Appendix B

SURVEYED STRUCTURES

B.1 OLDBURY CREEK STRUCTURES

B.1.1 Old Hume Highway plank bridge

Oldbury Creek flows under a plank bridge at the Old Hume Highway. The HEC-RAS model has included this structure based on a survey undertaken by Southern Cross Consulting Surveyors on 21 March 2014. The dimensions of the bridge structures included in the HEC-RAS model are:

- 650 mm thick plank
- No piers
- 5.4 m opening.



Photo 1.1 Old Hume Highway plank bridge

B.1.2 Hume Highway box culverts

Oldbury Creek flows through three large box culverts under the Hume Highway. The HEC-RAS model has included these structures based on a survey undertaken by Southern Cross Consulting Surveyors on 21 March 2014. The dimensions of the culvert structures included in the HEC-RAS model are:

■ Three cells, each 2 m high by 3 m wide.



Photo 1.2 Hume Highway box culverts

B.1.3 Inline structures

There are two inline structures on Oldbury Creek. The most upstream one is a concrete pad, and dirt mound. Under the concrete pad there are 5,300 mm pipes.



Photo 1.3 Upstream inline structure on Oldbury Creek

The downstream inline structure has a high embankment and the spillway is located near the road. There is a single1.6 diameter pipe. The pipe inlet is located at an RL 644.4 mAHD. Only when the water level is above this, will water be able to go through the pipe. At the time of survey the water level was 644.17 mAHD. This was assumed the initial water level in the XP RAFTS model.



Photo 1.4 Downstream inline structure on Oldbury Creek

B.1.4 Culverts under Medway Road

There are two 600mm pipes located under Medway Road to the west and a 900mm x 350mm box culvert located to the east.



Photo 1.5 Photo of Western twin pipe culvert looking upstream



Photo 1.6 Photo of Eastern box culvert looking downstream

B.1.5 Culvert under Hume Highway

There is a single 1.2 diameter pipe located under the Hume Highway, on a tributary that is North of Oldbury Creek. The culvert is located under a steep embankment.



Photo 1.7 Photo of culvert under Hume Highway on western side

B.1.6 Culvert under Old Rail embankment

There are 2 x 600mm diameter pipes located under the old Rail Embankment to the east and 2 x 450 mm diameter pipes located to the west. These are located south of Medway Road Culverts.



Photo 1.8 Photo of culvert under old rail embankment on eastern side



Photo 1.9 Photo of culvert under old rail embankment on eastern side

B.2 STONY CREEK STRUCTURES

B.2.1 Rail Bridge over Stony Creek

A rail bridge is located approximately 150m downstream of Berrima Road. The dimensions of the bridge structures used in the HEC-RAS model are listed below.

- 1.3m thick deck from top of track to bottom of bridge
- One pier under each of the northbound and southbound spans
- 12m opening



Photo 1.10 Photo of Stony Creek rail crossing facing downstream

B.2.2 Berrima Road culvert

A culvert is located on Stony Creek under Berrima Road. The HEC-RAS model has included these structures based on survey undertaken by Southern Cross Consulting Surveyors in February 2016. The Culvert has 4 box culverts each 3.6m x 2.15m. Cad drawings are shown of this culvert.

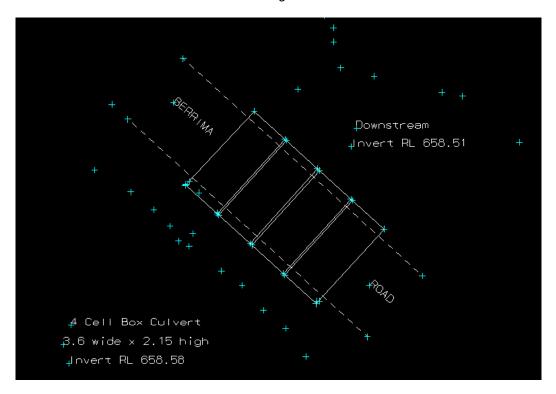


Figure 1.1 Cad drawing of culvert structure under Berrima Road

B.2.3 Structure at the northwest tributary of Oldbury Creek

B.2.3.1 CULVERT UNDER RAIL EMBANKMENT

There is a single box culvert 3.3 x 1.0 located under the railway, downstream of Berrima Road at the northwest tributary.



Photo 1.11 Photo of culvert structure under rail embankment on the northwest tributary facing upstream

B.2.3.2 CULVERT UNDER BERRIMA ROAD

There is a single 600mm diameter pipe located under Berrima Road at the northwester tributary of Stony Creek.



DOWNSTREAM

Photo 1.12 Photo of culvert structure under Berrima Road on the northwest tributary facing downstream

B.2.3.3 INLINE STORAGES ON NORTHWEST TRIBUTARY

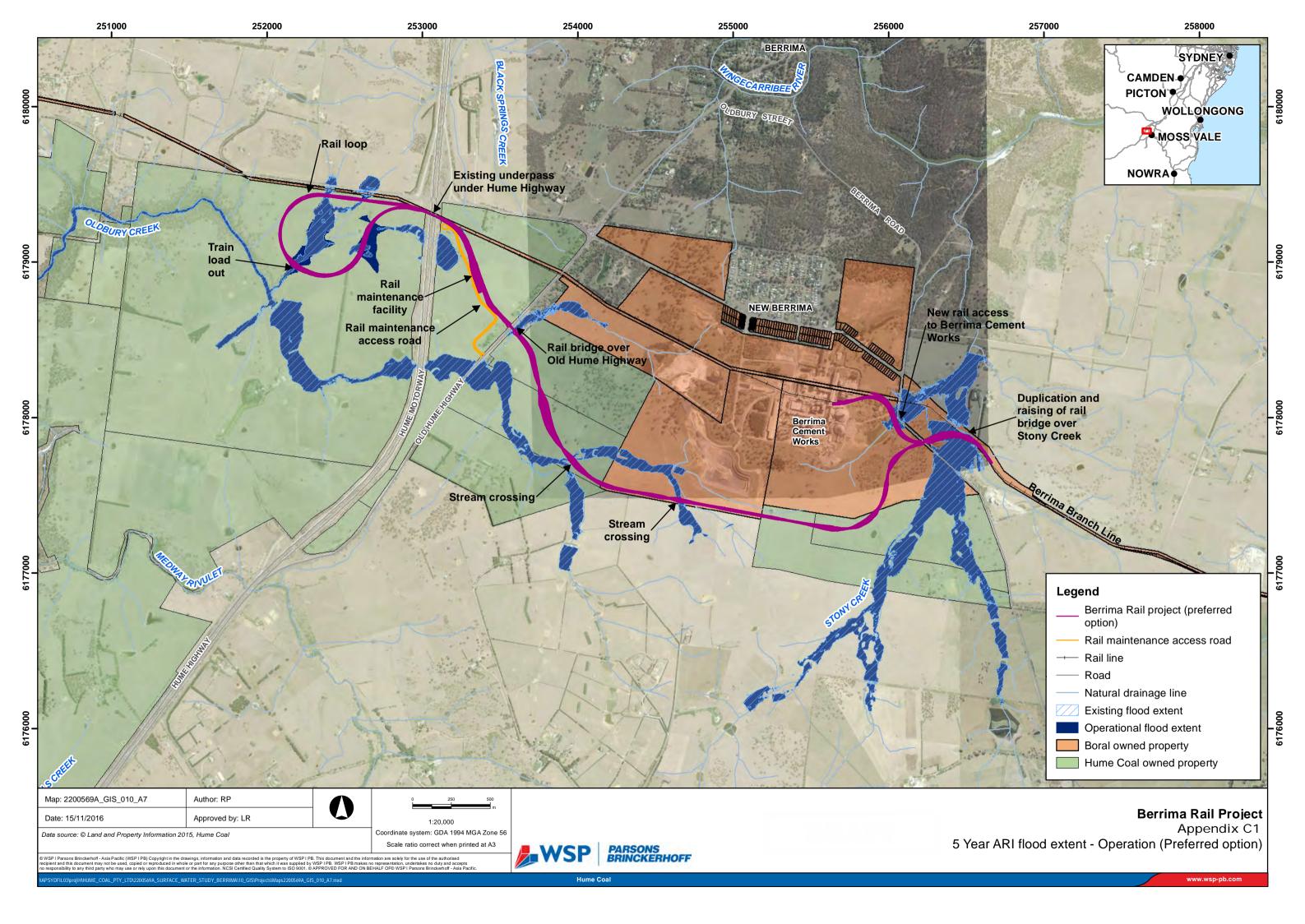
There are two inline storages located on the northwest tributary of Stony Creek. The dam walls and water levels were surveyed and included in the RAFTS model.

