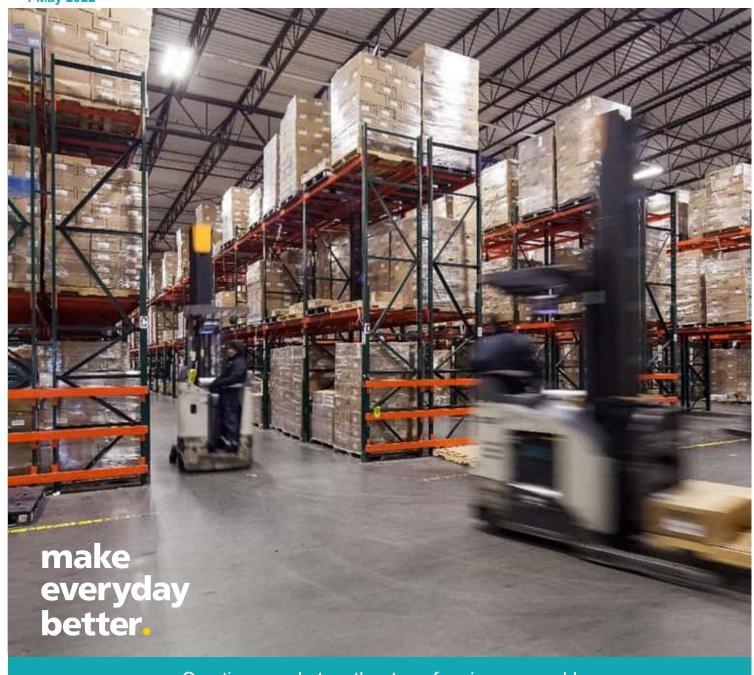


# **Stormwater Management Plan – Prospect South Expansion**

Prepared for Americold Logistics Ltd Prepared by Beca Pty Ltd ABN: 85 004 974 341

1 May 2022



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# **Revision History**

Revision Nº	Prepared By	Description	Date
Α	Mingxin Zheng	Issue For EIS	31 May 2022
В	Elisa Moraitis	Re-Issue For EIS	23 Jan. 23

#### **Document Acceptance**

Action	Name	Signed	Date
Prepared by	Elisa Moraitis	Ento	23 Jan. 23
Reviewed by	Ben Strang	Elson .	23 Jan. 23
Approved by	Matt Brookes	MatthBroken	23 Jan. 23
on behalf of	Beca Pty Ltd		

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### 1 Project Description

#### 1.1 Introduction

Americold Logistics Ltd (Americold) currently operates an existing two-building cold storage facility at 554-562 Reservoir Road, Prospect NSW 2148. To accommodate future customer needs, Americold is intending to expand the facility and provide additional storage area, loading docks, supporting facilities and extra parking spaces. To enable this development, Americold is submitting a State Significant Development Application (SSDA) including an Environmental Impact Statement (EIS) to the Department of Planning, Industry and Environment (DPIE) for approval.

Beca Pty Ltd (Beca) has been engaged to develop a Stormwater Management Plan as part of the required documentation to address the soil and stormwater related Secretary Environmental Assessment Requirements (SEARs).

#### 1.2 Purpose of this Report

This report includes the following items:

- Design requirements and approaches for site drainage management
- Detailed plans and a description of the integration of the proposed and existing surface and stormwater management system, including on-site detention, designed in accordance with Water Sensitive Urban Design principles
- · Details of proposed erosion and sediment controls during construction

#### 1.3 Existing Site Condition

The development site at 554-562 Reservoir Road, Prospect NSW 2148 and within Cumberland City Council extent. The site is accessed from Reservoir Road and mainly surrounded by other industrial buildings. There are currently two main industrial buildings within the site with circulation roadways at the northern side, southern side and between the two buildings. Girraween Creek is located to the south-east side to the site.



Figure 1-1 Locality Plan (Source: NEARMAP, 2022 Imagery)

#### 1.3.1 Existing Site Topography

A detailed survey was undertaken by LandPartners built environment consultants on 11/06/2020. The survey drawings (SY074943.000.3, Rev 1) show existing building outlines, site contours, indicative lighting and indicative existing utilities.

The overall site is falling from the from the north-west (Reservoir Road) to south-east side (Girraween Creek). The slope ranges from the entry driveway of about 8% slope with the rest the site averaging to 5%.

The total site area is 6.566ha. From an estimation, the existing impervious area is 4.013ha which is about 61% of the whole site, while the existing pervious area is 2.553ha which is about 39%.

#### 1.3.2 Existing Stormwater Infrastructure

There is an existing drainage pit and pipe network within the site that captures roof stormwater and surface runoff. The survey drawings (SY074943.000.3, Rev 1) provided by LandPartners have shown indicative pit locations, cover levels and invert levels. The actual pits and pipes network structure (pit linkages) and roof drainage connections have not been provided and have been deduced by site visit and logical assumptions. Existing stormwater system will be confirmed prior to construction. Based on the survey drawings (SY074943.000.3, Rev 1), it is assumed that there are two discharge outlets at the rear of the site to discharge stormwater to Girraween Creek. The two discharge outlets are as shown in Figure 1-2.



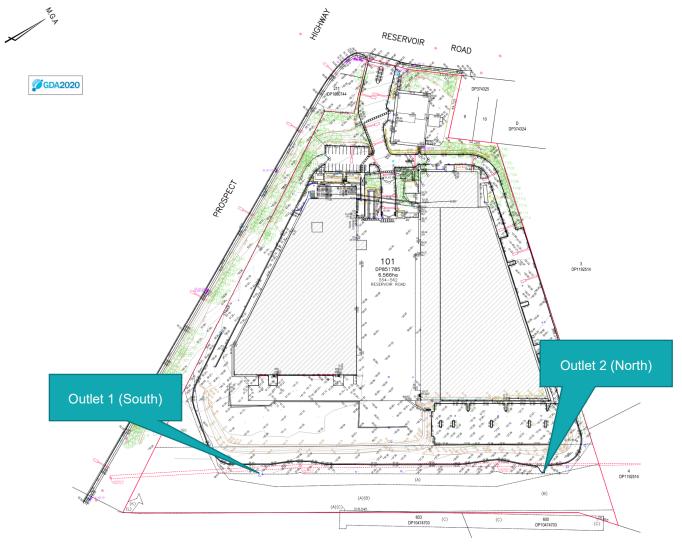


Figure 1-2 Survey plan (Source: LandPartners, 2020)

#### 1.4 Proposed Development

The latest architectural drawing set (2527456-DA-0101 to 8101, Rev A) includes the following:

- A new freezer store building adjacent to the southern warehouse, with attached annex
- A hardstand extension for pallet storage adjacent to the northern warehouse
- A new plastic-pallets building at the existing car park adjacent to the access driveway
- Ancillary buildings including back of house and plant rooms
- A new ramp and circulation route on the south-eastern boundary of the site to accommodate truck access
- A new passenger vehicle entrance for staff, separated from the heavy vehicle access driveway
- A new car park in the eastern corner of the site



From an analysis of site areas, the changes to the impervious and pervious percentages of the site are as per Table 1-1:

Table 1-1 Changes to impervious and pervious areas

	Pre-development		Post-development		Changes	
Impervious area	4.013ha	61%	5.005ha	76%	+0.992 ha	+15%
Pervious area	2.553ha	39%	1.561ha	24%	-0.992ha	-15%
Total site area	6.566ha					

## 2 Stormwater Management

#### 2.1 Design Criteria

#### 2.1.1 Stormwater Requirement and Standards

A Concept Stormwater Drainage package has been prepared for the proposed industrial extension development. The stormwater management drawings are designed to comply with the following requirements:

- AS 3500.3 National Plumbing, 2018
- Australian Rainfall and Runoff (AR&R), 2016
- Cumberland City Council, Development Control Plan (DCP), 2021
- Upper Parramatta River Catchment Trust On-site Stormwater Detention Handbook, Vol 4.
- Urban Stormwater Quality Planning Guideline, 2010
- Best Practice Erosion & Sediment Control for building and construction sites, 2008

#### 2.1.2 Stormwater Drainage System Design Criteria

The stormwater system shall be designed and constructed based on the following;

- According to Development Control Plan (DCP) 2021 from Cumberland City Council,
  - Minor storm to be designed to 5% Annual Exceedance Probability (AEP) storm event, which will be drained to an underground system.
  - o Major storm to be designed to 1% AEP storm event, which will be conveyed as overland flow.
- The existing stormwater discharge outlets will be retained.-
- The design is to adopt Water Sensitive Urban Design (WSUD) principles.
- On site detention to meet the requirements of the Upper Parramatta River Catchment Trust On-site Stormwater Detention Handbook and DRAINS model shall be provided.
- Materials shall have an approved watermark or Australian Standards stamp thereon. Pipe Classes shall be clearly stamped on all pipes.
- Minimum grade to be 0.5% fall.



- All stormwater pit grate and frames shall be class 'D' heavy duty trafficable types. All other pits to be minimum class 'B'.
- If any upgrade works are required within the endeavour easement, all infrastructure to be non-conductive in accordance with Endeavour Energy requirements.

#### 2.2 On-Site Detention (OSD) Design

It is noted that the site is identified as part of the Pemulwuy Northern Employment Lands as described in the Introduction to Cumberland Council DCP Part F3 which specifically calls out 'Lot 101 DP 851785l' and from figure 2 of the DCP Part F3, it is shown that the runoff from the site is directed to a DUAP detention basin. However, as the development site is within Upper Parramatta River Catchment as shown in Figure 21, On-Site Detention (OSD) is required to be provided to limit the discharge of runoff from areas of the proposed development to a specified PSD and SSR. Due to the size of the development DRAINS was used to determine the required volume and the relevant PSD was met. This is in accordance with Council's DCP Part G, Part G4, On-Site Detention, Design, C2. Alternative values for the required storage volume can be considered for larger sites greater than 3000sqm if the applicant demonstrates to Council's satisfaction using appropriate computer modelling that the relevant PSD shall be satisfied. The inputs into the drains model are discussed further in section 2.2.1.

Due to the site constraints and tailwater levels, two underground OSD tanks has been designed in accordance with Upper Parramatta River Catchment Trust On-site Stormwater Detention Handbook, Vol 4.

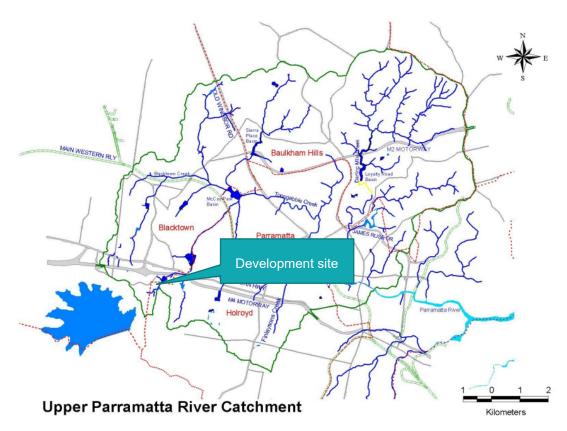


Figure 2-1 Upper Parramatta River Catchment Map (Source: Upper Parramatta River Catchment Trust On-Site Detention Handbook, Vol 4)



#### 2.2.1 Hydrologic and Hydraulic Modelling

DRAINS modelling was used to determine the volume of the two OSD tanks.

#### a. Rainfall Data

The rainfall data utilised in this assessment is the 2016 Intensity Frequency Duration (IFD) sourced from the Bureau of Meteorology for the site. The temporal patterns were obtained from the Australian Rainfall and Runoff (ARR) Data Hub. The decimal degrees that the Design Rainfall Data system uses to determine the location was input as -33.81007,150.92028.

#### b. Hydrologic Methodology

ILSAX method has been used for the analysis of stormwater runoff. The adopted loss parameters are summarised in Figure 2-1.

Table 2-1 Hydrological loss parameters

Parameter	
Impervious area depression storage	1mm
Pervious area depression storage	5mm
Soil type	3 (assumed)

#### 2.2.2 On-Site Detention (OSD) Detail

As previously shown in figure 12 the site currently has two outlets that discharge to the Girraween creek. Outlet one (South) and Outlet 2 (North). Due to the development and existing site constraints, the arrangement of catchments draining to each of the outlets has been changed. The revised catchment arrangement is shown on drawing 2527456-CA-901. An snip of the drawing has been prepared and is shown in Figure 4.



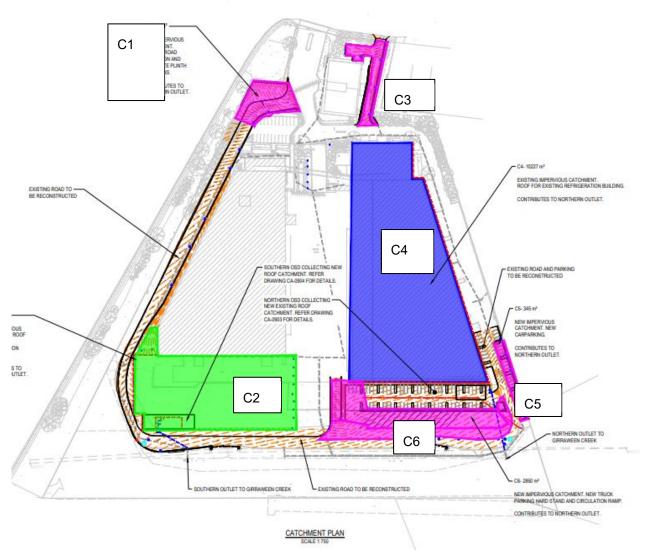


Figure 2 - Snip from Catchment Drawing 2527456-CA-901

From the above, the pink and green hatch areas are the new impervious areas being developed. The Green area is the proposed roof and is drained to the On-site Detention tank 1 (South). Due to the site constraints and tailwater levels, the areas shown in pink bypass the on-site detention. To meet the PSD requirements,



the area shown in blue (existing roof drainage) will be diverted to OSD 2 (North) and the equivalent runoff generated from the bypass flows will be controlled to the PSD rate.

For the Site PSD (As per UPRCT PSD = 80l/s/ha)

Total new development area = 1.0150ha

Therefore total PSD = 1.015\*80 = 81.2l/s = 0.0812m3/s

For the Southern Outlet, the Tail Water Level of the creek at the major storm event is RL 58.25 which is incorporated into the node 'Outlet South' in DRAINS.

Catchment area C1 shown in figure 4 has a total area of 0.09ha and has a runoff Q100 peak flow of 0.056m3/s (From Drains).

Catchment C2 has a total area of 0.5505ha and a runoff Q100 peak flow of 0.325m3/s

For the Northern Outlet, the Tail Water Level of the creek at the major storm even is RL 55.67 and is incorporated into node 'Outlet North' in Drains.

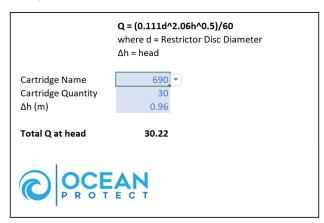
Catchment C3+C5+C6 has a total area of 0.3745ha and has a runoff Q100 peakflow of 0.233m3/s. These areas bypass the OSD

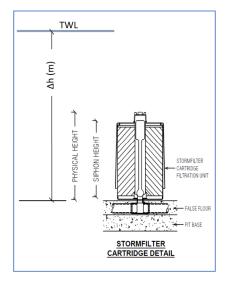
The Existing Roof (C4) has a total area of 1.023ha and a Q100 peakflow of 0.604m3/s

Therefore to determine the total allowable flow =

- = Roof runoff bypass runoff + (PSD of developed area)
- = 0.604 (0.056 + 0.233) + 0.0812
- = 0.396 m 3/s

In addition there is a flow discharging from the South OSD filter cartridge system as it separated from the HED pit. This flow is calculated from excel and shown in snip below.





Therefore the Total discharge from site =

OSD discharge + Stormfiter Cartrige flow = 0.018+0.329+0.03022 = 0.3772m3/s

As the allowable discharge is greater then the total site discharge it is compliant.

