

traffic modelling;

Cranbrook School, Bellevue Hill

For Cranbrook School 21st September 2018 parking; traffic; civil design; wayfinding; ptc.

Document Control

Cranbrook School, Bellevue Hill, Traffic modelling

| Issue | Date | Issue Details | Author | Reviewed | For the attention of |
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| 1 | 10/08/2018 | Draft Report | SW | AM | Mark Flanagan |
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1. Introduction

Ptc. have been engaged by Cranbrook School to prepare responses to the Traffic and Transport comments issued by the approving authorities, in relation to the proposed state significant development (SSD) at Cranbrook School, Bellevue Hill (SSD 8812)

This report relates specifically to the 'Response to Submission (RtS)' provided by the Department of Planning and Environment and is a response to Key Issue 2 –

- Examination and modelling of the following intersections is to be submitted as part of the RtS:
 - New South Head Road / Rose Bay Avenue
 - New South Head Road / Victoria Road

2. Traffic Impact Assessment

2.1 Existing Traffic Volumes

To determine the current traffic volumes, intersection surveys were conducted on the 8th August, 2018 between 7:00am to 9:00am and 3:00pm to 6:00pm on the subject intersections:

- New South Head Road & Rose Bay Avenue
- New South Head Road & Victoria Road

The location of these surveys is shown Figure 1.

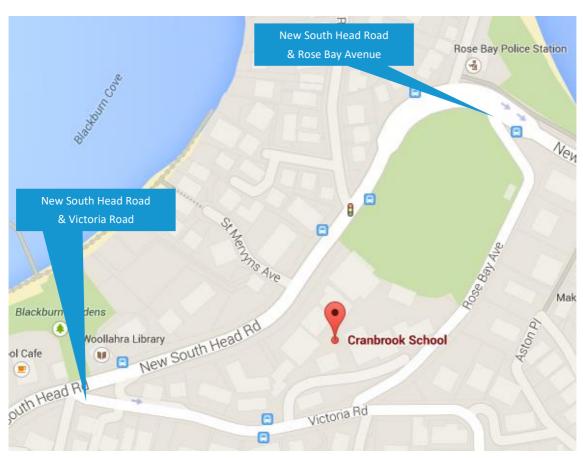
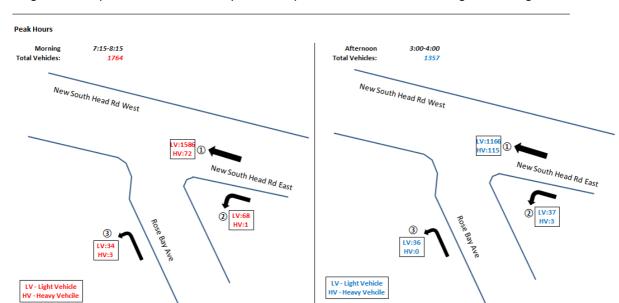


Figure 1 – Location of intersection surveys

The peak hours for the corresponding intersections were determined to be as follows:

- New South Head Road & Rose Bay Avenue:
 - Morning Peak hour: 7.15am to 8.15am
 - Evening Peak Hour: 3.00pm to 4.00pm
- New South Head Road & Victoria Road:
 - Morning Peak hour: 7.00am to 8.00am
 - Evening Peak Hour: 4.45pm to 5.45pm



A diagrammatic representation of the survey results for peak hour volume is shown in Figure 2 and Figure 3.

Figure 2 – New South Head Road & Rose Bay Avenue – Existing Traffic Volumes

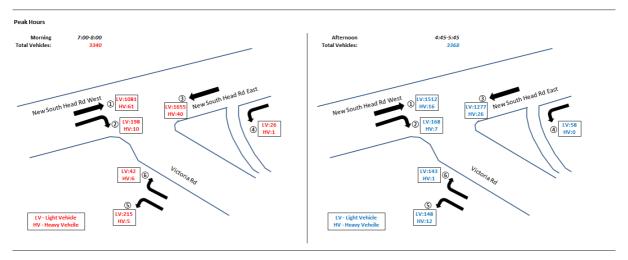


Figure 3 – New South Head Road & Victoria Road – Existing Traffic Volumes

From the survey data, a volume analysis was performed using SIDRA Intersection 7.0 software, a micro-analytical tool for individual intersections and whole-network modelling. The models are based on the collected traffic survey data. SIDRA provides a number of performance indicators, outlined below:

- Degree of Saturation The total usage of the intersection expressed as a factor of 1 with 1 representing 100% use/saturation. (e.g. 0.8=80% saturation)
- Average Delay- The average delay encountered by all vehicles passing through the intersection. It is often
 important to review the average delay of each approach as a side road could have a long delay time, while the
 large free flowing major traffic will provide an overall low average delay.
- Level of Service (LoS) This is a categorization of average delay, intended for simple reference. The RMS adopts the following bands:
- 95% Queue Lengths (Q95) is defined to be the queue length in metres that has only a 5-percent probability of being exceeded during the analysis time period. It transforms the average delay into measurable distance units.

