



166-174 Blaxland Road, Ryde

Integrated Water Management Plan

Prepared For: Sasco Property

Date: May 2026

Prepared By: Ian Harris

Project Ref: 0610

enscape studio Pty Ltd

Tel: 0411 267 151

E: info@enscapestudio.com.au



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1 Introduction

This Integrated Water Management Plan (IWMP) is submitted to the Department of Planning, Housing and Infrastructure (the Department) on behalf of Sasco Property (the Applicant) in support of a State Significant Development Application (SSDA) for a multistorey residential development at 166-174 Blaxland Road, Ryde (the site).

This IWMP outlines the conceptual DA level stormwater design for the proposed development on the site.

This IWMP illustrates that the proposed development complies with the Ryde City Council's DCP, Australian Rainfall and Runoff, Australian Standards and best engineering practise.

The purpose of this SMR is to evaluate the quantity and quality of stormwater associated with the proposed development plan so as to demonstrate to Council that an appropriate stormwater management strategy has been adopted.

The IWMP specifically addresses the following items for both the construction and operational phases of the development:

- Stormwater runoff volumes and detention (Stormwater Quantity);
- Stormwater quality treatment measures (Stormwater Quality),
- Erosion and Sedimentation Control.

The following will be achieved with the correct application of this SMR:

- Appropriate standards to be maintained on all aspects of stormwater within the site,
- Pollution control to be maintained,
- Establishment of a unified, clear and concise stormwater management strategy.



2 Existing Site Characteristics

2.1 Property Detail

Address: 166-174 Blaxland Road, Ryde

Total Site Area: 3,832m² (0.3832Ha)

The proposed development can be seen on the concept design drawings in Appendix A of this report.

The proposed development is situated within the Ryde City Council LGA and will consist of a multi-storey residential development over a basement car park.

The site is bounded by:

- Blaxland Road to the east
- Residential development on all other sides of the property

Refer to locality plan in figure 1.



Figure 1: Site Location Plan (Source: Nearmaps 2026)

2.2 Topography

The site falls to the rear away from Blaxland Road. Falls are generally to the south west of the site. Survey shows the site falls from RL 57.50m AHD to RL47.30m AHD.

2.3 Existing Stormwater Infrastructure Discharge

Currently the sites all drain to the rear of the properties. There is no formal connection to any inground drainage network or direct connections to the kerb and gutter on Blaxland Road. All stormwater runoff discharges overland onto the downstream properties.



3 The Proposed Development

SSD-95802958 is accompanied by a concurrent rezoning request and seeks consent for the demolition of existing structures and construction of a residential flat building development containing 114 residential apartments and basement car parking.

The development consists of a part 4, part 10 storey building above 4 levels of basement car parking containing a total of 171 car parking spaces including 12 accessible spaces, 11 motorcycle spaces and 21 bicycle spaces. A loading dock is provided on the ground floor level adjacent to a waste storage area.

Vehicle access to the basement is provided off Blaxland Road via a two way access driveway with an additional service access driveway provided for the loading dock.

The proposal provides two (2) separate residential lobby's with two (2) lift cores each servicing a maximum of 7 apartments.

Communal open space for all residential apartments is located at the ground floor and at the rooftop.

The proposed development is illustrated in the Architectural Plans prepared by Curzon + Partners.

4 Local Authority Requirements

The stormwater requirements of the subject site is governed by the Ryde City Council Stormwater and Floodplain Management Technical Manual. A summary of the key requirements for the development of the Stormwater management system for this development are summarised below.

4.1 Stormwater Conveyance Requirements

Council's manual states that the following design storm Average Recurrence Intervals ARI's should be allowed for when designing the Stormwater runoff conveyance systems for the development.

Table 1: Stormwater Drainage Serviceability

Design Parameter	Design Storm ARI (Years)	Conveyance Method
Minor Drainage System	20	In Ground (Piped)
Major Drainage System	100	Overland

4.2 On Site Detention Requirements

Council's manual states that as the site is larger than 3,000m². On site detention must be calculated using the Detailed calculation method.

The manual states "The OSD must be designed to ensure the level of stormwater runoff discharged from the area of development must not to exceed the peak stormwater discharge arising from the post-developed works, during a 5 year ARI storm event."



4.3 Stormwater Quality Treatment

Council's Development Control Plan identifies the requirements for WSUD required for Council Approval. It states that:

Water quality measures are installed that meet the following environmental targets for stormwater runoff leaving the site:

- 90% removal of gross pollutants (> 5mm);
- 85% removal of total suspended solids;
- 65% removal of total phosphorous; and
- 45% removal of total nitrogen.

5 Water Utility Provision

5.1 Water Mains

The development will be serviced via the existing 100mm water main located along the Blaxland Road frontage. A new 100mm stub connection is proposed to service the domestic water demand and supply to the fire storage tanks. The extract in Figure 2 shows the approximate location of these works.

Sewer Mains

The development will be serviced by the existing 225mm sewer main located along the rear of the site. Confirmation on whether the existing sewer through the site will need to be realigned or reinforced will be subject to the Section 73 application for the development. The extract in Figure 2 shows the approximate location of these works.

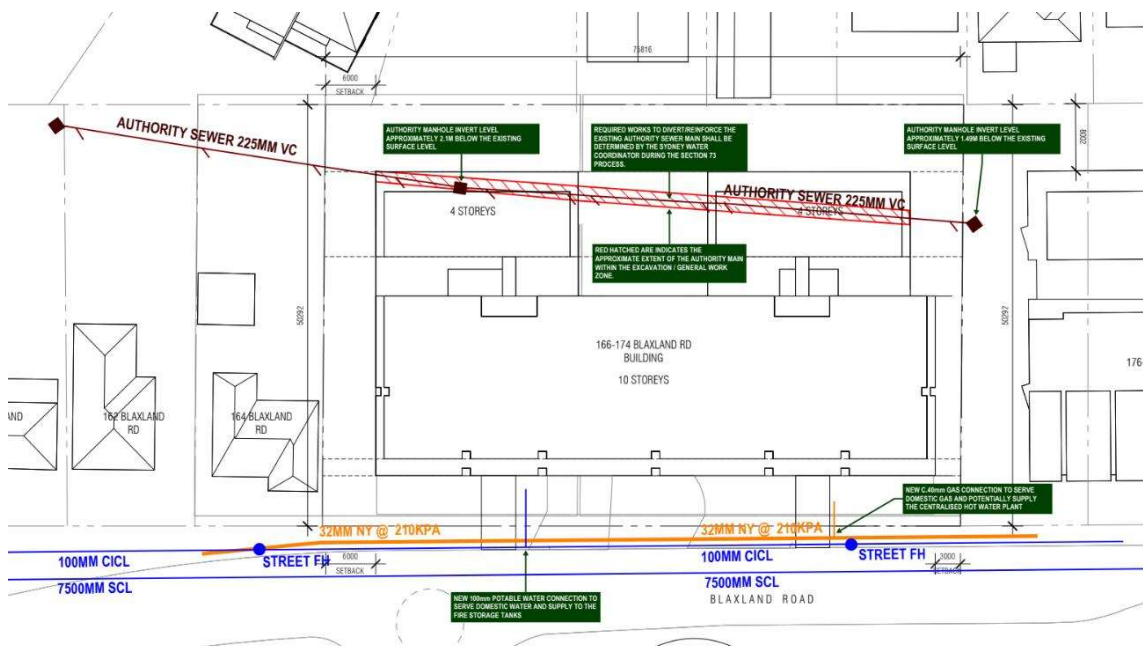


Figure 2: Sewer and Potable Water Provision (Source: Keegan 2026)



6 Stormwater Conveyance

This section of the report discusses the systems proposed to allow for stormwater to be conveyed across the site to the legal point of discharge.

As discussed in Section 3.1 of this report Council have set minimum design parameters for the flows they require to be conveyed through the in ground drainage system and what they will allow to be conveyed in a controlled manner overland across the site.

