26 March 2021

ptc.

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Dear Matthew

1. Waterloo Metro Quarter Over Station Development – Response to Authority Comments – Northern Precinct (SSD – 10440)

ptc. have been engaged by WL Developer Trust (WLD) to prepare a response to the traffic and parking related comments raised by the authorities in relation to the proposed Waterloo Metro Quarter Over Station Development, Northern Precinct (SSD – 10440).

2. Authority Comments and ptc. Responses

2.1 City of Sydney

2.1.1 Item 60 (a)

Concerns remain as to the pedestrian priority and functionality of the new shared street and the surrounding intersections during peak hours (having regard to Section 3D of the Waterloo Metro Design and Amenity Guide), particularly morning peak is of concern. The area will experience high levels of people walking to and from the station in the morning and afternoon peaks. Vehicle parking on the site should be constrained further to reduce conflicts between people walking to and from the site and people driving through the shared zone.

2.1.2 ptc. Response to Item 60 (a)

The projected AM and PM peak hour trip generation associated with the WMQ basement car park is approximately 57 trips within each peak period. This represents a net reduction of 41 trips when compared to the Concept Approval, which projected 98 trips. This net decrease of approx. 40% in vehicular trips effectively translates to reduced conflicts between pedestrians and vehicles.

Consideration has also been given to the design of the Shared Zone to ensure that drivers are aware that pedestrians have clear priority and appropriate speed control devices can be provided to forcibly reduce vehicle speeds within the Shared Zone for improved pedestrian safety.

With reference to the TfNSW Technical Direction TTD 2016/001, 'Design and Implementation of Shared Zones including Provision for Parking';

Shared zones are specifically designed for pedestrian priority and may be appropriate for a road, a network of roads or road related areas where there are comparatively high pedestrian volumes, and;

A shared zone will only be considered where adequate footpaths cannot be retained within the road reserve and where there are very low numbers of slow moving vehicles.

The projected traffic generation of 57 vehicles is deemed a low traffic volume (approximately 1 per minute) and the shared zone will be fully designed at the detailed design stage and submitted to TfNSW for approval.





As part of the detailed design process, the shared zone design will be subject to an independent safety audit process to assess the safety aspects of the proposed layout. Also, a Traffic Management Plan in accordance with the *TfNSW' Procedures for Use in the Preparation of a Traffic Management Plan (TMP)'* will be prepared and submitted to TfNSW for approval of the shared zone design and suitability.

2.1.3 Item 60 (b)

It is recommended that level of service for walking follow Transport for NSW's guidance to ensure that sufficient space is provided to achieve comfortable environments which encourage people to walk as relevant to the NSW context - https://www.rms.nsw.gov.au/business-industry/partners-suppliers/document-types/guides-manuals/walking-space-guide.html

2.1.4 ptc. Response to Item 60 (b) (Response provided by WSP)

The Walking Space Guide recommends:

- A minimum of LoS C should be achieved.
 - Footpaths internal and adjacent to WMQ achieve LoS C or better for both LoS criteria typically adopted in high pedestrian environment (interchange and the more onerous street criteria)
- Raglan Lane and Grit Lane can be treated as a Type 3 or 4 footpath due to the proximity to the metro station (within 200m) and the peak hour users (70-2000 per hour). For these footpath types, a minimum footpath width of 3.0-3.7m is recommended for LoS C. The 'not adjacent' width has been adopted as the proposed design includes additional footpath space (in addition to the clear width) that may include street furniture or retail frontage.
 - As per the MQD PDA which were adopted as the minimum footpath requirements for the project. A minimum clear width (free of retail frontages or furniture) of 3.5m for key connections has been provided. It is noted that the footpath provision at these locations is significantly wider, though may include some retail frontage or furniture. Based on the current designs, the minimum requirements are satisfied.
- Raglan Place may represent a Type 5 (minimum of 3.9m) footpath (within 50m of the metro station).
 - A footpath width 5.5-6.5m is proposed in the design, and LoS C or better is achieved based on the peak pedestrians per hour, hence the minimum requirements are satisfied.
- Other internal connections are treated as Type 2 or 3 due to their proximity and comparatively lower patronage:
 - Cope Street Plaza shared zone of sufficient width is proposed
 - Church Square shared zone of sufficient width is proposed
 - Church Lane and Church Yard behave as Type 2 connections as both developments front onto Wellington Street as their main walkable connection, hence the proposed widths in combination with the adjacent walkable landscaped areas provide sufficient width and capacity.

2.1.5 Item 61 (a)

The first objective of Section 3N of the Waterloo Metro Design and Amenity Guideline is to "prioritise walking and cycling trips in and around the Metro Quarter over vehicles". The vehicle parking proposed for residential and commercial use is excessive for a transit-oriented development and should be minimised to reflect and support the public transport access of the site and the significant investment in public transport.

2.1.6 Item 61 (b)

The amount of parking directly impacts the overall objective of the new metro line which aims to shift people from car driving to using the train and, in line with the desired outcomes under the Waterloo Metro Design and Amenity Guideline "create an urban environment that drives high usage of the Sydney Metro network responding directly to the principle of transit oriented development". The mode share targets to shift private car users to public and active transport uses will never be achieved without making the parking supply competitive. Availability of car parking spaces at origin and destination points is considered the most difficult obstacle to shifting people to use more sustainable transport methods.

2.1.7 Item 61 (c)

DPIE are strongly advised to insist the proponent work together with the development partners, TfNSW, RMS and strive for 'zero' car parking provision or absolute minimums. This way the development can be expected to generate much fewer new car trips and will not adversely affect the existing adjacent road network, which is already congested.

- This site should aim to be a world class transit-oriented development.
- Providing car parking on the site contradicts the transport and sustainability objectives and the investment in public transport. The development aims to shift people from private vehicles into public transport.
- 65 spaces were outlined in the Explanation of Intended Effect accompanying the proposed SEPP (State Significant Precincts) amendment. The proposal is for more than double this.

2.1.8 ptc. Response to Item 61(a) to (c)

The parking provisions within the development have been determined for each separate land-use in the context of the TOD character of the development. While some car parking is proposed rather than a zero provision as suggested by DPIE, the number of spaces has been determined to balance the demand for parking within the development and the surrounding on-street parking. This goes to car ownership within the residential component and the need to accommodate for all travel options in relation to the office component (i.e. the site as a destination is well connected but the origin may not be).

In reviewing the parking provision, it is important to assess each component as follows:

1. The residential component will accommodate 150 dwellings, which under the LEP would be permitted a maximum of 80 parking spaces. The capping of the parking provision is inline with the points raised by DPIE with regard to limiting car usage and encouraging the use public transport etc. The LEP recognises the need to limit parking (which typically averages 1 space per apartment in most NSW Government areas (when the mix of apartment sizes is considered) and in this case the maximum would represent 1 space per 1.8 apartments (or 0.55 cars per dwelling). This compares with an average car ownership of 0.8 cars per dwelling in Waterloo. In this regard, the maximum provision permitted under the LEP represents a 33% reduction in current car ownership in the area. The proposal further reduces the provision by providing 67 spaces for residents or 0.45 spaces per dwelling, which represents 84% of the maximum provision, and almost half the current average for the area. This represents a significant limitation of car ownership within the development.

Dwellings	Waterloo Car Ownership (Demand)	LEP Max.	Proposed
150	120 spaces	80 spaces 67% of demand	67 spaces 56% of demand

It is noted that car ownership and car usage are separate metrics in the context of residential development in that the restriction of parking at the destination is often the key to reducing car usage, while car ownership is more driven by convenience for less regular trips outside the typical weekday commute.



In terms of consistency with other developments, we note that sites within Green Square Town Centre (completed or approved) provide parking for residents ranging from 65 – 100% of the LEP maximum provision. This area is comparable being located immediately adjacent to Green Square station.

- The office component will accommodate 38,800m², which based on an average occupancy rate of 1 person 1 per 10m² (active office space) indicates a population of 3,200 persons. Given that the development represents a TOD, this has been taken into consideration when developing the parking provision. The LEP provides a maximum cap on the parking provision in recognition of the level of public transport accessibility throughout the CoS LGA but recognises that some parking demand occurs. The maximum permitted parking provision is 78 spaces under the LEP, and this is clearly reduced compared to commercial developments in unconstrained areas (1 space per 40m² would result in 970 parking spaces, which is not compatible with the inner-city areas, but highlights the degree to which that LEP restrains parking to less than 10% of the unrestrained demand. The parking provision further restricts parking within the development to 63 spaces (80% of the LEP maximum). The provision of 63 spaces in the context of 3,200 people within the buildings will not represent a notable shift in mode share towards car usage (if all spaces were used, this would represent less than 2% of occupants driving to the development). While the Metro will provide a high level of accessibility, combined with the high-density residential areas in relative close proximity of the site (Green Square, Redfern, Surry Hills, Newtown etc.). It is unrealistic that the commercial component of the development will not generate some parking demand. The development proposal dramatically restricts the number of trips that can be made to/from the development. In terms of traffic generation, the trips generated by the development have been assessed and found to have minimal impact on the surrounding road network, particularly compared to the approved scheme and the former uses of the site.
- 3. The student accommodation component has been designed with no parking provision, which is consistent with numerous similar developments in the area (Newtown, Chippendale, Broadway etc.) and reflects the low car ownership within the student demographic.
- 4. The retail and gym components of the development are not provided with any parking as these are considered to be ancillary to the overall development and the Metro station. In this regard they are not destination uses requiring the use of a car.

2.1.9 Item 61 (d)

If parking is to be provided, accessible car parking space provision should be prioritised and provided for as per SDCP. All accessible car spaces are to be allocated to adaptable units.

2.1.10ptc. Response to Item 61 (d)

The accessible car parking provision is allocated to adaptable units and based on the rationale provided by Morris Godding Access Consultants. Refer to the DDA Assessment in Appendix S of the SSD DA EIS for details.

2.1.11 Item 61 (e)

Parking for loading and servicing should be prioritised over general vehicle parking.

• Given the rate of vehicle parking provided the site should provide for the required amount of loading and servicing.

2.1.12ptc. Response to Item 61 (e)

Refer to response for Item 64(a).

2.1.13 Item 62 (a)

It is unclear from the submitted documentation if the traffic modelling includes the cumulative traffic generation from adjacent developments plus the projected traffic generation for the subject proposal.

2.1.14 ptc. Response to Item 62 (a)

The traffic modelling does not currently include defined traffic generation from adjacent developments as this information it not currently finalised or available. However, to ensure that the network is being tested to the extent that new developments are expected, the traffic modelling includes a background traffic growth up to the design year. State arterial roads such as Botany Road carry regional traffic from a vast coverage of areas within Greater Sydney and it is general industry practice to apply an overall background traffic growth rate to the existing surveyed traffic data. This is to account for the increase in traffic activity associated with developments within the Greater Sydney area, as it is not possible to account for individual developments.

2.1.15ltem 62 (b)

The zero trip generation rates for student housing are unrealistic.

2.1.16ptc. Response to Item 62 (b)

As outlined in Table 15 in WMQ-SITE-PTC-TF-RPT-001, there is no car parking provision proposed for the student accommodation component of the development, which is consistent with the numerous similar student accommodation developments in the area, which have no parking for residents. (e.g.; Iglu Broadway, Urbanest Darlington etc).

Furthermore, the proximity to high-frequency public transport services means residents and visitors to the student accommodation have numerous transport options when travelling to and from the site.

While there may also be some activity associated with taxi and carshare services, these vehicles are including within the background movements on the road network and generally occur as activity throughout the day, rather than concentrated within the morning or evening peak periods.

Based on the evidence of similar developments/buildings in the area (particularly those located where onstreet parking is severely restricted (e.g. Broadway) it is expected that the majority of student residents will not own motor vehicles and will utilise public transport. In the context of the WMQ site, the parking restrictions in the area would effectively discourage car ownership and the related trip generation.

In light of the above, the zero-trip generation is reflective of this car parking provision and the proposal has sought to minimise car parking, consistent with the objectives of the CoS to reduce car dependency.

2.1.17Item 62 (c)

The traffic modelling should include changes to the street network and intersections proposed as part of the Metro development.

2.1.18 ptc. Response to Item 62 (c)

The future road network improvements associated with the Metro development have been included in the traffic modelling and the findings can be found in Attachment 1.

2.1.19 Item 63 (b)

Bike parking for the student accommodation should be provided as per residential studio apartment rates (i.e. 1 per studio apartment) in accordance with design criteria 3 Section 3N of the Waterloo Metro Design and Amenity Guideline.

2.1.20 ptc. Response to Item 63 (b)

The proposed student accommodation is more akin to a boarding house configuration rather than a typical studio apartment. Therefore, the bike parking for the student accommodation has been calculated based on the requirements stipulated within the AHSEPP 2009 using rate of 1 space per 5 boarding rooms. Real-world bicycle parking occupancy surveys from other Iglu sites within the CoS were provided by Iglu to WLD.



The surveys found that the maximum utilisation of the provided bike spaces as a percentage of total rooms in the site was approx. 5%. This is significantly lower than the proposed bike parking provision which caters for 20% of rooms based on AHSEPP rates. As such, the proposed provision of 87 bike spaces for student accommodation is considered generous, appropriate and consistent with similar approved developments within the City of Sydney.

2.1.21 Item 64 (a)

The proposal presents a shortfall of loading and servicing and should be provided as per the SDCP 2012 rates.

2.1.22 ptc. Response to Item 64 (a)

The purpose of the proposed loading docks is to serve the servicing and maintenance needs of the WMQ site as a whole. Taking this into consideration, the approach for determining the service vehicle parking provision takes into account the ability of the service bays to accommodate more than one vehicle per day in each dock and the courier bays located within the basement. The efficiencies of grouping land uses are maximised when the service bays are shared amongst the various uses within the development.

