Our Ref: 18289

1 November 2022

ERM
Level 15,
309 Kent Street
Sydney NSW 2000

## Attention: Lucy Baker

Dear Lucy

## RE: HILLS OF GOLD WIND FARM - TRAFFIC AND TRANSPORT ASSESSMENT ADDENDUM TWO - AMENDED CONSTRUCTION PROPOSAL

As requested, please find herein The Transport Planning Partnership (TTPP)'s traffic and transport assessment for the Hills of Gold Windfarm amended project and transport route. This letter is an addendum to the previous traffic and transport assessment ${ }^{1}$ as part of the Environmental Impact Assessment and the subsequent response to submissions report².

Since the preparation and submission of the Response to Submissions report the proposed project has been amended and this addendum addresses the traffic and transport related to two issues, namely:

- The revised routes for heavy vehicles and over-sized over mass (OSOM) vehicles between Nundle and the project site.
- The option to source quarry material from a new quarry located within the NSW Forestry Land with access from Barry Road.

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## Background

The Hills of Gold Wind Farm is a project to install up to 64 Wind Turbine Generators (WTG) on land south-east of the township of Nundle in NSW.

A traffic and transport impact assessment was prepared for the Hills of Gold Windfarm and subsequent response to submissions report was also prepared. Previous reports include:

- Hills of Gold Wind Farm - Traffic and Transport Assessment - 12 November 2020 (TTPP)
- Hills of Gold Wind Farm - Traffic and Transport Addendum - 15 October 2021 (TTPP)

This document has been prepared to address the impacts on traffic and transport of amendments to the original plan related to vehicle routes near Nundle and sourcing of quarry material.

The route selection for the oversized and over mass (OSOM) vehicles has been further refined for the routes from Nundle to the site. The key change is that the proposed bypass road of "Devils Elbow", for use of Barry Road for Oversize Overmass vehicles, has now been removed from the scheme. The alternative OSOM routes now focus on accessing the site via Crawney Road. Vehicles returning from site that are configured as standard trucks (ie. OSOM packed down to a standard non-OSOM vehicle) would be able to use Barry Road on the return trip.

An option has been identified for a potential project quarry located within the Nundle State Forest. This has been assessed for information, however the original assumption for sourcing quarry materials at commercial locations has been retained as a worst-case for the purposes of planning assessment. Sourcing material from a quarry closer to the site would reduce the length of haulage routes for construction vehicles associated with the project and thus reduce the potential impacts to the road network and its users.

Overall, the proposed amendments to the proposal are expected to reduce the impacts on the road network compared to the previous proposal.

The amended proposal reduces the impacts on the road network through:

- Reduction in traffic forecast to use Barry Road and Morrisons Gap Road.
- The project will no longer require the Devils Elbow upgrade.
- Reducing the number of upgrades required for Morrisons Gap Road.
- Reducing the number of vehicles forecast to use Barry Road which would reduce the risk of crashes on this road.
- Reducing the number of vehicles forecast to travel through the centre of Nundle due to the portion of traffic proposed to use the new Route 2 Nundle Bypass (irrespective of the positive impact reduction of the optional quarry, should that be also utilised).

Further, it is forecast that this quarry option would significantly reduce the daily number of trucks travelling through Nundle compared to previous assessments.

## Revised Routes

Due to the limitations and concerns expressed by Tamworth Regional Council in their letter dated 25 May 2022, with the upgrade of the 'Devils Elbow' in order to use Barry Road for OSOM and upgrades proposing retaining walls on Morrisons Gap Road, alternative route options have been assessed and identified for the project. The intent of these changes is to reduce impacts for Tamworth Regional Council and residents previously impacted by the works and traffic associated with the previous route. Access to site via Head Peel Road was assessed but is not proposed due to the limited potential required upgrades on this road would bring the community. As such the Proponent assessed a new route using Crawney Road to access the site from the West.

The following sections describes the proposed routes and vehicles that would use them. Most of these routes have previously been described in the ElS document with a new route extended along Crawney Road.

The assessment of the physical ability of trucks to make turns and the required additional hardstand has been assessed by the logistics company, Rex J Andrews Transport and would form part of the future detailed traffic management plan. Figure 1 provides an indication of the types of OSOM vehicles proposed.

## ttpp

Figure 1: Typical Oversized Over Mass Vehicles


Source Rex J Andrews Transport

## Route 1 Nundle Blade Route (Blade Option 1)

The Nundle Blade Route (Option 1) was previously assessed in the Environmental Impact Assessment. This route would be used for the blade components only. It overcomes the limitations in road geometry that prevents blades from turning right from Oakenville Street into Jenkins Street. The route (see Figure 2) is a loop through Oakenville Street, Old Hanging Rock Road, Happy Valley Road, River Road, Jenkins Street then south on Crawney Road. The route would require additional hardstand to facilitate turns at:

- Oakenville Street and Old Hanging Rock Road
- Old Hanging Rock Road and Happy Valley Road
- Happy Valley Road to River Road.

OSOM vehicle movements would be under traffic control for key turns along the route.
Figure 2: Route 1 Nundle Blade Route


## Route 2 Nundle Bypass Route

The Nundle Bypass route (see Figure 3) would divert large vehicles from Oakenville Street onto Herring Street and Innes Street and then a right turn on to Jenkins Street and Crawney Road. This route would be used for all OSOM vehicles other than the blade routes.

Figure 3: Route 2 Nundle Bypass


An alternative option for blades (Blade Option 2) is being explored and would have blades utilising private property in an amendment of the Route 2 Nundle Bypass. This route may require removing an existing building and require a new hardstand. Swept paths for the alternative blade route are shown in Figure 4.

Figure 4: Alternative Blade Route (Blade Option 2)


## Route 3 Morrisons Gap Road via Barry Road

The route to site via Barry Road and Morrisons Gap Road (see Figure 5) remains to be utilised by general construction traffic and heavy vehicles. The route could potentially be used for some OSOM vehicles that have shorter loads and lower weight, such as the WTG blade tips and hubs. This route requires no additional infrastructure on Barry Road. Minor widening upgrades to Morrisons Gap Road would be required to accommodate the larger vehicles and create an opportunity for improved road management during construction use and as a long term improvement for road users. However, it has been assumed that no OSOM vehicles will use this route on the way to the site as part of this assessment.

Most returning OSOM vehicles can be 'packed down'. This means that trailers for OSOM vehicles can be reduced to standard sized trailers and therefore are able to make the return trip along this route using Morrisons Gap Road and Barry Road. This would promote a positive 'circular' flow of OSOM project traffic and spread the traffic impact on the local roads.

Figure 5: Route 3 Morrisons Gap Road via Barry Road


## Route 4 Crawney Road Extension

Previously the assessment had only considered a route that used Head of the Peel Road off Crawney Road. The amended route will continue past the Head of the Peel Road turn-off and use approximately 5.8 km of existing road down to the Option A site entrance, or 8.6 km down to Option C, as shown in Figure 6 (in red). Crawney Road continues as sealed rural road that is some 7.5 km (with unsealed shoulders) up until Back Creek Bridge, following which it is an unsealed road for approximately 2.1 km to the proposed site entrance (Option A).

The three options are currently identified for the access from Crawney Road are shown in Figure 7. OSOM vehicles turning from Crawney Road would be under traffic control.