6.1 Surface Drainage

The surface areas will be drained through a variety of methods, discussed below, in accordance with AS3500.3:2015 and Council's stormwater drainage guidelines.

6.1.1 In-Ground Drainage

The in-ground drainage has been designed to meet the following criteria:

- In the minor design storm event (20 year) there will be no surcharging of the in ground drainage system and;
- In the major design storm event (100 year) there will be no uncontrolled discharge from the site onto the residential properties to the West of the site.

Surface runoff from the development sites will be directed to stormwater inlet structures using the design topography of these elements. The inlet structures have been designed to adequately convey the surface runoff into the in ground drainage network.

The runoff will then be conveyed underground across the site through a pit and pipe system and then to the legal point of discharge using gravity and the geometric falls of the pipe system.

6.2 Legal Point of Discharge

It is proposed that the developed site will drain to the rear of the site and into a new stormwater drainage line located within council's existing stormwater drainage easement on 57 Samuel Street. The new drainage line will then connect into the existing council drainage infrastructure running through 57 Samuel Street. As the proposed development will control the discharge from the site and discharge through formal drainage infrastructure this will alleviate the current issue of uncontrolled discharge from the development site onto the adjacent downstream property owners.

Details of the drainage connection and stormwater discharge location are shown on the civil engineering drawings in Appendix A of this report.



7 Stormwater Attenuation

As noted in Section 4.2 of this report the stormwater discharge from the site is required to be attenuated to ensure the peak discharge rate for all storm events up to and including the 100 year design storm will be limited to the 5 year post-development discharge rate.

Enscape studio have utilised DRAINS hydraulic modelling software to assess the required storage for the development site.

Modelling indicates that a storage volume of 45m³ will be required to attenuate the site discharge flows back to the Permissible Site Discharge rate for the site. The table below summarises the outcome of the DRAINS assessment.

Design ARI (Years)	Unattenuated Discharge Rate (L/s)	Attenuated Discharge Rate (L/s)
5	109	67
100	187	109

Table 2: DRAINS modelling output

The attenuation volume required for the site will be provided using a below ground On Site Detention tank located on the south west corner of the site. The flows rates from the tank will be constrained using an orifice and overflow weir arrangement within the tank.

Details of the tank and discharge control are shown on the civil engineering drawings in Appendix A of this report.



8 Stormwater Quality

This section of the report demonstrates the Stormwater Quality Improvement Devices (SQID's) to be implemented to reduce the flow of pollutants from the site.

8.1 Potential Pollutants

There are a wide range of potential stormwater pollutant sources which occur from urbanised catchments, many which can be managed through appropriate stormwater quality treatment.

Typical urban pollutants may include:

- Atmospheric deposition
- Erosion (including that from subdivision and building activities)
- Litter and debris
- Traffic emissions and vehicle wear
- Animal droppings
- Pesticides and fertilisers
- Application, storage and wash-off of car oil, detergents and other household and commercial solvents and chemicals
- Solids accumulation and growth in stormwater systems
- Weathering of buildings

These pollutants in urban stormwater can be placed into various categories as follows. The pollutants underlined below are able to be readily modelled:

- Suspended Solids
- Litter
- Nutrients such as Nitrogen and Phosphorous
- Biological oxygen demand (BOD) and chemical oxygen demand (COD) materials
- Micro-organisms
- Toxic organics
- Trace metals
- Oils and surfactants

While only the key pollutants underlined above will be examined within the modelling, the stormwater Quality Improvement Devices implemented are expected to assist in reducing a wide range of pollutants. For example, heavy metals are commonly associated with, and bound to fine sediments. Thus reducing the discharge of fine sediment during the construction and operational phases will reduce the discharge of heavy metals to existing stormwater systems.

8.2 Pollutant Reduction System

In order to reduce the pollutants a series of treatment devices are proposed, which together, form a treatment train. The diagram below shows the proposed treatment train for this development.

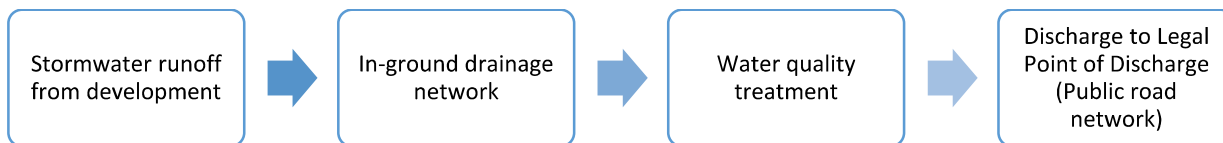


Figure 3: Proposed Water Quality Treatment Train



8.2.1 Stormwater360 Stormfilter Cartridges

The StormFilter stormwater treatment system uses rechargeable, self-cleaning, media-filled cartridges to absorb and retain the most challenging pollutants from stormwater runoff including total suspended solids, hydrocarbons, nutrients, soluble heavy metals, and other common pollutants.

Maintenance requirements and frequency are dependent on the pollutant load characteristics of each site. Maintenance activities may be required in the event of a chemical spill or due to excessive sediment loading from site erosion or extreme storms. Similarly, the system should be inspected after major storm events.

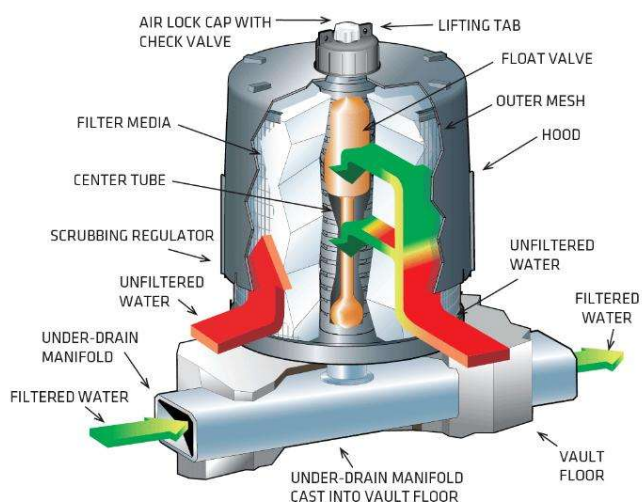


Figure 8: Stormwater360 Stormfilter (Source: Stormwater360)

Parameters	TSS	TP	TN	GP
Input (mg/L)	1000	10	100	1000
Output (mg/L)	675	6.2	63	100
Reduction (%)	33	48	37	90

8.3 Pollutant Reduction Modelling

In order to demonstrate that the proposed treatment train meets the required reduction targets, pollutant reduction modelling is proposed using the Model for Urban Stormwater Improvement Conceptualisation (MUSIC) Software program Version 6.2 by eWater CRC. Pollutant export rates are currently only available for Total Suspended Solids (TSS), Total Nitrogen (TN), Total Phosphorous (TP) and Gross Pollutants (GP). Therefore only quantitative modelling for TSS, TN, TP & GP has been undertaken using MUSIC.

Modelling has only been undertaken on the post-development proposal with SQID's installed so as to demonstrate the percentage reduction for each pollutant type.



8.3.1 MUSIC Program Setup

This section explains the setup of the MUSIC model with the detailed pollutant reduction calculations being included in the MUSIC results in Appendix B.

For Music Modelling (using MUSIC 6.3.0) the following parameters have been used in accordance with Ryde City Council WSUD Reference Guideline:

Table 3: MUSIC modelling parameters

Model Parameters	
Meteorological Data:	066037 Sydney Airport 1990-1999
Evaporation Data:	Sydney 1959
Time Step:	6 minute

Table 4: Catchment modelling parameters

Node Description	Area (Ha)	Percentage Impervious (%) / Area Impervious (Ha)		Land Use Rainfall and Pollutant Parameters
Urban Roof	0.044	100	0.044	Roof
Urban Podium	0.072	70	0.051	Mixed
	Total: 0.116Ha			

8.3.2 MUSIC Parameters

The following properties from Using MUSIC in Sydney's Drinking Water Catchment by NSW Sydney catchment authority have been used in the MUSIC Modelling based on the Land Use Rainfall and Pollutant Parameters.