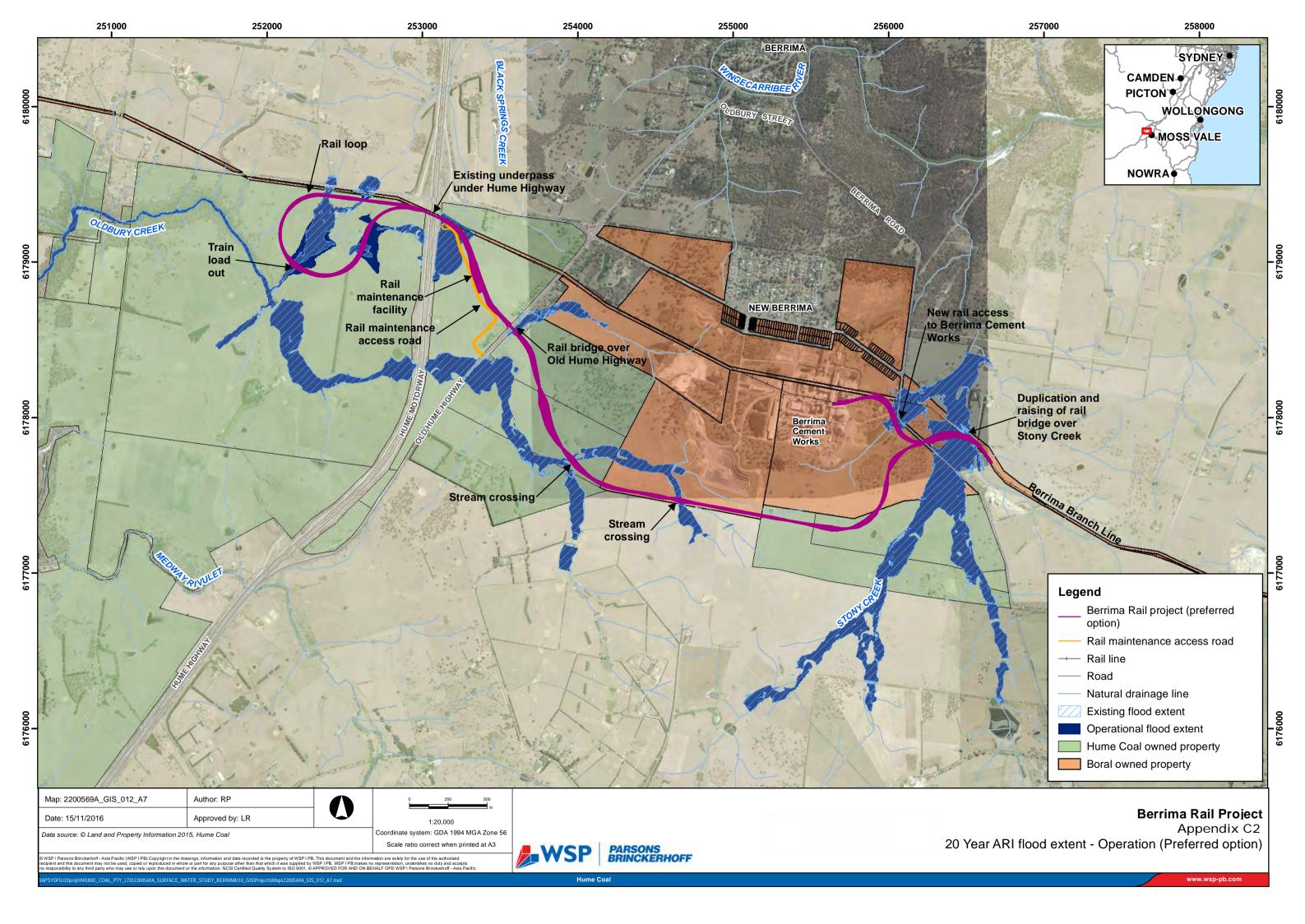


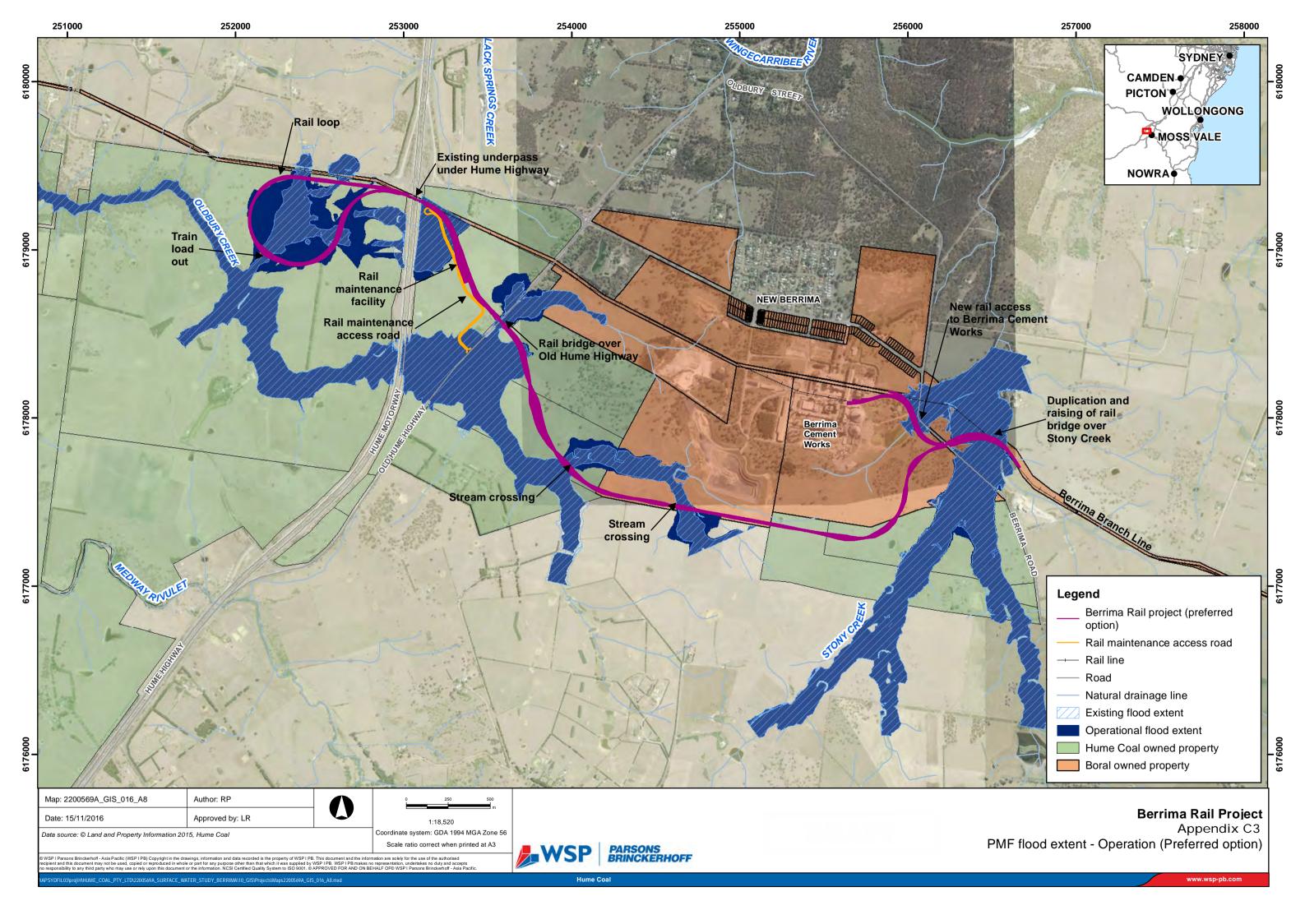
Photo 1.13 Photo of downstream inline storage on the north-western tributary

