



Proposed School –

507 MEDOWIE ROAD, MEDOWIE

Stormwater Management Plan

for

Catholic Schools Office

August 2018 – Revision 2

MPC Project Ref: 17-828

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- C Catchment Areas and Summary of Stormwater Design Intent
- D Erosion and Sediment Control Plan and Calculations
- E Water Quality (MUSIC) Model
- F Stormwater Maintenance Plan

1. Background Information

1.1 Preamble

The site is located at 507 Medowie Road, Medowie NSW (refer to **Appendix A** for site plans). The proposed redevelopment includes the following:

- 17 School Buildings
- Landscaped areas
- Carpark and Terrace Areas
- Site Detention system
- Rainwater harvesting and Re-use
- Stormwater pollution control

1.2 Stormwater Management Plan

In devising this Stormwater Management Plan for the proposed development, the following issues have been addressed:

- Water Quality Management
- Stormwater Management (Detention)
- Stormwater Harvesting (Rainwater re-use)

The stormwater and environmental management philosophy employed in the Stormwater Management Plan is discussed in Section 3.0.

As well as permanent water management controls, construction phase controls are also addressed, in section 5.0.

In preparing this Stormwater Management Plan a review has been undertaken with Port Stephens Council Development Control Plan, with the aim to incorporate Water Sensitive Urban Design measures.

1.3 Background Information

Based on our review of the DCP and previous experience on similar sites within the Port Stephens Council Precinct, we understand the following:

- that on-site stormwater harvesting measure would be required for the roof areas of the proposed development for reuse in landscape areas;
- Site stormwater detention would be required to limit the post developed stormwater discharge off the site to the pre-developed site discharge;
- Water quality measures to ensure that the stormwater discharge from the site creates a Neutral or Beneficial Effect on Water Quality (NorBE) on the receiving Grahamstown drinking water catchment.

2. Site and Catchment Details

2.1 The Existing Site

The existing site to be developed comprises of a total plan area of approximately 81200m².

The site currently has an existing dwelling and several sheds which are proposed to be demolished as part of the proposed works.

2.2 The Proposed Site

Architectural drawings prepared by Webber Architects have been provided to MPC and show the layout of the proposed development. These have been used as the basis of the stormwater management and sediment and erosion control concept design. A copy of the architectural site plan is included as **Appendix A** of this report.

The proposed site generally comprises of the following:

- 17 School Buildings;
- New carparking and roadworks;
- External paving and landscape areas.

2.3 Catchment

The site does contain survey marked water courses. Therefore the site is used to convey stormwater from the neighbouring catchment areas towards the South West of the site.

Runoff from Medowie road currently passes through an open swale to the south of the proposed school. Refer to stormwater plan for discharge locations.

The site is bounded residential dwellings to the North, Medowie road to the East and an ecology area to the West.

3. Stormwater and Environmental Management Philosophy

In preparing this Stormwater Management Plan we have consulted with Councils Development Control plan in relation to stormwater. The requirements to be addressed are as follows:

- Ensure that the rate of rainwater runoff from roofs and paved areas from the pre-developed site is not increased for the developed condition for all storms up to and including the 1 in 100 years ARI event;
- Provide detention of the post-developed flows such that they do not exceed the pre-developed conditions;
- Provide rainwater re-use where appropriate, Proprietary first flush devices would be proposed prior to rainwater entering the harvesting tank. Harvested rainwater will be used for irrigation purposes of the landscape areas and sporting fields;
- To ensure Water Sensitive principles are adopted, the site drainage system will also incorporate pollution control measures designed to remove and site generated pollutants in accordance with Port Stephens Councils DCP. The hydraulic engineering consultant will be required to design a system of pollution control in order to satisfy the requirements of the DCP prior to water overflowing from the harvesting tank;
- Water quality measures to ensure that the stormwater discharge from the site creates a Neutral or Beneficial Effect on Water Quality (NorBE) on the receiving Grahamstown drinking water catchment.
- Ensure that overland flow in the event of a choked or blocked piped system does not impact on neighbouring properties or other buildings on the site.
- Install appropriate erosion protection and soil stabilisation measures in association with the proposed site works. Such measures are to be designed in accordance with the requirements of the Managing Urban Stormwater: Soils and Construction 4th Edition – Vol.1 (the “Blue Book”) published by Landcom, 2004

4. Proposed Stormwater Management Facilities

4.1 Preamble

Section 4.2 gives an outline of the nature and function of stormwater management facilities to be incorporated in the proposed development.

Section 4.3 discusses the design storm events for which the stormwater management system is provided.

The site area is shown in **Appendix A**. The location and operation of stormwater management facilities for the catchment is discussed in Section 4.4.

On going maintenance and monitoring of the stormwater management system is discussed in Section 4.5.

4.2 Nature and Function of Stormwater Management Facilities

The stormwater management plan is shown in **Appendix B**. The principal stormwater management components and their function are listed below:

- a). The proposed works consists of redevelopment of the entire site. Stormwater systems are designed to cater for roof, hardstand, and landscaped areas.
- b). Roof rainwater from each new building will be directed through a new pipe/pit system to a 4000 litre above ground rainwater tank per building (Approximately 68000 litres minimum total capacity) with over flows being connected to the developments detention basins. Final tank locations are to be confirmed during detailed design of the site.
- c). Retention facilities will be incorporated into the network in accordance with councils DCP. Low flow outlet measures will be provided for minor and major rainfall events with all overflow being directed to the South West and Western areas of the site. The site has been designed to incorporate a mix of Atlantis infiltration tanks and bio filtration detention ponds, Gross Pollutant traps, pollutant pit inserts in the carpark, bio filtration systems and as such stormwater quality for the existing site will not be compromised by the proposed development (refer Appendix B);

4.3 Design Storm Events

The stormwater management system for the proposed 'Developed Site Area' will collect roof rainwater in Harvesting and Retention facilities that will be designed in accordance with councils DCP. Blocked system overflow locations for large storm events have been provided and will be fully detailed in final design documentation.

4.4 Stormwater Harvesting

Roof Rainwater Tanks

It is proposed to use a rainfall depth of 18.5mm for calculation of captured roof water for harvesting tank for each building for the purposes of irrigation. Roof rainwater is piped directly to the harvesting tanks via the downpipe system and a first flush devices.

A total roof area of the proposed development is approximately 15,000 m². This roof area will allow a total harvested volume of approximately 280kL. This volume will be divided amongst six rainwater tanks located around the site for the purposes of irrigating landscape areas and the sporting fields. The final location of the rainwater tanks will be determined during the detailed design phase of the project.

In order to ensure supply to the connected uses, there will be a control valve connected to mains supply to maintain a minimum of 10% tank capacity. Mains back-up will require interconnection with Hunter Water mains.

Backflow prevention methods will be provided to ensure the protection of the mains water supply. A demand pump will be provided to supply tank water to internal plumbing fixtures.

Rainwater tanks will be used as a retention system for the proposed roof catchment areas. Water will be released from the rainwater tanks at the calculated pre-developed flows.

A high level overflow pipe will be provided in the event of high rainfall periods and a blocked overflow which is directed to the sites detention ponds.

4.5 Stormwater Detention

This section refers to the requirements in Port Stephens DCP. The primary aim with site run-off under the DCP is to ensure that the run-off from the developed site replicates that of the natural conditions.

The drainage system is to be designed for peak run-off with this run-off being released at a rate comparable with natural conditions during peak rainfall.

The Stormwater Detention is proposed to be in a combination of underground Atlantis infiltration tanks and above ground bio-retention basins. Each of the detention tanks and bio retention ponds rely on infiltration as a slow release with high level piped systems allowing release of stormwater at pre-developed flow rates. This method has been adopted due to the flatness of the lower end of the site and that there is limited ability to provide a piped slow release on the site. We have also not used any detention in the carparks to ensure that they remain serviceable without nuisance water during rainfall events.

An infiltration rate of 25mm/hr has been adopted for each basin or tank.

Refer to markup in Attachment C for catchment area details.

Pre and Post-Developed Flows are summarised in **Tables 1 to 4** below.

Catchment Area 1 (13740m²)

Catchment area has multiple Atlantis detention tanks. The information in table 1 is cumulative and incorporates all tanks.

Table 1: Stormwater Detention Calculations

Item	Minor Storm	Major Storm
ARI (years)	20	100
Pre-Developed Flow	254 L/s	369 L/s
Control	Infiltration / Recharging the groundwater in each cell + Highlevel overflow	Highlevel outflow pipe from each atlantis cell
Basin Storage Volume	353 m ³	385m ³
Outflow (L/s)	121 L/s from Basin	232 L/s from Basin

Catchment Area 2 (10830m²)

Table 1: Stormwater Detention Calculations

Item	Minor Storm	Major Storm
ARI (years)	20	100
Pre-Developed Flow	210 L/s	305 L/s
Control	Infiltration / Recharging the groundwater in each cell + Highleve overflow	Highlevel outflow pipe from each atlantis cell
Basin Storage Volume	382 m ³	454m ³
Outflow (L/s)	41 L/s from Basin	83 L/s from Basin

Catchment Area 3 (37340m²)

Table 1: Stormwater Detention Calculations

Item	Minor Storm	Major Storm
ARI (years)	20	100
Pre-Developed Flow	540 L/s	858 L/s
Control	600 mm diameter orifice + Infiltration	600 mm diameter orifice + Weir RL 7.8
Basin Storage Volume	485 m ³	509m ³
Basin Water Level	7.34 AHD	7.59 AHD
Outflow (L/s)	425 L/s from Basin	634 L/s from Basin

Catchment Area 4 (19265m²)

Table 1: Stormwater Detention Calculations

Item	Minor Storm	Major Storm
ARI (years)	20	100
Pre-Developed Flow	311 L/s	494 L/s
Control	445 mm diameter orifice + Infiltration	450 mm diameter orifice + Weir RL 8.4
Basin Storage Volume	190 m ³	245m ³
Basin Water Level	8.13 m AHD	8.35 AHD
Outflow (L/s)	209 L/s from Basin	286 L/s from Basin

4.6 Site Flood Storage Analysis

The site is affected by floor towards the South and South East of the school building. A minimum design floor level of 9.30 AHD has been adopted and is above the maximum flood level provided by Port Stephens Council.

4.7 Water Quality

It is our intention to comply with the Protection of the Environment Operations Act 1997, in particular water quality exiting the site during construction and operation.

Stormwater quality requirements from the Port Stephens Council DCP, and in particular the Water Quality Targets within the DCP have been impropriated into the overall stormwater management design for the site.

Water Quality measures for the site have been modelled using MUSIC software and include the following:

- Rainwater from the roof of each building will be directed through a first flush device before being stored in a water re-use tank;
- Stormwater from impervious areas will be directed through enviropod inserts in each pit, atlantis cell infiltration tanks, GPT's then to a bio-retention basin.
- Proprietary "Gross Pollutant Trap" has been specified in the location shown on the stormwater management plans;
- Bio-filtration facilities have been incorporated in the catchment 3 and 4 basins.

As part of the water quality assessment to demonstrate that the water quality of the proposed development meets the requirements of the NorBE, we have modelled the predeveloped conditions and compared these results to the results from the developed site. The stormwater quality devices and systems have been specified on the stormwater management plans included in **Appendix B**, which collectively achieve the water quality targets listed below:

	Pre-Developed Residual Load	Developed Residual Load	% Reduction
Total Suspended Solids (kg/yr)	2390	608	91.8
Total Phosphorous (kg/yr)	6.82	6.08	63.5
Total Nitrogen (kg/yr)	60.4	60.2	53
Gross Pollutants (kg/yr)	29.8	0.0	100

A copy of the MUSIC model diagrams, including the receiving node pollution reductions achieved, are included in **Appendix E**.

The basin has also been sized as a temporary sediment control basin for initial bulk earthworks construction phase, in accordance with the procedures in the “*Soils and Construction – Managing Urban Stormwater*” guidelines. Additional details in this regard are included in **Appendix D**.

4.8 Maintenance of Stormwater Management Facilities

Maintenance of concrete pits, pipes and paved flow paths will be minimal as they are generally self-cleansing, and hence only involve very occasional cleaning. Regular inspections of control systems should be carried out to ensure satisfactory performance of the drainage systems proposed. Sediment/pollution control pits and proprietary pollution control devices will be provided prior to entering irrigation and retention facilities. Proprietary tanks or pollution control chambers located in roadway areas will also be accessible for cleaning and maintenance. Maintenance should occur on a 3 month basis or after major storm events. A maintenance plan has been included in **Appendix F**.

5. Construction Phase Erosion and Sediment Controls

The construction phase approach adopted for this site will incorporate principles recommended by the NSW Department of Housing, namely:

- Plan for erosion and sediment control concurrently with engineering design and in advance of earthworks proper assessment of site constraints and integration of the various needs;
- Minimise the area of soil exposure;
- Conserve the topsoil where possible;
- Control water flow from the top of the development area, through the works and out the bottom of the site, for example,
 - divert clean runoff above denuded areas
 - minimize slope gradient and length
 - keep runoff at non-erodible velocities
 - trap soil and water pollutants
- Rehabilitate disturbed lands quickly.

A preliminary design of erosion and sediment controls for the overall site development is shown in **Appendix D**. Controls will be provided on the site prior to and during all earthworks in accordance with EPA Site Work Practices. Features of the construction phase erosion and sediment controls adopted for this site include:

- Prevention of sediment and polluted runoff water from entering the existing adjacent watercourse. This involves the provision of silt fences, catch drains and sediment traps.
- Control of actual and potential soil erosion – grassing and stabilization of embankments and drainage outlets where required.
- Stabilised stockpile areas to prevent wind and water erosion.
- Scour protection at discharge locations.
- Stabilised site access to provide a firm base for vehicle entry/exit and to prevent the main access from becoming a source of sediment.

6. Summary

This stormwater management plan has been prepared by MPC Consulting Engineers for Catholic Schools Office, and the systems outlined in this report address the requirements of Port Stephens Council DCP.

For further information in relation to this stormwater management plan please contact the undersigned.

Signed:

MATTHEW SNELSON

BE (Civil)(Hons), MIEAust,

Director

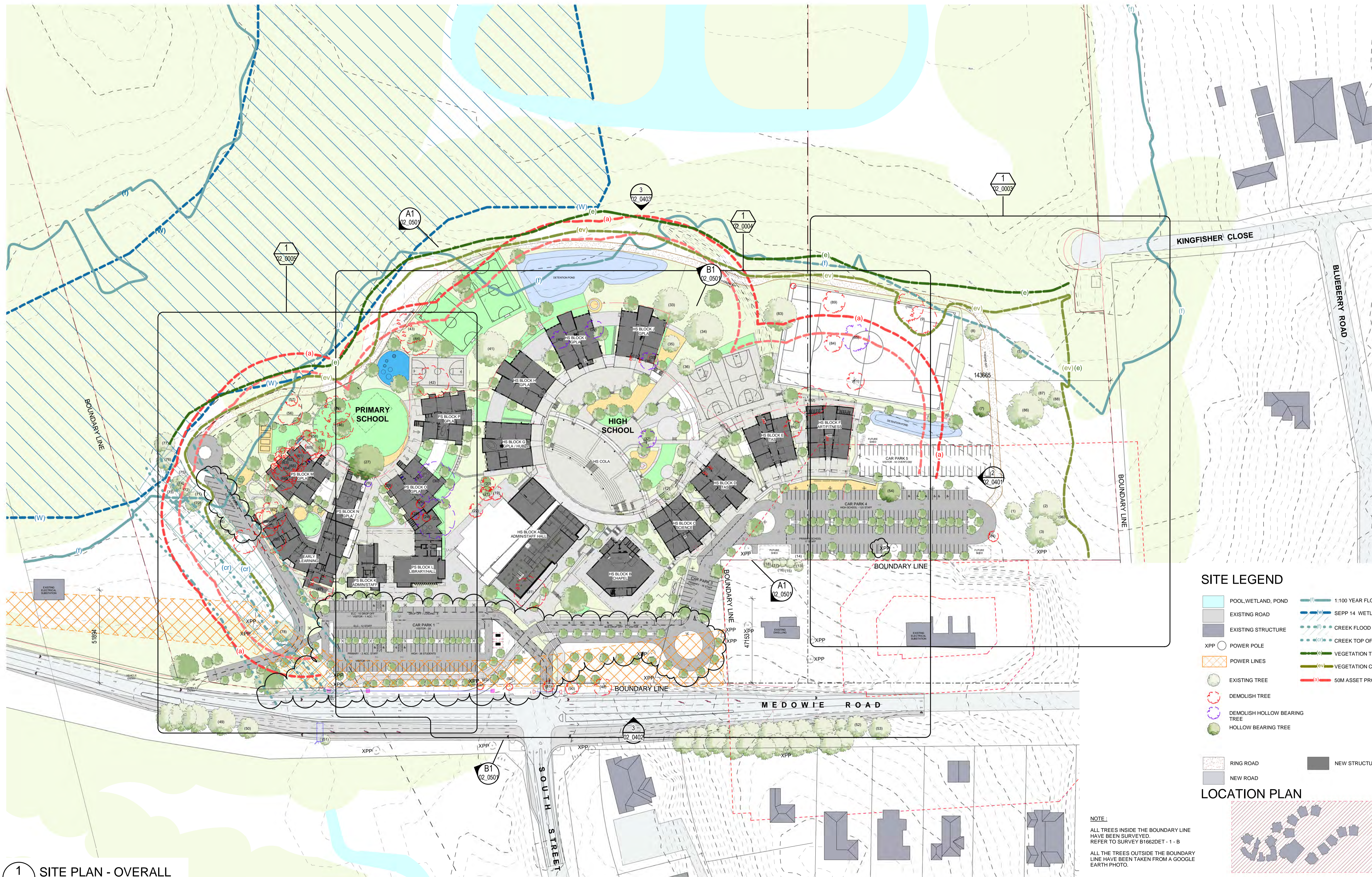
Date: 12 September 2018

Appendices:

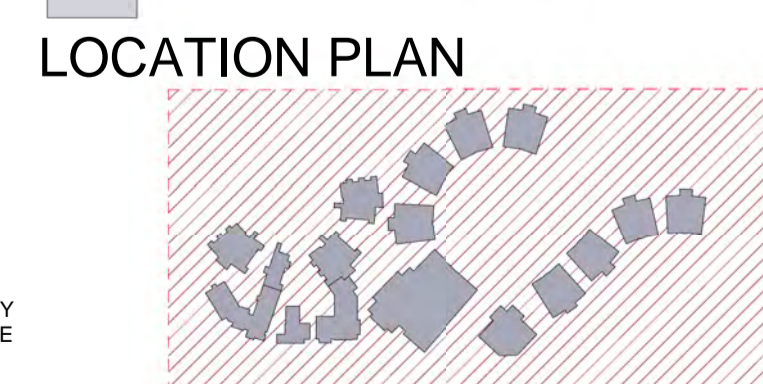
- A Site Plan
- B Stormwater Management Plan
- C Catchment and Summary of Stormwater Design Intent
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- E Stormwater Quality (Music) Model
- F Stormwater Maintenance Plan

Appendix A

Site Plan



- ### SITE LEGEND
- POOL, WETLAND, POND
 - EXISTING ROAD
 - EXISTING STRUCTURE
 - XPP POWER POLE
 - POWER LINES
 - EXISTING TREE
 - DEMOLISH TREE
 - DEMOLISH HOLLOW BEARING TREE
 - HOLLOW BEARING TREE
 - RING ROAD
 - NEW ROAD
 - NEW STRUCTURE
 - 1-100 YEAR FLOOD LINE
 - SEPP 14 WETLANDS
 - CREEK FLOOD LINE
 - CREEK TOP OF BANK
 - VEGETATION TRUNK LINE
 - VEGETATION CANOPY LINE
 - 50M ASSET PROTECTION ZONE



NOTE:
 ALL TREES INSIDE THE BOUNDARY LINE HAVE BEEN SURVEYED. REFER TO SURVEY B1662DET - 1 - B
 ALL THE TREES OUTSIDE THE BOUNDARY LINE HAVE BEEN TAKEN FROM A GOOGLE EARTH PHOTO.