Table 5: Recommended MUSIC Runoff Generation Parameters

Parameter	Urban Residential
Rainfall Threshold (mm)	1.0
Soil Capacity (mm)	120
Initial Storage (%)	25
Field Capacity	80
Infiltration Capacity Coefficient a	200
Infiltration Capacity Coefficient b	1.0
Initial Depth (mm)	10
Daily Recharge Rate (%)	25
Daily Drainage Rate (%)	5
Daily Deep Seepage Rate (%)	0



8.3.3 Pollutant Reduction Results

A number of management measures have been considered with a focus on reducing polluted runoff volumes from the site. The WSUD principals proposed for stormwater treatment includes:

- 17 x Stormfilter Cartridges.
- 1 x oceanguard sack

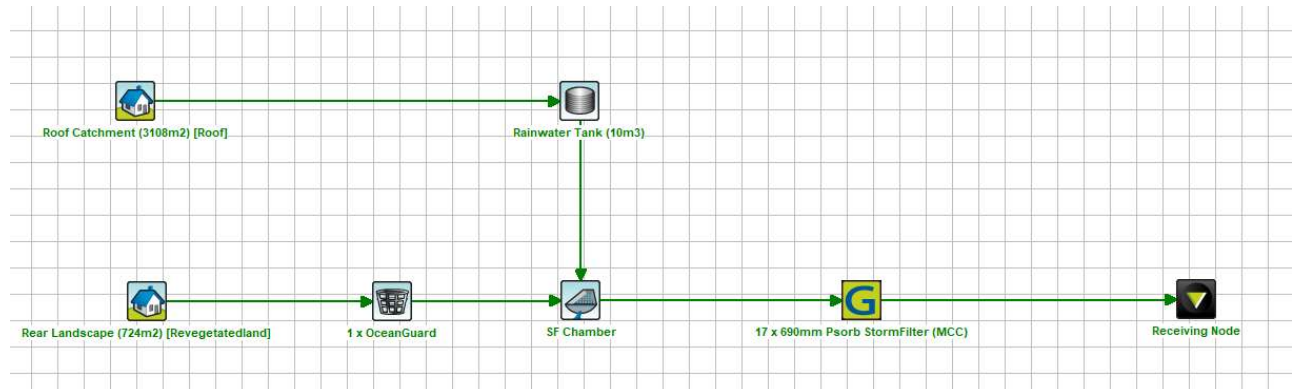


Figure 4 MUSIC Model

MUSIC Output

	Sources	Residual Load	% Reduction
Flow (ML/yr)	4.52	4.25	6.1
Total Suspended Solids (kg/yr)	128	17.9	86.1
Total Phosphorus (kg/yr)	0.669	0.149	77.7
Total Nitrogen (kg/yr)	9.73	4.56	53.1
Gross Pollutants (kg/yr)	101	0	100

Figure 5 MUSIC Results

The effectiveness of the treatment device proposed in the above section has been modelled using MUSIC with the overall treatment train efficiency results shown in Table 7 below.

Table 6: Treatment Train Efficiencies

Indicator	Total Site Reduction	Site Targets	Target Achieved
Gross Pollutants	100	90%	Yes
Total Suspended Solids (TSS)	86.1	85%	Yes
Total Phosphorus (TP)	77.7	65%	Yes
Total Nitrogen (TN)	53.1	45%	Yes

From the results presented above it can be seen that the proposed SQID's mean that the stormwater quality treatment meets with the reduction targets set for the development.



9 Erosion & Sedimentation Control

Landcom have published a design guide entitled “Managing Urban Stormwater - Soils and Construction” which is regarded as the standard to which erosion and sedimentation control should be designed to within NSW. Ryde City Council specifies compliance with the Landcom design guide in there Stormwater and Floodplain Management Technical Manual.

The control of erosion and sedimentation describes the measures incorporated during and following construction of a new development to prevent the pollution and degradation of the downstream watercourse.

Stormwater Drainage Infrastructure Inlets

Risk:

- Sediment from the construction site washing into the existing stormwater drainage inlet infrastructure.

Consequence:

- The sediment will then be conveyed into the downstream waterbody by stormwater runoff, contaminating the waterbody.
- The sediment will build up blocking the stormwater infrastructure and preventing stormwater conveyance to the downstream waterbody and impacting drainage upstream.

Mitigation:

- Sandbag protection will be installed surrounding all existing stormwater drainage infrastructure inlets to prevent sediment entering the system.

Maintenance:

- Frequent inspection of the sandbags to ensure they are arranged in a manner that prevents sediment from accessing the drainage system. If sediment is building up on the sandbags they should be cleared of sediment and re-established.

Construction Exit Protection

Risk:

- Spoil such as soil being conveyed from the site on the wheels of vehicles.

Consequence:

- Spoil being tracked onto the public road corridors where it is then washed into the existing stormwater drainage infrastructure and is then washed downstream polluting the downstream waterbody.
- Spoil being tracked onto the public road creating dangerous driving conditions for other road users.

Mitigation:

- A shaker grid and wash down facility will be installed at all exits from the construction site. All vehicles leaving the site will have their wheels washed down and pass over the shaker grid to remove any spoil collected on their wheels and retaining the spoil on site.

Maintenance:



- Frequent inspection of the shaker grid to ensure it is clean and still functioning.

Downstream Site Boundaries

Risk:

- Rainfall runoff falling on the site collecting sediment from the construction site and conveying it overland onto downstream properties and waterbodies.

Consequence:

- Sediment discharge polluting downstream properties and waterbodies.

Mitigation:

- Installation of sediment fences on all downstream boundaries of the site to collect sediment and prevent it discharging onto downstream properties or waterbodies.

Maintenance:

- Regular inspection of the sediment fences to ensure they are functioning correctly and are intact.
- If sediment build up is present it should be removed to ensure correct functionality of the fences.

A Soil and Water Management Plan has prepared as part of the development application documentation and is included in Appendix A of this report.

The maintenance of these control measures throughout their intended lifespan will ensure that the risk of erosion and sedimentation pollution of the downstream watercourse will be minimized.

Appendix A – Civil Engineering Drawings

CIVIL ENGINEERING WORKS

DEVELOPMENT APPLICATION



SITE LOCATION PLAN
SCALE 1:500

PROJECT SUMMARY

SITE ADDRESS: 166-174 BLAXLAND ROAD, RYDE
LOCAL GOVERNMENT AREA: CITY OF RYDE COUNCIL
DEVELOPMENT DESCRIPTION: MULTI STOREY RESIDENTIAL DEVELOPMENT WITH TWO LEVELS OF BASEMENT.

