

Technical Memorandum

Consultant Declaration Form

CONSULTANT DECLARATION

PROJECT DETAILS	
PROJECT NAME	
Application number	SSD-96505456
Address of subject land	19-23 Rosalind Street, Cammeray
Lot / DP	SP4657, SP5218 and SP16181
APPLICANT DETAILS	
Applicant name	Perifa Rosalind Development Pty Ltd
Applicant address	Level 7/111 Elizabeth Street, Sydney
REPORT DETAILS	
Name of report this declaration relates	Groundwater Assessment Report
Report reference no.	2025181_RPT02
Report date	29/01/2026
Company name (inc. ABN / ACN)	Elite Geoscience Pty. Ltd. ABN: 11 679 404 310
Author name	Terence Huang
Author qualifications	BEng (Civil and Geotechnical) Hons, University of Sydney MEng (Geotechnical and Mining), University of NSW Australia CPEng, NER, NSW PDP, DEP, PRE
Author address	Suite 65/97 Jones St Ultimo NSW 2007
DECLARATION BY CONSULTANT	
Name	Terence Huang
Registration no.	DEP0003150 PDP0001079 NER3243905
Organisation registered with	Elite Geoscience Pty. Ltd. ABN: 11 679 404 310
Declaration	The undersigned declares that Groundwater Assessment Report: <ul style="list-style-type: none">has been prepared in accordance with the following policy, guidelines, or legislative requirements:

PROJECT DETAILS

- Environmental Plannings and Assessment Act 1979
- Water Management Act 2000
- contains all available information relevant to the environmental assessment of the development, activity or infrastructure to which the Groundwater Assessment Report relates;
- does not contain information that is false or misleading;
- identifies and addresses the relevant Planning Secretary's environmental assessment requirements (SEARs) for the project;
- identifies and addresses the relevant statutory requirements for the project, including any relevant matters for consideration in environmental planning instruments to which the Groundwater Assessment report relates;
- contains a consolidated summary of the proposed or necessary mitigation measures

Signature 

Date **29/01/2026**

To	Perifa Rosalind Development Pty Ltd	From	Terence Huang Director Neal Li Principal Geotechnical Engineer
SSD Ref	SSD-96505456	Date	29/01/2026
Project	Proposed residential redevelopment	Location	19-23 Rosalind St, Cammeray
Subject	Groundwater inflow estimates and impact assessment for the proposed redevelopment		
Report Status	Revision 2	Attachments	Analysis results and groundwater measured records

1 Introduction

This Groundwater Assessment Report has been prepared by Elite Geosciences Pty Ltd (EG) on behalf of Perifa Rosalind Development Pty Ltd (Perifa) to assess the potential environmental impacts that could arise from the construction of a seniors housing development (the development) at 19-23 Rosalind Street, Cammeray (the site). This report supports the assessment of the proposed development under Part 4 of the *Environmental Planning and Assessment Act 1979*.

Industry specific SEARs were issued on 17 October 2025 for the development. Development for the purposes of seniors housing with an Estimated Development Cost (EDC) of more than \$30 million and includes a residential care facility is state significant development under Schedule 1, Section 28 of the State Environmental Planning Policy (Planning Systems) 2021 (Planning Systems SEPP). The proposed development has an EDC exceeding \$30 million and includes a residential care facility component. Accordingly, it is considered State Significant Development.

This report has been prepared to assess the groundwater inflow estimates and surface and groundwater impact assessment for the proposed residential redevelopment at 19-23 Rosalind Street, Cammeray NSW (the Site)

This report accompanies an Environmental Impact Statement (EIS) pursuant to Part 4 of the Environmental Plannings and Assessment Act 1979 (EP&A Act). In support of a State Significant Development Application (SSDA) for the construction of the proposed residential building, reference SSD-96505456. This report addresses the Secretary’s Environmental Assessment Requirements (SEARs) issued for the project notably:

SEAR Requirement	Section of Report where response is provided
<i>Item 12 Ground and Groundwater Conditions – Groundwater Impact Assessment in accordance with relevant Groundwater Guidelines</i>	Section 2-4 of this report.

Summary of the Development

The proposed development includes the construction of a new seniors housing development and comprises the following works:

- Site preparation works including demolition of three (3) existing residential flat buildings and associated parking facilities as well as bulk excavation;
- Construction of two (2), five (5) and six (6) storey buildings, Building A and B respectively, comprising the following:
 - Building A:
 - 7 x 2-bedroom ILUs;
 - 11 x 3-bedroom ILUs; and
 - Internal communal space for use by residents.
 - Building B:
 - 11 x 2-bedroom ILUs;
 - 20 x 3-bedroom ILUs;
 - Two (2) residential care facility beds and residential care hub; and
 - Internal communal facilities for use by all residents comprising a cinema, private dining room, gymnasium and pool.
- Communal open space and associated landscaping;
- Construction of two (2) basement levels to facilitate car parking accessible via Rosalind Street;
- Ground Level neighbourhood shop located in Building A;
- Extension and augmentation of utility infrastructure as required.

For a detailed project description refer to the Environmental Impact Statement prepared by Colliers Urban Planning.

It is expected that the proposed two (2) levels basement will reach the Finished Floor Level (FFL) at RL 77.4m and estimated Bulk Excavation Level (BEL) at about RL77.2m, up 9.56m deep of excavations; and the architectural plans indicate min. 2.2-3.0m setback is proposed for basement walls.

It is considered that proposed drained basement will be suitable for the excavation.

A geotechnical investigation has been undertaken for the subject site by EG in late October 2025. Findings of investigation are summarised and reported in the EG Geotechnical Investigation Report, Rev 1, dated 4th November 2025. This geotechnical investigation forms the basis of the subsurface conditions adopted in this assessment and should be read in conjunction with this report.

Upon the completion of the geotechnical investigation fieldworks, groundwater monitoring wells were installed on selected boreholes (BH1, BH3, BH4 and BH6) by EG with monitoring data loggers. Hourly monitoring data are available between 1st October and 9th November 2025. Monitoring data are attached in this report at Appendix B.

2 Modelling of Groundwater Inflows

2.1 Site Description

The site is located at 19–23 Rosalind Street, Cammeray within the North Sydney Local Government Area (LGA). It comprises three (3) allotments legally described as SP4657, SP5218 and SP16181, and occupies a total area of approximately 4,100m². A site aerial showing each allotment is provided at **Figure 1**. Each respective lot currently comprises a three (3) storey residential flat building with the two (2) of the rear buildings situated on battleaxe allotments connected to Rosalind Street.



Figure 1 Site Aerial

Source: Nearmap/Colliers Urban Planning

2.2 Subsurface Conditions

Existing geotechnical test locations are illustrated Appendix A. Logs are attached in Appendix B.

These geotechnical investigations form the basis of the subsurface conditions adopted in this assessment. The subsurface conditions are summarized in Table 1. Rocks are classified based on rock strength and defect spacing in accordance with Pells et al. (2019).



Figure 2 – Testing Plan

Table 1 – Subsurface Conditions

Geotechnical Unit	BH1	BH2	BH3	BH4	BH5	BH6
Surface RL (m)	84.34	85.34	86.67	84.45	85.42	86.76
Fill (Unit 1)	0.00 – 1.80m	0.10 – 1.20m	0.10 – 1.50m	0.10 – 1.80m	0.04 – 0.10m	0.00 – 0.50m
Residual Soils (Unit 2)	1.80 – 2.40m	1.20 – 1.50 m	1.50 – 1.90 m	1.80 – 3.30 m	0.10 – 0.50 m	0.50 – 1.20 m
Class V/IV Sandstone (Unit 3a)	2.40 – 5.25 m	1.50 – 3.39 m	1.90 – 4.70 m	3.30 – 3.78 m	0.50 – 0.60 m	1.20 – 1.30 m
Class III Sandstone (Unit 3b)	5.25 – 14.74 m Termination	3.39 – 10.23 m Termination	4.70 – 10.00 m Termination	3.78 – 11.00 m Termination	0.60 – 10.00 m Termination	1.30 – 10.00 m Termination

Table 2 – Design Parameter

Geotechnical Unit	Unit Weight, γ (kN/m ³)	Undrained Shear Strength S_u (kPa)	Effective Cohesion c' (kPa)	Effective Internal Friction Angle ϕ'' (Deg.)	Young's Modulus E' (MPa)	Poisson's Ratio, ν'
Fill (Unit 1)	17	-	2	27	8	0.35
Residual Soils (Unit 2)	19	100	10	28	20	0.35
Class V/IV Sandstone (Unit 3a)	23	-	50	30	100	0.30
Class III Sandstone (Unit 3b)	24	200	200	38	1000	0.20

2.3 Hydraulic Conductivity Parameters

The design hydraulic conductivity in horizontal direction was selected based on publish literature and our past experience on similar material at the project site. Adopted permeability parameters are summarized in Table 3 below.

Table 3 – Permeability Parameters

BH ID	Test Depth	Lithological Unit	Hydraulic Conductivity Soil Permeability
BH4	1 – 4m	Residual Silty Clay	Field Test on 1/10/2025: Test 1: 1.1E-8 m/sec (adopted) Test 2: 2.3E-9 m/sec Test 3: 2.6E-9 m/sec
BH1	8.74 – 14.74m	Sandstone Bedrock Predominately Class III BEL Excavation Material	Field Test on 1/10/2025: Test 1: 5.0E-8m/sec (adopted) Test 2: 4.3E-9 m/day Test 3: 2.8E-9 m/day

BH ID	Test Depth	Lithological Unit	Hydraulic Conductivity Soil Permeability
BH6	4 to 10m	Sandstone Bedrock Predominately Class IV BEL Excavation Material	Field Test on 1/10/2025: Test 1: 50E-7 m/sec (adopted) Test 2: 4.1E-7 m/day Test 3: 4.6E-7 m/day

2.4 Groundwater Monitoring

According to the geotechnical investigation report, groundwater table or seepage was not encountered in any of the boreholes during augering to depths of up to 3.3m bgl. Upon completion of borehole, installation of groundwater monitoring wells was carried out in BH1, BH3, BH4 and BH6 to the termination depths.

Wells were developed and drilling water was purged out allowing for future groundwater seepage monitoring and measurement. Revisit was carried out on 1st October 2025 to measure the standing water levels. Completed monitoring wells and measurements are summarised as follows:

Table 4 – Summary of Standpipe Measurement

BH ID	Standpipe Installation Depth (mBGL)	Surface RL	Response Zone Material	Screened Interval Depth (mBGL)	Groundwater Measurement Depth (mBGL / RL)	Measurement Date
BH1	14.74	84.34	Sandstone Class III	8.74 to 14.74m	9.2m BGL / RL75.14m	1/10/2025
BH3	10.00	86.67	Sandstone Class III	5 to 10m	6.1m BGL / RL80.57m	1/10/2025
BH4	11.00	84.45	Residual Soil	1 to 4m	2m BGL / RL84.45m	1/10/2025
BH6	10.00	86.76	Sandstone Class III	4 to 10m	4.2m BGL / RL82.56	1/10/2025

Diver ® and Baro ® Data loggers were installed on BH1, BH2 and BH4 on 1st October 2025 for a three months groundwater monitoring programme. Data loggers were hung inside the standpipe by suspending the logger on a 2mm stainless steel wire rope that is fixed to the well cap. The data loggers have been factory calibrated prior to installation and are set to record absolute water levels every hour.

Table 5 – Recorded Groundwater Level (between 1st October and 9th November 2025)

BH ID	Termination Depth (mBGL)	Surface RL (m)	Recorded Groundwater Level	Monitoring Period (in this report submission)
BH1	14.74	84.34	RL 74.95 m to 75.16 m Average: RL75.05m	1/10/2025 to 9/11/2025
BH3	10.00	86.67	RL 80.46m to 80.75m Average: RL80.62m	1/10/2025 to 9/11/2025
BH4	11.00	84.45	RL 83.02 m to 84.89m Average: RL83.95m	1/10/2025 to 9/11/2025

BH ID	Termination Depth (mBGL)	Surface RL (m)	Recorded Groundwater Level	Monitoring Period (in this report submission)
BH6	10,00	86.76	RL81.46 m to 83.01 m Average: RL82.58m	1/10/2025 to 9/11/2025

Based on EG’s assessment, the groundwater measurement from BH4 was inferred as water seepage from surface runoff and rainfall only. The design groundwater level has been assumed conservatively to be at RL 83.5m, which is 0.5m higher than the highest observed groundwater level on BH6.

2.5 Adopted Model

Steady-state groundwater inflows into the basement have been modelled using the commercially available software Seep/W by Geostudio which is a 2D finite element water seepage analysis tool.

Considering the site landform and subsurface variability across the site, the basement excavation has been modelled using two selected representative cross-sections as shown in Figure 3 to assess the soil variability around the excavation.



Figure 3 – Analysed Sections

The ground surrounding the proposed development was simulated as being a multi-layered numerical model to represent the subsurface conditions surrounding the site and to allow the vertical flow

components to be simulated more accurately. Details of the layering for analysed sections are provided in Figure 3. The model boundaries were extended approximately 80 m from each excavation boundaries.

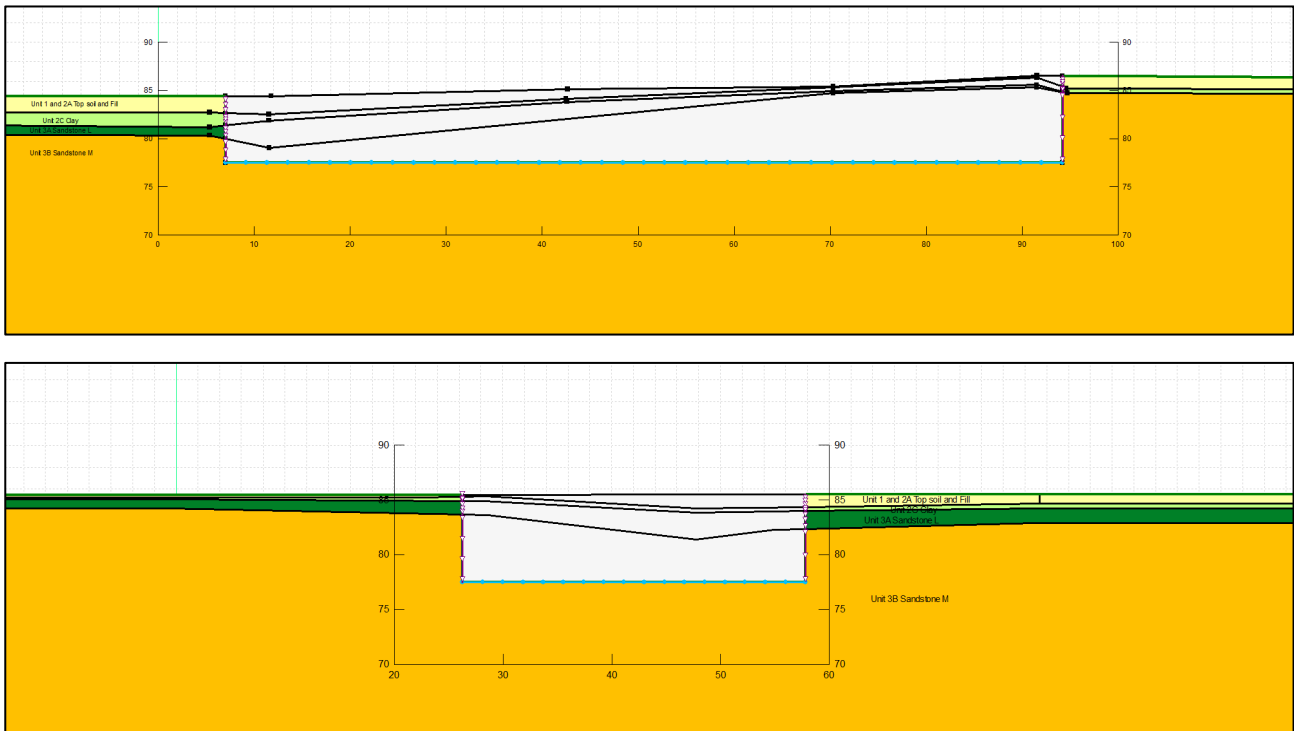


Figure 4 – Model Geometry of Analysed Section A-A (Top) and B-B (Bottom)

On the basis that groundwater monitoring data, for section A and B, the design ground water is conservatively adopted at RL 83.5mAHD, 1 m higher than the recorded highest groundwater standing level to model the sudden increase in groundwater flows into the excavation due to variations in rainfall duration. The basement excavation is expected to be crossing the measured groundwater level at both sides of excavation.

2.6 Results

The analysis was run under transient and steady-state conditions for analysed Section and comprised the following:

- **Transient condition:** A transient scenario to estimate the volume of water removed by sump-and-pump in phases of dewatering (i.e. 6-month construction period) in the ‘drained’ basement during basement excavation construction.
- **Steady-State condition:** Fully drained basement was adopted to estimate the inflows for a long-term condition.

Two sections (as shown in Figure 2) with different ground profiles are modelled to capture the flow rates considering soil variability over the project area. The modelling results for analysed section are summarised in tables below.

The Seep/W outputs are attached in Appendix C.

Table 5 - Modelling Results by Seep/W – Section A

Elapsed Time	Flow rate (m ³ / sec / m run)	Dewatering Flow Rate – Side Walls			Dewatering Flow Rate – Base		
		L / sec	m ³ / day	Average ML	L / sec	m ³ / day	Average ML
1 days	7.05E-06	3.63E-01	31.37	0.80 (First 6 months)	6.53E-02	5.64	0.22 (First 6 months)
3 days	3.92E-06	2.14E-01	18.47		3.03E-02	2.62	
7 days	2.40E-06	1.23E-01	10.60		2.25E-02	1.95	
14 days	1.80E-06	8.97E-02	7.75		1.80E-02	1.55	
30 days	1.31E-06	6.23E-02	5.39		1.47E-02	1.27	
60 days	1.01E-06	4.50E-02	3.89		1.29E-02	1.12	
90 days	8.73E-07	3.67E-02	3.17		1.22E-02	1.05	
120 days	8.01E-07	3.23E-02	2.79		1.19E-02	1.02	
180 days	7.23E-07	2.77E-02	2.39		1.15E-02	0.99	
Long-term	5.37E-07	9.41E-03	0.81	0.30 Per year	9.40E-03	0.81	0.30 Per year

Table 6 - Modelling Results by Seep/W – Section B

Elapsed Time	Flow rate (m ³ / sec / m run)	Dewatering Flow Rate – Side Walls			Dewatering Flow Rate – Base		
		L / sec	m ³ / day	Average ML	L / sec	m ³ / day	Average ML
1 days	2.57E-06	2.58E-01	22.30	1.23 (First 6 months)	9.41E-02	8.13	0.58 (First 6 months)
3 days	1.34E-06	1.28E-01	11.03		5.26E-02	4.54	
7 days	1.16E-06	1.13E-01	9.80		4.38E-02	3.78	
14 days	1.04E-06	1.02E-01	8.82		3.91E-02	3.38	
30 days	9.42E-07	9.04E-02	7.81		3.68E-02	3.18	
60 days	8.62E-07	7.82E-02	6.76		3.59E-02	3.10	
90 days	8.18E-07	7.12E-02	6.15		3.56E-02	3.07	
120 days	7.93E-07	6.74E-02	5.82		3.53E-02	3.05	
180 days	7.51E-07	6.10E-02	5.27		3.48E-02	3.01	
Long-term	2.50E-07	7.38E-03	0.64	0.24 Per year	1.44E-02	1.24	0.46 Per year

Flow rate along excavation walls and base are generally calculated separately. To accurately capture the impact of soil variability, seepage along the excavation side walls (Q_s) is calculated as

$$Q_s = q_{s,section A} * 2L_1 + q_{s,section B} * 2L_2$$

whereas, base seepage (Q_b) is calculated as

$$Q_b = q_{b,section A} * L_1 \text{ or } q_{b,section B} * L_2$$

as illustrated in the figure below.