1 SITE PLAN - OVERALL
 02_0401 SCALE 1:1000

REV	DATE	DESCRIPTION	BY	CHK
W	16.02.2018	DEVELOPED DESIGN - 90%	DF	
X	02.03.2018	FOR CONSULTANT COORDINATION	LK	
Y	09.03.2018	FOR QA	LK	
Z	14.03.2018	FOR CONSULTANT COORDINATION	LK	
AA	29.03.2018	FOR CLIENT APPROVAL	LK	TH
BB	04.04.2018	FOR CLIENT APPROVAL	LK	
CC	16.04.2018	FOR COUNCIL INFORMATION	LK	
DD	04.05.2018	FOR REVIEW	ME	
EE	10.05.2018	FOR CONSULTANT COORDINATION	LK	



webberarchitects
 commercial and residential
 Suite 3, Level 1 498 Hunter St Newcastle - PO Box 807 The Junction 2291
 T +61 2 4824 1078 Fax +61 2 4824 1078
 newcastle@webberarchitects.com www.webberarchitects.com

SITE PLAN - OVERALL
 CATHERINE MCAULEY CATHOLIC COLLEGE
 507 MEDOWIE ROAD, MEDOWIE

ISSUED: **NOT FOR CONSTRUCTION**

PROJECT COMMENCEMENT DATE: 01.05.2017

SCALE: 0 10 20 30 40 50 m
 SCALE 1:1000 @A1

SHEET NUMBER: **2544_02_0002_EE**

NORTH:

REVISIONS: 01.05.2017

Appendix B

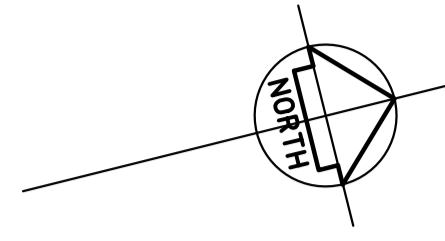
Stormwater Management Plan

STORMWATER PLAN SHEET 1

SCALE 1:200

STORMWATER NOTES

1. ALL WORKS TO BE IN ACCORDANCE WITH AS3500.3.
2. ALL PIPES TO HAVE A 1% MINIMUM FALL U.N.O.
3. ALL DOWNPIPES (DP) TO BE SPECIFIED BY ARCHITECT. FOR EXACT LOCATION OF DOWNPIPES, REFER TO ARCHITECTURAL DRAWINGS.
4. ALL PIPES TO BE UPVC U.N.O.
5. ALL UPVC PIPES TO BE SEWER GRADE AND TO AS1260.
6. ALL REINFORCED CONCRETE PIPES (RCP) TO BE SPIGOT AND SOCKET TYPE WITH RUBBER RINGS CLASS 2 TO AS4058.
7. PITS TO BE CI&D REINFORCED PRE-CAST CONCRETE PITS OR EQUIVALENT PROPRIETARY PITS.
8. ALL LIDS AND GRATES TO BE PROPRIETARY HEAVY DUTY IN AREAS OF VEHICULAR TRAFFIC, LIGHT DUTY ELSEWHERE, IN ACCORDANCE WITH AS3996.
9. MINIMUM COVER TO STORMWATER PIPES TO BE AS FOLLOW U.N.O:
TRAFFICABLE AREAS - 450mm, LANDSCAPED AREAS - 300mm.
PIPES TO BE CONCRETE ENCASED IF MINIMUM COVERS CANNOT BE OBTAINED IN TRAFFICABLE AREAS, REFER TO CLAUSE 3.8 AS3500.3.
ALTERNATIVELY USE UPVC SEWER GRADE PIPES UNDER ROAD AND BUILDINGS.
10. PROVIDE 100Φ AG DRAINS IN FILTER SOCKS TO ALL LANDSCAPED AREAS, PLANTER BEDS AND STORMWATER PIPE TRENCHES.
ALL AG DRAINS TO BE BEDDED IN COARSE AGGREGATE AND TO BE CONNECTED TO STORMWATER SYSTEM.
11. ALL PITS, DETENTION TANKS AND PROPRIETARY POLLUTION CONTROL DEVICES TO BE CLEANED OF SEDIMENT AT 3 MONTH MAXIMUM INTERVALS.
12. ALL EXISTING SERVICES TO BE LOCATED PRIOR TO COMMENCEMENT OF WORK.
13. ANY FOOTPATHS, KERB AND GUTTER OR ROADWAY DISTURBED BY WORKS TO BE REINSTATED TO CURRENT COUNCIL REQUIREMENTS.
14. PROVIDE ACCESS LADDER TO TANK AS REQUIRED, REFER TO AS1657.



LEGEND

- DENOTES STORMWATER PIPE
- DENOTES EXISTING CONTOUR
- DENOTES EXISTING LEVELS
- DENOTES DESIGN SPOT LEVELS
- K1** DENOTES 120 HIGH KERB U.N.O.
- K2** DENOTES ROLLED KERB TO ARCH DETAILS
- DENOTES RETAINING WALL TO ARCH DETAILS
- DENOTES LANDSCAPE RETAINING WALL TO ARCH DETAILS
- DENOTES DIRECTION OF SURFACE FLOWS

NOTE
ALL CARPARK PITS TO HAVE PIT INSERTS TO CAPTURE HYDROCARBENS PRIOR TO DETENTION AND GPT FOR ADDITIONAL TREATMENT

NOTE
ALL ROOF WATER TO CONNECT TO AT WITH FIRST FLUSH DEVICE TYPICAL

NOTE
ADDITIONAL PITS IN LANDSCAPE AREAS T.B.C. DURING DETAILED DESIGN PHASE

PROVIDE 4000 LITRE SLIMLINE WATER STORAGE TANKS STORING ROOF RAINWATER TO MANUFACTURERS SPECIFICATION TO EACH BLOCK. TANK SHALL BE FITTED WITH A FIRST FLUSH SYSTEM, PUMP TO SUPPLY TOILETS AND LAUNDRIES AND A DIVERSION SWITCH TO MAINS SUPPLY ON TANK BEING EMPTY. BACK FLOW PREVENTION TO MAINS WATER SHALL BE PROVIDED. TANK TO OVERFLOW TO STORMWATER SYSTEM. LOCATIONS TO ARCH DETAILS

MATCH LINE A

NOT FOR CONSTRUCTION

MATCH LINE C

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3	DEVELOPMENT APPLICATION	17.5.18			
2	DEVELOPMENT APPLICATION	15.5.18			
1	DEVELOPMENT APPLICATION	28.3.18			
0	REVIEW	16.3.18			
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Suite 3, Level 1,
16 Telford Street,
NEWCASTLE EAST, NSW 2300
PO BOX 553
THE JUNCTION, NSW 2291
Tel: (02) 4927 5566
Fax: (02) 4927 5577
Email: admin@mpceing.com.au
Web: www.mpceing.com.au
A.C.N. 098 542 575

CLIENT
CATHOLIC SCHOOLS OFFICE

TITLE
STORMWATER PLAN SHEET 1

PROJECT
CATHERINE McAULEY CATHOLIC COLLEGE
AT; LOT 412, DP 1063902,
No.507 MEDOWIE ROAD,
MEDOWIE

DO NOT SCALE DRAWING			
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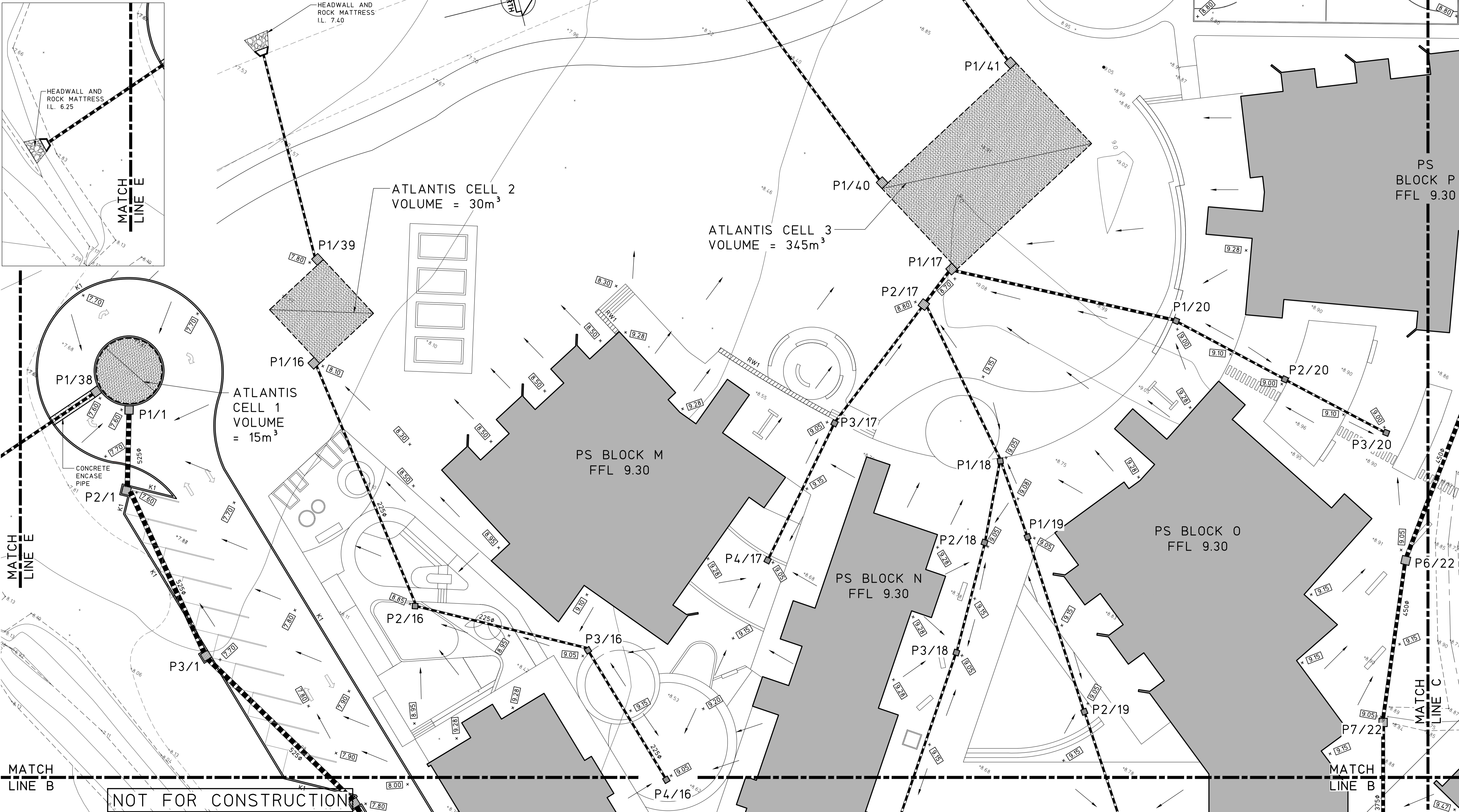
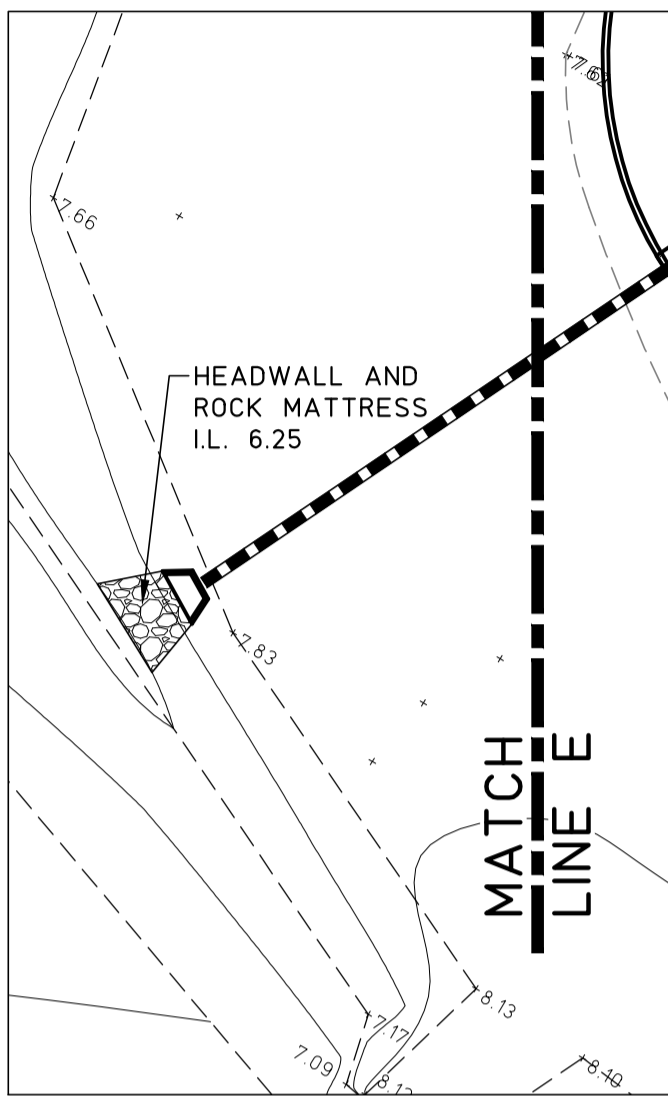
FULL SIZE ON ORIGINAL 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 cm

MATCH
LINE A

MATCH
LINE A

STORMWATER PLAN SHEET 2

SCALE 1:200
STORMWATER NOTES
1. REFER TO DRAWING C100 FOR STORMWATER NOTES AND PIT SCHEDULE
2. REFER TO DRAWING C108 FOR PIT SCHEDULE



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PO BOX 553
THE JUNCTION, NSW 2291
Tel: (02) 4927 5566
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**STORMWATER PLAN
SHEET 2**

PROJECT
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AT; LOT 412, DP 1063902,
No.507 MEDOWIE ROAD,
MEDOWIE**

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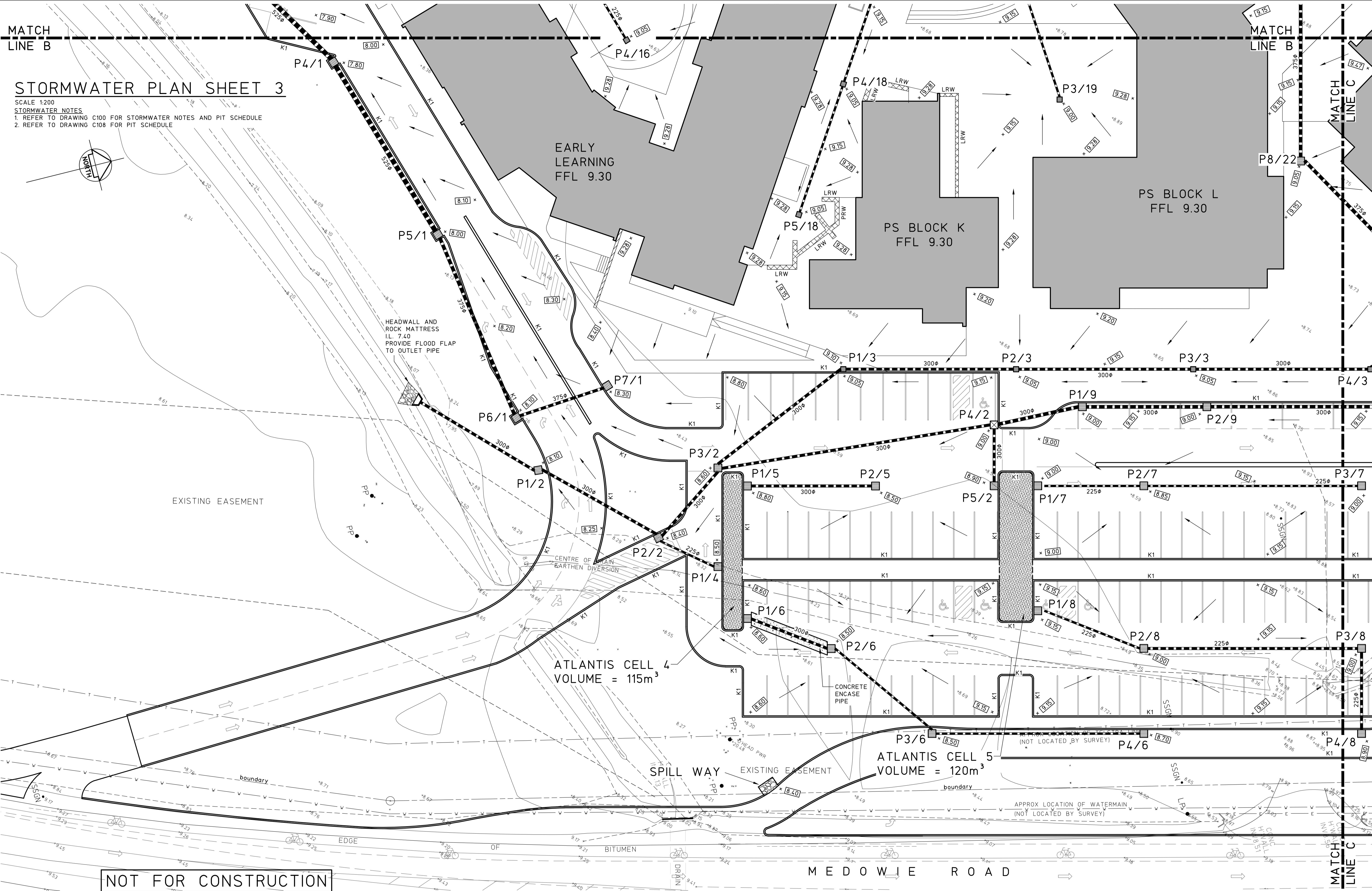
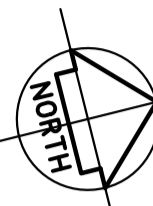
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MATCH LINE B