The calculations for the loading dock provision for the combined uses within the Northern and Southern Precincts are as follows:

User Type	Units / GFA / Spaces	LEP/DCP Parking Rate	Minimum Required Spaces	Proposed Parking Spaces
Northern Precinct - Commercial	34,734m²	1 space per 3,300m ² (DCP min)	11	4
Southern Precinct – Social Housing	70 units	1 space for 1st 50 units & 0.5 spaces per 50 units + (DCP min)	2	2
Shared B99 Courier Bays within Basement Car Park	-	-	-	5
Total Required S	Service Bays (Minimu	13	11	

The proposed loading docks and service bays within the basement car park will be managed by means of an integrated site-wide booking system. This will allow each bay to be pre-booked prior to arrival to ensure that there are available bays for any delivery or service vehicles. A concept timetable has been prepared as part of the FSMP to demonstrate that there are a large number of time slots available which allow the bays to be shared across the site amongst the different components of the development. In this regard, the proposed loading/servicing provision is considered acceptable and able to be managed for the coordination of deliveries and servicing.

It should also be noted that the Southern Loading Dock has been amended to provide an additional B99 courier service vehicle bay. The revised concept loading dock layout has been reviewed and a swept path assessment has been undertaken. A B99 vehicle is able to enter the site in a forward direction upon entry



and egress from the loading dock in a forward direction is also achieved. It is noted that the egress of the B99 courier/service vehicle will require the turntable to be vacant. This can be readily managed as deliveries and maintenance vehicles are managed through the site-wide online booking system.

Overall, ptc.'s review finds that the concept design for the Southern Loading Dock generally meets or is capable of complying with AS2890.1:2004 and AS2890.2:2018. Details of the swept path assessment is provided in Attachment 2.

2.1.23 Item 64 (b)

All loading and servicing should occur onsite and the development should not be potentially reliant on kerbside loading arrangements which are open to other users and subject to change.

2.1.24 ptc. Response to Item 64 (b)

Loading and servicing will occur within the designated loading docks on-site or the service vehicle bays within the basement car park. The proposed development does not rely on kerbside Loading Zones.

2.1.25 Item 64 (c)

Parking for loading and servicing should be prioritised over general vehicle parking.

2.1.26 ptc. Response to Item 64 (c)

The Northern and Southern loading docks are provided with access and egress driveways separate from the basement parking area and therefore have do not interact with the general parking activity.

The Northern loading dock includes provision for two MRV bays and two SRV bays whereas the Southern loading dock comprises one MRV bay and one B99 courier bay. The SRV bays will accommodate small delivery trucks and resident removalist trucks up to 6.4m in length. The MRV bays will accommodate the 9.25m CoS waste collection vehicle as well as delivery trucks up to 9.25m in length.

The remaining service bays located in the basement accommodates B99 car-derived vans and utes for the purposes of maintenance contractors (such as plumbers, electricians etc.). These service bays will be line marked and signed accordingly and will be solely for the use for service vehicles.

2.1.27 Item 64 (d)

The design of the loading areas to accommodate a City of Sydney 9.25m waste collection vehicle is supported. This needs to be ensured and should be conditioned.

2.1.28 ptc. Response to Item 64 (d)

ptc. confirms that the loading docks have been designed to accommodate entry and egress of a 9.25m Council waste vehicle.

2.1.29 Item 72

The City has developed a waste calculator to ensure development provides sufficient waste storage facilities in accordance with the City's Guidelines for Waste Management in New Developments. Please find attached a PDF with the estimated area required for waste bins. The City requests that the developer use the waste calculator and demonstrate that sufficient area has been provided to meet the needs of each use proposed on site. Please note that the City discourages more than 3 collections per week to minimise traffic movements.

2.1.30ptc. Response to Item 72

Due to the large scale of the development, waste collection will exceed the three collections per week recommended by the CoS. WLD intend to undertake five general waste pickups per week for the commercial uses. In terms of impact on the road network, the provision of a waste collection service on any



particular day would not register in any form of modelling or analysis. While the aim of 3 services per week for developments is noted and beneficial as an average across many types/sizes of development, it does not necessarily address the needs of large developments that by virtue of the populations involved required more regular waste collection.

It is highlighted that waste collection for the commercial, retail, childcare, student accommodation and retail (gym) uses will be undertaken by a private waste contractor. As such, waste collection can be scheduled and coordinated to ensure that they occur outside of peak periods to minimise any potential impact on the surrounding road network.

Where waste collection is to be undertaken by Council (i.e. for the market residential/affordable housing, and social housing uses), the estimated pickups required are approximately four pick-ups for market residential/affordable housing and three pick-ups for social housing. It is noted that these pickup frequencies include both general waste and recycling collection.

2.1.31 Item 73

The turntable is to be a minimum dimension of 10.5 metres in accordance with the City's Guidelines for Waste Management in New Developments and Section 3P of the Waterloo Metro Design and Amenity Guidelines.

2.1.32 ptc. Response to Item 73

Section B19 of the 'Policy for Waste Minimisation in New Developments' states the following:

'Minimum radius turning circle required 10.5 metres'.

The requirement relates to the turning radius of the waste vehicle accessing the service area and does not relate to the diameter of the turntable.

The Northern and Southern Loading Docks have been designed to include a 9.25m diameter turntable based on a swept path assessment for the 9.25m Council refuse vehicle. The swept paths demonstrate that there is a minimum of 300mm clearance around the body of the vehicle to any walls or vertical obstructions when the turntable is in operation. The provided turntable clearance satisfies the requirements of AS2890.2.

2.2 Transport for NSW

The following subsections outline the traffic and transport related comments raised by TfNSW and **ptc.**'s response.

2.2.1 Active Transport

Comment

It is noted that Section 8.1.5 of the Transport, Traffic and Parking Impact Assessment (Traffic Report) prepared to support the subject development application states the following:

"As one of the joint-venture partners of the development, Mirvac owns a large portfolio of commercial offices and confirms that visitor bicycle provisions are often significantly underutilised, whereas the demand for EoTF by building occupants is strong.

"In light of the overall provision of 390 visitor bicycle parking spaces throughout the precinct as part of the ISD, the bicycle provision is considered more than adequate to cater for the expected cycling demand."

Recommendation

The applicant should locate bicycle facilities in secure, convenient, accessible areas close to the main entries, incorporating adequate lighting and passive surveillance and in accordance with Austroads guidelines; and

The applicant should undertake annual staff travel surveys for the entire Waterloo Metro Quarter (WMQ) and allocate bicycle parking and End of Trip (EoT) facilities for residents, staff and bicycle couriers based on the results of the travel surveys during the operation of the development.

2.2.2 ptc. Response to Active Transport

Bicycle parking and EoTF have been located in secure, convenient and accessible locations in accordance with Austroads and Australian Standards. Details of the location of the proposed bicycle facilities are shown on the concept architectural drawings prepared by Woods Bagot and Bates Smart and the public domain landscape drawings prepared by Aspect Studios. A summary of the proposed bicycle parking and EOTF provisions is outlined in Table 1.

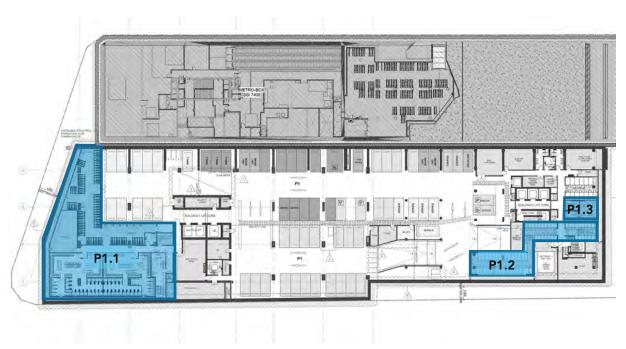


Figure 1 - Basement Floor Plan Level P1, Rev E (Source: Woods Bagot)

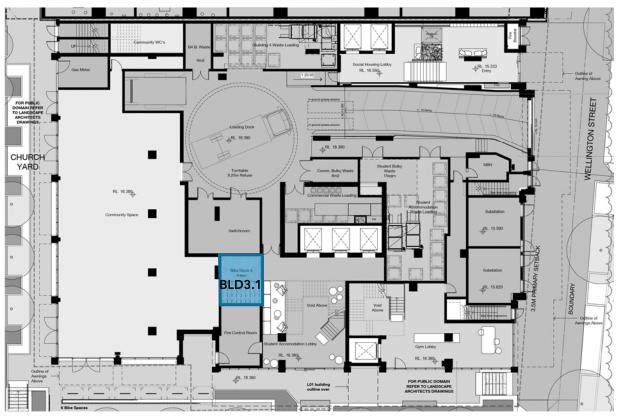


Figure 2 – Building 3 General Arrangement Plan – Ground Floor, Rev L (Source: Bates Smart)



Figure 3 – Building 3 General Arrangement Plan – Level 1, Rev K (Source: Bates Smart)

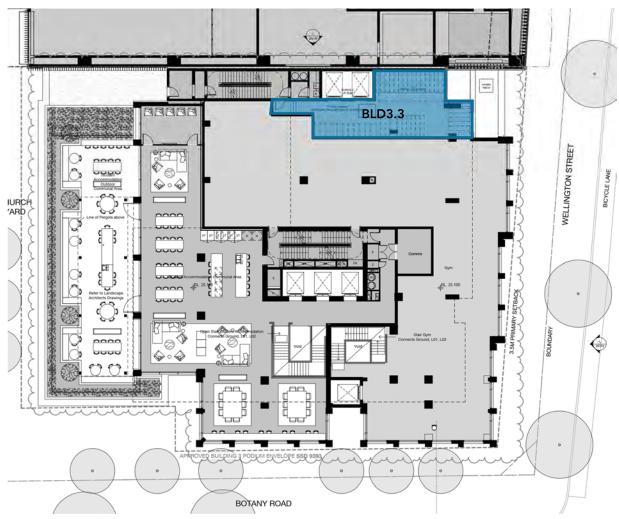


Figure 4 - Building 3 General Arrangement Plan – Level 2, Rev K (Source: Bates Smart)

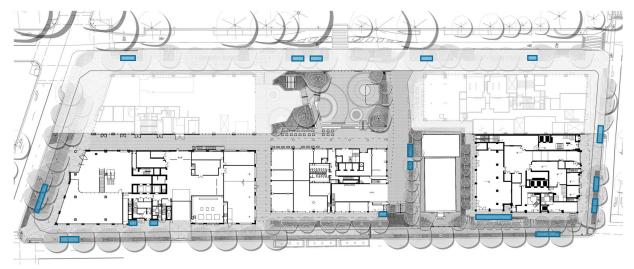


Figure 5 - Public Domain Masterplan - Ground Floor, Rev 4 (Source: Aspect Studios)

Table 1 - Bicycle Parking and EOTF Provision Summary

Location	Area Code	No. of Bicycle Spaces	EOTF Provision
	P1.1	236	30 showers
Basement Level P1			284 lockers
Basement Level P1	P1.2	65	-
	P1.3	14	-
Southern Precinct	BLD3.1	16	-
Building 3	BLD3.2	71	-
	BLD3.3	70	-
Ground Floor	Overall	140	-
Public Domain			
	TOTAL	612	30 showers 284 lockers

A Green Travel Plan (GTP) was submitted as part of the SSDA application will be revised during the post-approval stages in consultation with TfNSW, prior to implementation of the GTP and the issue of the Occupation Certificate. As part of the post development GTP process, annual staff travel surveys will be undertaken and based on these results bicycle parking allocation will be reviewed.

2.2.3 Safety Assessment of the Proposed Development

Comment

The proposed development is located in an environment with high level pedestrian activities associated with the Sydney Metro operation. There is potential conflicts with between vehicle accessing the over station developments including freight and servicing vehicles and pedestrians accessing the Sydney Metro station.

It is noted that Section 3.1 of the Traffic Report states that independent road safety audits have not been undertaken for the concept design scheme and will be undertaken (by a suitably qualified consultant) in the detailed design stage prior to the issue of Construction Certification.

It is advised that the earlier a project is audited the more likely that the road safety issues or risks identified can be significantly reduced or eliminated. As a result this minimises compromises in road safety and costly treatments at later stages of the project.

Recommendation

It is requested that the applicant undertakes a Stage 2 (Concept Plan) Road Safety Audit as part of the applicant's Response to Submissions for the proposed access arrangements to the loading docks in accordance with Austroads Guide to Road Safety Part 6: Managing Road Safety Audits and Austroads Guide to Road Safety Part 6A: Implementing Road Safety Audits by an independent TfNSW accredited road safety auditor.

Based on the results of the road safety audit, the applicant shall review the design drawings and implement safety measures in consultation with TfNSW as required.



2.2.4 ptc. Response to Safety Assessment of the Proposed Development

A Stage 2 (concept plan) Road Safety Audit (RSA) of the Waterloo Over Station Development has been undertaken by DC Traffic Engineering (an independent TfNSW accredited road safety auditor). The recommendations outlined within the RSA is currently being reviewed and the concept design will be updated as appropriate.

It is also highlighted that access to the site by freight and servicing vehicles will be managed and coordinated through the implementation of a Freight and Servicing Management Plan. The proposed loading docks and service bays within the basement car park will be managed by means of an integrated site-wide booking system. This will allow each bay to be pre-booked prior to arrival to ensure that vehicles conduct deliveries and maintenance occur outside of peak periods where possible to minimise potential conflicts between pedestrians and service vehicles.



2.2.5 Green Travel Plan

Comment

It is noted that a Green Travel Plan has been prepared as part of the development application. It is advised that the implementation of a Green Travel Plan could be an effective measure to reduce travel demand generated by private vehicle movements and reallocate, through encouragement and various other methods programs, these trips towards other modes of transportation.

Recommendation

It is requested that:

- The applicant be conditioned to update the Green Travel Plan in consultation with TfNSW, prior to the issue of the Occupation Certificate; and
- The Green Travel Plan must be implemented accordingly and updated annually to ensure sustainable transport outcomes and achieve the overall strategic planning objectives in the Future Transport 2056.

2.2.6 ptc. Response to Green Travel Plan

The Green Travel Plan (GTP) submitted as part of the SSDA application will be revised during the post-approval stages in consultation with TfNSW, prior to implementation of the GTP and the issue of the Occupation Certificate.

During implementation of the GTP upon completion of the development, annual updates to the GTP will be conducted to ensure that the objectives set out in the GTP remain relevant and continue to achieve the strategic planning objectives in the Future Transport 2056 guidelines.