Appendix C

PREFERRED OPTION FLOOD MAPS

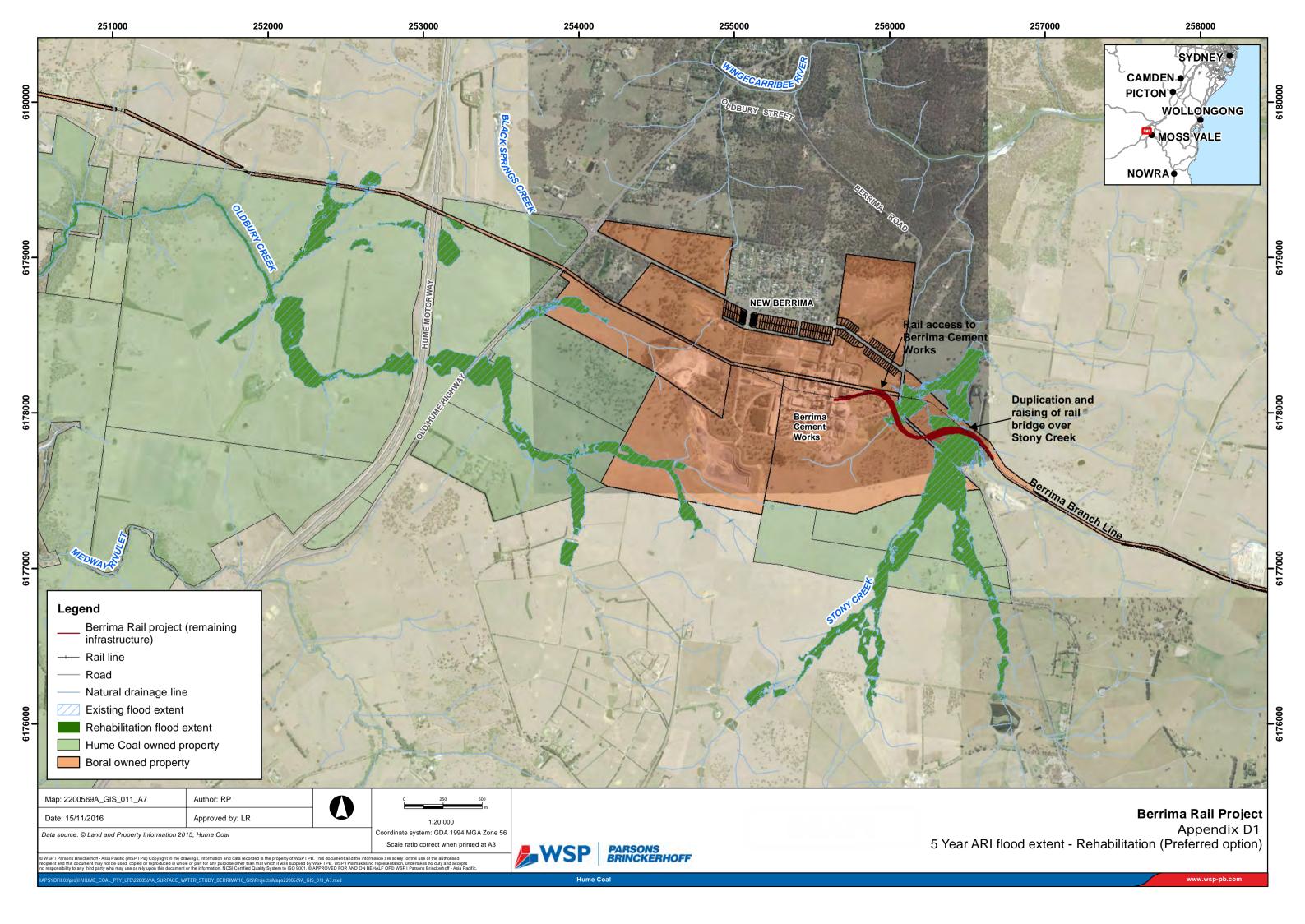


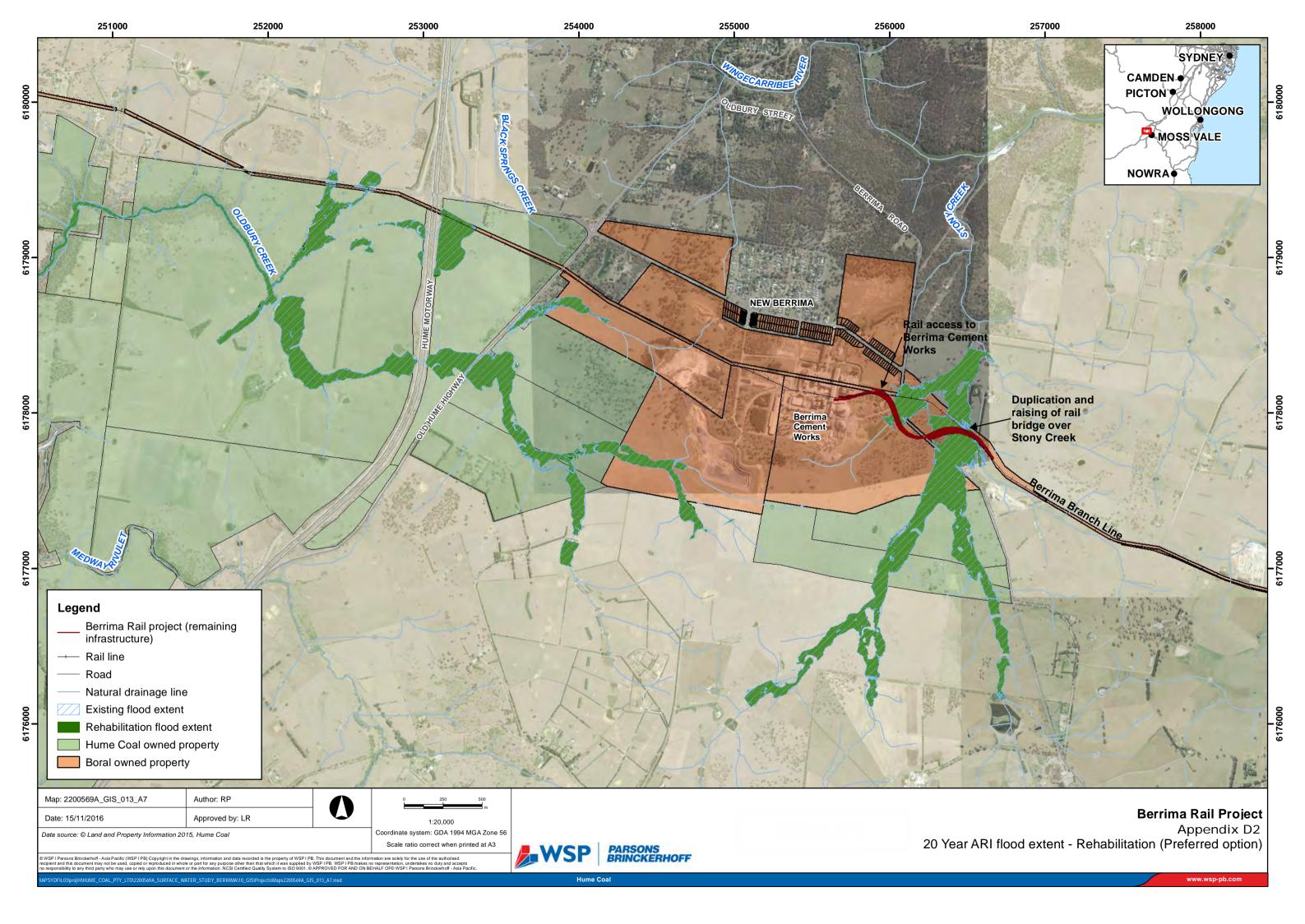


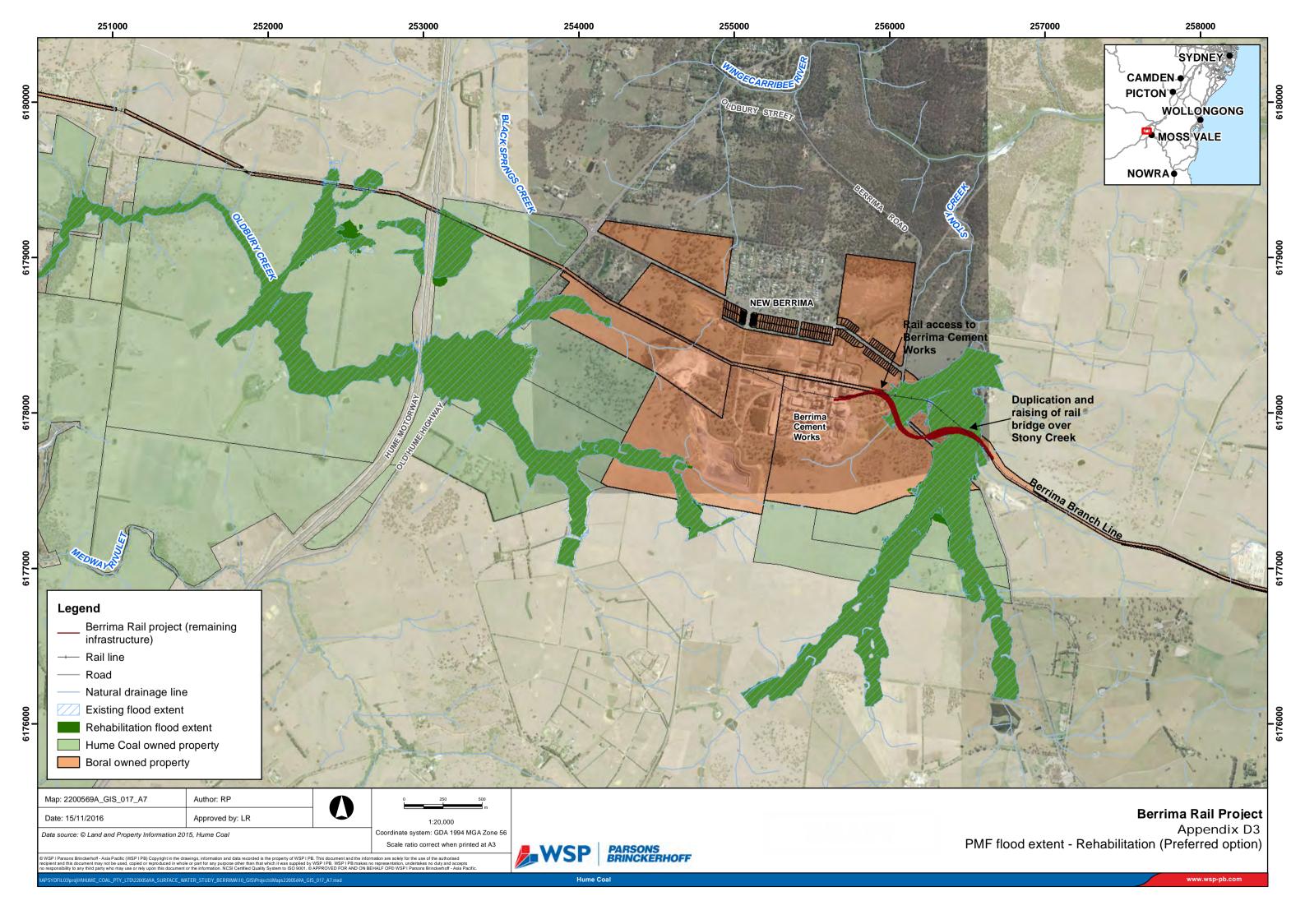


Appendix D

PREFERRED OPTION REHABILITATION FLOOD MAPS

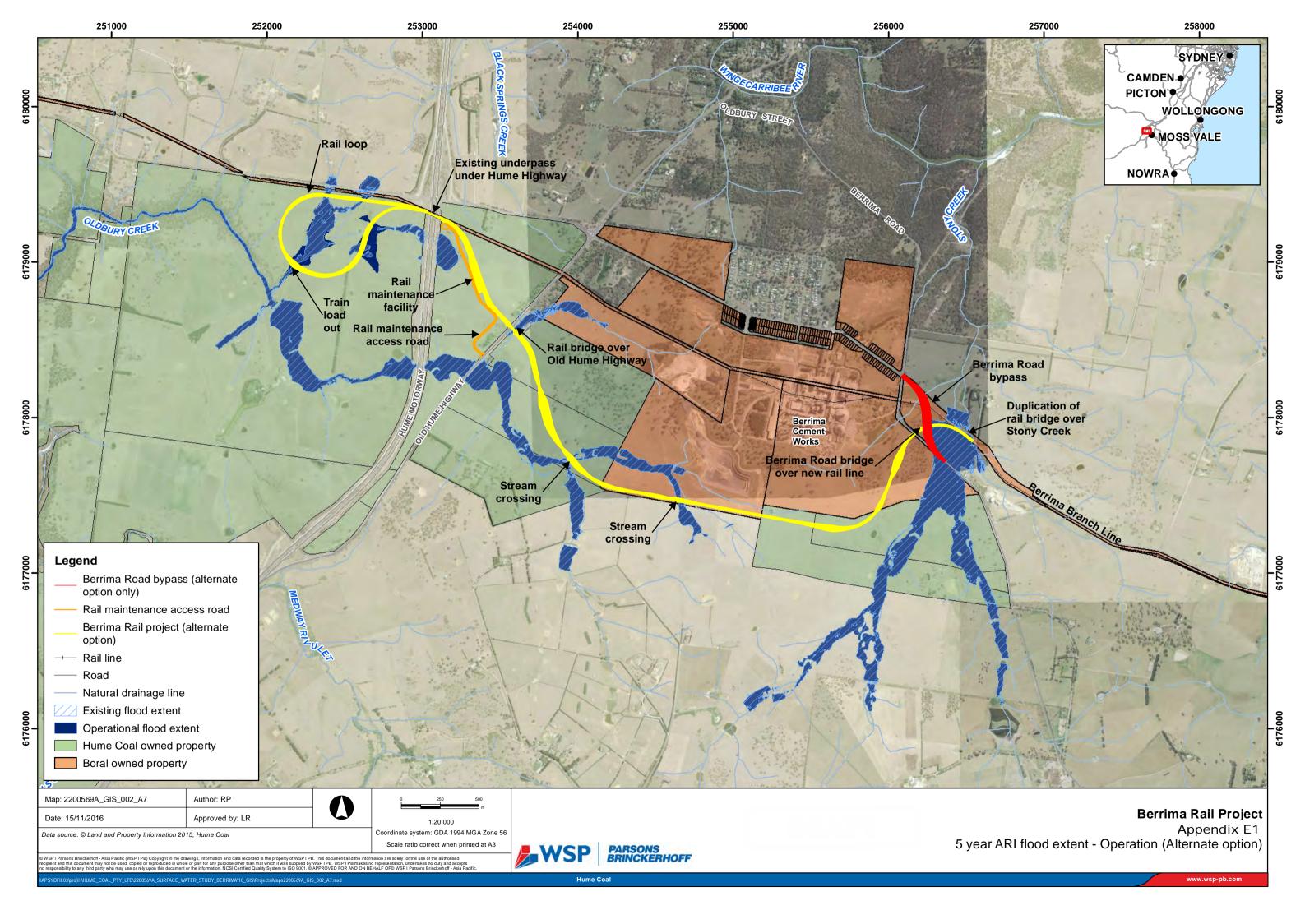


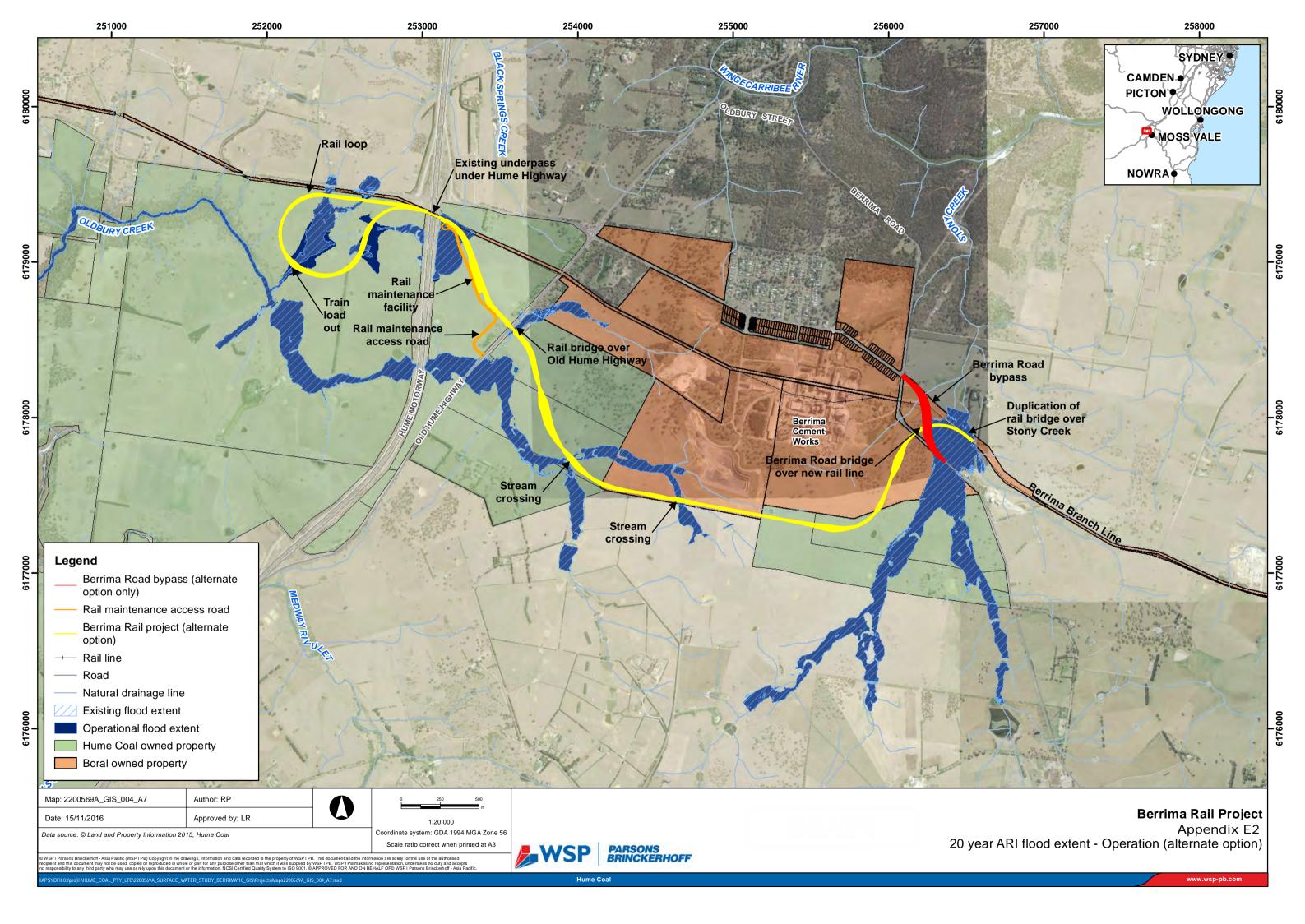


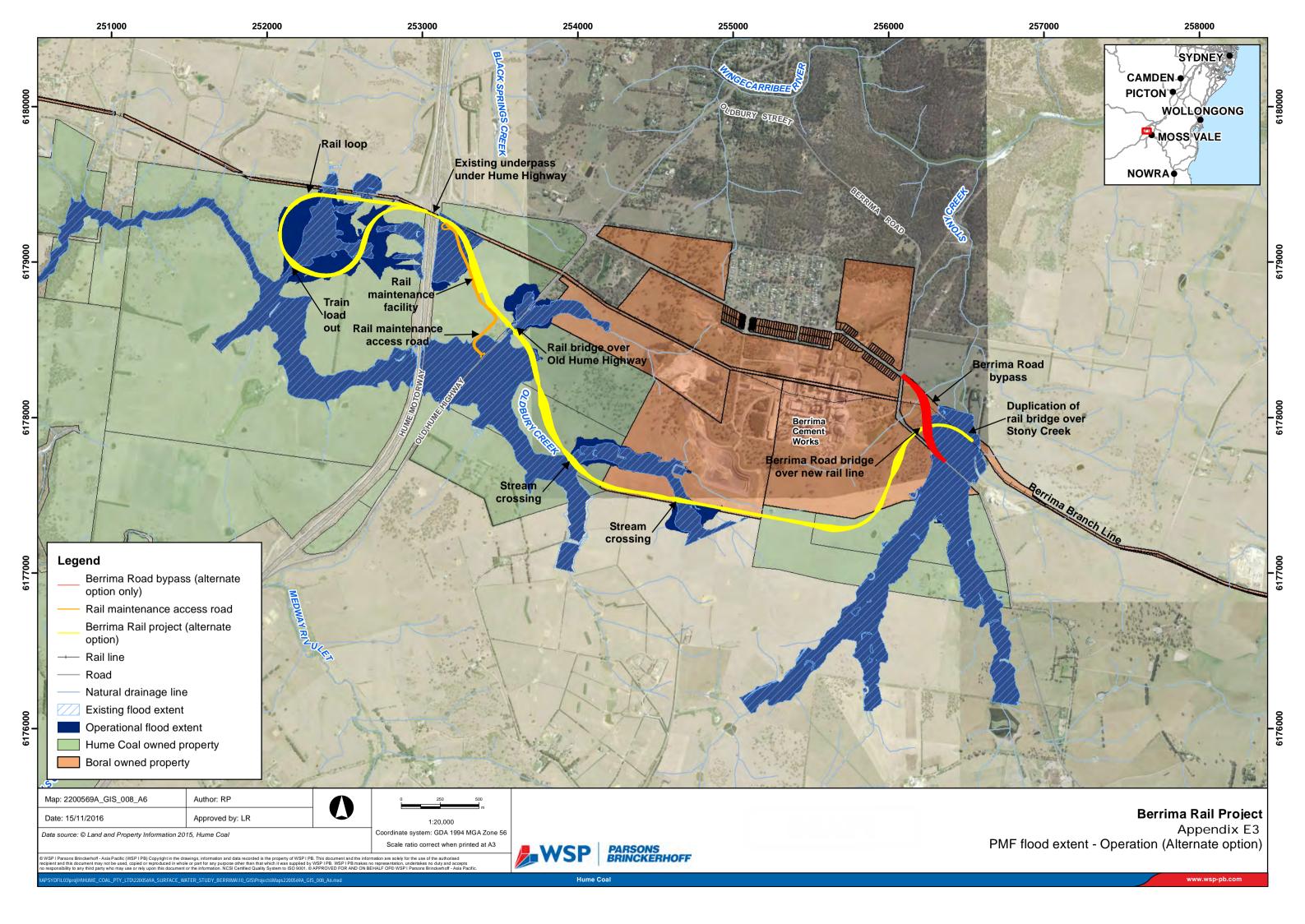


Appendix E

ALTERNATE OPTION FLOOD MAPS

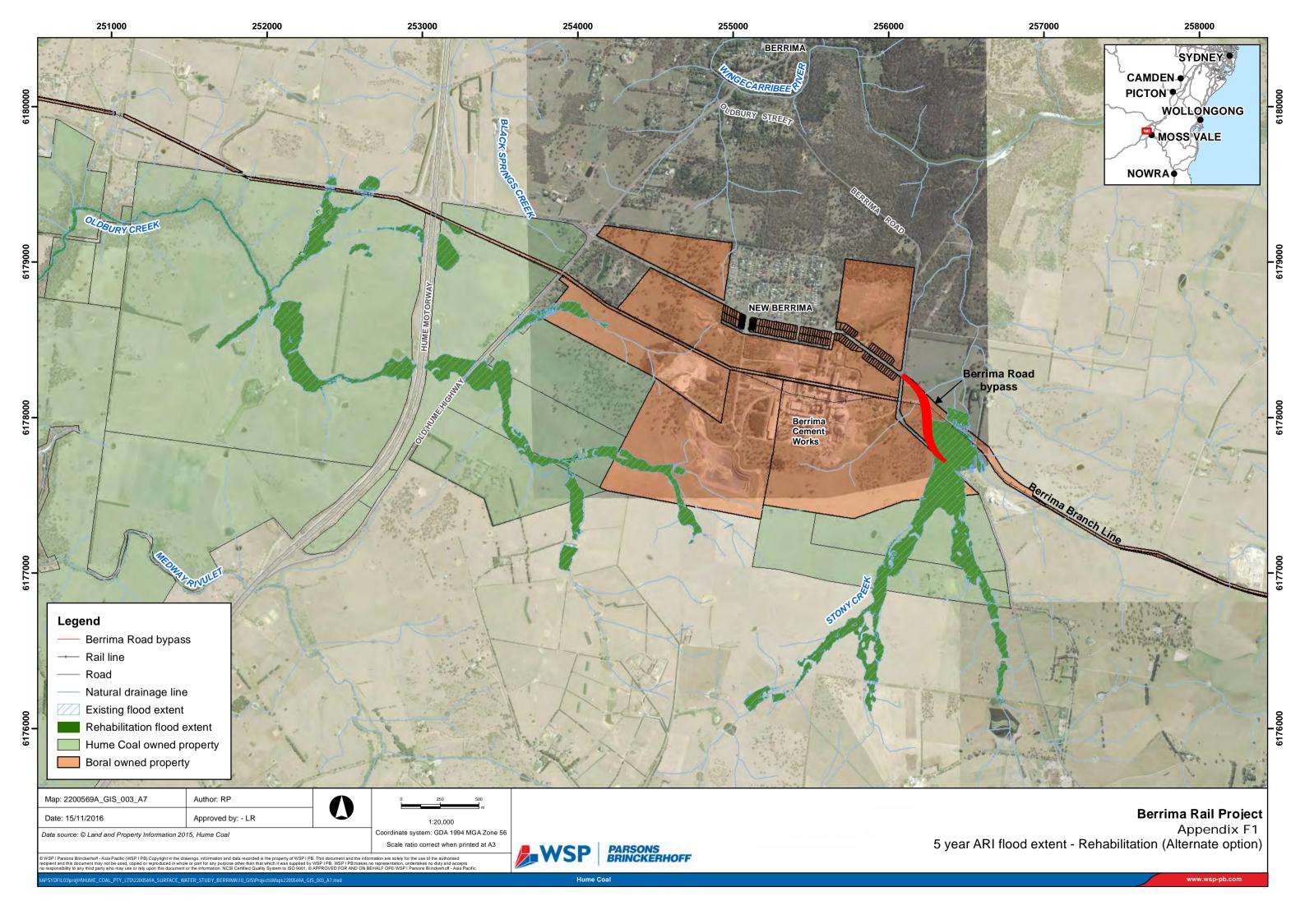


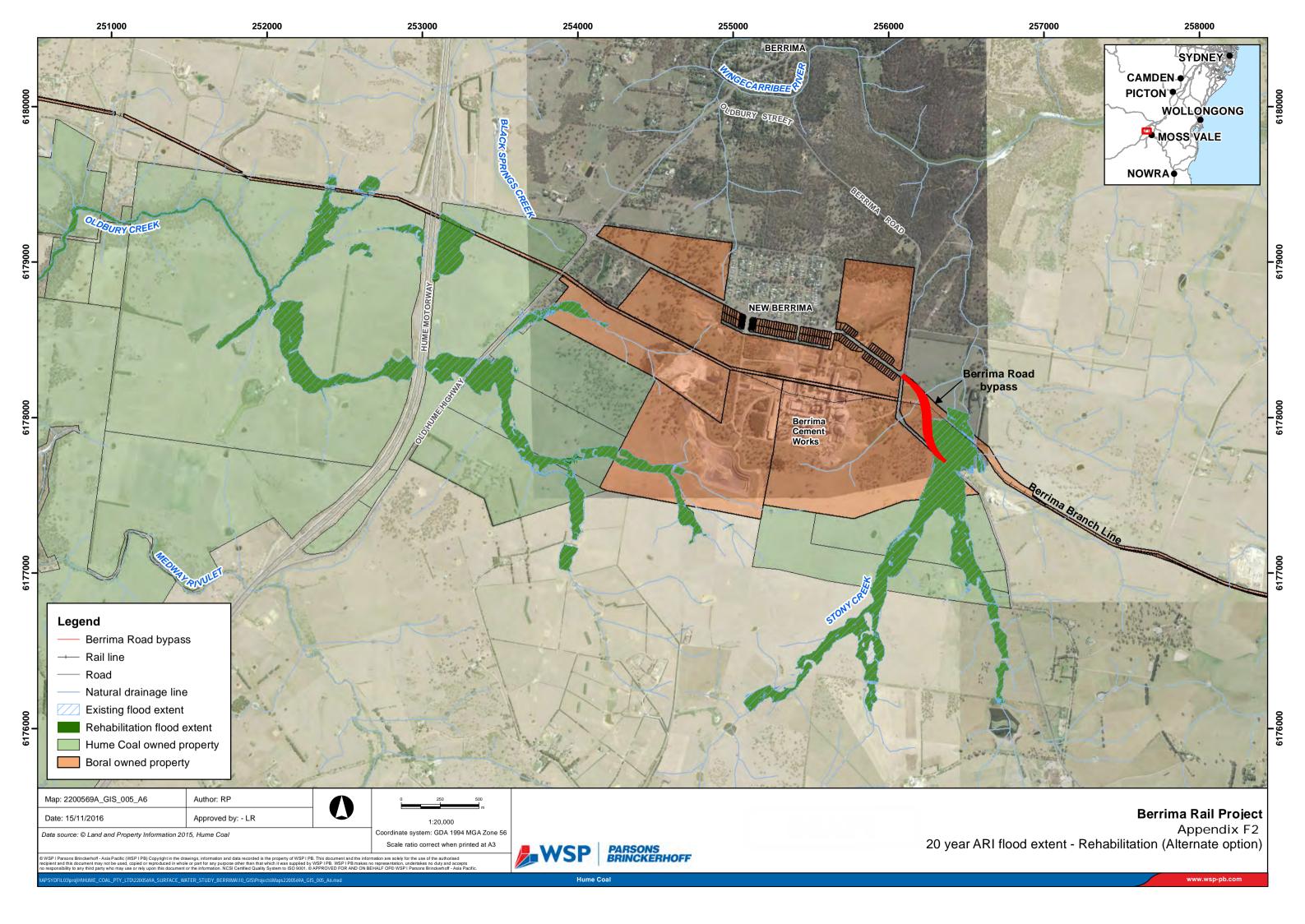


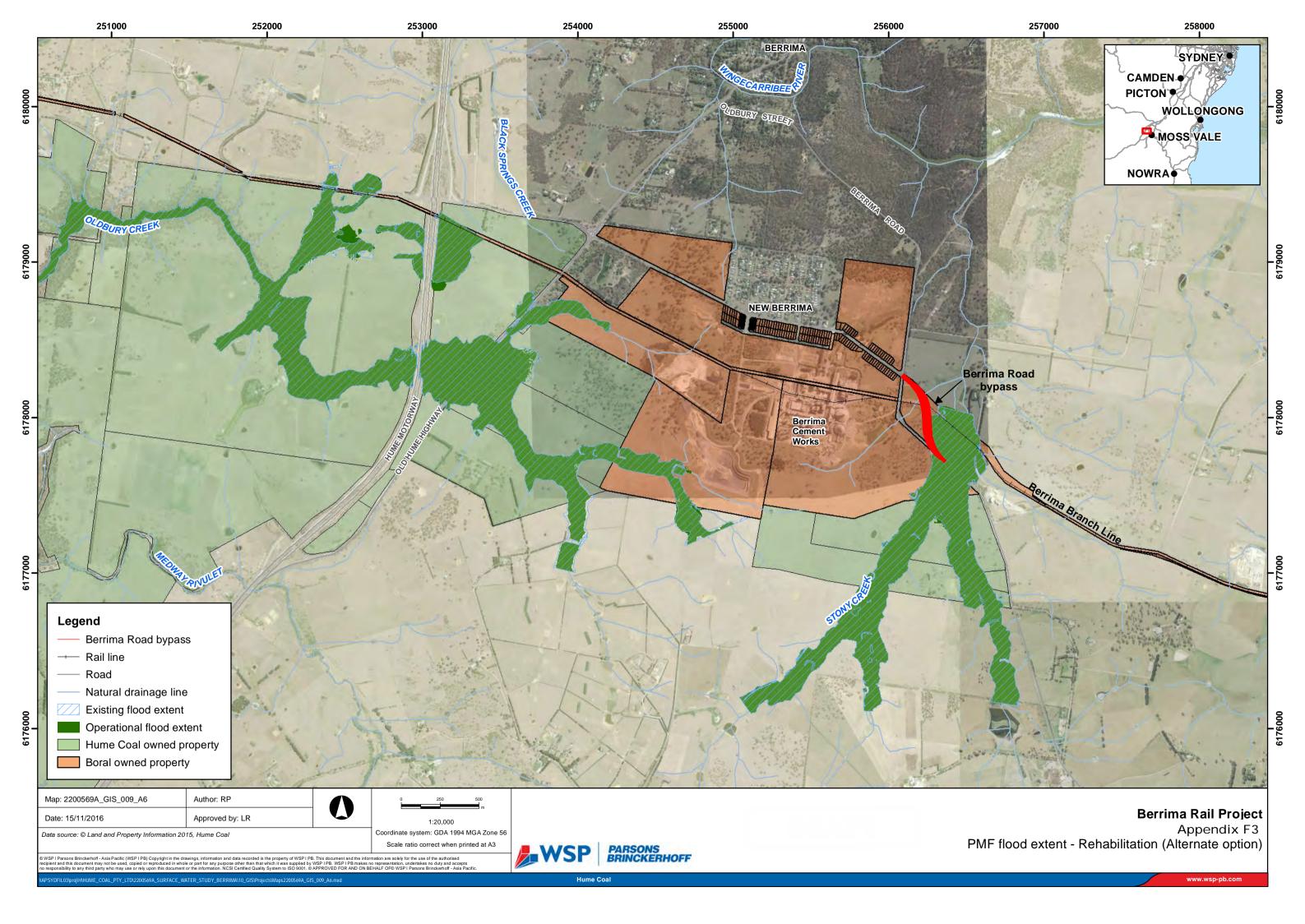


Appendix F