MATCH LINE B

STORMWATER PLAN SHEET 3

SCALE 1:200
 STORMWATER NOTES
 1. REFER TO DRAWING C100 FOR STORMWATER NOTES AND PIT SCHEDULE
 2. REFER TO DRAWING C108 FOR PIT SCHEDULE



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 Tel: (02) 4927 5566
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TITLE
 STORMWATER PLAN
 SHEET 3

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 No.507 MEDOWIE ROAD,
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0	REVIEW	16.3.18			

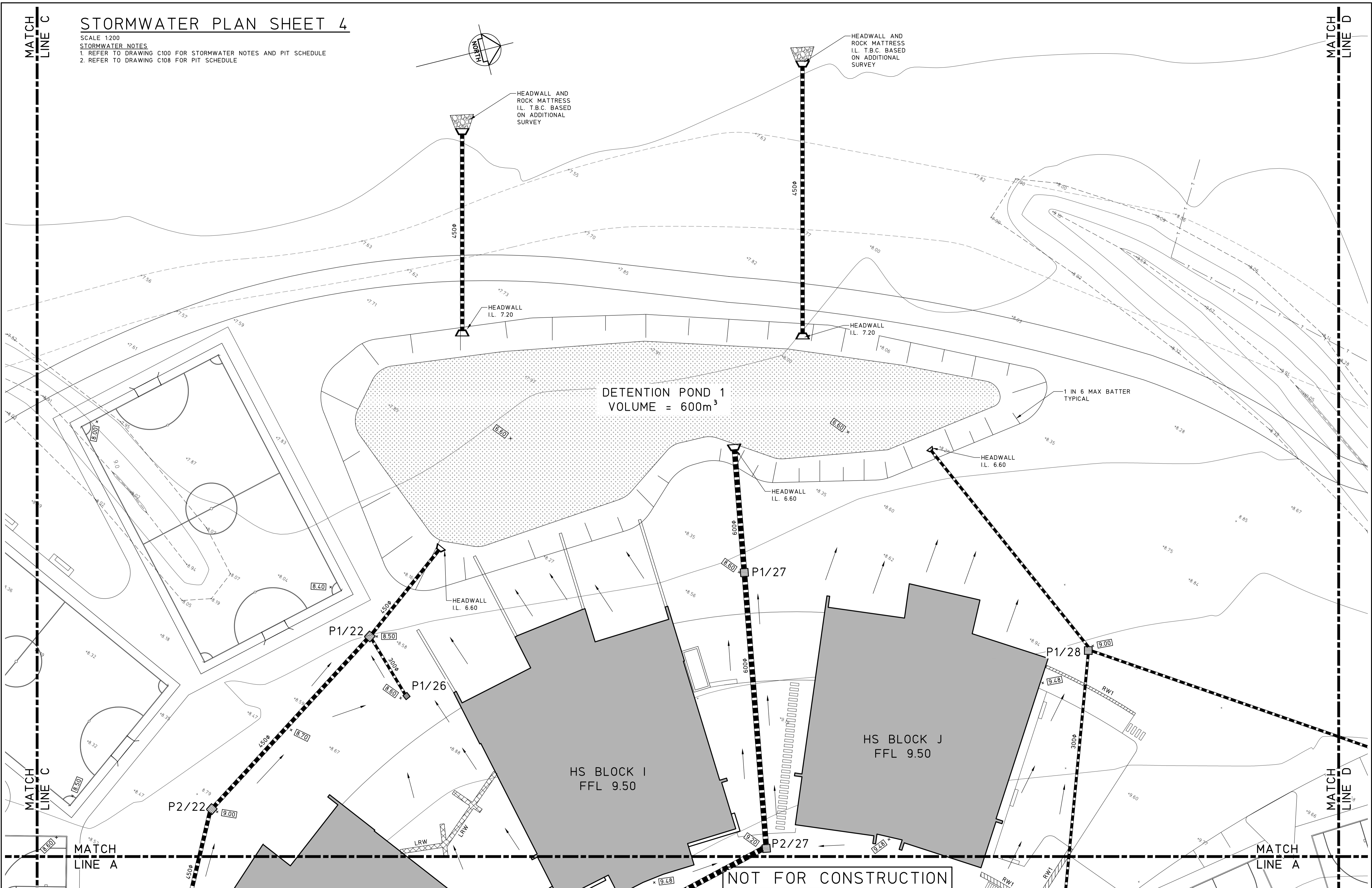
FULL SIZE ON ORIGINAL 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 cm

MATCH LINE C

STORMWATER PLAN SHEET 4

SCALE 1:200
 STORMWATER NOTES
 1. REFER TO DRAWING C100 FOR STORMWATER NOTES AND PIT SCHEDULE
 2. REFER TO DRAWING C108 FOR PIT SCHEDULE

MATCH LINE D



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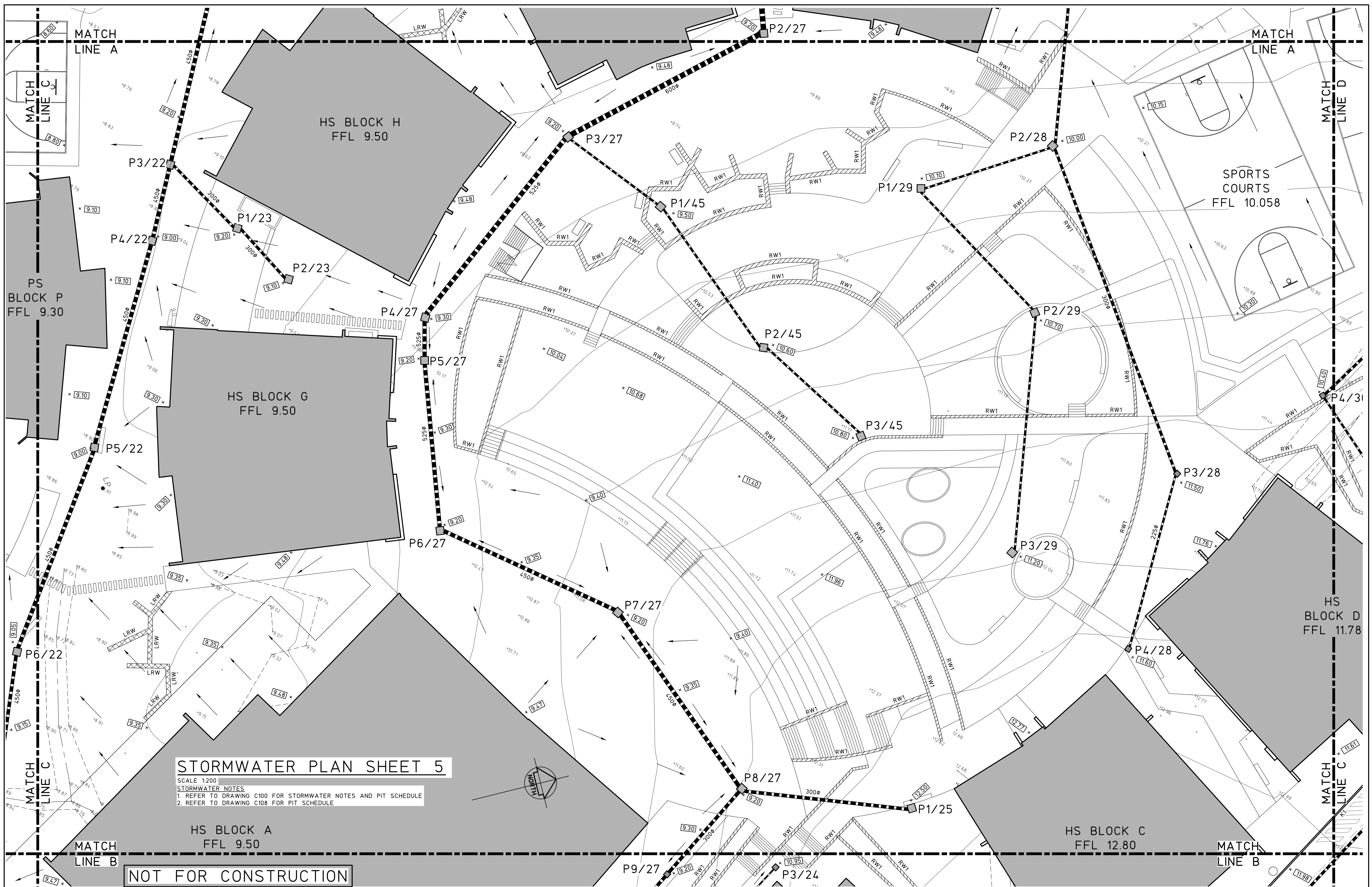
CLIENT
CATHOLIC SCHOOLS OFFICE

TITLE
STORMWATER PLAN SHEET 4

PROJECT
**CATHERINE McAULEY CATHOLIC COLLEGE
 AT; LOT 412, DP 1063902,
 No.507 MEDOWIE ROAD,
 MEDOWIE**

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J.P.	M.S.	--	A1
SCALES	JOB No	DRAWING No	ISSUE
1:200	17-828	C103	3



STORMWATER PLAN SHEET 5

SCALE 1:200
STORMWATER NOTES
 1. REFER TO DRAWING C100 FOR STORMWATER NOTES AND PIT SCHEDULE
 2. REFER TO DRAWING C108 FOR PIT SCHEDULE

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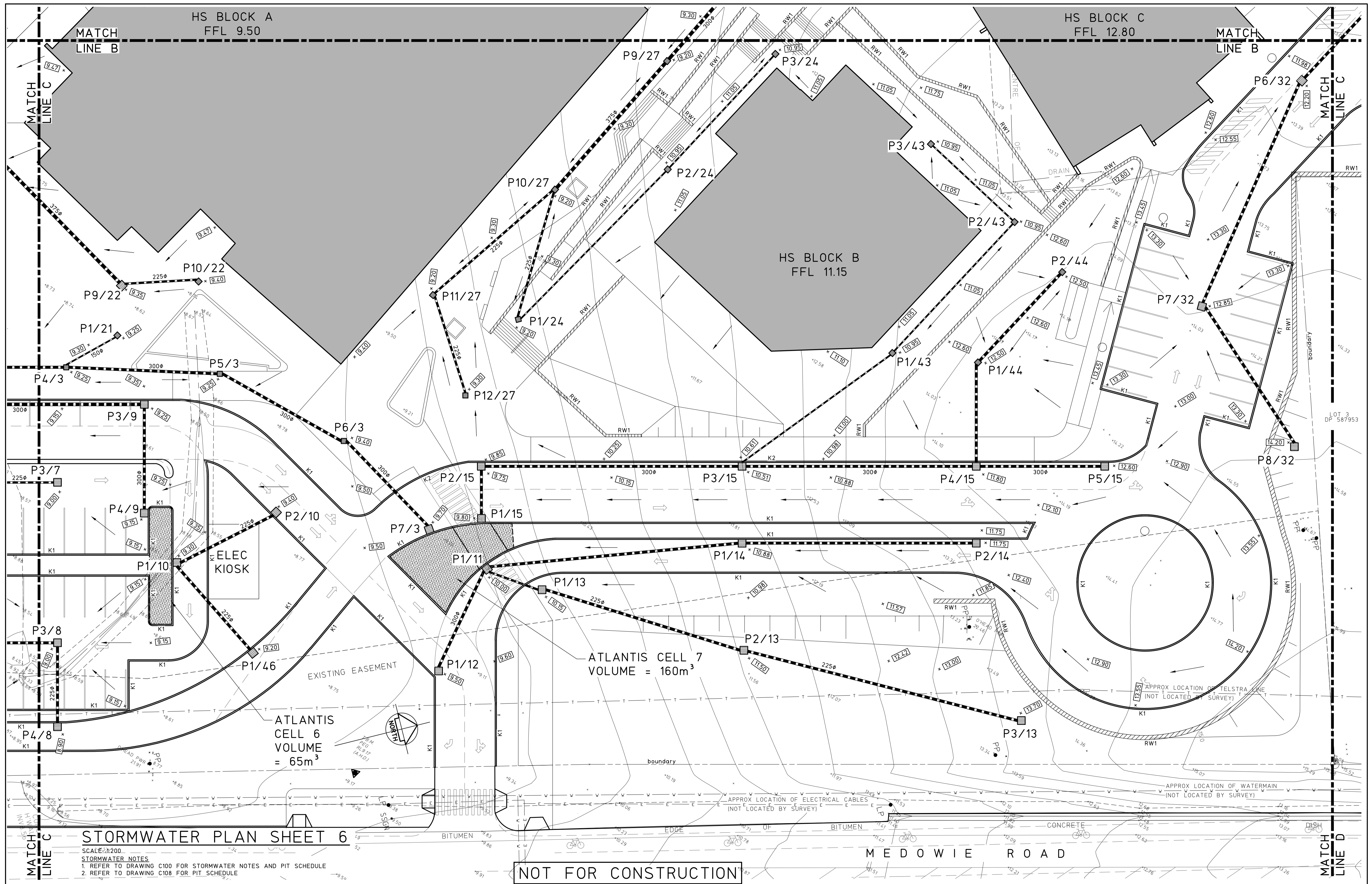
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TITLE
STORMWATER PLAN SHEET 5

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 AT; LOT 412, DP 1063902,
 No.507 MEDOWIE ROAD,
 MEDOWIE**

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SCALES 1:200	JOB No 17-828	DRAWING No C104	ISSUE 3



STORMWATER PLAN SHEET 6

SCALE: 1:200
 STORMWATER NOTES
 1. REFER TO DRAWING C100 FOR STORMWATER NOTES AND PIT SCHEDULE
 2. REFER TO DRAWING C108 FOR PIT SCHEDULE

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TITLE
STORMWATER PLAN SHEET 6

PROJECT
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 AT; LOT 412, DP 1063902,
 No.507 MEDOWIE ROAD,
 MEDOWIE**

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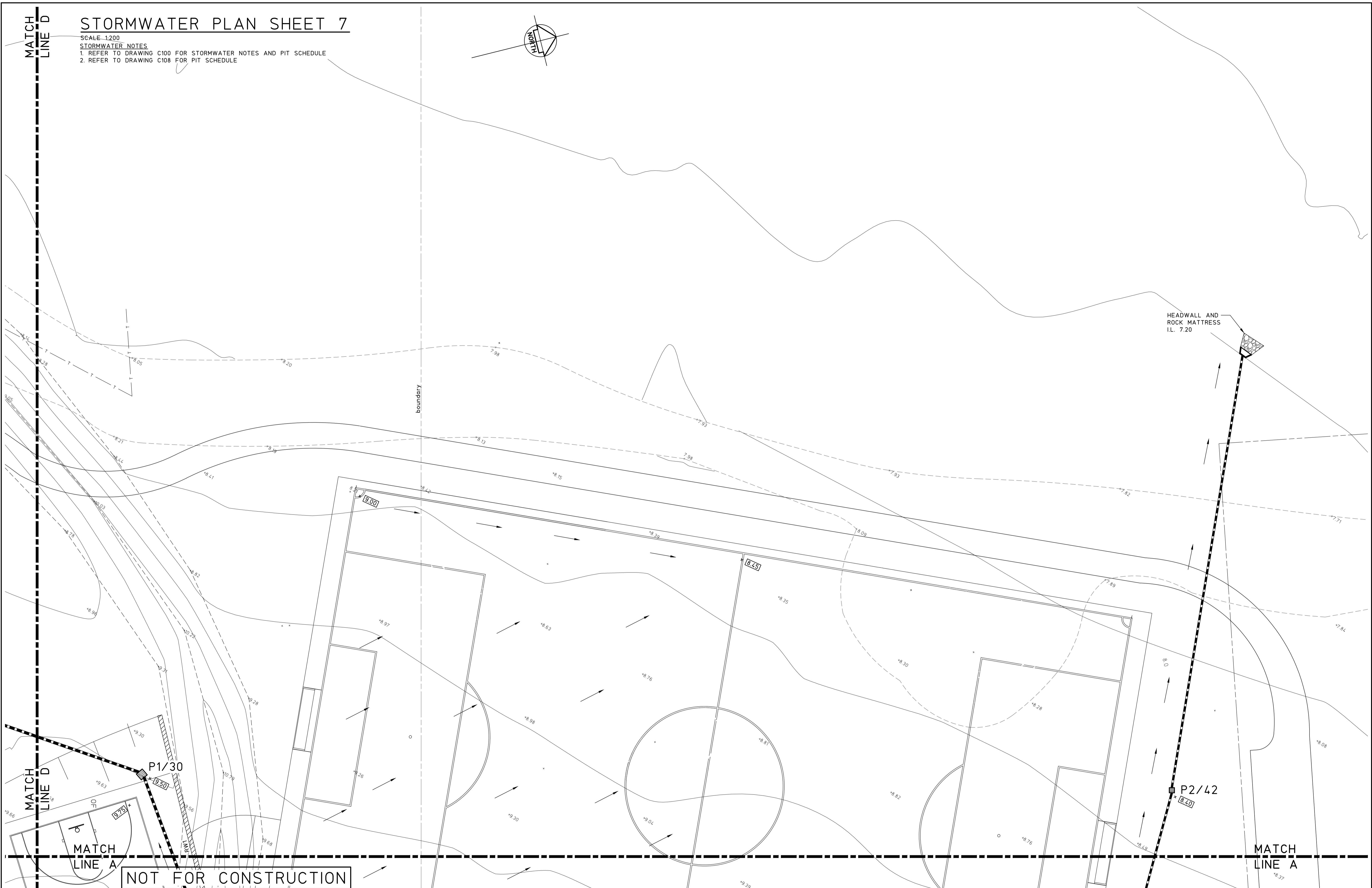
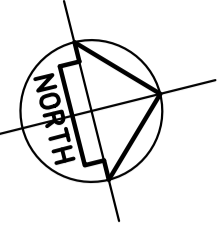
MATCH LINE D