Table 2-2 OSD parameters (South OSD)



OSD Parameters (South OSD)				
Surface Area	14m x 26m			
Top Water Level (TWL) at 1% AEP	59.31			
Weir Crest Level	59.25			
Orifice Diameter	88mm			
Outlet Pipe Diamter and IL	225mm & IL 58.22			
Volume (Excluding Filters)	350m3			

OSD Parameters (North OSD)				
Surface Area	9.4m x 17.8m			
Top Water Level (TWL) at 1% AEP	56.63			
Weir Crest Level	56.5			
Orifice Diameter	450mm			
Outlet Pipe Diamter and IL	525mm & IL 56.07			
Volume (Excluding Filters)	100m3			

#### 2.3 Water Sensitive Urban Design (WSUD)

According to Section 2.5 in Cumberland City Council Development Control Plan Part G – Miscellaneous Development Control, in order to protect and enhance natural water system, the development site is required to implement water sensitive urban design.

Table 2-3shows the pollutant reduction requirements for all developments.

Table 2-3 Stormwater Quality Targets

Pollutant	Litter	Reduction in Load
Litter e.g. cans, bottles, wrapping materials, food scraps	All anthropogenic materials with a minimum dimension >5mm	90%
Coarse sediment	Coarse sand and soil particles (<0.5mm diameter)	85%
Nutrients	Total phosphorous nitrogen	60%
Fine particles	Coarse sand and soil particles (<0.05mm diameter)	85%
Cooking oil and grease	Free floating oils that do not emulsify aqueous solutions	90%
Hydrocarbons inc. motor fuels, oils and greases	Anthropogenic hydrocarbons that can be emulsified	90%



#### 2.3.1 WSUD Methodology

To estimate the effectiveness of the proposed treatment elements on the water quality of the post-developed site, MUSIC (v. 6.3) modelling has been utilised. The MUSIC model has only assessed the development area within the site. The non-development area is excluded from the modelling.

MUSIC basefile model use the following parameters:

- Rainfall data from Parramatta Rainfall station (1984-2007)
- Source nodes from Blacktown City Council
- Treatment nodes from OceanProtect (manufacturer of treatment devices used in this proposal)

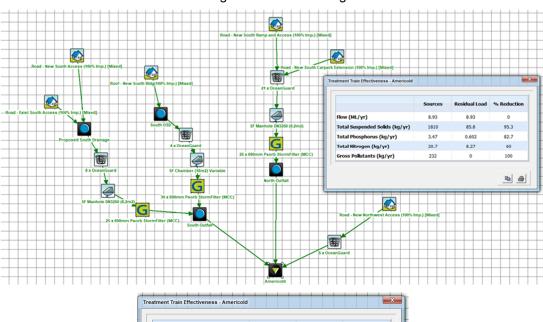
WSUD treatment train has been developed based on the proposed drainage concept design.

#### 2.3.2 WSUD Results

Litter and fine particles are proposed to be removed by installing Oceanguard®, to serves as an in-line gross pollutant trap (GPT), in all inlet pits and at the south building on-site-detention (OSD) tank. Suspended solids, nutrients and hydrocarbons are proposed to be addressed by using StormFilter®. One StormFilter® manhole will each be installed downstream of the south and north pavement drainage, prior to discharging to the existing outlet pit. A Stormfilter® chamber is to be incorporated in the south building OSD tank.

For catchment plans and corresponding treatments, refer to Appendix C.

The layout and results of the MUSIC modelling are as shown in Figure 2-3.



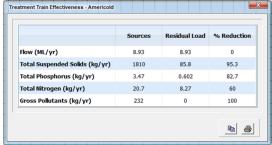


Figure 2-3 MUSIC model layout and results



From the Treatment Train Effectiveness result above, the suspended solids, fine particles and nutrients have met the water quality requirement. As the MUSIC model is not able to evaluate the reduction of hydrocarbons at this stage, Beca has consulted with OceanProtect. It has been recommended to have Mycelx Oil Socks® to be included with each OceanGuard pit insert. Oil baffle is also to be installed within the inlet chamber to retain oils in the OSD. This arrangement has been found to be satisfactory with the Cumberland Council.

#### 2.4 Erosion and Sediment Control

An erosion and sediment control plan has been developed to manage erosion and sediment during construction of the development site. This shall be in accordance with the standards outlined in *Best Practice Erosion & Sediment Control – for building and construction sites, 2008.* The key measures are:

- Sediment fences are to be installed along or adjacent to the downslope boundary of the site. Sediment
  fences shall be constructed from an approved geotextile filter fabric to capture the sediment from
  stormwater runoff. Contractors are required to remove excessive sediment building up behind the
  fence regularly in order to for the fence to stay effective.
- Sandbag sediment traps are provided at the interface of development area and non-development area.
- Sediment basin is to be provided at the downstream limit of the site.
- · All new pits and existing pits are protected by silt traps
- Stockpiles to be protected with appropriate sediment & erosion control measures. Clean run-off from upstream is to be diverted around disturbed areas.

A concept Erosion and Sediment Control is attached in Appendix B.

#### 3 Conclusion

The Stormwater Management Plan for the extension development proposed by Americold Logistics has been completed. The key outcomes are:

- An OSD tank is designed to limit the discharge flow leaving the property to a pre-developed flow for storms up to the 1% AEP event will be provided.
- A new pit and pipe system with sufficient capacity to collect and convey runoff from the development area in 5% AEP storm events will be provided.
- The overland flow paths have the capacity to safely convey the 1% AEP flows that exceed the capacity
  of the minor system around the proposed buildings in such a manner that does not encroach on
  adjacent properties.
- A MUSIC model has been developed to ensure that water quality meets the pollutant reduction requirement set by Cumberland City Council.
- A concept Eorsion and Sediment Control plan has been developed to describe the control measures during construction stages.



| Conclusion |

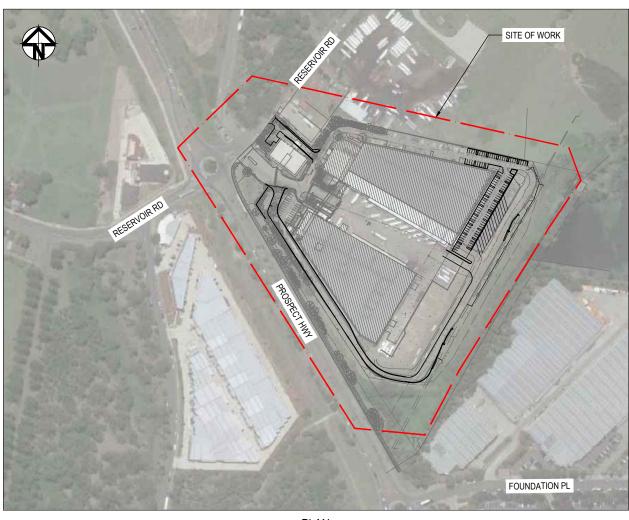






# PROSPECT SOUTH EXPANSION CIVIL WORKS

560 RESERVOIR ROAD PROSPECT NSW 2148



DRAWING LIST					
DRAWING NUMBER	DRAWING NUMBER DRAWING TITLE				
2527456-DWG-CA-0101	COVER SHEET AND DRAWING LIST	В			
2527456-DWG-CA-0102	GENERAL NOTES	В			
2527456-DWG-CA-0111	EROSION AND SEDIMENT CONTROL	В			
2527456-DWG-CA-0301	CUT AND FILL DEPTH PLAN	В			
2527456-DWG-CA-0501	ROAD & PAVEMENT PLAN - SHEET 1	В			
2527456-DWG-CA-0502	ROAD & PAVEMENT PLAN - SHEET 2	В			
2527456-DWG-CA-0503	ROAD & PAVEMENT PLAN - SHEET 3	В			
2527456-DWG-CA-0504	ROAD & PAVEMENT PLAN - SHEET 4	В			
2527456-DWG-CA-0701	DRAINAGE PLAN - SHEET 1	В			
2527456-DWG-CA-0702	DRAINAGE PLAN - SHEET 2	В			
2527456-DWG-CA-0703	DRAINAGE PLAN - SHEET 3	В			
2527456-DWG-CA-0704	DRAINAGE PLAN - SHEET 4	В			
2527456-DWG-CA-0801	CIVIL AND DRAINAGE DETAILS	В			
2527456-DWG-CA-0901	CATCHMENT PLAN	A			
2527456-DWG-CA-0903	OSD DETAILS - SHEET 1	A			
2527456-DWG-CA-0904	OSD DETAILS - SHEET 2	A			
2527456-DWG-CA-1001	VEHICLE TRACKING SHEET 1	С			
2527456-DWG-CA-1002	VEHICLE TRACKING SHEET 2	С			
2527456-DWG-CA-1003	VEHICLE TRACKING SHEET 3	С			
2527456-DWG-CA-1004	VEHICLE TRACKING SHEET 4	С			

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#### **GENERAL NOTES**

- ORIGIN OF LEVELS: AUSTRALIAN HEIGHT DATUM (A.H.D.)
- CONTRACTOR MUST VERIFY ALL DIMENSIONS AND EXISTING LEVELS ON SITE PRIOR TO COMMENCEMENT OF WORK
- ALL WORK IS TO BE UNDERTAKEN IN ACCORDANCE WITH THE DETAILS SHOWN ON THE DRAWINGS, THE SPECIFICATIONS AND THE DIRECTIONS OF THE SUPERINTENDENT
- EXISTING SERVICES HAVE BEEN PLOTTED FROM SUPPLIED DATA AND AS SUCH THEIR ACCURACY CANNOT BE GUARANTEED. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ESTABLISH THE LOCATION AND LEVEL OF ALL EXISTING SERVICES PRIOR TO THE COMMENCEMENT OF ANY WORK, ANY DISCREPANCIES SHALL BE REPORTED TO THE SUPERINTENDENT. CLEARANCES SHALL BE OBTAINED FROM THE RELEVANT SERVICE AUTHORITY.
- WHERE NEW WORKS ABUT EXISTING THE CONTRACTOR SHALL ENSURE THAT A SMOOTH EVEN PROFILE, FREE FROM ABRUPT CHANGES IS OBTAINED.
- THE CONTRACTOR SHALL ARRANGE ALL SURVEY SETOUT TO BE CARRIED OUT BY A REGISTERED SURVEYOR
- CARE IS TO BE TAKEN WHEN EXCAVATING NEAR EXISTING SERVICES. NO MECHANICAL EXCAVATIONS ARE TO BE UNDERTAKEN OVER COMMUNICATIONS, ELECTRICAL AND GAS SERVICES. HAND EXCAVATE IN THESE AREAS.
- ALL SERVICE TRENCHES UNDER VEHICULAR PAVEMENTS SHALL BE BACKFILLED WITH AN APPROVED NON-NATURAL GRANULAR MATERIAL AND COMPACTED TO 98% STANDARD MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS.1289.
- ALL TRENCH BACKFILL MATERIAL SHALL BE COMPACTED TO THE SAME DENSITY AS THE ADJACENT MATERIAL UNLESS NOTES OTHERWISE
- ON COMPLETION OF PIPE INSTALLATION ALL DISTURBED AREAS MUST BE RESTORED TO ORIGINAL, INCLUDING KERBS, FOOTPATHS, CONCRETE AREAS, GRAVEL AND GRASSED AREAS AND ROAD PAVEMENTS.
- 11. PROVIDE 12mm WIDE EXPANDING CORK JOINTS BETWEEN CONCRETE PAVEMENTS AND ALL BUILDINGS, WALLS, FOOTINGS, COLUMNS, KERBS, DISH DRAINS, GRATED DRAINS, BOLLARD FOOTINGS ETC UNLESS NOTED OTHERWISE
- CONTRACTOR TO OBTAIN ALL AUTHORITY APPROVALS.
- 13. ALL BATTERS TO BE GRASSED LINED WITH MINIMUM 100 TOPSOIL AND APPROVED COUCH LAID AS TURF.
- MAKE SMOOTH TRANSITION TO EXISTING SERVICES AND MAKE GOOD.
- THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY DIVERSION DRAINS AND MOUNDS TO ENSURE THAT AT ALL TIMES EXPOSED SURFACES ARE FREE DRAINING AND WHERE NECESSARY EXCAVATE SUMPS AND PROVIDE PUMPING EQUIPMENT TO DRAIN EXPOSED AREAS
- 16. THESE PLANS SHALL BE READ IN CONJUNCTION WITH APPROVED STRUCTURAL DRAWINGS AND SPECIFICATIONS.
- TRENCHES THROUGH EXISTING ROAD AND CONCRETE PAVEMENTS SHALL BE SAWCUT TO FULL DEPTH OF CONCRETE AND A MIN 50mm IN BITUMINOUS PAVING
- 18. ON COMPLETION OF WORKS ALL DISTURBED AREAS MUST BE RESTORED TO ORIGINAL INCLUDING, BUT NOT LIMITED TO, KERBS, FOOTPATHS, CONCRETE AREAS, GRASS AND LANDSCAPED AREAS.

#### UTILITY LOCATION AND CONFLICTS NOTES

- THE INFORMATION SHOWN IS NOT INTENDED TO PROVIDE THE CONTRACTOR WITH COMPLETE AND ACCURATE INFORMATION CONCERNING THE LOCATION AND EXTENT OF EXISTING UTILITY SERVICES. THE CONTRACTOR IS TO MAKE ENQUIRIES TO THE RELEVANT SERVICE AUTHORITIES AS TO THE LOCATION, DEPTH AND EXTENT OF UTILITY SERVICES PRIOR TO THE COMMENCEMENT OF ANY WORK ON
- THE CONTRACTOR SHALL IDENTIFY, LOCATE AND MARK ALL EXISTING SERVICES PRIOR TO COMMENCING CONSTRUCTION AND PROTECT AND MAKE ARRANGEMENTS WITH THE RELEVANT AUTHORITY TO RELOCATE AND/OR ADJUST IF NECESSARY
- WHEN AN EXISTING UNDERGROUND SERVICE NOT PREVIOUSLY IDENTIFIED, IS FOUND DURING CONSTRUCTION, THE CONTRACTOR IS TO IMMEDIATELY ADVISE THE PRINCIPAL AND RELEVANT SERVICE AUTHORITY. ALLOW A HOLD TIME FOR THE SERVICE AUTHORITY TO WITNESS AND DOCUMENT THE UNIDENTIFIED
- THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ANY DAMAGE INCURRED TO THE EXISTING SERVICES AS A RESULT OF THE EXECUTION OF THE WORK UNDER THE CONTRACT

#### **EXISTING SERVICES**

ALL EXISTING SERVICES MADE REDUNDANT FROM THE WORKS AND EXISTING REDUNDANT SERVICES SHALL BE REMOVED OR SEALED OFF IN ACCORDANCE WITH THIS DRAWING, THE CONTRACT DOCUMENTS AND TO THE SATISFACTION OF LOCAL AUTHORITY TO THE UTILITY PROVIDERS DETAILS.