ALTERNATE OPTION REHABILITATION FLOOD MAPS

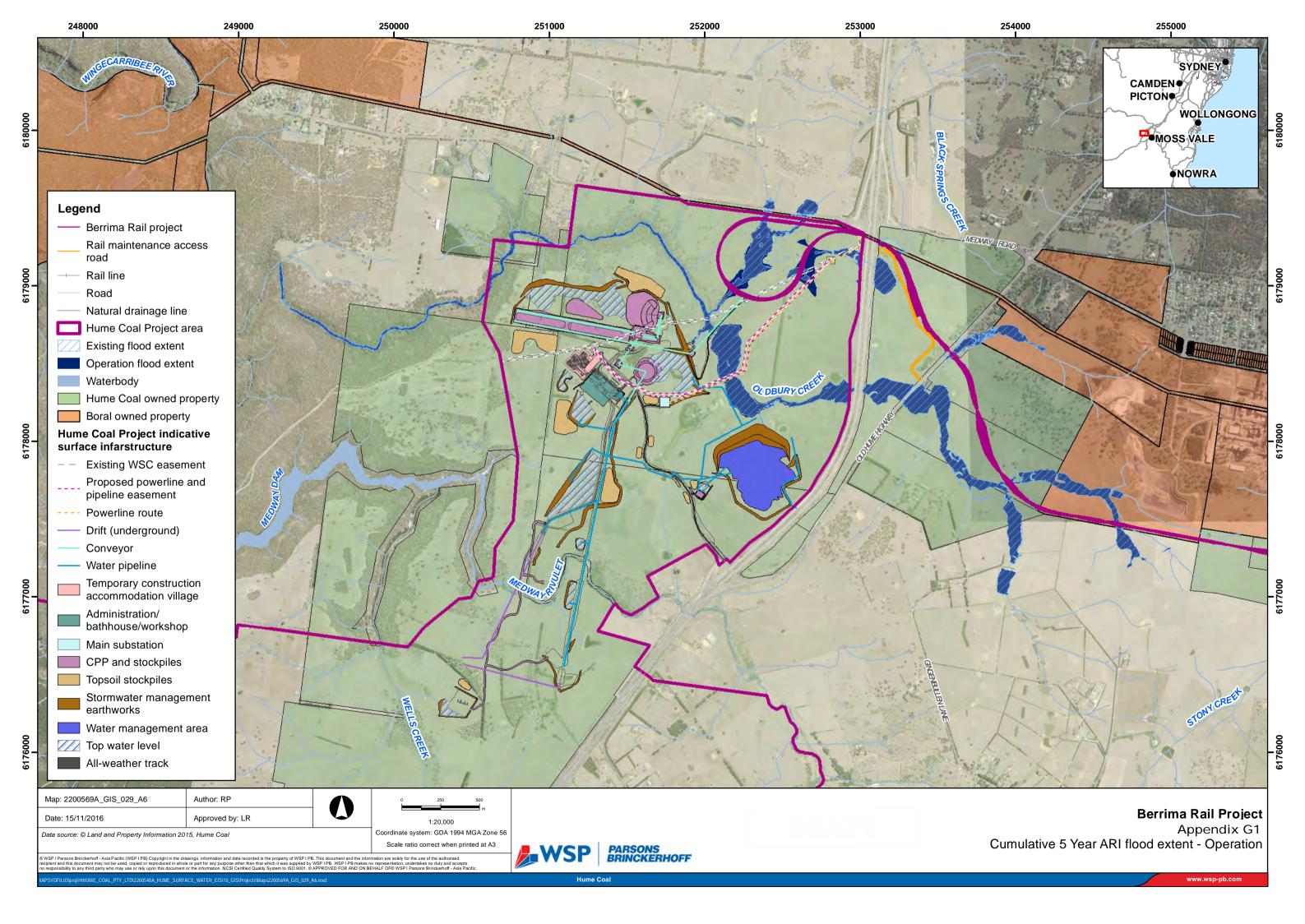


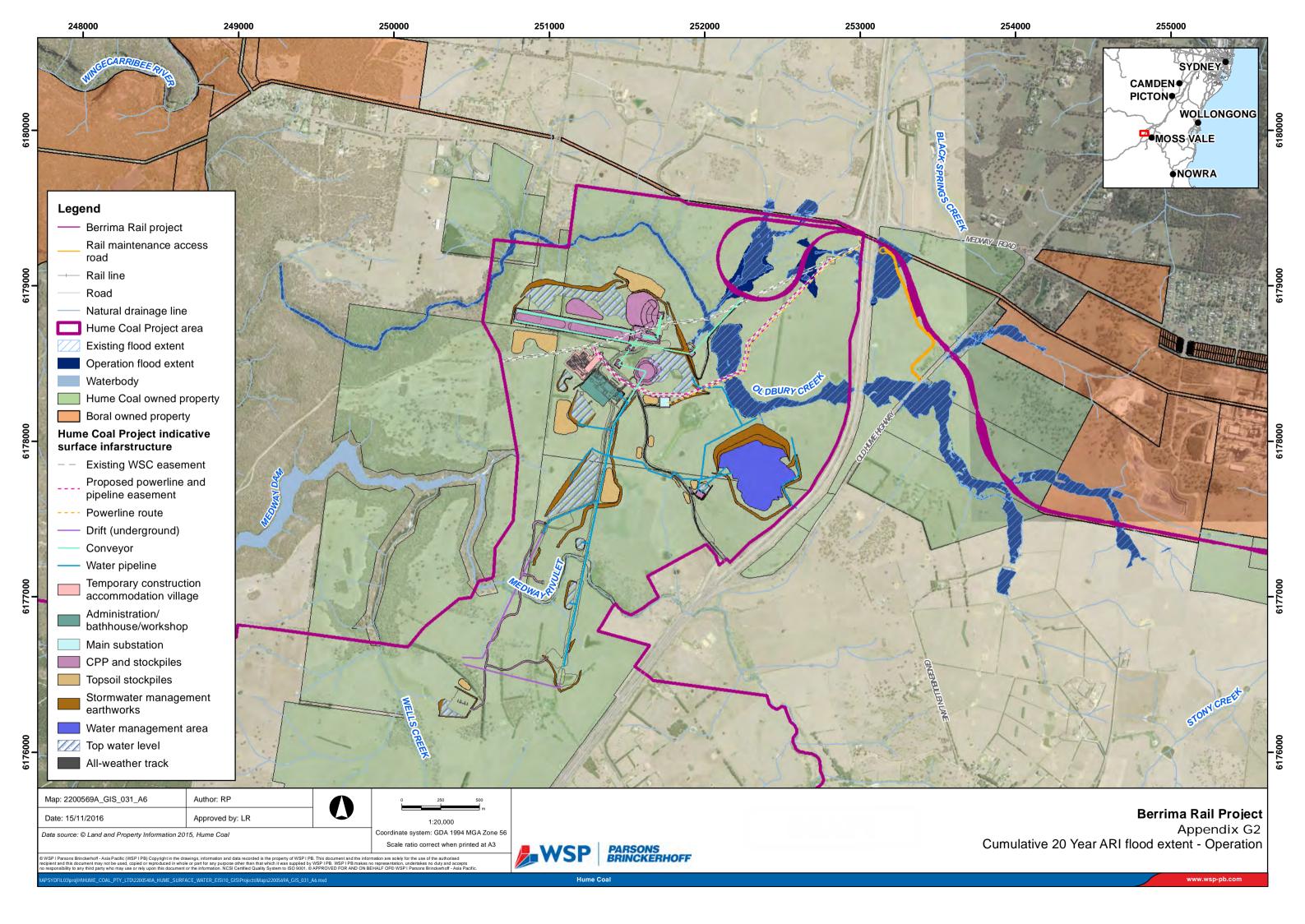


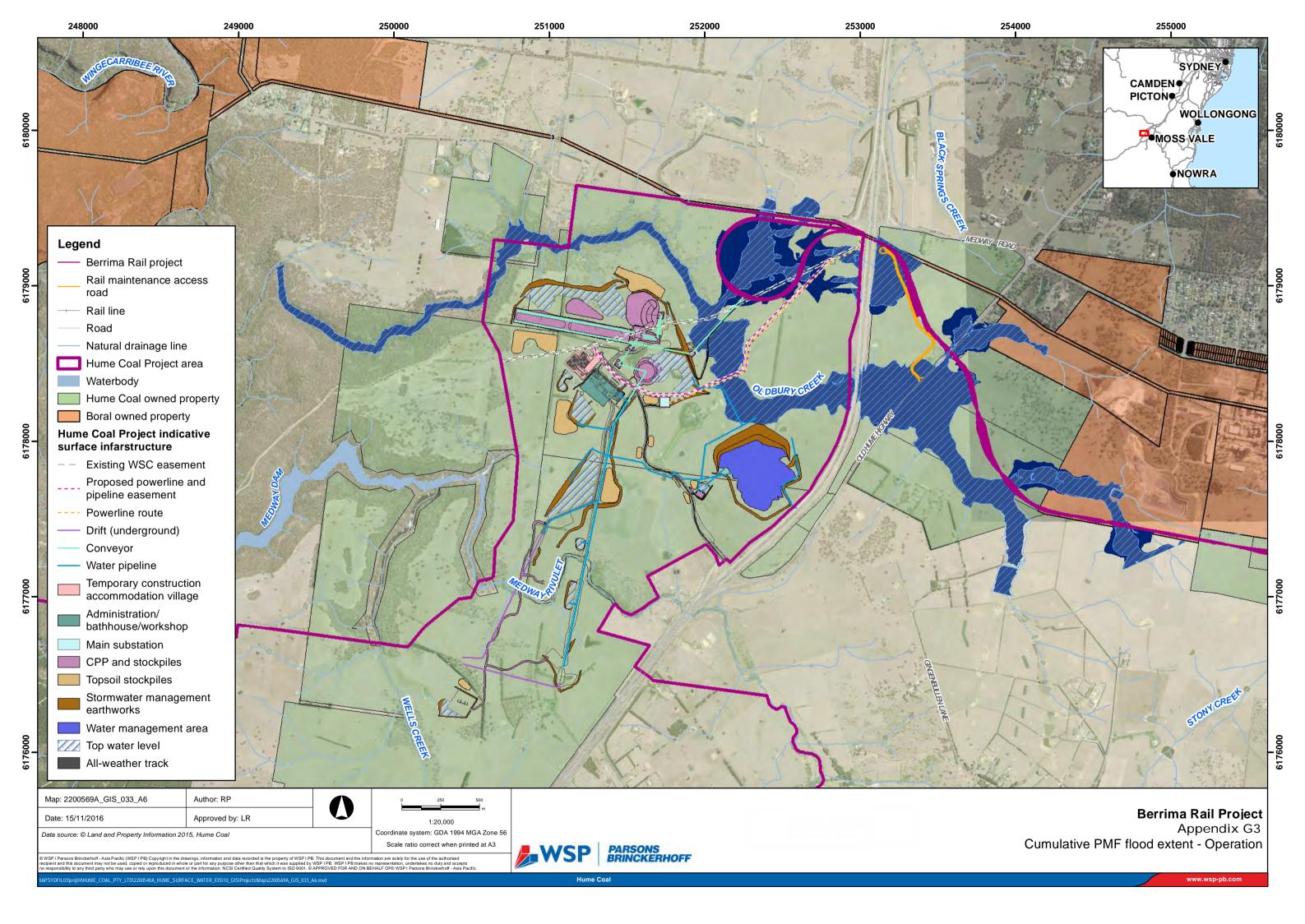


Appendix G

CUMULATIVE FLOOD MAPS

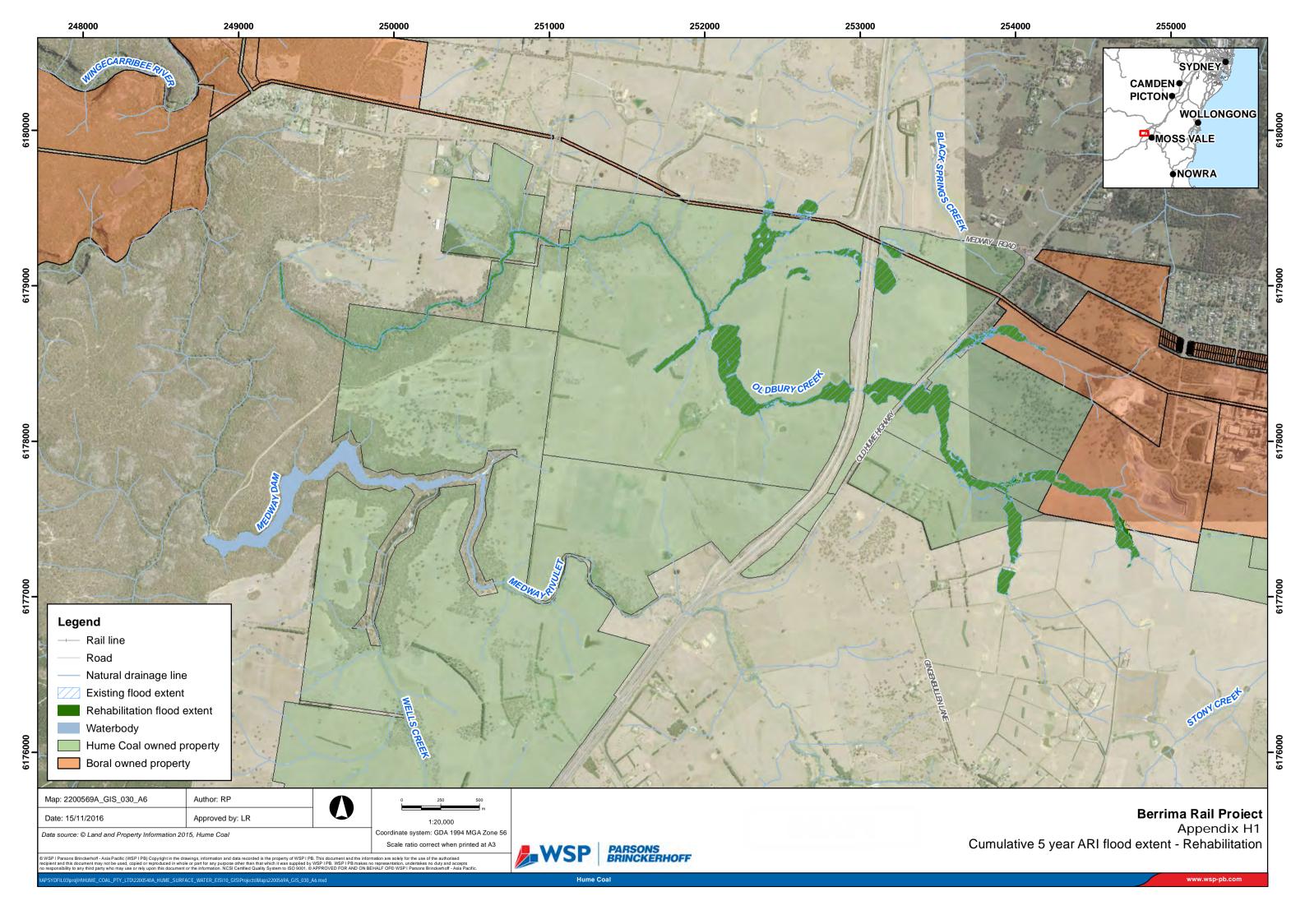


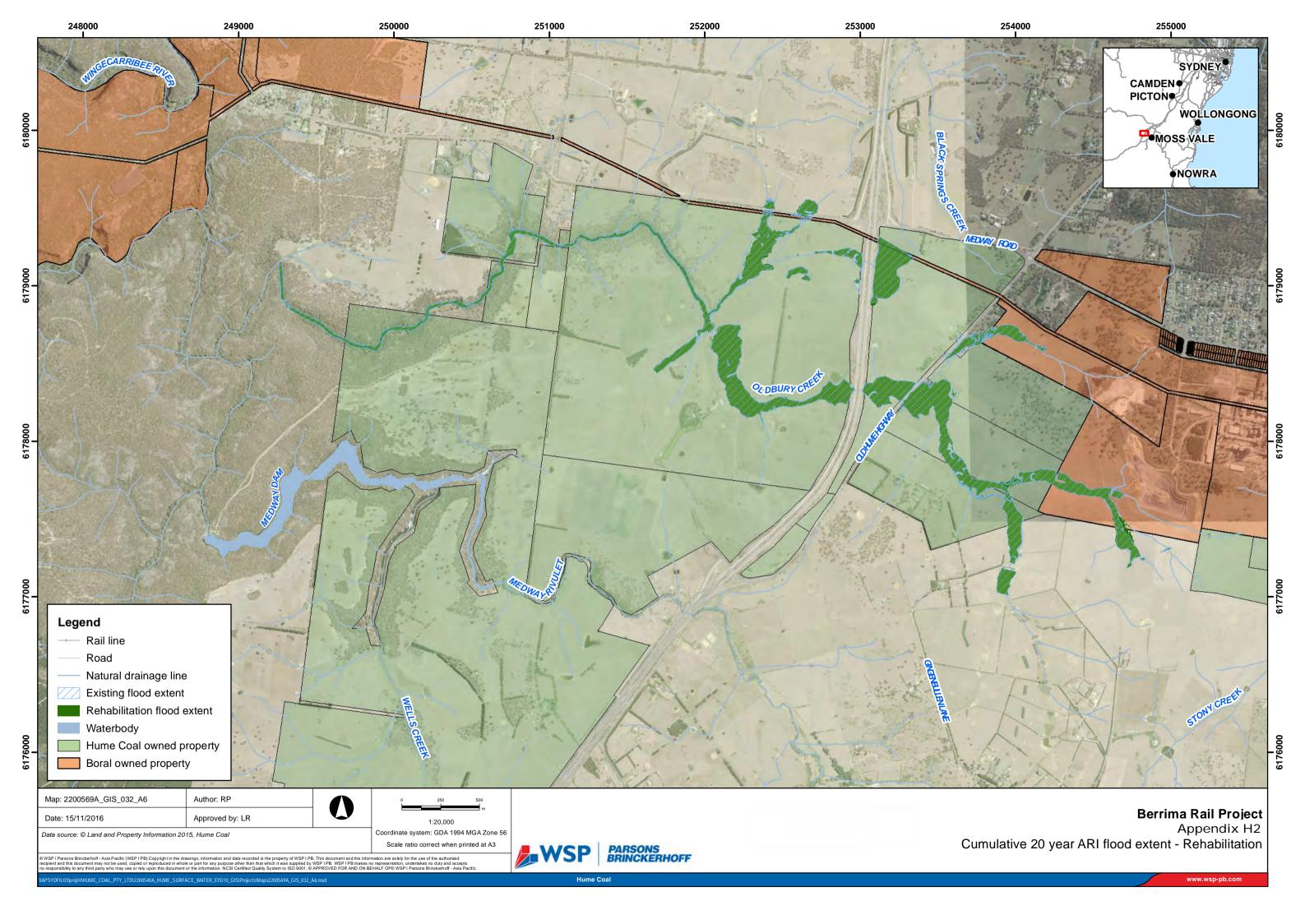


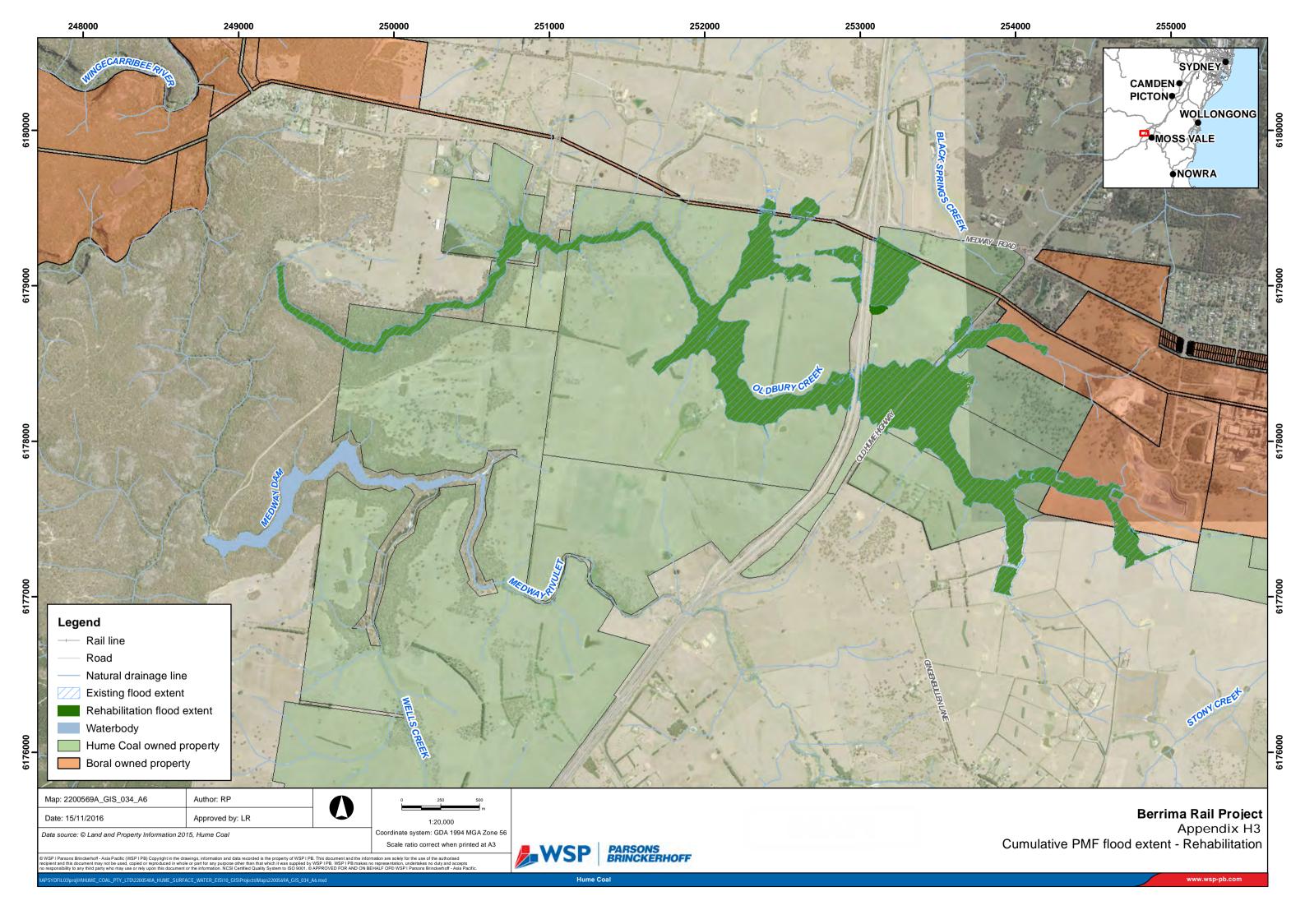


Appendix H

CUMULATIVE REHABILITATION FLOOD MAPS







Appendix I

ADJUSTMENT OF NUTRIENT BACKGROUND CONCENTRATIONS IN SWALES

A key input parameter in MUSIC treatment nodes is the background pollutant levels, C*(stormflow