SASCO PROPERTY

CLIENT

166-174 BLAXLAND ROAD, RYDE

PROJECT

enscape
studio

M 0011 347 151
info@enscapestudio.com.au
Aussie by day and night

0610

PROJECT No

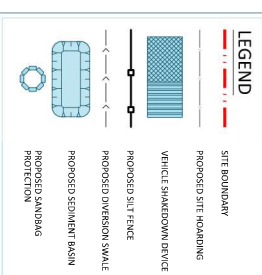
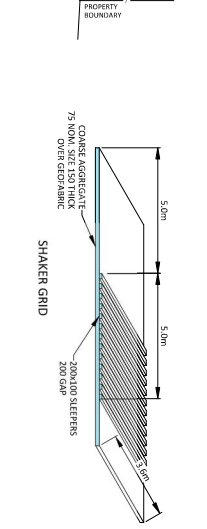
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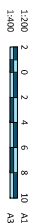
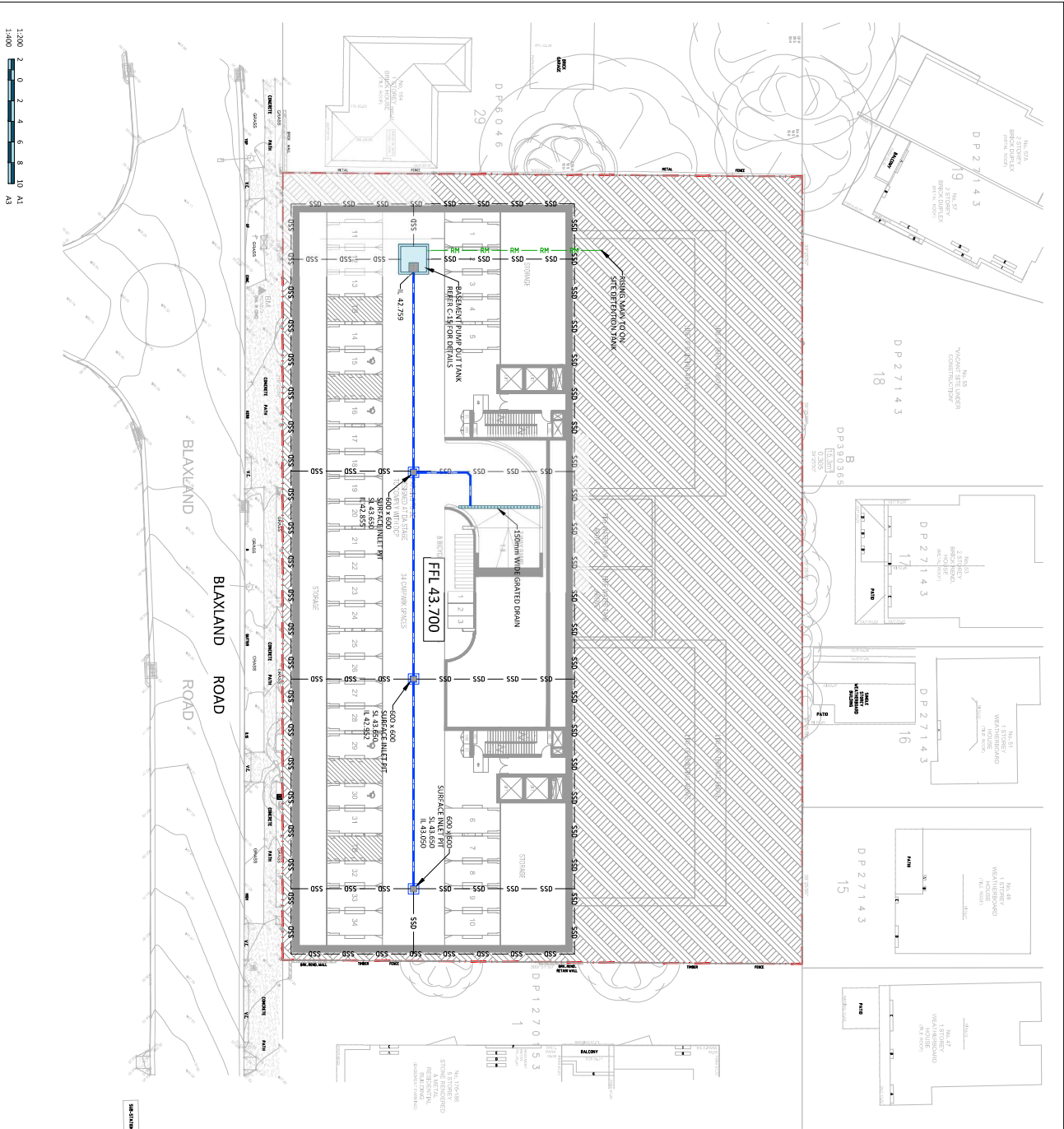
1. PRIOR TO COMMENCEMENT OF INSTALLATION, THE FOLLOWING SLOPE MANAGEMENT PROCEDURES MUST BE OBSERVED:
 - a. THE SLOPE MUST BE PROTECTED FROM THE SLOPE AND ACROSS ALL POTENTIAL RUNOFF SITES.
 - b. CO-ORDINATE CONSTRUCTION ENTRY POINTS WITH PROJECT TRAILHEADS AND SLOPE MANAGEMENT PLAN.
 - c. CONTROL SYSTEMS TO PREVENT UPLAND RUNOFF FROM EXISTING STORMWATER SYSTEM.
 - d. PROVIDE SAMPLER/SEDIMENT TRAPS UPSTREAM OF EXISTING TRAILS AND TRAILHEADS TO COLLECT RUNOFF FROM THE WINDWARD SLOPE SITES AT THE TOP CENTER AND BOTTOM AND AT 1/4 MILE INTERVALS.
 - e. POSTERIOR SLOPE AREA TO BE REGULARLY WATERED TO REDUCE DUST.
2. CONSTRUCT ELEVATED ALLEYS IN PROXIMITY TO EXISTING TRAILS.
3. CONSTRUCT ELEVATED ALLEYS IN PROXIMITY AROUND ALL DISPOSED PILES OF THE FILL CONSTRUCTION.
4. PROVIDE AND MAINTAIN A STEEP SLOPE ON BOTH SIDES OF ALL ROADWAYS AROUND THE FILL CONSTRUCTION.
5. PROVIDE AND MAINTAIN A STEEP SLOPE ON BOTH SIDES OF ALL ROADWAYS AROUND THE FILL CONSTRUCTION.


enscape
 studio

Tel 0411 367 151
 Email info@enscapedesign.com.au
 Web www.enscape.com.au


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LEGEND

PROPOSED SITE BOUNDARY

PROPOSED STORMWATER PRE-DRAINAGE

PROPOSED GRADED PIT

ON SITE RETENTION PIT AND DISCHARGE PUMP OUT

RAINWATER TANK

PROPOSED PIPE SIZE AND FLOW DIRECTION

PROPOSED DOWNSPIRE

OVERLAND FLOW

STORMWATER MANAGEMENT NOTES

SITE INFORMATION

ADDRESS: 166-174 BLAXLAND ROAD, RYDE
LOCAL GOVERNMENT AREA: RYDE LOCAL COUNCIL
SITE AREA: 3.82ha²

DEVELOPMENT CONTROL REQUIREMENTS

GOVERNING DOCUMENT: CITY OF RYDE COUNCIL - STORMWATER AND HOOD-UP-AN IMPROVEMENT TECHNICAL MANUAL.

DESIGN STORMS:
MINOR - 20 YEAR
MAJOR - 100 YEAR

DISCHARGE ATTENUATION REQUIREMENT:

1. THE OSD MUST BE DESIGNED TO ENSURE THE LEVEL OF STORMWATER RUNOFF DISCHARGED FROM THE AREA OF DEVELOPMENT MUST NOT TO EXCEED THE PEAK STORMWATER RUNOFF RATE (AS DERIVED FROM THE 100 YEAR DEVELOPED WATERS) DURING A 15 MIN 5% TDSM EVENT.

WATER QUALITY TREATMENT REQUIREMENTS:

- 80% REMOVAL OF TOTAL SUSPENDED SOLIDS (TSS);
- 85% REMOVAL OF TOTAL PHOSPHORUS; AND
- 45% REMOVAL OF TOTAL NITROGEN.

SITE ATTENUATION DESIGN

ALLOWABLE PEAK DISCHARGE RATES:

5 YEAR ARI	= 108 L/s
1 YEAR ARI	= 11 L/s
100 YEAR ARI	= 2 L/s

BYPASS DISCHARGE RATES:

5 YEAR ARI	= 108 L/s
1 YEAR ARI	= 108 L/s
100 YEAR ARI	= 108 L/s

UNATTENUATED POST DEVELOPMENT DISCHARGE RATES:

5 YEAR ARI	= 108 L/s
1 YEAR ARI	= 108 L/s
100 YEAR ARI	= 108 L/s

POST DEVELOPMENT DISCHARGE RATES:

5 YEAR ARI	= 57 L/s
100 YEAR ARI	= 107 L/s
100 YEAR ARI	= 108 L/s

TOTAL DEVELOPMENT DISCHARGE RATES:

5 YEAR ARI	= 58 L/s
100 YEAR ARI	= 108 L/s

REFERS TO DRAINAGE CALCULATIONS BY ENSCAPE STUDIO FOR MORE INFORMATION.

