AS1289 6.1.1**laboratory compaction method : **AS1289 5.1.1**

sample number :		AREA B - CBH8	AREA D - CBH9		
depth:	m	0.6-1.6	0.6-1.8		
location:		-	-		
date sampled:		-	-		
date tested:		19/11/04	19/11/04		
material description:		(SP) SAND: fine to medium, grey.	(SP) SAND: fine to medium, brown.		
maximum dry density:	t/m ³	1.81	1.71		
optimum moisture content:	%	11.0	13.3		
field moisture content	%	5.6	9.7		
retained on 19mm AS sieve:	%	0	0		
+ 19mm material included:		Not Applicable	Not Applicable		
C.B.R. test	before soaking	dry density	t/m ³	1.80	1.71
		density ratio	%	99.0	100.0
		moisture content	%	11.8	13.1
		moisture ratio	%	107.0	98.0
	after soaking	dry density	t/m ³	1.79	1.70
		density ratio	%	99.0	99.0
		moisture content	%	14.0	15.2
	number of days soaked:			4	4
	surcharge:		kg	4.5	4.5
	moisture content	top 30 mm		13.7	16.0
		remaining sample		13.9	15.1
	after test		%		
	swell after soaking:		%	0.5	0.5
	penetration:		mm	2.5/5.0	2.5/5.0
	C.B.R. value:		%	20/25	20/20

remarks :

Samples supplied by the client on the 5/11/04.

The tests, calibrations or measurements covered by this document have been performed in accordance with NATA requirements which include the requirements of ISO/IEC 17025 and are traceable to national standards of measurement. This document shall not be reproduced except in full.

NATA Accredited Laboratory No. 431 Date : **23 November 2004**

Authorised Signature:

Garry Collins
Senior Technical Officer

test resultsclient : **DEPARTMENT OF COMMERCE**job no : **E12723/3**

principal :

laboratory : **SYDNEY**project : **ENVIRONMENTAL & GEOTECHNICAL INVESTIGATION**report date : **23 November, 2004**location : **Longbay Correctional Complex - Malabar, NSW**

test report :

test procedure : **AS1289.5.1.1**test date : **9/11/04**

SAMPLE IDENTIFICATION	STANDARD MAXIMUM DRY DENSITY	STANDARD OPTIMUM MOISTURE CONTENT	MATERIAL RETAINED ON THE 19mm SIEVE
	(t/m3)	(%)	(%)
AREA A - Bulk Sample (0.2-0.5m)	1.80	12.3	0
AREA B - CBH8 (0.6-1.6m)	1.81	11.0	0
AREA D - CBH9 (0.6-1.8m)	1.71	13.3	0

remarks : **Samples supplied on the 5/11/04.**

The tests, calibrations or measurements covered by this document have been performed in accordance with NATA requirements which include the requirements of ISO/IEC 17025 and are traceable to national standards of measurement. This document shall not be reproduced except in full.

NATA Accredited Laboratory No. 431 Date : **23 November, 2004**

Authorised Signature:

Garry Collins
Senior Technical Officer

Job No **E12723/04**

Sheet **3** of **3**

Point Load Strength Index Test Results

Client	NSW DEPT. OF COMMERCE			Office	SYDNEY
Principal	NSW DEPT. OF CORRECTIONAL SERVICES			Date	30/6/2005
Project	LONG BAY CORRECTIONAL COMPLEX			By	SG
Location	MALABAR			Checked	