2.2.7 Transport Access Guide

Comment

It is noted that a Travel Access Guide has been prepared as part of the development application. It is advised that a Transport Access Guide would inform residents, employees and visitors the travel choices available to them.

Recommendation

It is requested that the applicant be conditioned to update the Transport Access Guide, in consultation with TfNSW, prior to the issue of the Occupation Certificate.

2.2.8 ptc. Response to Transport Access Guide

The Transport Access Guide submitted as part of the Green Travel Plans for the SSDA will be updated in consultation with TfNSW, prior to the issue of the Occupation Certificate.

2.2.9 Construction Pedestrian and Traffic Management

Comment

Several construction projects, including the Sydney Metro City and Southwest Project, are likely to occur at the same time as this development. The cumulative increase in construction vehicle movements from these projects could have the potential to impact on general traffic and public transport operations within the Waterloo Precinct, as well as the safety of pedestrians and cyclists particularly during commuter peak periods.

Recommendation

It is requested that the applicant be conditioned to prepare a Construction Pedestrian and Traffic Management Plan (CPTMP) in consultation with TfNSW and submit a copy of the final CPTMP to



sco@transport.nsw.gov.au for endorsement, prior to the issue of any construction certificate or any preparatory, demolition or excavation works, whichever is the earlier.

2.2.10 ptc. Response to Construction Pedestrian and Traffic Management

The Construction Pedestrian and Traffic Management Plan submitted as part of the SSDA application will be updated in consultation with TfNSW and submitted to the Sydney Coordination Office for endorsement, prior to the issue of the Construction Certification or any preparatory, demolition or excavation works.

2.2.11 Freight and Servicing Management

Comment

It is noted that a Freight and Servicing Management Plan has been prepared as part of the development application.

It is noted that:

- Northern loading dock, Southern loading dock and Basement service bays are proposed to provide the freight and servicing requirements for the proposed OSD sites within the WMQ;
- Section 4.7.6 of the EIS prepared to support the subject development application states that five dedicated service vehicle bays are provided separately within the basement to facilitate general servicing; and
- Section 2 of the Traffic Report states that Northern loading dock comprises two SRV bays and two MRV bays and the Southern loading dock accommodates one MRV bay. In addition, five courier bays accommodating B99 car-derived vans/utes are provided within the Basement car park. The shared use of the 10 proposed service bays will be managed through the implementation of a Freight and Servicing Management Plan.

It is advised that:

- The Freight and Servicing Management Plan for the subject development needs to reflect that the total ten (10) spaces are being managed centrally by a single booking management system for the whole of Waterloo Metro Quarter (WMQ). This would minimise queuing of freight vehicles to access the loadings and the impact on the general traffic and public transport operation;
- There are inconsistencies in the documentation for the development applications in relation to the management of service bays for the whole of Waterloo Metro Quarter (WMQ) in particular in the documents prepared for the Southern Precinct SSDA; and
- All new developments should not rely on on-street parking or loading zones. Kerb side restrictions can be changed at any time and the development should not rely on current kerb side restrictions to service the site.

Recommendation

It is requested that:

- The applicant provides further details in relation to the management of service bays for the whole of WMQ as part of the response to submissions; and
- The applicant be conditioned to update the Freight and Servicing Management Plan in consultation with TfNSW, prior to the issue of any Construction Certificate.

2.2.12 ptc. Response to Freight and Servicing Management

The loading and servicing associated with the Northern, Central and Southern Precincts are wholly undertaken within the site in the dedicated loading docks and within the courier bays situated within the basement and do not rely on on-street parking controls. The use of the loading docks and courier bays will



be coordinated and managed through the implementation of a site-wide Freight and Servicing Management Plan (FSMP). ptc. confirm that WL Developer Trust will provide a site wide FSMP, prior to the issue of the construction certificate.

2.2.13 Vehicle Crossover on Botany Road

Comment

It is noted that Section 4.7.1 of the EIS indicates that a vehicle crossover will be established on Botany Road for the creation of a loading dock.

Recommendation

It is advised that concurrence under Section 138 of the Roads Act 1993 needs to be obtained from TfNSW for any vehicle crossover and associated works on Botany Road and the design plans are be provided to TfNSW via development.sydney@transport.nsw.gov.au.

2.2.14 ptc. Response to Vehicle Crossover on Botany Road

As per Section 138 of the Roads Act 1993, concurrence for the provision of the proposed vehicle crossover and associated works on Botany Road will be obtained from TfNSW through the design development stage. The design plans will also be submitted to TfNSW for approval prior to Construction Certification.



2.3 NSW Department of Planning, Industry & Environment

2.3.1 Item 7(b)

Provide further justification that the proposed childcare centre use is suitable for the tenancy and consent can be granted for the use, without operational management and fit-out details. This must include further consideration and demonstration of:

• Children and staff numbers with respect to car parking and access.

2.3.2 ptc. Response to Item 7(b)

The car parking provision for the childcare centre balances the parking requirements outlined within Section 7.8.3 of the Sydney DCP as well as the likely demographics of the end users. Being a development which provides a place for people to live, work and play, it is expected that the child care centre will primarily serve the residents and employees within the WMQ development. Therefore, trips associated with the childcare centre would be undertaken as part of a combined trip, utilising the parking already provided within the development or the numerous public transport modes available.

It is highlighted that there are similar approved developments within the CoS such as the Barangaroo Early Learning Centre (BELC)located at 62 Sussex Street, Barangaroo South (DA Ref: D/2016/1012) and 505 George Street, Sydney (DA Ref: D/2019/857) which does not provide off-street car parking for staff or visitors. The BELC approval states that there is some underlying traffic generation associated with the BELC which has been absorbed into the parking generation of the entire building. Furthermore, the BELC utilises existing kerbside parking for short term parking to support the operation of the childcare centre.

Similarly, the WMQ development is a mixed-use, transit-oriented development (TOD) which provides a large number of public transport services including buses, trains and metro within close proximity, which reduces the demand for driving to and from the site. The City of Sydney also has plans to provide 10 priority routes across the inner city including the Waterloo Precinct as part of its cycle network strategy to facilitate cycling. As such, the proposed zero parking provision for pick-up/drop-off is consistent with other approved developments within the CoS.

2.3.3 Item 7(e)

Provide further information on how the proposed retail premises can be serviced from basement loading docks.

2.3.4 ptc. Response to Item 7(e)

Access from the Central Precinct retail tenancies to the Northern loading dock is provided in the following diagrams.

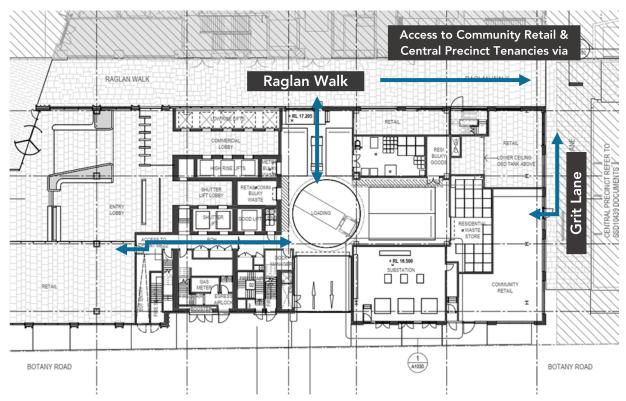


Figure 6 - Access to Retail Tenancies from Northern Loading Dock

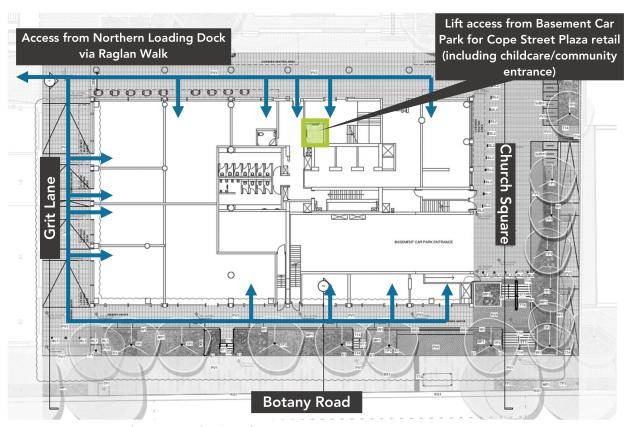


Figure 7 – Access to Retail Tenancies within Central Precinct



3. Conclusion

We trust this additional information satisfies the issues raised. However, if you require any additional details, please do not hesitate to contact me.

Regards,

Steve Wellman

Project Director / Senior Traffic Engineer

Document Control: Prepared by *HL* on *26 March 2021*. Reviewed by *SW* on *26 March 2021*.



Attachment 1- Traffic Modelling Statement

10 February 2021

ptc.

Anthony Witherdin
Department of Planning, Industry and Environment
4 Paramatta Square 12 Darcy Street
Parramatta
NSW 2150

Dear Anthony

Waterloo Metro Quarter Overstation Development – Response to Authority Comments

ptc. has previously been engaged to undertake traffic modelling for the Waterloo Metro Quarter development. The following comment has been received from City of Sydney in relation to the traffic model prepared by **ptc.**:

"The traffic modelling should include changes to the street network and intersections proposed as part of the Metro development."

ptc. has been advised that there are currently two intersection upgrades in the planning stage. The following are the intersections currently in the discussion stage:

- Cope Street and Raglan Street Intersection
- Cope Street and Wellington Street Intersection

Therefore, the proposed future layout has been integrated into the traffic model to assess the performance of the local road network in 2036 and the consequent potential impact of the proposed development. This letter is to be read in conjunction with the Transport, Traffic and Parking Impact Assessments prepared by **ptc.**

1.1 Planned Upgrades

The planned works for the two intersections are as follows:

1.1.1 Cope Street / Raglan Street

The intersection is currently configured as a roundabout. The intersection is planned to be converted to a signalised intersection. The arrangement of the intersection is illustrated in Figure 1.

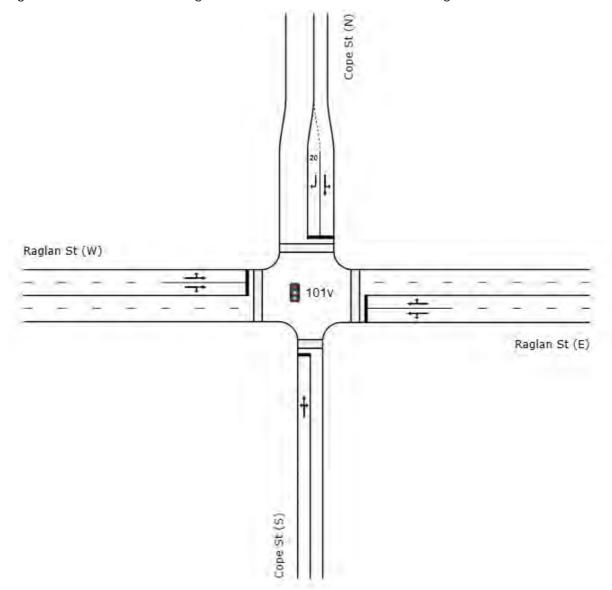


Figure 1 - Cope Street and Raglan Street Intersection Upgrade

1.1.2 Cope Street / Wellington Street

The intersection is currently configured as a roundabout. The intersection is planned to be converted to a priority intersection. The arrangement of the intersection is illustrated in Figure 2.

It is noted that the future layout provides pedestrian crossings along the northern leg of Cope Street and eastern leg of Wellington Street. The model takes a conservative approach whereby it assumes that 500 pedestrians will be utilising the pedestrian crossings during the peak hour. The assumption will provide a more robust assessment of the projected performance of the intersection in the future scenario.

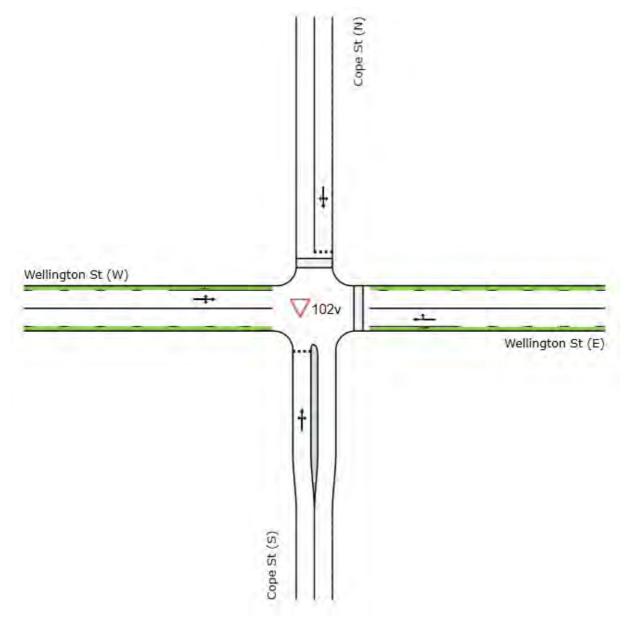


Figure 2 - Cope Street and Wellington Street Intersection Upgrade

1.2 Traffic Modelling

Traffic modelling of the new layout has been undertaken. The network layout has been amended to reflect future road upgrades. The layout of the road network is illustrated in Figure 3.