OSD DESIGN SUMMARY

OSD VOLUME REQUIRED:	45.0m ³
OSD VOLUME PROVIDED:	45.02m ³

OSD DIMENSIONS:

WIDTH	3.43m
LENGTH	8.10m
DEPTH	1.65m

INVERT OF OUTLET: 46.05m AHD

INVERT OF INLET: 42.759m AHD

REV	DESCRIPTION	DRAWN	APPD	DATE
A	ISSUED FOR APPROVAL	HI	HI	13.03.26

SASCO PROPERTY

CURZON + PARTNERS

ARCHITECT

166-174 BLAXLAND ROAD RYDE

STORMWATER MANAGEMENT PLAN BASEMENT 2

TITLE

enscape studio

enscapestudio.com.au

tel 0411 262 151

166-174 BLAXLAND ROAD RYDE NSW 1513

FOR APPROVAL

NOT FOR CONSTRUCTION

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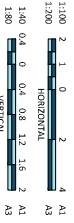
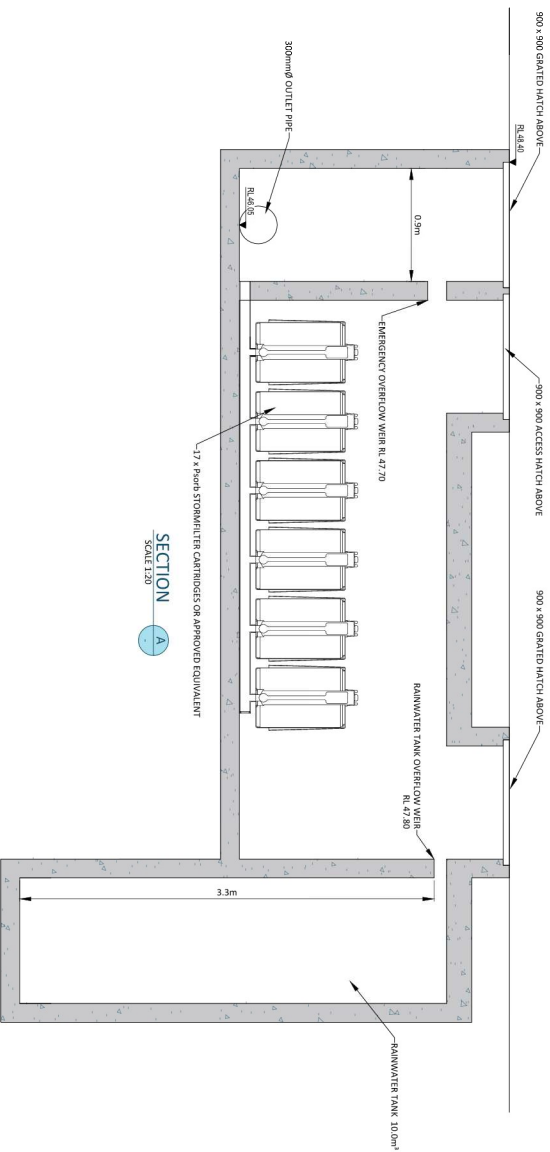
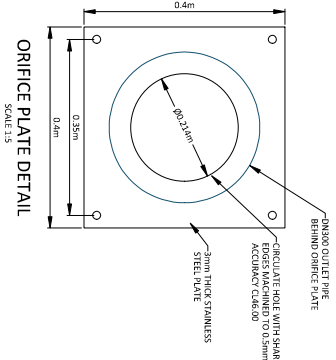
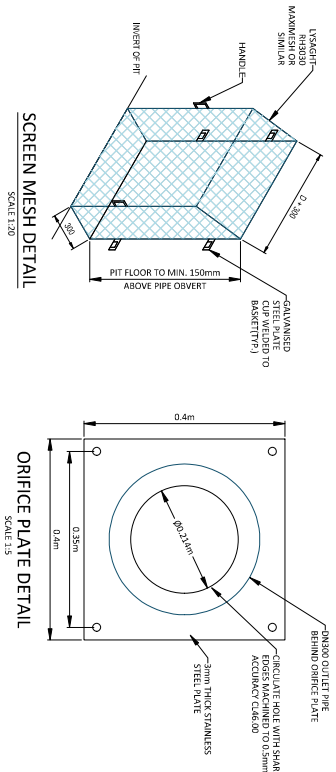
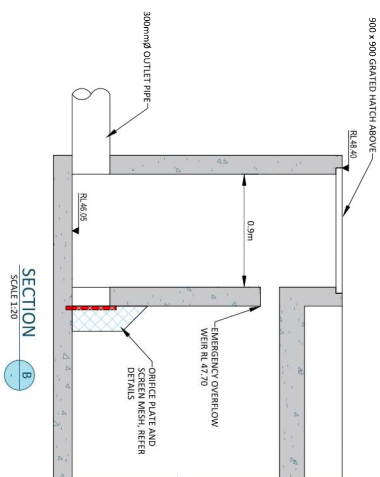
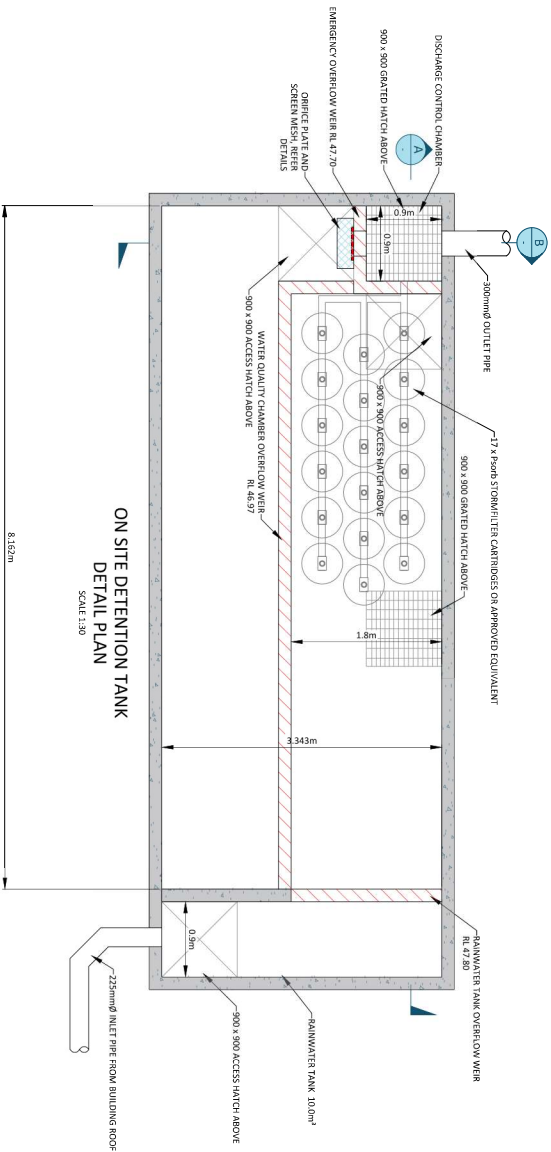
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X	FOR APPROVAL	11.03.26
Y	FOR APPROVAL	11.03.26
Z	FOR APPROVAL	11.03.26

SASCO PROPERTY	CURZON + PARTNERS
CLIENT	ARCHITECT

166-174 BLAXLAND ROAD RYDE	STORMWATER MANAGEMENT DETAILS SHEET 1
PROJECT	TITLE

enscape studio	FOR APPROVAL
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Appendix B – MUSIC Model Output