Test Method	AS 4133.4.1 - 1993 Methods of Testing Rocks for Engineering Purposes, Determination of Point Load Strength Index					Sampling Technique		NMLC triple tube diamond coring		Sampling Date		10-16/03/2005																						
Test Machine	GSA (bench-mounted)					Storage History		Coffey Sydney office indoor core storage area		Testing Date		18/3/2005																						
Calibration Date	27/4/2004					Moisture Condition		Natural		Tested By		SG																						
Rock Type						Location		Depth (m)		Axial, Block, and Irregular Lump Tests					Strength Classification																			
										Diametral Tests		Failure Mode				W (mm)		D (mm)		L (mm)		P (kN)		I _s (MPa)		I _{sg0} (MPa)		Failure Mode						
Sandstone						CBH76		1.95		51		170		1.98		0.77		Parallel to bedding		51		38		-		1.93		0.78		0.78		Through substance		M
Sandstone						CBH76		2.95		51		400		2.63		1.02		Parallel to bedding		51		35		-		2.84		1.25		1.22		Through substance		H
Sandstone						CBH76		3.95		51		950		1.97		0.76		Parallel to bedding		51		33		-		1.97		0.92		0.89		Through substance		

Point Load Strength Index Test Results

Client	NSW DEPT. OF COMMERCE			Job No	E12723/04
Principal	NSW DEPT. OF CORRECTIONAL SERVICES			Sheet	1 of 3
Project	LONG BAY CORRECTIONAL COMPLEX			Office	SYDNEY
Location	MALABAR			Date	30/6/2005
				By	SG
				Checked	

Test Method	AS 4133.4.1 - 1993 Methods of Testing Rocks for Engineering Purposes, Determination of Point Load Strength Index	Sampling Technique	NMLC triple tube diamond coring	Sampling Date	10-16/03/2005										
Test Machine	GSA (bench-mounted)	Storage History	Coffey Sydney office indoor core storage area	Testing Date	18/3/2005										
Calibration Date	27/4/2004	Moisture Condition	Natural	Tested By	SG										
		Loading Rate	< 30 seconds												
Rock Type	Location	Depth (m)	Diametral Tests					Axial, Block, and Irregular Lump Tests					Strength Classification		
			D (mm)	L (mm)	P (kN)	I _{s(50)} (MPa)	Failure Mode	W (mm)	D (mm)	L (mm)	P (kN)	I _s (MPa)		I _{s(50)} (MPa)	Failure Mode
Sandstone	CBH91	0.95	51	90	1.89	0.73	Parallel to bedding	51	39	-	1.73	0.68	0.69	Through substance	M
Sandstone	CBH91	1.95	51	110	1.3	0.5	Parallel to bedding	51	39	-	1.6	0.63	0.63	Through substance	M
Sandstone	CBH91	2.95	51	110	1.13	0.44	Parallel to bedding	51	34	-	1.35	0.61	0.59	Through substance	M
Sandstone	CBH91	3.85	51	550	0.54	0.21	Parallel to bedding	51	41	-	0.33	0.12	0.13	Through substance	L

Coley Geosciences Pty Ltd ACN 056 335 310			
Point Load Strength Index Test Results			
Client	NSW DEPT. OF COMMERCE		
Principal	NSW DEPT. OF CORRECTIONAL SERVICES		
Project	LONG BAY CORRECTIONAL COMPLEX		
Location	MARRABONG		
Job No	E12723/04		
Sheet	1	of	2
Office	SYDNEY		
Date	30/6/2005		
By	SG		
Checked			

Point Load Strength Index Test Results

Coffey

Client	NSW DEPT. OF COMMERCE	Office	SYDNEY												
Principal	NSW DEPT. OF CORRECTIONAL SERVICES	Date	30/6/2005												
Project	LONG BAY CORRECTIONAL COMPLEX	By	SG												
Location	MALABAR	Checked													
Test Method	AS 4133.4.1 - 1993 Methods of Testing Rocks for Engineering Purposes, Determination of Point Load Strength Index	Sampling Technique	NMLC triple tube diamond coring												
Test Machine	GSA (bench-mounted)	Storage History	Coffey Sydney office indoor core storage area												
Calibration Date	27/4/2004	Moisture Condition	Natural												
		Loading Rate	< 30 seconds												
Rock Type	Location	Depth (m)	Diametral Tests						Axial, Block, and Irregular Lump Tests						Strength Classification
			D (mm)	L (mm)	P (kN)	I _{s(50)} (MPa)	Failure Mode	W (mm)	D (mm)	L (mm)	P (kN)	I _s (MPa)	I _{s(50)} (MPa)	Failure Mode	
Sandstone	CBH94	1.95	51	60	0.82	0.32	Parallel to bedding	51	30	-	1.54	0.79	0.75	Through substance	M
Sandstone	CBH94	2.95	51	170	2.95	1.14	Parallel to bedding	51	38	-	2.85	1.15	1.15	Through substance	H
Sandstone	CBH94	3.95	51	240	1.33	0.52	Parallel to bedding	51	35	-	2.06	0.91	0.89	Through substance	M
Sandstone	CBH94	4.15	51	100	1.46	0.57	Parallel to bedding	51	29	-	1.22	0.65	0.61	Through substance	M