STORMWATER PLAN SHEET 7

SCALE 1:200

STORMWATER NOTES

1. REFER TO DRAWING C100 FOR STORMWATER NOTES AND PIT SCHEDULE
2. REFER TO DRAWING C108 FOR PIT SCHEDULE



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TITLE
STORMWATER PLAN SHEET 7

PROJECT
CATHERINE McAULEY CATHOLIC COLLEGE AT; LOT 412, DP 1063902, No.507 MEDOWIE ROAD, MEDOWIE

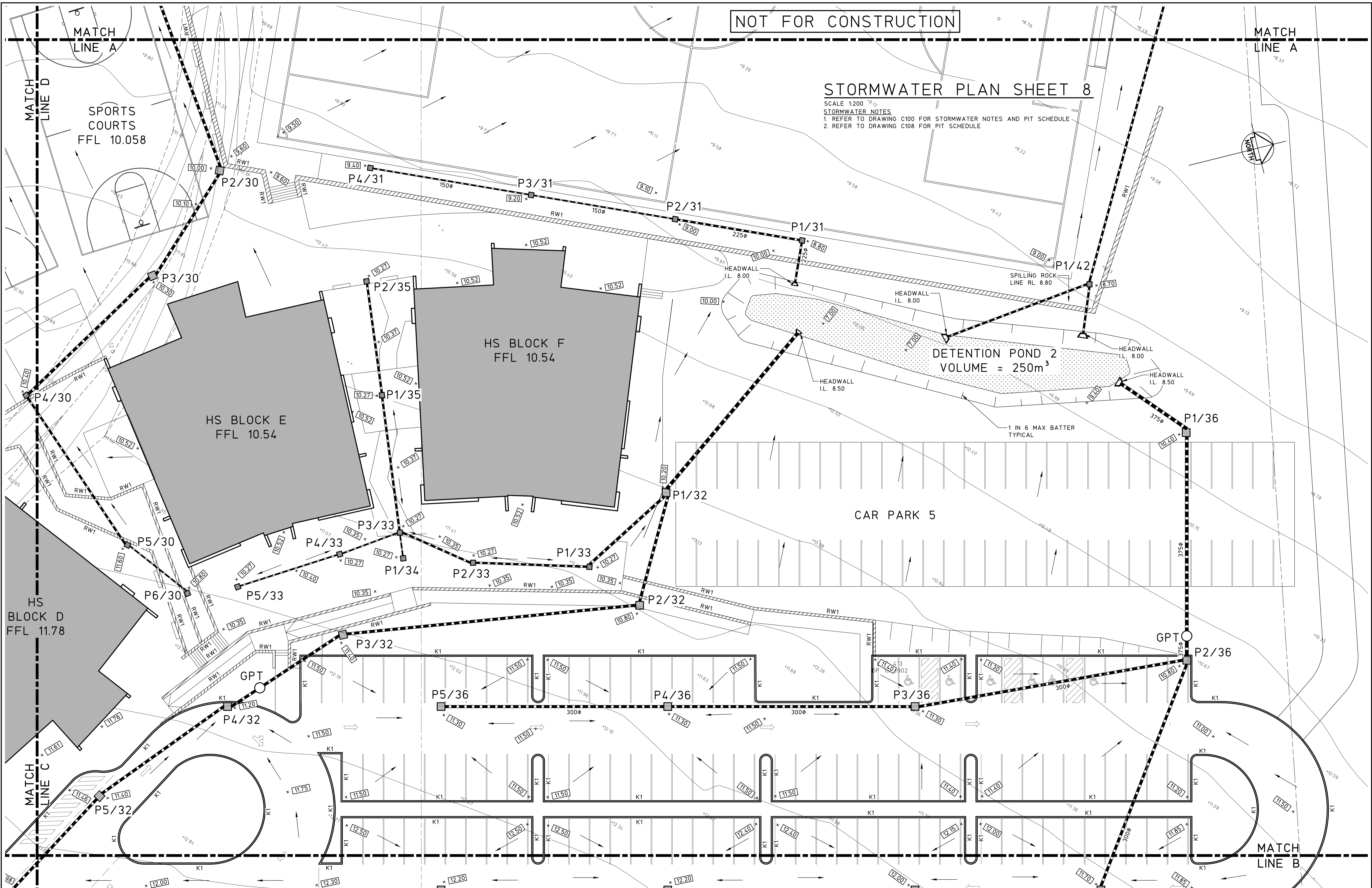
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SCALES	JOB No	DRAWING No	ISSUE
1:200	17-828	C106	3

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STORMWATER PLAN SHEET 8

SCALE 1:200
STORMWATER NOTES
1. REFER TO DRAWING C100 FOR STORMWATER NOTES AND PIT SCHEDULE
2. REFER TO DRAWING C108 FOR PIT SCHEDULE



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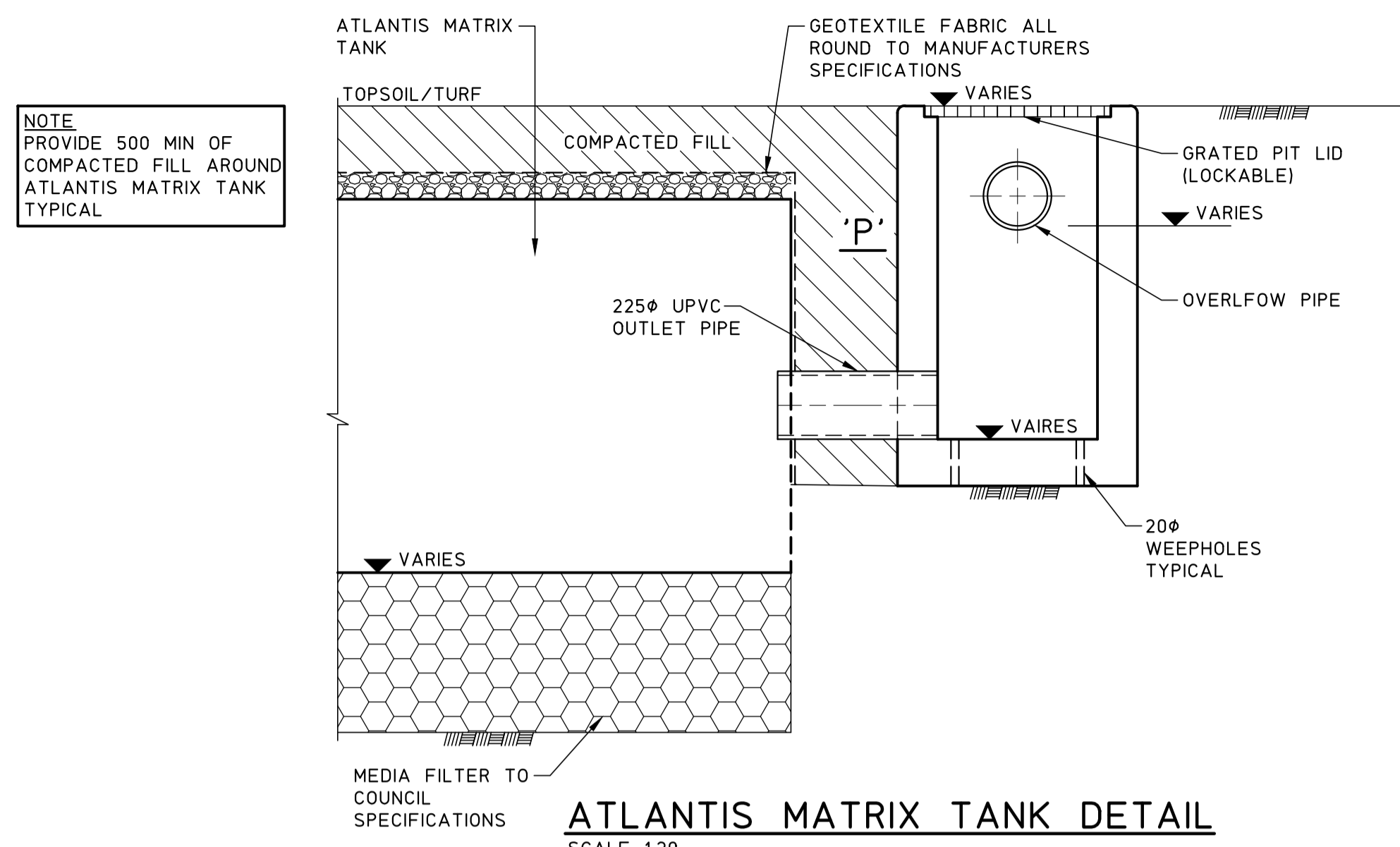
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TITLE
STORMWATER PLAN SHEET 8

PROJECT
**CATHERINE McAULEY CATHOLIC COLLEGE
AT; LOT 412, DP 1063902,
No.507 MEDOWIE ROAD,
MEDOWIE**

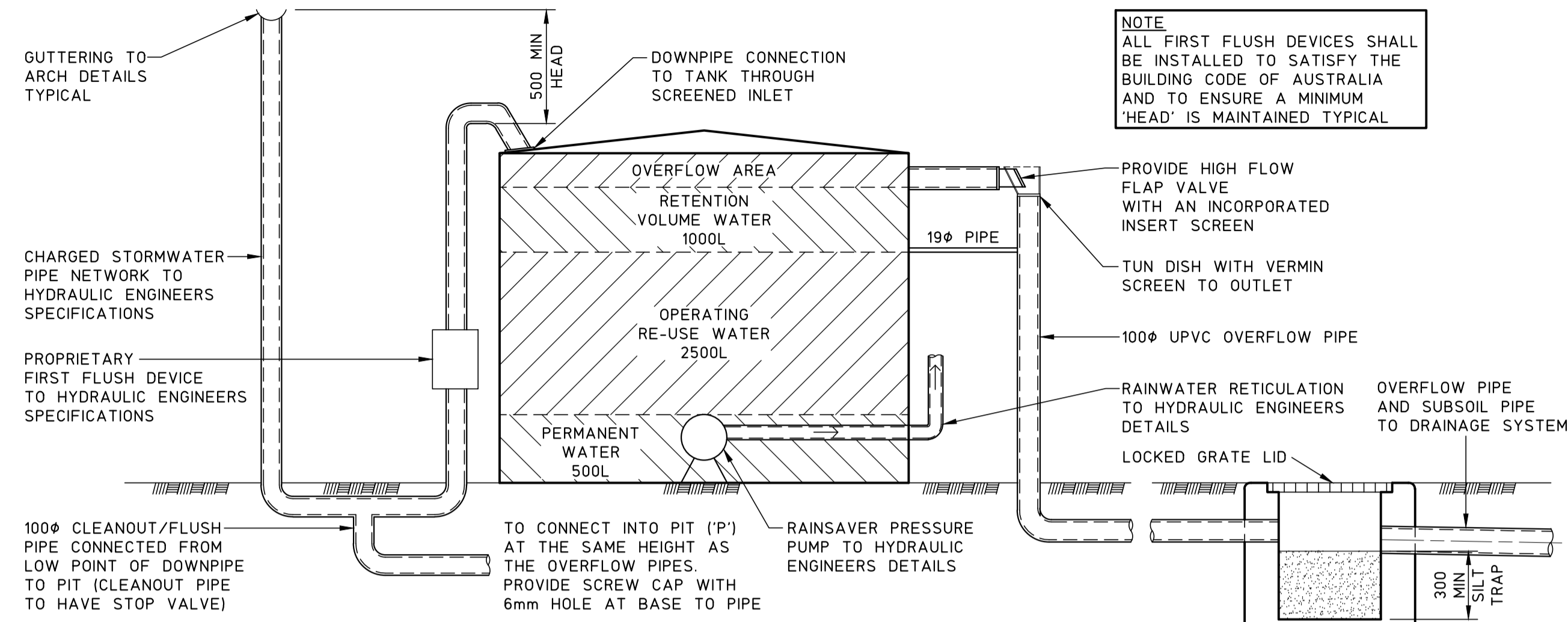
DO NOT SCALE DRAWING

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SCALES	JOB No	DRAWING No	ISSUE
1:200	17-828	C107	3

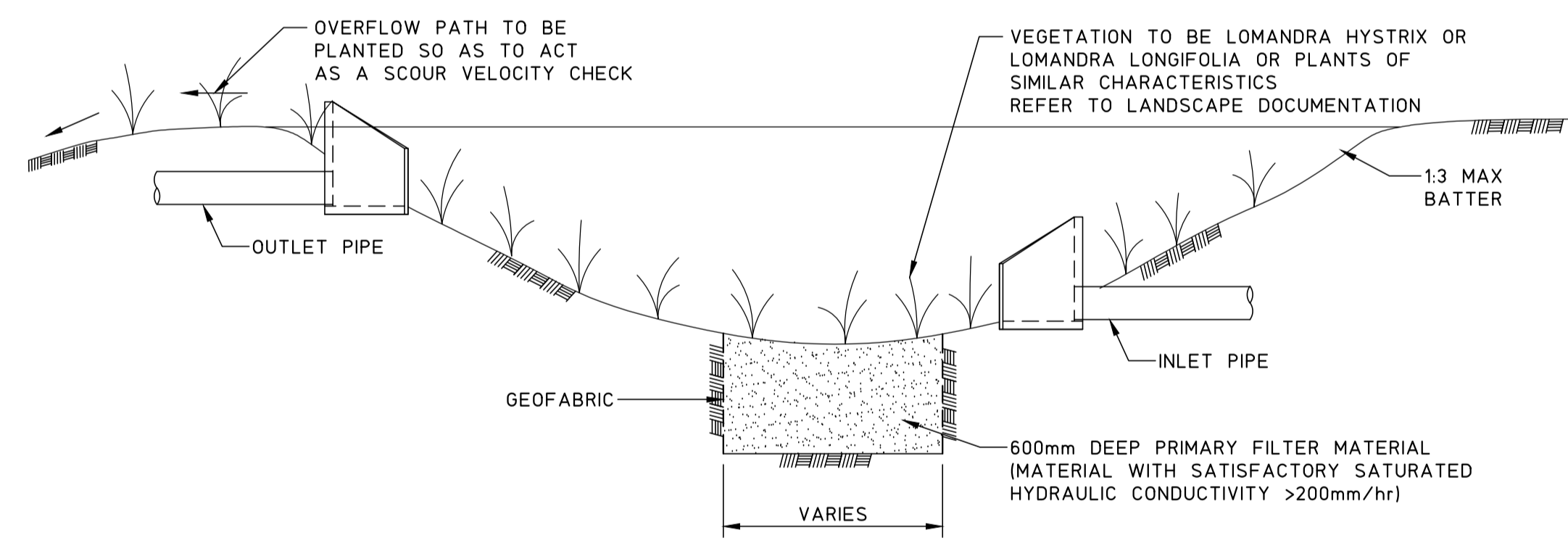


ATLANTIS MATRIX TANK DETAIL
SCALE 1:20

- ATLANTIS MATRIX TANK NOTES**
1. TRENCHING SHALL BE CLEAR OF STRUCTURAL FOUNDATIONS WITHIN THE RANGE OF 1m (MIN) IN CLEAN SAND AND 5m (MIN) IN CLAY.
 2. THE TRENCHING SHALL BE PLACED LEVEL ALONG THE CONTOUR OF THE NATURAL OR FINISHED SURFACE.
 3. THE TRENCHING SHALL BE PLACED WITHIN THE PROPERTY TO ACHIEVE MAX. AREA, SLOPING AWAY FROM THE TRENCH, FOR DISPOSAL OF WATER.
 4. IT IS THE OWNERS RESPONSIBILITY TO REGULARLY CLEAN THE PIT AND MAINTAIN THE SYSTEM.
 5. PROVIDE 150Ø INSPECTION POINTS IN ACCORDANCE WITH MANUFACTURERS SPECIFICATION



TYPICAL TANK (T1) DETAIL
SCALE 1:20



TYPICAL DETENTION POND DETAIL
SCALE 1:50

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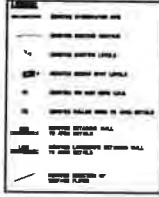
Appendix C