Source nodes
 Location, Roof Catchment (3108m2), Rear Landscape (724m2)
 ID, 1, 2
 Node Type, UrbanSourceNode, UrbanSourceNode
 Zoning Surface Type, Roof, Revegetatedland
 Total Area (ha), 0.311, 0.072
 Area Impervious (ha), 0.311, 0
 Area Pervious (ha), 0, 0.072
 Field Capacity (mm), 80, 80
 Pervious Area Infiltration Capacity coefficient - a, 200, 200
 Pervious Area Infiltration Capacity exponent - b, 1, 1
 Impervious Area Rainfall Threshold (mm/day), 1, 1
 Pervious Area Soil Storage Capacity (mm), 120, 120
 Pervious Area Soil Initial Storage (% of Capacity), 25, 25
 Groundwater Initial Depth (mm), 10, 10
 Groundwater Daily Recharge Rate (%), 25, 25
 Groundwater Daily Baseflow Rate (%), 5, 5
 Groundwater Daily Deep Seepage Rate (%), 0, 0
 Stormflow Total Suspended Solids Mean (log mg/L), 1.3, 1.95
 Stormflow Total Suspended Solids Standard Deviation (log mg/L), 0.32, 0.32
 Stormflow Total Suspended Solids Estimation Method, Stochastic, Stochastic
 Stormflow Total Suspended Solids Serial Correlation, 0, 0
 Stormflow Total Phosphorus Mean (log mg/L), -0.89, -0.66
 Stormflow Total Phosphorus Standard Deviation (log mg/L), 0.25, 0.25
 Stormflow Total Phosphorus Estimation Method, Stochastic, Stochastic
 Stormflow Total Phosphorus Serial Correlation, 0, 0
 Stormflow Total Nitrogen Mean (log mg/L), 0.3, 0.3
 Stormflow Total Nitrogen Standard Deviation (log mg/L), 0.19, 0.19
 Stormflow Total Nitrogen Estimation Method, Stochastic, Stochastic
 Stormflow Total Nitrogen Serial Correlation, 0, 0
 Baseflow Total Suspended Solids Mean (log mg/L), 1.1, 1.15
 Baseflow Total Suspended Solids Standard Deviation (log mg/L), 0.17, 0.17
 Baseflow Total Suspended Solids Estimation Method, Stochastic, Stochastic
 Baseflow Total Suspended Solids Serial Correlation, 0, 0
 Baseflow Total Phosphorus Mean (log mg/L), -0.82, -1.22
 Baseflow Total Phosphorus Standard Deviation (log mg/L), 0.19, 0.19
 Baseflow Total Phosphorus Estimation Method, Stochastic, Stochastic
 Baseflow Total Phosphorus Serial Correlation, 0, 0
 Baseflow Total Nitrogen Mean (log mg/L), 0.32, -0.05
 Baseflow Total Nitrogen Standard Deviation (log mg/L), 0.12, 0.12
 Baseflow Total Nitrogen Estimation Method, Stochastic, Stochastic
 Baseflow Total Nitrogen Serial Correlation, 0, 0
 Flow based constituent generation - enabled, Off, Off
 Flow based constituent generation - flow file, ,
 Flow based constituent generation - base flow column, ,
 Flow based constituent generation - pervious flow column, ,
 Flow based constituent generation - impervious flow column, ,
 Flow based constituent generation - unit, ,
 OUT - Mean Annual Flow (ML/yr), 4.19, 0.331
 OUT - TSS Mean Annual Load (kg/yr), 107, 21.4
 OUT - TP Mean Annual Load (kg/yr), 0.615, 53.8E-3
 OUT - TN Mean Annual Load (kg/yr), 9.21, 0.515
 OUT - Gross Pollutant Mean Annual Load (kg/yr), 101, 0.00
 Rain In (ML/yr), 4.63264, 1.07251
 ET Loss (ML/yr), 0.4392, 0.741085
 Deep Seepage Loss (ML/yr), 0, 0
 Baseflow Out (ML/yr), 0, 0.175303
 Imp. Stormflow Out (ML/yr), 4.19344, 0

Perv. Stormflow Out (ML/yr),0,0.156122
 Total Stormflow Out (ML/yr),4.19344,0.156122
 Total Outflow (ML/yr),4.19344,0.331425
 Change in Soil Storage (ML/yr),0,-2E-6
 TSS Baseflow Out (kg/yr),0,2.68157
 TSS Total Stormflow Out (kg/yr),107.044,18.7325
 TSS Total Outflow (kg/yr),107.044,21.4141
 TP Baseflow Out (kg/yr),0,0.011626
 TP Total Stormflow Out (kg/yr),0.615182,0.042127
 TP Total Outflow (kg/yr),0.615182,0.053753
 TN Baseflow Out (kg/yr),0,0.162496
 TN Total Stormflow Out (kg/yr),9.21138,0.352678
 TN Total Outflow (kg/yr),9.21138,0.515174
 GP Total Outflow (kg/yr),101.472,0

No Imported Data Source nodes

USTM treatment nodes

Location,Rainwater Tank (10m3),SF Chamber
 ID,3,4
 Node Type,RainWaterTankNode,SedimentationBasinNode
 Lo-flow bypass rate (cum/sec),0,0
 Hi-flow bypass rate (cum/sec),100,100
 Inlet pond volume,0,0
 Area (sqm),5,6.3
 Initial Volume (m^3),0,0
 Extended detention depth (m),0.2,0.77
 Number of Rainwater tanks,1,
 Permanent Pool Volume (cubic metres),10,0
 Proportion vegetated,0,0
 Equivalent Pipe Diameter (mm),50,97
 Overflow weir width (m),10,2
 Notional Detention Time (hrs),0.107,70.0E-3
 Orifice Discharge Coefficient,0.6,0.6
 Weir Coefficient,1.7,1.7
 Number of CSTR Cells,2,1
 Total Suspended Solids - k (m/yr),400,1
 Total Suspended Solids - C* (mg/L),12,20
 Total Suspended Solids - C** (mg/L),0,20
 Total Phosphorus - k (m/yr),300,1
 Total Phosphorus - C* (mg/L),0.13,0.13
 Total Phosphorus - C** (mg/L),0,0.13
 Total Nitrogen - k (m/yr),40,1
 Total Nitrogen - C* (mg/L),1.4,1.4
 Total Nitrogen - C** (mg/L),0,1.4
 Threshold Hydraulic Loading for C** (m/yr),0,3500
 Horizontal Flow Coefficient, ,
 Reuse Enabled,On,Off
 Max drawdown height (m),2,
 Annual Demand Enabled,On,Off
 Annual Demand Value (ML/year),0.3,
 Annual Demand Distribution,PET,
 Annual Demand Monthly Distribution: Jan, ,
 Annual Demand Monthly Distribution: Feb, ,
 Annual Demand Monthly Distribution: Mar, ,
 Annual Demand Monthly Distribution: Apr, ,
 Annual Demand Monthly Distribution: May, ,
 Annual Demand Monthly Distribution: Jun, ,