background pollutant concentration) and C** (baseflow background pollutant levels) values. The default MUSIC background concentrations for TP and TN in swales are set high and are required to be revised to simulate more realistic values.

The C* estimate for total suspended solids (TSS) is obtained by Fletcher (2004) from the particle size at which only 20% removal is achieved. The method and particle distribution figures are provided in Fletcher (2004).

Applying the Fletcher (2004) methodology for TP results in a C* value of 0.18mg/L and an EMC of 0.26mg/L. This is significantly higher than the TP EMC recorded in the Hume Coal project baseline water quality data at 0.14mg/L. Therefore, it was considered reasonable to adjust the C* and C** values to reflect the lower recorded TP concentrations in the watercourses in the project area.

The C* was adjusted by identifying the ratio between the C* and the EMC identified in Fletcher (2004) and the EMC from the project area in the equation below.

$$C_{new}^* = EMC_{from \ baseline \ data} * \frac{C_{fletcher}^*}{EMC_{fletcher}}$$

MUSIC applies the same value for both C^* and C^{**} for swales. Therefore, the C^*_{new} value was applied for both C^* and C^{**} .

The same method was applied for TN and the revised C* and C** for swales for both TP and TN are shown in the table below.

Table 1: Revised C* and C** values for swales

	k	C*	C**
TSS	8000	20	14
TP	6000	0.096^	0.096^
TN	500	0.89^	0.89^

^C* and C** values that were revised

By adjusting the C^* it is assumed that the background concentration of the swales will not increase because the swales will be properly maintained.