Catchment Areas and Summary of Stormwater Design Intent

CATCHMENT AREA PLAN

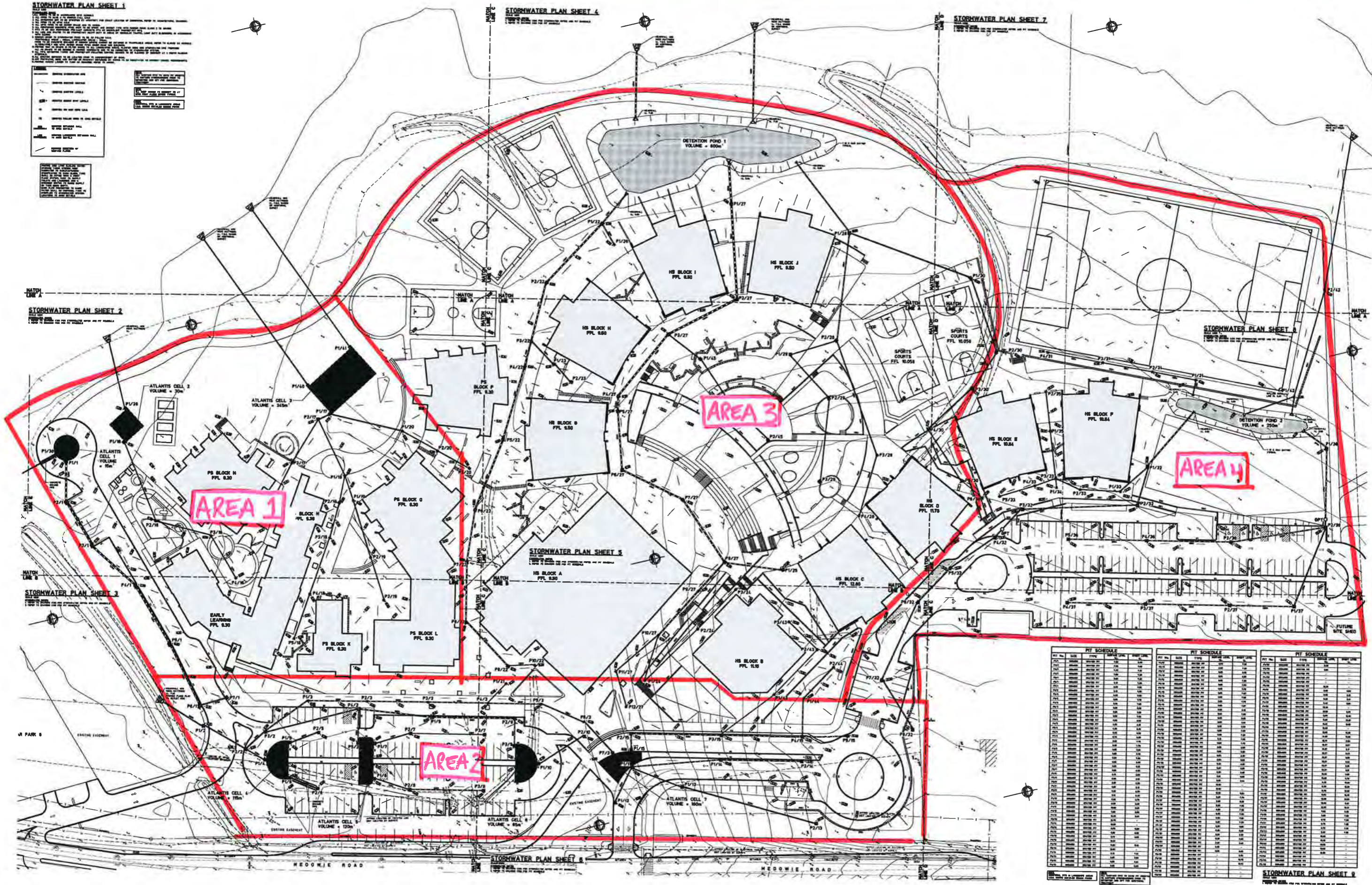
MPC REF: 17-828.

STORMWATER PLAN SHEET 1
THIS SHEET SHOWS THE GENERAL LAYOUT OF STORMWATER SYSTEMS IN THIS AREA. THE EXACT LOCATION OF STORMWATER SYSTEMS IS INDICATED BY THE DASHED LINES. THE EXACT LOCATION OF STORMWATER SYSTEMS IS INDICATED BY THE DASHED LINES. THE EXACT LOCATION OF STORMWATER SYSTEMS IS INDICATED BY THE DASHED LINES.



STORMWATER PLAN SHEET 4

STORMWATER PLAN SHEET 7



PIT SCHEDULE			
PIT NO.	TYPE	DEPTH (M)	DIAMETER (M)
P1/10	MANHOLE	1.0	0.75
P1/11	MANHOLE	1.0	0.75
P1/12	MANHOLE	1.0	0.75
P1/13	MANHOLE	1.0	0.75
P1/14	MANHOLE	1.0	0.75
P1/15	MANHOLE	1.0	0.75
P1/16	MANHOLE	1.0	0.75
P1/17	MANHOLE	1.0	0.75
P1/18	MANHOLE	1.0	0.75
P1/19	MANHOLE	1.0	0.75
P1/20	MANHOLE	1.0	0.75
P1/21	MANHOLE	1.0	0.75
P1/22	MANHOLE	1.0	0.75
P1/23	MANHOLE	1.0	0.75
P1/24	MANHOLE	1.0	0.75
P1/25	MANHOLE	1.0	0.75
P1/26	MANHOLE	1.0	0.75
P1/27	MANHOLE	1.0	0.75
P1/28	MANHOLE	1.0	0.75
P1/29	MANHOLE	1.0	0.75
P1/30	MANHOLE	1.0	0.75
P1/31	MANHOLE	1.0	0.75
P1/32	MANHOLE	1.0	0.75
P1/33	MANHOLE	1.0	0.75
P1/34	MANHOLE	1.0	0.75
P1/35	MANHOLE	1.0	0.75
P1/36	MANHOLE	1.0	0.75
P1/37	MANHOLE	1.0	0.75
P1/38	MANHOLE	1.0	0.75
P1/39	MANHOLE	1.0	0.75
P1/40	MANHOLE	1.0	0.75
P1/41	MANHOLE	1.0	0.75
P1/42	MANHOLE	1.0	0.75
P1/43	MANHOLE	1.0	0.75
P1/44	MANHOLE	1.0	0.75
P1/45	MANHOLE	1.0	0.75
P1/46	MANHOLE	1.0	0.75
P1/47	MANHOLE	1.0	0.75
P1/48	MANHOLE	1.0	0.75
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P1/60	MANHOLE	1.0	0.75
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P1/62	MANHOLE	1.0	0.75
P1/63	MANHOLE	1.0	0.75
P1/64	MANHOLE	1.0	0.75
P1/65	MANHOLE	1.0	0.75
P1/66	MANHOLE	1.0	0.75
P1/67	MANHOLE	1.0	0.75
P1/68	MANHOLE	1.0	0.75
P1/69	MANHOLE	1.0	0.75
P1/70	MANHOLE	1.0	0.75
P1/71	MANHOLE	1.0	0.75
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P1/99	MANHOLE	1.0	0.75
P1/100	MANHOLE	1.0	0.75

STORMWATER PLAN SHEET 9

Appendix D

Erosion and Sediment Control Plan and Calculations

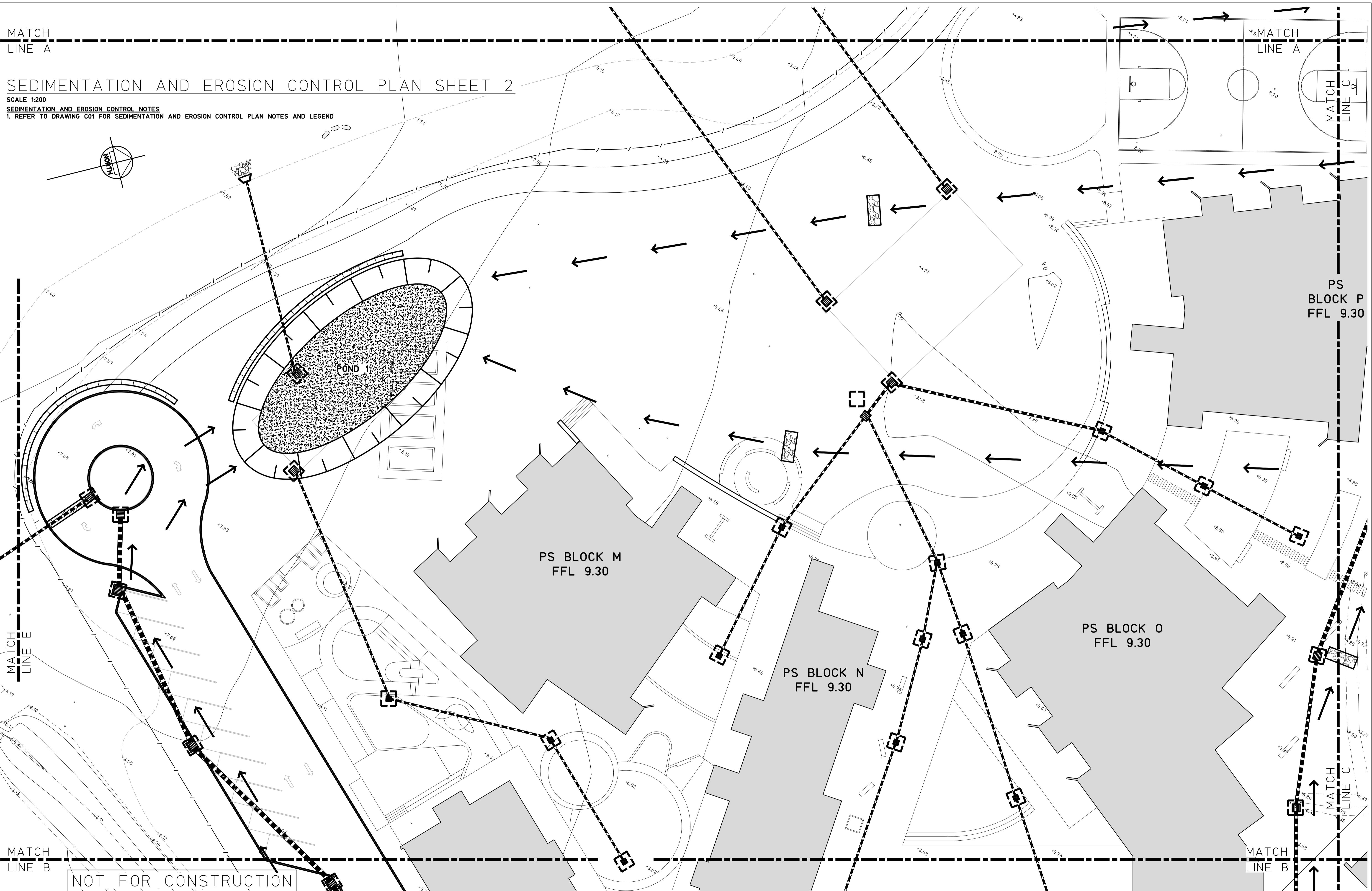
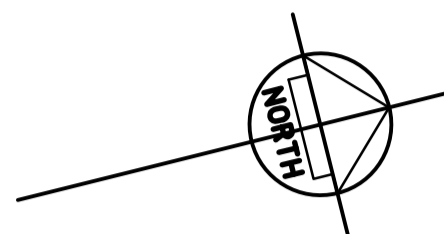
MATCH
LINE A

SEDIMENTATION AND EROSION CONTROL PLAN SHEET 2

SCALE 1:200

SEDIMENTATION AND EROSION CONTROL NOTES

1. REFER TO DRAWING C01 FOR SEDIMENTATION AND EROSION CONTROL PLAN NOTES AND LEGEND



MATCH
LINE B

NOT FOR CONSTRUCTION

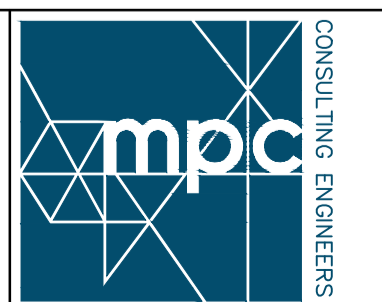
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TITLE
SEDIMENTATION AND
EROSION CONTROL PLAN SHEET 2

PROJECT
CATHERINE McAULEY CATHOLIC COLLEGE
AT; LOT 412, DP 1063902,
No.507 MEDOWIE ROAD,
MEDOWIE

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FULL SIZE ON ORIGINAL 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 cm

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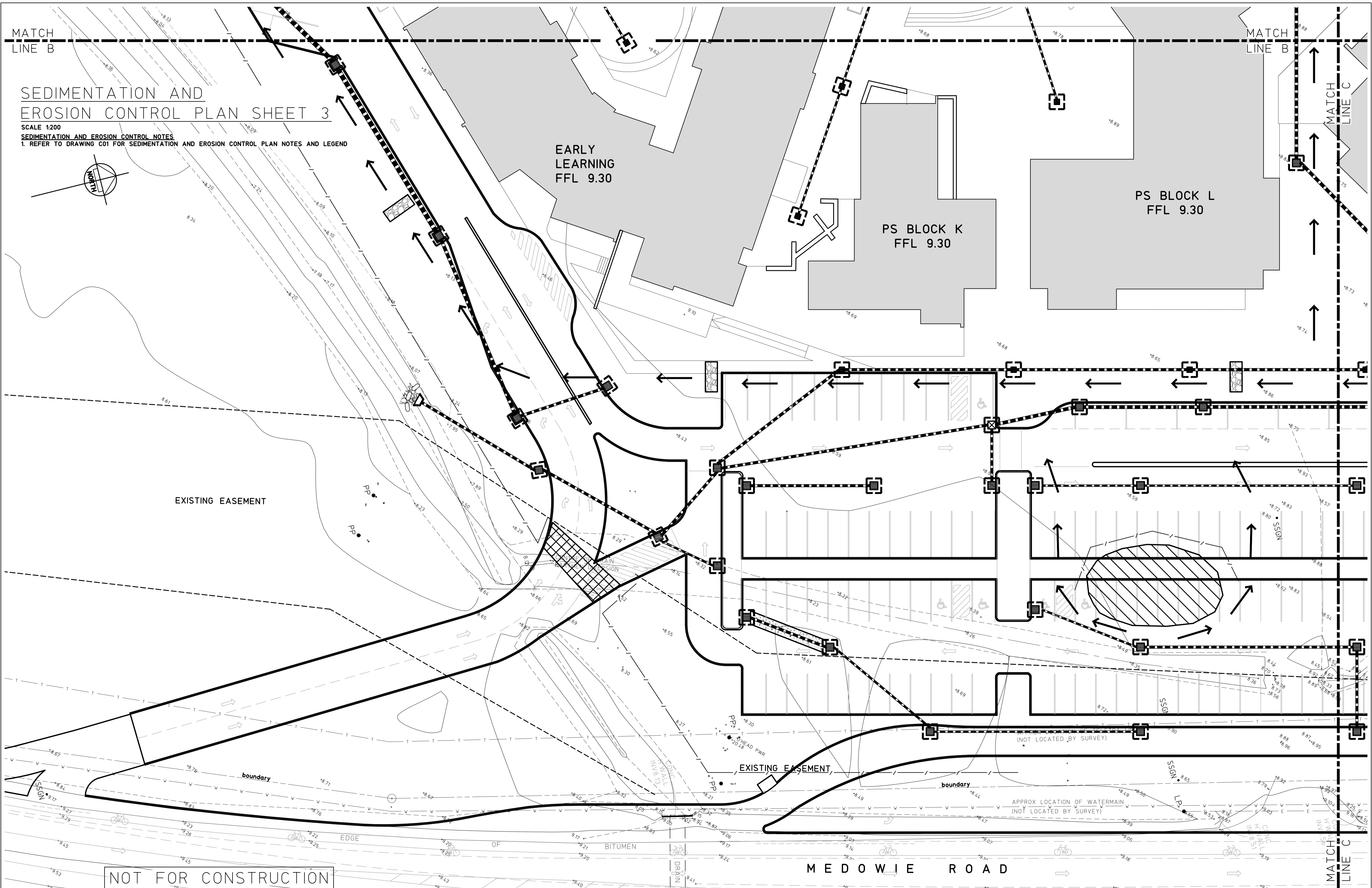
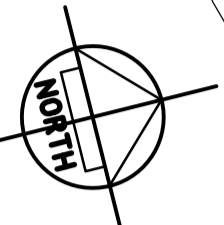
MATCH LINE B

MATCH LINE C

MATCH LINE C

SEDIMENTATION AND EROSION CONTROL PLAN SHEET 3

SCALE 1:200
SEDIMENTATION AND EROSION CONTROL NOTES
1. REFER TO DRAWING C01 FOR SEDIMENTATION AND EROSION CONTROL PLAN NOTES AND LEGEND



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No.507 MEDOWIE ROAD,
MEDOWIE