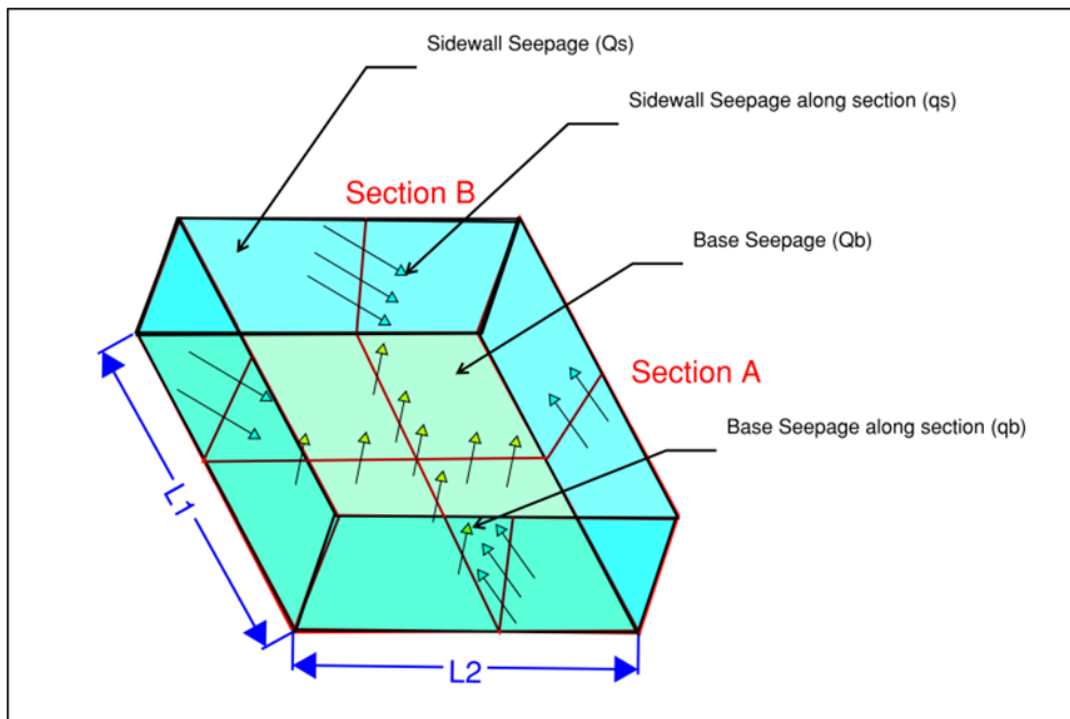


Figure 5 – Flow Rates

However, because a impermeable shoring system was adopted, the side wall seepage Q_s is assumed to negligible. The seepage rates is solely depending on the base seepage Q_b and the size of the proposed excavation, the expected seepage to the temporary excavation during construction is modelled to be:

- Short Term Flows for Temporary Dewatering During Construction will be maximum **2.61ML over the first 6 months** (i.e., $0.80 + 1.23 + 0.58$ (max. flow through the base out of two models)) over the first 6 months.
- Long Term Flow during Service Life of Structure is expected to be **1.0 ML/year (i.e. $0.24 + 0.30 + 0.46$)**.

Projected seepage inflows for a drained basement are below the WaterNSW threshold of 3ML/year. It is confirmed that this proposed development can be granted the Groundwater Access License exemptions.

During the early stages of the excavation, initial inflow rates will be higher due to the absorbed water within the cohesive soil and fractured rock layers. The water flow will then gradually decrease as the hydraulic gradient around the excavation decreases.

The actual flow rate will only be known once the excavation is completed, and the inflow can be observed and measured. However, it is expected that the actual seepage would be less than the estimated volume as the basement footprint reduces at the lowest level.

Appropriate planning should be in place to compensate for possible variations and increases in the actual inflow rate. This may include grouting the excavation face if locally fractured rock and high inflows through these fractured zones are observed at the basement excavation face or floor.

3 Assessments

The topography and measured standing groundwater levels indicate that there is a hydraulic gradient, and groundwater inflows (if any) may be predominately from the southern side excavation wall.

Conservative groundwater modelling has been undertaken and it is predicted that the maximum groundwater inflow of near 2.61 ML may be expected (nominated 6 months period). The groundwater inflow rate is predicted to reduce with time and is predicted to be less than 3ML/year for a drained basement in the long term.

It is evident from the results of EG's seepage analysis that a drained basement is recommended for the predicted long-term groundwater inflow potential. The volume of water entering the basement is limited to the duration of the bulk excavation stages to BEL. During construction period, an automatic pump-out system with back up pump is suggested and sufficient to be adopted to reduce the likelihood of stormwater catch or flooding to the basement. In the long term, drainage should be monitored around the perimeter of the basement and the basement slab to check any minor water leakage.

Dewatering management plan has been prepared for this project for any future discharge of seepage water during basement excavation.

All observed groundwater seepage has been mainly introduced by bedrock units, where groundwater flows are dictated by joints, beddings, and contacts. Numerical modelling analyses have been carried out to check the potential ground movement due to the water draw-down effect. Based on the analyses results, the effects of groundwater drawdown on adjacent properties and structures are considered to be negligible. Considering the negligible draw-down impact within 50m distance around the shoring wall, the surrounding structures impacts are considered to be negligible. It is expected that there will be no harmful impact to the surrounding receptors during the basement construction and dewatering.

Based on both the groundwater modelling assessment and EG's previous experience in the region, the expected groundwater inflows will be manageable during temporary construction with a typical sump and pump setup which has the capacity to handle the inflows calculated in this assessment.

Given that the predicted inflow is less than 3 ML/year during construction only, Water Access License from Water NSW and Water Supply Works Approval is exempted for construction dewatering at the project site at 19 – 23 Rosalind Avenue, Cammeray NSW.

4 NSW Aquifer Interference Policy Impact Assessment

The Water Management Act 2000 includes the concept of ensuring "no more than minimal harm" for the granting of approvals.

For the purposes of this assessment and based on the regional hydrogeological profile, the groundwater source at the site is a "less productive" source as it does not contain water supply works that can yield water at a rate greater than 5 L/sec. The Groundwater Source is assessed as a "Porous or Fractured Rock Water Source".

The impact assessment has been based on the following three assessment criteria:

4.1 Water Table

Less than or equal to 10% cumulative variation in the water table, allowing for typical climatic "post-water sharing plan" variations, 40m from any:

(a) high priority groundwater dependent ecosystem; or

(b) high priority culturally significant site;

listed in the schedule of the relevant water sharing plan.

A maximum of a 2 m decline cumulatively at any water supply work.

Analysis presented within this report shows that all drawdown effects of the proposed works are considered negligible to all nearby receivers, therefore Elite Geosciences confirms that the proposed works will have “minimal impact” on the Water Table.

4.2 Water Pressure

A cumulative pressure head decline of not more than a 2m decline, at any water supply work.

No registered water supply works are present within 50 m of the proposed excavation. Analysis presented within this report shows that all drawdown effects of the proposed works are considered negligible to all nearby receivers, therefore Elite Geosciences confirms that the proposed works will have “minimal impact” on the Water Pressure.

4.3 Water Quality

Any change in the groundwater quality should not lower the beneficial use category of the groundwater source beyond 40m from the activity.

Dewater management plan has been prepared specifically for this project including baseline groundwater quality condition. General Terms of Approval with conditions has been granted with water quality monitoring will be carried out throughout the works as part of the DMP requirements.

It is expected that the works are not projected to cause any detrimental effect to the water quality beyond 40m from the activity. Based on the assessment of the proposed works achieving the requirements of Level 1 minimal impact considerations, Elite Geosciences confirms that the proposed works will do no more than minimal harm.

To	Prepared by	Date
Perifa Rosalind Development Pty Ltd Pty Ltd	 Terence Huang Director Principal Geotechnical Engineer – Elite Geosciences Pty Ltd BEng (Hons) & MEngSC (Geo), MIEAust, CPEng, NER	29/01/2025

Appendix A – Site Plan



⊗ NEW BOREHOLE AND TEST LOCATION

TITLE:

Site Plan
19-23 Rosalind St, Cammeray – Proposed Redevelopment Geotechnical Investigation

ELITE GEOSCIENCES

PROJECT NO:

2025181

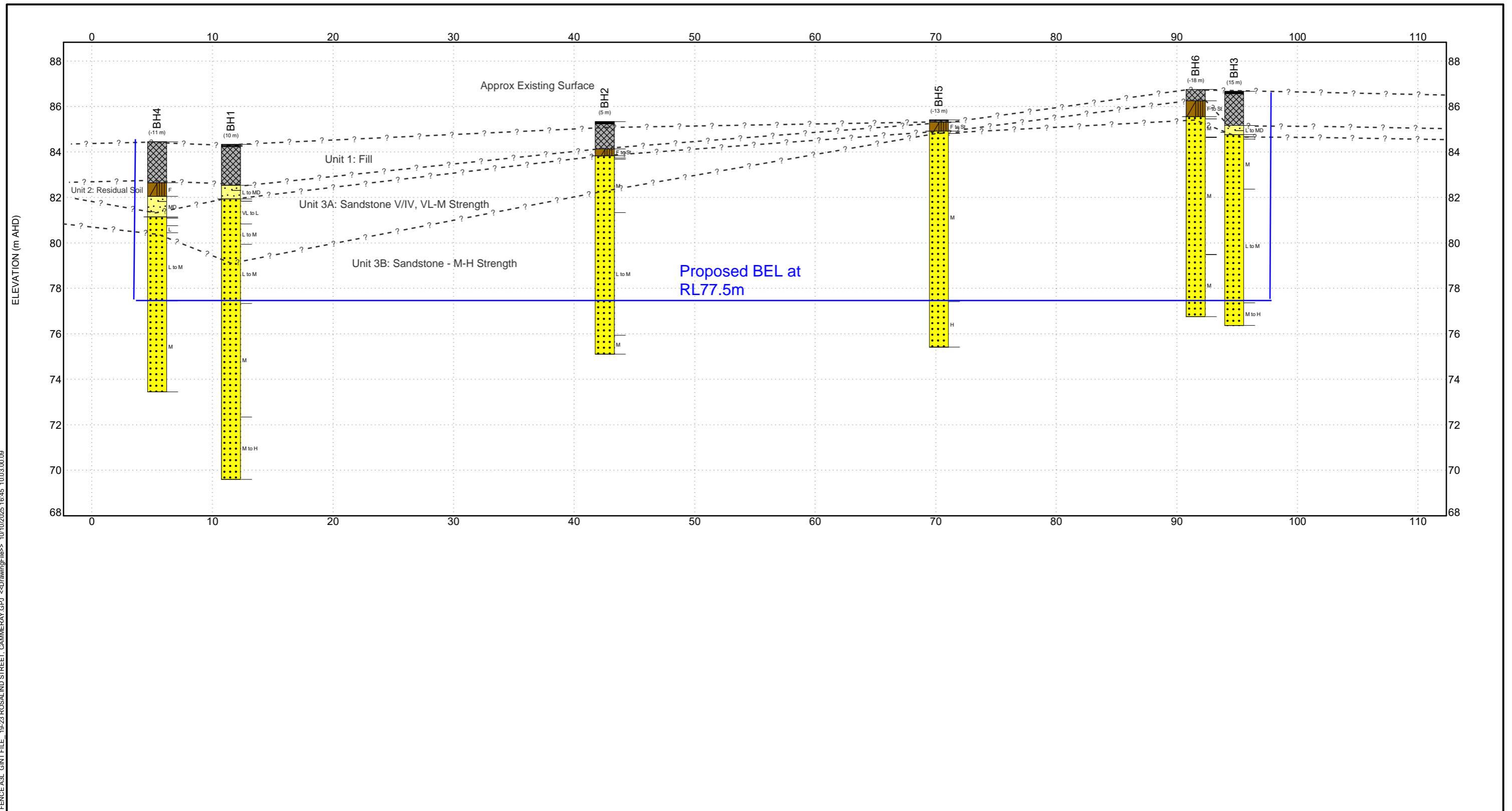
TEST DATE:

25 Sep 2025

PREPARED BY:

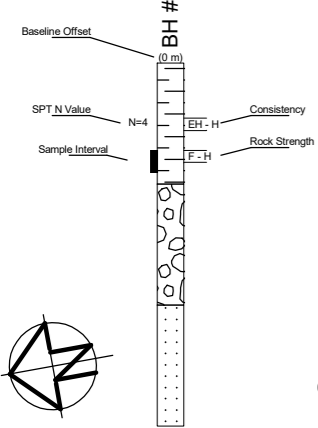
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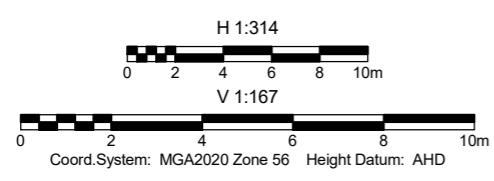


POST LEGEND

MATERIAL GRAPHIC

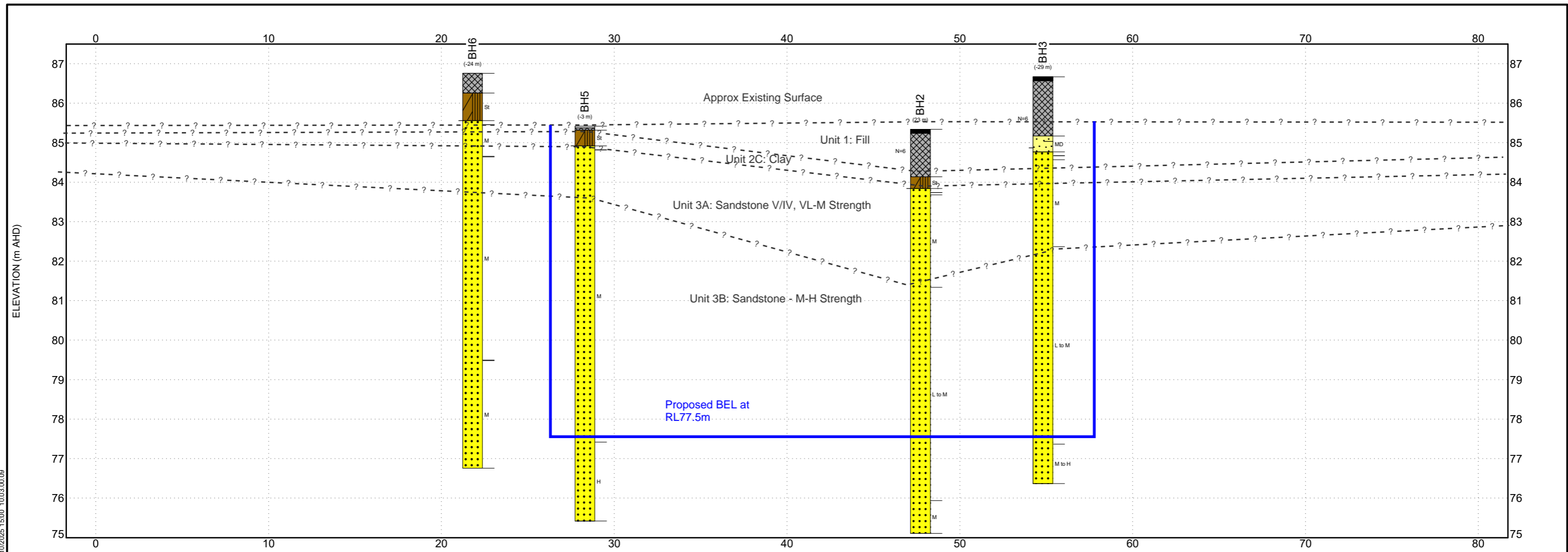


- ASPHALTIC CONCRETE
- FILL
- SANDSTONE
- CI-CH - Medium to High Plasticity CLAY
- Clayey SAND
- CONCRETE



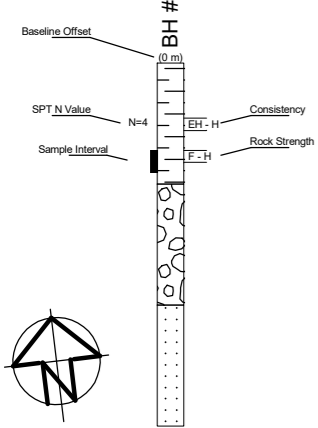
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CHECKED	DATE 10/10/2025		
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			FIGURE No Figure 1

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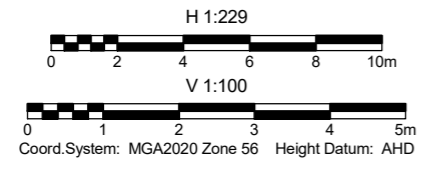


POST LEGEND

MATERIAL GRAPHIC



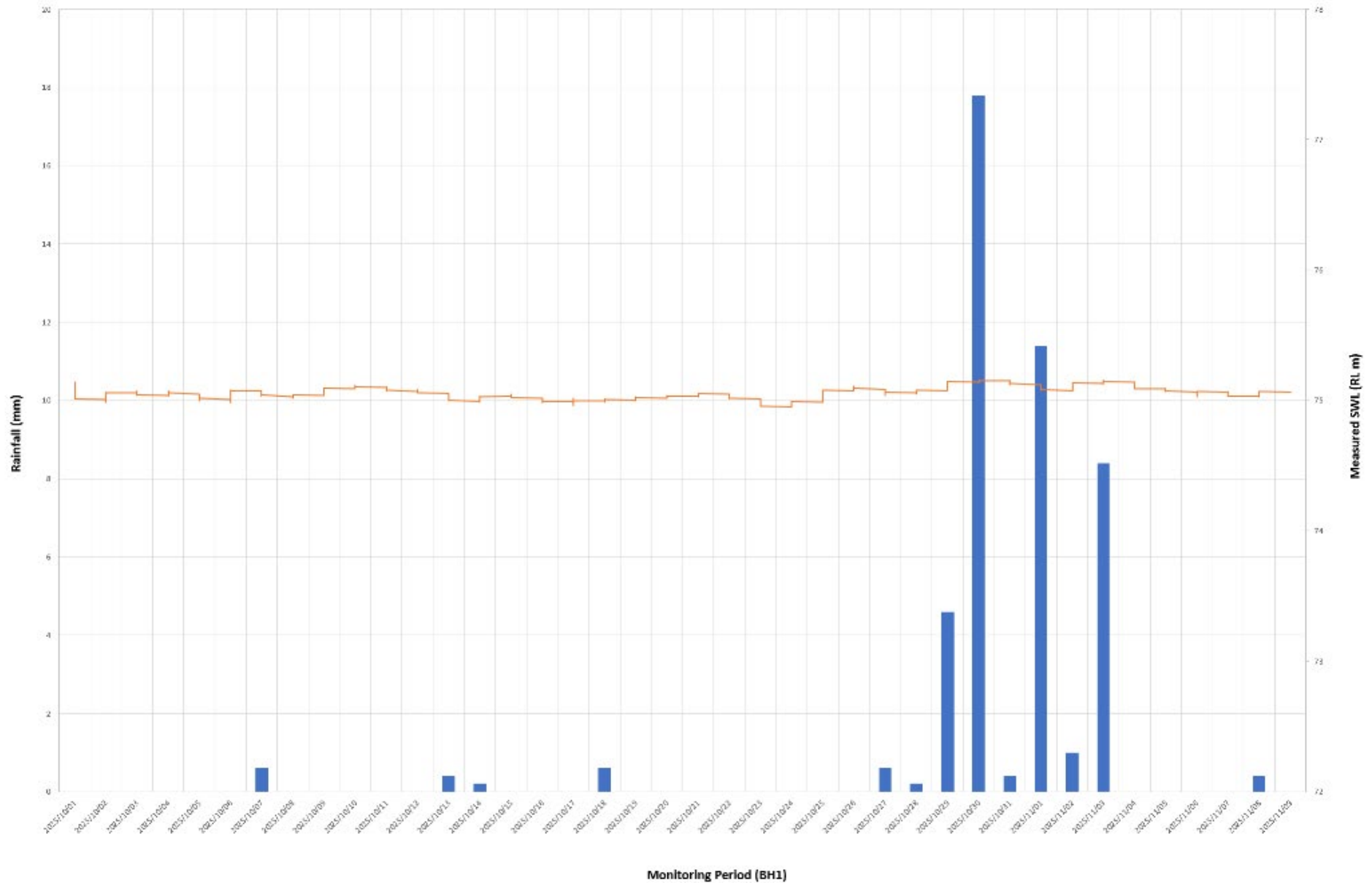
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- FILL
- CI-CH - Medium to High Plasticity CLAY
- SANDSTONE
- Clayey SAND
- CONCRETE