#### **EROSION & SEDIMENT CONTROL**

- ALL EROSION AND SEDIMENT CONTROL MEASURES TO BE CARRIED OUT IN ACCORDANCE WITH THE QUDM AND THE INSTITUTION OF ENGINEERS, AUSTRALIA (QLD) "SOIL FROSION AND SEDIMENT CONTROL - ENGINEERING GUIDELINES FOR QUEENSLAND CONSTRUCTION SITES' AND IPSWICH COUNCIL PLANNING SCHEME **POLICY**
- THE CONTRACTOR SHALL IMPLEMENT ALL SOIL EROSION AND SEDIMENT CONTROL MEASURES RELATING TO A PARTICULAR UPSTREAM CATCHMENTS PRIOR TO STRIPPING OF TOPSOIL FROM THAT CATCHMENTS. WHERE IT IS NECESSARY TO LINDERTAKE STRIPPING IN ORDER TO CONSTRUCT A SEDIMENT CONTROL DEVICE ONLY SUFFICIENT GROUND SHALL BE STRIPPED TO ALLOW CONSTRUCTION
- ALL SOIL EROSION AND SEDIMENT CONTROL MEASURES SHALL BE CONSTRUCTED AND MAINTAINED AS INDICATED IN THE DRAWINGS. THE LOCATION AND EXTENT OF SOIL AND WATER MANAGEMENT DEVICES IS DIAGRAMMATICAL ONLY AND THE ACTUAL REQUIREMENTS SHALL BE CONFIRMED ON SITE PRIOR TO COMMENCEMENT
- CONFORMITY WITH THE EROSION AND SEDIMENT CONTROL PLAN, DRAWING NO. THE GOLD COAST CITY COUNCIL SHALL IN NO WAY REDUCE THE RESPONSIBILITY OF THE CONTRACTOR TO PROTECT AGAINST WATER DAMAGE DURING THE COURSE OF THE CONTRACT. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT ANY NECESSARY CONTROL IS IN PLACE EVEN THOUGH SUCH CONTROL MAY NOT BE SHOWN ON THE PLAN.
- THE CONTRACTOR SHALL INFORM ALL SUBCONTRACTORS AND ALL EMPLOYEES OF THEIR RESPONSIBILITIES IN MINIMISING THE POTENTIAL FOR SOIL EROSION AND POLLUTION TO DOWNSTREAM AREAS.
- THE CONTRACTOR SHALL REGULARLY MAINTAIN SEDIMENT AND EROSION CONTROL STRUCTURES AND DE-SILT SUCH STRUCTURES PRIOR TO THE REDUCTION IN CAPACITY OF 30% DUE TO ACCUMULATED SILT. THE SEDIMENT SHALL BE DISPOSED OF ON SITE IN A MANNER APPROVED BY THE PRINCIPAL
- TOPSOIL AND SPOIL SHALL BE STOCKPILED IN NON-HAZARDOUS AREAS AND PROTECTED FROM SURFACE RUNOFF BY DIVERSION DRAINS OR SIMILAR STOCKPILES SHALL BE SURROUNDED ON DOWNSTREAM SIDES BY SILT FENCING.STOCKPILES SHALL BE SUITABLY COMPACTED TO INHIBIT EROSION WHERE THE STOCKPILING PERIOD EXCEEDS FOUR (4) WEEKS, THE STOCKPILE SHALL BE SEEDED TO ENCOURAGE VEGETATION GROWTH.
- TOPSOIL SHALL BE RESPREAD AND STABILISED AS SOON AS POSSIBLE. DISTURBED AREAS SHALL BE LEFT WITH A SCARIFIED SURFACE TO ENCOURAGE WATER INFILTRATION AND ASSIST KEYING IN TOPSOIL.
- THE CONTRACTOR SHALL TEMPORARILY REHABILITATE WITHIN 40 DAYS ANY DISTURBED AREAS. WHERE FINAL SHAPING HAS OCCURRED THE CONTRACTOR SHALL PROVIDE FINAL REHABILITATION WITHIN 20 DAYS.
- THE CONTRACTOR SHALL MAINTAIN GRASS COVER UNTIL ALL WORKS HAVE BEEN COMPLETED, INCLUDING THE MAINTENANCE PERIOD, BY FREQUENT WATERING AND MOWING WHERE REQUIRED. PLANT, MACHINERY AND VEHICLES SHALL NOT BE DRIVEN OVER GRASSED AREAS UNLESS ON AN APPROVED HAULAGE ROUTE.
- THE CONTRACTOR SHALL PROVIDE INLET SEDIMENT TRAPS AT ALL PITS DURING CONSTRUCTION AND REPLACE WITH KERB INLET CONTROL AT COMPLETION OF LINTEL CONSTRUCTION
- ALL DRAINAGE WORKS SHALL BE CONSTRUCTED AND STABILISED AS QUICKLY AS POSSIBLE TO MINIMISE RISK OF EROSION.
- VEHICULAR TRAFFIC SHALL BE CONTROLLED DURING CONSTRUCTION CONFINING ACCESS WHERE POSSIBLE TO PROPOSED OR EXISTING ROAD ALIGNMENTS PLUS 3 METRES WHERE NECESSARY, AREAS TO BE LEFT UNDISTURBED SHALL BE MARKED.
- 14. SITE ACCESS SHALL BE RESTRICTED TO A NOMINATED POINT, CONTRACTOR TO INSTALL VIBRATION GRID AT EXIT POINTS TO LIMIT DISPLACEMENT OF ON-SITE SILTY

#### TREE REMOVAL

- 1. ALL TREES AND SHRUBS (UNLESS NOTED TO BE PROTECTED ON THE LANDSCAPE PLANS), RUBBLE, EXISTING PAVEMENT AND EXISTING STRUCTURES WITHIN THE SITE SHALL BE REMOVED AND REUSED OR RECYCLED WHERE POSSIBLE WHERE NOT POSSIBLE, THIS MATERIAL SHALL BE REMOVED FROM SITE AND DISPOSED OF AS
- ANY TREES WITHIN THE WORKS AREA WHICH, IN THE OPINION OF THE CONTRACT ADMINISTRATOR, ARE UNSOUND OR WOULD CONSTITUTE A DANGER, SHALL BE CUT DOWN AND REMOVED (EXCEPT THOSE IDENTIFIED AS BEING PROTECTED). ALL STUMPS OF TREES CUT DOWN WITHIN THE BOUNDS OF THE CONSTRUCTION AREA WHICH ARE LARGER THAN 250mm IN GIRTH, SHALL BE COMPLETELY REMOVED.
- ALL ROOTS SHALL BE REMOVED FOR A DEPTH OF 1m. CAVITIES FORMED BY THE REMOVAL OF ROOTS SHALL BE BACKFILLED AND COMPACTED.
- AFTER CLEARING AND GRUBBING ARE COMPLETE, THE CONTRACTOR SHALL STRIP AND STOCKPILE TOPSOIL FROM THE CLEARED AREA (INCLUDING AREAS THAT HAVE BEEN CLEARED AND GRUBBED). REMOVAL OF TOPSOIL FROM ANY SECTION OF THE WORKS SHALL ONLY COMMENCE AFTER SEDIMENT AND EROSION CONTROLS HAVE BEEN IMPLEMENTED

#### STORMWATER

- ALL 225 DIA. DRAINAGE PIPES AND LARGER SHALL BE CLASS "2" APPROVED SPIGOT AND SOCKET FRC PIPES WITH RUBBER RING JOINTS. (U.N.O.).
- EQUIVALENT STRENGTH REINFORCED CONCRETE PIPES MAY BE USED
- ALL PIPE JUNCTIONS UP TO AND INCLUDING 450 DIA. AND TAPERS SHALL BE VIA PURPOSE MADE FITTINGS.
- MINIMUM GRADE TO STORMWATER LINES TO BE 0.5%. (U.N.O.)
- CONTRACTOR TO SUPPLY AND INSTALL ALL FITTINGS AND SPECIALS INCLUDING VARIOUS PIPE ADAPTORS TO ENSURE PROPER CONNECTION BETWEEN DISSIMILAR **PIPFWORK**
- ALL CONNECTIONS TO EXISTING DRAINAGE PITS SHALL BE MADE IN A TRADESMAN-LIKE MANNER AND THE INTERNAL WALL OF THE PIT AT THE POINT OF ENTRY SHALL BE CEMENT RENDERED TO ENSURE A SMOOTH FINISH.
- PRECAST PITS SHALL NOT BE USED UNLESS WRITTEN APPROVAL IS OBTAINED FROM THE SITE PRINCIPAL
- WHERE TRENCHES ARE IN ROCK, THE PIPE SHALL BE BEDDED ON A MIN. 50mm CONCRETE BED (OR 75mm THICK BED OF 12mm BLUE METAL) UNDER THE BARREL OF THE PIPE. THE PIPE COLLAR AT NO POINT SHALL BEAR ON THE ROCK, IN OTHER THAN ROCK, PIPES SHALL BE LAID ON A 75mm THICK SAND BED. IN ALL CASES BACKEILL THE TRENCH WITH SAND TO 200mm ABOVE THE PIPE. WHERE THE PIPE IS LINDER PAVEMENTS BACKELL REMAINDER OF TRENCH WITH SAND OR APPROVED GRANULAR BACKFILL COMPACTED IN 150mm LAYERS TO 98% STANDARD MAX. DRY DENSITY.
- BEDDING SHALL BE (U.N.O.) TYPE H1, IN ACCORDANCE WITH CURRENT RELEVANT AUSTRALIAN STANDARDS
- WHERE SUBSOIL DRAINAGE LINES PASS UNDER VEHICULAR PAVEMENTS UNSLOTTED UPVC SEWER GRADE PIPE SHALL BE USED.
- 11. PROVIDE 3.0m LENGTH OF 100 DIA. SUBSOIL DRAINAGE PIPE WRAPPED IN FABRIC SOCK, AT UPSTREAM END OF EACH PIT.

#### LINE MARKING

- THE WORK SHALL INCLUDE ALL LINE MARKING TO ROADS, HARDSTANDS, PATHS CAR PARKS AND THE TRAFFICABLE AREAS.
- THE PAVEMENT MARKING AND PAINT SHALL BE IN ACCORDANCE WITH AS 1742.2 AND THE RMS DELINEATION MANUAL
- LINE MARKING SHALL BE SPOTTED OUT AND APPROVED PRIOR TO SPRAYING
- PAINT SHALL BE APPLIED AT A WET THICKNESS OF BETWEEN 0.35mm TO 0.40mm.
- ALL EXISTING PAVEMENT MARKING WHICH IS LOCATED ON EXISTING PAVEMENT TO BE RETAINED SHALL BE REMOVED BY GRINDING WHERE THE EXISTING MARKINGS ARE MADE REDUNDANT BY THE PROPOSED WORKS

#### PAVEMENTS AND HARDSTAND

THE SURFACE OF ANY COMPACTED LAYER SHALL BE KEPT MOIST, IN GOOD CONDITION AND FREE FROM CONTAMINATION UNTIL ANY SUBSEQUENT PAVEMENT WORK UNDER THE CONTRACT IS COMMENCED.

#### CONCRETE NOTES

#### **GENERAL**

- ALL WORKMANSHIP AND MATERIALS SHALL COMPLY WITH AS 3600 CURRENT EDITIONS WITH AMENDMENTS, AND THE ACSE CONCRETE SPECIFICATION EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.
- VERIFY ALL SETTING OUT DIMENSIONS WITH THE ARCHITECT AND/OR THE SURVEYOR
- DO NOT OBTAIN DIMENSIONS BY SCALING THE DRAWINGS.
- 4. IN CASE OF DOUBT ASK.

#### DESIGN LOADS

N/A

PLACE CONCRETE OF THE FOLLOWING CHARACTERISTIC COMPRESSIVE STRENGTH F"C AS DEFINED IN AS.3600 OR M.R. FORM 609. ADD WATER REDUCING ADMIXTURE EQUAL TO WRDA

LOCATION	AS.3600 F'c MPa AT 28 DAYS	SPECIFIED SLUMP	NOMINAL AGG. SIZE
ALL KERB PITS ETC.	25	80	20
VEHICULAR PAVEMENTS	32	80	20

- USE "A.C.S.E. SPECIFICATION TYPE A" CEMENT.
- ALL CONCRETE SHALL BE SUBJECT TO PROJECT CONTROL SAMPLE AND TESTING TO AS.3600.
- CONSOLIDATE BY VIBRATION.

#### REINFORCEMENT

- FIX REINFORCEMENT AS SHOWN ON DRAWINGS. THE TYPE AND GRADE IS INDICATED BY A SYMBOL AS SHOWN BELOW. ON THE DRAWING N IS FOLLOWED BY A NUMERAL WHICH INDICATES THE SIZE IN MILLIMETRES. A MARK NUMERAL (IF USED) FOLLOWS THIS NUMERAL
- N. HOT ROLLED DEFORMED BAR, GRADE 410Y S. HOT ROLLED DEFORMED BAR, GRADE 230S
- R. PLAIN ROUND BAR, GRADE 230R RL.
- SL.HARD DRAWN WIRE FABRIC.

#### **CONCRETE NOTES - CONTINUED**

- 2. PROVIDE BAR SUPPORTS OR SPACERS TO GIVE THE FOLLOWING CONCRETE COVER TO ALL REINFORCEMENT UNLESS NOTED OTHERWISE.
- 75 BOTTOM, 65 TOP AND SIDES SLABS

20 TOP AND BOTTOM, 30 WHEN EXPOSED TO WEATHER.

#### **BEAMS**

50 BOTTOM AND SIDES (TO STIRRIPS) TOP COVER AS DETAILED COLUMNS 40 TO TIES AND SPIRALS 50 WHEN EXPOSED TO WEATHER

#### WALLS

- 25 GENERALLY 30 WHEN CAST IN FORMS BUT LATER EXPOSED TO WEATHER OR GROUND
- 65 WHEN CAST DIRECTLY IN CONTACT WITH GROUND

CURE ALL CONCRETE IN ACCORDANCE TO THE METHOD PROVIDED IN THE SPECIFICATION.

#### ASPHALTIC CONCRETE NOTES

#### 1. GENERAL

- MINERAL AGGREGATES TO COMPLY WITH AUSTRALIAN STANDARDS
- MINERAL FILLER TO COMPLY WITH AS 2357 MINERAL FILLERS OR ASPHALT.
- BITUMEN BINDER SHALL COMPLY WITH AS 2008

#### 2. MIX PROPORTIONS

- a) JOB MIX 14mm NOMINAL SIZE AGGREGATE. MINIMUM BITUMEN CONTENT BY MASS OF TOTAL MASS - 5.1%
- MIX STABILITY BETWEEN 16kN AND 36kN AS DETERMINED BY AS 2891 AIR VOIDS IN COMPACTED MIX - BETWEEN 4% AND 7% OF THE VOLUME OF THE MIX.
- VOIDS FILLED IN BINDER 65-80% OF AIR VOIDS IN THE TOTAL MINERAL AGGREGATE FILLED BY BINDER IN ACCORDANCE WITH AUSTRALIAN STANDARDS

#### 3. PAVEMENT PREPARATION

- a) THE EXISTING SURFACE TO BE SEALED SHALL BE DRY AND BROOMED BEFORE COMMENCEMENT OF WORK TO ENSURE COMPLETE REMOVAL OF ALL SUPERFICIAL
- ALL DEPRESSIONS OR UNEVEN AREAS ARE TO BE TACK-COATED AND BROUGHT UP TO GENERAL LEVEL OF PAVEMENT WITH ASPHALTIC CONCRETE BEFORE LAYING OF MAIN COURSE.

#### 4. TACK COAT

THE WHOLE OF THE AREA TO BE SHEETED WITH ASPHALTIC CONCRETE SHALL BE LIGHTLY AND EVENLY COATED WITH RAPID SETTING BITUMEN COMPLYING WITH AUSTRALIAN STANDARDS, APPLICATION RATE FOR RESIDUAL BITUMEN SHALL BE 0.15 TO 0.30 LITRES/SQUARE METRE. APPLICATION SHALL BE BY MEANS OF A MECHANICAL SPRAYER WITH SPRAY BAR.

#### SPREADING

- a) ALL ASPHALTIC CONCRETE SHALL BE SPREAD WITH A SELF PROPELLED PAVING MACHINE
- THE ASPHALTIC CONCRETE SHALL BE LAID AT A MIX TEMPERATURE AS SHOWN BELOW

ROAD SURFACE	MIX TEMPERATURES
TEMPERATURE IN	(°C)
SHADE (°C)	
5 - 10	NOT PERMITTED
10 - 15	150
15 - 25	145
OVED 25	1/10

- c) ASPHALTIC CONCRETE SHALL NOT BE LAID WHEN THE ROAD SURFACE IS WET OR WHEN COLD WINDS CHILL THE MIX ADVERSELY AFFECT SPREADING AND COMPACTION
- THE MINIMUM COMPACTED THICKNESS IS 30mm OVER EXISTING SEALED PAVEMENTS AND 50mm OVER NEW PAVEMENTS

#### 6. JOINTS

- a) THE NUMBER OF JOINTS BOTH LONGITUDINAL AND TRANSVERSE SHALL BE KEPT TO A MINIMUM.
- THE DENSITY AND SURFACE FINISH AT JOINTS SHALL BE SIMILAR TO THOSE OF THE REMAINDER OF THE LAYER.