Job No **E12723/04**

Point Load Strength Index Test Results

Sheet **2** of **2**

Client	NSW DEPT. OF COMMERCE
Principal	NSW DEPT. OF CORRECTIONAL SERVICES
Project	LONG BAY CORRECTIONAL COMPLEX
Location	MALABAR
Office	SYDNEY
Date	30/6/2005
By	SG
Checked	

Test Method	AS 4133.4.1 - 1993 Methods of Testing Rocks for Engineering Purposes, Determination of Point Load Strength Index	Sampling Technique	NMLC triple tube diamond coring	Sampling Date	10-16/03/2005										
Test Machine	GSA (bench-mounted)	Storage History	Coffey Sydney office indoor core storage area	Testing Date	18/3/2005										
Calibration Date	27/4/2004	Moisture Condition	Natural	Tested By	SG										
		Loading Rate	< 30 seconds												
Rock Type	Location	Depth (m)	Diametral Tests					Axial, Block, and Irregular Lump Tests					Strength Classification		
			D (mm)	L (mm)	P (kN)	I _{s(50)} (MPa)	Failure Mode	W (mm)	D (mm)	L (mm)	P (kN)	I _s (MPa)		I _{s(50)} (MPa)	Failure Mode
Sandstone	CBH122	1.95	51	500	1.01	0.39	Parallel to bedding	51	45	-	0.84	0.29	0.3	Through substance	L / M
Sandstone	CBH122	2.95	51	70	0.44	0.17	Parallel to bedding	51	37	-	0.4	0.17	0.17	Through substance	L
Sandstone	CBH122	3.95	51	60	2.85	1.11	Parallel to bedding	51	36	-	2.23	0.95	0.94	Through substance	M / H
Sandstone	CBH122	4.30	51	240	1.62	0.63	Parallel to bedding	51	29	-	1.71	0.91	0.85	Through substance	M

Job No E12723/4

Sheet 1 of 1

Point Load Strength Index Test Results

Client	NSW DEPT. OF COMMERCE			Office	SYDNEY
Principal	NSW DEPT. OF CORRECTIONAL SERVICES			Date	24/3/2005
Project	LONG BAY CORRECTIONAL COMPLEX			By	CFW
Location	MALABAR			Checked	

Test Method	AS 4133.4.1 - 1993 Methods of Testing Rocks for Engineering Purposes, Determination of Point Load Strength Index				Sampling Technique		NMLC triple tube diamond coring		Sampling Date	16/3/2005									
Test Machine	GSA (bench-mounted)				Storage History		Coffey Sydney office indoor core storage area		Testing Date	24/3/2005									
Calibration Date	8/9/2000				Moisture Condition		Natural		Tested By	CFW									
Rock Type					Location	Depth (m)	Diametral Tests				Axial, Block, and Irregular Lump Tests				Strength Classification				
							D (mm)	L (mm)	P (kN)	I _{s(50)} (MPa)	Failure Mode	W (mm)	D (mm)	L (mm)		P (kN)	I _s (MPa)	I _{s(50)} (MPa)	Failure Mode
Sandstone					CBH105	12.90	50	220	0.13	0.05	Parallel to bedding	41	50	41	0.1	0.04	0.04	Through substance	VL
Sandstone							50	110	0.33	0.13	Parallel to bedding	22	50	22	0.08	0.06	0.05	Through substance	VL / L
Sandstone							50	100	0.22	0.09	Parallel to bedding	25	50	25	0.19	0.12	0.11	Through substance	VL / L
Sandstone							50	200	0.38	0.15	Parallel to bedding	30	50	30	0.4	0.21	0.2	Through substance	L
Sandstone							50	60	0.75	0.3	Parallel to bedding	25	50	25	0.48	0.3	0.27	Through substance	L
							50	60	0.54	0.22	Parallel to bedding	28	50	28	0.32	0.18	0.17	Through substance	L