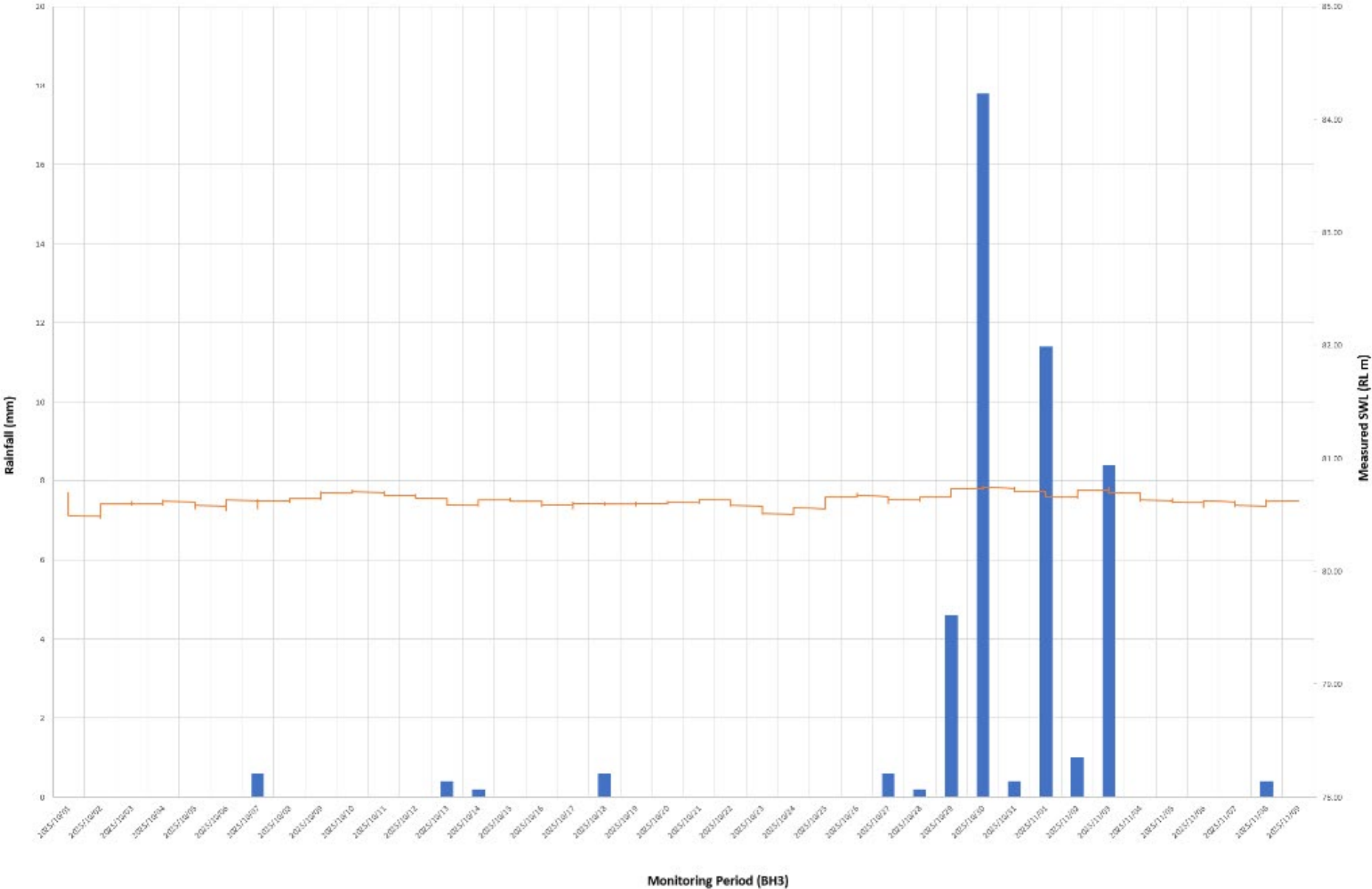
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DRAWN	DATE 10/10/2025	TITLE Subsurface Condition Long Section B-B	
CHECKED	DATE 10/10/2025		
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			FIGURE No Figure 2

Appendix B – Groundwater and Borehole Record

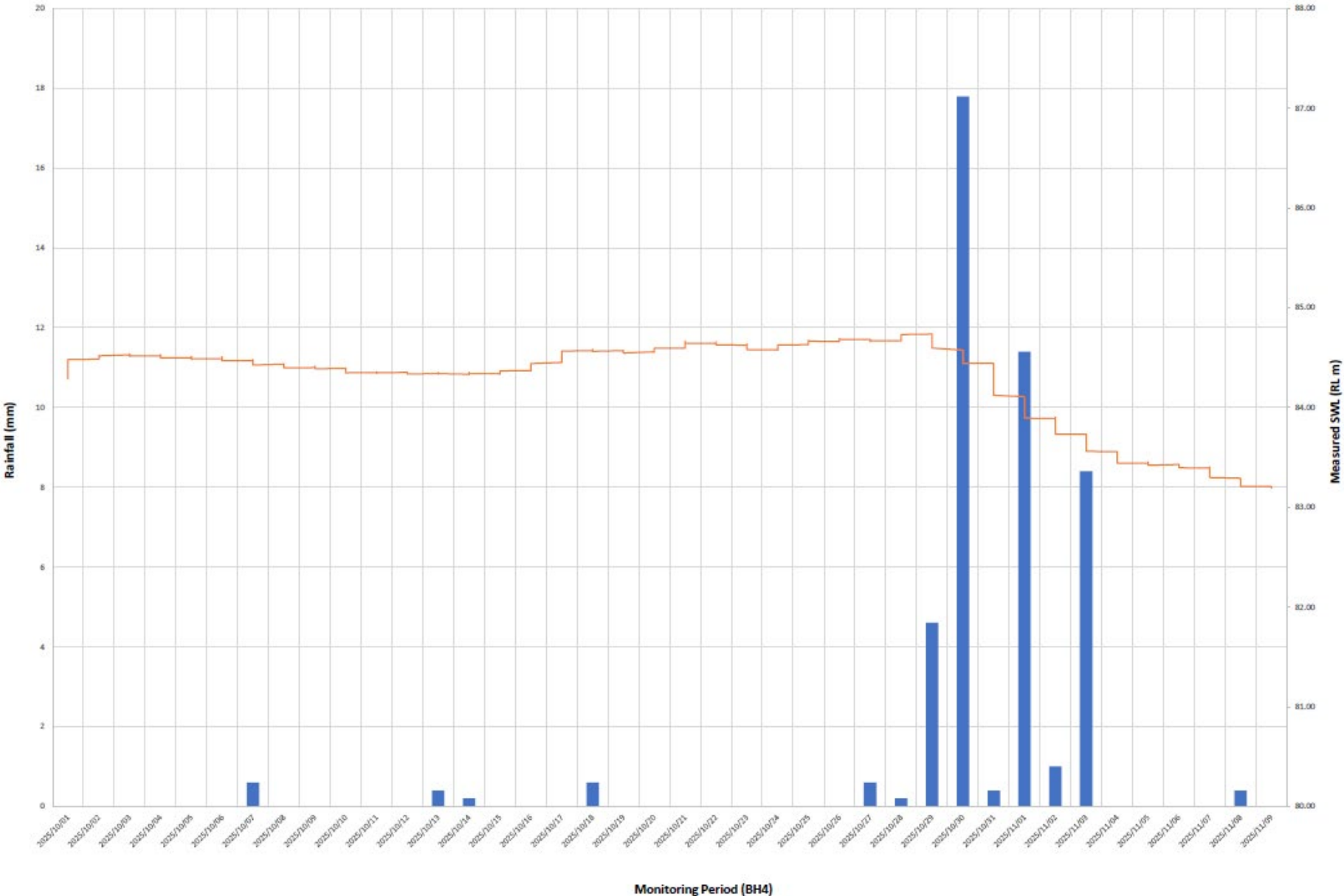
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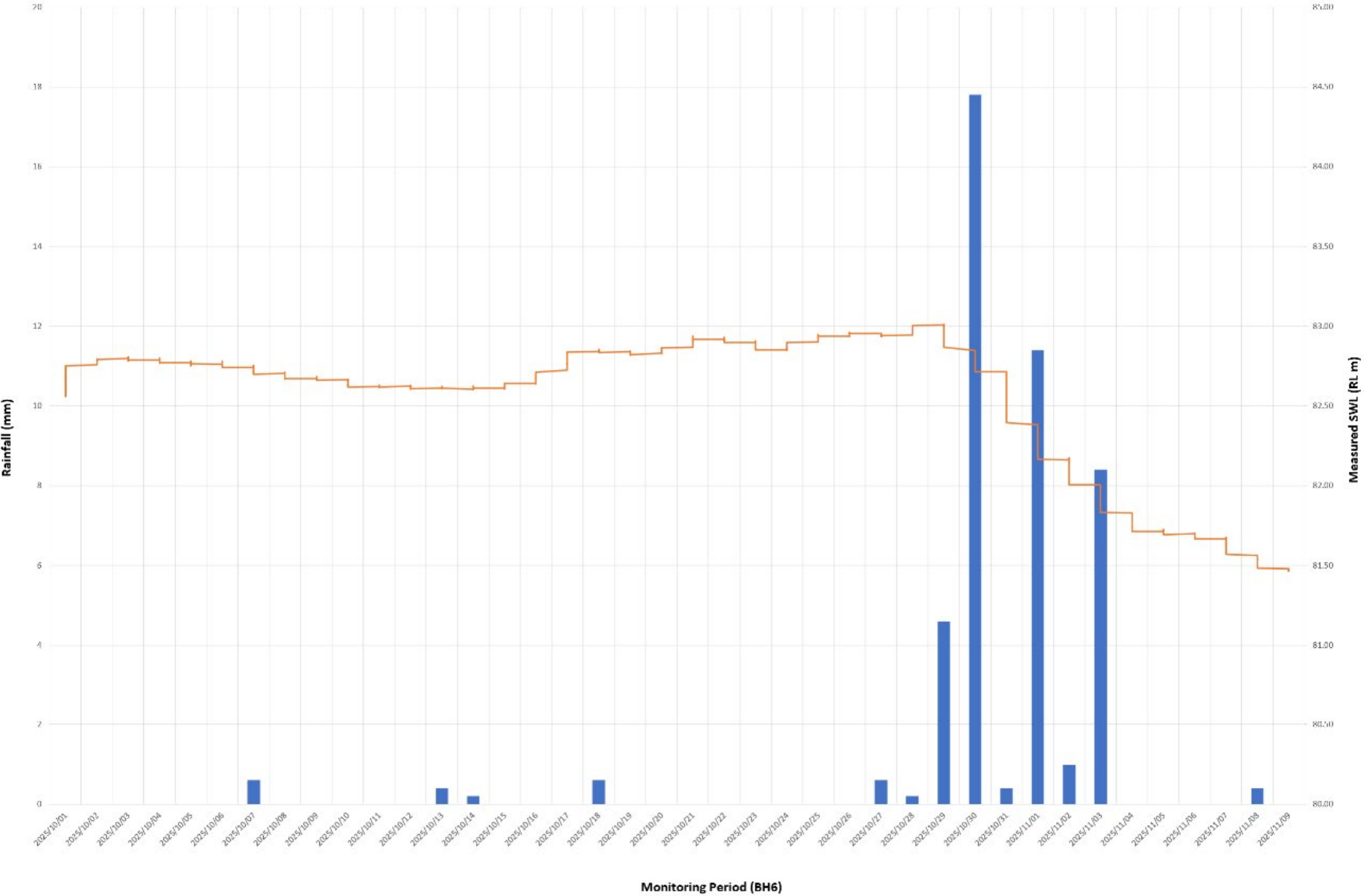
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Explanatory Notes

The methods of description and classification of soils and rocks used in this report are based on the Australian Standard AS1726-2017 Geotechnical Site Investigations. Material descriptions are deduced from field observation or engineering examination, and may be appended or confirmed by in situ or laboratory testing. The information is dependent on the scope of investigation, the extent of sampling and testing, and the inherent variability of the conditions encountered.

Subsurface investigation may be conducted by one or a combination of the following methods.

Method	
Test Pitting: excavation/trench	
BH	Backhoe bucket
EX	Excavator bucket
R	Ripper
H	Hydraulic Hammer
X	Existing excavation
N	Natural exposure
Manual drilling: hand operated tools	
HA	Hand Auger
Continuous sample drilling	
PT	Push tube
PS	Percussion sampling
SON	Sonic drilling
Hammer drilling	
AH	Air hammer
AT	Air track
Spiral flight auger drilling	
AS	Auger screwing
AD/V	Continuous flight auger: V-bit
AD/T	Continuous spiral flight auger: TC-Bit
HFA	Continuous hollow flight auger
Rotary non-core drilling	
WB	Washbore drilling
RR	Rock roller
Rotary core drilling	
PQ	85mm core (wire line core barrel)
HQ	63.5mm core (wire line core barrel)
NMLC	51.94mm core (conventional core barrel)
NQ	47.6mm core (wire line core barrel)
DT	Diatube (concrete coring)

Sampling is conducted to facilitate further assessment of selected materials encountered.

Sampling method	
Soil sampling	
B	Bulk disturbed sample
D	Disturbed sample
C	Core sample
ES	Environmental soil sample
SPT	Standard Penetration Test sample
U	Thin wall tube 'undisturbed' sample
WS	Environmental water sample
P	Piston Sampler

Field testing may be conducted as a means of assessment of the in situ conditions of materials.

Field testing	
SPT	Standard Penetration Test
HV/PP	Hane Vane (P-Peak R-Residual) / Pocket Penetrometer
Dynamic Penetrometers (blows per noted increment)	
DCP	Dynamic Cone Penetrometer
PSP	Perth Sand Penetrometer
MC	Moisture Content
VS	Vane Shear
PBT	Plate Bearing Test
SP	Single Packer Test
DP	Double Packer Test

If encountered, refusal (R), virtual refusal (VR) or hammer bouncing (HB) of penetrometers may be noted.

The quality of the rock can be assessed by the degree of natural defects/fractures and the following.

Rock quality description	
TCR	Total Core Recovery (%) (length of core recovered divided by the length of core run)
RQD	Rock Quality Designation (%) (sum of axial lengths of core greater than 100mm long divided by the length of core run)

Notes on groundwater conditions encountered may include.

Groundwater	
Not Encountered	Excavation is dry in the short term
Not Observed	Water level observation not possible
Seepage	Water seeping into hole
Inflow	Water flowing/flooding into hole

Perched groundwater may result in a misleading indication of the depth to the true water table. Groundwater levels are also likely to fluctuate with variations in climatic and site conditions.

Notes on the stability of excavations may include.

Excavation conditions	
Stable	No obvious/gross short term instability noted
Spalling	Material falling into excavation (minor/major)
Unstable	Collapse of the majority, or one or more face of the excavation

Explanatory Notes: General Soil Description

The methods of description and classification of soils used in this report are based on Australian Standard AS1726-2017 Geotechnical Site Investigations. In practice, a material is described as a soil if it can be remoulded by hand in its field condition. The dominant component is shown in upper case, with secondary components in lower case. In general descriptions cover: soil type, plasticity or particle size/shape, colour, strength or density, moisture and inclusions.

In general, soil types are classified according to the dominant particle on the basis of the following particle sizes.

Soil Classification		Particle Size (mm)
CLAY		< 0.002
SILT		0.002 to 0.075
SAND	fine	0.075 to 0.21
	medium	0.21 to 0.6
	coarse	0.6 to 2.36
GRAVEL	fine	2.36 to 6.7
	medium	6.7 to 19
	coarse	19 to 63
COBBLES		63 to 200
BOULDERS		> 200

Soil types may be qualified by the presence of minor components on the basis of field examination methods and/or the soil grading.

Terminology	In coarse grained soils		In fine soils
	% fines	% coarse	% coarse
Trace	≤5	≤15	≤15
With	>5, ≤12	>15, ≤30	>15, ≤30

The strength of cohesive soils is classified by engineering assessment or field/lab testing as follows.

Strength	Symbol	Undrained shear strength
Very Soft	VS	≤12kPa
Soft	S	12kPa to ≤25kPa
Firm	F	25kPa to ≤50kPa
Stiff	St	50kPa to ≤100kPa
Very Stiff	VSt	100kPa to ≤200kPa
Hard	H	>200kPa

Cohesionless soils are classified on the basis of relative density as follows.

Relative Density	Symbol	Density Index
Very Loose	VL	<15%
Loose	L	15% to ≤35%
Medium Dense	MD	35% to ≤65%
Dense	D	65% to ≤85%
Very Dense	VD	>85%

The plasticity of cohesive soils is defined by the Liquid Limit (LL) as follows.

Plasticity	Silt LL	Clay LL
Low plasticity	≤ 35%	≤ 35%
Medium plasticity	N/A	> 35% ≤ 50%
High plasticity	> 50%	> 50%

The moisture condition of soil (*w*) is described by appearance and feel and may be described in relation to the Plastic Limit (PL), Liquid Limit (LL) or Optimum Moisture Content (OMC).

Moisture condition and description	
Dry	Cohesive soils: hard, friable, dry of plastic limit. Granular soils: cohesionless and free-running
Moist	Cool feel and darkened colour: Cohesive soils can be moulded. Granular soils tend to cohere
Wet	Cool feel and darkened colour: Cohesive soils usually weakened and free water forms when handling. Granular soils tend to cohere

The structure of the soil may be described as follows.

Zoning	Description
Layer	Continuous across exposure or sample
Lens	Discontinuous layer (lenticular shape)
Pocket	Irregular inclusion of different material

The structure of soil layers may include: defects such as softened zones, fissures, cracks, joints and root-holes; and coarse grained soils may be described as strongly or weakly cemented.