Level of Service is a good indicator of overall performance for individual intersections, with each level summarised in Table 1

Table 1 - Intersection Performance - Levels of Service

| Level of Service | Average Delay (secs/vehicle) | Traffic Signals, Roundabout | Give Way & Stop Signs |
|---------------------|------------------------------|---|---|
| Α | <14 | Good operation | |
| В | 15 to 28 | Good with acceptable delays & spare capacity | Acceptable delays & spare capacity |
| С | 29 to 42 | Satisfactory | Satisfactory, but accident study required |
| D | 43 to 56 | Operating near capacity | Near capacity & accident study required |
| E | 57 to 70 | At capacity. At signals, incidents would cause excessive delays. Roundabouts require other control mode | At capacity, requires other control mode |
| F | >70 | Extra capacity required | Extreme delay, major treatment required |

The SIDRA results are displayed in Table 2 and the movement summaries can be found in Attachment 1..

Table 2 – Summary of Existing Traffic Conditions

| Intersection | Period | Level of Service | Average Delay (sec) | Degree of Saturation | Average Length of Queue (m) |
|---------------------|---------|---------------------|---------------------|-------------------------|-----------------------------------|
| New South Head Road | AM Peak | LOS A | 0.3 | 0.460 | 1.0 |
| & Rose Bay Avenue | PM Peak | LOS A | .2 | 0.454 | 0.6 |
| New South Road | AM Peak | LOS B | 20.8 | 0.858 | 181.9 |
| & Victoria Road | PM Peak | LOS B | 15.8 | 0.764 | 134.9 |

2.2 Proposed Development Traffic Generation

As outlined in Section 3 of the 'Parking and Traffic Assessment' (prepared by **ptc.** dated 10th May 2018, that the development does not propose any significant increases in the staff FTE and no increase in student population beyond the existing enrolment cap.

The measures outlined in Section 6.2 (Drop off and Pick Up facilities), Section 6.4 (On Street Parking Provision) and Section 6.5 (On Site Parking Provision) of the 'Parking and Traffic Assessment' (prepared by **ptc.** dated 10th May 2018), should not change the origin and destination of staff and students accessing and egressing the site and therefore should not affect the peak hour traffic movements on the external road network.



Attachment 1 - SIDRA Movement Summary

Site: 1 [[AM Existing] New South Head Road / Rose Bay Avenue]

中中Network: N101 [AM Existing]

Date: 09/08/18 Time: 7:15-8:15 Site Category: (None) Giveway / Yield (Two-Way)

| Move | ment F | Performan | ce - V | ehicles | | | | | | | | | | |
|---------|--------|-------------|---------|-----------|------|-------|---------|----------|------------|----------|--------|-----------|-----------|---------|
| Mov | Т | Demand F | Flows | Arrival I | lows | Deg. | Average | Level of | Aver. Back | of Queue | Prop. | Effective | Aver. No. | Average |
| ID | Turn | Total | HV | Total | HV | Satn | Delay | Service | Vehicles | Distance | Queued | Stop Rate | Cycles | Speed |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | | km/h |
| South: | Rose E | Bay Avenue |) | | | | | | | | | | | |
| 1 | L2 | 39 | 8.1 | 39 | 8.1 | 0.103 | 13.8 | LOS A | 0.1 | 1.0 | 0.73 | 0.88 | 0.73 | 32.9 |
| Approa | ach | 39 | 8.1 | 39 | 8.1 | 0.103 | 13.8 | LOS A | 0.1 | 1.0 | 0.73 | 0.88 | 0.73 | 32.9 |
| East: 1 | New So | uth Head R | Road (E | East) | | | | | | | | | | |
| 4 | L2 | 73 | 1.4 | 73 | 1.4 | 0.040 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.58 | 0.00 | 48.8 |
| 5 | T1 | 1745 | 4.3 | 1745 | 4.3 | 0.460 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| Approa | ach | 1818 | 4.2 | 1818 | 4.2 | 0.460 | 0.3 | NA | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 58.9 |
| West: | New So | outh Head F | Road (| West) | | | | | | | | | | |
| 11 | T1 | 1253 | 5.6 | 1253 | 5.6 | 0.333 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| Approa | ach | 1253 | 5.6 | 1253 | 5.6 | 0.333 | 0.0 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| All Vel | nicles | 3109 | 4.8 | 3109 | 4.8 | 0.460 | 0.3 | NA | 0.1 | 1.0 | 0.01 | 0.02 | 0.01 | 59.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 3 [[AM Existing] New South Head Road / Victoria Road]