Job No **E12723/04**

Sheet **1** of **3**

Point Load Strength Index Test Results

Client	NSW DEPT. OF COMMERCE			Office	SYDNEY
Principal	NSW DEPT. OF CORRECTIONAL SERVICES			Date	30/6/2005
Project	LONG BAY CORRECTIONAL COMPLEX			By	SG
Location	MALABAR			Checked	

Test Method	AS 4133.4.1 - 1993 Methods of Testing Rocks for Engineering Purposes, Determination of Point Load Strength Index										Sampling Technique	NMLC triple tube diamond coring			Sampling Date	10-16/03/2005		
Test Machine	GSA (bench-mounted)										Storage History	Coffey Sydney office indoor core storage area			Testing Date	18/3/2005		
Calibration Date	27/4/2004										Moisture Condition	Natural			Tested By	SG		
											Loading Rate	< 30 seconds						
Rock Type	Location	Depth (m)	Diametral Tests					Axial, Block, and Irregular Lump Tests							Strength Classification			
			D (mm)	L (mm)	P (kN)	I _{s(50)} (MPa)	Failure Mode	W (mm)	D (mm)	L (mm)	P (kN)	I _s (MPa)	I _{s(50)} (MPa)	Failure Mode				
Sandstone	CBH100	5.60	46	140	0.48	0.22	Parallel to bedding	46	34	-	0.4	0.2	0.19	Through substance	L			
Sandstone	CBH100	6.50	51	350	1.14	0.44	Parallel to bedding	51	37	-	0.94	0.39	0.39	Through substance	M			
Sandstone	CBH100	7.10	51	150	1.84	0.71	Parallel to bedding	51	40	-	2.39	0.92	0.93	Through substance	M			

Job No E12723/04

Sheet 3 of 3

Office SYDNEY

Date 30/6/2005

By SG

Checked

Point Load Strength Index Test Results

Client NSW DEPT. OF COMMERCE

Principal NSW DEPT. OF CORRECTIONAL SERVICES

Project LONG BAY CORRECTIONAL COMPLEX

Location MALABAR

Test Method	AS 4133.4.1 - 1993 Methods of Testing Rocks for Engineering Purposes, Determination of Point Load Strength Index	Sampling Technique	NMLC triple tube diamond coring	Sampling Date	10-16/03/2005										
Test Machine	GSA (bench-mounted)	Storage History	Coffey Sydney office indoor core storage area	Testing Date	18/3/2005										
Calibration Date	27/4/2004	Moisture Condition	Natural	Tested By	SG										
		Loading Rate	< 30 seconds												
Rock Type	Location	Depth (m)	Diametral Tests					Axial, Block, and Irregular Lump Tests					Strength Classification		
			D (mm)	L (mm)	P (kN)	I _{ISO} (MPa)	Failure Mode	W (mm)	D (mm)	L (mm)	P (kN)	I _s (MPa)		I _{ISO} (MPa)	Failure Mode
Sandstone	CBH108	2.95	51	140	1.52	0.59	Parallel to bedding	51	38	-	1.81	0.73	0.73	Through substance	M
Sandstone	CBH108	3.90	51	250	1.36	0.53	Parallel to bedding	51	35	-	1.38	0.61	0.59	Through substance	M
Sandstone	CBH108	4.95	51	220	0.71	0.28	Parallel to bedding	51	37	-	1.17	0.49	0.48	Through substance	L / M
Sandstone	CBH108	5.20	51	60	1.27	0.49	Parallel to bedding	51	32	-	1.14	0.55	0.53	Through substance	M