The soil origin may also be noted if possible to deduce.

Soil origin and description	
Fill	Anthropogenic deposits or disturbed material
Topsoil	Zone of soil affected by roots and root fibres
Peat	Significantly organic soils
Colluvial	Transported down slopes by gravity/water
Aeolian	Transported and deposited by wind
Alluvial	Deposited by rivers
Estuarine	Deposited in coastal estuaries
Lacustrine	Deposited in freshwater lakes
Marine	Deposits in marine environments
Residual soil	Soil formed by in situ weathering of rock, with no structure/fabric of parent rock evident
Extremely weathered material	Formed by in situ weathering of geological formations, with the structure/fabric of parent rock intact but with soil strength properties

The origin of the soil generally cannot be deduced solely on the appearance of the material and the inference may be supplemented by further geological evidence or other field observation. Where there is doubt, the terms 'possibly' or 'probably' may be used

Explanatory Notes: General Rock Description

Description of Rock

- i. Rock name (BLOCK LETTERS)
- ii. Grain size and mineralogy
- iii. Colour
- iv. Fabric and texture
- v. Features, inclusions, minor components, moisture content and durability
- vi. Strength
- vii. Weathering and/or alteration
- viii. Rock mass properties – discontinuities and structure of rock
- ix. Interpreted stratigraphic unit
- x. Additional observations including geological structure

Simple rock names are used to provide a reasonable engineering description, rather than a precise geological classification and have been completed in general accordance with AS1726-2017. The rock name is chosen by considering the nature and shape of the grains or crystals, the texture and fabric of the rock material, the geological structure and setting, and information from the geological map of the area. Further guidance on the naming of rocks can be found in AS1726-2017, Tables 15, 16, 17 and 18. Typical rock types are described below, though subject to site specific variations.

Rock Type	Description	Example of Rock Name
Sedimentary	Formed by deposited beds of sediments, have grains that are cemented together and often rounded. Significant porosity	<p>COMMON: Conglomerate, Breccia, Sandstone, Mudstone, Siltstone, Claystone</p> <p>≥90% CARBONATE: Limestone, Dolomite, Calcirudite, Calcarenite, Calcsiltite, Calcilutite</p> <p>PYROCLASTIC: Agglomerate, Volcanic Breccia, Tuff</p>
Igneous	Formed from molten rock and have a crystalline texture. Typically massive and low porosity. Rock types are from coarse to fine grained.	<p>HIGH QUARTZ CONTENT: Granite, Microgranite, Rhyolite</p> <p>MODERATE QUARTZ CONTENT: Diorite, Microdiorite, Andesite</p> <p>LOW QUARTZ CONTENT: Gabbro, Dolerite, Basalt</p>

Metamorphic	Formed when rocks are subject to heat and/or pressure and have typically have directional fabric. Typically have low porosity and crystalline structure. Rock types are from coarse to fine grained	<p>FOLIATED: Gneiss, Schist, Phyllite, Slate</p> <p>NON-FOLIATED: Marble, Quartzite, Serpentinite, Hornfels</p>
Duricrust	Formed as part of a weathering profile and show evidence of being cemented in situ. Cementation is typically irregular and exhibits replacement textures.	<p>Ferricrete (Iron oxides and hydroxides)</p> <p>Silicrete (Silica)</p> <p>Calcrete (Calcium carbonate)</p> <p>Gypcrete (Gypsum)</p>

Note: () denotes dominant cementing mineralogy

Colour

Colour is described in the moist condition, using simple terms such as black, white, grey, red, brown, orange, yellow, purple, green, blue, etc. These may be modified as necessary, e.g. by 'pale', 'dark' or 'mottled'. Borderline colours are described as a combination of these colours. Refer to the core photographs accompanying borelogs for colour charts to assist with colour identification.

Grain Size

Terms describing dominate grain size in sedimentary rocks.

Term	Grain size
Coarse	Mainly 0.6 mm to 2 mm
Medium	Mainly 0.2 mm to 0.6 mm
Fine	Mainly 0.06mm (just visible) to 0.2 mm

Terms describing dominate grain size in igneous and metamorphic rocks

Term	Grain size
Coarse	Mainly greater than 2 mm
Medium	0.06 mm to 2 mm
Fine	Mainly less than 0.06 mm (just visible) to 0.2mm

Bedding and Fabric

Term	Definition
Massive	No obvious development of bedding – rock appears homogenous
Bedding	Layering produced by changes in sedimentation which may be defined by grain size, color or other features
Laminations	Similar to bedding but developed in layer thicknesses of less than 20mm
Foliation	The parallel arrangement of minerals due to metamorphic processes

Cleavage	A type of foliation developed in fine grained metamorphic rocks such as slates
Indistinct Fabric	There is little effect on strength - properties
Distinct Fabric	The rock may break more easily parallel to the fabric

Rock Strength

Term (Code)	UCS (MPa)	Is(50) (MPa)	Field Guide to Strength
Very Low (VL)	0.6 – 2	> 0.03 to ≤0.1	Material crumbles under firm blows with sharp end of pick; can be peeled with knife; too hard to cut a triaxial sample by hand. Pieces up to 3 cm thick can be broken by finger pressure.
Low (L)	2 - 6	> 0.1 to ≤ 0.3	Easily scored with a knife; indentations 1 mm to 3 mm show in the specimen with firm blow of the pick point; has dull sound under hammer. A piece of core 150 mm long 50 mm in diameter may be broken by hand. Sharp edges of core may be friable and break during handling.
Medium (M)	6 - 20	> 0.3 to ≤ 1.0	Readily scored with a knife; a piece of core 150 mm long by 50 mm in diameter can be broken by hand with difficulty.
High (H)	20 - 60	> 1 to ≤ 3	A piece of core 150 mm long by 50 mm in diameter cannot be broken by hand but can be broken by a pick with a single firm blow; rock rings under hammer.
Very High (VH)	60 - 200	> 3 to ≤ 10	Hand specimen breaks with pick after more than one blow; rock rings under hammer.
Extremely High (EH)	>200	> 10	Specimen requires many blows with geological pick to break through intact material; rock rings under hammer.

Rock strength is assessed by laboratory Uniaxial Compressive Strength (UCS) testing and/or Point Load Strength Index (PLT) testing to obtain the Is(50) the strength table implies a 20 times correlation between Is(50) and UCS used for classification. Note however, multiplier may range from 4 (e.g. some carbonated and low strength rocks) to 40 (e.g. some igneous rocks and/or some high strength rocks). A site specific correlation based on testing, previous investigation or literature may be used where the strength of the rock mass which may be considered weaker due to the available. These terms refer to the strength of the rock material and not to effect of rock defects.

Rock Weathering

Term (Code)	Definition
Residual soil (RS)	Soil developed on extremely weathered rock. The rock mass structure and substance fabric are no longer evident but the soil has not been significantly transported.
Extremely weathered (EW)	Rock is weathered to such an extent that it has 'soil' properties, i.e, it either disintegrates or can be remoulded in water, b the texture of original rock is still evident.

Highly weathered (HW)	Whole rock material is discoloured usually by extent that iron staining or bleaching and other signs of chemical or physical decomposition are evident. Porosity and strength may be increased or decreased compared to the fresh rock usually as a result of iron leaching or deposition. The colour and strength of the original rock substance is no longer recognisable	*Where is it not practical to distinguish between 'HW' and MW'. Rock strength usually changed by weathering. The rock may be highly discoloured, usually by iron staining. Porosity may be increased by leaching, or may be decreased due to deposition of weathering products in pores
Distinctly weathered (DW)*	Whole rock material is discoloured usually by staining that original colour of the fresh rock is no longer recognisable	
Moderately weathered (MW)		
Slightly weathered (SW)	Rock is slightly discoloured but shows little or no change of strength from fresh rock	
Fresh rock (F)	Rock shows no sign of decomposition or staining	

Rock Alteration

Term (Code)	Definition	
Extremely altered (XA)	Rock is altered to such an extent that it has 'soil' properties, i.e, it either disintegrates or can be remoulded in water, b the texture of original rock is still evident.	
Highly Altered (HA)	Whole rock material is discoloured usually by extent that iron staining or bleaching and other signs of chemical or physical decomposition are evident. Porosity and strength may be increased or decreased compared to the fresh rock usually as a result of iron leaching or deposition. The colour and strength of the original rock substance is no longer recognisable	*Where is it not practical to distinguish between 'HA' and MA'. Rock strength usually changed by alteration. The rock may be highly discoloured, usually by iron staining. Porosity may be increased by leaching, or may be decreased due to deposition of
Distinctly Altered (DA)		

Moderately Altered (MA)	Whole rock material is discoloured usually by staining that original colour of the fresh rock is no longer recognisable	weathering products in pores
Slightly altered (SA)	Rock is slightly discoloured but shows little or no change of strength from fresh rock	

Rock Core Recovery

TCR = Total Core Recovery (%)

$$\frac{\text{Length of Core Recovered}}{\text{Length of Core Run}} \times 100$$

SCR = Solid Core Recovery (%)

$$\frac{\text{Sum Length of Cylindrical Core Recovered}}{\text{Length of Core Run}} \times 100$$

RQD = Rock Quality Designation (%)

$$\frac{\text{Sum Length of Sound Core Pieces > 100mm in length}}{\text{Length of Core Run}} \times 100$$

Types of Discontinuities

Term	Code	Description
Parting	BP	A surface or crack across which the rock has little or no tensile strength. Parallel or sub-parallel to layering (e.g. bedding) or a planar anisotropy in the rock material (e.g. cleavage). May be open or closed.
Joint	JT	A surface or crack with no apparent shear displacement and across which the rock has little or no tensile strength, but which is not parallel or sub-parallel to layering or to planar anisotropy in the rock material. May be open or closed.
Sheared Surface	SS	A near planar, curved or undulating surface which is usually smooth, polished or slickensided and which show evidence of shear displacement
Shear Zone	SZ	A zone with roughly parallel planar boundaries of rock substance consisting of closely spaced joints with smooth slickensided surfaces often curved. The joints divide the rock mass into unit blocks usually of lenticular or wedge shape.

Sheared Seam	SSe	Seam of soil material with roughly parallel almost planar boundaries, composed of soil materials with roughly parallel near planar, curved or undulating boundaries cut by closely spaced joints, sheared surfaces or other defects. Some of the defects are usually curved and intersect to divide the mass into lenticular or wedge-shaped blocks.
Crushed Seam	CS	A zone or seam with roughly parallel planar boundaries of rock substance composed of disoriented, usually angular, fragments of the host rock substance which may be more weathered than the host rock. The seam has soil properties
Infilled Seam	IS	Seam of soil material usually with distinct roughly parallel boundaries formed by the migration of soil into an open cavity or joint, infilled seams less than 1 mm thick may be described as a veneer or coating on a joint surface.
Extremely Weathered Seam	EWS	Seam of soil material, often with gradational boundaries. Formed by weathering of the rock material in place.
Seam	SM	Seam of soil material, often with gradational boundaries whereby the origin is not able to be distinguished in the field

Note: RMS specific terms such as drill breaks (DB) and handling breaks (HB) are used on the logs which are not strictly defined by AS1726-2017. These breaks are not included as natural discontinuity for fracture spacing calculations

Discontinuity Observation

Term	Code	Description
Clean	CN	No visible coating or infill
Stain	SN	No visible coating or infill but surfaces are discoloured by mineral staining
Veneer	VNR	A visible coating or soil or mineral substance but usually unable to be measured. If discontinuous over the plane, patchy veneer.
Coating <1 mm	CT	A visible coating or infilling of soil or mineral substance. Describe composition and thickness.
Filling (Filled) >1 mm	FLD	A visible filling of soil or mineral substance. Describe composition and

Infill Material

Code	Description	Code	Description
Ca	Calcite	Gp	Gypsum
Ch	Chlorite	Mn	Manganese
Cl	Clay	MS	Secondary mineral

Co	Coal	Py	Pyrite
Fe	Limonite / Ironstone	Um	Unidentified mineral
Fe Cl	Iron oxide clay	Qz	Quartz
Fl	Feldspar	X	Carbonaceous

Visual log

A diagrammatic plot of defects showing type, spacing and orientation in relation to the core axis.

- Defects open in situ or clay sealed
- Defects closed in-situ
- Drill induced fractures or handling breaks
- Infilled seam

Discontinuity Planarity

Term	Definition
CU	Curved – A defect with a gradual change in orientation
IR	Irregular – A defect with many sharp changes in orientation
PL	Planar – Defect forms a continuous plane without variation in orientation
ST	Stepped – A defect with distinct sharp steps or step
UN	Undulose – A defect with undulations

Water

WATER			
	Water level at date shown		Partial water loss
	Water inflow		Complete water loss

Discontinuity Roughness

Abbreviation	Description
RF	Rough – Many small surface irregularities generally related to the grain size of the parent rock
SM	Smooth – Few or no surface irregularities related to the grain size of the parent rock
PO	Polished – Planes have a distinct sheen or a smoothness
S	Slickensided – Planes have a polished, grooved or striated surface consistent with differential movement of the parent rocks along the plane
VR	Very rough – many large surface irregularities, amplitude generally more than 1mm

Discontinuity Spacing

Spacing (mm)	Description
>6000	Extremely Widely Spaced
2000 - 6000	Very Widely Spaced
600 - 2000	Widely Spaced
200 - 600	Medium Spaced
60 - 200	Closely Spaced
20 - 60	Very Closely Spaced
<20	Extremely Closely Spaced

NON-CORE DRILL HOLE - GEOLOGICAL LOG

HOLE NO : BH1
 FILE / JOB NO : 2025181
 SHEET : 1 OF 3

PROJECT : Proposed Senior Living Development
 LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334191.41, N: 6255814.00 (56 MGA2020) SURFACE ELEVATION : 84.34 (AHD) ANGLE FROM HORIZONTAL : 90°
 RIG TYPE : HANJIN DB8 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : AW
 DATE STARTED : 23/9/25 DATE COMPLETED : 23/9/25 DATE LOGGED : 23/9/25 LOGGED BY : PL CHECKED BY : TH

DRILLING					MATERIAL						
PROGRESS		DRILLING PENETRATION	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION	CONSISTENCY RELATIVE DENSITY	STRUCTURE & Other Observations
DRILLING & CASING	WATER										
ADT ↑	HC Casing ↑	E	Not Encountered	1.00m SPT 3, 3, 4 N=7	0.0 84.3	[Cross-hatched pattern]	0.10m	SPRAY SEAL: 5 to 10mm aggregate size	=PL		ROAD SURFACE FILL
		F		1.45m 2.10m ES	1.0 83.3 2.0 82.3	[Diagonal lines pattern]	1.80m	CLAYEY SAND: pale grey to yellow, fine to medium grained sand, medium to high plasticity clay, trace of ironstones, sub-angular, fine to medium grained	M	MD	RESIDUAL SOIL
		H		2.40m	2.40m	[Dotted pattern]	2.40m	SANDSTONE: recovered as clayey sand, pale grey to yellow, fine to medium grained, medium to high plasticity, with sandstone gravel, angular to sub-angular, fine to medium grained, inferred extremely weathered to highly weathered, very low strength			WEATHERED ROCK
					2.50m		2.50m	Continued as Cored Drill Hole			
					3.0 81.3						
					4.0 80.3						
					5.0 79.3						
					6.0 78.3						
					7.0 77.3						
					8.0 76.3						

RMS LIB 40.3 EXTERNAL REV1.3.GLB Log RTA NON-CORE DRILL HOLE 2 GINT FILE_19-23 ROSALIND STREET, CAMMERAY.GPJ <DrawingFile> 16/Oct/2025 12:48 10.03.00.09

See Explanatory Notes for details of abbreviations & basis of descriptions.

ELITE GEOSCIENCES PTY LTD



CORED DRILL HOLE LOG

HOLE NO : BH1
FILE / JOB NO : 2025181
SHEET : 2 OF 3

PROJECT : Proposed Senior Living Development
LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334191.41, N: 6255814.00 (56 MGA2020) SURFACE ELEVATION : 84.34 (AHD) ANGLE FROM HORIZONTAL : 90°
RIG TYPE : HANJIN DB8 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : AW
DATE STARTED : 23/9/25 DATE COMPLETED : 23/9/25 DATE LOGGED : 23/9/25 LOGGED BY : PL CHECKED BY : TH
CASING DIAMETER : HQ BARREL (Length) : 3.00 m BIT : Step BIT CONDITION : Good

DRILLING				MATERIAL				BIT CONDITION						
PROGRESS		NO CORE DRILL PER RUN %	ROD (%)	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	DESCRIPTION ROCK TYPE : Colour, Grain size, Structure (texture, fabric, mineral composition, hardness alteration, cementation, etc as applicable)	Weathering	ESTIMATED STRENGTH Is(50)				NATURAL FRACTURE (mm)	ADDITIONAL DATA (joints, partings, seams, zones, etc) Description, orientation, infilling or coating, shape, roughness, thickness, other
DRILLING & CASING	WATER								VL	L	M	H		
					0.0 84.3									
					1.0 83.3									
					2.0 82.3									
					2.50m		2.50m START CORING AT 2.50m							
		0% NO CORE	74	Is(50) d=0.06 a=0.13 MPa	3.0 81.3		SANDSTONE: orange to brown, pale grey to grey, medium grained, bedded at 0° - 5°, with ironstains	HW to MW					2.60: BP 5° CN PR S 2.62: BP 0 - 5° CN PR S 2.72: BP 5° CN PR S 2.80: BP 0° CN PR S 2.90: BP 0° CN IR S	
				UCS =7.9 MPa	4.0 80.3			MW					3.08: BP 0° CN PR S 3.14: BP 0° CN IR S 3.23: BP 0° CN PR S 3.27: BP 10° CN PR S 3.40: BP 5° CN PR S 3.48: BP 0° CN PR S 3.60: BP 0° CN IR S 3.72: BP 10° CN PR S 3.88: JT 20° CN PR S 3.95: BP 10° CN PR S	
				Is(50) d=0.4 a=0.54 MPa	5.0 79.3								4.15: BP 0° CN PR S 4.27: BP 0° CN IR S 4.29: BP 0° CN PR S 4.46: BP 0 - 5° CN PR S 4.48: BP 0° CN PR S 4.70: BP 0° CN PR S 4.85: BP 0° CN PR S 4.97: BP 5° CN IR S 4.99: BP 0° CN IR S 5.10: BP 0° CN PR S 5.12: BP 0° CN PR S 5.14: BP 0° CN PR S 5.19: BP 0° CN PR S 5.30: JT 20° CN PR S	
		0% NO CORE	87	Is(50) d=0.26 a=0.24 MPa	5.50		5.25m SANDSTONE: pale grey to grey, red to brown, orange to brown, medium grained, bedded at 0° - 5°, with iron stains, 5% to 10% of carbonaceous lamination at 0° - 5°, grey						5.60: BP 10° CN PR S 5.90: BP 0° CN PR S 5.92: BP 0° CN PR S 6.18: BP 0° CN PR S 6.40: BP 0° CN PR S 6.43: JT 30° CN IR S 6.48: BP 0° CN IR S 6.60: BP 0° CN IR S 6.72: BP 0° CN PR S	
				Is(50) d=0.19 a=0.48 MPa	6.0 78.3								6.98: BP 0° CN PR S 7.22: BP 0° CN PR S 7.30: BP 0 - 5° CN PR S 7.38: BP 0° CN PR S 7.57: BP 0° CN PR S 7.72: BP 0 - 5° CN PR S 7.85: BP 0° CN PR S	
					7.0 77.3			SW						
					7.90m									
					8.0 76.3									

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See Explanatory Notes for details of abbreviations & basis of descriptions.