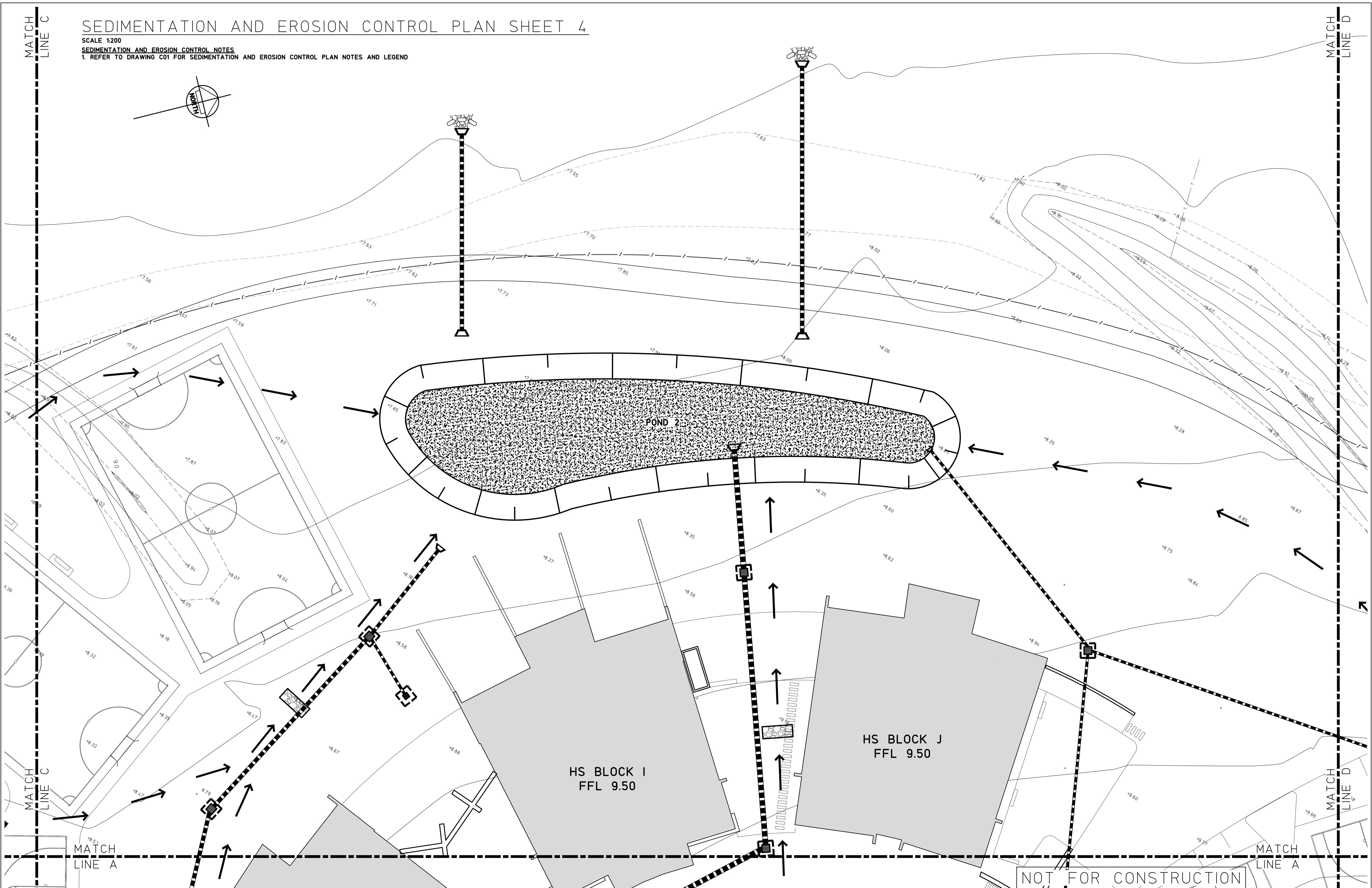
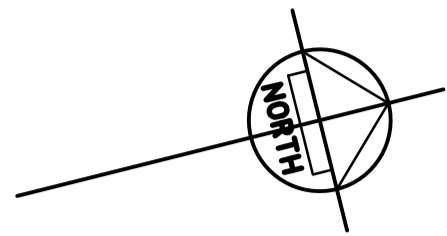
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SCALES 1:200	JOB No 17-828	DRAWING No C04	ISSUE 2

FULL SIZE ON ORIGINAL 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 cm

SEDIMENTATION AND EROSION CONTROL PLAN SHEET 4

SCALE 1:200

SEDIMENTATION AND EROSION CONTROL NOTES
1. REFER TO DRAWING C01 FOR SEDIMENTATION AND EROSION CONTROL PLAN NOTES AND LEGEND



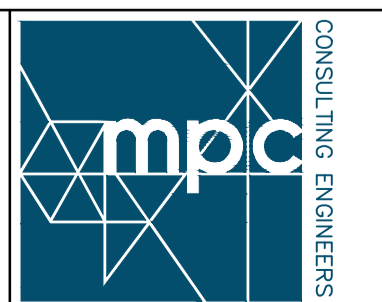
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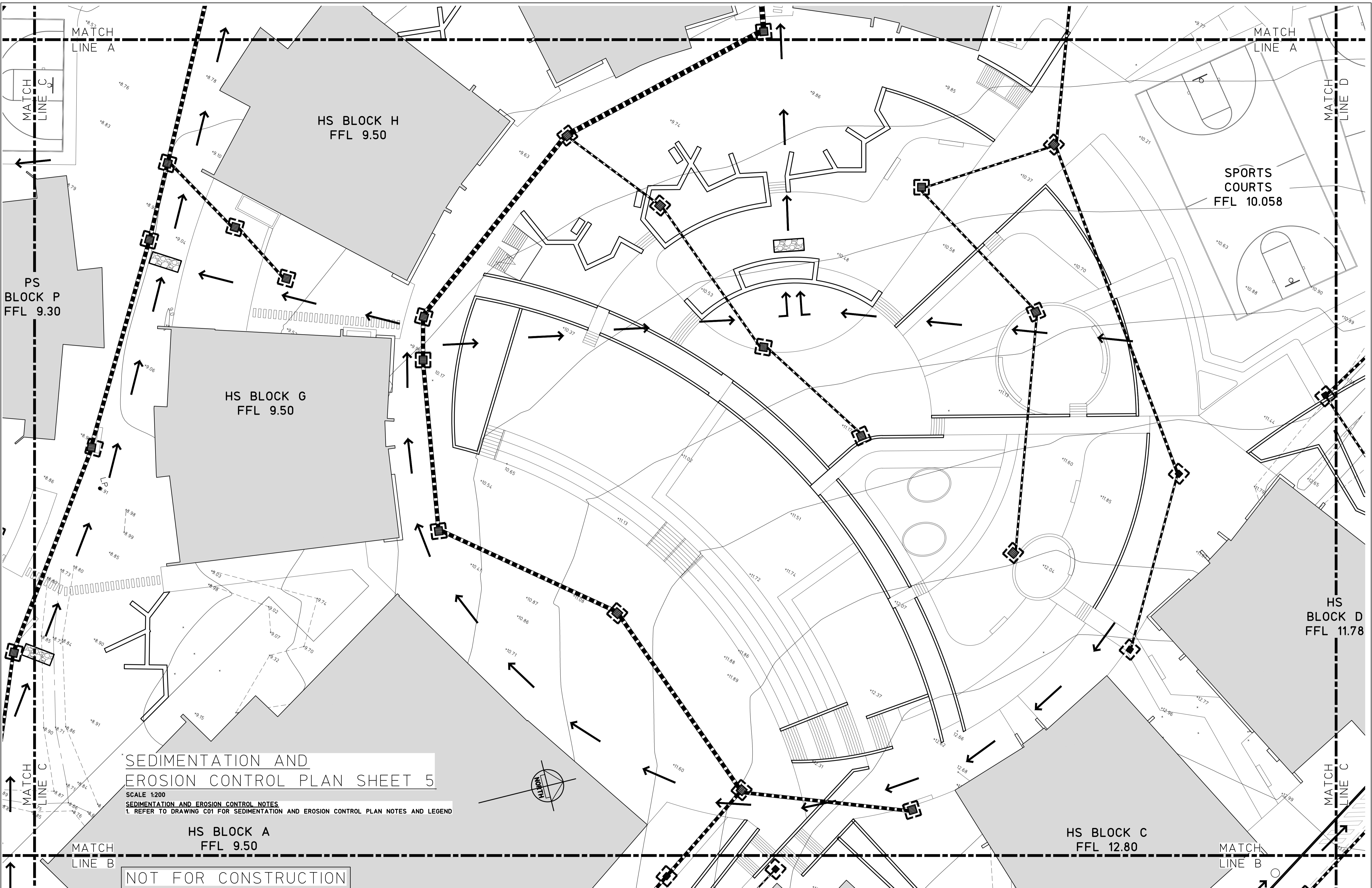
CLIENT
CATHOLIC SCHOOLS OFFICE

TITLE
SEDIMENTATION AND
EROSION CONTROL PLAN SHEET 4

PROJECT
CATHERINE McAULEY CATHOLIC COLLEGE
AT; LOT 412, DP 1063902,
No.507 MEDOWIE ROAD,
MEDOWIE

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FULL SIZE ON ORIGINAL 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 cm



SEDIMENTATION AND EROSION CONTROL PLAN SHEET 5

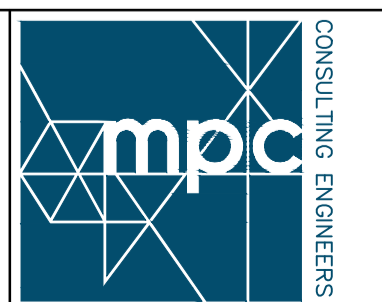
SCALE 1:200
 SEDIMENTATION AND EROSION CONTROL NOTES
 1. REFER TO DRAWING C01 FOR SEDIMENTATION AND EROSION CONTROL PLAN NOTES AND LEGEND

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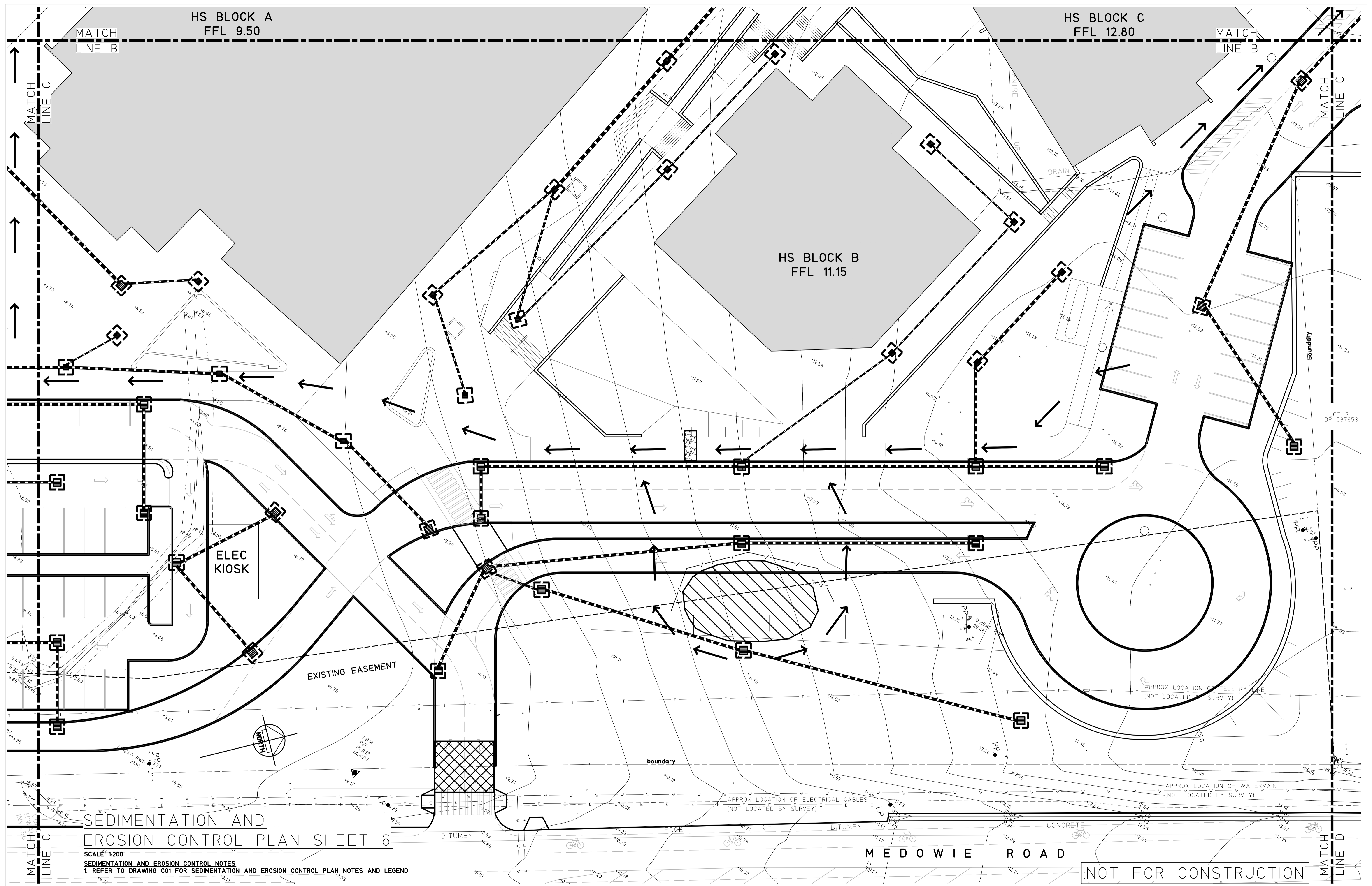
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 16 Telford Street,
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TITLE
 SEDIMENTATION AND EROSION CONTROL PLAN SHEET 5

PROJECT
 CATHERINE McAULEY CATHOLIC COLLEGE
 AT; LOT 412, DP 1063902,
 No.507 MEDOWIE ROAD,
 MEDOWIE

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SEDIMENTATION AND EROSION CONTROL PLAN SHEET 6

SCALE 1:200
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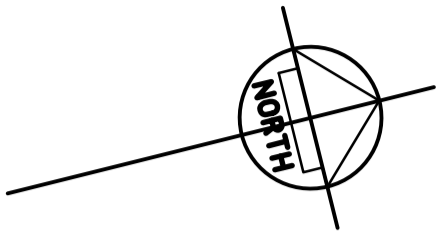
PROJECT
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 AT; LOT 412, DP 1063902,
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SCALES 1:200	JOB No 17-828	DRAWING No C07	ISSUE 2

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SEDIMENTATION AND EROSION CONTROL PLAN SHEET 7

SCALE 1:200
SEDIMENTATION AND EROSION CONTROL NOTES
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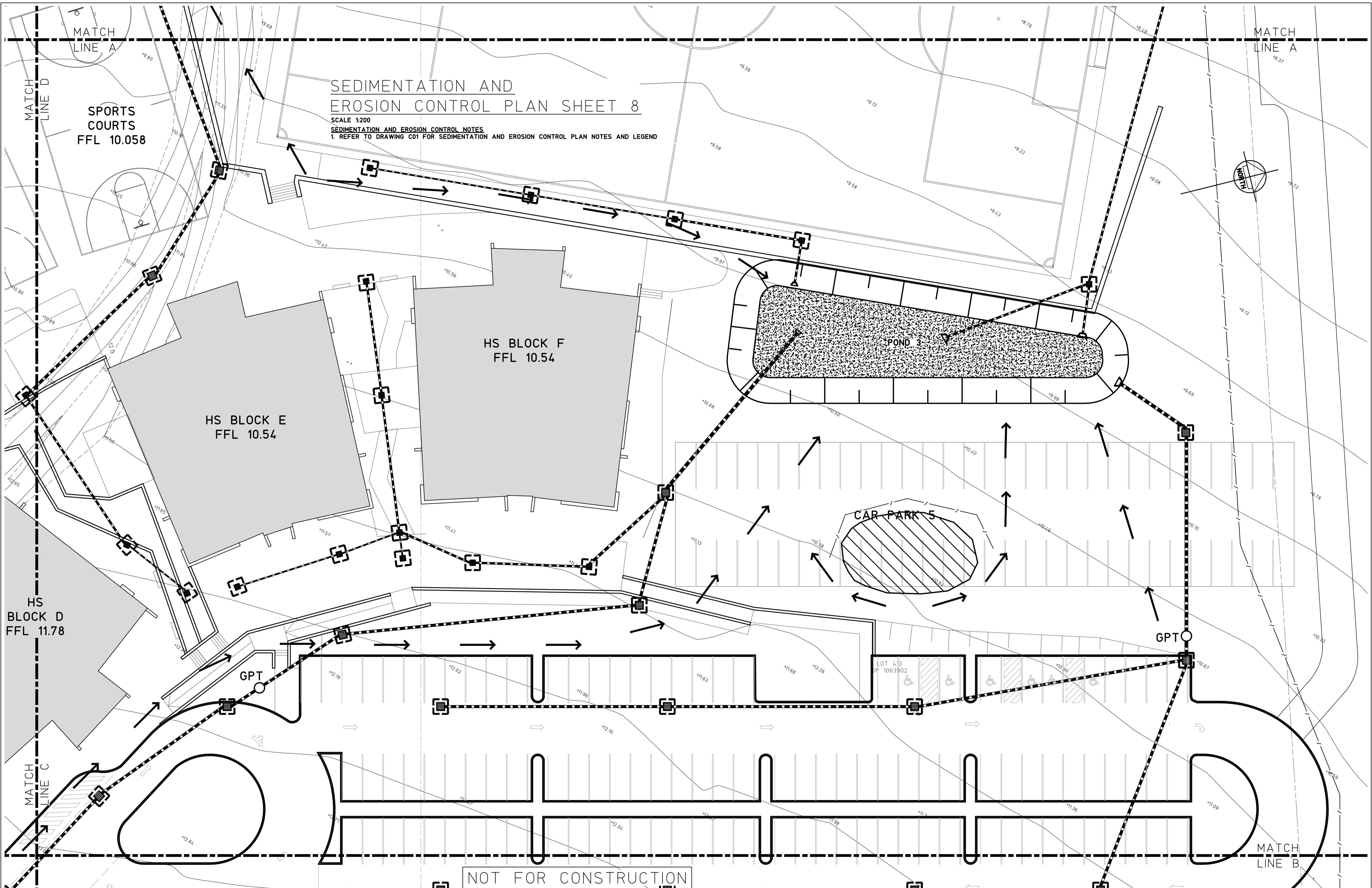
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TITLE
SEDIMENTATION AND EROSION CONTROL PLAN SHEET 7

PROJECT
CATHERINE MCAULEY CATHOLIC COLLEGE
AT; LOT 412, DP 1063902,
No.507 MEDOWIE ROAD,
MEDOWIE

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SCALES 1:200	JOB No 17-828	DRAWING No C08	ISSUE 2