**SYDNEY
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Page 1 of 3

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A.B.N. 81 829 182 852

ANALYTICAL REPORT for:

COFFEY GEOSCIENCES PTY LTD

PO BOX 125
NORTH RYDE 2113

ATTN: PHILIP CARVA

JOB NO: SAL15286
CLIENT ORDER: E12723/3
DATE RECEIVED: 09/11/04
DATE COMPLETED: 19/11/04
TYPE OF SAMPLES: SOILS
NO OF SAMPLES: 2

NATA Accredited Laboratory

Number: 1884



NATA ENDORSED TEST REPORT
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except in full.

.....
Issued on 26/11/04
Lance Smith
(Chief Chemist)

**SYDNEY
ANALYTICAL
LABORATORIES**

Page 2 of 3

ANALYTICAL REPORT

JOB NO: SAL15286
CLIENT ORDER: E12723/3

SAMPLES	pH 1:5	Cl %	SO4 %
1 AREA B-CBH8/1.0	5.5	0.002	0.001
2 AREA D-CBH9/1.0	7.3	0.002	0.002
MDL	0.1	0.001	0.001
Method Code	WA1	WA4	WA6
Preparation	P5	P5	P5

RESULTS ON DRY BASIS

ANALYTICAL REPORT

JOB NO: SAL15286

CLIENT ORDER: E12723/3

METHODS OF PREPARATION AND ANALYSIS

The tests contained in this report have been carried out on the samples as received by the laboratory.

P5 Sample dried, split and crushed to -150um

WA1 pH - 1:5 soil/water extract
 Determined by APHA 4500B

WA4 Chloride - 1:5 soil/water extract
 Determined by APHA 4110B

WA6 Sulphate - 1:5 soil/water extract
 Determined by APHA 4110B

A preliminary report was faxed on 19/11/04

AS4419-NS Natural Soil or Soil Blend Analysis

Test Type AS4419-NS (no lge parts)

Order No 24236 Job No:

Reference

Sample Name Area D

Sample No. 85063a

Date Received 12/11/2004 Total No Pages: 1 of 1

Client: Coffey Geosciences Pty Ltd
James Russell

LANE COVE WEST NSW

2066

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Email: sesl@sesl.com.au

Tests are performed under a quality system certified as complying with ISO 9002.

Results & Conclusions assume that sampling is representative. This document shall not be reproduced except in full

Characteristic	Unit	Results:	Acceptance Range	Comments
Bulk Density	kg/L	1.2	>0.7	acceptable
Organic Matter Content				
By Loss On Ignition	% dry wt		3-15	
By Walkely-Black	% dry wt	2.33	3-15	slightly low
Wettability	mm/min	600.0	> 5	acceptable
pH in water 1:5	pH units	7.5	5.5-7.5	acceptable
EC	mS/cm 1:5	0.09	< 1.2	acceptable
Ammonium	mg/L		NR	
Phosphorus	mg/kg	11.2	< 5 very P Sensitive < 20 moderately P Sensitive	acceptable
Dispersibility				
in water 1:5	Category	3	1-2	dispersive
in CaCl ₂ 1:5	Category	1	1-2	acceptable
Toxicity Index	mm	<5	≥ 70	very low
N Drawdown Index	-		> 0	
Permeability	cm/hr	8.6	2-100	acceptable
Texture	light sandy clay loam			
Large Particles				
< 10mm	%			
10-20mm	%		< 8	
> 20mm	%		< 2	
Sieve - Top Dressing				
Minerals > 2mm	%		0	
Organic 2-5mm	%		< 15	
Organic > 5mm	%		2	

Summary and Recommendations

This soil appears to have some good physical properties and an acceptable chemistry, however the toxicity index shows the seedlings did not grow in this mix. We find this to be a strange result as no chemical properties are greatly outstanding and would result in toxicity to the seeds, therefore we are going to re-test this mix. Currently the mix would fail the requirements of the AS4419, due to other factors. The results do indicate that the soil is slightly dispersive, again we recommend applying gypsum at 200g/m² to improve the soils stability. Organic matter could also be applied to the soil as well composted green waste.