CORED DRILL HOLE LOG

HOLE NO : BH1
FILE / JOB NO : 2025181
SHEET : 3 OF 3

PROJECT : Proposed Senior Living Development
LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334191.41, N: 6255814.00 (56 MGA2020) SURFACE ELEVATION : 84.34 (AHD) ANGLE FROM HORIZONTAL : 90°
RIG TYPE : HANJIN DB8 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : AW
DATE STARTED : 23/9/25 DATE COMPLETED : 23/9/25 DATE LOGGED : 23/9/25 LOGGED BY : PL CHECKED BY : TH
CASING DIAMETER : HQ BARREL (Length) : 3.00 m BIT : Step BIT CONDITION : Good

DRILLING				MATERIAL			BIT CONDITION		
PROGRESS	ESTIMATED STRENGTH (s(50))		NATURAL FRACTURE (mm)	ADDITIONAL DATA		Weathering	DEPTH (m)	GRAPHIC LOG	DESCRIPTION
DRILLING & CASING	WATER	(NO CORE) DRILL DEPTH		RQD (%)	SAMPLES & FIELD TESTS				
	0% LOSS	0% NO CORE	87	Is(50) d=0.45 a=0.76 MPa	UCS =11 MPa	SW	8.0 78.3		SANDSTONE: pale brown to grey, pale grey to grey, medium to coarse grained, bedded at 0° - 10°, with iron stains, 5% to 10% of carbonaceous lamination at 0° - 10°, grey (continued)
	0% LOSS	0% NO CORE	92	Is(50) d=0.41 a=0.77 MPa			9.0 75.3		
	0% LOSS	0% NO CORE	73	Is(50) d=0.65 a=0.68 MPa			11.0 73.3		
	0% LOSS	0% NO CORE		Is(50) d=0.71 a=1.14 MPa			12.0 72.3		
	0% LOSS	0% NO CORE					13.0 71.3	13.20m	SANDSTONE: orange to brown, pale grey to grey, medium to coarse grained, bedded at 0° - 20°, 10% to 15% of carbonaceous lamination at 0° - 20°, dark grey, with ironstains
	0% LOSS	0% NO CORE					14.0 70.3	14.74m	
							15.0 69.3		BOREHOLE BH1 TERMINATED AT 14.74 m Target depth Well installed upon completion of borehole
							16.0 68.3		

RMS LIB 40.3 EXTERNAL REV1.3.GLB Log RTA CORED DRILL HOLE 5 GINTI FILE_19-23 ROSALIND STREET, CAMMERAY.GPJ <<DrawingFiles>> 16/Oct/2025 12:42 10.03.00.09

See Explanatory Notes for details of abbreviations & basis of descriptions.



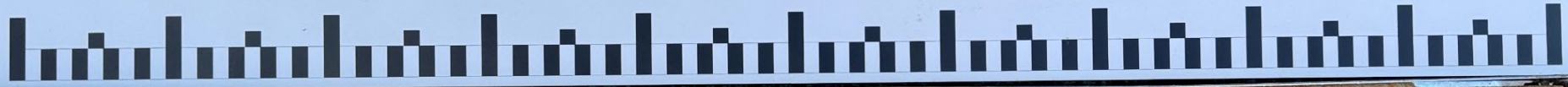
ELITE GEOSCIENCES

Project: 19-23 ROSALIND ST CAMMERAY
Project Number: 2025181

BH ID: BH1
Depth: 2.5m to 7.0m
Core Tray No.: 1
Date: 23/09/25



X Chalk marks denote handling or drilling breaks



ELITE GEOSCIENCES	TITLE: Borehole Core Photographs - 19-23 Rosalind Street, Cammeray			
	PROJECT NO: 2025181	TEST DATE: 23/09/2025	INCLINATION: -90 degree	CORE LENGTH: 2.50m to 7.00m
	DRILL RIG: RIG 32	CONTRACTOR: BG Drilling	LOGGED BY: PL	CHECKED BY: TH

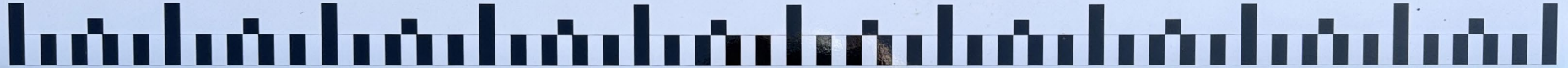
ELITE GEOSCIENCES

Project: 19-23 ROSALIND ST CAMMERAY
Project Number: 2025181

BH ID: BH1
Depth: 7.0m to 12.0m
Core Tray No.: 2
Date: 23/09/25



✗ Chalk marks denote handling or drilling breaks



TITLE:

Borehole Core Photographs - 19-23 Rosalind Street, Cammeray

ELITE GEOSCIENCES

PROJECT NO:
2025181

TEST DATE:
23/09/2025

INCLINATION:
-90 degree

CORE LENGTH:
7.00m to 12.00m

DRILL RIG:
RIG 32

CONTRACTOR:
BG Drilling

LOGGED BY:
PL

CHECKED BY:
TH

ELITE GEOSCIENCES

Project: 19-23 ROSALIND ST CAMMERAY
Project Number: 2025181

BH ID: BH1
Depth: 12.00m to 14.74m
Core Tray No.: 3
Date: 23/09/25



✗ Chalk marks denote handling or drilling breaks



ELITE GEOSCIENCES	TITLE: Borehole Core Photographs - 19-23 Rosalind Street, Cammeray			
	PROJECT NO: 2025181	TEST DATE: 23/09/2025	INCLINATION: -90 degree	CORE LENGTH: 12.00 m to 14.74m
	DRILL RIG: RIG 32	CONTRACTOR: BG Drilling	LOGGED BY: PL	CHECKED BY: TH

NON-CORE DRILL HOLE - GEOLOGICAL LOG

HOLE NO : BH2
 FILE / JOB NO : 2025181
 SHEET : 1 OF 3

PROJECT : Proposed Senior Living Development
 LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334180.40, N: 6255784.49 (56 MGA2020) SURFACE ELEVATION : 85.34 (AHD) ANGLE FROM HORIZONTAL : 90°
 RIG TYPE : HANJIN DB8 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : AW
 DATE STARTED : 23/9/25 DATE COMPLETED : 23/9/25 DATE LOGGED : 23/9/25 LOGGED BY : PL CHECKED BY : TH

DRILLING					MATERIAL						
PROGRESS		DRILLING PENETRATION	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION	CONSISTENCY RELATIVE DENSITY	STRUCTURE & Other Observations
DRILLING & CASING	WATER										
AD/T HQ Casing		E	Not Encountered	0.50m SPT 2.3.3 N=6	0.0			0.10m SPRAY SEAL: 5 to 10mm aggregate size	=PL	St	ROAD SURFACE
					0.95m			FILL: SANDY CLAY: brown to red, yellow to grey, medium to high plasticity, fine to medium grained sand, trace sub-angular, fine to medium grained gravel			FILL
					1.20m			SANDY CLAY: orange to brown, medium to high plasticity, fine to medium grained sand			RESIDUAL SOIL
					1.50m			SANDSTONE: recovered as sandy clay, orange to brown, medium to high plasticity, fine to medium grained, with sandstone gravel, angular to sub-angular, fine to medium grained, inferred extremely weathered to highly weathered, very low strength			WEATHERED ROCK
					1.60m			Continued as Cored Drill Hole			
					2.0						
					3.0						
					4.0						
					5.0						
					6.0						
					7.0						
					8.0						

RMS LIB 40.3 EXTERNAL REV1.3.GLB Log RTA NON-CORE DRILL HOLE 2 GINT FILE_19-23 ROSALIND STREET, CAMMERAY.GPJ <DrawingFile>_16/Oct/2025 12:48 10.03.00.09

See Explanatory Notes for details of abbreviations & basis of descriptions.

ELITE GEOSCIENCES PTY LTD



CORED DRILL HOLE LOG

HOLE NO : BH2
FILE / JOB NO : 2025181
SHEET : 2 OF 3

PROJECT : Proposed Senior Living Development
LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334180.40, N: 6255784.49 (56 MGA2020) SURFACE ELEVATION : 85.34 (AHD) ANGLE FROM HORIZONTAL : 90°
RIG TYPE : HANJIN DB8 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : AW
DATE STARTED : 23/9/25 DATE COMPLETED : 23/9/25 DATE LOGGED : 23/9/25 LOGGED BY : PL CHECKED BY : TH
CASING DIAMETER : HQ BARREL (Length) : 3.00 m BIT : Step BIT CONDITION : Good

DRILLING				MATERIAL				BIT CONDITION						
PROGRESS		NO CORE DRILL PER RUN %	ROD (%)	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	DESCRIPTION ROCK TYPE : Colour, Grain size, Structure (texture, fabric, mineral composition, hardness alteration, cementation, etc as applicable)	Weathering	ESTIMATED STRENGTH Is(50)				NATURAL FRACTURE (mm)	ADDITIONAL DATA (joints, partings, seams, zones, etc) Description, orientation, infilling or coating, shape, roughness, thickness, other
DRILLING & CASING	WATER								VL	L	M	H		
					0.0 85.3									
					1.0 84.3									
					1.60m		1.60m START CORING AT 1.60m							
		0% NO CORE	71	Is(50) d=0.33 a=0.83 MPa	2.0 83.3		SANDSTONE: pale brown to orange, medium grained, bedded at 0° - 5°, with ironstains	EW MW					1.60-1.66: EWS 1.77: BP 0° CN PR S 1.84: BP 0° CN PR S 1.91: BP 0° CN PR S 2.10: BP 0° CN IR S 2.15: BP 0° CN PR S 2.23: BP 0° CN IR S 2.26: BP 0° CN PR S 2.40: BP 0° CN PR S	
		0% NO CORE	92	Is(50) d=0.85 a=0.96 MPa	3.0 82.3		SANDSTONE: pale grey, red to brown, medium grained, bedded at 0° - 5°, with ironstains						2.74: BP 0 - 5° CN PR S 2.81: BP 0 - 5° CN PR S 2.93: BP 5° CN IR S 2.95: BP 5° CN PR S 3.13: BP 0 - 5° CN PR S 3.18: BP 0° CN PR S 3.29: BP 0° CN PR S 3.39: BP 0° CN PR S 3.67: BP 0° CN PR S	
		0% NO CORE		UCS =13 MPa Is(50) d=0.1 a=0.35 MPa	4.0 81.3		SANDSTONE: pale grey, pale brown, medium grained, bedded at 0° - 5°, trace of carbonaceous lamination, trace of ironstains						3.98-4.00: CZ 4.10: BP 0 - 5° CN PR S 4.31: BP 5° CN PR S 4.53: BP 0° CN IR S 4.71: BP 0° CN IR S 4.94: BP 5° CN PR S 5.29: BP 0° CN PR S	
		0% NO CORE	93	Is(50) d=0.34 a=0.44 MPa	5.0 80.3		SANDSTONE: orange to brown, pale grey, medium to coarse grained, bedded at 0° - 10°, with ironstains, 5% to 10% of carbonaceous lamination at 0° - 10°, dark grey, with ironstains						5.64: BP 0° CN PR S 5.79: BP 0 - 5° CN PR S 5.92: BP 0° CN PR S 6.01: BP 0 - 5° CN PR S 6.37: BP 0° CN PR S 6.62: BP 0° CN PR S	
		0% NO CORE		UCS =9 MPa Is(50) d=0.26 a=0.39 MPa	6.0 79.3		SANDSTONE: orange to brown, pale grey, medium to coarse grained, bedded at 0° - 10°, with ironstains, 5% to 10% of carbonaceous lamination at 0° - 10°, dark grey, with ironstains						6.37: BP 0° CN PR S 6.62: BP 0° CN PR S	
					7.0 78.3								7.02-7.06: CZ 7.31: BP 0 - 5° CN PR S 7.35: BP 0° CN PR S	
					8.0 77.3								7.80: BP 5° CN PR S	

See Explanatory Notes for details of abbreviations & basis of descriptions.

ELITE GEOSCIENCES PTY LTD



RMS LIB 40.3 EXTERNAL REV1.3.GLB Log RTA CORED DRILL HOLE 5 GINT FILE_19-23 ROSALIND STREET, CAMMERAY.GPJ <<DrawingFiles>> 16/Oct/2025 12:42 10.03.00.09

CORED DRILL HOLE LOG

HOLE NO : BH2
FILE / JOB NO : 2025181
SHEET : 3 OF 3

PROJECT : Proposed Senior Living Development
LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334180.40, N: 6255784.49 (56 MGA2020) SURFACE ELEVATION : 85.34 (AHD) ANGLE FROM HORIZONTAL : 90°
RIG TYPE : HANJIN DB8 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : AW
DATE STARTED : 23/9/25 DATE COMPLETED : 23/9/25 DATE LOGGED : 23/9/25 LOGGED BY : PL CHECKED BY : TH
CASING DIAMETER : HQ BARREL (Length) : 3.00 m BIT : Step BIT CONDITION : Good

DRILLING				MATERIAL				FRACTURES			
PROGRESS		NO CORE (% PER RUN %)	ROD (%)	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	DESCRIPTION ROCK TYPE : Colour, Grain size, Structure (texture, fabric, mineral composition, hardness alteration, cementation, etc as applicable)	Weathering	ESTIMATED STRENGTH Is(50) ● Axial ○ Diametral	NATURAL FRACTURE (mm)	ADDITIONAL DATA (joints, partings, seams, zones, etc) Description, orientation, infilling or coating, shape, roughness, thickness, other
DRILLING & CASING	WATER										
NMLC	0% LOSS	0% NO CORE	93		8.0 77.3	●	SANDSTONE: orange to brown, pale grey, medium to coarse grained, bedded at 0° - 10°, with ironstains, 5% to 10% of carbonaceous lamination at 0° - 10°, dark grey, with ironstains (<i>continued</i>)	MW	●	8.25: BP 0° CN IR S 8.45: BP 0° CN PR S 8.65: BP 0° CN PR S 9.20: BP 5 - 10° CN PR S	
	0% LOSS	0% NO CORE	100	Is(50) d=0.28 a=0.39 MPa	9.0 76.3	●	SANDSTONE: pale grey to grey, medium to coarse grained, bedded at 0° - 10°, 5% to 10% of carbonaceous lamination at 0° - 10°, dark grey	SW	●	9.91: BP 0° CN IR S 10.10: BP 0° CN PR S	
		10.23		Is(50) d=0.57 a=0.87 MPa	10.0 75.3	●	BOREHOLE BH2 TERMINATED AT 10.23 m Target depth Backfilled with spoil				
					11.0 74.3						
					12.0 73.3						
					13.0 72.3						
					14.0 71.3						
					15.0 70.3						
					16.0 69.3						

RMS LIB 40.3 EXTERNAL REV1.3.GLB Log RTA CORED DRILL HOLE 5 GINTI FILE_19-23 ROSALIND STREET, CAMMERAY.GPJ <-DrawingFiles> 16/Oct/2025 12:42 10.03.00.09

See Explanatory Notes for details of abbreviations & basis of descriptions.



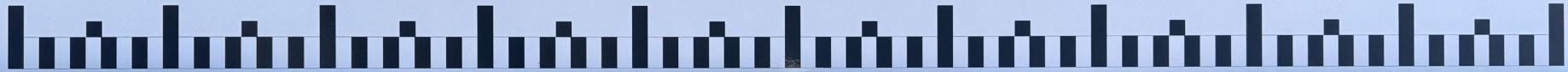
ELITE GEOSCIENCES

Project: 19-23 ROSALIND ST CAMMERAY
Project Number: 2025181

BH ID: BH2
Depth: 1.6m to 6.0m
Core Tray No.: 1
Date: 23/09/25



X Chalk marks denote handling or drilling breaks



BH2 STARTS CORING AT 1.60m

2m

3m

4m

5m

TITLE:

Borehole Core Photographs - 19-23 Rosalind Street, Cammeray

PROJECT NO:
2025181

TEST DATE:
23/09/2025

INCLINATION:
-90 degree

CORE LENGTH:
1.60m to 6.00m

DRILL RIG:
RIG 32

CONTRACTOR:
BG Drilling

LOGGED BY:
PL

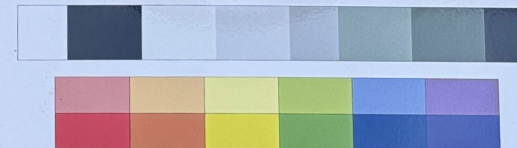
CHECKED BY:
TH

ELITE GEOSCIENCES

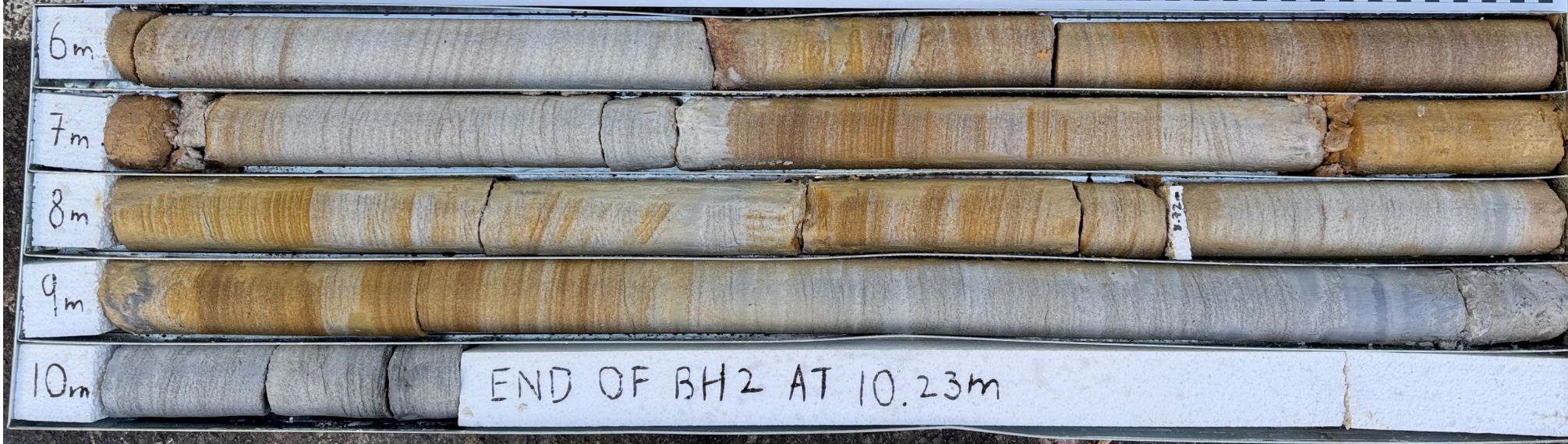
ELITE GEOSCIENCES

Project: 19-23 ROSALIND ST CAMMERAY
Project Number: 2025181

BH ID: BH2
Depth: 6.00_m to 10.23m
Core Tray No.: 2
Date: 23/09/25



✗ Chalk marks denote handling or drilling breaks



ELITE GEOSCIENCES	TITLE: Borehole Core Photographs - 19-23 Rosalind Street, Cammeray			
	PROJECT NO: 2025181	TEST DATE: 23/09/2025	INCLINATION: -90 degree	CORE LENGTH: 6.00m to 10.23m
	DRILL RIG: RIG 32	CONTRACTOR: BG Drilling	LOGGED BY: PL	CHECKED BY: TH

NON-CORE DRILL HOLE - GEOLOGICAL LOG

HOLE NO : BH3
 FILE / JOB NO : 2025181
 SHEET : 1 OF 3

PROJECT : Proposed Senior Living Development
 LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334181.20, N: 6255731.27 (56 MGA2020) SURFACE ELEVATION : 86.67 (AHD) ANGLE FROM HORIZONTAL : 90°
 RIG TYPE : HANJIN DB8 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : AW
 DATE STARTED : 24/9/25 DATE COMPLETED : 24/9/25 DATE LOGGED : 24/9/25 LOGGED BY : PL CHECKED BY : TH

DRILLING					MATERIAL				
PROGRESS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION	CONSISTENCY	RELATIVE DENSITY	STRUCTURE & Other Observations	
									DRILLING & CASING
ADT HQ Casing	86.7	0.10m	SP	SPRAY SEAL: 5 to 10mm aggregate size				ROAD SURFACE	
E	85.7	1.00m	SPT	FILL: SANDY CLAY: brown to red, yellow to grey, medium to high plasticity, fine to medium grained sand, trace sub-angular, fine to medium grained gravel	=PL			FILL	
Not Encountered	85.7	1.00m	SPT					1.00: SPT Recovery: 450 m	
F	85.7	1.45m	ES	CLAYEY SAND: pale grey to grey, fine to medium grained sand, medium to high plasticity clay				RESIDUAL SOIL	
H	84.7	1.90m	SC	SANDSTONE: recovered as clayey sand, pale grey to grey, fine to medium grained, medium to high plasticity, with sandstone gravel, angular to sub-angular, fine to medium grained, inferred extremely weathered to highly weathered, very low strength				WEATHERED ROCK	
	84.7	2.00m		Continued as Cored Drill Hole					
	83.7	3.0							
	82.7	4.0							
	81.7	5.0							
	80.7	6.0							
	79.7	7.0							
	78.7	8.0							

RMS LIB 40.3 EXTERNAL REV1.3.GLB Log RTA NON-CORE DRILL HOLE 2 GINT FILE_19-23 ROSALIND STREET, CAMMERAY.GPJ <DrawingFile>_16/Oct/2025 12:48 10.03.00.09

See Explanatory Notes for details of abbreviations & basis of descriptions.