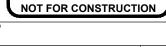
#### 7. COMPACTION

- a) ALL COMPACTION SHALL BE UNDERTAKEN USING SELF PROPELLED ROLLERS.
- b) INITIAL ROLLING SHALL BE COMPLETE BEFORE THE MIX TEMPERATURE FALLS BELOW 105°C c) SECONDARY ROLLING SHALL BE COMPLETED BEFORE THE MIX TEMPERATURE
- FALLS BELOW 60°C d) MINIMUM CHARACTERISTICS VALUE OF RELATIVE COMPACTION OF A LOT WHEN
- TESTED IN ACCORDANCE WITH AS2150

#### FINISHED PAVEMENT PROPERTIES

FINISHED SURFACES SHALL BE SMOOTH, DENSE AND TRUE TO SHAPE AND SHALL NOT VARY MORE THAN 10mm FROM THE SPECIFIED PLAN LEVEL AT ANY POINT AND SHALL NOT DEVIATE FROM THE BOTTOM OF A 3m STRAIGHT EDGE LAID IN ANY DIRECTION BY MORE THAN 5mm





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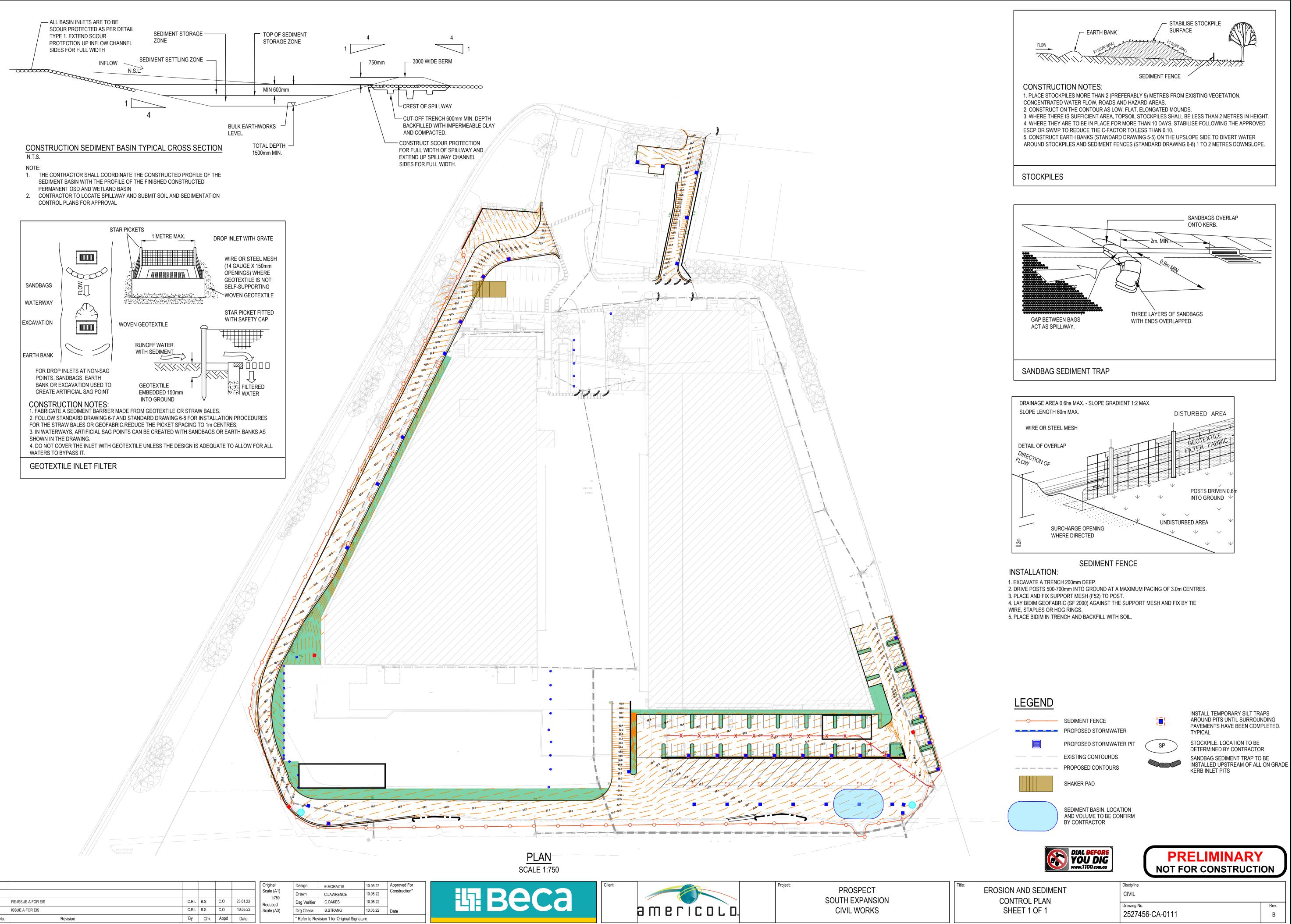


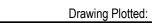
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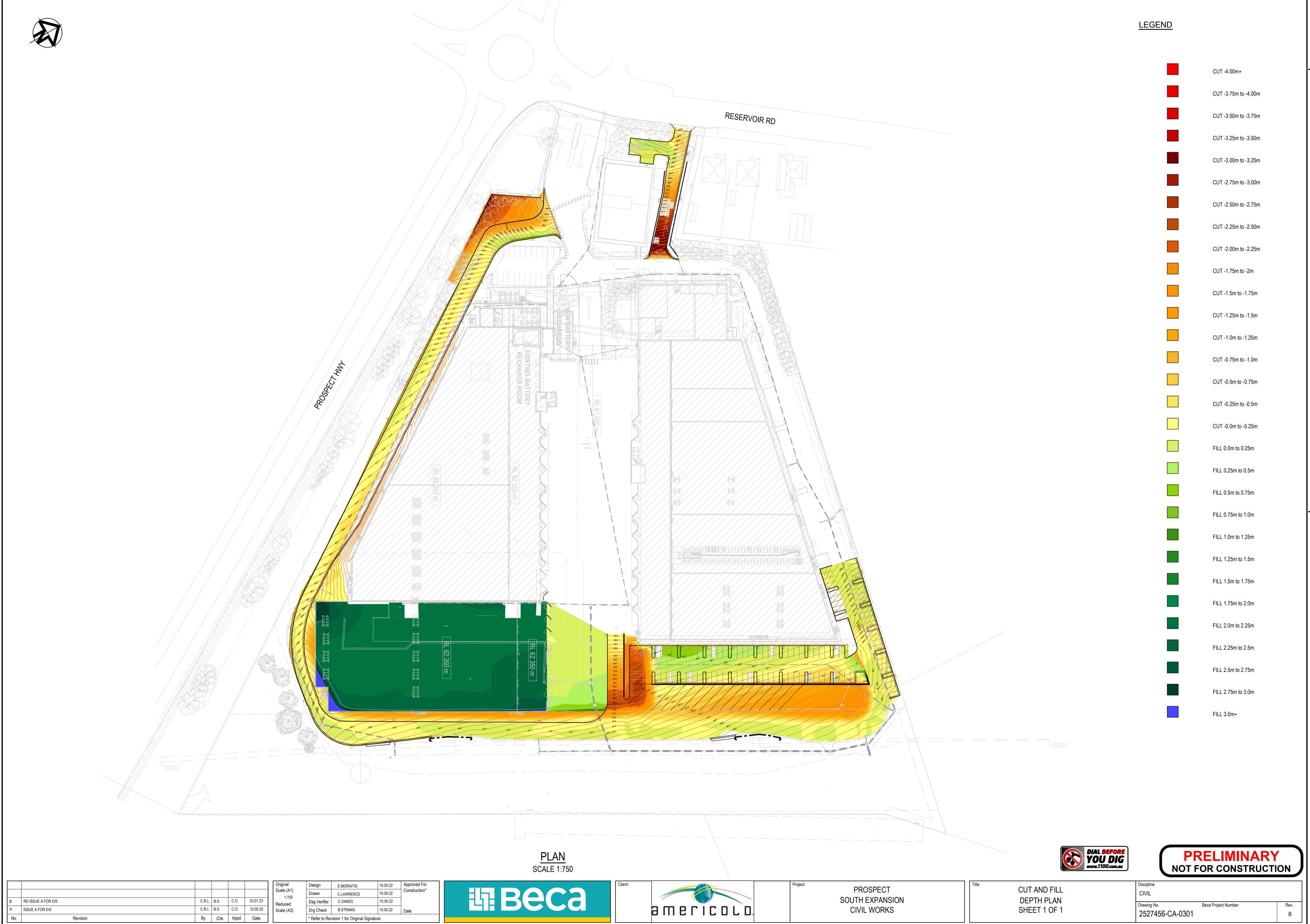
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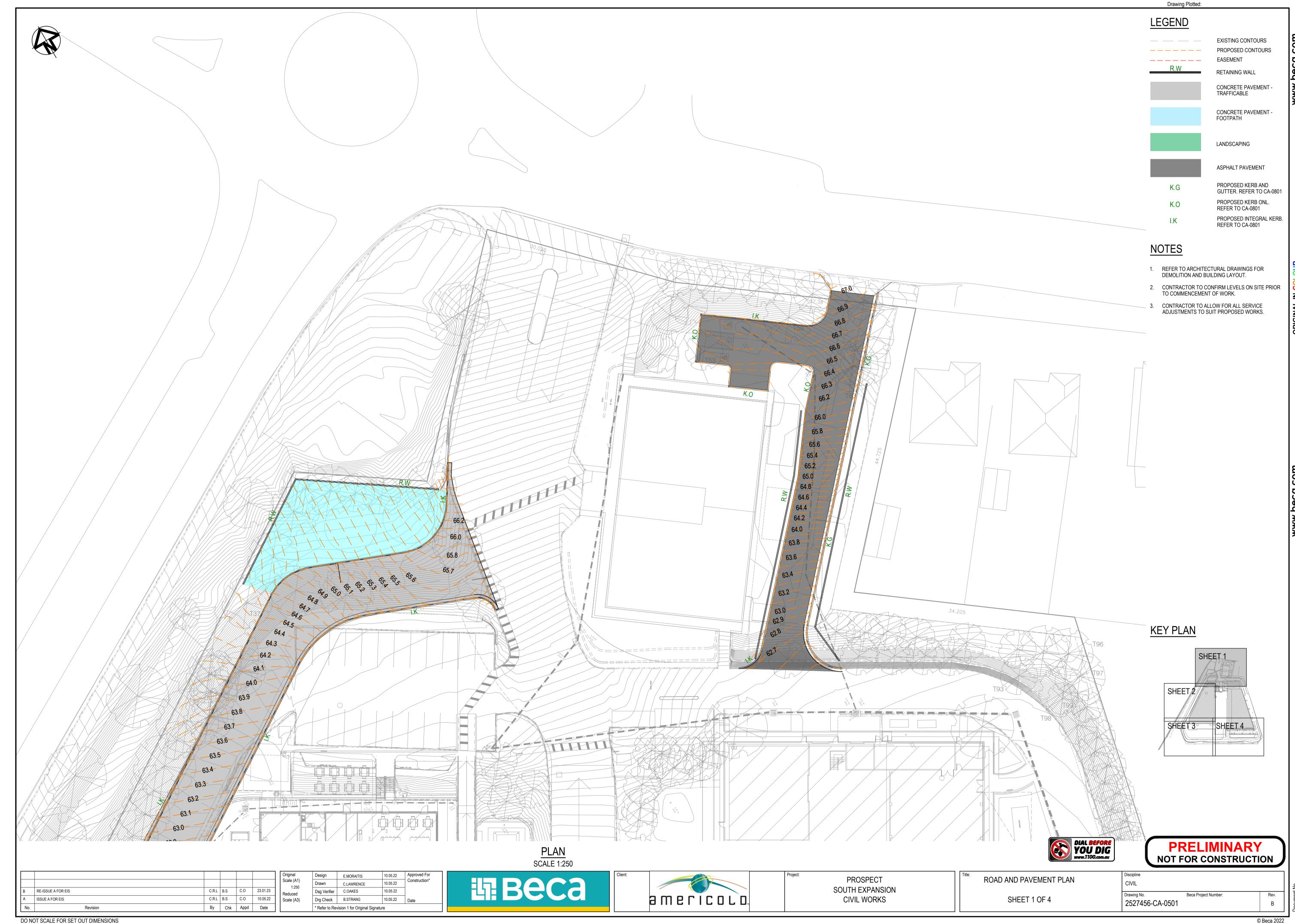
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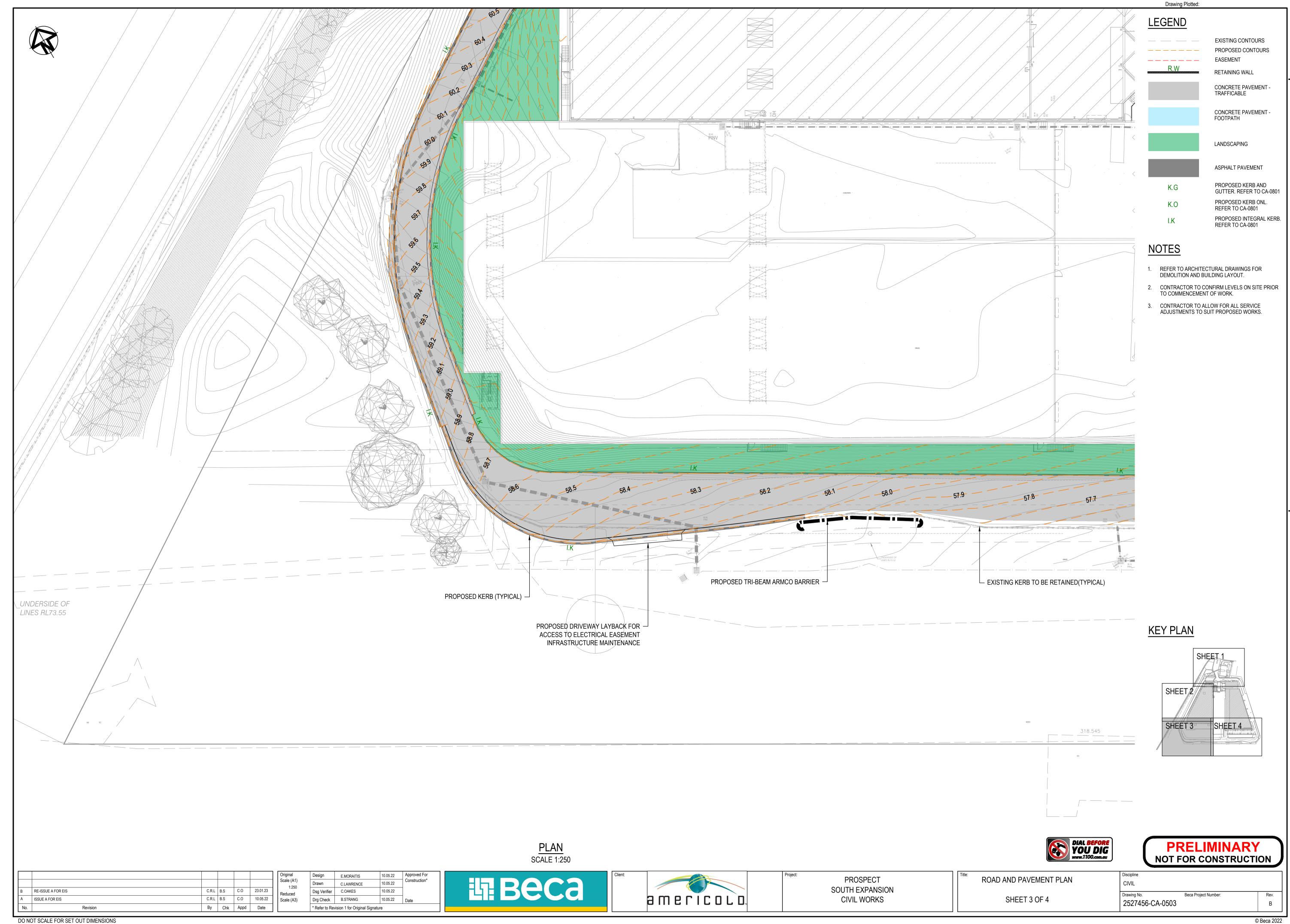