Date: 09/08/18 Time: 7:00-8:00 Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 100 seconds (Site User-Given Cycle Time)

| Move | ment F | Performan | ice - V | ehicles | ; | | | | | | | | | |
|---------|---------|------------|---------|---------|-------|-------|---------|----------|------------|----------|--------|-----------|-----------|---------|
| Mov | Turn | Demand | Flows | Arrival | Flows | Deg. | Average | Level of | Aver. Back | of Queue | Prop. | Effective | Aver. No. | Average |
| ID | Tuiti | Total | HV | Total | HV | Satn | Delay | Service | Vehicles | Distance | Queued | Stop Rate | Cycles | Speed |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | | km/h |
| South | Victori | a Road | | | | | | | | | | | | |
| 1 | L2 | 232 | 2.3 | 232 | 2.3 | 0.330 | 34.8 | LOS C | 4.5 | 32.0 | 0.82 | 0.78 | 0.82 | 28.3 |
| 3 | R2 | 51 | 12.5 | 51 | 12.5 | 0.330 | 48.0 | LOS D | 2.3 | 17.2 | 0.94 | 0.77 | 0.94 | 13.8 |
| Appro | ach | 282 | 4.1 | 282 | 4.1 | 0.330 | 37.2 | LOS C | 4.5 | 32.0 | 0.84 | 0.78 | 0.84 | 25.9 |
| East: I | New So | uth Head F | Road (e | east) | | | | | | | | | | |
| 4 | L2 | 28 | 3.7 | 28 | 3.7 | 0.016 | 5.7 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 52.8 |
| 5 | T1 | 1784 | 2.4 | 1784 | 2.4 | 0.858 | 25.8 | LOS B | 25.5 | 181.9 | 0.91 | 0.90 | 0.99 | 39.5 |
| Appro | ach | 1813 | 2.4 | 1813 | 2.4 | 0.858 | 25.5 | LOS B | 25.5 | 181.9 | 0.90 | 0.89 | 0.97 | 39.6 |
| West: | New So | outh Head | Road (| west) | | | | | | | | | | |
| 11 | T1 | 1202 | 5.3 | 1202 | 5.3 | 0.578 | 5.3 | LOS A | 10.6 | 77.6 | 0.43 | 0.39 | 0.43 | 47.2 |
| 12 | R2 | 219 | 4.8 | 219 | 4.8 | 0.646 | 46.2 | LOS D | 6.0 | 44.0 | 0.98 | 0.98 | 1.39 | 24.2 |
| Appro | ach | 1421 | 5.3 | 1421 | 5.3 | 0.646 | 11.6 | LOS A | 10.6 | 77.6 | 0.52 | 0.48 | 0.58 | 38.6 |
| All Vel | nicles | 3516 | 3.7 | 3516 | 3.7 | 0.858 | 20.8 | LOS B | 25.5 | 181.9 | 0.74 | 0.72 | 0.80 | 38.1 |

♦Network: N101 [AM Existing]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Move | Movement Performance - Pedestrians | | | | | | | | | | | | | |
|--------|------------------------------------|--------|---------|----------|-----------------|----------|--------|-----------|--|--|--|--|--|--|
| Mov | | Demand | Average | Level of | Average Back of | Queue | Prop. | Effective | | | | | | |
| ID | Description | Flow | Delay | Service | Pedestrian | Distance | Queued | Stop Rate | | | | | | |
| | | ped/h | sec | | ped | m | | | | | | | | |
| P4 | West Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 | | | | | | |
| All Pe | destrians | 53 | 44.3 | LOS E | | | 0.94 | 0.94 | | | | | | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 2 [[PM Existing] New South Head Road / Rose Bay Avenue]

中中Network: N101 [PM Existing]

Date: 09/08/18 Time: 3:00-4:00 Site Category: (None) Giveway / Yield (Two-Way)