Figure 6: Route 4 Crawney Road Extension


Figure 7: Route 4 Crawney Road Access Options


Source: ERM
transport planning

## Oversized Over Mass Volumes and Routes

The routes available by vehicle type are presented in Table 1.
Table 1: Estimated number of OSOM Trips

| Component | Vehicle Type | Total Number of Trips | Route 1 <br> Nundle Loop | Route 2 <br> Nundle <br> Bypass | Route 3 Barry Road | Return Trip Via Route 3 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Blades (root section) | Prime Mover with extender Blade Trailer | 192 | $\checkmark$ | $\checkmark^{*}$ | $x$ | $\checkmark$ |
| Blades (tip section)** | Prime Mover with Platform Trailer | 64 | $x$ | $\checkmark$ | $\checkmark$ | $\checkmark$ |
| Nacelles | Prime Mover with Platform Trailer | 64 | $x$ | $\checkmark$ | $x$ | $x$ |
| Drivetrain | Prime Mover with Platform Trailer | 64 | $x$ | $\checkmark$ | $x$ | $\checkmark$ |
| Hubs | Prime Mover with Platform Trailer | 64 | $x$ | $\checkmark$ | $\checkmark$ | $\checkmark$ |
| Tower Sections | Prime Mover Platform Trailer and Dolly Jinker | 448 | $x$ | $\checkmark$ | $x$ | $x$ |
| Other (40ft Container) | Prime Mover with Platform Trailer | 128 | $x$ | $\checkmark$ | $x$ | $\checkmark$ |
| Substation | Prime Mover with Platform Trailer | 20 | $x$ | $\checkmark$ | x | $\checkmark$ |
| Switching Station | Prime Mover with Platform Trailer | 20 | $x$ | $\checkmark$ | $x$ | $\checkmark$ |
| Overhead Cabling | Prime Mover with Platform Trailer | 120 | $x$ | $\checkmark$ | $x$ | $\checkmark$ |
| Underground Cabling | Prime Mover with Platform Trailer | 20 | $x$ | $\checkmark$ | $x$ | $\checkmark$ |
| Battery System | Standard SemiTrailer | 158 | $x$ | $\checkmark$ | $x$ | $\checkmark$ |
| Mobile Batch Plant | Standard SemiTrailer | 2 | $x$ | $\checkmark$ | $x$ | $\checkmark$ |
| Transformer | Low Loader | 2 | $x$ | $\checkmark$ | $x$ | $x$ |

*Potential route requiring new hardstand
** Blade (tip section) - these movement would only occur if blades are split into two units. Note: Three tips can be transported on one truck

All vehicles could use Route 2 however, the blade root sections would require the additional hard stand as discussed in the description of Route 2. Hubs and blade tips could use either the southern route via Crawney Road or the Route 3 via Barry Road but are assumed to use Route 2 exclusively in this assessment. Noting that Route 3 was previously assessed for all OSOM traffic.

Most vehicles returning from site could also use either Crawney Road or Barry Road with exception of the trailers for the Nacelles, Tower Sections and Transformers.

The estimated total number of OSOM vehicle trips during the 9 months it is expected to take to transport components to site are shown in Table 2 and Table 3 for trips to site and from site respectively.

Table 2: Estimated Practical Number of OSOM Trips to Site by Route

| Route | Name | Percentage | Number of Trips (one-way) |
| :--- | :---: | :---: | :---: |
| Route 1 | Nundle Loop | $100 \%$ | 192 |
| Route 2 | Nundle Bypass | $100 \%$ | 1174 |
| Route 3 | Barry Road | $0 \%$ | - |
| Route 4 | Crawney Road | $100 \%$ | 1366 |
| Total |  |  | 1366 |

Table 3: Estimated Practical Number of Trips from Site

| Route | Name | Percentage | Number of Trips (one-way) |
| :--- | :---: | :---: | :---: |
| Route 1 | Nundle Loop | $0 \%$ | - |
| Route 2 | Nundle Bypass | $100 \%^{* *}$ | 940 |
| Route 3 | Barry Road | $50 \%^{*}$ | 426 |
| Route 4 | Crawney Road | $100 \%^{* *}$ | 940 |
| Total |  |  | 1366 |

*50\% of the potential trips to use Barry Road to Return
** $100 \%$ of trips that can only return via Crawney Road plus $50 \%$ trips that could return in pack down via Barry Road
Transportation of the components is forecast to be undertaken over a period of approximately 9 months. This equates to a total of 1366 OSOM movements to site over 9 months or an average of some 6 movements per day assuming 6 available transport days per week.

The timing of these trips would be determined after the transport and logistics contractor is engaged for the project, and after necessary OSOM vehicle transport permits are consulted including required vehicle escorts. It is recommended the traffic management plan adopt a policy of avoiding school peaks in Nundle. Some delays may be experienced while the larger blade roots make turns under traffic control and may require public vehicles to stop for approximately 1 minute, based on reviewing videos of similar manoeuvres for other wind farm projects in Australia. However, this is only expected to occur 5 times a week based on 192 trips over 36 weeks.

Returning vehicles in pack down are not expected to need traffic control and would have less impact on the road network.

To further reduce impacts, hardstand laybys will be provided to allow for passing of wide vehicles during the movement of OSOM vehicles. There are proposed to be three (3) laybys on Crawney Road, three (3) on Barry Road and one (1) on Morrisons Gap Road. The location of the laybys are shown in Figure 8.

Figure 8: Layby Locations


## Construction Related Traffic

In addition to the OSOM trips there are estimated to be some 311 light vehicle and regular trucks return daily trips generated by the site as presented in the response to submissions report. Most of these trips would be light vehicles to and from site in the morning and afternoons. The estimated number of trips during the morning peak are shown in Table 4, and evening peak is assumed to be the inverse.

Table 4: Morning Peak Traffic Generation Estimate

| Table Heading | Units | Morning to Site <br> (trips) | Morning from site <br> (trips) | Morning total <br> (trips) | Daily <br> (trips) |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Light vehicles | 174 workers | 70 | 15 | 85 | 155 |
| Buses | - | - | - | - | - |
| Water trucks | 15 per day | 3 | 3 | 6 | 30 |
| Trucks | 63 per day | 7 | 7 | 14 | 126 |
| Total | - | 80 | 25 | 105 | 311 |

With the new construction proposal there will be a change in the forecast directional splits from Nundle for the general construction traffic. As a practical scenario for the purposes of assessment, it is assumed that:

- $65 \%$ of construction traffic (excluding OSOM) would use Barry Road.
- $35 \%$ of construction traffic (excluding OSOM) would use Crawney Road.

The previous assessment assumed nearly $100 \%$ of traffic would use Barry Road. The new forecasts estimate that more traffic will use the southern route to Crawney Road due to the following reasons:

- The western access provides a shorter route for vehicles required to work in the western area of the wind farm
- The proposed Optional substation and associated transmission line works will require use of the Crawney Road to access the construction area
- Proposed ancillary infrastructure such as laydown areas, batching plants and temporary site facilities have been proposed to create greater construction flexibility.

The resultant peak hour traffic volumes on the two routes are shown in Table 4 with the afternoon peak estimated to be the inverse.

Table 5: Traffic Splits Morning Peak Hour

| Table Heading | Route via Barry Road <br> (vehicles per hour) |  | Route via Crawney Road <br> (vehicles per hour) |  |
| :--- | :---: | :---: | :---: | :---: |
|  | To Site | From Site | To Site | From Site |
| Light Vehicles | 46 | 6 | 25 | 4 |
| Heavy Vehicles | 6 | 6 | 4 | 4 |
| Total | 52 | 12 | 29 | 8 |
| Combined Two-Way |  | 64 |  | 37 |

The revised estimated traffic volumes at the intersection of Oakenville Street and Jenkins Street are presented in Figure 9 and Figure 10 for the morning and evening peak respectively.

Figure 9: Morning Peak Forecast Traffic Volumes (Construction)

Existing


Construction Traffic


Forecast Traffic Volumes


Figure 10: Evening Peak Forecast Traffic Volumes (Construction)

Existing


Construction Traffic


Forecast Traffic Volumes


The forecast traffic volumes are relatively low. Previous traffic modelling shows that there is capacity at the intersection and that the forecast construction traffic would not have significant impact on the operation of the intersection. Further volumes are less than 100 vehicles per hour two-way on all approaches which is much lower than the environmental capacity of 300 vehicles per hour which is considered the level of traffic on local streets above which amenity is adversely affected.

The amended traffic volumes would reduce traffic to Morrisons Gap Road compared to the previous assessment. It was assessed that the warrant for a channelised right turn lane was not met in the previous assessment. Therefore, in accordance with Austroads it is still recommended that a Basic Right Turn treatment is provided for all intersections that do not meet the warrant for a channelised right turn treatment.

Sidra Intersection models were updated and results are presented in the Table 6. The modelling indicates that there would be minimal impact on the intersection of Oakenville Street and Jenkins Street.