Method: AS4419-2003

Test results apply to the sample submitted for analysis and do not necessarily imply that the product meets all the requirements of this standard.

Checked by Principal.....
Simon Leake Date of Report 23/11/2004

Consultant.....
C. Moore

AS4419-NS Natural Soil or Soil Blend Analysis

Test Type AS4419-NS (no lge parts)

Order No 24236 Job No:

Reference

Sample Name Area B

Sample No. 85061a

Date Received 12/11/2004 Total No Pages: 1 of 1

Client: Coffey Geosciences Pty Ltd
James Russell

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Results & Conclusions assume that sampling is representative. This document shall not be reproduced except in full

Characteristic	Unit	Results:	Acceptance Range	Comments
Bulk Density	kg/L	1.3	>0.7	acceptable
Organic Matter Content				
By Loss On Ignition	% dry wt		3-15	
By Walkely-Black	% dry wt	2.00	3-15	slightly low
Wettability	mm/min	360.0	> 5	acceptable
pH in water 1:5	pH units	5.8	5.5-7.5	acceptable
EC	mS/cm 1:5	0.03	< 1.2	acceptable
Ammonium	mg/L		NR	
Phosphorus	mg/kg	3	< 5 very P Sensitive < 20 moderately P Sensitive	acceptable
Dispersibility				
in water 1:5	Category	3	1-2	slightly dispersive
in CaCl ₂ 1:5	Category	1	1-2	acceptable
Toxicity Index	mm	39	≥ 70	low
N Drawdown Index	-		> 0	
Permeability	cm/hr	6.8	2-100	acceptable
Texture		sandy loam		
Large Particles				
< 10mm	%			
10-20mm	%		< 8	
> 20mm	%		< 2	
Sieve - Top Dressing				
Minerals > 2mm	%		0	
Organic 2-5mm	%		< 15	
Organic > 5mm	%		2	

Summary and Recommendations

This soil has very good physical properties, however its chemistry is slightly unbalanced. This would cause it to fail the requirements of the AS4419 for Natural soil. The organic matter of the soil is slightly low, this could be improved by the addition of organic matter such as well composted green waste to the soil mix. Alternatively organic matter can be applied as mulch to the surface of the soil. The results also indicate this soil is slightly dispersive, this is related to the low Calcium content. We recommend applying gypsum to the soil at 200g/m², this will add Calcium to the soil and therefore improve its stability.

Method: AS4419-2003

Test results apply to the sample submitted for analysis and do not necessarily imply that the product meets all the requirements of this standard.

Checked by Principal.....
Simon Leake Date of Report 23/11/2004Consultant.....
C. Moore

Soil Chemistry Profile

Test Type: FS
Order No: 24236 Job No:
Reference
Sample Name: Area B
Sample No: 85061
Date Received 12/11/2004 Total No Pages: 1 of 1
CLIENT: Coffey Geosciences Pty Ltd
James Russell



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TEST	RESULT	COMMENTS
pH in water 1:2	5.7	medium acidity
pH in CaCl ₂ 1:2	4.5	very strong acidity
EC mS/cm 1:2	0.06	very low
Chlorides mg/kg		