| Move | ment F | Performan | ce - V | ehicles | | | | | | | | | | |
|---------|--------|-------------|---------|---------|------|-------|---------|----------|------------|----------|--------|-----------|-----------|---------|
| Mov | _ | Demand F | lows | Arrival | lows | Deg. | Average | Level of | Aver. Back | of Queue | Prop. | Effective | Aver. No. | Average |
| ID | Turn | Total | HV | Total | HV | Satn | Delay | Service | Vehicles | Distance | Queued | Stop Rate | Cycles | Speed |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | | km/h |
| South: | Rose I | Bay Avenue |) | | | | | | | | | | | |
| 1 | L2 | 38 | 0.0 | 38 | 0.0 | 0.066 | 10.0 | LOS A | 0.1 | 0.6 | 0.58 | 0.80 | 0.58 | 37.4 |
| Approa | ach | 38 | 0.0 | 38 | 0.0 | 0.066 | 10.0 | LOS A | 0.1 | 0.6 | 0.58 | 0.80 | 0.58 | 37.4 |
| East: 1 | New So | outh Head R | Road (E | East) | | | | | | | | | | |
| 4 | L2 | 42 | 7.5 | 42 | 7.5 | 0.024 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.57 | 0.00 | 47.7 |
| 5 | T1 | 1348 | 9.0 | 1348 | 9.0 | 0.366 | 0.0 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| Approa | ach | 1391 | 8.9 | 1391 | 8.9 | 0.366 | 0.2 | NA | 0.0 | 0.0 | 0.00 | 0.02 | 0.00 | 59.0 |
| West: | New So | outh Head F | Road (| West) | | | | | | | | | | |
| 11 | T1 | 1760 | 1.0 | 1760 | 1.0 | 0.454 | 0.1 | LOS A | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| Approa | ach | 1760 | 1.0 | 1760 | 1.0 | 0.454 | 0.1 | NA | 0.0 | 0.0 | 0.00 | 0.00 | 0.00 | 59.9 |
| All Vel | nicles | 3188 | 4.5 | 3188 | 4.5 | 0.454 | 0.2 | NA | 0.1 | 0.6 | 0.01 | 0.02 | 0.01 | 59.5 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Site: 3 [[PM Existing] New South Head Road / Victoria Road]

Date: 09/08/18 Time: 4:45-5:45 Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 100 seconds (Site User-Given Cycle Time)

| Move | ment F | Performan | ce - V | ehicles | | | | | | | | | | |
|---------|-----------|-------------|---------|-----------|-----|-------|---------|---------|------------|----------|--------|-----------|--------|-------|
| Mov | Turn | Demand F | lows | Arrival F | | Deg. | Average | | Aver. Back | | Prop. | | | |
| טו | | Total | HV | Total | HV | Satn | Delay | Service | Vehicles | Distance | Queued | Stop Rate | Cycles | Speed |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | | km/h |
| South | : Victori | a Road | | | | | | | | | | | | |
| 1 | L2 | 168 | 7.5 | 168 | 7.5 | 0.281 | 31.6 | LOS C | 3.7 | 27.4 | 0.77 | 0.77 | 0.77 | 29.6 |
| 3 | R2 | 152 | 0.7 | 152 | 0.7 | 0.631 | 52.1 | LOS D | 4.5 | 31.7 | 1.00 | 0.82 | 1.03 | 12.9 |
| Appro | ach | 320 | 4.3 | 320 | 4.3 | 0.631 | 41.3 | LOS C | 4.5 | 31.7 | 0.88 | 0.79 | 0.89 | 21.7 |
| East: I | New So | uth Head R | load (e | east) | | | | | | | | | | |
| 4 | L2 | 61 | 0.0 | 61 | 0.0 | 0.033 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.53 | 0.00 | 53.3 |
| 5 | T1 | 1372 | 2.0 | 1372 | 2.0 | 0.687 | 17.6 | LOS B | 15.5 | 110.2 | 0.77 | 0.70 | 0.77 | 44.3 |
| Appro | ach | 1433 | 1.9 | 1433 | 1.9 | 0.687 | 17.1 | LOS B | 15.5 | 110.2 | 0.74 | 0.69 | 0.74 | 44.6 |
| West: | New So | outh Head F | Road (| west) | | | | | | | | | | |
| 11 | T1 | 1608 | 1.0 | 1608 | 1.0 | 0.764 | 7.7 | LOS A | 19.1 | 134.9 | 0.55 | 0.52 | 0.57 | 43.1 |
| 12 | R2 | 184 | 4.0 | 184 | 4.0 | 0.464 | 32.7 | LOS C | 4.8 | 34.9 | 0.92 | 0.88 | 1.02 | 29.1 |
| Appro | ach | 1793 | 1.4 | 1793 | 1.4 | 0.764 | 10.3 | LOS A | 19.1 | 134.9 | 0.59 | 0.56 | 0.61 | 40.0 |
| All Vel | nicles | 3545 | 1.8 | 3545 | 1.8 | 0.764 | 15.8 | LOS B | 19.1 | 134.9 | 0.68 | 0.63 | 0.69 | 40.3 |

♦Network: N101 [PM Existing]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

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SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Move | ement Performance - P | Pedestrians | | | | | | |
|--------|-----------------------|-------------|---------|----------|-----------------|----------|--------|-----------|
| Mov | | Demand | Average | Level of | Average Back of | Queue | Prop. | Effective |
| ID | Description | Flow | Delay | Service | Pedestrian | Distance | Queued | Stop Rate |
| | | ped/h | sec | | ped | m | | |
| P4 | West Full Crossing | 53 | 44.3 | LOS E | 0.1 | 0.1 | 0.94 | 0.94 |
| All Pe | destrians | 53 | 44.3 | LOS E | | | 0.94 | 0.94 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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