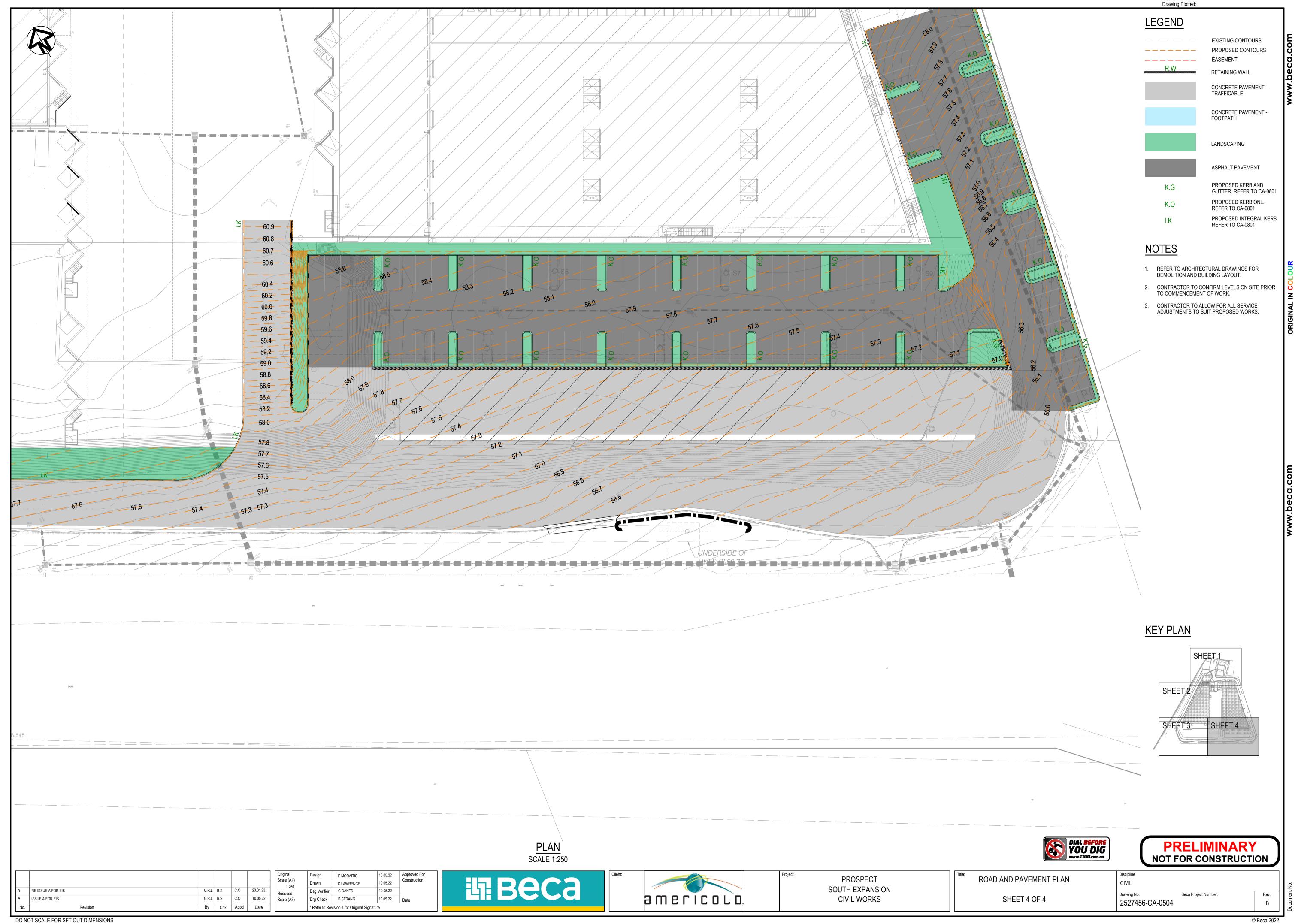


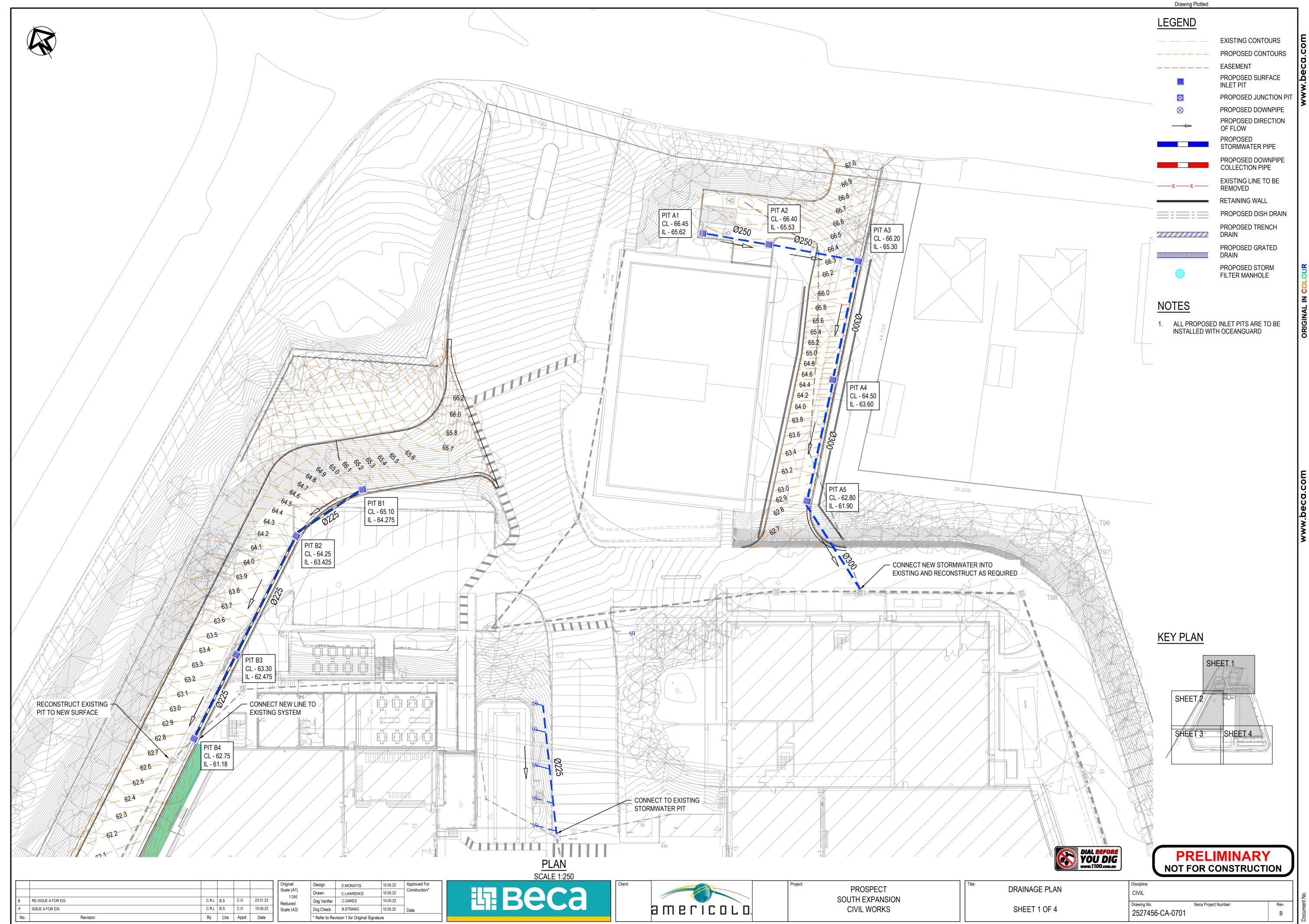


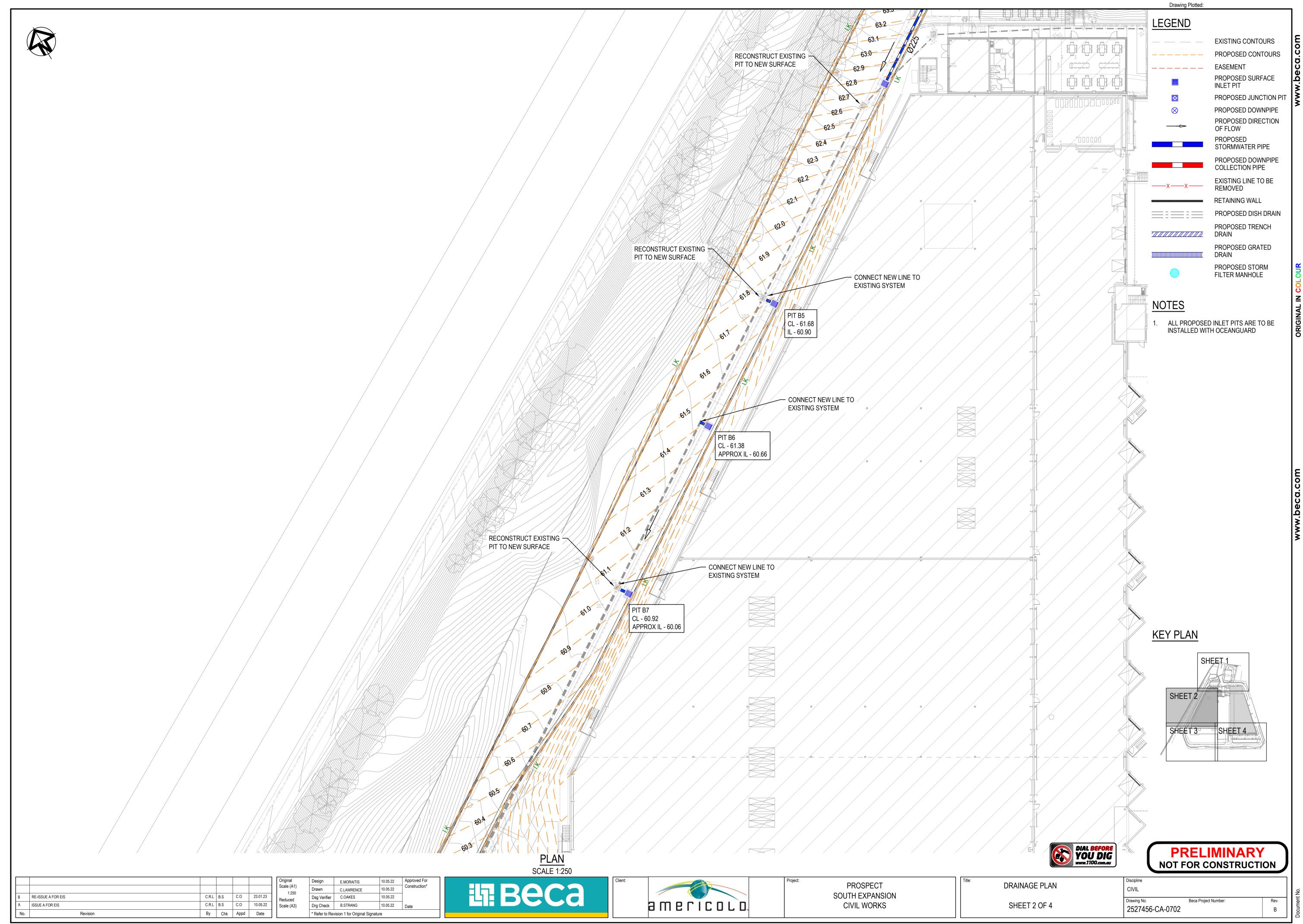


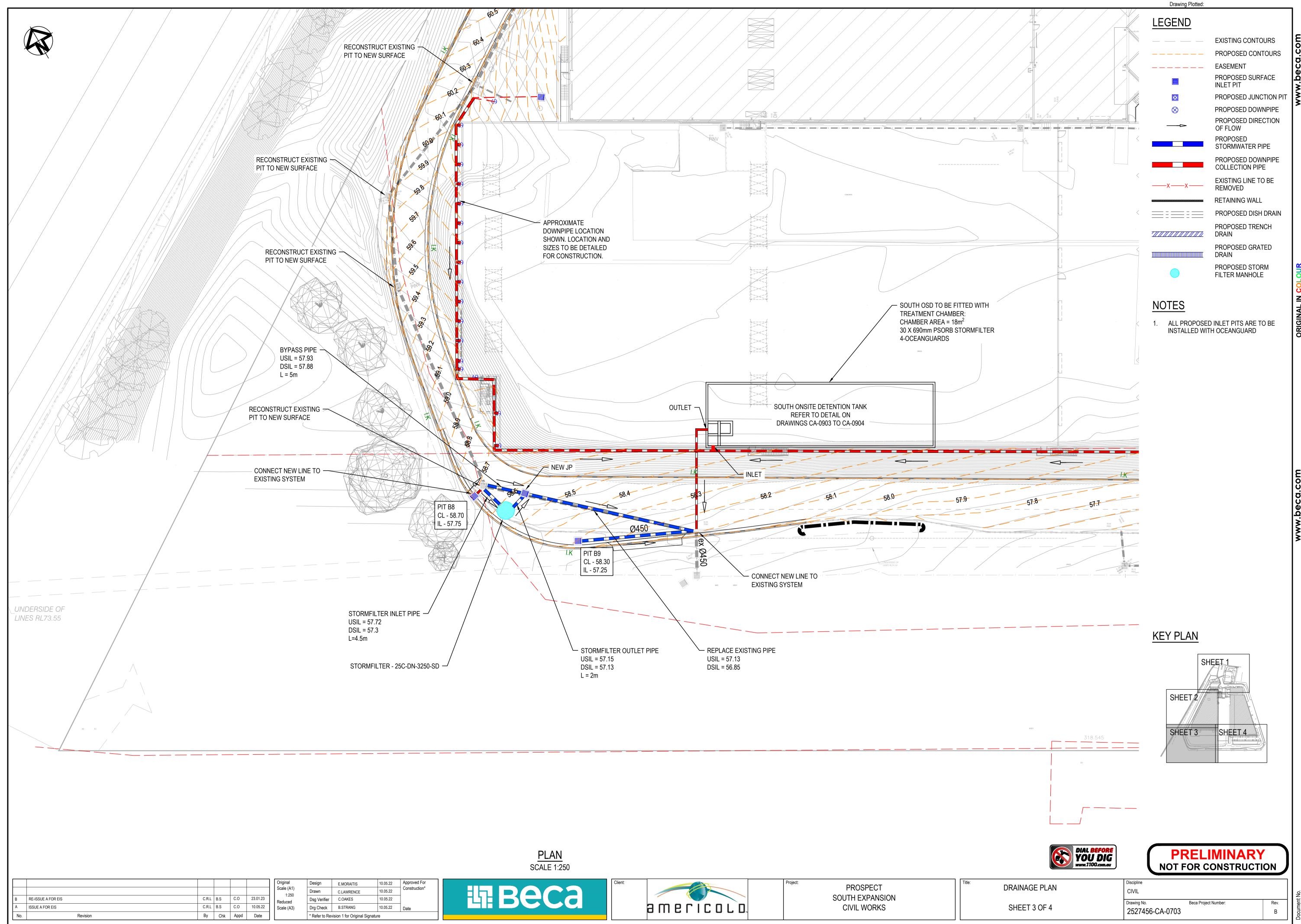


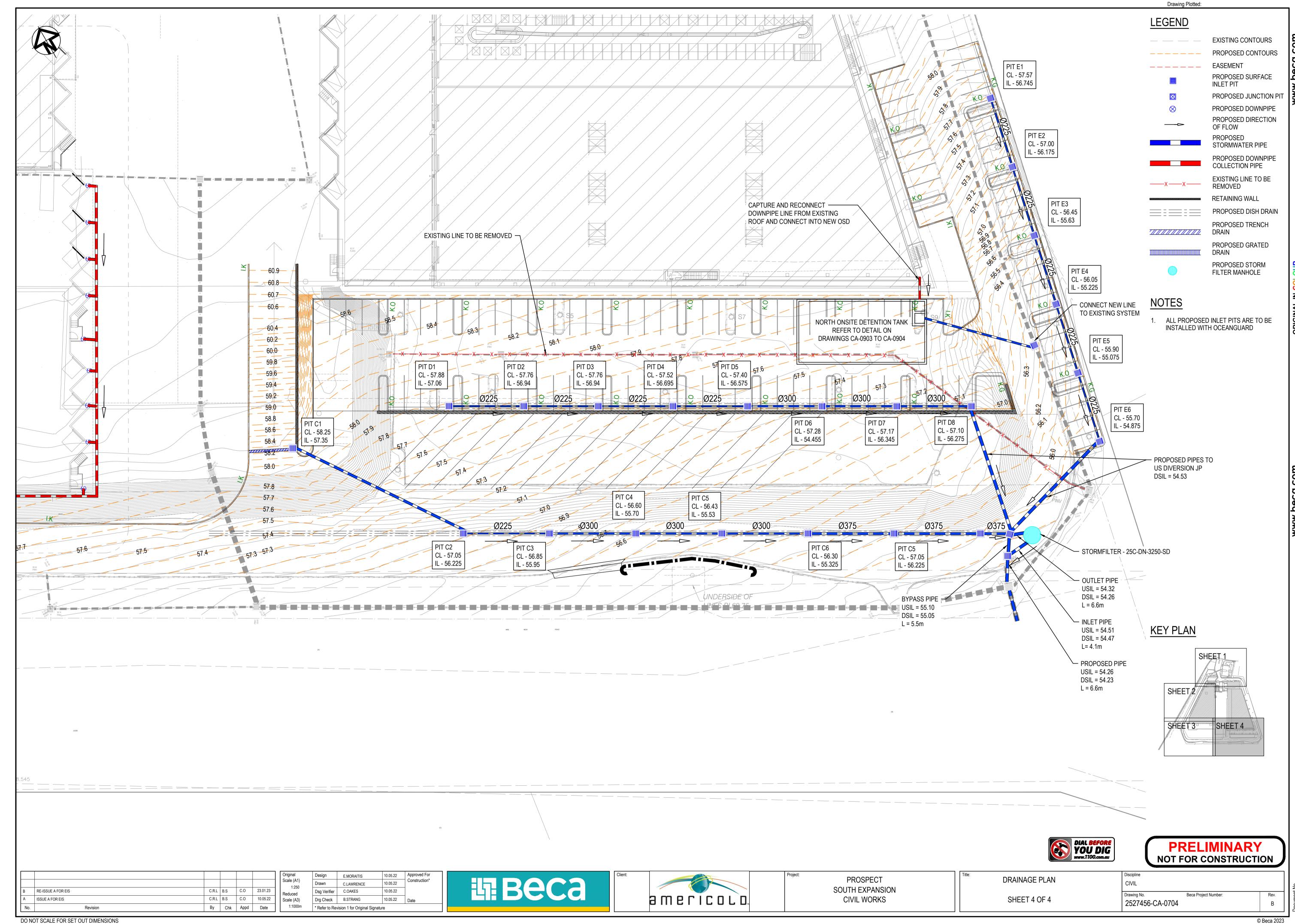


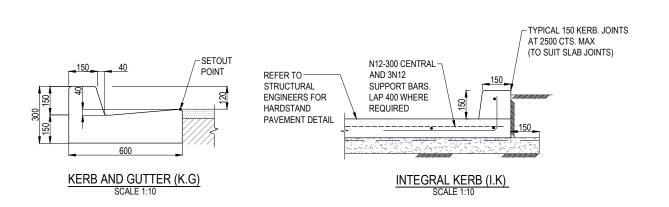


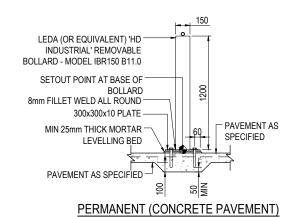






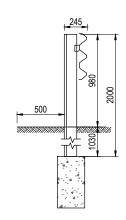






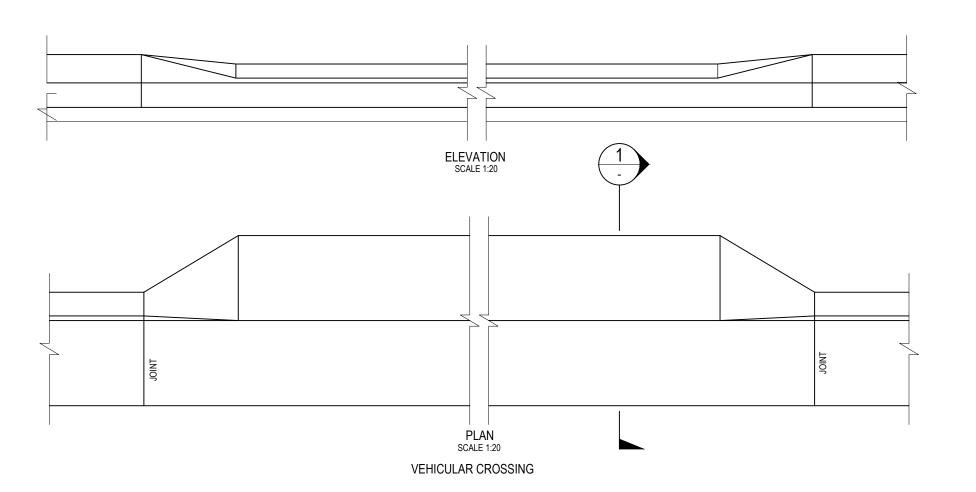
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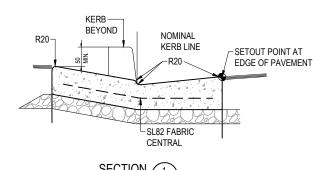
NOTE:
ALL ELEMENTS TO BE GALVANISED.
CONTRACTOR TO ENSURE BOLLARDS DO NOT CONFLICT WITH PROPOSED OR EXISTING SERVICES.
BOLLARD TO BE STRICTLY INSTALLED TO MANUFACTURERS DETAILS AND SPECIFICATIONS



THRIE BEAM BARRIERS

1:20 REFER TO MANUFACTURERS DETAILS





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ı	В	RE-ISSUE A FOR EIS	C.R.L	B.S	C.O	23.01.23	П
ı	Α	ISSUE A FOR EIS	C.R.L	B.S	C.O	10.05.22	П
ı	No.	Revision	Ву	Chk	Appd	Date	ΙL

Original	Design	E.MORAITIS	10.05.22	Approved For	. [
Scale (A1)	Drawn	C.LAWRENCE	10.05.22	Construction*	Н
Reduced	Dsg Verifier	C.OAKES	10.05.22		1
Scale (A3)	Drg Check	B.STRANG	10.05.22	Date	П
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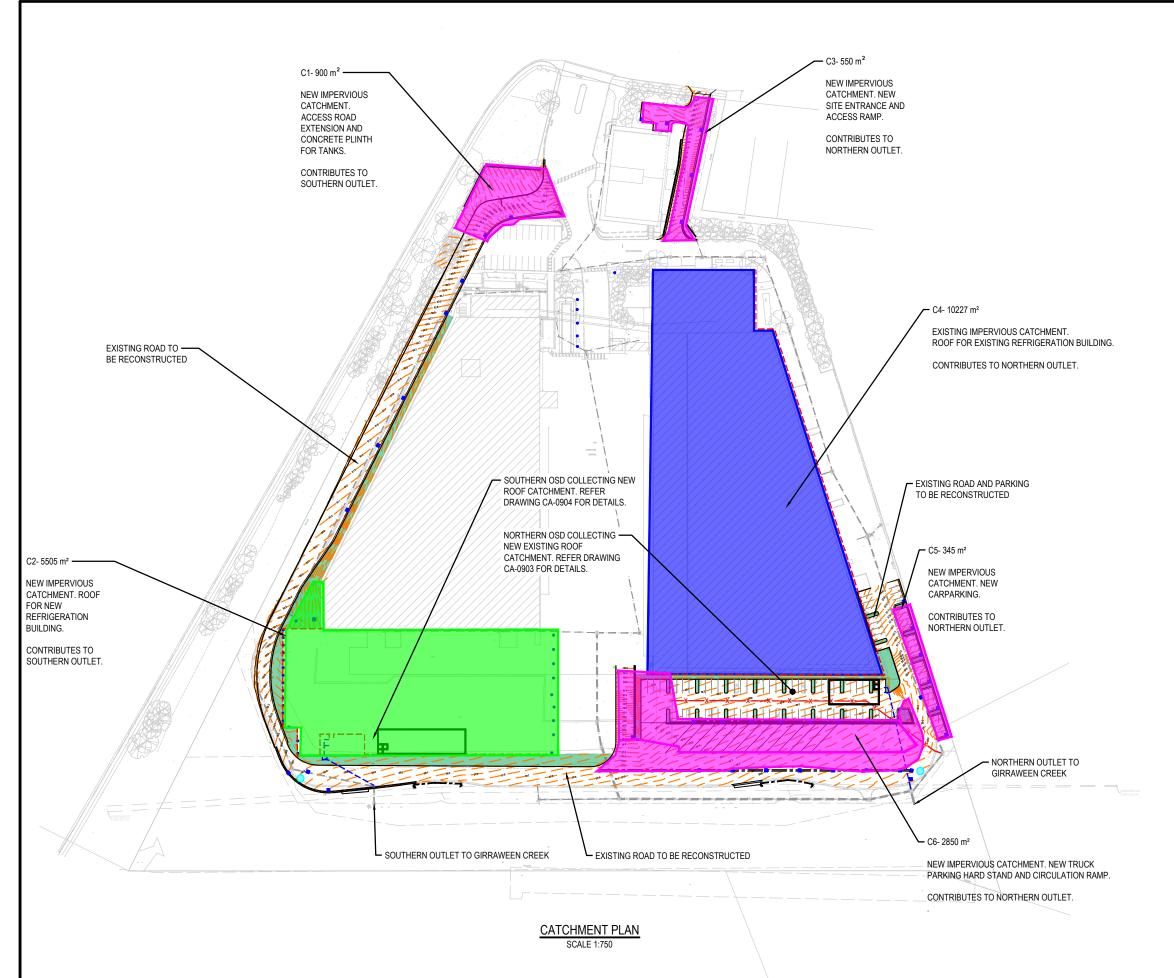




SOUTH EXPANSION CIVIL WORKS

CIVIL AND DRAINAGE DETAILS SHEET 1 OF 1

Drawing No. 2527456-CA-0801