Table 6: Updated Sidra Model Results

| Intersection | Period | Existing |  | Existing + Construction Traffic |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  | Ave. Delay (sec) | LoS* | Ave. Delay (sec) | LoS* |  |
|  | Evening Peak | 6 | A | 7 | A |

*Level of Service - Based on the intersection delay and criteria from RTA Guide to Traffic Generation 2002. Level of Service ranges from A to $F$. A is free flow traffic minimal delay. $F$ is above capacity.

There are no proposed changes from the previous assessment at the other intersections.

## Midblock Capacity Assessment

The midblock capacity assessment has been updated from the response to submissions paper based on the latest travel demand splits.

The assessment of traffic capacity has been based on the volume capacity ratio (V/C), rural road level of service and the Environmental Capacity for urban areas based on the RTA (2002) Guide to Traffic Generating Development.

The V/C ratio indicates the level of congestion by comparing the forecast traffic volumes to the theoretical lane capacity. For this assessment, the rural roads are assumed to have a capacity of 1000 vehicles / hour / lane. As V/C ratios approach 0.9 it should be expected that flow would become significantly interrupted.

To account for overtaking, the level of service can be estimated based on the RTA Guide to Traffic Generating Developments table for the rural roads this is shown in Table 7. This is generally applicable for two-way two lane rural roads with a $100 \mathrm{~km} / \mathrm{h}$ speed limit.

Table 7: Rural Road Level of Service

| Terrain | Level of Service | Percent of Heavy Vehicles |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: |
|  |  | $\mathbf{0}$ | $\mathbf{5}$ | $\mathbf{1 0}$ | $\mathbf{1 5}$ |
|  | B | 630 | 590 | 560 | 530 |
|  | C | 1030 | 970 | 920 | 870 |
|  | Rolling | D | 1630 | 1550 | 1480 |
| Mountainous | E | 2630 | 2500 | 2390 | 2290 |
|  | B | 500 | 420 | 360 | 310 |
|  | C | 920 | 760 | 650 | 570 |
|  | D | 1370 | 1140 | 970 | 700 |
|  | E | 2420 | 2000 | 1720 | 1510 |
|  | B | 340 | 230 | 180 | 150 |
|  | C | 600 | 410 | 320 | 260 |
|  | D | 1050 | 680 | 500 | 400 |
|  | E | 2160 | 1400 | 1040 | 820 |

Source: RTA Guide to Traffic Generating Developments
The desirable Level of Service as recommended by the RTA Guide to Traffic Generating Developments is Level of Service C.

The environmental capacity is an assessment of the impact on the amenity of an environment. The environmental capacities are estimated by considering a range of differing
transport planning
perceptions of traffic impacts in a particular area. The assessment has used the tables provided in the RTA Guide to Traffic Generating Developments as shown in Table 8.

Table 8: Environmental Capacity

| Road Class | Road Type | Maximum Speed <br> $(\mathrm{km} / \mathrm{hr})$ | Maximum peak hour <br> volume <br> $(\mathrm{veh} / \mathrm{hr})$ |
| :--- | :---: | :---: | :---: |
|  | Access way | 25 | 100 |
|  | Street | 40 | 200 environmental goal |
|  | Street |  <br> Collector | 50 |

Source: RTA Guide to Traffic Generating Developments
The traffic impacts during construction have been analysed based on a scenario without the proposed car park and shuttle bus. The results of the analysis are shown in Table 9. The table presents the existing traffic volumes and corresponding Volume Capacity ( $V / C$ ) ratio and the forecast traffic volumes and V/C during the construction period. The V/C ratio is the comparison of the traffic volumes to the theoretical capacity. This assumes all traffic would go to Morrisons Gap Road.

Table 9: Peak Construction Period Generation and V/C Ratio

| Road | Period | Direction | Existing Volumes |  | With Construction Volumes |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | Volume (vph) | V/C | Project Generated traffic (vph) | Total Volume (vph) | V/C |
| Nundle Road (north of Lindsays Gap Road) | Morning Peak | Northbound | 19 | 0.019 | 17 | 36 | 0.036 |
|  |  | Southbound | 13 | 0.013 | 54 | 67 | 0.067 |
|  | Evening Peak | Northbound | 6 | 0.006 | 54 | 60 | 0.060 |
|  |  | Southbound | 9 | 0.009 | 17 | 26 | 0.026 |
| Lindsays Gap Road | Morning Peak | Northbound | 3 | 0.003 | 24 | 27 | 0.027 |
|  |  | Southbound | 9 | 0.009 | 8 | 17 | 0.017 |
|  | Evening Peak | Northbound | 13 | 0.013 | 8 | 21 | 0.021 |
|  |  | Southbound | 5 | 0.005 | 24 | 29 | 0.029 |
| Nundle Road (between Lindsay Gap Road and Nundle) | Morning Peak | Westbound | 20 | 0.02 | 25 | 45 | 0.045 |
|  |  | Eastbound | 30 | 0.03 | 78 | 108 | 0.108 |
|  | Evening Peak | Westbound | 16 | 0.016 | 78 | 94 | 0.094 |
|  |  | Eastbound | 17 | 0.017 | 25 | 42 | 0.042 |
| From Nundle to Project Area (Oakenville Street, Barry Rd, Morrions Gap Road) | Morning Peak | Westbound | 21 | 0.021 | 17 | 38 | 0.038 |
|  |  | Eastbound | 19 | 0.019 | 54 | 73 | 0.073 |
|  | Evening Peak | Westbound | 24 | 0.024 | 18 | 42 | 0.042 |
|  |  | Eastbound | 27 | 0.027 | 53 | 80 | 0.080 |
| From Nundle to site via Crawney Road | Morning Peak | Northbound | 32 | 0.032 | 9 | 41 | 0.041 |
|  |  | Southbound | 20 | 0.020 | 29 | 49 | 0.049 |
|  | Evening Peak | Northbound | 33 | 0.033 | 30 | 63 | 0.063 |
|  |  | Southbound | 24 | 0.024 | 10 | 34 | 0.034 |

The analysis indicates that the forecast volumes would not have a significant impact on the road network efficiency with V/C ratios less than 0.2 on all roads that were assessed.

The Level of Service based on the criteria in Table 7 is shown in Table 10. This is based on assuming $15 \%$ heavy vehicles, rolling terrain or mountainous for Oakenville Street (Barry Road).

Table 10: Midblock Level of Service

| Road | Period | Existing Volumes |  | With Construction Volumes |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Volume (vph) | Level of Service | Total Volume (vph) | Level of Service |
| Nundle Road North of Lindsays Gap Road | Morning | 32 | A | 103 | A |
|  | Evening | 15 | A | 86 | A |
| Lindsays Gap Road | Morning | 32 | A | 44 | A |
|  | Evening | 15 | A | 50 | A |
| Nundle Road | Morning | 50 | A | 153 | A |
|  | Evening | 33 | A | 136 | A |
| Oakenville Street (Barry Road) | Morning | 40 | A | 111 | A |
|  | Evening | 51 | A | 122 | A |
| Jenkins Street (Crawney Road) | Morning | 52 | A | 90 | A |
|  | Evening | 57 | A | 97 | A |

The assessment shows that almost all the roads would operate at Level of Service A during the peak of construction.. In all cases the level of service is equal or better than the Level of Service A.

In terms of environmental capacity, the forecast volumes would be less than the maximum 300 vehicles for collector roads and less than 200 vehicles per hour for local roads. Thus, the Project related traffic would operate within environmental capacity guidelines.

## Transmission Line Access

Construction of the transmission lines to support the project will be constructed over an estimated 9 month period. A 330kV overhead transmission line will connect the onsite substation to the existing 330 kV TransGrid Liddell to Tamworth overhead transmission line network, located approximately 21 km west of the substation. A switching station will be constructed to connect the Project to the 330kV TransGrid Liddell to Tamworth line. The transmission line location is shown in Figure 11.

Figure 11: Transmission Line Location


Access to the switching station for construction would be via local roads including Old Wallabadah Road, Wallabadah Creek Road and Basin Creek Road. These roads are unsealed access roads. The estimated construction traffic using these roads are expected to be low compared to the rest of the project.

Notwithstanding, the condition and impacts on the local roads would be assessed by the project as part of a dilapidation report and any impacts on the pavement would be remediated and returned to existing condition at the completion of the project.