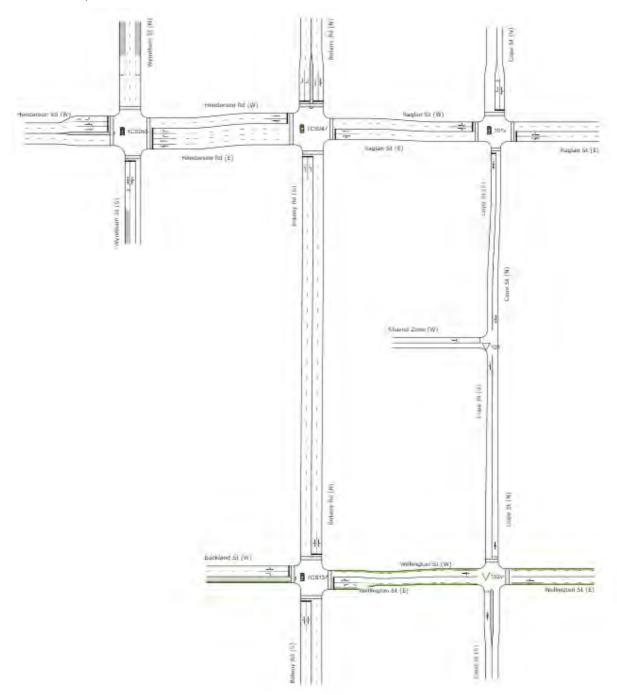


Figure 3 - Local Road Network Layout

A volume analysis was performed using the SIDRA Intersection 8 software, a micro-analytical tool for individual intersection and whole-network modelling. The models are based on the collected traffic survey data. SIDRA provides a number of performance indicators outlined below:

- Degree of Saturation The total usage of the intersection expressed as a factor of 1 with 1 representing 100% use/saturation. (e.g. 0.8=80% saturation)
- Average Delay The average delay encountered by all vehicles passing through the intersection. It is often important to review the average delay of each approach as a side road could have a long delay time, while the large free flowing major traffic will provide an overall low average delay.
- 95% Queue Lengths (Q95) is defined to be the queue length in metres that has only a 5-percent probability of being exceeded during the analysis time period. It transforms the average delay into measurable distance units.
- Level of Service (LoS) This is a categorization of average delay, intended for simple reference. It is a good indicator of overall performance for individual intersections. The RMS adopts the following bands:

Table 1 - Intersection Performance - Levels of Service

Level of Service	Average Delay (secs/vehicle)	Traffic Signals, Roundabout	Give Way & Stop Signs
А	<14	Good operation	
В	15 to 28	Good with acceptable delays & spare capacity	Acceptable delays & spare capacity
С	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Operating near capacity	Near capacity & accident study required
Е	57 to 70	At capacity. At signals, incidents would cause excessive delays. Roundabouts require other control mode	At capacity, requires other control mode
F	>70	Extra capacity required	Extreme delay, major treatment required

The summary of the 2036 model and 2036 model + post development performance of the local road network is summarised in Table 2.

Table 2 - Summary of Intersection Performance

Intersection	Time	Period (2036)	Level of Service	Degree of Saturation (v/c)	Average Delay (s)	95% Queue Length (m)
	AM Peak	Base	D	1.028	56.3	172.7
Henderson Street	AIVITEAN	Development	D	1.028	54.6	172.7
and Wyndham Street	PM Peak	Base	D	1.037	47.6	176.1
	1 WITE CAR	Development	D	1.037	50.8	176.1
	AM Peak	Base	D	1.000	48.0	340.2
Botany Road and	AWITEAR	Development	D	1.000	45.3	323.3
Raglan Street	PM Peak	Base	С	0.884	35.1	211.9
	1 WITCOK	Development	D	1.059	53.7	317.7
	AM Peak	Base	В	0.159	20.7	47.0
Cope Street and	AWITCUR	Development	В	0.195	27.3	58.3
Raglan Street	PM Peak	Base	В	0.206	26.0	61.9
	TIVITCUR	Development	В	0.219	25.9	61.8
	AM Peak	Base	В	0.293	17.3	8.7
Cope Street and	AWITCUR	Development	В	0.296	17.3	8.8
Wellington Street	PM Peak	Base	В	0.378	20.8	11.4
_	TWITCH	Development	В	0.420	22.0	12.2
	AM Peak	Base	В	0.582	14.9	164.5
Botany Road, Wellington Road	7 IVI I CUR	Development	А	0.573	13.5	160.5
and Buckland Street	PM Peak	Base	В	0.518	17.2	139.2
	1 WIT GOK	Development	В	0.716	16.3	172.1
	AM Peak	Base	А	0.051	5.0	0.0
Cope Street and	AIVI I GAK	Development	А	0.052	5.1	0.8
Shared Zone	PM Peak	Base	А	0.053	5.1	0.0
	1 WITEGR	Development	А	0.06	5.2	1.0

1.3 Conclusion

ptc. has undertaken an assessment of the potential performance of the local road network in 2036 with the proposed intersection upgrades along Cope Street & Raglan Street and Cope Street & Wellington Street. The assessment indicates that the local road network will operate at acceptable levels of service or at a level of service less than the approved development SSD 9393 and therefore, the development will have no detrimental impact on the network operation, over and above the approved scheme.

Kind regards,

Steve Wellman

Project Director

Document Control: Prepared by *JJ* on *10 February 2021*. Reviewed by *AP/AM* on *10 February 2021*.

Attachment 1 SIDRA Movement Summaries

Site: TCS055 [1. AM Base Henderson Road / Wyndham Street]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None)

Mov	ement	: Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Quel		Prop. Queued	Effective Stop	Aver. No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles D	istance m		Rate	Cycles	Speed km/h
Sout	h: Wyn	dham St (S		V C 11/11	70	V/O			VOIT					13111/11
1	L2	13	8.3	13	8.3	1.013	115.2	LOS F	21.9	158.9	1.00	1.36	1.79	19.4
2	T1	487	7.6	487	7.6	1.013	110.8	LOS F	23.2	169.0	1.00	1.36	1.79	19.7
3	R2	3	33.3	3	33.3	1.013	114.7	LOS F	23.2	169.0	1.00	1.36	1.78	12.6
Appr	oach	503	7.7	503	7.7	1.013	111.0	LOS F	23.2	169.0	1.00	1.36	1.79	19.7
East	Hende	erson Rd (E)											
4	L2	197	3.2	197	3.2	0.337	8.9	LOS A	3.9	27.8	0.19	0.39	0.19	42.5
5	T1	634	4.8	634	4.8	0.337	3.6	LOS A	4.2	30.2	0.18	0.22	0.18	45.4
6	R2	927	5.1	927	5.1	0.882	43.4	LOS D	15.7	114.2	1.00	0.98	1.26	24.1
Appr	oach	1758	4.8	1758	4.8	0.882	25.2	LOS B	15.7	114.2	0.61	0.64	0.75	30.8
West	: Hend	erson Rd ((W)											
10	L2	526	6.6	526	6.6	1.028	118.8	LOS F	23.4	172.7	1.00	1.20	1.76	18.8
11	T1	288	3.6	288	3.6	0.487	36.3	LOS C	13.7	98.4	0.87	0.74	0.87	25.2
Appr	oach	815	5.6	815	5.6	1.028	89.6	LOS F	23.4	172.7	0.95	1.04	1.45	19.9
All Ve	ehicles	3076	5.5	3076	5.5	1.028	56.3	LOS D	23.4	172.7	0.77	0.86	1.10	23.7

♦♦ Network: N101 [AM Base

(2036)1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians											
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate				
P1	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95				
P2	East Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95				
P3	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95				
P4	West Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95				
All Pe	edestrians	211	54.3	LOS E			0.95	0.95				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

♦♦ Network: N101 [AM Base Site: TCS047 [2. AM Base Botany Road / Raglan Street -Lane Widening]

(2036)]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 120 seconds (Network Site User-Given Phase Times)

Mov	ement	: Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% B Que		Prop. Queued	Effective Stop	Aver. A	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles veh	Distance m		Rate	Cycles S	Speed km/h
Sout	h: Bota	ny Rd (S)												
1	L2	892	5.3	892	5.3	1.000	105.6	LOS F	46.6	340.2	1.00	1.20	1.63	6.9
Appr	oach	892	5.3	892	5.3	1.000	105.6	LOS F	46.6	340.2	1.00	1.20	1.63	6.9
East	: Raglaı	n St (E)												
4	L2	4	0.0	4	0.0	0.541	64.9	LOS E	7.9	56.7	1.00	0.80	1.00	4.4
5	T1	267	3.9	267	3.9	0.541	59.7	LOS E	8.1	58.4	1.00	0.80	1.00	4.5
Appr	oach	272	3.9	272	3.9	0.541	59.8	LOS E	8.1	58.4	1.00	0.80	1.00	4.5
Nort	h: Botar	ny Rd (N)												
7	L2	61	8.6	61	8.6	0.496	11.7	LOS A	16.1	120.0	0.45	0.44	0.45	41.6
8	T1	1327	7.6	1327	7.6	0.496	6.1	LOS A	16.1	120.0	0.42	0.40	0.42	42.8
9	R2	609	5.0	609	5.0	0.656	47.7	LOS D	16.1	117.0	0.95	0.84	0.95	21.8
Appr	oach	1998	6.8	1998	6.8	0.656	18.9	LOS B	16.1	120.0	0.58	0.54	0.58	33.2
Wes	t: Hend	erson Rd (W)											
11	T1	251	2.1	250	2.1	0.856	57.5	LOS E	10.5	74.1	1.00	0.86	1.09	4.9
12	R2	49	14.9	49	14.9	0.856	67.4	LOS E	7.8	57.7	1.00	0.94	1.26	4.5
Appr	oach	300	4.2	300	4.2	0.856	59.1	LOS E	10.5	74.1	1.00	0.87	1.12	4.8
All V	ehicles	3461	6.0	3461	6.0	1.000	48.0	LOS D	46.6	340.2	0.76	0.76	0.93	18.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians											
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate				
P1	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95				
P2	East Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95				
P3	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95				
P4	West Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95				
All Pe	destrians	211	54.3	LOS E			0.95	0.95				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101v [3. AM Base Cope Street / Raglan Street - Signal]

♦♦ Network: N101 [AM Base (2036)]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 120 seconds (Network User-Given Cycle Time)

Mov	ement	t Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival		Deg. Satn	Average Delay	Level of Service	95% Bac Queue		Prop. Queued	Effective Stop	Aver. <i>A</i> No.	Averag e
		Total		Total	HV				Vehicles Dis	stance		Rate	Cycles S	Speed
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
	•	e St (S)												
1	L2	32	0.0	32	0.0	0.158	32.2	LOS C	5.1	28.7	0.72	0.63	0.72	12.0
2	T1	87	1.2	87	1.2	0.158	27.7	LOS B	5.1	28.7	0.72	0.63	0.72	26.8
3	R2	12	0.0	12	0.0	0.158	32.2	LOS C	5.1	28.7	0.72	0.63	0.72	29.4
Appr	oach	131	8.0	131	8.0	0.158	29.2	LOS C	5.1	28.7	0.72	0.63	0.72	25.1
East:	Ragla	n St (E)												
4	L2	4	75.0	4	75.0	0.130	19.5	LOS B	3.7	27.4	0.52	0.44	0.52	35.9
5	T1	205	5.6	205	5.6	0.130	14.7	LOS B	3.7	27.4	0.53	0.46	0.53	34.8
6	R2	28	0.0	28	0.0	0.130	18.9	LOS B	3.1	20.9	0.54	0.50	0.54	36.8
Appr	oach	238	6.2	238	6.2	0.130	15.3	LOS B	3.7	27.4	0.53	0.47	0.53	35.2
North	і: Соре	St (N)												
7	L2	14	0.0	14	0.0	0.055	30.7	LOS C	1.7	10.1	0.68	0.57	0.68	34.1
8	T1	31	0.0	31	0.0	0.055	26.5	LOS B	1.7	10.1	0.68	0.57	0.68	27.1
9	R2	47	4.4	47	4.4	0.108	34.3	LOS C	1.9	13.7	0.72	0.71	0.72	25.9
Appr	oach	92	2.3	92	2.3	0.108	31.2	LOS C	1.9	13.7	0.70	0.64	0.70	28.0
West	:: Ragla	an St (W)												
10	L2	54	3.9	54	3.9	0.159	15.6	LOS B	3.1	22.5	0.36	0.42	0.36	37.8
11	T1	232	3.6	232	3.6	0.159	18.0	LOS B	6.6	47.0	0.61	0.57	0.61	34.3
12	R2	21	5.0	21	5.0	0.159	28.2	LOS B	6.6	47.0	0.82	0.70	0.82	9.6
Appr	oach	306	3.8	306	3.8	0.159	18.2	LOS B	6.6	47.0	0.58	0.55	0.58	34.0
All Ve	ehicles	766	3.8	766	3.8	0.159	20.7	LOS B	6.6	47.0	0.60	0.55	0.60	31.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pe	destrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m		Effective Stop Rate
P1	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P2	East Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P3	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P4	West Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
All Pe	edestrians	211	54.3	LOS E			0.95	0.95

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.