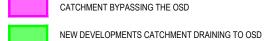
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**LEGEND** OUTLET PIPES

INFLOW PIPES — — EXISTING CONTOURDS

---- PROPOSED CONTOURS

EXISTING ROOF TO BE RE-DIVERTED TO NEW OSD (NORTH)



#### OSD SIZING CALCULATIONS

TOTAL NEW CATCHMENT = C1 + C2 + C3 + C5 + C6 = 10150 m<sup>2</sup>

TOTAL UPRCT PSD = 1.0150 ha x 80 l/s=81.2 l/s = 0.0812 m<sup>3</sup>/s

DUE TO SITE CONSTRAINTS WE WILL REDIRECT EXISTING ROOF CATCHMENT C4 TO A NEW OSD TO OFFSET PART OF THE NEW DEVELOPED AREAS THAT CANNOT DRAIN TO AN OSD EFFECTIVELY.

EXISTING C4 Q100 = 0.604 m³/s (DEVELOPED FROM DRAINS)

Q100 RUN-OFF FROM CATCHMENTS BYPASSING OSD = C1 + C3 + C5 + C6 = 0.289 m³/s (DEVELOPED FROM DRAINS)

ALLOWABLE DISCHARGE = EXISTING C4 Q100 - Q100 RUN-OFF FROM CATCHMENTS BYPASSING OSD + TOTAL UPRCT PSD

0.604 m³/s - 0.289 m³/s + 0.0812 m³/s

DISCHARGE FROM SOUTHERN AND NORTHERN OSDS = 0.329 m³/s + 0.018 m³/s

IN ADDITION THERE IS DISCHARGE FROM THE FILTER CARTRIDGE SYSTEM LOCATED WITHIN THE SOUTHERN OSD. DISCHARGE FLOW IS CALCULATED USING A FORMULA IN EXCEL (REFER STORMWATER MANAGEMENT REPORT)

Therefore TOTAL SITE DISCHARGE = 0.0302+0.347 = 0.3772m3/S

THEREFORE OVERALL DISCHARGE FROM THE SITE MEETS THE PSD REQUIREMENTS FOR NEW DEVELOPED AREAS.

REFER TO DESIGN REPORT FOR FURTHER DETAILS AND BREAKDOWN.