## New Quarry Location

An option is being considered where quarry material could be sourced from a new quarry located within the Nundle State Forest. The route from the proposed quarry to the site is shown in Figure 12. This quarry remains an option subject to further assessment for extent of project viability, and thus the previous traffic assessment has been maintained on a worstcase of imported quarry materials from commercial locations. However, to demonstrate the reduction in traffic on local roads should this quarry be determined viable, the following assessment is provided for information purposes.

Figure 12: Route From Proposed Quarry to Site


There are two haulage routes from the proposed quarry to site:

1. Via Barry Road and Morrisons Gap road, accessing the north of the project (shown in red); and
2. Via Barry Road travelling west towards Nundle and down Crawney Road to the southern site entrance (shown in blue). It is suggested that during the preparation of the projects Traffic Management Plan, Tamworth Regional Council is consulted to confirm their preference for these trucks either turning off Oakenville Street directly into Jenkins Street, or alternatively using Herron, Innes, Jenkins Street.

For the purpose of this assessment, it is assumed $65 \%$ of quarry traffic would use the Option 1 route to the north, and $35 \%$ of traffic would use the Option 2 to the south/west.

Should the quarry become operational then it is suggested that it would operate from 7am 6:00pm. The vehicles used to transport the rock material would be standard truck and dog vehicles.

It is estimated that in peak operation that there would be up to 14 truck trips per hour. That is 7 trips to and 7 trips from the site. Using this site would minimise the impacts of the project by
sourcing material near the site and overall having the benefit of removing truck movements through Nundle and other local roads.

The ability to source material from this quarry could reduce the number of truck movements through Nundle by up to 40 truck trips a day (a trip is considered a one-way journey) assuming $65 \%$ of the 63 truck trips previously forecast would use Morrisons Gap Road.

A review of the 5 year crash history from the Centre for Road Safety indicates that there have been no crashes recorded on the route from the quarry to the site via Morrisons Gap Road in the period from 2017 to 2021 inclusive. There were four crashes recorded in the same period along Barry Road. This included three serious crashes and one fatal crash that occurred in 2019. All crashes involved single vehicles running off the road on bends and hitting an object. The crash locations are shown in Figure 13. There were no crashes recorded on Crawney Road.

Figure 13: Crash Locations on Barry Road


However, the roads are currently used extensively by B-Double 'jinker' trucks associated with the NSW Forestry and are therefore considered suitable for heavy vehicles. Further, it is noted that the amended proposal reduces the forecast number of general traffic to use Barry Road.

To mitigate road safety issues, the recommendations in the EIS traffic and transport assessment to address driver training and making drivers aware of local conditions such as ice on the road should be implemented to reduce potential hazards.

Intersection of Forrest Way and Barry Road has been used by NSW Forestry trucks in the past 5 years with no recorded crashes in the five year crash history. Forestry NSW operate four trucks an hour each way using the intersect. The intersection is a priority cross intersection with priority along Barry Road. The intersection is shown in Figure 14. The safe intersection sight distance for $80 \mathrm{~km} / \mathrm{h}$ is 180 m and is achieved through the clearing of vegetation.

Figure 14: Intersection of Barry Road / Forest Way / Hanging Rock Lookout Road


The trees around the intersection on the Forest Way have been cleared to provide better sight distance to the intersection and warning signs are provided on the western approach to the intersection. The intersection showing the clearing is shown in Figure 15.

Figure 15: Intersection of Barry Road and Forest Way (Barry Road looking West)


The estimated traffic volumes both existing and proposed when the quarry would be use is shown in Figure 16 and Figure 17.
transport planning

Figure 16: Morning Peak Forecast Traffic Volumes Barry Road and Forest Way


Figure 17: Evening Peak Peak Forecast Traffic Volumes Barry Road and Forest Way

Existing


Construction Traffic


Forecast Traffic Volumes


The volumes have been tested in a Sidra Intersection model and found to have minimal impact on the intersection operation. The results of the Sidra modelling are shown in Table 11.

Table 11: Sidra Model Results Barry Road and Forest Way

| Intersection | Period | Existing |  | Existing + Construction Traffic |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Ave. Delay (sec) | LoS* | Ave. Delay (sec) | LoS* |
| Barry Road and <br> Forest Way | Morning Peak | 7 | A | 7 | A |
|  | Evening Peak | 7 | A | 7 | A |

*Level of Service - Based on the intersection delay and criteria from RTA Guide to Traffic Generation 2002. Level of Service ranges from A to F. A is free flow traffic minimal delay. $F$ is above capacity.

The intersection of Barry Road and Forest Way is considered appropriate for proposed quarry vehicles under this option.

## Summary and Conclusion

The Transport Planning Partnership has assessed the proposed amendments to the construction and transport operations for the Hills of Gold Wind Farm project. Overall the proposed amendments to the proposal are expected to reduce the impacts on the road network compared to the previous proposal.

The amendments include adjusted routes and for OSOM vehicles with the focus on the trips travelling to the south via Crawney Road. The assessment indicates that the new OSOM route would not have significant additional impacts compared with the previous scheme.

The forecast for light vehicles and general construction vehicles would now be split between Crawney Road and Morrisons Gap Road with $35 \%$ going south to Crawney Road and $65 \%$ using Morrisons Gap Road. The revised assessment shows that the capacity of the road network would be unaffected and that the volumes are significantly below the prescribed environmental capacities.

The amended proposal reduces the impacts on the road network through:

- Reduction in traffic forecast to use Barry Road and Morrisons Gap Road.
- The project will no longer require the Devils Elbow upgrade.
- Reducing the number of upgrades required for Morrisons Gap Road.
- Reducing the number of vehicles forecast to use Barry Road which would reduce the risk of crashes on this road.
- Reducing the number of vehicles forecast to travel through the centre of Nundle due to the portion of traffic proposed to use the new Route 2 Nundle Bypass (irrespective of the positive impact reduction of the optional quarry, should that be also utilised).

An option to source quarry material from the Nundle State Forest is being considered. This option would reduce the haulage route distance for this material and reduce the impacts or the project on the broader road network. It is forecast that this quarry option would significantly reduce the daily number of trucks travelling through Nundle compared to previous assessments.

An assessment of the intersection of Barry Road and Forest Way which would be used for the quarry in the state forest has found that the intersection is appropriate.

We trust the above is to your satisfaction. Should you have any queries regarding the above or require further information, please do not hesitate to contact the undersigned on 84377800.