V Site: 102v [4. AM Base Cope Street / Wellington Street -

+ Network: N101 [AM Base (2036)]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None) Giveway / Yield (Two-Way)

Mov	Movement Performance - Vehicles													
Mov ID	Turn	Demand				Deg. Satn	Average Delay	Level of Service	95% Back Queue		Prop. Queued	Effective Stop	Aver. No.	Averag e
		Total		Total	HV				Vehicles Dis	tance		Rate	Cycles	Speed
Caut	h. Cana	veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
	h: Cope	` '	0.0	4.5	0.0	0.007	6.0	1.00.4	0.2	2.0	0.00	0.70	0.00	20.2
1	L2	15	0.0	15	0.0	0.087	6.9	LOSA	0.3	2.0	0.62	0.76	0.62	36.3
2	T1	20	0.0	20	0.0	0.087	11.5	LOS A	0.3	2.0	0.62	0.76	0.62	36.3
3	R2	5	0.0	5	0.0	0.087	14.0	LOS A	0.3	2.0	0.62	0.76	0.62	40.2
Appr	oach	40	0.0	40	0.0	0.087	10.1	LOS A	0.3	2.0	0.62	0.76	0.62	37.1
East	: Wellin	gton St (E)											
4	L2	12	0.0	12	0.0	0.029	6.7	LOS A	0.1	0.6	0.52	0.53	0.52	40.4
5	T1	46	2.3	46	2.3	0.127	4.5	LOS A	0.5	3.6	0.61	0.64	0.61	39.2
6	R2	22	0.0	22	0.0	0.127	15.7	LOS B	0.5	3.6	0.63	0.67	0.63	40.4
Appr	oach	80	1.3	80	1.3	0.127	7.9	NA	0.5	3.6	0.60	0.63	0.60	39.7
Nort	n: Cope	St (N)												
7	L2	13	0.0	13	0.0	0.164	14.2	LOS A	0.6	4.0	0.72	0.85	0.72	36.6
8	T1	21	0.0	21	0.0	0.164	11.0	LOS A	0.6	4.0	0.72	0.85	0.72	33.4
9	R2	21	15.0	21	15.0	0.164	17.3	LOS B	0.6	4.0	0.72	0.85	0.72	14.7
Appr	oach	55	5.8	55	5.8	0.164	14.2	LOS A	0.6	4.0	0.72	0.85	0.72	31.1
Wes	t: Wellir	ngton St (V	V)											
10	L2	85	1.2	85	1.2	0.293	5.9	LOS A	1.2	8.7	0.39	0.46	0.40	22.1
11	T1	158	3.3	158	3.3	0.293	5.3	LOS A	1.2	8.7	0.36	0.45	0.37	43.4
12	R2	33	3.2	33	3.2	0.293	4.7	LOS A	1.2	8.7	0.33	0.44	0.35	41.3
Appr	oach	276	2.7	276	2.7	0.293	5.4	NA	1.2	8.7	0.37	0.45	0.38	40.9
All V	ehicles	451	2.6	451	2.6	0.293	7.4	NA	1.2	8.7	0.47	0.56	0.48	38.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Organisation: PARKING AND TRAFFIC CONSULTANTS | Processed: Wednesday, 10 February 2021 9:59:44 AM
Project: Z:\PCI - PROJECT WORK FILES\NSW\MIRVAC - WATERLOO METRO STATION\4. DA Stage\3. Modelling & Surveys\210209 - ptc -Waterloo Metro SIDRA Network Model.sip8

Site: TCS137 [5. AM Base Botany Road / Wellington Street / Photwork: N101 [AM Base Buckland Street - Upgrade] (2036)]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 120 seconds (Site User-Given Phase Times)

Mov	ement	t Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Que		Prop. Queued	Effective Stop	Aver No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles [veh	Distance m		Rate	Cycles S	Speed km/h
Sout	h: Bota	ny Rd (S)	70	VCII/II	70	V/C	300		VOII					IXIII/II
1	L2	2	0.0	2	0.0	0.582	12.0	LOS A	22.5	164.5	0.56	0.51	0.56	34.6
2	T1	849	5.7	849	5.7	0.582	11.2	LOS A	22.5	164.5	0.58	0.53	0.58	38.2
3	R2	81	5.2	81	5.2	0.582	25.1	LOS B	7.0	51.2	0.72	0.70	0.72	31.1
Аррі	oach	933	5.6	933	5.6	0.582	12.4	LOS A	22.5	164.5	0.59	0.55	0.59	37.4
East	: Wellin	gton St (E))											
4	L2	52	6.1	52	6.1	0.080	48.1	LOS D	1.3	9.4	0.86	0.70	0.86	22.8
5	T1	22	0.0	22	0.0	0.127	46.8	LOS D	2.2	11.1	0.89	0.70	0.89	17.8
6	R2	22	4.8	22	4.8	0.127	51.1	LOS D	2.2	11.1	0.89	0.70	0.89	4.8
Appı	oach	96	4.4	96	4.4	0.127	48.5	LOS D	2.2	11.1	0.88	0.70	0.88	18.7
Nort	h: Botaı	ny Rd (N)												
7	L2	21	0.0	21	0.0	0.536	14.0	LOS A	19.2	143.2	0.53	0.49	0.53	31.9
8	T1	1340	8.1	1340	8.1	0.536	9.6	LOS A	19.2	143.2	0.53	0.49	0.53	42.3
9	R2	2	0.0	2	0.0	0.536	11.6	LOS A	19.1	142.6	0.53	0.49	0.53	31.1
Аррі	oach	1363	8.0	1363	8.0	0.536	9.6	LOS A	19.2	143.2	0.53	0.49	0.53	42.2
Wes	t: Buckl	land St (W))											
10	L2	11	0.0	11	0.0	0.346	48.0	LOS D	9.4	50.7	0.90	0.74	0.90	21.2
11	T1	178	1.2	178	1.2	0.346	43.5	LOS D	9.4	50.7	0.90	0.74	0.90	21.2
12	R2	24	17.4	24	17.4	0.093	49.1	LOS D	1.2	9.6	0.86	0.71	0.86	29.5
Аррі	oach	213	3.0	213	3.0	0.346	44.4	LOS D	9.4	50.7	0.90	0.73	0.90	22.5
All V	ehicles	2604	6.6	2604	6.6	0.582	14.9	LOS B	22.5	164.5	0.59	0.54	0.59	37.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec		verage Back Pedestrian ped	of Queue Distance m	Prop. Queued S	Effective Stop Rate				
P1	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95				
P2	East Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95				
P3	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95				
P4	West Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95				
All Pe	destrians	211	54.3	LOS E			0.95	0.95				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement.



V Site: 101 [6. AM Base Cope Street / Shared Zone]

♦♦ Network: N101 [AM Base (2036)]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Mov	ement	Performa	ance	- Vehic	cles									
Mov ID	Turn	Demand I	Flows	Arrival		Deg. Satn	Average Delay	Level of Service		Back of eue	Prop. Queued	Effective Stop	Aver. A	Averag e
		Total veh/h	HV %	Total veh/h	HV %	v/c	sec		Vehicles veh	Distance m		Rate	Cycles S	Speed km/h
South: Cope St (S)														
1	L2	1	0.0	1	0.0	0.051	3.8	LOS A	0.0	0.0	0.00	0.01	0.00	43.2
2	T1	127	8.0	127	8.0	0.051	0.0	LOS A	0.0	0.0	0.00	0.01	0.00	43.0
Appro	oach	128	0.8	128	8.0	0.051	0.0	NA	0.0	0.0	0.00	0.01	0.00	43.0
North	: Cope	St (N)												
8	T1	55	7.7	55	7.7	0.027	0.1	LOS A	0.0	0.0	0.01	0.04	0.01	46.3
9	R2	1	0.0	1	0.0	0.027	4.9	LOS A	0.0	0.0	0.01	0.04	0.01	47.7
Appro	oach	56	7.5	56	7.5	0.027	0.2	NA	0.0	0.0	0.01	0.04	0.01	46.4
West	: Share	ed Zone (W	/)											
10	L2	1	0.0	1	0.0	0.002	4.8	LOS A	0.0	0.0	0.20	0.50	0.20	43.7
12	R2	1	0.0	1	0.0	0.002	5.0	LOS A	0.0	0.0	0.20	0.50	0.20	43.7
Appro	oach	2	0.0	2	0.0	0.002	4.9	LOS A	0.0	0.0	0.20	0.50	0.20	43.7
All Ve	ehicles	186	2.8	186	2.8	0.051	0.2	NA	0.0	0.0	0.00	0.02	0.00	44.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2019 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: PARKING AND TRAFFIC CONSULTANTS | Processed: Wednesday, 10 February 2021 9:59:44 AM
Project: Z:\PCI - PROJECT WORK FILES\NSW\MIRVAC - WATERLOO METRO STATION\4. DA Stage\3. Modelling & Surveys\210209 - ptc -Waterloo Metro SIDRA Network Model.sip8

Site: TCS055 [1. PM Base Henderson Road / Wyndham Street]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None)

Move	Movement Performance - Vehicles													
Mov ID	Turn	Demand F	lows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Bad Queu		Prop. Queued	Effective Stop	Aver No.	Averag e
		Total		Total	HV				Vehicles D	istance		Rate	Cycles S	_
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
South	ı: Wyn	dham St (S)											
1	L2	16	0.0	16	0.0	1.037	129.3	LOS F	23.5	164.6	1.00	1.41	1.90	18.3
2	T1	493	1.5	493	1.5	1.037	124.6	LOS F	25.2	176.1	1.00	1.41	1.89	18.3
3	R2	2	0.0	2	0.0	1.037	128.9	LOS F	25.2	176.1	1.00	1.42	1.88	11.5
Appro	oach	511	1.4	511	1.4	1.037	124.7	LOS F	25.2	176.1	1.00	1.41	1.89	18.3
East:	Hende	erson Rd (E	:)											
4	L2	165	1.3	165	1.3	0.410	15.6	LOS B	14.1	98.5	0.52	0.55	0.52	38.0
5	T1	915	0.9	915	0.9	0.410	8.3	LOS A	14.1	98.5	0.42	0.41	0.42	40.9
6	R2	851	3.5	851	3.5	0.759	12.2	LOS A	10.1	72.6	0.58	0.73	0.58	37.7
Appro	oach	1931	2.1	1931	2.1	0.759	10.7	LOS A	14.1	98.5	0.50	0.56	0.50	39.2
West	Hend	erson Rd (\	N)											
10	L2	525	1.6	525	1.6	1.021	114.1	LOS F	22.8	161.6	1.00	1.18	1.73	19.3
11	T1	311	0.0	311	0.0	0.522	37.5	LOS C	15.1	103.9	0.88	0.76	0.88	24.8
Appro	oach	836	1.0	836	1.0	1.021	85.7	LOS F	22.8	161.6	0.96	1.02	1.41	20.4
All Ve	hicles	3277	1.7	3277	1.7	1.037	47.6	LOS D	25.2	176.1	0.69	0.81	0.95	25.7

♦ Network: N101 [PM Base

(2036)1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate				
P1	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95				
P2	East Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95				
P3	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95				
P4	West Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95				
All Pe	edestrians	211	54.3	LOS E			0.95	0.95				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

♦♦ Network: N101 [PM Base Site: TCS047 [2. PM Base Botany Road / Raglan Street -Lane Widening]

(2036)]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 120 seconds (Network Site User-Given Phase Times)

Mov	ement	t Perform	ance ·	- Vehi	cles									
Mov ID	Turn	Demand Total	HV	Total	HV	Deg. Satn	Average Delay	Level of Service	95% Ba Que Vehicles [ue	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	
South	h: Bota	veh/h ny Rd (S)	<u>%</u>	veh/h	%	v/c	sec		veh	m				km/h
1	L2	838	2.9	838	2.9	0.884	61.4	LOS E	29.6	211.9	1.00	0.98	1.23	10.8
Appro		838	2.9	838	2.9	0.884	61.4	LOS E	29.6	211.9	1.00	0.98	1.23	10.8
	_	n St (E)												
4	L2	9	0.0	9	0.0	0.600	62.8	LOS E	7.9	54.9	1.00	0.82	1.00	4.5
5	T1	315	1.3	315	1.3	0.600	56.3	LOS D	11.0	76.5	1.00	0.82	1.00	4.7
Appr	oach	324	1.3	324	1.3	0.600	56.5	LOS E	11.0	76.5	1.00	0.82	1.00	4.7
North	n: Botai	ny Rd (N)												
7	L2	87	8.4	87	8.4	0.498	15.6	LOS B	18.2	130.0	0.55	0.53	0.55	38.0
8	T1	1227	3.2	1227	3.2	0.498	9.2	LOS A	18.2	130.0	0.51	0.48	0.51	39.7
9	R2	757	1.5	757	1.5	0.746	42.5	LOS C	22.8	159.9	0.94	0.87	0.97	23.1
Appro	oach	2072	2.8	2072	2.8	0.746	21.7	LOS B	22.8	159.9	0.67	0.62	0.68	31.5
West	: Hend	erson Rd ((W)											
11	T1	262	8.0	262	8.0	0.634	28.7	LOS C	6.4	44.4	0.66	0.53	0.66	8.8
12	R2	37	0.0	37	0.0	0.634	54.3	LOS D	6.4	44.4	0.95	0.77	0.96	5.5
Appro	oach	299	0.7	299	0.7	0.634	31.8	LOS C	6.4	44.4	0.69	0.56	0.70	8.2
All Ve	ehicles	3533	2.5	3533	2.5	0.884	35.1	LOS C	29.6	211.9	0.78	0.72	0.84	21.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate						
P1	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95						
P2	East Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95						
P3	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95						
P4	West Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95						
All Pe	II Pedestrians		54.3	LOS E			0.95	0.95						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101v [3. PM Base Cope Street / Raglan Street - Conversion]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 120 seconds (Network User-Given Cycle Time)

Mov	ement	t Perform	nance	- Vehi	cles									
Mov ID	Turn	Demand Total		Arrival Total	Flows	Deg. Satn	Average Delay	Level of Service	95% Back Queue Vehicles Dis		Prop. Queued	Effective Stop Rate	Aver. A No. Cycles S	e
		veh/h		veh/h	%	v/c	sec		veh	m		rtato	Cyclos c	km/h
Sout	h: Cope	e St (S)												
1	L2	55	0.0	55	0.0	0.127	25.3	LOS B	3.9	24.6	0.62	0.61	0.62	14.5
2	T1	54	0.0	54	0.0	0.127	20.9	LOS B	3.9	24.6	0.62	0.61	0.62	30.9
3	R2	5	20.0	5	20.0	0.127	25.7	LOS B	3.9	24.6	0.62	0.61	0.62	32.0
Appr	oach	114	0.9	114	0.9	0.127	23.3	LOS B	3.9	24.6	0.62	0.61	0.62	26.1
East	Ragla	n St (E)												
4	L2	1	100.0	1	100. 0	0.135	24.8	LOS B	4.0	28.3	0.61	0.50	0.61	32.4
5	T1	197	2.1	197	2.1	0.135	20.4	LOS B	4.0	28.3	0.62	0.51	0.62	32.0
6	R2	9	0.0	9	0.0	0.135	25.5	LOS B	3.0	21.1	0.63	0.53	0.63	37.6
Appr	oach	207	2.5	207	2.5	0.135	20.6	LOS B	4.0	28.3	0.62	0.51	0.62	32.4
North	n: Cope	St (N)												
7	L2	24	0.0	24	0.0	0.074	23.8	LOS B	3.4	15.5	0.61	0.53	0.61	32.7
8	T1	79	1.3	79	1.3	0.074	20.2	LOS B	3.4	15.5	0.61	0.53	0.61	26.8
9	R2	94	0.0	94	0.0	0.205	28.3	LOS B	3.5	22.2	0.67	0.72	0.67	27.4
Appr	oach	197	0.5	197	0.5	0.205	24.5	LOS B	3.5	22.2	0.64	0.62	0.64	28.1
West	:: Ragla	an St (W)												
10	L2	83	1.3	83	1.3	0.206	31.4	LOS C	8.2	57.5	0.85	0.75	0.85	29.0
11	T1	256	3.3	256	3.3	0.206	30.6	LOS C	8.8	61.9	0.91	0.77	0.91	28.3
12	R2	11	0.0	11	0.0	0.206	37.0	LOS C	8.8	61.9	0.94	0.79	0.94	7.5
Appr	oach	349	2.7	349	2.7	0.206	31.0	LOS C	8.8	61.9	0.89	0.77	0.89	28.2
All V	ehicles	867	1.9	867	1.9	0.206	26.0	LOS B	8.8	61.9	0.73	0.65	0.73	28.8