Annual Demand Monthly Distribution: Jul, ,
 Annual Demand Monthly Distribution: Aug, ,
 Annual Demand Monthly Distribution: Sep, ,
 Annual Demand Monthly Distribution: Oct, ,
 Annual Demand Monthly Distribution: Nov, ,
 Annual Demand Monthly Distribution: Dec, ,
 Daily Demand Enabled,Off,Off
 Daily Demand Value (ML/day), ,
 Custom Demand Enabled,Off,Off
 Custom Demand Time Series File, ,
 Custom Demand Time Series Units, ,
 Filter area (sqm), ,
 Filter perimeter (m), ,
 Filter depth (m), ,
 Filter Median Particle Diameter (mm), ,
 Saturated Hydraulic Conductivity (mm/hr), ,
 Infiltration Media Porosity, ,
 Length (m), ,
 Bed slope, ,
 Base Width (m), ,
 Top width (m), ,
 Vegetation height (m), ,
 Vegetation Type, ,
 Total Nitrogen Content in Filter (mg/kg), ,
 Orthophosphate Content in Filter (mg/kg), ,
 Is Base Lined?, ,
 Is Underdrain Present?, ,
 Is Submerged Zone Present?, ,
 Submerged Zone Depth (m), ,
 B for Media Soil Texture,-9999,-9999
 Proportion of upstream impervious area treated, ,
 Exfiltration Rate (mm/hr),0,0
 Evaporative Loss as % of PET,0,0
 Depth in metres below the drain pipe, ,
 TSS A Coefficient, ,
 TSS B Coefficient, ,
 TP A Coefficient, ,
 TP B Coefficient, ,
 TN A Coefficient, ,
 TN B Coefficient, ,
 Sfc, ,
 S*, ,
 Sw, ,
 Sh, ,
 Emax (m/day), ,
 Ew (m/day), ,
 IN - Mean Annual Flow (ML/yr),4.19,4.25
 IN - TSS Mean Annual Load (kg/yr),107,98.8
 IN - TP Mean Annual Load (kg/yr),0.615,0.608
 IN - TN Mean Annual Load (kg/yr),9.21,8.95
 IN - Gross Pollutant Mean Annual Load (kg/yr),101,0.00
 OUT - Mean Annual Flow (ML/yr),3.92,4.25
 OUT - TSS Mean Annual Load (kg/yr),93.3,98.3
 OUT - TP Mean Annual Load (kg/yr),0.570,0.607
 OUT - TN Mean Annual Load (kg/yr),8.54,8.95
 OUT - Gross Pollutant Mean Annual Load (kg/yr),0.00,0.00
 Flow In (ML/yr),4.19344,4.24732
 ET Loss (ML/yr),0,0

Infiltration Loss (ML/yr),0,0
 Low Flow Bypass Out (ML/yr),0,0
 High Flow Bypass Out (ML/yr),0,0
 Orifice / Filter Out (ML/yr),1.78907,3.78878
 Weir Out (ML/yr),2.12662,0.458685
 Transfer Function Out (ML/yr),0,0
 Reuse Supplied (ML/yr),0.277278,0
 Reuse Requested (ML/yr),0.299966,0
 % Reuse Demand Met,92.4365,0
 % Load Reduction,6.62344,-0.00341392
 TSS Flow In (kg/yr),107.044,98.8054
 TSS ET Loss (kg/yr),0,0
 TSS Infiltration Loss (kg/yr),0,0
 TSS Low Flow Bypass Out (kg/yr),0,0
 TSS High Flow Bypass Out (kg/yr),0,0
 TSS Orifice / Filter Out (kg/yr),42.4691,87.6321
 TSS Weir Out (kg/yr),50.858,10.6625
 TSS Transfer Function Out (kg/yr),0,0
 TSS Reuse Supplied (kg/yr),4.47963,0
 TSS Reuse Requested (kg/yr),0,0
 TSS % Reuse Demand Met,0,0
 TSS % Load Reduction,12.8143,0.516976
 TP Flow In (kg/yr),0.615182,0.60778
 TP ET Loss (kg/yr),0,0
 TP Infiltration Loss (kg/yr),0,0
 TP Low Flow Bypass Out (kg/yr),0,0
 TP High Flow Bypass Out (kg/yr),0,0
 TP Orifice / Filter Out (kg/yr),0.26137,0.541528
 TP Weir Out (kg/yr),0.308783,0.0659027
 TP Transfer Function Out (kg/yr),0,0
 TP Reuse Supplied (kg/yr),0.0375704,0
 TP Reuse Requested (kg/yr),0,0
 TP % Reuse Demand Met,0,0
 TP % Load Reduction,7.31962,0.0574715
 TN Flow In (kg/yr),9.21138,8.95124
 TN ET Loss (kg/yr),0,0
 TN Infiltration Loss (kg/yr),0,0
 TN Low Flow Bypass Out (kg/yr),0,0
 TN High Flow Bypass Out (kg/yr),0,0
 TN Orifice / Filter Out (kg/yr),3.91893,7.98386
 TN Weir Out (kg/yr),4.62532,0.964174
 TN Transfer Function Out (kg/yr),0,0
 TN Reuse Supplied (kg/yr),0.550419,0
 TN Reuse Requested (kg/yr),0,0
 TN % Reuse Demand Met,0,0
 TN % Load Reduction,7.24245,0.0358163
 GP Flow In (kg/yr),101.472,0
 GP ET Loss (kg/yr),0,0
 GP Infiltration Loss (kg/yr),0,0
 GP Low Flow Bypass Out (kg/yr),0,0
 GP High Flow Bypass Out (kg/yr),0,0
 GP Orifice / Filter Out (kg/yr),0,0
 GP Weir Out (kg/yr),0,0
 GP Transfer Function Out (kg/yr),0,0
 GP Reuse Supplied (kg/yr),0,0
 GP Reuse Requested (kg/yr),0,0
 GP % Reuse Demand Met,0,0
 GP % Load Reduction,100,100

PET Scaling Factor, ,

Generic treatment nodes

Location,17 x 690mm Psorb StormFilter (MCC),1 x OceanGuard
ID,5,7

Node Type,GenericNode,GPTNode

Lo-flow bypass rate (cum/sec),0,0

Hi-flow bypass rate (cum/sec),0.0153,0.02

Flow Transfer Function

Input (cum/sec),0,0

Output (cum/sec),0,0

Input (cum/sec),10,10

Output (cum/sec),10,10

Input (cum/sec), ,

Output (cum/sec), ,

Input (cum/sec), ,

Output (cum/sec), ,

Input (cum/sec), ,

Output (cum/sec), ,

Input (cum/sec), ,

Output (cum/sec), ,

Input (cum/sec), ,

Output (cum/sec), ,

Input (cum/sec), ,

Output (cum/sec), ,

Input (cum/sec), ,

Output (cum/sec), ,

Input (cum/sec), ,

Output (cum/sec), ,

Gross Pollutant Transfer Function

Enabled,True,True

Input (kg/ML),0,0

Output (kg/ML),0,0

Input (kg/ML),14.9393,14.7808

Output (kg/ML),0,0

Input (kg/ML), ,

Output (kg/ML), ,

Input (kg/ML), ,

Output (kg/ML), ,

Input (kg/ML), ,

Output (kg/ML), ,

Input (kg/ML), ,

Output (kg/ML), ,

Input (kg/ML), ,

Output (kg/ML), ,

Input (kg/ML), ,

Output (kg/ML), ,

Input (kg/ML), ,

Output (kg/ML), ,

Input (kg/ML), ,

Output (kg/ML), ,

Total Nitrogen Transfer Function

Enabled,True,True

Input (mg/L),0,0

Output (mg/L),0,0

Input (mg/L),100,50

Output (mg/L),44.1,39.5

Input (mg/L), ,

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Output (mg/L) , ,
Input (mg/L) , ,
Output (mg/L) , ,
Input (mg/L) , ,
Output (mg/L) , ,
Input (mg/L) , ,
Output (mg/L) , ,
Input (mg/L) , ,
Output (mg/L) , ,
Input (mg/L) , ,
Output (mg/L) , ,
Input (mg/L) , ,
Output (mg/L) , ,
Input (mg/L) , ,
Output (mg/L) , ,
Total Phosphorus Transfer Function
Enabled,True,True
Input (mg/L) ,0,0
Output (mg/L) ,0,0
Input (mg/L) ,10,10
Output (mg/L) ,1.39,7
Input (mg/L) , ,
Output (mg/L) , ,
Input (mg/L) , ,
Output (mg/L) , ,
Input (mg/L) , ,
Output (mg/L) , ,
Input (mg/L) , ,
Output (mg/L) , ,
Input (mg/L) , ,
Output (mg/L) , ,
Input (mg/L) , ,
Output (mg/L) , ,
Input (mg/L) , ,
Output (mg/L) , ,
Total Suspended Solids Transfer Function
Enabled,True,True
Input (mg/L) ,0,0
Output (mg/L) ,0,0
Input (mg/L) ,1000,20.8
Output (mg/L) ,66,8
Input (mg/L) , ,40.3
Output (mg/L) , ,14.1
Input (mg/L) , ,60.6
Output (mg/L) , ,19.3
Input (mg/L) , ,79.3
Output (mg/L) , ,23.4
Input (mg/L) , ,99.9
Output (mg/L) , ,26.9
Input (mg/L) , ,121
Output (mg/L) , ,30
Input (mg/L) , ,
Output (mg/L) , ,
Input (mg/L) , ,
Output (mg/L) , ,
Input (mg/L) , ,