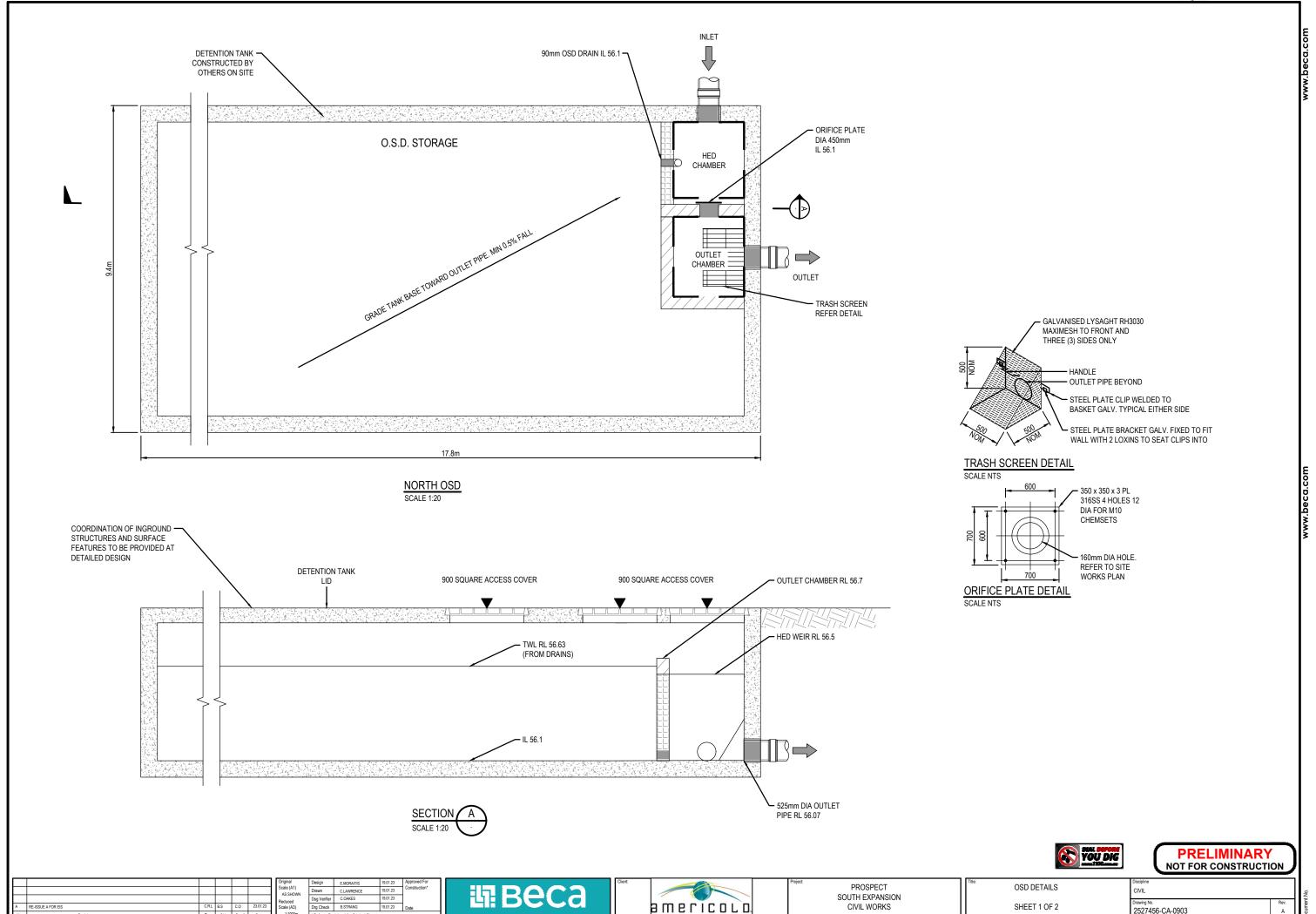
CATCHMENT PLAN SHEET 1 OF 1

SOUTH EXPANSION

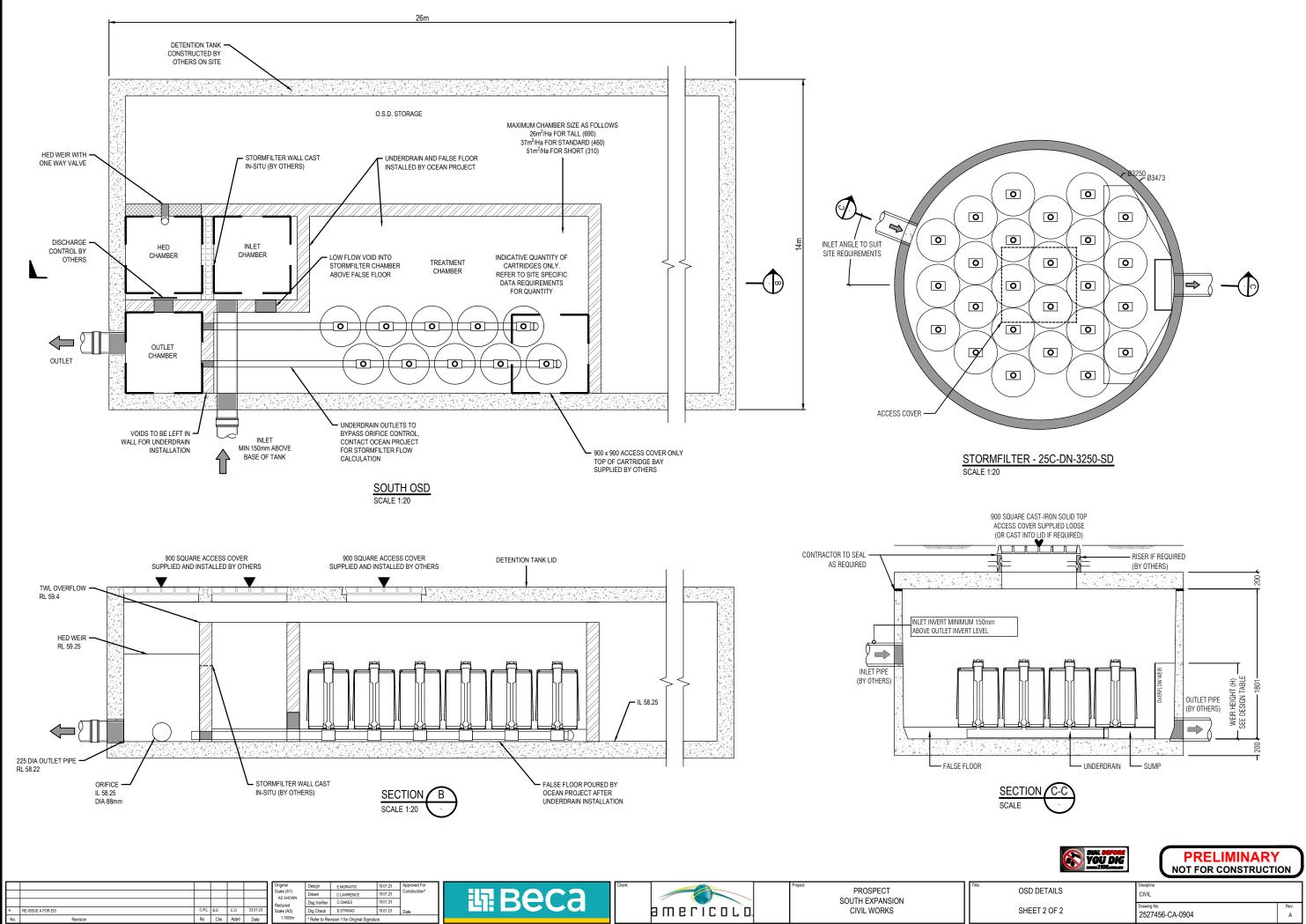
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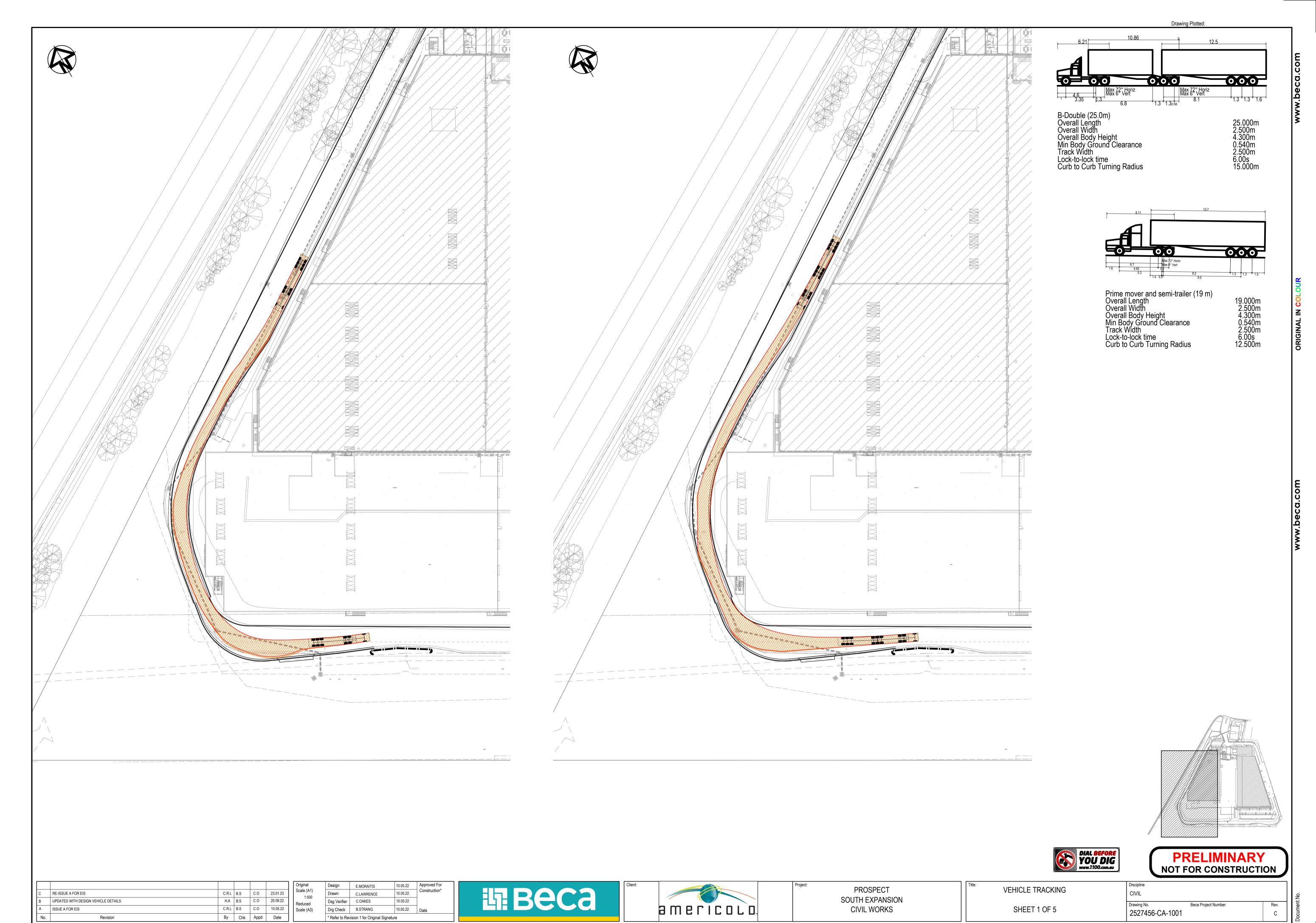
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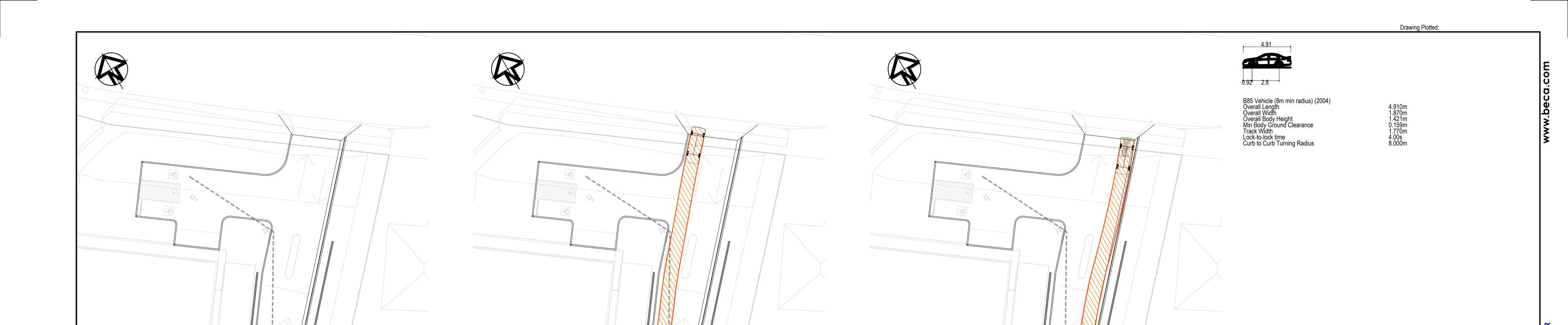


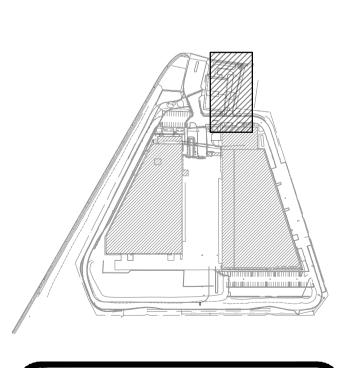






Beca 2023









Nο	Revision	Rv	Chk	Annd	Date		* Pefer to Per	vision 1 for Original Signatu	rα	
Α	ISSUE A FOR EIS	C.R.L	B.S	C.O	10.05.22	Scale (A3)	Drg Check	B.STRANG	10.05.22	Date
В	UPDATED WITH DESIGN VEHICLE DETAILS	H.A	B.S	C.O	20.09.22	Reduced	Dsg Verifier	C.OAKES	10.05.22	
С	RE-ISSUE A FOR EIS	C.R.L	B.S	C.O	23.01.23	Scale (A1) 1:250	Drawn	C.LAWRENCE	10.05.22	Construction*
						Original	Design	E.MORAITIS	10.05.22	Approved For