♦ Network: N101 [PM Base

(2036)]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - P	edestrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P2	East Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P3	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P4	West Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
All Pe	destrians	211	54.3	LOS E			0.95	0.95

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

V Site: 102v [4. PM Base Cope Street / Wellington Street -

♦ Network: N101 [PM Base (2036)]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None) Giveway / Yield (Two-Way)

Mov	/ement	Perform	nance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Bac Queue		Prop. Queued	Effective Stop	Aver. No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles Div			Rate	Cycles	Speed km/h
Sou	th: Cope		/0	VCII/II	/0	V/C	360	_	VCII	m	_		_	KIII/II
1	L2	29	0.0	29	0.0	0.095	7.2	LOS A	0.3	2.2	0.60	0.76	0.60	37.9
2	T1	14	0.0	14	0.0	0.095	13.1	LOS A	0.3	2.2	0.60	0.76	0.60	37.9
3	R2	4	0.0	4	0.0	0.095	16.8	LOS B	0.3	2.2	0.60	0.76	0.60	42.5
App	roach	47	0.0	47	0.0	0.095	9.8	LOS A	0.3	2.2	0.60	0.76	0.60	38.6
Eas	t: Wellin	gton St (E	<u>:</u>)											
4	L2	8	0.0	8	0.0	0.053	6.5	LOS A	0.2	8.0	0.55	0.50	0.55	36.8
5	T1	141	1.5	141	1.5	0.325	5.7	LOS A	1.6	11.4	0.65	0.72	0.76	37.9
6	R2	43	0.0	43	0.0	0.325	18.6	LOS B	1.6	11.4	0.67	0.77	0.81	39.7
Арр	roach	193	1.1	193	1.1	0.325	8.6	NA	1.6	11.4	0.65	0.72	0.77	37.9
Nort	h: Cope	St (N)												
7	L2	15	0.0	15	0.0	0.378	9.3	LOS A	1.7	8.6	0.78	0.95	1.02	31.7
8	T1	24	0.0	24	0.0	0.378	15.0	LOS B	1.7	8.6	0.78	0.95	1.02	28.3
9	R2	75	2.8	75	2.8	0.378	20.8	LOS B	1.7	8.6	0.78	0.95	1.02	11.8
App	roach	114	1.9	114	1.9	0.378	18.1	LOS B	1.7	8.6	0.78	0.95	1.02	22.3
Wes	t: Wellir	ngton St (\	V)											
10	L2	54	2.0	54	2.0	0.243	6.6	LOS A	0.9	6.6	0.45	0.51	0.45	23.7
11	T1	118	1.8	118	1.8	0.243	5.0	LOS A	0.9	6.6	0.45	0.51	0.45	43.8
12	R2	22	0.0	22	0.0	0.243	4.9	LOS A	0.9	6.6	0.45	0.52	0.45	43.1
App	roach	194	1.6	194	1.6	0.243	5.4	NA	0.9	6.6	0.45	0.51	0.45	42.0
All \	ehicles/	547	1.3	547	1.3	0.378	9.6	NA	1.7	11.4	0.60	0.70	0.69	36.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Organisation: PARKING AND TRAFFIC CONSULTANTS | Processed: Wednesday, 10 February 2021 11:29:53 AM
Project: Z:\PCI - PROJECT WORK FILES\NSW\MIRVAC - WATERLOO METRO STATION\4. DA Stage\3. Modelling & Surveys\210209 - ptc -Waterloo Metro SIDRA Network Model.sip8

Site: TCS137 [5. PM Base Botany Road / Wellington Street / Photwork: N101 [PM Base Buckland Street - Upgrade] (2036)]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 120 seconds (Network Site User-Given Phase Times)

Mov	/ement	t Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand I	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Quei		Prop. Queued	Effective Stop	Aver No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles E	istance m		Rate	Cycles S	Speed km/h
Sou	th: Bota	ny Rd (S)	70	V C 11/11	70	V/O			VOII					13111/11
1	L2	2	0.0	2	0.0	0.518	13.2	LOS A	19.5	139.2	0.56	0.51	0.56	34.2
2	T1	783	2.8	783	2.8	0.518	12.7	LOS A	19.5	139.2	0.58	0.54	0.58	37.0
3	R2	76	0.0	76	0.0	0.518	24.9	LOS B	8.0	57.0	0.70	0.67	0.70	31.4
App	roach	861	2.6	861	2.6	0.518	13.8	LOS A	19.5	139.2	0.59	0.55	0.59	36.4
East	: Wellin	gton St (E)												
4	L2	147	2.9	147	2.9	0.191	45.8	LOS D	3.6	26.0	0.86	0.75	0.86	23.5
5	T1	59	0.0	59	0.0	0.252	41.8	LOS C	5.6	27.3	0.87	0.73	0.87	18.6
6	R2	58	0.0	58	0.0	0.252	46.1	LOS D	5.6	27.3	0.87	0.73	0.87	5.3
App	roach	264	1.6	264	1.6	0.252	45.0	LOS D	5.6	27.3	0.86	0.74	0.86	19.7
Nort	h: Botaı	ny Rd (N)												
7	L2	21	0.0	21	0.0	0.501	15.5	LOS B	18.4	131.2	0.55	0.51	0.55	30.2
8	T1	1239	3.1	1239	3.1	0.501	11.0	LOS A	18.5	131.7	0.55	0.50	0.55	41.2
9	R2	1	0.0	1	0.0	0.501	13.0	LOS A	18.5	131.7	0.55	0.50	0.55	30.6
App	roach	1261	3.1	1261	3.1	0.501	11.1	LOS A	18.5	131.7	0.55	0.50	0.55	41.1
Wes	t: Buckl	and St (W)												
10	L2	12	0.0	12	0.0	0.200	43.4	LOS D	4.8	30.7	0.84	0.68	0.84	23.4
11	T1	93	2.3	93	2.3	0.200	38.9	LOS C	4.8	30.7	0.84	0.68	0.84	23.4
12	R2	33	0.0	33	0.0	0.110	46.4	LOS D	1.6	10.9	0.84	0.72	0.84	30.2
App	roach	137	1.5	137	1.5	0.200	41.1	LOS C	4.8	30.7	0.84	0.69	0.84	25.6
All V	ehicles/	2523	2.7	2523	2.7	0.518	17.2	LOS B	19.5	139.2	0.61	0.55	0.61	35.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. E Queued S	Effective top Rate						
P1	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95						
P2	East Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95						
P3	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95						
P4	West Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95						
All Pe	edestrians	211	54.3	LOS E			0.95	0.95						



V Site: 101 [6. PM Base Cope Street / Shared Zone]

♦ Network: N101 [PM Base (2036)]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Move	ement	Performa	ance -	· Vehic	cles									
Mov ID	Turn	Demand I				Deg. Satn	Average Delay	Level of Service	95% Ba Quet		Prop. Queued	Effective Stop	Aver. A	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles D veh	istance m		Rate	Cycles S	Speed km/h
South	: Cope	St (S)												
1	L2	1	0.0	1	0.0	0.053	4.3	LOS A	0.0	0.0	0.00	0.01	0.00	47.3
2	T1	111	1.0	111	1.0	0.053	0.0	LOS A	0.0	0.0	0.00	0.01	0.00	47.6
Appro	ach	112	0.9	112	0.9	0.053	0.0	NA	0.0	0.0	0.00	0.01	0.00	47.5
North	North: Cope St (N)													
8	T1	91	2.3	91	2.3	0.029	0.2	LOS A	0.0	0.0	0.01	0.05	0.01	38.4
9	R2	1	0.0	1	0.0	0.029	4.9	LOS A	0.0	0.0	0.01	0.05	0.01	45.7
Appro	oach	92	2.3	92	2.3	0.029	0.2	NA	0.0	0.0	0.01	0.05	0.01	38.7
West	Share	d Zone (W	')											
10	L2	1	0.0	1	0.0	0.002	4.8	LOS A	0.0	0.0	0.20	0.50	0.20	43.7
12	R2	1	0.0	1	0.0	0.002	5.1	LOS A	0.0	0.0	0.20	0.50	0.20	43.7
Appro	ach	2	0.0	2	0.0	0.002	5.0	LOS A	0.0	0.0	0.20	0.50	0.20	43.7
All Ve	hicles	205	1.5	205	1.5	0.053	0.2	NA	0.0	0.0	0.00	0.03	0.00	42.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

SIDRA INTERSECTION 8.0 | Copyright © 2000-2019 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: PARKING AND TRAFFIC CONSULTANTS | Processed: Wednesday, 10 February 2021 11:29:53 AM
Project: Z:\PCI - PROJECT WORK FILES\NSW\MIRVAC - WATERLOO METRO STATION\4. DA Stage\3. Modelling & Surveys\210209 - ptc -Waterloo Metro SIDRA Network Model.sip8

Site: TCS055 [1. AM Base + Dev 3 Henderson Road / ♦♦ Network: N101 [AM Base + Wyndham Street]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 120 seconds (Network Site User-Given Phase Times)

Mov	ement	t Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Quet		Prop. Queued	Effective Stop	Aver No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c			Vehicles D			Rate	Cycles	
South	า: Wvn	dham St (S		ven/n	70	V/C	sec		veh	m				km/h
1	L2	13	8.3	13	8.3	1.013	115.2	LOS F	21.9	158.9	1.00	1.36	1.79	19.4
2	T1	487	7.6	487	7.6	1.013	110.8	LOS F	23.2	169.0	1.00	1.36	1.79	19.7
3	R2	3	33.3	3	33.3	1.013	114.7	LOS F	23.2	169.0	1.00	1.36	1.78	12.6
Appr	oach	503	7.7	503	7.7	1.013	111.0	LOS F	23.2	169.0	1.00	1.36	1.79	19.7
East:	Hende	erson Rd (E	Ξ)											
4	L2	197	3.2	197	3.2	0.339	9.6	LOS A	4.5	32.4	0.22	0.41	0.22	41.9
5	T1	640	4.8	640	4.8	0.339	3.3	LOS A	4.5	32.4	0.16	0.21	0.16	45.6
6	R2	937	5.1	937	5.1	0.891	38.2	LOS C	15.7	114.2	0.98	0.96	1.22	25.7
Appro	oach	1774	4.7	1774	4.7	0.891	22.4	LOS B	15.7	114.2	0.60	0.63	0.73	32.1
West	: Hend	erson Rd (W)											
10	L2	526	6.6	526	6.6	1.028	118.8	LOS F	23.4	172.7	1.00	1.20	1.76	18.8
11	T1	291	3.6	291	3.6	0.491	36.3	LOS C	13.8	99.3	0.87	0.74	0.87	25.2
Appro	oach	817	5.5	817	5.5	1.028	89.5	LOS F	23.4	172.7	0.95	1.04	1.44	19.9
All Ve	ehicles	3094	5.4	3094	5.4	1.028	54.6	LOS D	23.4	172.7	0.76	0.86	1.09	24.1

Prop Dev (2036)1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - P	edestrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P2	East Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P3	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P4	West Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
All Pe	destrians	211	54.3	LOS E			0.95	0.95

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: TCS047 [2. AM Base + Dev 3 Botany Road / Raglan Street]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None)

Mov	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand				Deg. Satn	Average Delay	Level of Service	95% Ba Quei	ле	Prop. Queued	Effective Stop	No.	Averag
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles E veh	istance m		Rate	Cycles	Speed km/h
South	n: Botai	ny Rd (S)												
1	L2	892	5.3	892	5.3	1.000	96.4	LOS F	44.3	323.3	1.00	1.16	1.55	7.5
Appro		892	5.3	892	5.3	1.000	96.4	LOS F	44.3	323.3	1.00	1.16	1.55	7.5
East:	Raglar	n St (E)												
4	L2	4	0.0	4	0.0	0.571	64.6	LOS E	8.4	59.8	1.00	0.80	1.00	4.4
5	T1	283	3.7	283	3.7	0.571	59.7	LOS E	8.6	61.7	1.00	0.80	1.00	4.5
Appro	oach	287	3.7	287	3.7	0.571	59.7	LOS E	8.6	61.7	1.00	0.80	1.00	4.5
North	: Botar	ny Rd (N)												
7	L2	63	8.3	63	8.3	0.497	11.7	LOS A	16.2	120.2	0.45	0.44	0.45	41.6
8	T1	1327	7.6	1327	7.6	0.497	6.1	LOS A	16.2	120.2	0.42	0.40	0.42	42.8
9	R2	609	5.0	609	5.0	0.656	47.7	LOS D	16.1	117.0	0.95	0.84	0.95	21.8
Appro	oach	2000	6.8	2000	6.8	0.656	18.9	LOS B	16.2	120.2	0.59	0.54	0.59	33.2
West	: Hende	erson Rd (W)											
11	T1	252	2.1	252	2.1	0.869	52.6	LOS D	9.9	69.8	0.97	0.83	1.05	5.3
12	R2	49	14.9	49	14.9	0.869	70.0	LOS E	7.7	57.1	1.00	0.91	1.21	4.3
Appro	oach	301	4.2	301	4.2	0.869	55.4	LOS D	9.9	69.8	0.97	0.84	1.07	5.1
All Ve	ehicles	3480	6.0	3480	6.0	1.000	45.3	LOS D	44.3	323.3	0.76	0.75	0.91	18.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate						
P1	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95						
P2	East Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95						
P3	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95						
P4	West Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95						
All Pe	II Pedestrians		54.3	LOS E			0.95	0.95						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101v [3. AM Base + Dev 3 Cope Street / Raglan Street - + Network: N101 [AM Base + Signal]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 120 seconds (Network User-Given Cycle Time)