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Output (mg/L), ,
 TSS Flow based Efficiency Enabled,Off,Off
 TSS Flow based Efficiency, ,
 TP Flow based Efficiency Enabled,Off,Off
 TP Flow based Efficiency, ,
 TN Flow based Efficiency Enabled,Off,Off
 TN Flow based Efficiency, ,
 GP Flow based Efficiency Enabled,Off,Off
 GP Flow based Efficiency, ,
 IN - Mean Annual Flow (ML/yr),4.25,0.331
 IN - TSS Mean Annual Load (kg/yr),98.3,21.4
 IN - TP Mean Annual Load (kg/yr),0.607,53.8E-3
 IN - TN Mean Annual Load (kg/yr),8.95,0.515
 IN - Gross Pollutant Mean Annual Load (kg/yr),0.00,0.00
 OUT - Mean Annual Flow (ML/yr),4.25,0.331
 OUT - TSS Mean Annual Load (kg/yr),17.9,5.48
 OUT - TP Mean Annual Load (kg/yr),0.149,37.6E-3
 OUT - TN Mean Annual Load (kg/yr),4.56,0.407
 OUT - Gross Pollutant Mean Annual Load (kg/yr),0.00,0.00
 Flow In (ML/yr),4.24733,0.331427
 ET Loss (ML/yr),0,0
 Infiltration Loss (ML/yr),0,0
 Low Flow Bypass Out (ML/yr),0,0
 High Flow Bypass Out (ML/yr),0.524057,0
 Orifice / Filter Out (ML/yr),0,0
 Weir Out (ML/yr),0,0
 Transfer Function Out (ML/yr),3.7234,0.331427
 Reuse Supplied (ML/yr),0,0
 Reuse Requested (ML/yr),0,0
 % Reuse Demand Met,0,0
 % Load Reduction,-0.00317847,0
 TSS Flow In (kg/yr),98.2929,21.4142
 TSS ET Loss (kg/yr),0,0
 TSS Infiltration Loss (kg/yr),0,0
 TSS Low Flow Bypass Out (kg/yr),0,0
 TSS High Flow Bypass Out (kg/yr),12.1924,0
 TSS Orifice / Filter Out (kg/yr),0,0
 TSS Weir Out (kg/yr),0,0
 TSS Transfer Function Out (kg/yr),5.68251,5.4783
 TSS Reuse Supplied (kg/yr),0,0
 TSS Reuse Requested (kg/yr),0,0
 TSS % Reuse Demand Met,0,0
 TSS % Load Reduction,81.8146,74.4174
 TP Flow In (kg/yr),0.607447,0.053752
 TP ET Loss (kg/yr),0,0
 TP Infiltration Loss (kg/yr),0,0
 TP Low Flow Bypass Out (kg/yr),0,0
 TP High Flow Bypass Out (kg/yr),0.075502,0
 TP Orifice / Filter Out (kg/yr),0,0
 TP Weir Out (kg/yr),0,0
 TP Transfer Function Out (kg/yr),0.07394,0.037626
 TP Reuse Supplied (kg/yr),0,0
 TP Reuse Requested (kg/yr),0,0
 TP % Reuse Demand Met,0,0
 TP % Load Reduction,75.3983,30.0007
 TN Flow In (kg/yr),8.94817,0.515171
 TN ET Loss (kg/yr),0,0
 TN Infiltration Loss (kg/yr),0,0

TN Low Flow Bypass Out (kg/yr),0,0
 TN High Flow Bypass Out (kg/yr),1.10274,0
 TN Orifice / Filter Out (kg/yr),0,0
 TN Weir Out (kg/yr),0,0
 TN Transfer Function Out (kg/yr),3.45966,0.406987
 TN Reuse Supplied (kg/yr),0,0
 TN Reuse Requested (kg/yr),0,0
 TN % Reuse Demand Met,0,0
 TN % Load Reduction,49.0131,20.9996
 GP Flow In (kg/yr),0,0
 GP ET Loss (kg/yr),0,0
 GP Infiltration Loss (kg/yr),0,0
 GP Low Flow Bypass Out (kg/yr),0,0
 GP High Flow Bypass Out (kg/yr),0,0
 GP Orifice / Filter Out (kg/yr),0,0
 GP Weir Out (kg/yr),0,0
 GP Transfer Function Out (kg/yr),0,0
 GP Reuse Supplied (kg/yr),0,0
 GP Reuse Requested (kg/yr),0,0
 GP % Reuse Demand Met,0,0
 GP % Load Reduction,100,100

Other nodes

Location,Receiving Node
 ID,6
 Node Type,ReceivingNode
 IN - Mean Annual Flow (ML/yr),4.25
 IN - TSS Mean Annual Load (kg/yr),17.9
 IN - TP Mean Annual Load (kg/yr),0.149
 IN - TN Mean Annual Load (kg/yr),4.56
 IN - Gross Pollutant Mean Annual Load (kg/yr),0.00
 OUT - Mean Annual Flow (ML/yr),4.25
 OUT - TSS Mean Annual Load (kg/yr),17.9
 OUT - TP Mean Annual Load (kg/yr),0.149
 OUT - TN Mean Annual Load (kg/yr),4.56
 OUT - Gross Pollutant Mean Annual Load (kg/yr),0.00
 % Load Reduction,6.14
 TSS % Load Reduction,86.1
 TN % Load Reduction,53.1
 TP % Load Reduction,77.7
 GP % Load Reduction,100

Links

Location,Drainage Link,Drainage Link,Drainage Link,Drainage Link,Drainage Link,Drainage Link
 Source node ID,1,3,4,5,7,2
 Target node ID,3,4,5,6,4,7
 Muskingum-Cunge Routing,Not Routed,Not Routed,Not Routed,Not Routed,Not Routed,Not Routed
 Muskingum K, , , , , ,
 Muskingum theta, , , , , ,
 IN - Mean Annual Flow (ML/yr),4.19,3.92,4.25,4.25,0.331,0.331
 IN - TSS Mean Annual Load (kg/yr),107,93.3,98.3,17.9,5.48,21.4
 IN - TP Mean Annual Load (kg/yr),0.615,0.570,0.607,0.149,37.6E-3,53.8E-3
 IN - TN Mean Annual Load (kg/yr),9.21,8.54,8.95,4.56,0.407,0.515
 IN - Gross Pollutant Mean Annual Load (kg/yr),101,0.00,0.00,0.00,0.00,0.00
 OUT - Mean Annual Flow (ML/yr),4.19,3.92,4.25,4.25,0.331,0.331

OUT - TSS Mean Annual Load (kg/yr),107,93.3,98.3,17.9,5.48,21.4
OUT - TP Mean Annual Load (kg/yr),0.615,0.570,0.607,0.149,37.6E-3,53.8E-3
OUT - TN Mean Annual Load (kg/yr),9.21,8.54,8.95,4.56,0.407,0.515
OUT - Gross Pollutant Mean Annual Load
(kg/yr),101,0.00,0.00,0.00,0.00,0.00

Catchment Details

Catchment Name,0610-Blaxland Rd - MUSIC Model
Timestep,6 Minutes
Start Date,1/01/1959
End Date,31/12/1959 11:54:00 PM
Rainfall Station, 66062 SYDNEY
ET Station,Monthly User Defined
Mean Annual Rainfall (mm), 1490
Mean Annual ET (mm), 1260



Tel: 0411 267 151

E: info@enscapestudio.com.au