ELITE GEOSCIENCES PTY LTD



CORED DRILL HOLE LOG

HOLE NO : BH3
FILE / JOB NO : 2025181
SHEET : 2 OF 3

PROJECT : Proposed Senior Living Development
LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334181.20, N: 6255731.27 (56 MGA2020) SURFACE ELEVATION : 86.67 (AHD) ANGLE FROM HORIZONTAL : 90°
RIG TYPE : HANJIN DB8 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : AW
DATE STARTED : 24/9/25 DATE COMPLETED : 24/9/25 DATE LOGGED : 24/9/25 LOGGED BY : PL CHECKED BY : TH
CASING DIAMETER : HQ BARREL (Length) : 3.00 m BIT : Step BIT CONDITION : Good

DRILLING				MATERIAL				BIT CONDITION						
PROGRESS		NO CORE (% PER RUN %)	ROD (%)	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	DESCRIPTION ROCK TYPE : Colour, Grain size, Structure (texture, fabric, mineral composition, hardness alteration, cementation, etc as applicable)	Weathering	ESTIMATED STRENGTH Is(50)				NATURAL FRACTURE (mm)	ADDITIONAL DATA (joints, partings, seams, zones, etc) Description, orientation, infilling or coating, shape, roughness, thickness, other
DRILLING & CASING	WATER								VL	L	M	H		
					0.0 86.7									
					1.0 85.7									
					2.0 84.7		2.00m START CORING AT 2.00m							
		0-10% LOSS	72	Is(50) d=0.52 a=1.03 MPa	2.20		SANDSTONE: pale grey to grey, medium grained, indistinctly bedded at 0° - 5°	EW					2.00-2.10: EWS	
		0% NO CORE			2.50		SANDSTONE: red to dark brown, medium grained, bedded at 0° - 5°, with ironstains	MW					2.16: BP 0° CN PR S	
		0% NO CORE	86	UCS =13 MPa	3.0 83.7								2.20: BP 0° CN PR S	
		0-5% LOSS		Is(50) d=0.96 a=1.23 MPa	4.0 82.7		SANDSTONE: pale brown to grey, medium to coarse grained, bedded at 0° - 5°, 5% of carbonaceous lamination at 0° - 5°, dark grey						2.34: BP 0 - 5° CN PR S	
		0% NO CORE			4.20								2.45: BP 0 - 5° CN IR S	
		0% LOSS		UCS =7 MPa	5.0 81.7								2.74: BP 20° CN PR S	
		0% NO CORE	84	Is(50) d=0.25 a=0.34 MPa	6.0 80.7		SANDSTONE: pale grey to grey, medium to coarse grained, bedded at 0° - 10°, 5% to 10% of carbonaceous lamination at 0° - 10°, dark grey						2.88: BP 5° CN IR S	
		0% LOSS		Is(50) d=0.34 a=0.55 MPa	6.25								2.97: BP 0° CN PR S	
		0% NO CORE		Is(50) d=0.29 a=0.36 MPa	7.0 79.7								3.35: JT 20° CN PR S	
					7.02								3.43: BP 0 - 5° CN PR S	
					7.03								3.58: BP 10° CN PR S	
					7.03								3.81: BP 5° CN IR S	
					7.03								3.99: BP 10° CN PR S	
					7.03								4.06: BP 5° CN PR S	
					7.03								4.20: BP 0° CN PR S	
					7.03								4.28: BP 5° CN PR S	
					7.03								4.33: BP 0° CN PR S	
					7.03								4.47: BP 0° CN PR S	
					7.03								4.54: BP 5° CN PR S	
					7.03								4.70: JT 30° CN PR S	
					7.03								5.32: BP 5° CN PR S	
					7.03								5.60: BP 10° CN PR S	
					7.03								5.70: BP 0 - 5° CN IR S	
					7.03								5.72: BP 0 - 5° CN IR S	
					7.03								5.90: BP 0 - 5° CN PR S	
					7.03								5.97: BP 5° CN PR S	
					7.03								6.08: BP 0° CN PR S	
					7.03								6.23: BP 5° CN PR S	
					7.03								6.25: BP 0° CN PR S	
					7.03								6.64: BP 5° CN PR S	
					7.03								7.02: BP 0° CN PR S	
					7.03								7.03: BP 0° CN PR S	
					7.03								7.69: BP 0° CN PR S	
					7.03								7.76: JT 20° CN PR S	
					7.03								7.85: BP 0 - 5° CN PR S	

RMS LIB 40.3 EXTERNAL REV1.3.GLB Log RTA CORED DRILL HOLE 5 GINT FILE_19-23 ROSALIND STREET, CAMMERAY.GPJ <-DrawingFiles> 16/Oct/2025 12:42 10.03.00.09

See Explanatory Notes for details of abbreviations & basis of descriptions.

ELITE GEOSCIENCES PTY LTD



CORED DRILL HOLE LOG

HOLE NO : BH3
FILE / JOB NO : 2025181
SHEET : 3 OF 3

PROJECT : Proposed Senior Living Development
LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334181.20, N: 6255731.27 (56 MGA2020) SURFACE ELEVATION : 86.67 (AHD) ANGLE FROM HORIZONTAL : 90°
RIG TYPE : HANJIN DB8 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : AW
DATE STARTED : 24/9/25 DATE COMPLETED : 24/9/25 DATE LOGGED : 24/9/25 LOGGED BY : PL CHECKED BY : TH
CASING DIAMETER : HQ BARREL (Length) : 3.00 m BIT : Step BIT CONDITION : Good

DRILLING				MATERIAL				BIT CONDITION : Good			
PROGRESS		NO CORE DRILL DEPTH	ROD (%)	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	DESCRIPTION ROCK TYPE : Colour, Grain size, Structure (texture, fabric, mineral composition, hardness alteration, cementation, etc as applicable)	Weathering	ESTIMATED STRENGTH Is(50)	NATURAL FRACTURE (mm)	ADDITIONAL DATA (joints, partings, seams, zones, etc) Description, orientation, infilling or coating, shape, roughness, thickness, other
DRILLING & CASING	WATER										
NMLC	0% LOSS	0% NO CORE	84	Is(50) d=0.2 a=0.33 MPa	8.0 78.7	9.30m	SANDSTONE: pale grey to grey, medium to coarse grained, bedded at 0° - 10°, 5% to 10% of carbonaceous lamination at 0° - 10°, dark grey <i>(continued)</i>	MW	● Axial ○ Diametral	20 40 100 300 1000	8.04: BP 5° CN PR S
	0% LOSS	0% NO CORE	89		8.53						9.0 77.7
	0% LOSS	0% NO CORE	10.30	Is(50) d=1 a=1.04 MPa	10.0 76.7	10.30m	SANDSTONE: grey to brown, medium to coarse grained, bedded at 0° - 5°, with ironstains, 5% of carbonaceous lamination at 0° - 5°, dark grey	SW			9.58: BP 0° CN PR S 9.89: BP 0 - 5° CN PR S 10.05: BP 0° CN PR S
					11.0 75.7		BOREHOLE BH3 TERMINATED AT 10.30 m Target depth Well installed upon completion of borehole				
					12.0 74.7						
					13.0 73.7						
					14.0 72.7						
					15.0 71.7						
					16.0 70.7						

RMS LIB 40.3 EXTERNAL REV1.3.GLB Log RTA CORED DRILL HOLE 5 GINTI FILE_19-23 ROSALIND STREET, CAMMERAY.GPJ <-DrawingFiles> 16/Oct/2025 12:42 10.03.00.09

See Explanatory Notes for details of abbreviations & basis of descriptions.

ELITE GEOSCIENCES PTY LTD



ELITE GEOSCIENCES

Project: 19-23 ROSALIND ST, CAMMERAY
Project Number: 2025181

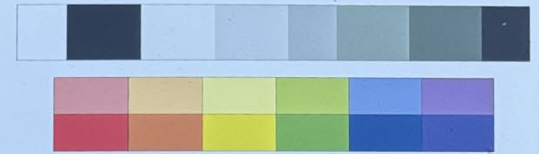
BH3 STARTS CORING AT 2.0m

BH ID: BH3

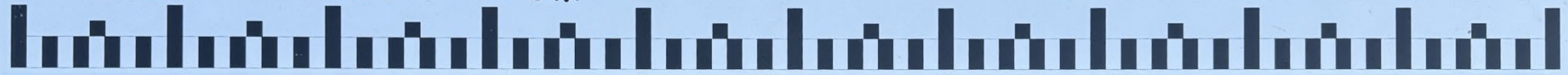
Depth: 2.00m to 7.00m

Core Tray No.: 1

Date: 24/9/2025



X Chalk marks denote handling or drilling breaks



TITLE:

Borehole Core Photographs – 19-23 Rosalind Street, Cammeray

ELITE GEOSCIENCES

PROJECT NO:
2025181

TEST DATE:
24/09/2025

INCLINATION:
-90 degree

CORE LENGTH:
2.00m to 7.00m

DRILL RIG:
RIG 32

CONTRACTOR:
BG Drilling

LOGGED BY:
PL

CHECKED BY:
TH

ELITE GEOSCIENCES

BH ID: BH3

Depth: 7.00 m to 10.30 m

Core Tray No.: 2

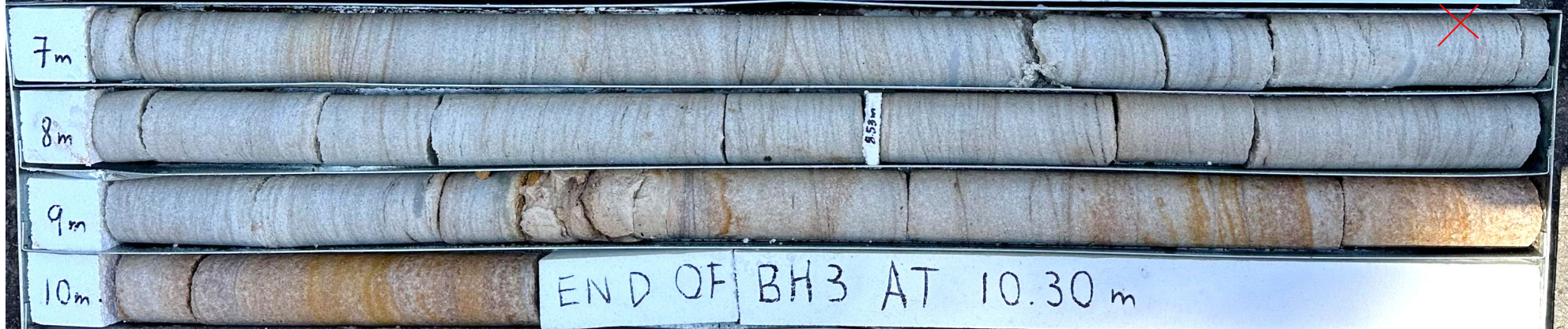
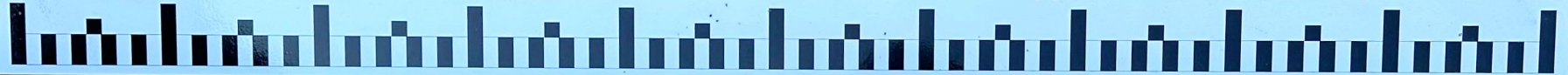
Date: 24/9/2025

Project: 19-23 ROSALIND ST, CAMMERAY

Project Number: 2025181

BH3 STARTS COREING AT 7.0m

✗ Chalk marks denote handling or drilling breaks



TITLE:

Borehole Core Photographs - 19-23 Rosalind Street, Cammeray

PROJECT NO:
2025181

TEST DATE:
24/09/2025

INCLINATION:
-90 degree

CORE LENGTH:
7.00m to 10.30m

DRILL RIG:
RIG 32

CONTRACTOR:
BG Drilling

LOGGED BY:
PL

CHECKED BY:
TH

ELITE GEOSCIENCES

NON-CORE DRILL HOLE - GEOLOGICAL LOG

HOLE NO : BH4
 FILE / JOB NO : 2025181
 SHEET : 1 OF 3

PROJECT : Proposed Senior Living Development
 LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334171.06, N: 6255823.95 (56 MGA2020) SURFACE ELEVATION : 84.45 (AHD) ANGLE FROM HORIZONTAL : 90°
 RIG TYPE : CE180 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : CK
 DATE STARTED : 23/9/25 DATE COMPLETED : 23/9/25 DATE LOGGED : 23/9/25 LOGGED BY : PL CHECKED BY : TH

DRILLING					MATERIAL								
PROGRESS		GROUND WATER LEVELS	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION	CONSISTENCY RELATIVE DENSITY	STRUCTURE & Other Observations			
DRILLING & CASING	WATER												
AD/T HQ Casing	E	Not Encountered	1.00m SPT 3, 4, 4 N=8	0.0 84.5	0.10m	FILL	FILL: SANDY CLAY: brown to dark brown, medium to high plasticity, fine to medium grained sand, with sub-angular, fine to medium grained gravel, with rootlets, covered by grass FILL: SANDY CLAY: brown to dark brown, grey, medium to high plasticity, fine to medium grained sand, trace sub-angular, fine to medium grained gravel	=PL	=PL	TOPSOIL FILL			
	F			1.00m	1.00m	1.80m	SILTY CLAY			SILTY CLAY: pale grey to grey, medium to high plasticity, trace fine to medium grained sand	St	RESIDUAL SOIL	
	H			2.50m	2.40m	2.40m	CLAYEY SAND			CLAYEY SAND: red to brown, fine to medium grained sand, medium to high plasticity clay, trace of ironstones, sub-angular, fine to medium grained	M	MD	2.50: SPT Recovery: 450 m
	H			2.95m	3.00m	3.30m	SANDSTONE			SANDSTONE: recovered as clayey sand, red to brown, fine to medium grained, medium to high plasticity, with sandstone gravel, angular to sub-angular, fine to medium grained, inferred extremely weathered to highly weathered, very low strength Continued as Cored Drill Hole	M	MD	WEATHERED ROCK
	H			3.30m	3.30m	3.35m	SANDSTONE			SANDSTONE: recovered as clayey sand, red to brown, fine to medium grained, medium to high plasticity, with sandstone gravel, angular to sub-angular, fine to medium grained, inferred extremely weathered to highly weathered, very low strength Continued as Cored Drill Hole	M	MD	WEATHERED ROCK
				4.0 80.5									
				5.0 79.5									
				6.0 78.5									
				7.0 77.5									
				8.0 76.5									

RMS LIB 40.3 EXTERNAL REV1.3.GLB Log RTA NON-CORE DRILL HOLE 2 GINT FILE _ 19-23 ROSALIND STREET, CAMMERAY.GPJ <-DrawingFile> 16/Oct/2025 12:48 10.03.00.09

See Explanatory Notes for details of abbreviations & basis of descriptions.

ELITE GEOSCIENCES PTY LTD



CORED DRILL HOLE LOG

HOLE NO : BH4
FILE / JOB NO : 2025181
SHEET : 2 OF 3

PROJECT : Proposed Senior Living Development
LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334171.06, N: 6255823.95 (56 MGA2020) SURFACE ELEVATION : 84.45 (AHD) ANGLE FROM HORIZONTAL : 90°
RIG TYPE : CE180 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : CK
DATE STARTED : 23/9/25 DATE COMPLETED : 23/9/25 DATE LOGGED : 23/9/25 LOGGED BY : PL CHECKED BY : TH
CASING DIAMETER : HQ BARREL (Length) : 3.00 m BIT : Step BIT CONDITION : Good

DRILLING				MATERIAL				FRACTURES				
PROGRESS		DEPTH (m) RL (m AHD)	GRAPHIC LOG	DESCRIPTION ROCK TYPE : Colour, Grain size, Structure (texture, fabric, mineral composition, hardness alteration, cementation, etc as applicable)	Weathering	ESTIMATED STRENGTH Is(50)				NATURAL FRACTURE (mm)	ADDITIONAL DATA (joints, partings, seams, zones, etc) Description, orientation, infilling or coating, shape, roughness, thickness, other	
DRILLING & CASING	WATER					(% NO CORE) DRILL PER RUN %	RQD (%)	SAMPLES & FIELD TESTS	VL			L
		0.0 84.5										
		1.0 83.5										
		2.0 82.5										
		3.0 81.5										
		3.35m		3.35m START CORING AT 3.35m								
		4.0 80.5		SANDSTONE: red to brown, medium grained, bedded at 0° - 10°, with ironstains	EW							3.35-3.69: EWS 3.76: BP 0° CN PR S 3.78: BP 0 - 5° CN PR S 4.07: BP 0 - 5° CN PR S 4.43: BP 5° CN IR S
		5.0 79.5		SANDSTONE: dark brown, medium grained, bedded at 0° - 5°, with ironstains, trace of carbonaceous lamination	MW							5.04: BP 5° CN PR S 5.18: BP 0° CN IR S 5.69: BP 0° CN PR S 5.74: BP 0° CN PR S 5.85: BP 0 - 5° CN PR S
		6.0 78.5										
		6.41m		6.41m								
		7.0 77.5		SANDSTONE: dark brown, medium grained, bedded at 0° - 5°, with ironstains, trace of carbonaceous lamination	SW							7.10: BP 0° CN PR S 7.40: BP 5 - 10° CN PR S
		8.0 76.5										7.96: BP 10° CN IR S

See Explanatory Notes for details of abbreviations & basis of descriptions.