Мо	vement	Perform	nance	- Vehi	cles									
Mo ^s ID	/ Turn	Demand				Deg. Satn	Average Delay	Level of Service	95% Bacl Queue		Prop. Queued	Effective Stop	No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles Dis			Rate	Cycles	Speed km/h
Sou	ıth: Cope		70	ven/m	70	V/C	Sec		ven	m				KIII/II
1	L2	47	0.0	47	0.0	0.176	26.0	LOS B	5.7	33.5	0.64	0.62	0.64	14.1
2	T1	87	1.2	87	1.2	0.176	21.5	LOS B	5.7	33.5	0.64	0.62	0.64	28.8
3	R2	28	0.0	28	0.0	0.176	26.0	LOS B	5.7	33.5	0.64	0.62	0.64	31.9
App	roach	163	0.6	163	0.6	0.176	23.6	LOS B	5.7	33.5	0.64	0.62	0.64	27.1
Eas	t: Ragla	n St (E)												
4	L2	7	42.9	7	42.9	0.159	25.0	LOS B	4.6	33.7	0.62	0.52	0.62	32.2
5	T1	205	5.6	205	5.6	0.159	21.3	LOS B	4.6	33.7	0.63	0.54	0.63	31.0
6	R2	28	0.0	28	0.0	0.159	26.8	LOS B	3.8	25.3	0.66	0.58	0.66	34.1
App	roach	241	6.1	241	6.1	0.159	22.0	LOS B	4.6	33.7	0.64	0.55	0.64	31.6
Nor	th: Cope	St (N)												
7	L2	14	0.0	14	0.0	0.045	24.2	LOS B	1.4	8.7	0.60	0.52	0.60	36.3
8	T1	31	0.0	31	0.0	0.045	20.0	LOS B	1.4	8.7	0.60	0.52	0.60	30.0
9	R2	47	4.4	47	4.4	0.090	27.8	LOS B	1.7	12.1	0.64	0.69	0.64	28.5
App	roach	92	2.3	92	2.3	0.090	24.7	LOS B	1.7	12.1	0.62	0.61	0.62	30.6
We	st: Ragla	ın St (W)												
10	L2	54	3.9	54	3.9	0.195	35.6	LOS C	8.1	58.3	0.90	0.77	0.90	27.8
11	T1	232	3.6	232	3.6	0.195	33.2	LOS C	8.1	58.3	0.94	0.79	0.94	27.4
12	R2	24	4.3	24	4.3	0.195	39.6	LOS C	7.7	55.0	0.97	0.81	0.97	6.9
App	roach	309	3.7	309	3.7	0.195	34.1	LOS C	8.1	58.3	0.94	0.79	0.94	26.6
All '	Vehicles	805	3.7	805	3.7	0.195	27.3	LOS B	8.1	58.3	0.75	0.66	0.75	28.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Ave Service Pe		of Queue Distance m	Prop. E Queued St	ffective op Rate						
P1	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95						
P2	East Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95						
P3	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95						
P4	West Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95						
All Pe	edestrians	211	54.3	LOS E			0.95	0.95						

V Site: 102v [4. AM Base + Dev 3 Cope Street / Wellington

Street - Priority

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None) Giveway / Yield (Two-Way)

♦♦ Network: N101 [AM Base + Prop Dev (2036)]

Mov	ement	Perform	ance	- Vehic	cles									
Mov ID		Demand				Deg. Satn	Average Delay	Level of Service	95% Back Queue	of	Prop. Queued	Effective Stop	Aver. A	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles Dis	tance m		Rate	Cycles S	Speed km/h
Sout	h: Cope													
1	L2	15	0.0	15	0.0	0.080	4.1	LOS A	0.3	1.8	0.04	0.46	0.04	37.0
2	T1	20	0.0	20	0.0	0.080	11.6	LOS A	0.3	1.8	0.04	0.46	0.04	37.0
3	R2	5	0.0	5	0.0	0.080	14.2	LOS A	0.3	1.8	0.04	0.46	0.04	40.6
Appr	oach	40	0.0	40	0.0	0.080	9.2	LOS A	0.3	1.8	0.04	0.46	0.04	37.8
East	: Wellin	gton St (E))											
4	L2	12	0.0	12	0.0	0.029	6.7	LOS A	0.1	0.6	0.52	0.53	0.52	40.4
5	T1	46	2.3	46	2.3	0.132	4.5	LOS A	0.5	3.7	0.61	0.64	0.61	39.0
6	R2	23	0.0	23	0.0	0.132	16.0	LOS B	0.5	3.7	0.63	0.67	0.63	40.2
Appr	oach	81	1.3	81	1.3	0.132	8.1	NA	0.5	3.7	0.60	0.64	0.60	39.6
North	n: Cope	St (N)												
7	L2	18	0.0	18	0.0	0.204	14.5	LOS B	8.0	5.2	0.74	0.87	0.75	36.3
8	T1	21	0.0	21	0.0	0.204	11.4	LOS A	8.0	5.2	0.74	0.87	0.75	33.1
9	R2	28	11.1	28	11.1	0.204	17.3	LOS B	8.0	5.2	0.74	0.87	0.75	14.4
Appr	oach	67	4.7	67	4.7	0.204	14.8	LOS B	0.8	5.2	0.74	0.87	0.75	30.4
West	t: Wellin	ngton St (W	V)											
10	L2	87	1.2	87	1.2	0.296	6.0	LOS A	1.2	8.8	0.39	0.46	0.40	22.1
11	T1	158	3.3	158	3.3	0.296	5.4	LOS A	1.2	8.8	0.36	0.45	0.38	43.4
12	R2	33	3.2	33	3.2	0.296	4.7	LOS A	1.2	8.8	0.34	0.44	0.35	41.3
Appr	oach	278	2.7	278	2.7	0.296	5.5	NA	1.2	8.8	0.37	0.45	0.38	40.8
All V	ehicles	466	2.5	466	2.5	0.296	7.6	NA	1.2	8.8	0.43	0.55	0.44	38.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Organisation: PARKING AND TRAFFIC CONSULTANTS | Processed: Wednesday, 10 February 2021 9:59:58 AM

Project: Z:\PCI - PROJECT WORK FILES\NSW\MIRVAC - WATERLOO METRO STATION\4. DA Stage\3. Modelling & Surveys\210209 - ptc -Waterloo Metro SIDRA Network Model.sip8

Site: TCS137 [5. AM Base + Dev 3 Botany Road / Wellington Prop Dev (2036)]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 120 seconds (Network Site User-Given Phase Times)

Mov	/ement	t Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Ba Que		Prop. Queued	Effective Stop	Aver No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles [veh			Rate	Cycles	
Sou	th: Bota	ny Rd (S)	70	ven/m	70	V/C	560		ven	m				km/h
1	L2	2	0.0	2	0.0	0.573	11.9	LOS A	22.0	160.5	0.55	0.51	0.55	34.6
2	T1	849	5.7	849	5.7	0.573	10.9	LOS A	22.0	160.5	0.57	0.53	0.57	38.4
3	R2	83	5.1	83	5.1	0.573	22.0	LOS B	7.0	51.3	0.67	0.67	0.67	32.9
App	roach	935	5.6	935	5.6	0.573	11.9	LOS A	22.0	160.5	0.58	0.54	0.58	37.8
East	t: Wellin	gton St (E))											
4	L2	59	5.4	59	5.4	0.091	48.3	LOS D	1.5	10.7	0.86	0.71	0.86	22.8
5	T1	22	0.0	22	0.0	0.127	46.8	LOS D	2.2	11.1	0.89	0.70	0.89	17.8
6	R2	22	4.8	22	4.8	0.127	51.1	LOS D	2.2	11.1	0.89	0.70	0.89	4.8
App	roach	103	4.1	103	4.1	0.127	48.6	LOS D	2.2	11.1	0.88	0.70	0.88	19.0
Nort	h: Botaı	ny Rd (N)												
7	L2	21	0.0	21	0.0	0.536	10.9	LOS A	12.7	94.3	0.35	0.33	0.35	36.0
8	T1	1340	8.1	1340	8.1	0.536	7.0	LOS A	14.3	106.8	0.37	0.35	0.37	44.0
9	R2	2	0.0	2	0.0	0.536	9.6	LOS A	14.3	106.8	0.40	0.36	0.40	31.9
App	roach	1363	8.0	1363	8.0	0.536	7.1	LOS A	14.3	106.8	0.37	0.35	0.37	44.0
Wes	t: Buckl	land St (W))											
10	L2	11	0.0	11	0.0	0.346	48.0	LOS D	9.4	50.7	0.90	0.74	0.90	21.2
11	T1	178	1.2	178	1.2	0.346	43.5	LOS D	9.4	50.7	0.90	0.74	0.90	21.2
12	R2	24	17.4	24	17.4	0.094	49.1	LOS D	1.2	9.6	0.86	0.71	0.86	29.5
App	roach	213	3.0	213	3.0	0.346	44.4	LOS D	9.4	50.7	0.90	0.73	0.90	22.5
All V	ehicles/	2614	6.6	2614	6.6	0.573	13.5	LOSA	22.0	160.5	0.51	0.46	0.51	38.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pe	destrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P2	East Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P3	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P4	West Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
All Pe	edestrians	211	54.3	LOS E			0.95	0.95



V Site: 101 [6. AM Base + Dev 3 Cope Street / Shared Zone]

♦♦ Network: N101 [AM Base + Prop Dev (2036)]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Mov	ement	: Perform	ance ·	- Vehi	cles									
Mov ID	Turn	Demand Total		Arrival Total	Flows	Deg. Satn	Average Delay	Level of Service	95% Back Queue Vehicles Dis		Prop. Queued	Effective Stop Rate	Aver. A No. Cycles S	ě
		veh/h		veh/h	%	v/c	sec		veh	m			0,0.00	km/h
South	า: Соре	e St (S)												
1	L2	3	0.0	3	0.0	0.052	3.8	LOS A	0.0	0.0	0.00	0.02	0.00	43.3
2	T1	127	8.0	127	8.0	0.052	0.0	LOS A	0.0	0.0	0.00	0.02	0.00	42.6
Appro	oach	131	8.0	131	8.0	0.052	0.1	NA	0.0	0.0	0.00	0.02	0.00	42.7
North	orth: Cope St (N)													
8	T1	55	7.7	55	7.7	0.032	0.2	LOS A	0.0	0.3	0.05	0.10	0.05	43.5
9	R2	8	0.0	8	0.0	0.032	4.9	LOS A	0.0	0.3	0.05	0.10	0.05	47.0
Appro	oach	63	6.7	63	6.7	0.032	8.0	NA	0.0	0.3	0.05	0.10	0.05	44.9
West	: Share	ed Zone (W	٧)											
10	L2	32	0.0	32	0.0	0.032	4.9	LOS A	0.1	8.0	0.20	0.51	0.20	43.7
12	R2	13	0.0	13	0.0	0.032	5.1	LOS A	0.1	8.0	0.20	0.51	0.20	43.7
Appro	oach	44	0.0	44	0.0	0.032	4.9	LOS A	0.1	8.0	0.20	0.51	0.20	43.7
All Ve	ehicles	238	2.2	238	2.2	0.052	1.2	NA	0.1	0.8	0.05	0.13	0.05	43.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: Z:\PCI - PROJECT WORK FILES\NSW\MIRVAC - WATERLOO METRO STATION\4. DA Stage\3. Modelling & Surveys\210209 - ptc -Waterloo Metro SIDRA Network Model.sip8

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None)

Mov	ement	: Performa	ance ·	- Vehi	cles									
Mov ID	Turn	Demand F	lows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Bad Queu		Prop. Queued	Effective Stop	Aver. No.	Averag e
		Total		Total	HV				Vehicles D	istance		Rate	Cycles	
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
South	ո։ Wyn	dham St (S)											
1	L2	16	0.0	16	0.0	1.037	129.3	LOS F	23.5	164.6	1.00	1.41	1.90	18.3
2	T1	493	1.5	493	1.5	1.037	124.6	LOS F	25.2	176.1	1.00	1.41	1.89	18.3
3	R2	2	0.0	2	0.0	1.037	128.9	LOS F	25.2	176.1	1.00	1.42	1.88	11.5
Appro	oach	511	1.4	511	1.4	1.037	124.7	LOS F	25.2	176.1	1.00	1.41	1.89	18.3
East:	Hende	erson Rd (E	<u>:</u>)											
4	L2	165	1.3	161	1.3	0.401	15.7	LOS B	14.7	102.7	0.56	0.58	0.56	37.9
5	T1	916	0.9	894	0.9	0.401	8.7	LOS A	14.7	102.7	0.45	0.43	0.45	40.6
6	R2	854	3.5	833	3.4	0.743	22.5	LOS B	12.3	88.4	0.86	0.82	0.87	31.8
Appro	oach	1935	2.1	1888 ^N	¹ 2.0	0.743	15.4	LOS B	14.7	102.7	0.64	0.62	0.65	36.0
West	: Hend	erson Rd (\	N)											
10	L2	525	1.6	525	1.6	1.021	114.1	LOS F	22.8	161.6	1.00	1.18	1.73	19.3
11	T1	317	0.0	317	0.0	0.533	37.6	LOS C	15.4	106.5	0.89	0.76	0.89	24.7
Appro	oach	842	1.0	842	1.0	1.021	85.3	LOS F	22.8	161.6	0.96	1.02	1.41	20.4
All Ve	hicles	3287	1.7	3241 ^N	1 1.7	1.037	50.8	LOS D	25.2	176.1	0.78	0.85	1.04	24.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

N1 Arrival Flow value is reduced due to capacity constraint at oversaturated upstream lanes.