CATION ANALYSIS

TEST	SOLUBLE		EXCHANGEABLE		
Unit	meq%	Comment	meq%	% of ECEC	Comment
Sodium			.11	7.30	elevated
Potassium			.06	4.00	low - deficient
Calcium			.77	51.00	low - deficient
Magnesium			.51	33.80	high
Aluminium			.06	4.00	elevated
ECEC			1.51		
Ca/Mg			1.50		unbalanced

mg/kg

Phosphate as P	1.1	deficient - inadequate for all but some natives
Ammonium as N	7.6	low
Nitrate as N	4.6	low
Sulphate as S	8	low
Iron	44.2	slightly low
Zinc	5.8	adequate
Copper	.9	low - possibly inadequate
Manganese	3.2	low
Boron		

Recommendations

This sample show very similar chemistry to the previous sample. The chemistry shows the soil to be deficient in major nutrients, however this can be easily fixed with the addition of fertilisers. The soil has a medium to strong acidity. An application of lime at 500g/m² would be suitable to create more neutral conditions. The lime should then be followed by an application of the *Patons No. 27 (RED)* or equivalent at 50g/m². In this area it may also be suitable to apply a complete trace element mix at 10g/m² to improve the balance of these in the soil.

Explanation of the Methods:

pH, EC, Soluble Cations, Nitrate: Bradley et al (1983). Exchangeable Cations, ECEC: Method 15A1 Rayment & Higginson (1992). Chloride: Vogel (1961). Aluminium: Method 3500 APHA (1992). Phosphate: Method 9E1 Rayment & Higginson (1992). Ammonium, Sulphate, Iron, Copper, Manganese + Zinc: Method 83-1 to 83-5 Black (1983). Boron: Method 12C2 Rayment & Higginson (1992).

Checked by Principal.....
Simon Leake Date of Report 19/11/2004

Consultant.....
C. Moore

Soil Chemistry Profile

Test Type: FS
Order No: 24236 Job No:
Reference
Sample Name: Area D
Sample No: 85063
Date Received 12/11/2004 Total No Pages: 1 of 1
CLIENT: Coffey Geosciences Pty Ltd
James Russell



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LANE COVE WEST NSW

2066

Tests are performed under a quality system certified as complying with ISO 9002.

Results & Conclusions assume that sampling is representative. This document shall not be reproduced except in full

TEST	RESULT	COMMENTS
pH in water 1:2	7.8	slight alkalinity
pH in CaCl ₂ 1:2	7.0	neutral
EC mS/cm 1:2	0.17	very low - not saline
Chlorides mg/kg		

CATION ANALYSIS

TEST Unit	SOLUBLE		EXCHANGEABLE		
	meq%	Comment	meq%	% of ECEC	Comment
Sodium			.11	1.80	low - good
Potassium			.09	1.40	low - deficient
Calcium			5.63	89.60	high
Magnesium			.45	7.20	very low
Aluminium					
ECEC			6.28		
Ca/Mg			12.50		unbalanced

mg/kg

Phosphate as P	18.4	adequate for pasture
Ammonium as N	6.4	low
Nitrate as N	7.8	low
Sulphate as S	11	low
Iron	52.9	slightly low
Zinc	20.3	adequate
Copper	8.5	adequate
Manganese	5.9	adequate
Boron		

Recommendations

This chemistry of this soil is slightly unbalanced. The pH is slightly alkaline and could be slightly acidified with an application of Iron Sulphur at 100g/m². This will also improve the Iron levels in the soil. The general nutrient levels are low and should be increased with the addition of fertiliser. We recommend applying *Patons Nitroca* or equivalent at 30-40g/m² as this should create optimal growth conditions in this soil.

Explanation of the Methods:

pH, EC, Soluble Cations, Nitrate: Bradley et al (1993). Exchangeable Cations, ECEC: Method 15A1 Rayment & Higginson (1992). Chloride: Vogel (1981). Aluminium: Method 3500 PHA (1992). Phosphate: Method 9E1 Rayment & Higginson (1992). Ammonium, Sulphate, Iron, Copper, Manganese + Zinc: Method 83-1 to 83-5 Black (1983). Boron: Method 12C2 Rayment & Higginson (1992).

Checked by Principal.....
Simon Leake Date of Report

Consultant.....
C. Moore