ELITE GEOSCIENCES PTY LTD



RMS.LIB.40.3.EXTERNAL.REV1.3.GLB.Log.RTA.CORED.DRILL.HOLE.5.GINT.FILE_19-23.ROSALIND.STREET.CAMMERAY.GPJ <-DrawingFiles> 16/Oct/2025 12:42 10.03.00.09

CORED DRILL HOLE LOG

HOLE NO : BH4
FILE / JOB NO : 2025181
SHEET : 3 OF 3

PROJECT : Proposed Senior Living Development
LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334171.06, N: 6255823.95 (56 MGA2020) SURFACE ELEVATION : 84.45 (AHD) ANGLE FROM HORIZONTAL : 90°
RIG TYPE : CE180 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : CK
DATE STARTED : 23/9/25 DATE COMPLETED : 23/9/25 DATE LOGGED : 23/9/25 LOGGED BY : PL CHECKED BY : TH
CASING DIAMETER : HQ BARREL (Length) : 3.00 m BIT : Step BIT CONDITION : Good

DRILLING				MATERIAL				FRACTURES			
PROGRESS		NO CORE DRILL DEPTH	ROD (%)	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	DESCRIPTION ROCK TYPE : Colour, Grain size, Structure (texture, fabric, mineral composition, hardness alteration, cementation, etc as applicable)	Weathering	ESTIMATED STRENGTH Is(50) ● Axial ○ Diametral	NATURAL FRACTURE (mm)	ADDITIONAL DATA (joints, partings, seams, zones, etc) Description, orientation, infilling or coating, shape, roughness, thickness, other
DRILLING & CASING	WATER										
0% NO CORE	0% LOSS	99		Is(50) d=0.43 a=0.47 MPa	8.0 76.5	●	SW	●	20 40 100 300 1000	7.98: BP 5° CNTR S 8.14: BP 5 - 10° CN PR S	
0% NO CORE	0% LOSS	91		Is(50) d=0.49 a=0.47 MPa	9.0 75.5	●	SW	●	20 40 100 300 1000	9.29: BP 10 - 15° CN PR S	
0% NO CORE	0% LOSS	11.00		Is(50) d=0.63 a=0.6 MPa	10.0 74.5	●	SW	●	20 40 100 300 1000	9.85: BP 0 - 5° CN PR S 9.87: BP 0 - 5° CN PR S 9.89: BP 0 - 5° CN PR S 9.91: BP 0 - 5° CN PR S 9.93: BP 0 - 5° CN PR S 9.97: BP 5° CN PR S 9.99: BP 0° CN IR S	
					11.0 73.5	●	SW	●	20 40 100 300 1000	10.58: BP 0 - 5° CN IR S	
					12.0 72.5						
					13.0 71.5						
					14.0 70.5						
					15.0 69.5						
					16.0 68.5						

See Explanatory Notes for details of abbreviations & basis of descriptions.

ELITE GEOSCIENCES PTY LTD

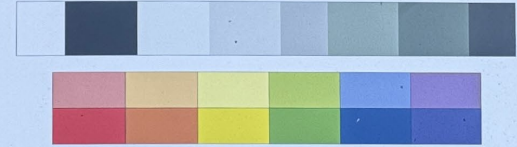


RMS LIB 40.3 EXTERNAL REV1.3.GLB Log RTA CORED DRILL HOLE 5 GINTI FILE_19-23 ROSALIND STREET, CAMMERAY.GPJ <-DrawingFiles> 16/Oct/2025 12:42 10.03.00.09

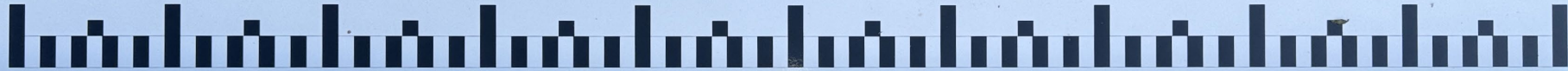
ELITE GEOSCIENCES

Project: 19-23 ROSALIND ST CAMMERAY
Project Number: 2025181

BH ID: BH4
Depth: 3.35m To 8.00m
Core Tray No.: 1
Date: 23/09/25



✗ Chalk marks denote handling or drilling breaks



TITLE:

Borehole Core Photographs - 19-23 Rosalind Street, Cammeray

ELITE GEOSCIENCES

PROJECT NO:
2025181

TEST DATE:
23/09/2025

INCLINATION:
-90 degree

CORE LENGTH:
3.35m to 8.00m

DRILL RIG:
CE 180

CONTRACTOR:
BG Drilling

LOGGED BY:
PL

CHECKED BY:
TH

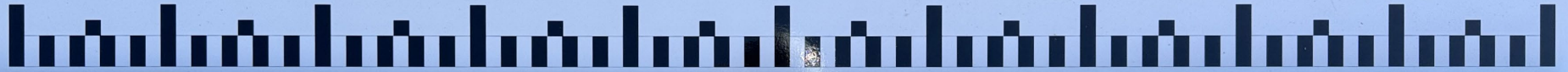
ELITE GEOSCIENCES

Project: 19-23 ROSALIND ST CAMMERAY
Project Number: 2025181

BH ID: BH4
Depth: 8.0m to 11.0m
Core Tray No.: 2
Date: 23/09/25



✗ Chalk marks denote handling or drilling breaks



ELITE GEOSCIENCES	TITLE: Borehole Core Photographs – 19-23 Rosalind Street, Cammeray			
	PROJECT NO: 2025181	TEST DATE: 23/09/2025	INCLINATION: -90 degree	CORE LENGTH: 8.00m to 11.00m
	DRILL RIG: RIG 32	CONTRACTOR: BG Drilling	LOGGED BY: PL	CHECKED BY: TH

NON-CORE DRILL HOLE - GEOLOGICAL LOG

HOLE NO : BH5
 FILE / JOB NO : 2025181
 SHEET : 1 OF 3

PROJECT : Proposed Senior Living Development
 LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334157.98, N: 6255760.41 (56 MGA2020) SURFACE ELEVATION : 85.42 (AHD) ANGLE FROM HORIZONTAL : 90°
 RIG TYPE : CE180 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : CK
 DATE STARTED : 23/9/25 DATE COMPLETED : 23/9/25 DATE LOGGED : 23/9/25 LOGGED BY : AS CHECKED BY : TH

DRILLING					MATERIAL							
PROGRESS		DRILLING PENETRATION	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION	CONSISTENCY RELATIVE DENSITY	STRUCTURE & Other Observations	
DRILLING & CASING	WATER											
ADT ↑ HQ Casing ↓		E	Not Encountered	0.20m	0.0	[Hatched Box]	0.04m 0.10m	BRICK PAVER	M		TOP SURFACE	
		F		ES	0.50m	0.50m	[Diagonal Lines Box]	CI-CH	FILL: SILTY SAND: brown to dark brown, fine to medium grained sand SANDY CLAY: pale grey to grey, medium to high plasticity, fine to medium grained sand, with silt	=PL	St	FILL RESIDUAL SOIL
		H			0.60m	0.60m	[Dotted Box]		SANDSTONE: recovered as sandy clay, yellow to grey, medium to high plasticity, fine to medium grained, with sandstone gravel, angular to sub-angular, fine to medium grained, inferred extremely weathered to highly weathered, very low strength			WEATHERED ROCK
					1.0			Continued as Cored Drill Hole				
					2.0							
					3.0							
					4.0							
					5.0							
					6.0							
					7.0							
					8.0							

RMS LIB 40.3 EXTERNAL REV1.3.GLB Log RTA NON-CORE DRILL HOLE 2 GINT FILE_19-23 ROSALIND STREET, CAMMERAY.GPJ <DrawingFile>_16/Oct/2025 12:48 10.03.00.09

See Explanatory Notes for details of abbreviations & basis of descriptions.

ELITE GEOSCIENCES PTY LTD



CORED DRILL HOLE LOG

HOLE NO : BH5
FILE / JOB NO : 2025181
SHEET : 2 OF 3

PROJECT : Proposed Senior Living Development
LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334157.98, N: 6255760.41 (56 MGA2020) SURFACE ELEVATION : 85.42 (AHD) ANGLE FROM HORIZONTAL : 90°
RIG TYPE : CE180 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : CK
DATE STARTED : 23/9/25 DATE COMPLETED : 23/9/25 DATE LOGGED : 23/9/25 LOGGED BY : AS CHECKED BY : TH
CASING DIAMETER : HQ BARREL (Length) : 3.00 m BIT : Step BIT CONDITION : Good

DRILLING				MATERIAL				BIT CONDITION						
PROGRESS		NO CORE DRILL PER RUN %	ROD (%)	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	DESCRIPTION ROCK TYPE : Colour, Grain size, Structure (texture, fabric, mineral composition, hardness alteration, cementation, etc as applicable)	Weathering	ESTIMATED STRENGTH Is(50)				NATURAL FRACTURE (mm)	ADDITIONAL DATA (joints, partings, seams, zones, etc) Description, orientation, infilling or coating, shape, roughness, thickness, other
DRILLING & CASING	WATER								VL	L	M	H		
		0% NO CORE	100	Is(50) d=0.59 a=0.74 MPa	0.0 85.4									
		0% NO CORE	97	Is(50) d=0.62 a=0.65 MPa	1.0 84.4	0.60m	START CORING AT 0.60m	MW					0.98: BP 0° CN PR S 1.14: BP 5° CN PR S 1.48: BP 5° CN PR S 1.98: BP 10° CN PR S 2.51: BP 5° CN PR S	
		0% NO CORE	92	Is(50) d=0.38 a=0.3 MPa	2.0 83.4	2.80m	SANDSTONE: orange to brown, medium grained, bedded at 5° - 10°, 5% of carbonaceous lamination at 5° - 10°, dark grey, with ironstains	MW to SW					2.82: BP 5 - 10° CN UN S 2.90: BP 5 - 10° CN IR S 3.52: BP 5° CN PR S 3.70: BP 15° CN PR S 3.82: BP 0° CN PR S 4.07: BP 10° CN PR S	
		0% NO CORE	92	Is(50) d=0.4 a=0.54 MPa	4.0 81.4								4.96: BP 5° CN PR S 4.98: BP 5° CN PR S 5.56: BP 0° CN PR S 5.80: BP 10° CN PR S 5.85: BP 10° CN PR S	
		0% NO CORE		Is(50) d=0.4 a=0.75 MPa	6.0 79.4								6.64: BP 15° CN PR S 6.74: BP 5 - 10° CN CU S 6.83: BP 0° CN PR S	
					7.0 78.4								7.75: BP 5° CN PR S	
					8.0 77.4									

See Explanatory Notes for details of abbreviations & basis of descriptions.



RMS.LIB.40.3.EXTERNAL.REV1.3.GLB.Log.RTA.CORED.DRILL.HOLE.5.GINTI.FILE_19-23.ROSALIND.STREET.1.CAMMERAY.GPJ <<DrawingFiles>> 16/Oct/2025 12:42 10.03.00.09

CORED DRILL HOLE LOG

HOLE NO : BH5
FILE / JOB NO : 2025181
SHEET : 3 OF 3

PROJECT : Proposed Senior Living Development
LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334157.98, N: 6255760.41 (56 MGA2020) SURFACE ELEVATION : 85.42 (AHD) ANGLE FROM HORIZONTAL : 90°
RIG TYPE : CE180 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : CK
DATE STARTED : 23/9/25 DATE COMPLETED : 23/9/25 DATE LOGGED : 23/9/25 LOGGED BY : AS CHECKED BY : TH
CASING DIAMETER : HQ BARREL (Length) : 3.00 m BIT : Step BIT CONDITION : Good

DRILLING				MATERIAL				FRACTURES			
PROGRESS		NO CORE DRILL DEPTH	ROD (%)	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	DESCRIPTION ROCK TYPE : Colour, Grain size, Structure (texture, fabric, mineral composition, hardness alteration, cementation, etc as applicable)	Weathering	ESTIMATED STRENGTH Is(50)	NATURAL FRACTURE (mm)	ADDITIONAL DATA (joints, partings, seams, zones, etc) Description, orientation, infilling or coating, shape, roughness, thickness, other
DRILLING & CASING	WATER										
NMLC	0% LOSS	8.15	92	Is(50) d=0.81 a=0.87 MPa	8.0 77.4	•	SW	●	20 40 100 300 1000	8.40: BP 0° CN PR S 8.51: BP 0 - 5° CN UN S 8.61: BP 5 - 10° CN IR S 9.10: BP 5 - 10° CN IR S 9.64: BP 0 - 5° CN PR S	
	0% LOSS	10.00		Is(50) d=1.92 a=1.47 MPa	10.0 75.4	•					
BOREHOLE BH5 TERMINATED AT 10.00 m Target depth Backfilled with spoil											
					11.0 74.4						
					12.0 73.4						
					13.0 72.4						
					14.0 71.4						
					15.0 70.4						
					16.0 69.4						

RMS LIB 40.3 EXTERNAL REV1.3.GLB Log RTA CORED DRILL HOLE 5 GINTI FILE_19-23 ROSALIND STREET, CAMMERAY.GPJ <-DrawingFiles> 16/Oct/2025 12:42 10.03.00.09

See Explanatory Notes for details of abbreviations & basis of descriptions.

ELITE GEOSCIENCES PTY LTD



ELITE GEOSCIENCES

Project: 19-23 ROSALIND ST CAMMERAY
Project Number: 2025181

BH ID: BH5
Depth: 0.60m to 5.00m
Core Tray No.: 1
Date: 24/09/25



✗ Chalk marks denote handling or drilling breaks



BH5 STARTS CORING AT 0.60m



TITLE:

Borehole Core Photographs - 19-23 Rosalind Street, Cammeray

ELITE GEOSCIENCES

PROJECT NO:
2025181

TEST DATE:
24/09/2025

INCLINATION:
-90 degree

CORE LENGTH:
0.60m to 5.00m

DRILL RIG:
CE 180

CONTRACTOR:
BG Drilling

LOGGED BY:
PL

CHECKED BY:
TH

ELITE GEOSCIENCES

Project: 19-23 ROSALIND ST CAMMERAY
Project Number: 2025181

BH ID: BH5
Depth: 5.00m to 10.00m
Core Tray No.: 2
Date: 24/09/25
END OF BOREHOLE AT 10.00m~~X~~



Chalk marks denote handling or drilling breaks



TITLE:

Borehole Core Photographs - 19-23 Rosalind Street, Cammeray

ELITE GEOSCIENCES

PROJECT NO:
2025181

TEST DATE:
24/09/2025

INCLINATION:
-90 degree

CORE LENGTH:
5.00m to 10.00m

DRILL RIG:
RIG 32

CONTRACTOR:
BG Drilling

LOGGED BY:
PL





CHECKED BY:
TH

NON-CORE DRILL HOLE - GEOLOGICAL LOG

HOLE NO : BH6
 FILE / JOB NO : 2025181
 SHEET : 1 OF 3

PROJECT : Proposed Senior Living Development
 LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334149.05, N: 6255740.39 (56 MGA2020) SURFACE ELEVATION : 86.76 (AHD) ANGLE FROM HORIZONTAL : 90°
 RIG TYPE : CE180 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : CK
 DATE STARTED : 24/9/25 DATE COMPLETED : 24/9/25 DATE LOGGED : 24/9/25 LOGGED BY : AS CHECKED BY : TH

DRILLING					MATERIAL						
PROGRESS		DRILLING PENETRATION	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	CLASSIFICATION SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION	CONSISTENCY RELATIVE DENSITY	STRUCTURE & Other Observations
DRILLING & CASING	WATER										
ADT	HQ Casing	E	Not Encountered		0.0 86.8			FILL: SILTY CLAY: brown to dark brown, medium to high plasticity, with fine to medium grained sand, with subangular, fine to medium grained gravel, with rootlets			FILL
		F		0.90m ES	0.50m		CI-CH	SANDY CLAY: pale grey to brown, medium to high plasticity, fine to medium grained sand, trace subangular, fine to medium grained gravel, with silt	=PL		RESIDUAL SOIL
		H		1.20m	0.90m		CI-CH	SANDY CLAY: pale grey to grey, medium to high plasticity, fine to medium grained sand, with silt		St	
					1.20m		CI-CH	SANDSTONE: recovered as sandy clay, red to brown, medium to high plasticity, fine to medium grained, with sandstone gravel, angular to sub-angular, fine to medium grained, inferred extremely weathered to highly weathered, very low strength			WEATHERED ROCK
Continued as Cored Drill Hole											
					2.0 84.8						
					3.0 83.8						
					4.0 82.8						
					5.0 81.8						
					6.0 80.8						
					7.0 79.8						
					8.0 78.8						

RMS LIB 40.3 EXTERNAL REV1.3.GLB Log RTA NON-CORE DRILL HOLE 2 GINT FILE_19-23 ROSALIND STREET, CAMMERAY.GPJ <DrawingFile>_16/Oct/2025 12:48 10.03.00.09

See Explanatory Notes for details of abbreviations & basis of descriptions.

ELITE GEOSCIENCES PTY LTD



CORED DRILL HOLE LOG

HOLE NO : BH6
FILE / JOB NO : 2025181
SHEET : 2 OF 3

PROJECT : Proposed Senior Living Development
LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334149.05, N: 6255740.39 (56 MGA2020) SURFACE ELEVATION : 86.76 (AHD) ANGLE FROM HORIZONTAL : 90°
RIG TYPE : CE180 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : CK
DATE STARTED : 24/9/25 DATE COMPLETED : 24/9/25 DATE LOGGED : 24/9/25 LOGGED BY : AS CHECKED BY : TH
CASING DIAMETER : HQ BARREL (Length) : 3.00 m BIT : Step BIT CONDITION : Good

DRILLING				MATERIAL				BIT FRACTURES						
PROGRESS		NO CORE DRILL DEPTH	ROD (%)	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	DESCRIPTION ROCK TYPE : Colour, Grain size, Structure (texture, fabric, mineral composition, hardness alteration, cementation, etc as applicable)	Weathering	ESTIMATED STRENGTH Is(50)				NATURAL FRACTURE (mm)	ADDITIONAL DATA (joints, partings, seams, zones, etc) Description, orientation, infilling or coating, shape, roughness, thickness, other
DRILLING & CASING	WATER								VL	L	M	H		
					0.0 86.8									
					1.0 85.8									
					1.30m		1.30m START CORING AT 1.30m							
		0% NO CORE	89	Is(50) d=0.4 a=1.12 MPa	2.0 84.8		SANDSTONE: red to brown, medium to coarse grained, bedded at 0° - 5°, with ironstains, trace of carbonaceous lamination	MW					1.38: BP 5 - 10° CN IR S 1.47: BP 0 - 5° CN PR S	
		0-5% LOSS			2.0		SANDSTONE: pale grey, medium grained, bedded at 0° - 5°, 5% to 10% of carbonaceous lamination at 0° - 5°, grey	EW MW					1.92: BP 0 - 5° CN PR S 2.10-2.12: EWS 2.18: BP 5 - 10° CN UN S	
		3.00			3.0 83.8		SANDSTONE: red to brown, grey, medium to coarse grained, bedded at 0° - 10°, with ironstains, trace of carbonaceous lamination	MW					2.37: BP 5° CN PR S 2.45: BP 0° CN PR S	
		0% NO CORE	96	Is(50) d=0.69 a=0.81 MPa	3.0		SANDSTONE: red to brown, grey, medium to coarse grained, bedded at 0° - 10°, with ironstains, trace of carbonaceous lamination							
		0% LOSS			3.39m		SANDSTONE: pale grey, medium grained, bedded at 5° - 10°, 5% to 10% of carbonaceous lamination at 5° - 10°, grey	MW to SW						
					4.0 82.8									
		0% LOSS			4.42m		SANDSTONE: pale grey, medium grained, bedded at 0° - 10°, 5% of carbonaceous lamination at 0° - 10°, grey						4.79: BP 0 - 5° CN PR S	
					5.0 81.8								5.00-5.02: CZ 5.19: BP 5 - 10° CN PR S	
		0% LOSS			6.0 80.8								5.87: BP 0 - 5° CN PR S 5.96: BP 5 - 10° CN IR S	
		6.00			6.0									
		0% NO CORE	95	Is(50) d=0.89 a=0.98 MPa	7.0 79.8		SANDSTONE: pale grey to grey, medium grained, bedded at 10°, 5% to 10% of carbonaceous lamination at 10°, dark grey	EW MW to SW					6.88: BP 5° CN PR S 7.01: BP 10° CN PR S	
		0% LOSS			7.00m		SANDSTONE: pale grey to grey, medium grained, bedded at 10°, 5% to 10% of carbonaceous lamination at 10°, dark grey						7.26-7.28: EWS 7.36: BP 5 - 10° CN PR S	
					7.72m		SANDSTONE: pale grey to grey, medium grained, bedded at 0° - 20°, 10% to 15% of carbonaceous lamination at 0° - 20°, dark grey	SW					7.87: BP 0 - 5° CN PR S 7.94: BP 5 - 10° CN IR S	
					8.0 78.8									

See Explanatory Notes for details of abbreviations & basis of descriptions.