Yours sincerely,
G, hown

## Stephen Read

## Associate

transport planning

## Attachment One

## Updated Sidra Outputs

## MOVEMENT SUMMARY

$\nabla$ Site: 101 [2021 PM Oakenville Rd - Jenkins St (Site Folder:
Project Scenario)]
New Site
Site Category: (None)
Give-Way (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { INP } \\ & \text { VOLU } \\ & \text { [ Total } \\ & \text { veh/h } \end{aligned}$ | $\begin{aligned} & \text { JT } \\ & \text { MES } \\ & \text { HV ] } \\ & \% \end{aligned}$ |  | $\begin{aligned} & \text { IND } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn <br> v/c | Aver. Delay sec $\qquad$ | Level of Service | $\begin{gathered} \text { 95\% B B } \\ \text { Q } \\ \text { [ Veh. } \\ \text { veh } \end{gathered}$ | $\begin{gathered} \text { CK OF } \\ \text { UE } \\ \text { Dist ] } \\ \text { m } \end{gathered}$ | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. Speed $\mathrm{km} / \mathrm{h}$ |
| South: Oakenville Street |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 31 | 2.0 | 33 | 2.0 | 0.026 | 5.8 | LOSA | 0.1 | 0.7 | 0.15 | 0.54 | 0.15 | 53.1 |
| 2 T1 | 9 | 2.0 | 9 | 2.0 | 0.042 | 5.6 | LOSA | 0.2 | 1.2 | 0.29 | 0.55 | 0.29 | 53.2 |
| 3 R2 | 24 | 2.0 | 25 | 2.0 | 0.042 | 6.7 | LOSA | 0.2 | 1.2 | 0.29 | 0.55 | 0.29 | 52.9 |
| Approach | 64 | 2.0 | 67 | 2.0 | 0.042 | 6.1 | LOS A | 0.2 | 1.2 | 0.22 | 0.54 | 0.22 | 53.0 |
| East: Oakenville Street |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 12 | 2.0 | 13 | 2.0 | 0.007 | 5.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.58 | 0.00 | 53.5 |
| 5 T1 | 59 | 15.0 | 62 | 15.0 | 0.042 | 0.0 | LOSA | 0.1 | 0.5 | 0.03 | 0.08 | 0.03 | 59.1 |
| 6 R2 | 9 | 2.0 | 9 | 2.0 | 0.042 | 5.6 | LOS A | 0.1 | 0.5 | 0.03 | 0.08 | 0.03 | 57.1 |
| Approach | 80 | 11.6 | 84 | 11.6 | 0.042 | 1.5 | NA | 0.1 | 0.5 | 0.02 | 0.15 | 0.02 | 58.0 |
| North: Oakenville Street |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 9 | 2.0 | 9 | 2.0 | 0.007 | 5.6 | LOSA | 0.0 | 0.2 | 0.08 | 0.55 | 0.08 | 53.3 |
| 8 T1 | 7 | 2.0 | 7 | 2.0 | 0.020 | 5.6 | LOSA | 0.1 | 0.6 | 0.30 | 0.54 | 0.30 | 53.4 |
| 9 R2 | 9 | 2.0 | 9 | 2.0 | 0.020 | 6.8 | LOSA | 0.1 | 0.6 | 0.30 | 0.54 | 0.30 | 52.7 |
| Approach | 25 | 2.0 | 26 | 2.0 | 0.020 | 6.0 | LOS A | 0.1 | 0.6 | 0.22 | 0.54 | 0.22 | 53.1 |
| West: Oakenville Street |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 5 | 2.0 | 5 | 2.0 | 0.003 | 5.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.58 | 0.00 | 53.5 |
| 11 T1 | 21 | 15.0 | 22 | 15.0 | 0.024 | 0.2 | LOS A | 0.1 | 0.7 | 0.14 | 0.23 | 0.14 | 57.1 |
| 12 R 2 | 15 | 2.0 | 16 | 2.0 | 0.024 | 5.9 | LOSA | 0.1 | 0.7 | 0.14 | 0.23 | 0.14 | 55.7 |
| Approach | 41 | 8.7 | 43 | 8.7 | 0.024 | 2.9 | NA | 0.1 | 0.7 | 0.12 | 0.27 | 0.12 | 56.1 |
| All Vehicles | 210 | 7.0 | 221 | 7.0 | 0.042 | 3.7 | NA | 0.2 | 1.2 | 0.13 | 0.34 | 0.13 | 55.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^1]
## MOVEMENT SUMMARY

$\nabla$ Site: 101 [2021 PM Oakenville Rd - Jenkins St (Site Folder:
Existing)]
New Site
Site Category: (None)
Give-Way (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  |  |  | $\begin{aligned} & \text { IND } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn v/c | Aver. Delay sec | Level of Service |  | CK OF UE Dist ] | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. Speed km/h |
| South: Oakenville Street |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 3 | 2.0 | 3 | 2.0 | 0.002 | 5.6 | LOS A | 0.0 | 0.1 | 0.04 | 0.56 | 0.04 | 53.4 |
| 2 T1 | 9 | 2.0 | 9 | 2.0 | 0.035 | 5.0 | LOSA | 0.1 | 1.0 | 0.16 | 0.52 | 0.16 | 53.7 |
| 3 R2 | 22 | 2.0 | 23 | 2.0 | 0.035 | 6.0 | LOSA | 0.1 | 1.0 | 0.16 | 0.52 | 0.16 | 53.3 |
| Approach | 34 | 2.0 | 36 | 2.0 | 0.035 | 5.7 | LOS A | 0.1 | 1.0 | 0.15 | 0.52 | 0.15 | 53.4 |
| East: Oakenville Street |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 11 | 2.0 | 12 | 2.0 | 0.006 | 5.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.58 | 0.00 | 53.5 |
| 5 T1 | 7 | 15.0 | 7 | 15.0 | 0.011 | 0.0 | LOS A | 0.0 | 0.3 | 0.05 | 0.32 | 0.05 | 56.7 |
| 6 R2 | 9 | 2.0 | 9 | 2.0 | 0.011 | 5.6 | LOSA | 0.0 | 0.3 | 0.05 | 0.32 | 0.05 | 54.9 |
| Approach | 27 | 5.4 | 28 | 5.4 | 0.011 | 4.1 | NA | 0.0 | 0.3 | 0.03 | 0.43 | 0.03 | 54.8 |
| North: Oakenville Street |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 8 | 2.0 | 8 | 2.0 | 0.006 | 5.6 | LOS A | 0.0 | 0.2 | 0.03 | 0.56 | 0.03 | 53.4 |
| 8 T1 | 12 | 2.0 | 13 | 2.0 | 0.018 | 5.0 | LOS A | 0.1 | 0.5 | 0.15 | 0.50 | 0.15 | 54.0 |
| 9 R2 | 5 | 2.0 | 5 | 2.0 | 0.018 | 5.8 | LOSA | 0.1 | 0.5 | 0.15 | 0.50 | 0.15 | 53.3 |
| Approach | 25 | 2.0 | 26 | 2.0 | 0.018 | 5.3 | LOS A | 0.1 | 0.5 | 0.11 | 0.52 | 0.11 | 53.7 |
| West: Oakenville Street |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 5 | 2.0 | 5 | 2.0 | 0.003 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.58 | 0.00 | 53.5 |
| 11 T1 | 5 | 15.0 | 5 | 15.0 | 0.007 | 0.0 | LOSA | 0.0 | 0.2 | 0.07 | 0.30 | 0.07 | 56.7 |
| 12 R2 | 6 | 2.0 | 6 | 2.0 | 0.007 | 5.7 | LOSA | 0.0 | 0.2 | 0.07 | 0.30 | 0.07 | 55.4 |
| Approach | 16 | 6.1 | 17 | 6.1 | 0.007 | 3.9 | NA | 0.0 | 0.2 | 0.05 | 0.39 | 0.05 | 55.2 |
| All <br> Vehicles | 102 | 3.5 | 107 | 3.5 | 0.035 | 4.9 | NA | 0.1 | 1.0 | 0.09 | 0.48 | 0.09 | 54.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^2]
## MOVEMENT SUMMARY