FULL SIZE ON ORIGINAL 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 cm



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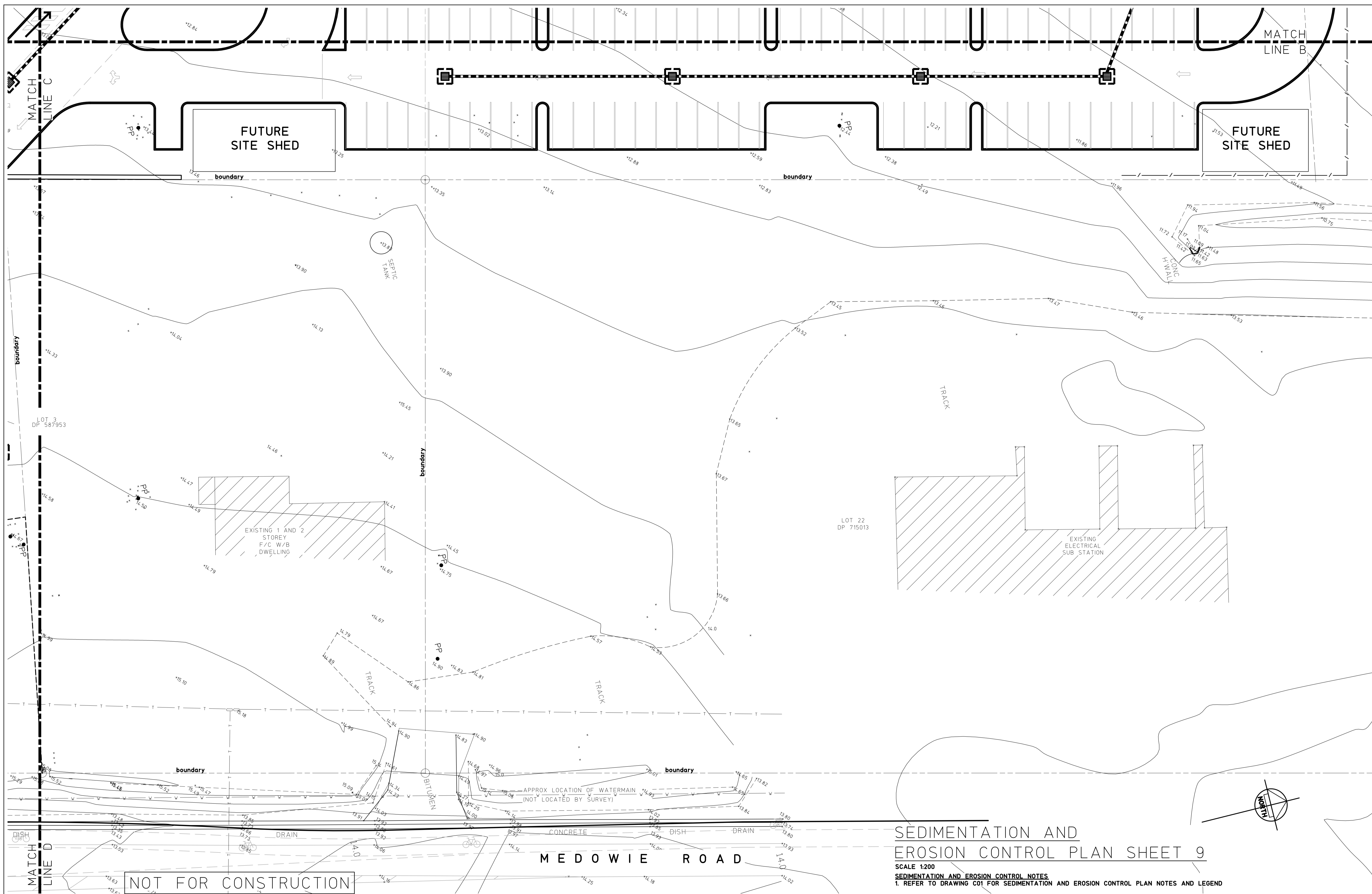
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TITLE
SEDIMENTATION AND EROSION CONTROL PLAN SHEET 8

PROJECT
CATHERINE McAULEY CATHOLIC COLLEGE
AT; LOT 412, DP 1063902,
No.507 MEDOWIE ROAD,
MEDOWIE

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SCALES 1:200	JOB No 17-828	DRAWING No C09	ISSUE 2

FULL SIZE ON ORIGINAL 0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 cm



SEDIMENTATION AND EROSION CONTROL PLAN SHEET 9

SCALE 1:200
 SEDIMENTATION AND EROSION CONTROL NOTES
 1. REFER TO DRAWING C01 FOR SEDIMENTATION AND EROSION CONTROL PLAN NOTES AND LEGEND

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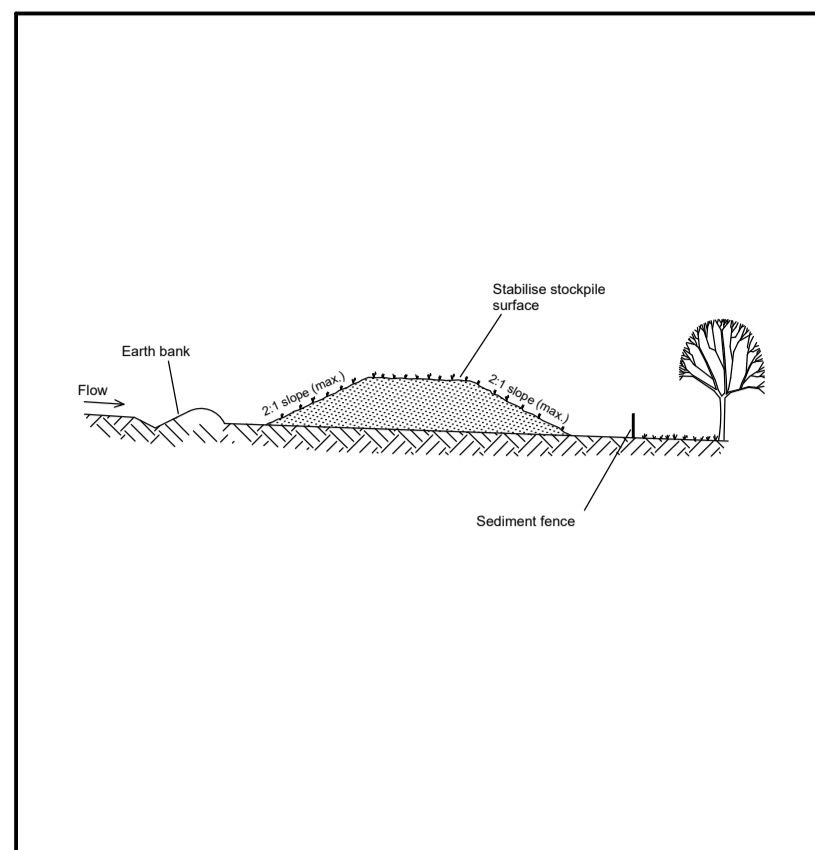
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 CATHERINE McAULEY CATHOLIC COLLEGE
 AT; LOT 412, DP 1063902,
 No.507 MEDOWIE ROAD,
 MEDOWIE

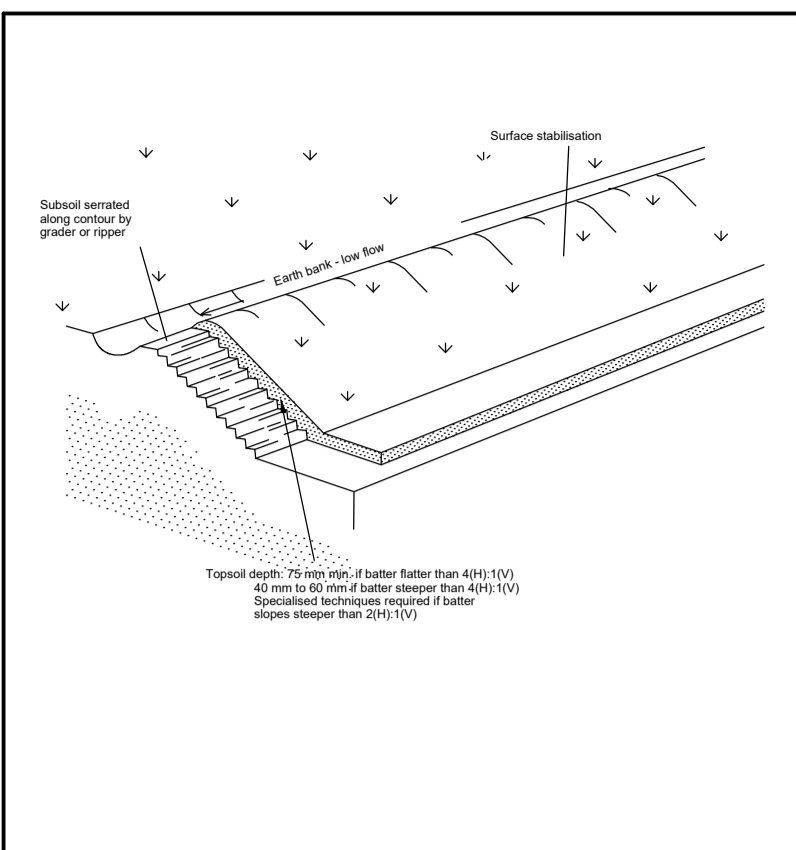
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SCALES 1:200	JOB No 17-828	DRAWING No C10	ISSUE 2



Construction Notes

- Place stockpiles more than 2 (preferably 5) metres from existing vegetation, concentrated water flow, roads and hazard areas.
- Construct on the contour as low, flat, elongated mounds.
- Where there is sufficient area, topsoil stockpiles shall be less than 2 metres in height.
- Where they are to be in place for more than 10 days, stabilise following the approved ESCP or SWMP to reduce the C-factor to less than 10.
- Construct earth banks (Standard Drawing 5-4) on the upslope side to divert water around stockpiles and sediment fences (Standard Drawing 6-8) 1 to 2 metres downslope.

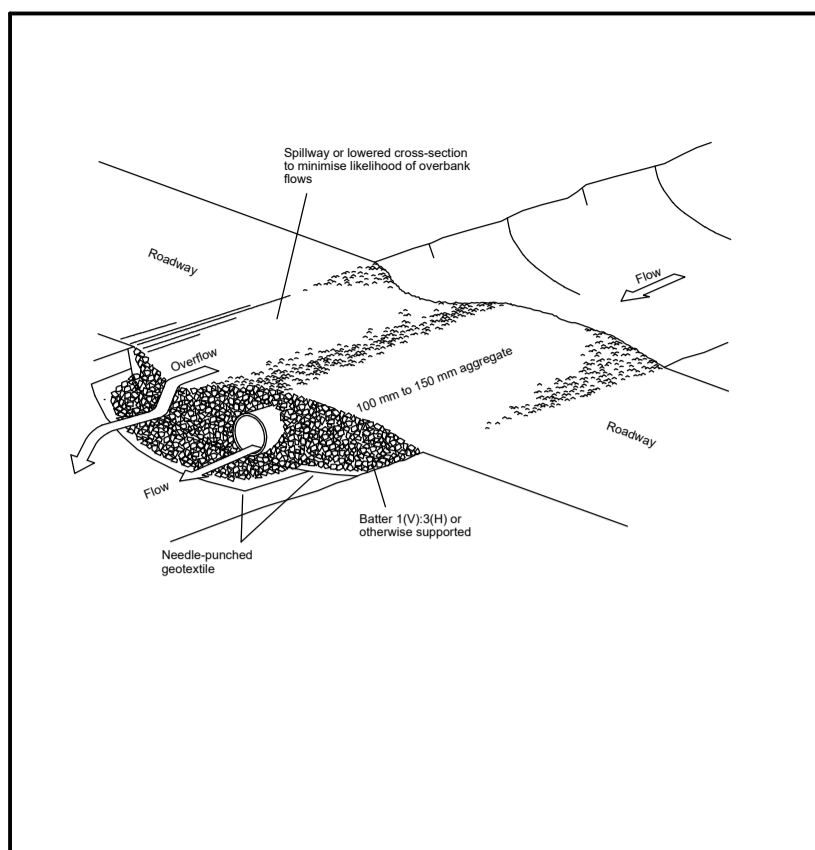
STOCKPILES SD 4-1



Construction Notes

- Scarify the ground surface along the line of the contour to a depth of 50 mm to 100 mm to break up any hardsetting surfaces and to provide a good bond between the respread material and subsoil.
- Add soil ameliorants as required by the ESCP or SWMP.
- Rip to a depth of 300 mm if compacted layers occur.
- Where possible, replace topsoil to a depth of 40 to 60 mm on lands where the slope exceeds 4(H):1(V) and to at least 75 mm on lower gradients.

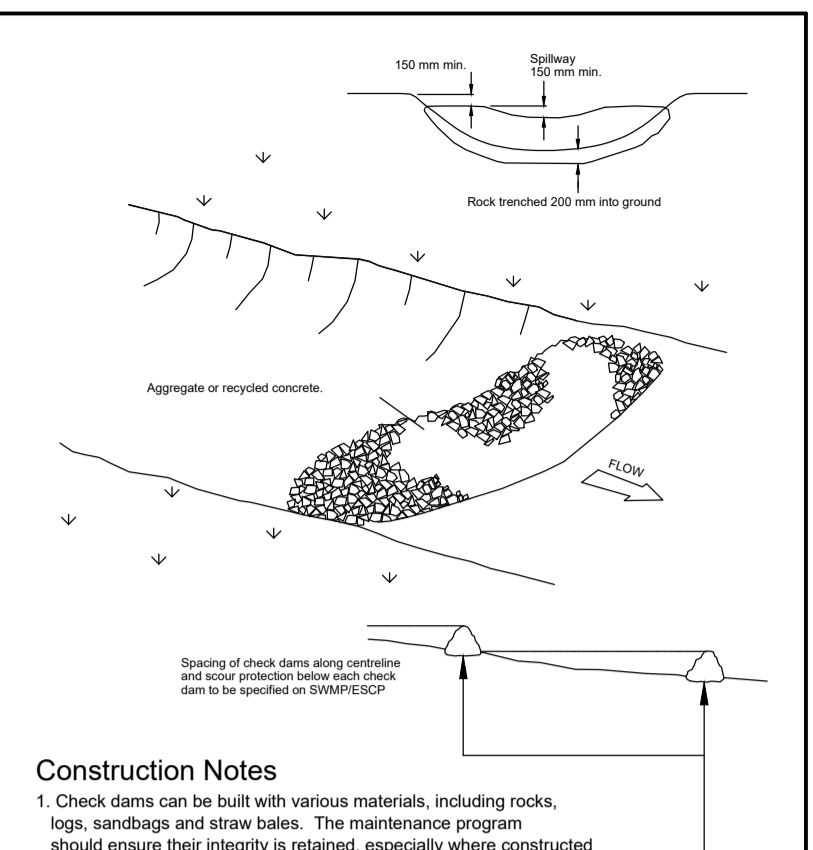
REPLACING TOPSOIL SD 4-2



Construction Notes

- Prohibit all traffic until the access way is constructed.
- Strip any topsoil and place a needle-punched textile over the base of the crossing.
- Place clean, rigid, non polluting aggregate or gravel in the 100 mm to 150 mm size class over the fabric to a minimum depth of 200 mm.
- Provide a 3(H):1 (V) slope on side batters.
- Install a lower section to act as an emergency spillway in greater than 1:1 slopes.
- Ensure that culvert outlets extend beyond the toe of fill embankments.

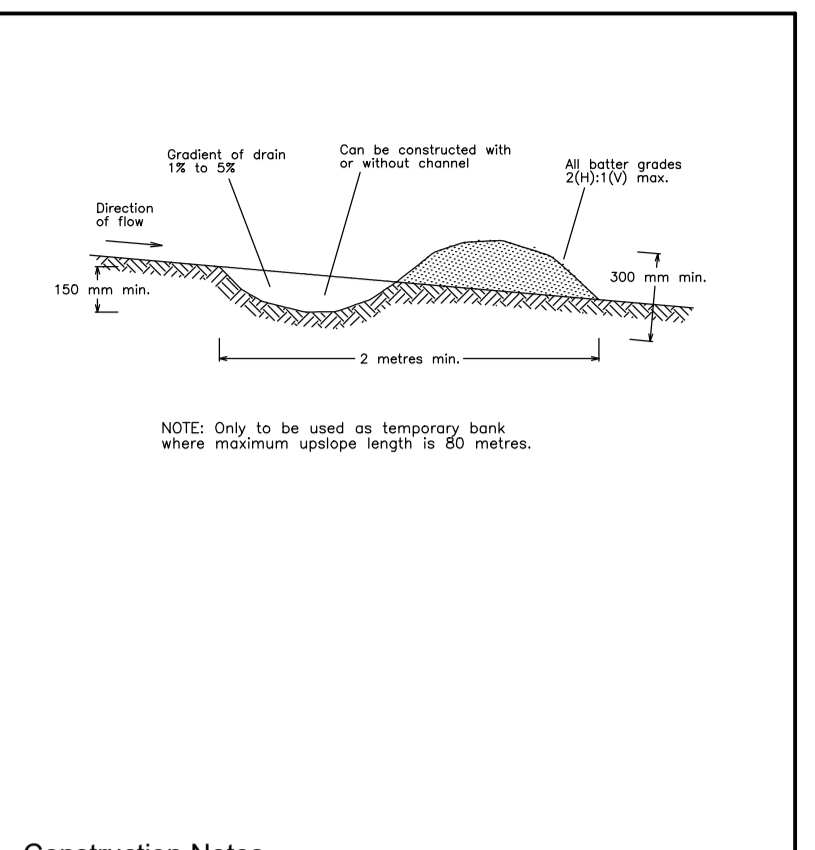
TEMPORARY WATERWAY CROSSING SD 5-1



Construction Notes

- Check dams can be built with various materials, including rocks, logs, sandbags and straw bales. The maintenance program should ensure their integrity is retained, especially where constructed with straw bales. In the case of bales, this might require their replacement each two to four months.
- Trench the check dam 200 mm into the ground across its whole width. Where rock is used, fill the trenches to at least 100 mm above the ground surface to reduce the risk of undercutting.
- Normally, their maximum height should not exceed 600 mm above the gully floor. The centre should act as a spillway, being at least 150 mm lower than the outer edges.
- Space the dams so the toe of the upstream dam is level with the spillway of the next downstream dam.