Appendix J

MUSIC MODELLING OUTPUT - CUMULATIVE FREQUENCY PLOTS

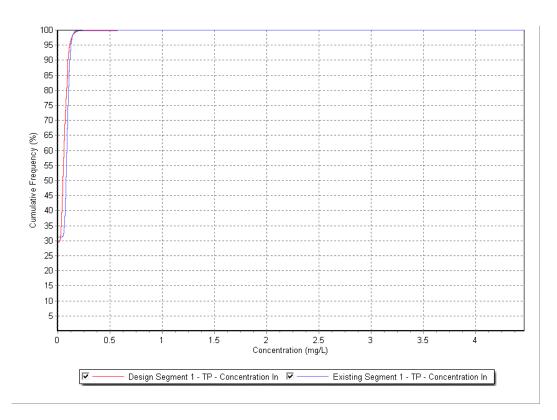


Figure J1: Cumulative Frequency Plots of TP for pre development (existing) and post development (operation) with treatment for Segment 1 of Oldbury Creek preferred and alternate options

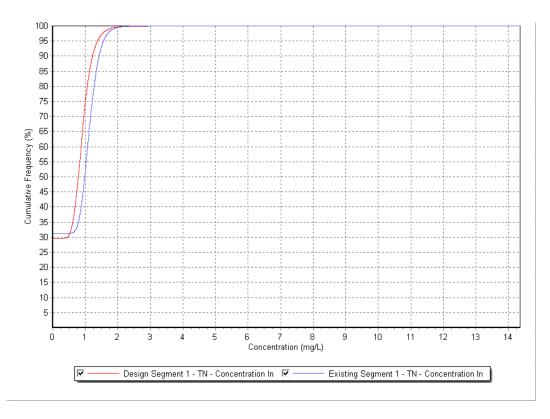


Figure J2: Cumulative Frequency Plots of TN for pre development (existing) and post development (operation) with treatment for Segment 1 of Oldbury Creek preferred and alternate options

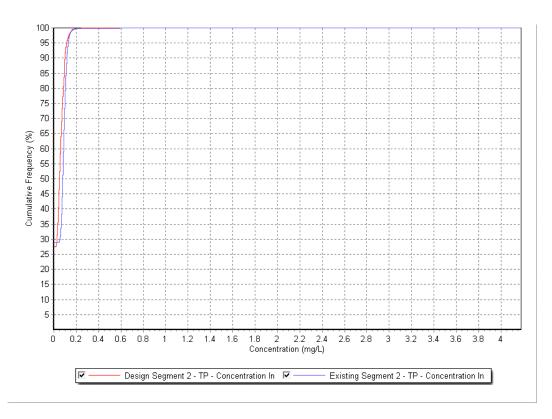


Figure J3: Cumulative Frequency Plots of TP for pre development (existing) and post development (operation) with treatment for Segment 2 of Oldbury Creek preferred and alternate options

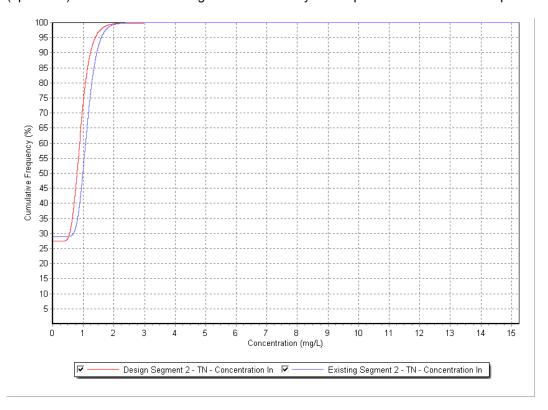


Figure J4: Cumulative Frequency Plots of TN for pre development (existing) and post development (operation) with treatment for Segment 2 of Oldbury Creek preferred and alternate options

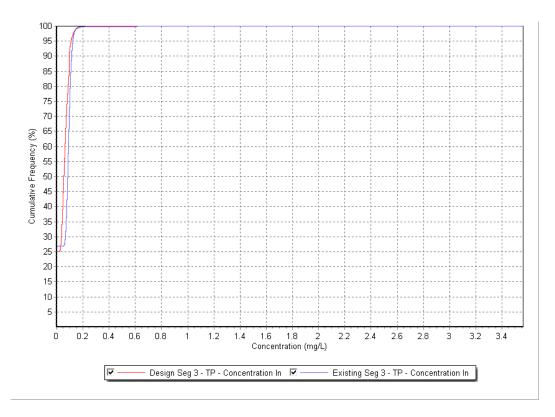


Figure J5: Cumulative Frequency Plots of TP for pre development (existing) and post development (operation) with treatment for Segment 3 of Oldbury Creek preferred and alternate options

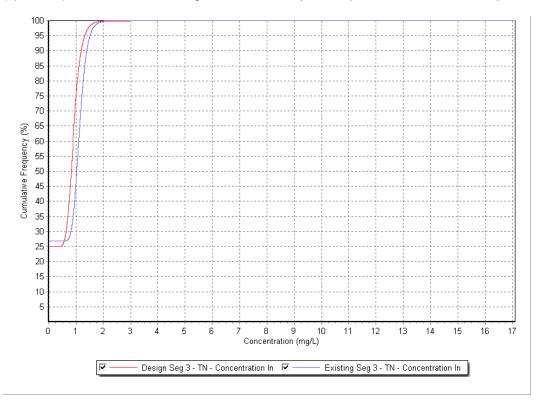


Figure J6: Cumulative Frequency Plots of TN for pre development (existing) and post development (operation) with treatment for Segment 3 of Oldbury Creek preferred and alternate options

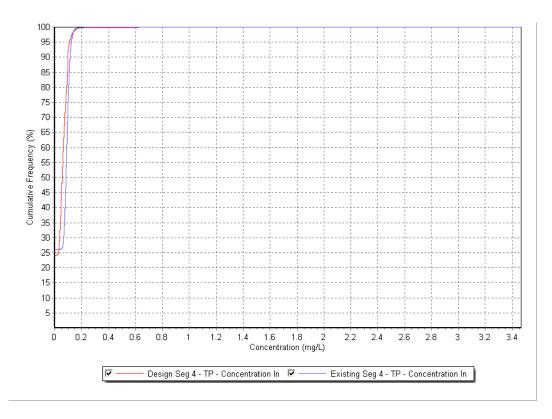


Figure J7: Cumulative Frequency Plots of TP for pre development (existing) and post development (operation) with treatment for Segment 4 of Oldbury Creek preferred and alternate options

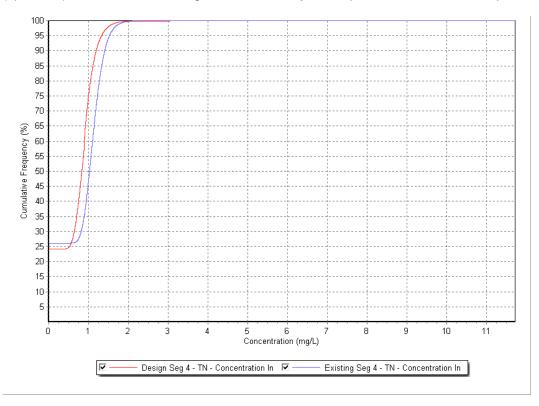


Figure J8: Cumulative Frequency Plots of TN for pre development (existing) and post development (operation) with treatment for Segment 4 of Oldbury Creek preferred and alternate options

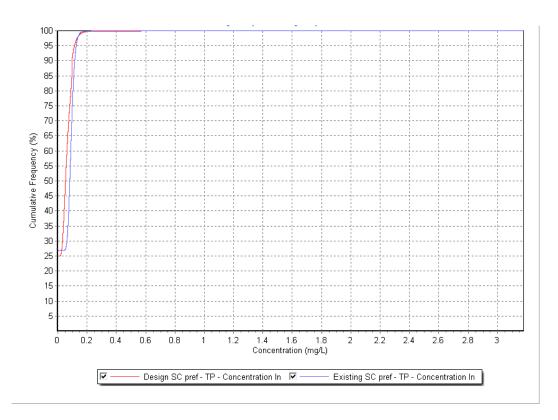


Figure J9: Cumulative Frequency Plots of TP for pre development (existing) and post development (operation) with treatment for Stony Creek preferred option

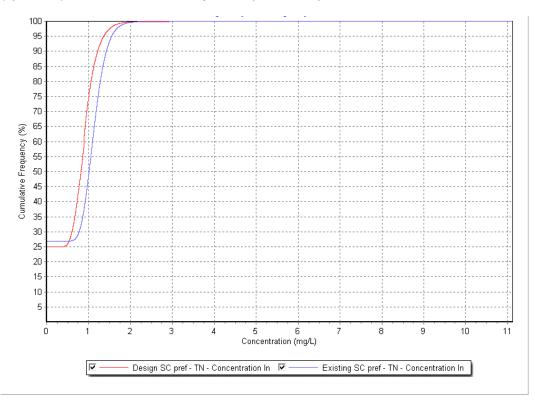


Figure J10: Cumulative Frequency Plots of TN for pre development (existing) and post development (operation) with treatment for Stony Creek preferred option

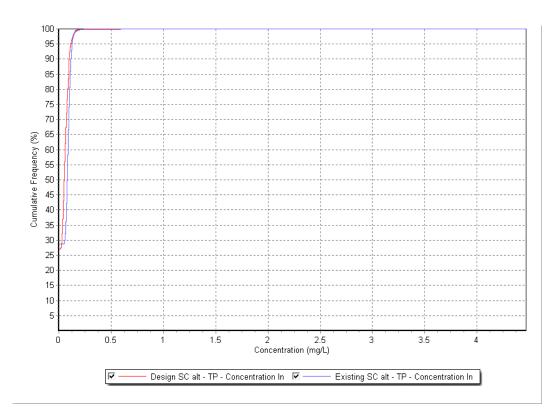


Figure J11: Cumulative Frequency Plots of TP for pre development (existing) and post development (operation) with treatment for Stony Creek alternate option

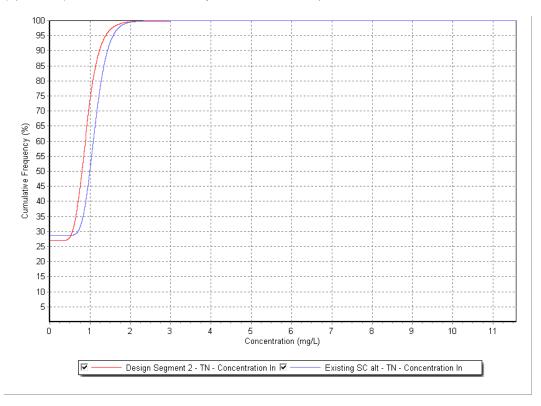


Figure J12: Cumulative Frequency Plots of TN for pre development (existing) and post development (operation) with treatment for Stony Creek alternate option





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