$\nabla$ Site: 101 [2021 AM Oakenville Rd - Jenkins St (Site Folder:
Existing)]
New Site
Site Category: (None)
Give-Way (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ | $\begin{aligned} & \text { INF } \\ & \text { VOLI } \\ & \text { [ Total } \\ & \text { veh/h } \end{aligned}$ | $\begin{aligned} & \text { JT } \\ & \text { MES } \\ & \text { HV ] } \\ & \% \end{aligned}$ |  | $\begin{aligned} & \text { IND } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn <br> v/c | Aver. Delay sec $\qquad$ | Level of Service | $\begin{gathered} \text { 95\% B B } \\ \text { Q } \\ \text { [ Veh. } \\ \text { veh } \end{gathered}$ | $\begin{gathered} \text { CK OF } \\ \text { UE } \\ \text { Dist ] } \\ \text { m } \end{gathered}$ | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. Speed $\mathrm{km} / \mathrm{h}$ |
| South: Oakenville Street |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 11 | 2.0 | 12 | 2.0 | 0.009 | 5.6 | LOS A | 0.0 | 0.2 | 0.05 | 0.55 | 0.05 | 53.4 |
| 2 T1 | 13 | 2.0 | 14 | 2.0 | 0.022 | 4.9 | LOSA | 0.1 | 0.6 | 0.15 | 0.50 | 0.15 | 53.9 |
| 3 R2 | 8 | 2.0 | 8 | 2.0 | 0.022 | 5.9 | LOSA | 0.1 | 0.6 | 0.15 | 0.50 | 0.15 | 53.6 |
| Approach | 32 | 2.0 | 34 | 2.0 | 0.022 | 5.4 | LOS A | 0.1 | 0.6 | 0.11 | 0.52 | 0.11 | 53.7 |
| East: Oakenville Street |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 3 | 2.0 | 3 | 2.0 | 0.002 | 5.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.58 | 0.00 | 53.5 |
| 5 T1 | 10 | 15.0 | 11 | 15.0 | 0.010 | 0.0 | LOSA | 0.0 | 0.3 | 0.04 | 0.22 | 0.04 | 57.7 |
| 6 R2 | 6 | 2.0 | 6 | 2.0 | 0.010 | 5.6 | LOS A | 0.0 | 0.3 | 0.04 | 0.22 | 0.04 | 55.8 |
| Approach | 19 | 8.8 | 20 | 8.8 | 0.010 | 2.7 | NA | 0.0 | 0.3 | 0.04 | 0.28 | 0.04 | 56.4 |
| North: Oakenville Street |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 9 | 2.0 | 9 | 2.0 | 0.007 | 5.6 | LOSA | 0.0 | 0.2 | 0.03 | 0.56 | 0.03 | 53.4 |
| 8 T1 | 7 | 2.0 | 7 | 2.0 | 0.018 | 4.9 | LOSA | 0.1 | 0.5 | 0.15 | 0.52 | 0.15 | 53.8 |
| 9 R2 | 9 | 2.0 | 9 | 2.0 | 0.018 | 6.0 | LOSA | 0.1 | 0.5 | 0.15 | 0.52 | 0.15 | 53.1 |
| Approach | 25 | 2.0 | 26 | 2.0 | 0.018 | 5.5 | LOS A | 0.1 | 0.5 | 0.11 | 0.53 | 0.11 | 53.4 |
| West: Oakenville Street |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 8 | 2.0 | 8 | 2.0 | 0.005 | 5.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.58 | 0.00 | 53.5 |
| 11 T1 | 4 | 15.0 | 4 | 15.0 | 0.008 | 0.0 | LOS A | 0.0 | 0.2 | 0.06 | 0.37 | 0.06 | 56.2 |
| 12 R2 | 8 | 2.0 | 8 | 2.0 | 0.008 | 5.6 | LOSA | 0.0 | 0.2 | 0.06 | 0.37 | 0.06 | 54.9 |
| Approach | 20 | 4.6 | 21 | 4.6 | 0.008 | 4.5 | NA | 0.0 | 0.2 | 0.04 | 0.45 | 0.04 | 54.6 |
| All <br> Vehicles | 96 | 3.9 | 101 | 3.9 | 0.022 | 4.7 | NA | 0.1 | 0.6 | 0.08 | 0.46 | 0.08 | 54.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^3]
## MOVEMENT SUMMARY

$\nabla$ Site: 101 [2021 AM Oakenville Rd - Jenkins St (Site Folder:
Project Scenario)]
New Site
Site Category: (None)
Give-Way (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID |  | $\begin{gathered} \text { JT } \\ \text { VES } \\ \text { HV ] } \\ \% \end{gathered}$ |  | $\begin{aligned} & \text { IND } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn <br> v/c | Aver. Delay <br> sec | Level of Service |  | CK OF <br> UE Dist ] m | Prop. Que | Effective Stop Rate |  | Aver Speed <br> km/h |
| South: Oakenville Street |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 18 | 2.0 | 19 | 2.0 | 0.015 | 5.7 | LOS A | 0.1 | 0.4 | 0.09 | 0.54 | 0.09 | 53.3 |
| 2 T1 | 13 | 2.0 | 14 | 2.0 | 0.028 | 5.7 | LOSA | 0.1 | 0.8 | 0.31 | 0.53 | 0.31 | 53.4 |
| 3 R2 | 10 | 2.0 | 11 | 2.0 | 0.028 | 6.8 | LOSA | 0.1 | 0.8 | 0.31 | 0.53 | 0.31 | 53.0 |
| Approach | 41 | 2.0 | 43 | 2.0 | 0.028 | 6.0 | LOS A | 0.1 | 0.8 | 0.21 | 0.54 | 0.21 | 53.2 |
| East: Oakenville Street |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 4 | 2.0 | 4 | 2.0 | 0.002 | 5.6 | LOSA | 0.0 | 0.0 | 0.00 | 0.58 | 0.00 | 53.5 |
| 5 T1 | 26 | 15.0 | 27 | 15.0 | 0.020 | 0.1 | LOS A | 0.0 | 0.3 | 0.07 | 0.11 | 0.07 | 58.6 |
| 6 R2 | 6 | 2.0 | 6 | 2.0 | 0.020 | 5.8 | LOSA | 0.0 | 0.3 | 0.07 | 0.11 | 0.07 | 56.7 |
| Approach | 36 | 11.4 | 38 | 11.4 | 0.020 | 1.6 | NA | 0.0 | 0.3 | 0.06 | 0.16 | 0.06 | 57.7 |
| North: Oakenville Street |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 9 | 2.0 | 9 | 2.0 | 0.008 | 5.8 | LOSA | 0.0 | 0.2 | 0.14 | 0.54 | 0.14 | 53.1 |
| 8 T1 | 7 | 2.0 | 7 | 2.0 | 0.020 | 5.7 | LOS A | 0.1 | 0.6 | 0.31 | 0.54 | 0.31 | 53.3 |
| 9 R2 | 9 | 2.0 | 9 | 2.0 | 0.020 | 6.9 | LOS A | 0.1 | 0.6 | 0.31 | 0.54 | 0.31 | 52.6 |
| Approach | 25 | 2.0 | 26 | 2.0 | 0.020 | 6.2 | LOS A | 0.1 | 0.6 | 0.25 | 0.54 | 0.25 | 53.0 |
| West: Oakenville Street |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 8 | 2.0 | 8 | 2.0 | 0.005 | 5.6 | LOS A | 0.0 | 0.0 | 0.00 | 0.58 | 0.00 | 53.5 |
| 11 T1 | 56 | 15.0 | 59 | 15.0 | 0.060 | 0.1 | LOS A | 0.2 | 1.6 | 0.08 | 0.22 | 0.08 | 57.4 |
| 12 R 2 | 36 | 2.0 | 38 | 2.0 | 0.060 | 5.7 | LOS A | 0.2 | 1.6 | 0.08 | 0.22 | 0.08 | 56.1 |
| Approach | 100 | 9.3 | 105 | 9.3 | 0.060 | 2.5 | NA | 0.2 | 1.6 | 0.07 | 0.25 | 0.07 | 56.6 |
| All Vehicles | 202 | 7.3 | 213 | 7.3 | 0.060 | 3.5 | NA | 0.2 | 1.6 | 0.12 | 0.33 | 0.12 | 55.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^4]
## MOVEMENT SUMMARY