Move	ement Performance - Pe	edestrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued S	Effective Stop Rate
P1	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P2	East Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P3	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P4	West Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
All Pe	edestrians	211	54.3	LOS E			0.95	0.95

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: TCS047 [2. PM Base + Dev 3 Botany Road / Raglan Prop Dev (2036)]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None)

Mov	ement	Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand I Total		Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% B Que Vehicles	eue	Prop. Queued	Effective Stop Rate	Aver. A No. Cycles S	e
0 1	D (veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
		ny Rd (S)												
1	L2	838	2.9	838	2.9	1.059	137.7	LOS F	44.3	317.7	1.00	1.27	1.82	5.4
Appr		838	2.9	838	2.9	1.059	137.7	LOS F	44.3	317.7	1.00	1.27	1.82	5.4
East:	Raglaı	n St (E)												
4	L2	9	0.0	9	0.0	0.640	63.4	LOS E	8.0	55.6	1.00	0.84	1.03	4.5
5	T1	319	1.3	319	1.3	0.640	53.4	LOS D	11.1	77.4	1.00	0.83	1.01	4.9
Appr	oach	328	1.3	328	1.3	0.640	53.7	LOS D	11.1	77.4	1.00	0.83	1.01	4.9
North	n: Botar	ny Rd (N)												
7	L2	97	7.6	97	7.6	0.504	16.1	LOS B	18.7	133.5	0.56	0.55	0.56	37.5
8	T1	1227	3.2	1227	3.2	0.504	9.5	LOS A	18.7	133.5	0.52	0.49	0.52	39.4
9	R2	757	1.5	757	1.5	0.786	45.7	LOS D	24.3	170.4	0.96	0.90	1.03	22.2
Appr	oach	2081	2.8	2081	2.8	0.786	23.0	LOS B	24.3	170.4	0.68	0.64	0.71	30.8
West	: Hend	erson Rd (\	W)											
11	T1	268	0.8	268	0.8	0.666	29.3	LOS C	6.5	44.9	0.67	0.54	0.67	8.7
12	R2	37	0.0	37	0.0	0.666	58.1	LOS E	6.5	44.9	0.98	0.79	1.00	5.2
Appr	oach	305	0.7	305	0.7	0.666	32.8	LOS C	6.5	44.9	0.70	0.57	0.71	8.0
All Ve	ehicles	3553	2.5	3553	2.5	1.059	53.7	LOS D	44.3	317.7	0.79	0.80	1.00	16.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - P	edestrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P2	East Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P3	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P4	West Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
All Pe	All Pedestrians		54.3	LOS E			0.95	0.95

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101v [3. PM Base + Dev 3 Cope Street / Raglan Street - + Network: N101 [PM Base + Conversion] Prop Dev (2036)]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 120 seconds (Network User-Given Cycle Time)

Mov	Movement Performance - Vehicles													
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	95% Back Queue		Prop. Queued	Effective Stop	No.	Averag e
		Total		Total	HV				Vehicles Dis	tance		Rate	Cycles	
Court	h: Cope	veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
	•	` '	0.0		0.0	0.445	00.0	1 00 D	4.0	07.0	0.05	0.00	0.05	40.0
1	L2	59	0.0	59	0.0	0.145	26.8	LOS B	4.3	27.6	0.65	0.63	0.65	13.8
2	T1	54	0.0	54	0.0	0.145	22.4	LOS B	4.3	27.6	0.65	0.63	0.65	30.2
3	R2	9	11.1	9	11.1	0.145	27.0	LOS B	4.3	27.6	0.65	0.63	0.65	31.5
Appr	oach	122	0.9	122	0.9	0.145	24.9	LOS B	4.3	27.6	0.65	0.63	0.65	25.4
East	: Ragla	n St (E)												
4	L2	18	5.9	18	5.9	0.142	23.3	LOS B	4.2	29.8	0.60	0.52	0.60	32.8
5	T1	197	2.1	197	2.1	0.142	19.3	LOS B	4.2	29.8	0.60	0.52	0.60	32.4
6	R2	9	0.0	9	0.0	0.142	24.4	LOS B	3.2	22.2	0.61	0.52	0.61	38.1
Appr	oach	224	2.3	224	2.3	0.142	19.8	LOS B	4.2	29.8	0.60	0.52	0.60	32.9
Nort	h: Cope	St (N)												
7	L2	24	0.0	24	0.0	0.077	25.1	LOS B	3.5	16.0	0.63	0.54	0.63	32.3
8	T1	79	1.3	79	1.3	0.077	21.5	LOS B	3.5	16.0	0.63	0.54	0.63	26.3
9	R2	94	0.0	94	0.0	0.219	29.8	LOS C	3.6	22.9	0.69	0.72	0.69	26.8
Appr	oach	197	0.5	197	0.5	0.219	25.9	LOS B	3.6	22.9	0.66	0.63	0.66	27.6
Wes	t: Ragla	n St (W)												
10	L2	83	1.3	83	1.3	0.214	30.3	LOS C	8.7	61.3	0.84	0.74	0.84	29.5
11	T1	256	3.3	256	3.3	0.214	29.4	LOS C	8.8	61.8	0.89	0.77	0.89	28.7
12	R2	26	0.0	26	0.0	0.214	36.5	LOS C	8.8	61.8	0.93	0.79	0.93	7.5
Appr	oach	365	2.6	365	2.6	0.214	30.1	LOS C	8.8	61.8	0.88	0.76	0.88	28.1
All V	ehicles	908	1.9	908	1.9	0.219	25.9	LOS B	8.8	61.8	0.73	0.66	0.73	28.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Po	edestrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P2	East Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P3	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
P4	West Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95
All Pe	edestrians	211	54.3	LOS E			0.95	0.95

V Site: 102v [4. PM Base + Dev 3 Cope Street / Wellington

Street - Priority1

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None) Giveway / Yield (Two-Way)

		Performa							050/ B			- · ·		
Mov ID	Turn	Demand I	-lows	Arrival	Flows	Deg. Satn	Average Delay		95% Back of Queue		Prop. Queued	Effective	Aver. A	
טו		Total	HV	Total	HV	Salli	Delay	Service	Vehicles Di		Queueu	Stop Rate	Cycles S	e Speed
		veh/h		veh/h	%	v/c	sec		veh	m		rtato	0,0000	km/h
South: Cope St (S)														
1	L2	29	0.0	29	0.0	0.097	7.2	LOS A	0.3	2.2	0.60	0.76	0.60	37.8
2	T1	14	0.0	14	0.0	0.097	13.4	LOS A	0.3	2.2	0.60	0.76	0.60	37.8
3	R2	4	0.0	4	0.0	0.097	17.1	LOS B	0.3	2.2	0.60	0.76	0.60	42.4
Appro	oach	47	0.0	47	0.0	0.097	9.9	LOS A	0.3	2.2	0.60	0.76	0.60	38.5
East: Wellington St (E)														
4	L2	8	0.0	8	0.0	0.053	6.5	LOS A	0.2	0.8	0.55	0.50	0.55	36.8
5	T1	141	1.5	141	1.5	0.343	5.9	LOS A	1.7	12.2	0.65	0.73	0.79	37.5
6	R2	47	0.0	47	0.0	0.343	19.4	LOS B	1.7	12.2	0.67	0.79	0.85	39.1
Appro	oach	197	1.1	197	1.1	0.343	9.2	NA	1.7	12.2	0.65	0.74	0.79	37.5
North	: Cope	St (N)												
7	L2	17	0.0	17	0.0	0.420	17.8	LOS B	1.9	9.9	0.82	1.00	1.12	31.0
8	T1	24	0.0	24	0.0	0.420	16.1	LOS B	1.9	9.9	0.82	1.00	1.12	27.5
9	R2	77	2.7	77	2.7	0.420	22.0	LOS B	1.9	9.9	0.82	1.00	1.12	11.0
Appro	oach	118	1.8	118	1.8	0.420	20.2	LOS B	1.9	9.9	0.82	1.00	1.12	21.4
West	: Wellir	gton St (W	/)											
10	L2	61	1.7	61	1.7	0.252	6.6	LOS A	1.0	6.9	0.46	0.52	0.46	23.6
11	T1	118	1.8	118	1.8	0.252	5.0	LOS A	1.0	6.9	0.46	0.52	0.46	43.7
12	R2	22	0.0	22	0.0	0.252	4.9	LOS A	1.0	6.9	0.45	0.53	0.45	43.0
Appro	oach	201	1.6	201	1.6	0.252	5.5	NA	1.0	6.9	0.46	0.52	0.46	41.7
All Ve	hicles	563	1.3	563	1.3	0.420	10.2	NA	1.9	12.2	0.61	0.72	0.73	35.6

♦♦ Network: N101 [PM Base +

Prop Dev (2036)]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Organisation: PARKING AND TRAFFIC CONSULTANTS | Processed: Wednesday, 10 February 2021 10:00:06 AM Project: Z:\PCI - PROJECT WORK FILES\NSW\MIRVAC - WATERLOO METRO STATION\4. DA Stage\3. Modelling & Surveys\210209 - ptc -Waterloo Metro SIDRA Network Model.sip8

Site: TCS137 [5. PM Base + Dev 3 Botany Road / Wellington Prop Dev (2036)]

Traffic Surveys 12/03/2020 AM Peak: 7:45 - 8:45 PM Peak: 17:15 - 18:15 Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 120 seconds (Network Site User-Given Phase Times)

Movement Performance - Vehicles														
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average		95% Ba Quei		Prop. Queued	Effective Stop	Aver. / No.	
טו		Total	HV	Total	HV	Sauri	Delay	Service	Vehicles E		Queueu	Rate	Cycles S	e Speed
		veh/h		veh/h	%	v/c	sec		veh	m			0,0.00	km/h
South: Botany Rd (S)														
1	L2	2	0.0	2	0.0	0.716	16.1	LOS B	24.1	172.1	0.70	0.65	0.70	33.3
2	T1	783	2.8	783	2.8	0.716	16.0	LOS B	24.1	172.1	0.71	0.67	0.73	34.7
3	R2	83	0.0	83	0.0	0.716	28.5	LOS B	10.7	75.7	0.77	0.78	0.84	29.6
Appr	oach	868	2.5	868	2.5	0.716	17.2	LOS B	24.1	172.1	0.72	0.68	0.74	34.2
East	Wellin	gton St (E)											
4	L2	149	2.8	149	2.8	0.194	45.8	LOS D	3.7	26.4	0.86	0.75	0.86	23.5
5	T1	59	0.0	59	0.0	0.326	43.0	LOS D	5.8	28.1	0.89	0.74	0.89	18.4
6	R2	58	0.0	58	0.0	0.326	47.3	LOS D	5.8	28.1	0.89	0.74	0.89	5.2
Appr	oach	266	1.6	266	1.6	0.326	45.5	LOS D	5.8	28.1	0.87	0.74	0.87	19.6
North	n: Botar	ny Rd (N)												
7	L2	21	0.0	21	0.0	0.502	10.6	LOS A	10.3	73.7	0.31	0.29	0.31	36.4
8	T1	1239	3.1	1239	3.1	0.502	6.8	LOS A	12.1	86.1	0.34	0.31	0.34	44.1
9	R2	1	0.0	1	0.0	0.502	9.4	LOS A	12.1	86.1	0.36	0.33	0.36	31.9
Appr	oach	1261	3.1	1261	3.1	0.502	6.9	LOS A	12.1	86.1	0.33	0.31	0.33	44.0
West	:: Buckl	and St (W)											
10	L2	12	0.0	12	0.0	0.209	43.5	LOS D	4.9	30.8	0.84	0.68	0.84	23.3
11	T1	93	2.3	93	2.3	0.209	39.0	LOS C	4.9	30.8	0.84	0.68	0.84	23.3
12	R2	33	0.0	33	0.0	0.111	46.4	LOS D	1.6	11.0	0.84	0.72	0.84	30.2
Appr	oach	137	1.5	137	1.5	0.209	41.2	LOS C	4.9	30.8	0.84	0.69	0.84	25.6
All Ve	ehicles	2533	2.7	2533	2.7	0.716	16.3	LOS B	24.1	172.1	0.55	0.50	0.56	36.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians											
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate			
P1	South Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95			
P2	East Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95			
P3	North Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95			
P4	West Full Crossing	53	54.3	LOS E	0.2	0.2	0.95	0.95			
All Pe	edestrians	211	54.3	LOS E			0.95	0.95			



V Site: 101 [6. PM Base + Dev 3 Cope Street / Shared Zone]

+ Network: N101 [PM Base + Prop Dev (2036)]

New Site Site Category: (None) Giveway / Yield (Two-Way)

Movement Performance - Vehicles														
Mov ID	Turn	Demand	Flows	Arrival		Deg. Satn	Average Delay	Level of Service	95% Back Queue		Prop. Queued	Effective Stop	Aver. No.	Averag e
		Total veh/h		Total veh/h	HV %	v/c	sec		Vehicles Dis veh	tance m		Rate	Cycles	Speed km/h
South	South: Cope St (S)													
1	L2	13	0.0	13	0.0	0.060	4.4	LOS A	0.0	0.0	0.00	0.06	0.00	47.0
2	T1	111	1.0	111	1.0	0.060	0.0	LOS A	0.0	0.0	0.00	0.06	0.00	44.6
Appro	oach	123	0.9	123	0.9	0.060	0.5	NA	0.0	0.0	0.00	0.06	0.00	45.7
North	: Cope	St (N)												
8	T1	91	2.3	91	2.3	0.053	0.3	LOS A	0.2	1.0	0.11	0.17	0.11	35.9
9	R2	32	0.0	32	0.0	0.053	4.9	LOS A	0.2	1.0	0.11	0.17	0.11	44.9
Appro	oach	122	1.7	122	1.7	0.053	1.5	NA	0.2	1.0	0.11	0.17	0.11	41.1
West	: Share	ed Zone (W	/)											
10	L2	8	0.0	8	0.0	0.009	4.8	LOS A	0.0	0.2	0.20	0.50	0.20	43.7
12	R2	3	0.0	3	0.0	0.009	5.2	LOS A	0.0	0.2	0.20	0.50	0.20	43.7
Appro	oach	12	0.0	12	0.0	0.009	4.9	LOS A	0.0	0.2	0.20	0.50	0.20	43.7
All Ve	hicles	257	1.2	257	1.2	0.060	1.2	NA	0.2	1.0	0.06	0.13	0.06	42.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Attachment 2 - Southern Loading Dock Swept Path Assessment

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