RMS LIB 40.3 EXTERNAL REV1.3.GLB Log RTA CORED DRILL HOLE 5 GINT FILE_19-23 ROSALIND STREET, CAMMERAY.GPJ <<DrawingFiles>> 16/Oct/2025 12:42 10.03.00.09

CORED DRILL HOLE LOG

HOLE NO : BH6
FILE / JOB NO : 2025181
SHEET : 3 OF 3

PROJECT : Proposed Senior Living Development
LOCATION : 19-23 Rosalind Street, Cammeray

POSITION : E: 334149.05, N: 6255740.39 (56 MGA2020) SURFACE ELEVATION : 86.76 (AHD) ANGLE FROM HORIZONTAL : 90°
RIG TYPE : CE180 MOUNTING : Track CONTRACTOR : BG Drilling DRILLER : CK
DATE STARTED : 24/9/25 DATE COMPLETED : 24/9/25 DATE LOGGED : 24/9/25 LOGGED BY : AS CHECKED BY : TH
CASING DIAMETER : HQ BARREL (Length) : 3.00 m BIT : Step BIT CONDITION : Good

DRILLING				MATERIAL				FRACTURES			
PROGRESS		NO CORE DRILL DEPTH	RQD (%)	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	DESCRIPTION ROCK TYPE : Colour, Grain size, Structure (texture, fabric, mineral composition, hardness alteration, cementation, etc as applicable)	Weathering	ESTIMATED STRENGTH Is(50)	NATURAL FRACTURE (mm)	ADDITIONAL DATA (joints, partings, seams, zones, etc) Description, orientation, infilling or coating, shape, roughness, thickness, other
DRILLING & CASING	WATER										
NMLC	0% LOSS	0% NO CORE	95		8.0 78.8	●	SW	●	20 40 100 300 1000	8.06: BP 0 - 5° CN PR S 8.31: BP 10° CN PR S 8.36: BP 5 - 10° CN IR S 8.95: BP 5° CN PR S 9.18: BP 5 - 10° CN PR S 9.64: BP 5° CN PR S	
	0% LOSS	0% NO CORE	100	Is(50) d=0.57 a=0.78 MPa	9.0 77.8	●		●			
	0% LOSS	0% NO CORE		Is(50) d=0.74 a=1.06 MPa	10.0 76.8	●	10.00m	●		BOREHOLE BH6 TERMINATED AT 10.00 m Target depth Well installed upon completion of borehole	
					11.0 75.8						
					12.0 74.8						
					13.0 73.8						
					14.0 72.8						
					15.0 71.8						
					16.0 70.8						

RMS LIB 40.3 EXTERNAL REV1.3.GLB Log RTA CORED DRILL HOLE 5 GINT FILE_19-23 ROSALIND STREET, CAMMERAY.GPJ <-DrawingFiles> 16/Oct/2025 12:42 10.03.00.09

See Explanatory Notes for details of abbreviations & basis of descriptions.

ELITE GEOSCIENCES PTY LTD



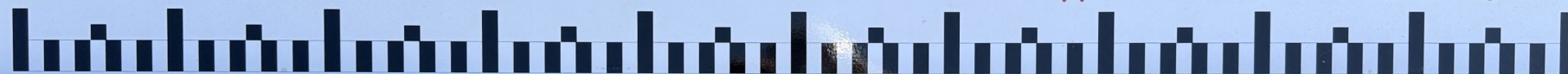
ELITE GEOSCIENCES

Project: 19-23 ROSALIND ST CAMMERAY
Project Number: 2025181

BH ID: BH6
Depth: 1.30m to 6.00m
Core Tray No.: 1
Date: 23/09/25



X Chalk marks denote handling or drilling breaks



ELITE GEOSCIENCES	TITLE: Borehole Core Photographs - 19-23 Rosalind Street, Cammeray			
	PROJECT NO: 2025181	TEST DATE: 23/09/2025	INCLINATION: -90 degree	CORE LENGTH: 1.30m to 6.00m
	DRILL RIG: CE 180	CONTRACTOR: BG Drilling	LOGGED BY: PL	CHECKED BY: TH

ELITE GEOSCIENCES

Project: 19-23 ROSALIND ST CAMMERAY
Project Number: 2025181

BH ID: BH6
Depth: 6.0m to 10.0m
Core Tray No.: 2
Date: 23/09/25



✗ Chalk marks denote handling or drilling breaks

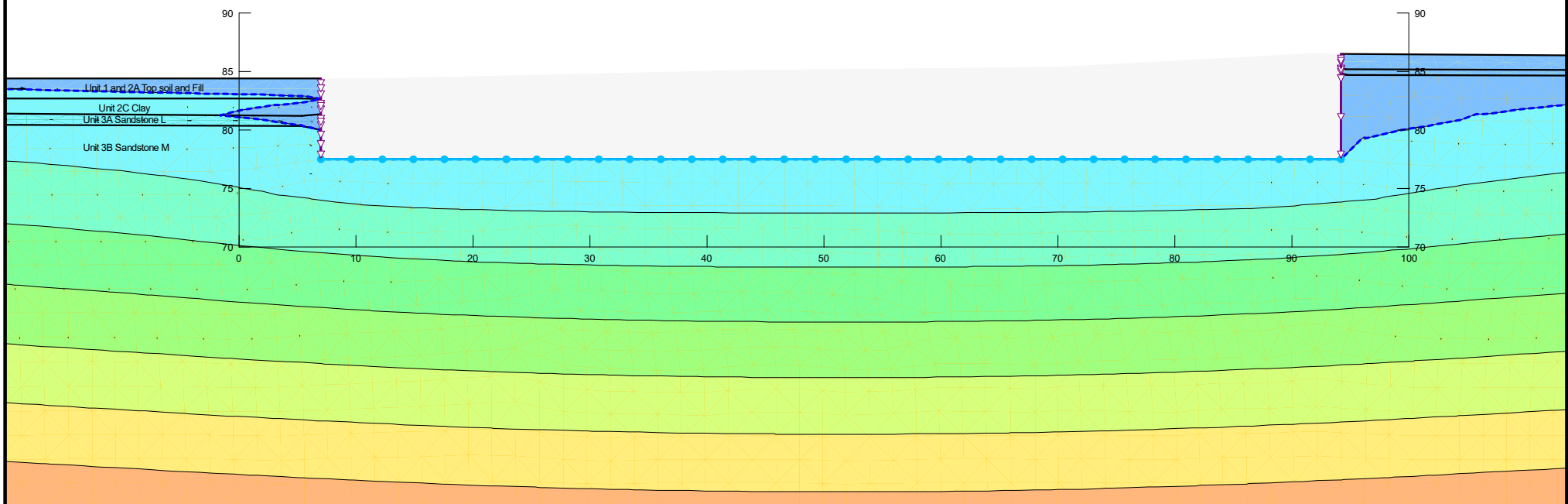
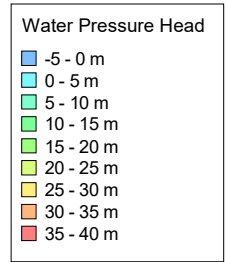


ELITE GEOSCIENCES	TITLE: Borehole Core Photographs – 19-23 Rosalind Street, Cammeray			
	PROJECT NO: 2025181	TEST DATE: 23/09/2025	INCLINATION: -90 degree	CORE LENGTH: 6.00m to 10.00m
	DRILL RIG: RIG 32	CONTRACTOR: BG Drilling	LOGGED BY: PL	CHECKED BY: TH

Technical Memorandum

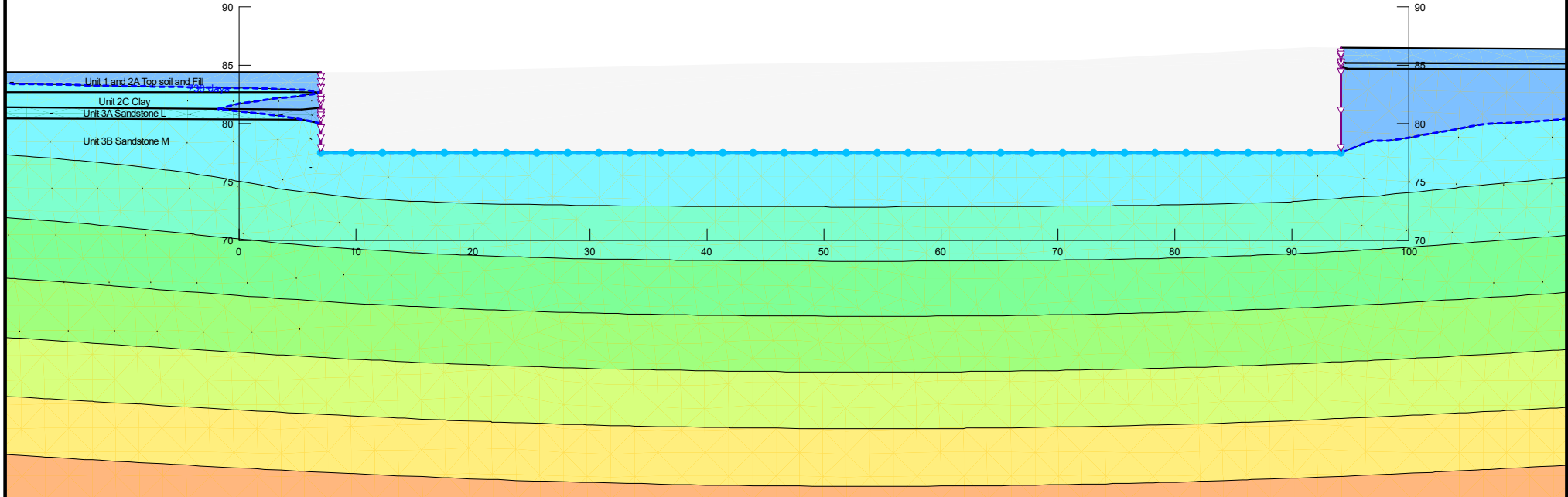
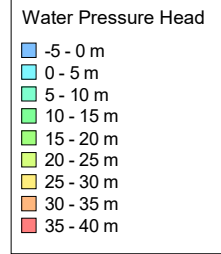
Appendix C – Seep/W Model Outputs

Color	Name	Hydraulic Material Model	Vol. WC. Function	K-Function	Ky/Kx' Ratio	Kz/Kx' Ratio	Dip Direction (°)	Dip (°)
Yellow	Unit 1 and 2A Top Soil and Sand	Saturated / Unsaturated	Sand	Sand	0.5	1	90	0
Light Green	Unit 2C Clay	Saturated / Unsaturated	Clay	Clay	0.3	1	90	0
Dark Green	Unit 3A Sandstone VL	Saturated / Unsaturated	3A	3a	0.3	1	90	0
Orange	Unit 3B Sandstone M	Saturated / Unsaturated	3B	3B	0.3	1	90	0



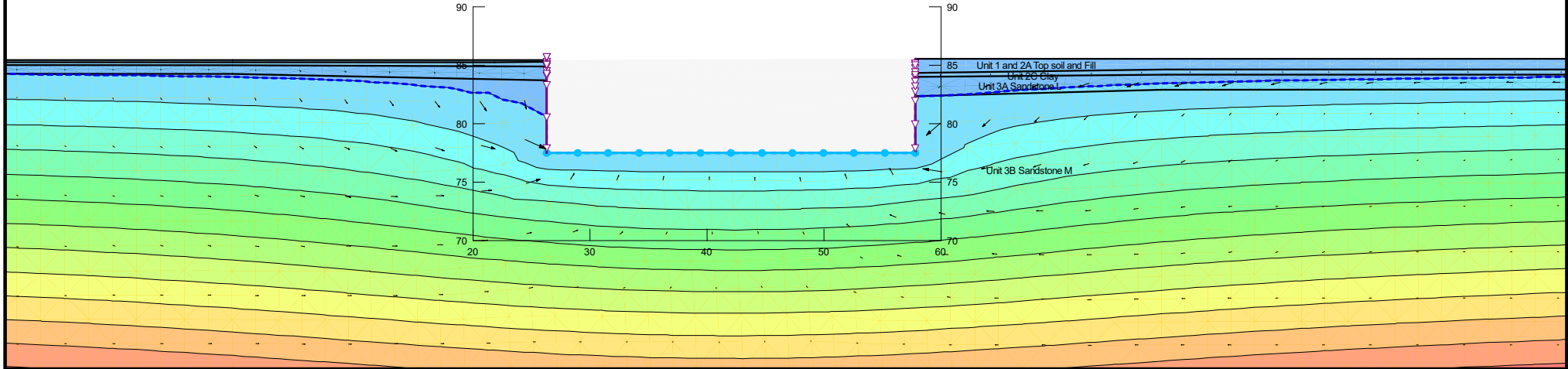
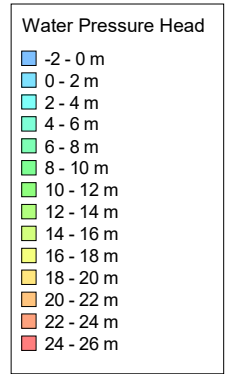
01 Transient (AA)	Time: 180 d
Section A.gsz	
31/10/2025	1:500

Color	Name	Hydraulic Material Model	Vol. WC. Function	K-Function	Ky/Kx' Ratio	Kz/Kx' Ratio	Dip Direction (°)	Dip (°)
Yellow	Unit 1 and 2A Top Soil and Sand	Saturated / Unsaturated	Sand	Sand	0.5	1	90	0
Light Green	Unit 2C Clay	Saturated / Unsaturated	Clay	Clay	0.3	1	90	0
Dark Green	Unit 3A Sandstone VL	Saturated / Unsaturated	3A	3a	0.3	1	90	0
Orange	Unit 3B Sandstone M	Saturated / Unsaturated	3B	3B	0.3	1	90	0



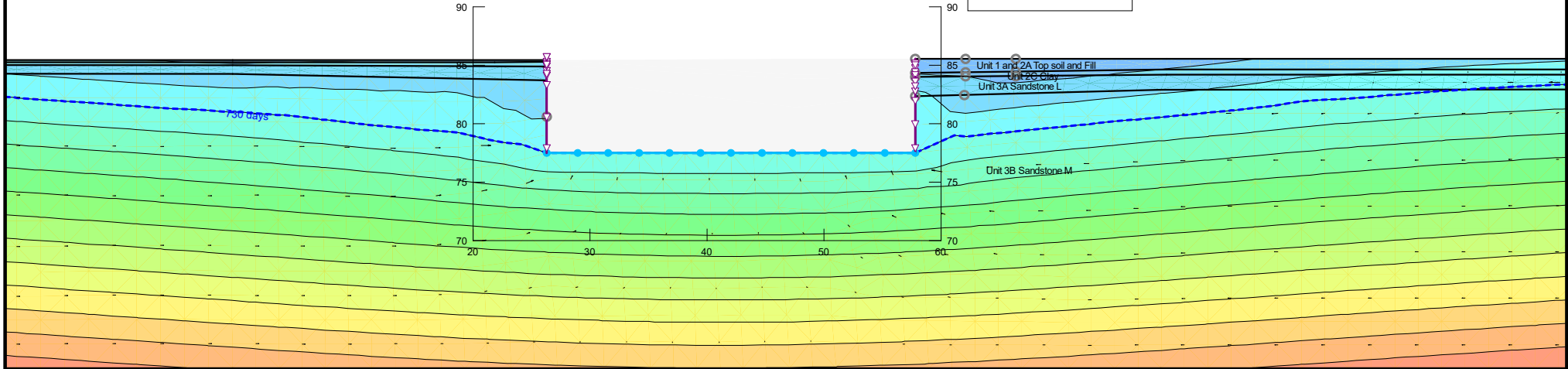
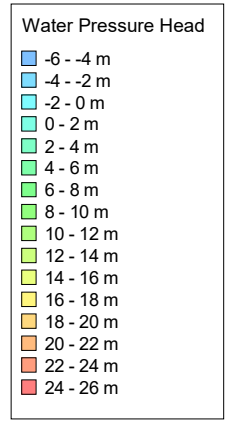
02 Strady State (AA)
Section A.gsz
31/10/2025 1:500

Color	Name	Hydraulic Material Model	Vol. WC. Function	K-Function	Ky/Kx' Ratio	Kz/Kx' Ratio	Dip Direction (°)	Dip (°)
Light Yellow	Unit 1 and 2A Top Soil and Sand	Saturated / Unsaturated	Sand	Sand	0.5	1	90	0
Light Green	Unit 2C Clay	Saturated / Unsaturated	Clay	Clay	0.3	1	90	0
Dark Green	Unit 3A Sandstone VL	Saturated / Unsaturated	3A	3a	0.3	1	90	0
Orange	Unit 3B Sandstone M	Saturated / Unsaturated	3B	3B	0.3	1	90	0



01 Transient (BB)	Time: 180 d
Section A.gsz	
31/10/2025	1:500

Color	Name	Hydraulic Material Model	Vol. WC. Function	K-Function	Ky/Kx' Ratio	Kz/Kx' Ratio	Dip Direction (°)	Dip (°)
Light Yellow	Unit 1 and 2A Top Soil and Sand	Saturated / Unsaturated	Sand	Sand	0.5	1	90	0
Light Green	Unit 2C Clay	Saturated / Unsaturated	Clay	Clay	0.3	1	90	0
Dark Green	Unit 3A Sandstone VL	Saturated / Unsaturated	3A	3a	0.3	1	90	0
Orange	Unit 3B Sandstone M	Saturated / Unsaturated	3B	3B	0.3	1	90	0



02 Steady State (BB)

Section A.gsz

31/10/2025 1:500