$\nabla$ Site: 101 [2021 PM Barry Road and Forest Way (Site Folder:
Project Scenario)]
New Site
Site Category: (None)
Give-Way (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $\begin{aligned} & \text { Mov Turn } \\ & \text { ID } \end{aligned}$ |  | $\begin{aligned} & \text { JT } \\ & \text { MES } \\ & \text { HV ] } \\ & \% \\ & \hline \end{aligned}$ | $\begin{gathered} \text { DEM } \\ \text { FLO } \\ \text { [ Total } \\ \text { veh/h } \end{gathered}$ | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn <br> v/c | Aver. Delay <br> sec | Level of Service | $\begin{gathered} 95 \% \text { E } \\ \text { Q } \\ \text { [ Veh. } \\ \text { veh } \end{gathered}$ | $\begin{gathered} \text { CK OF } \\ \text { UE } \\ \text { Dist ] } \\ \mathrm{m} \end{gathered}$ | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. Speed <br> km/h |
| South: Hanging Rock Lookout Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 1 | 0.0 | 1 | 0.0 | 0.003 | 7.2 | LOS A | 0.0 | 0.1 | 0.19 | 0.57 | 0.19 | 65.1 |
| 2 T1 | 1 | 0.0 | 1 | 0.0 | 0.003 | 6.1 | LOS A | 0.0 | 0.1 | 0.19 | 0.57 | 0.19 | 65.2 |
| 3 R2 | 1 | 0.0 | 1 | 0.0 | 0.003 | 7.1 | LOSA | 0.0 | 0.1 | 0.19 | 0.57 | 0.19 | 64.7 |
| Approach | 3 | 0.0 | 3 | 0.0 | 0.003 | 6.8 | LOS A | 0.0 | 0.1 | 0.19 | 0.57 | 0.19 | 65.0 |
| East: Barry Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 1 | 0.0 | 1 | 0.0 | 0.046 | 7.0 | LOS A | 0.0 | 0.2 | 0.01 | 0.04 | 0.01 | 74.0 |
| 5 T1 | 80 | 0.0 | 84 | 0.0 | 0.046 | 0.0 | LOS A | 0.0 | 0.2 | 0.01 | 0.04 | 0.01 | 79.2 |
| 6 R2 | 4 | 0.0 | 4 | 0.0 | 0.046 | 6.7 | LOS A | 0.0 | 0.2 | 0.01 | 0.04 | 0.01 | 73.3 |
| Approach | 85 | 0.0 | 89 | 0.0 | 0.046 | 0.4 | NA | 0.0 | 0.2 | 0.01 | 0.04 | 0.01 | 78.8 |
| North: Forest Way |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 4 | 0.0 | 4 | 0.0 | 0.011 | 7.0 | LOS A | 0.0 | 0.3 | 0.11 | 0.61 | 0.11 | 65.2 |
| 8 T1 | 1 | 0.0 | 1 | 0.0 | 0.011 | 6.1 | LOS A | 0.0 | 0.3 | 0.11 | 0.61 | 0.11 | 65.3 |
| 9 R2 | 7 | 0.0 | 7 | 0.0 | 0.011 | 7.1 | LOS A | 0.0 | 0.3 | 0.11 | 0.61 | 0.11 | 64.8 |
| Approach | 12 | 0.0 | 13 | 0.0 | 0.011 | 7.0 | LOS A | 0.0 | 0.3 | 0.11 | 0.61 | 0.11 | 65.0 |
| West: Barry Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 7 | 0.0 | 7 | 0.0 | 0.018 | 7.0 | LOS A | 0.0 | 0.1 | 0.02 | 0.16 | 0.02 | 72.0 |
| 11 T1 | 24 | 0.0 | 25 | 0.0 | 0.018 | 0.0 | LOS A | 0.0 | 0.1 | 0.02 | 0.16 | 0.02 | 76.9 |
| 12 R 2 | 1 | 0.0 | 1 | 0.0 | 0.018 | 6.8 | LOS A | 0.0 | 0.1 | 0.02 | 0.16 | 0.02 | 71.4 |
| Approach | 32 | 0.0 | 34 | 0.0 | 0.018 | 1.7 | NA | 0.0 | 0.1 | 0.02 | 0.16 | 0.02 | 75.6 |
| All <br> Vehicles | 132 | 0.0 | 139 | 0.0 | 0.046 | 1.5 | NA | 0.0 | 0.3 | 0.03 | 0.13 | 0.03 | 76.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^5]
## MOVEMENT SUMMARY

$\nabla$ Site: 101 [2021 AM Barry Road and Forest Way (Site Folder:
Existing)]
New Site
Site Category: (None)
Give-Way (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID |  | $\begin{aligned} & \text { JT } \\ & \text { VES } \\ & \text { HV ] } \\ & \% \end{aligned}$ |  | $\begin{aligned} & \text { ND } \\ & \text { VS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn <br> v/c | Aver. Delay <br> sec | Level of Service |  | CK OF <br> UE Dist ] m | Prop. Que | Effective Stop Rate | Aver. No. Cycles | Aver. Speed <br> km/h |
| South: Hanging Rock Lookout Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 1 | 0.0 | 1 | 0.0 | 0.002 | 7.0 | LOS A | 0.0 | 0.1 | 0.09 | 0.59 | 0.09 | 65.6 |
| 2 T1 | 1 | 0.0 | 1 | 0.0 | 0.002 | 5.9 | LOS A | 0.0 | 0.1 | 0.09 | 0.59 | 0.09 | 65.7 |
| 3 R2 | 1 | 0.0 | 1 | 0.0 | 0.002 | 6.8 | LOSA | 0.0 | 0.1 | 0.09 | 0.59 | 0.09 | 65.1 |
| Approach | 3 | 0.0 | 3 | 0.0 | 0.002 | 6.5 | LOS A | 0.0 | 0.1 | 0.09 | 0.59 | 0.09 | 65.5 |
| East: Barry Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 1 | 0.0 | 1 | 0.0 | 0.011 | 7.0 | LOS A | 0.0 | 0.1 | 0.01 | 0.06 | 0.01 | 73.6 |
| 5 T1 | 19 | 0.0 | 20 | 0.0 | 0.011 | 0.0 | LOS A | 0.0 | 0.1 | 0.01 | 0.06 | 0.01 | 78.8 |
| 6 R2 | 1 | 0.0 | 1 | 0.0 | 0.011 | 6.7 | LOS A | 0.0 | 0.1 | 0.01 | 0.06 | 0.01 | 73.0 |
| Approach | 21 | 0.0 | 22 | 0.0 | 0.011 | 0.7 | NA | 0.0 | 0.1 | 0.01 | 0.06 | 0.01 | 78.2 |
| North: Forest Way |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 1 | 0.0 | 1 | 0.0 | 0.005 | 7.0 | LOSA | 0.0 | 0.1 | 0.10 | 0.60 | 0.10 | 65.4 |
| 8 T1 | 1 | 0.0 | 1 | 0.0 | 0.005 | 5.9 | LOSA | 0.0 | 0.1 | 0.10 | 0.60 | 0.10 | 65.5 |
| 9 R2 | 4 | 0.0 | 4 | 0.0 | 0.005 | 6.8 | LOSA | 0.0 | 0.1 | 0.10 | 0.60 | 0.10 | 64.9 |
| Approach | 6 | 0.0 | 6 | 0.0 | 0.005 | 6.7 | LOS A | 0.0 | 0.1 | 0.10 | 0.60 | 0.10 | 65.1 |
| West: Barry Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 4 | 0.0 | 4 | 0.0 | 0.014 | 7.0 | LOS A | 0.0 | 0.1 | 0.01 | 0.12 | 0.01 | 72.6 |
| 11 T1 | 21 | 0.0 | 22 | 0.0 | 0.014 | 0.0 | LOS A | 0.0 | 0.1 | 0.01 | 0.12 | 0.01 | 77.7 |
| 12 R 2 | 1 | 0.0 | 1 | 0.0 | 0.014 | 6.7 | LOS A | 0.0 | 0.1 | 0.01 | 0.12 | 0.01 | 72.0 |
| Approach | 26 | 0.0 | 27 | 0.0 | 0.014 | 1.3 | NA | 0.0 | 0.1 | 0.01 | 0.12 | 0.01 | 76.6 |
| All <br> Vehicles | 56 | 0.0 | 59 | 0.0 | 0.014 | 1.9 | NA | 0.0 | 0.1 | 0.02 | 0.18 | 0.02 | 75.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^6]
## MOVEMENT SUMMARY