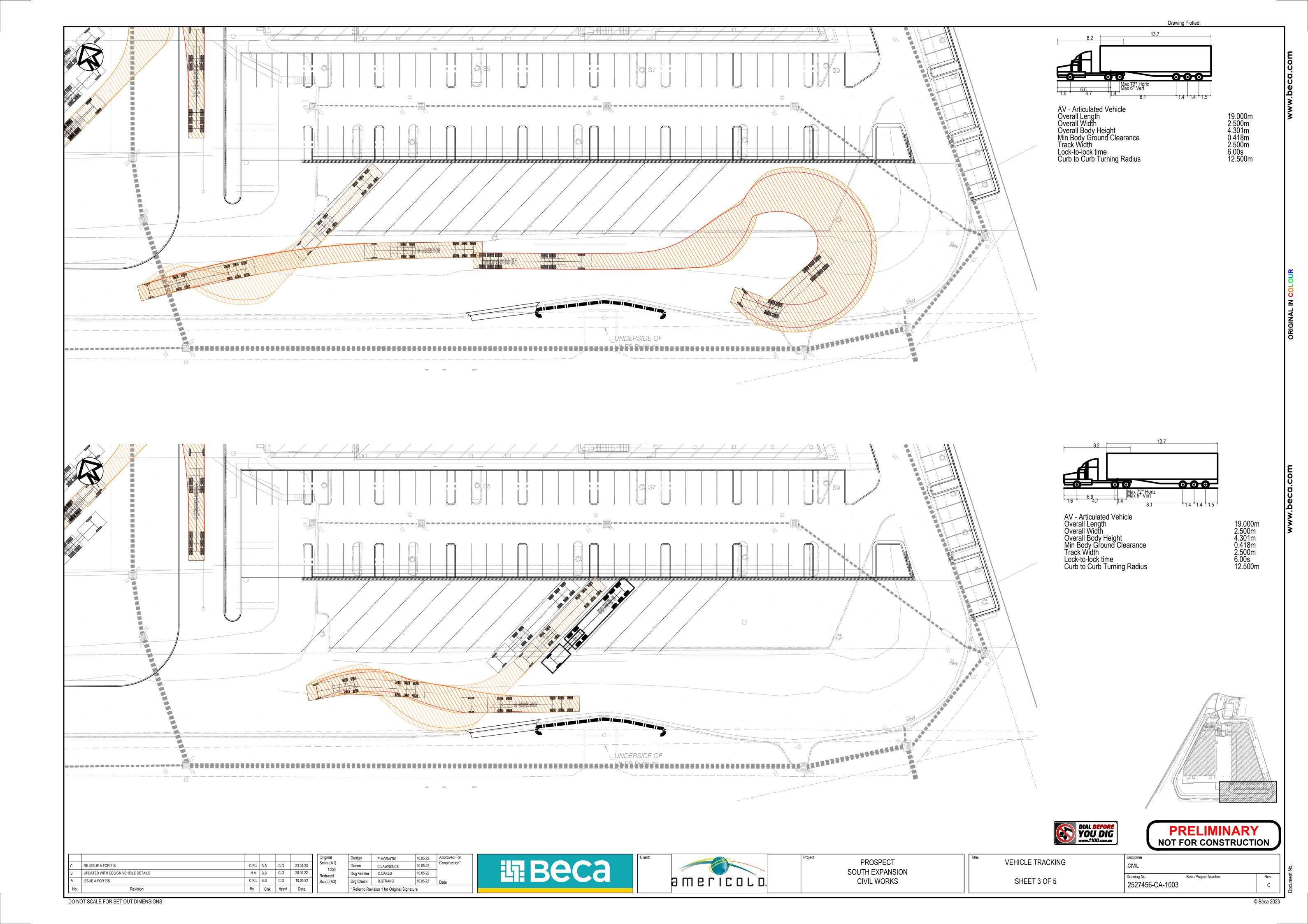


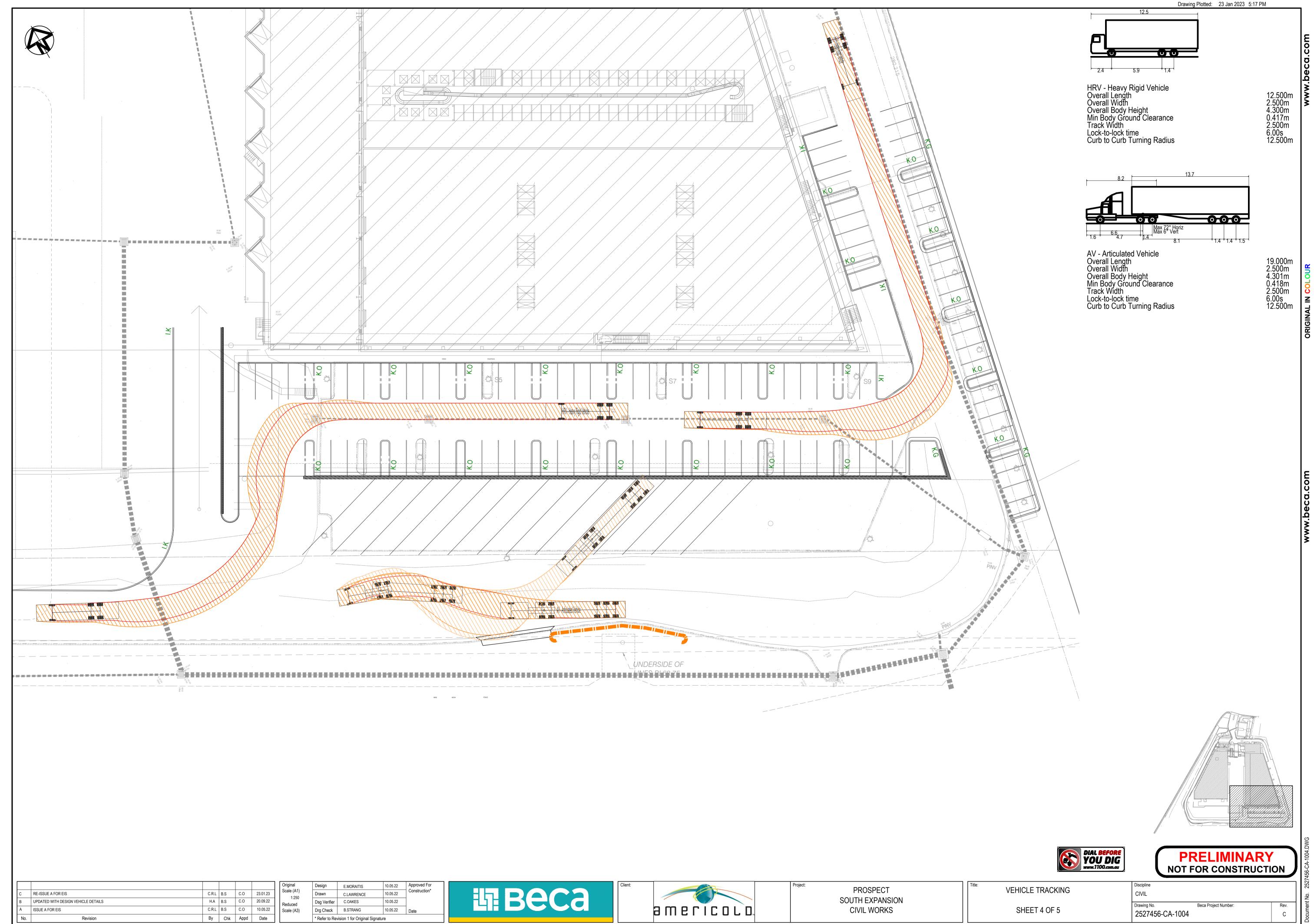


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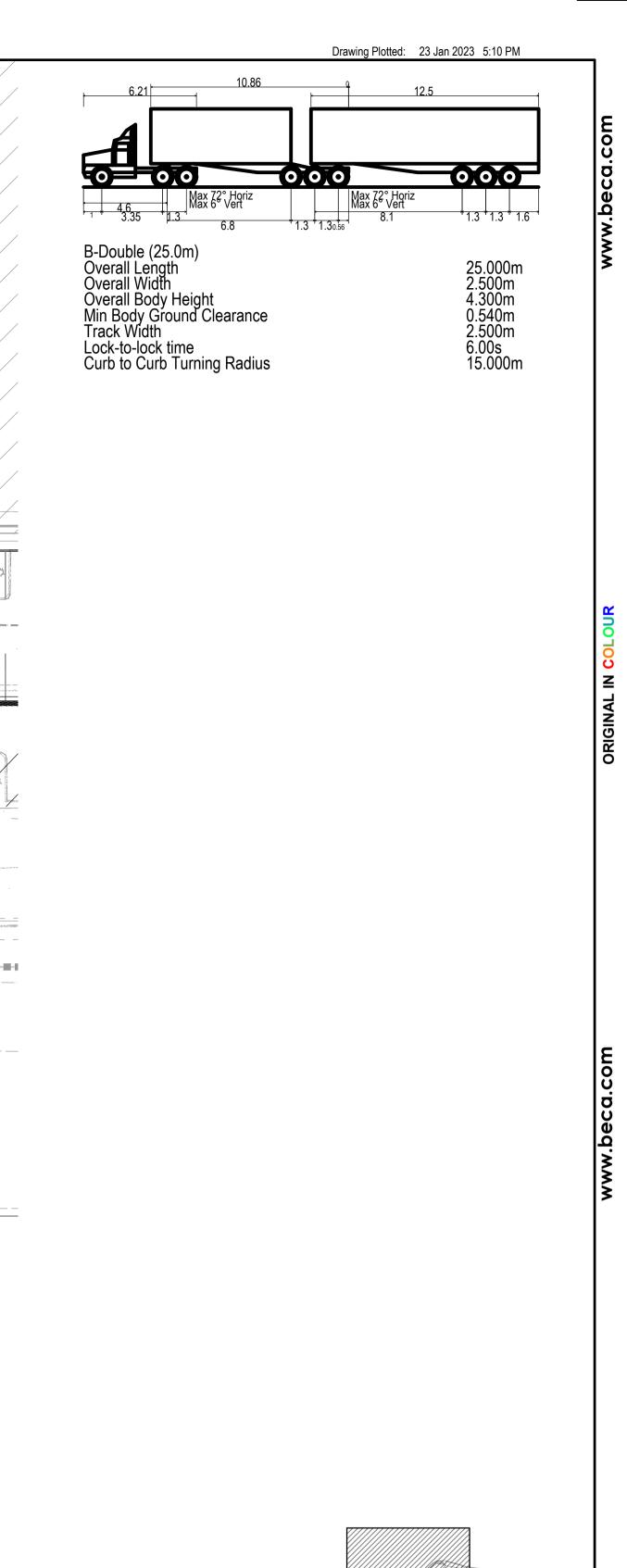
VEHICLE TRACKING
SHEET 2 OF 5

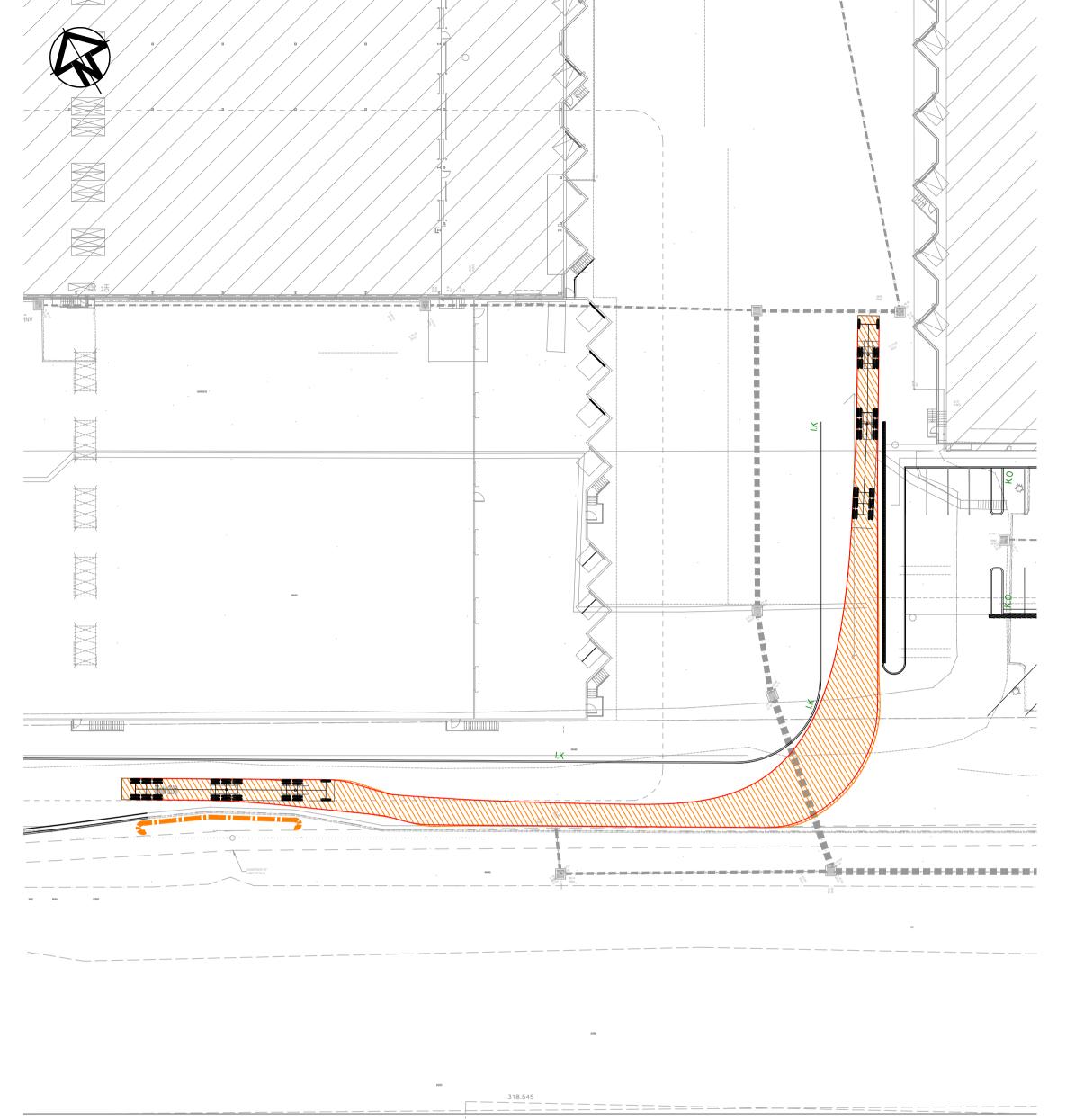
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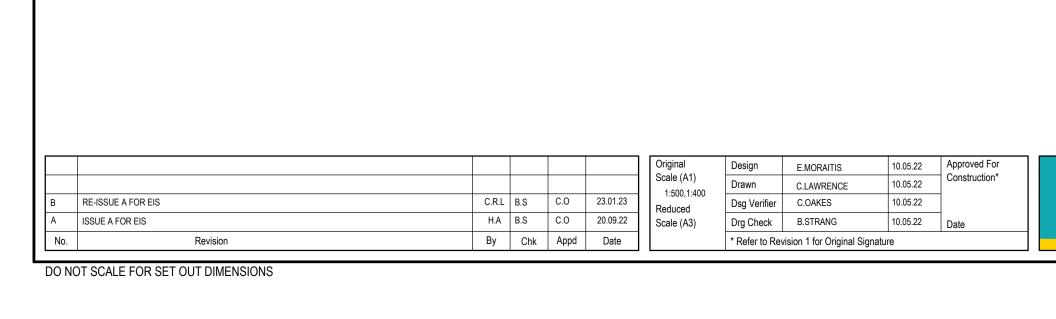
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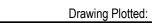
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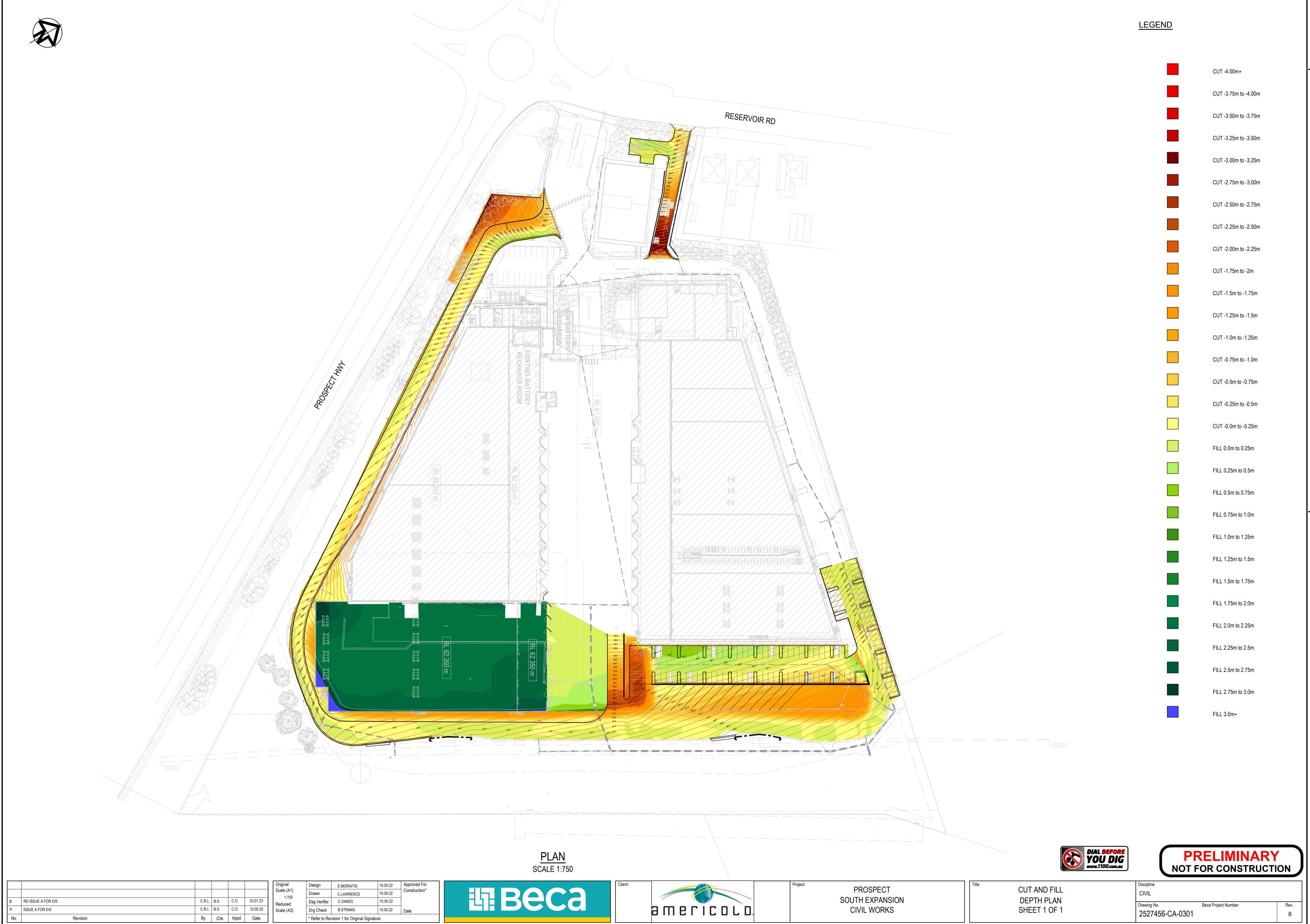
VEHICLE TRACKING SHEET 5 OF 5

Drawing No. 2527456-CA-1005 Beca Project Number: Rev.

Appendix B – Erosion and Sediment Control Plan









Appendix C – WSUD Catchment Plan