$\nabla$ Site: 101 [2021 PM Barry Road and Forest Way - Copy (Site
Folder: Existing)]
New Site
Site Category: (None)
Give-Way (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | $\begin{aligned} & \text { INP } \\ & \text { VOLu } \\ & \text { [ Total } \\ & \text { veh/h } \end{aligned}$ | $\begin{aligned} & \text { JT } \\ & \text { MES } \\ & \text { HV ] } \\ & \% \end{aligned}$ |  | $\begin{aligned} & \text { ND } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \\ & \hline \end{aligned}$ | Deg. Satn v/c | Aver. Delay <br> sec | Level of Service |  | $\begin{gathered} \text { CK OF } \\ \text { UE } \\ \text { Dist ] } \\ \text { m } \\ \hline \end{gathered}$ | Prop. Que | Effective Stop Rate | $\begin{aligned} & \text { Aver. } \\ & \text { No. } \\ & \text { Cycles } \end{aligned}$ | Aver. Speed <br> km/h |
| South: Hanging Rock Lookout Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 1 | 0.0 | 1 | 0.0 | 0.002 | 7.0 | LOSA | 0.0 | 0.1 | 0.10 | 0.59 | 0.10 | 65.5 |
| 2 T1 | 1 | 0.0 | 1 | 0.0 | 0.002 | 5.9 | LOSA | 0.0 | 0.1 | 0.10 | 0.59 | 0.10 | 65.6 |
| 3 R2 | 1 | 0.0 | 1 | 0.0 | 0.002 | 6.8 | LOSA | 0.0 | 0.1 | 0.10 | 0.59 | 0.10 | 65.0 |
| Approach | 3 | 0.0 | 3 | 0.0 | 0.002 | 6.6 | LOS A | 0.0 | 0.1 | 0.10 | 0.59 | 0.10 | 65.4 |
| East: Barry Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 1 | 0.0 | 1 | 0.0 | 0.016 | 7.0 | LOS A | 0.0 | 0.1 | 0.01 | 0.05 | 0.01 | 73.9 |
| 5 T1 | 27 | 0.0 | 28 | 0.0 | 0.016 | 0.0 | LOSA | 0.0 | 0.1 | 0.01 | 0.05 | 0.01 | 79.1 |
| 6 R2 | 1 | 0.0 | 1 | 0.0 | 0.016 | 6.7 | LOSA | 0.0 | 0.1 | 0.01 | 0.05 | 0.01 | 73.3 |
| Approach | 29 | 0.0 | 31 | 0.0 | 0.016 | 0.5 | NA | 0.0 | 0.1 | 0.01 | 0.05 | 0.01 | 78.7 |
| North: Forest Way |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 1 | 0.0 | 1 | 0.0 | 0.005 | 7.0 | LOSA | 0.0 | 0.1 | 0.10 | 0.60 | 0.10 | 65.4 |
| 8 T1 | 1 | 0.0 | 1 | 0.0 | 0.005 | 5.9 | LOSA | 0.0 | 0.1 | 0.10 | 0.60 | 0.10 | 65.5 |
| 9 R2 | 4 | 0.0 | 4 | 0.0 | 0.005 | 6.8 | LOSA | 0.0 | 0.1 | 0.10 | 0.60 | 0.10 | 64.9 |
| Approach | 6 | 0.0 | 6 | 0.0 | 0.005 | 6.7 | LOSA | 0.0 | 0.1 | 0.10 | 0.60 | 0.10 | 65.1 |
| West: Barry Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 4 | 0.0 | 4 | 0.0 | 0.012 | 7.0 | LOS A | 0.0 | 0.1 | 0.01 | 0.15 | 0.01 | 72.1 |
| 11 T1 | 16 | 0.0 | 17 | 0.0 | 0.012 | 0.0 | LOSA | 0.0 | 0.1 | 0.01 | 0.15 | 0.01 | 77.1 |
| 12 R 2 | 1 | 0.0 | 1 | 0.0 | 0.012 | 6.7 | LOSA | 0.0 | 0.1 | 0.01 | 0.15 | 0.01 | 71.5 |
| Approach | 21 | 0.0 | 22 | 0.0 | 0.012 | 1.6 | NA | 0.0 | 0.1 | 0.01 | 0.15 | 0.01 | 75.8 |
| All <br> Vehicles | 59 | 0.0 | 62 | 0.0 | 0.016 | 1.8 | NA | 0.0 | 0.1 | 0.02 | 0.17 | 0.02 | 75.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^7]
## MOVEMENT SUMMARY

$\nabla$ Site: 101 [2021 AM Barry Road and Forest Way (Site Folder:
Project Scenario)]
New Site
Site Category: (None)
Give-Way (Two-Way)

| Vehicle Movement Performance |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mov Turn ID | $\begin{aligned} & \text { INP } \\ & \text { VOLu } \\ & \text { [ Total } \\ & \text { veh/h } \end{aligned}$ | $\begin{aligned} & \text { JT } \\ & \text { MES } \\ & \text { HV ] } \\ & \% \end{aligned}$ |  | $\begin{aligned} & \text { WD } \\ & \text { NS } \\ & \text { HV ] } \\ & \% \end{aligned}$ | Deg. Satn v/c | Aver. Delay <br> sec | Level of Service |  | $\begin{gathered} \text { CK OF } \\ \text { UE } \\ \text { Dist ] } \\ \text { m } \\ \hline \end{gathered}$ | Prop. Que | Effective Stop Rate | $\begin{aligned} & \text { Aver. } \\ & \text { No. } \\ & \text { Cycles } \end{aligned}$ | Aver. Speed <br> km/h |
| South: Hanging Rock Lookout Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 L2 | 1 | 0.0 | 1 | 0.0 | 0.003 | 7.0 | LOSA | 0.0 | 0.1 | 0.13 | 0.59 | 0.13 | 65.4 |
| 2 T1 | 1 | 0.0 | 1 | 0.0 | 0.003 | 6.1 | LOS A | 0.0 | 0.1 | 0.13 | 0.59 | 0.13 | 65.5 |
| 3 R2 | 1 | 0.0 | 1 | 0.0 | 0.003 | 7.1 | LOSA | 0.0 | 0.1 | 0.13 | 0.59 | 0.13 | 64.9 |
| Approach | 3 | 0.0 | 3 | 0.0 | 0.003 | 6.8 | LOS A | 0.0 | 0.1 | 0.13 | 0.59 | 0.13 | 65.2 |
| East: Barry Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4 L2 | 1 | 0.0 | 1 | 0.0 | 0.023 | 7.1 | LOS A | 0.0 | 0.2 | 0.04 | 0.08 | 0.04 | 73.1 |
| 5 T1 | 36 | 0.0 | 38 | 0.0 | 0.023 | 0.0 | LOSA | 0.0 | 0.2 | 0.04 | 0.08 | 0.04 | 78.2 |
| 6 R2 | 4 | 0.0 | 4 | 0.0 | 0.023 | 6.8 | LOS A | 0.0 | 0.2 | 0.04 | 0.08 | 0.04 | 72.5 |
| Approach | 41 | 0.0 | 43 | 0.0 | 0.023 | 0.9 | NA | 0.0 | 0.2 | 0.04 | 0.08 | 0.04 | 77.5 |
| North: Forest Way |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7 L2 | 4 | 0.0 | 4 | 0.0 | 0.011 | 7.2 | LOS A | 0.0 | 0.3 | 0.19 | 0.59 | 0.19 | 64.9 |
| 8 T1 | 1 | 0.0 | 1 | 0.0 | 0.011 | 6.1 | LOS A | 0.0 | 0.3 | 0.19 | 0.59 | 0.19 | 65.0 |
| 9 R2 | 7 | 0.0 | 7 | 0.0 | 0.011 | 7.2 | LOSA | 0.0 | 0.3 | 0.19 | 0.59 | 0.19 | 64.4 |
| Approach | 12 | 0.0 | 13 | 0.0 | 0.011 | 7.1 | LOS A | 0.0 | 0.3 | 0.19 | 0.59 | 0.19 | 64.6 |
| West: Barry Road |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 10 L2 | 7 | 0.0 | 7 | 0.0 | 0.045 | 7.0 | LOSA | 0.0 | 0.1 | 0.00 | 0.06 | 0.00 | 73.6 |
| 11 T1 | 75 | 0.0 | 79 | 0.0 | 0.045 | 0.0 | LOSA | 0.0 | 0.1 | 0.00 | 0.06 | 0.00 | 78.8 |
| 12 R 2 | 1 | 0.0 | 1 | 0.0 | 0.045 | 6.7 | LOSA | 0.0 | 0.1 | 0.00 | 0.06 | 0.00 | 73.0 |
| Approach | 83 | 0.0 | 87 | 0.0 | 0.045 | 0.7 | NA | 0.0 | 0.1 | 0.00 | 0.06 | 0.00 | 78.3 |
| All <br> Vehicles | 139 | 0.0 | 146 | 0.0 | 0.045 | 1.4 | NA | 0.0 | 0.3 | 0.03 | 0.12 | 0.03 | 76.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).
Vehicle movement LOS values are based on average delay per movement.
Minor Road Approach LOS values are based on average delay for all vehicle movements.
NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.
Delay Model: SIDRA Standard (Geometric Delay is included).
Queue Model: SIDRA Standard.
Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).
HV (\%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

[^8]
[^0]:    ${ }^{1}$ Hills of Gold Wind Farm - Traffic and Transport Assessment - 12 November 2020 (TTPP)
    ${ }^{2}$ Hills of Gold Wind Farm - Traffic and Transport Addendum - 15 October 2021 (TTPP)

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