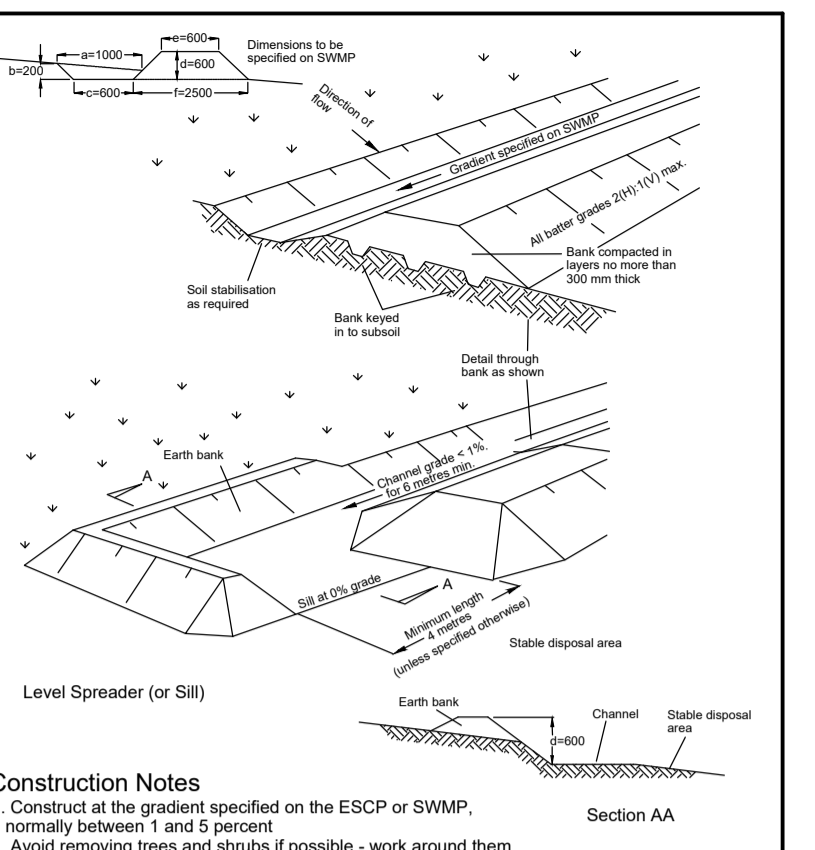
ROCK CHECK DAM SD 5-4



Construction Notes

- Build with gradients between 1 percent and 5 percent.
- Avoid removing trees and shrubs if possible - work around them.
- Ensure the structures are free of projections or other irregularities that could impede water flow.
- Build the drains with circular, parabolic or trapezoidal cross sections, not V-shaped, at the dimensions shown on the SWMP.
- Ensure the banks are properly compacted to prevent failure.
- Complete permanent or temporary stabilisation within 10 days of construction following Table 5.2 in Landcom (2004).
- Where discharging to erodible lands, ensure they outlet through a properly constructed level spreader.
- Construct the level spreader at the gradient specified on the ESCP or SWMP, normally less than 1 percent or level.
- Where possible, ensure they discharge waters onto either stabilised or undisturbed disposal sites within the same subcatchment area from which the water originated. Approval might be required to discharge into other subcatchments.

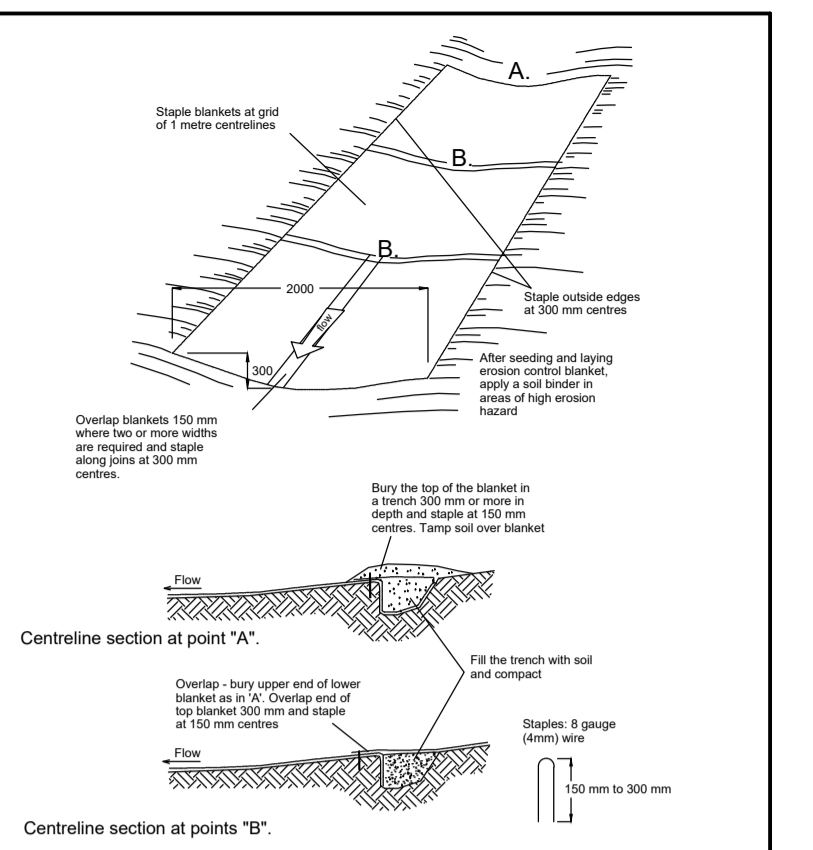
EARTH BANK (LOW FLOW) SD 5-5



Construction Notes

- Construct at the gradient specified on the ESCP or SWMP, normally between 1 and 5 percent.
- Avoid removing trees and shrubs if possible - work around them.
- Ensure the structures are free of projections or other irregularities that could impede water flow.
- Build the drains with circular, parabolic or trapezoidal cross sections, not V-shaped, at the dimensions shown on the SWMP.
- Ensure the banks are properly compacted to prevent failure.
- Complete permanent or temporary stabilisation within 10 days of construction following Table 5.2 in Landcom (2004).
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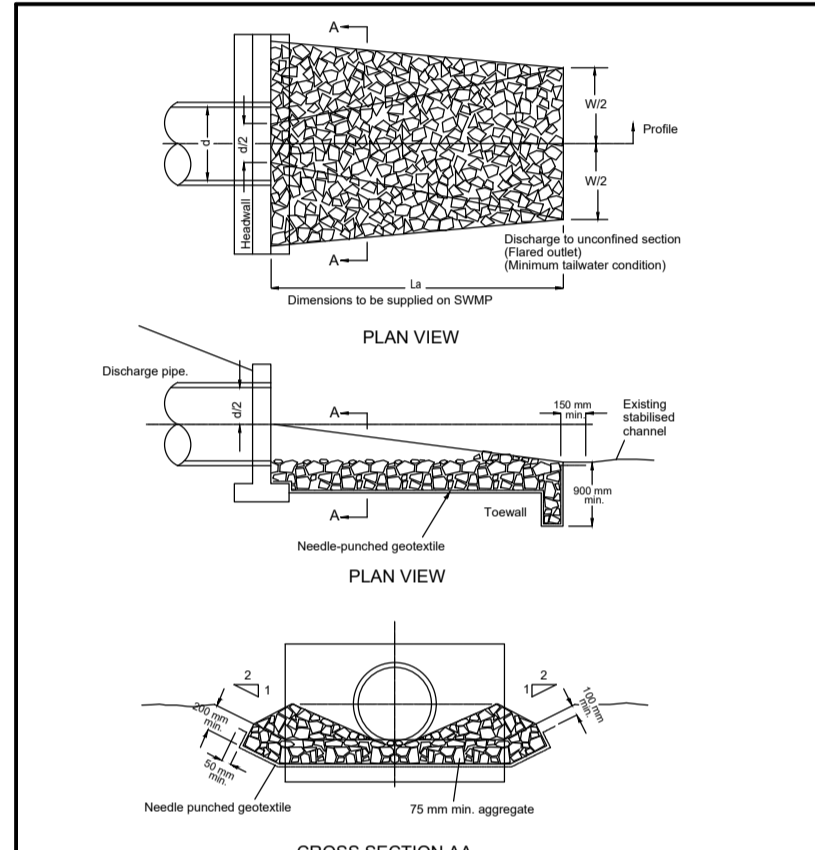
EARTH BANK (HIGH FLOWS) SD 5-6



Construction Notes

- Remove any rocks, clods, sticks or grass from the surface before laying matting.
- Ensure that topsoil is at least 75 mm deep.
- Complete fertilising and seeding before laying the matting.
- Ensure fabric will be continuously in contact with the soil by grading the surface carefully first.
- Lay the fabric in 'single-batten', with the end of each upstream roll overlapping those downstream. Ensure each roll is anchored properly at its upslope end.
- Ensure that the full width of flow in the channel is covered by the matting up to the design storm event, usually in the 10-year ARI time of concentration storm event.
- Divert water from the structure until vegetation is stabilised properly.

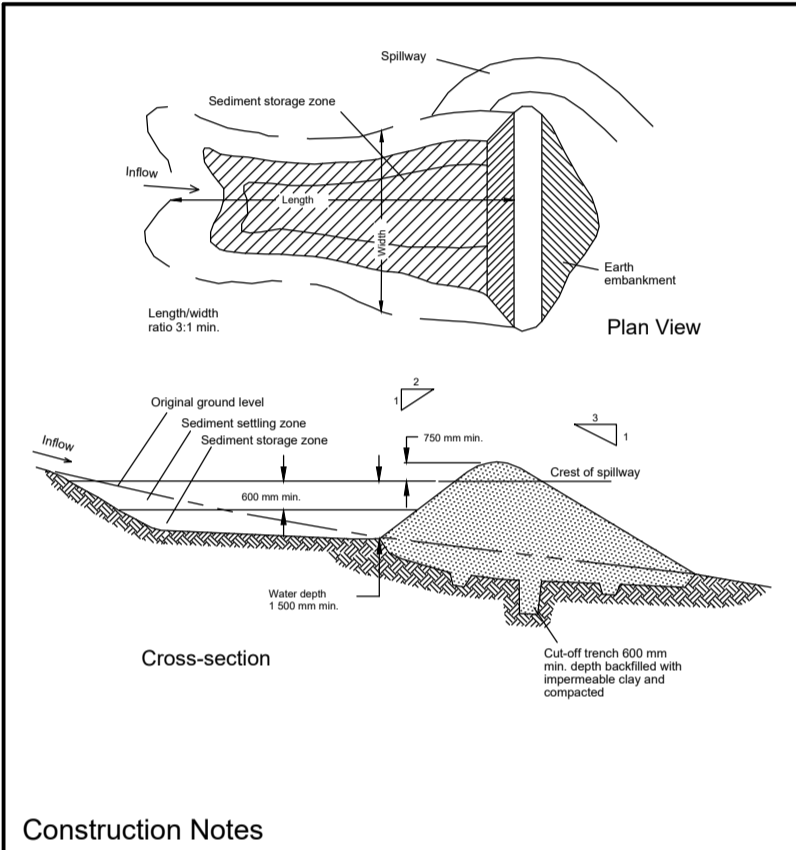
RECP: CONCENTRATED FLOW SD 5-7



Construction Notes

- Compact the subgrade to the density of the surrounding undisturbed material.
- Prepare a smooth, even foundation for the structure that will ensure that the needle-punched geotextile does not sustain serious damage when covered with rock.
- Should any minor damage to the geotextile occur, repair it before spreading any aggregate. For repairs, patch one piece of fabric over the damage, making sure that all joints and patches overlap more than 300 mm.
- Lay rock following the drawing, according to Table 5.2 of Landcom (2004) and with a minimum diameter of 75 mm.
- Ensure that any concrete or spiral used for the energy dissipater or the outlet protection conforms to the grading limits specified on the SWMP.

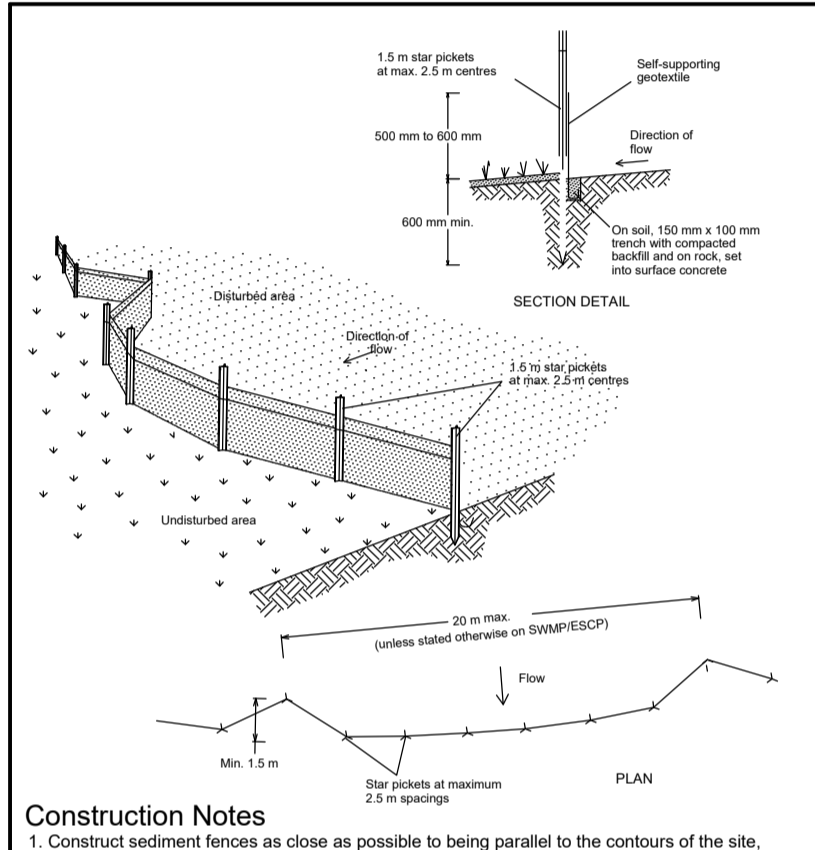
ENERGY DISSIPATER SD 5-8



Construction Notes

- Remove all vegetation and topsoil from under the dam wall and from within the storage area.
- Construct a cut-off trench 500 mm deep and 1,200 mm wide along the centreline of the embankment extending to a point on the gully wall level with the rear crest.
- Maintain the trench free of water and recompact the materials with equipment as specified in the SWMP to 95 per cent Standard Proctor Density.
- Select fill following the SWMP that is free of roots, wood, rock, large stone or foreign material.
- Prepare the site under the embankment by ripping to at least 100 mm to help bond compacted fill to the existing substrate.
- Spread the fill in 100 mm to 150 mm layers and compact it at optimum moisture content following the SWMP.
- Construct the emergency spillway.
- Rehabilitate the structure following the SWMP.

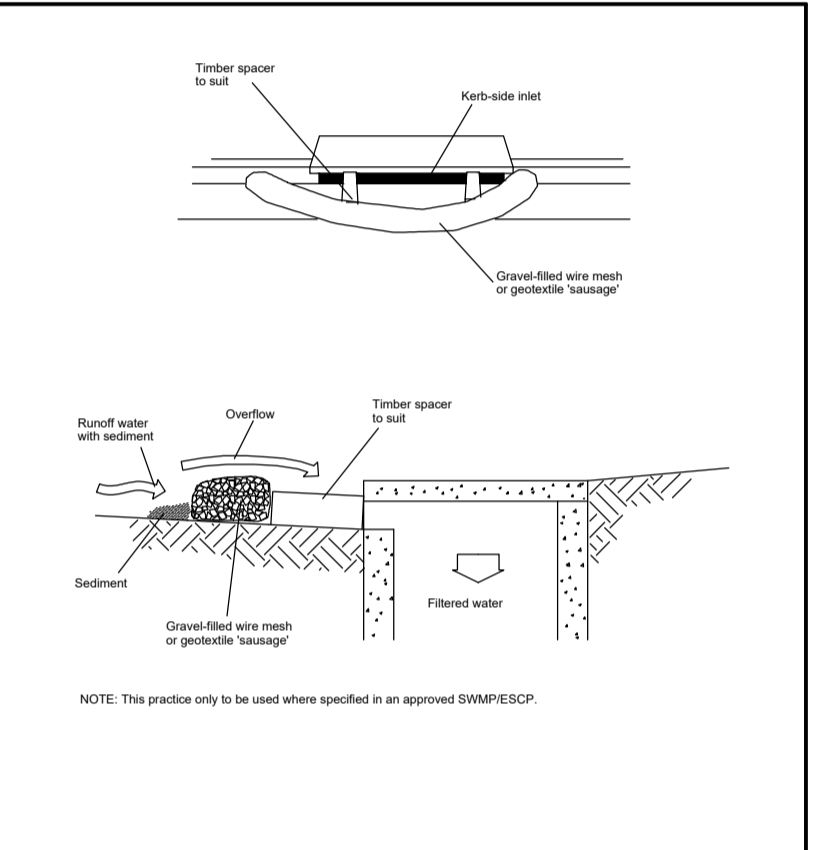
EARTH BANK - WET (APPLIES TO TYPE 'E' AND 'F' SOILS ONLY) SD 6-4



Construction Notes

- Construct sediment fences as close as possible to being parallel to the contours of the site, but with small returns as shown in the drawing to limit the catchment area of any one section. The catchment area should be small enough to limit water flow if concentrated at one point to 50 litres per second in the design storm event, usually the 10-year event.
- Cut a 150-mm deep trench along the upslope line of the fence from the bottom of the fabric to be entrenched.
- Drive 1.5 metre long star pickets into ground at 2.5 metre intervals (max) at the downslope edge of the trench. Ensure any star pickets are fitted with safety caps.
- Fix self-supporting geotextile to the upslope side of the posts ensuring it goes to the base of the trench. Fix the geotextile with wire ties or as recommended by the manufacturer. Only use geotextile specifically produced for sediment fencing. The use of shade cloth for this purpose is not satisfactory.
- Join sections of fabric at a support post with a 150-mm overlap.
- Backfill the trench over the base of the fabric and compact it thoroughly over the geotextile.

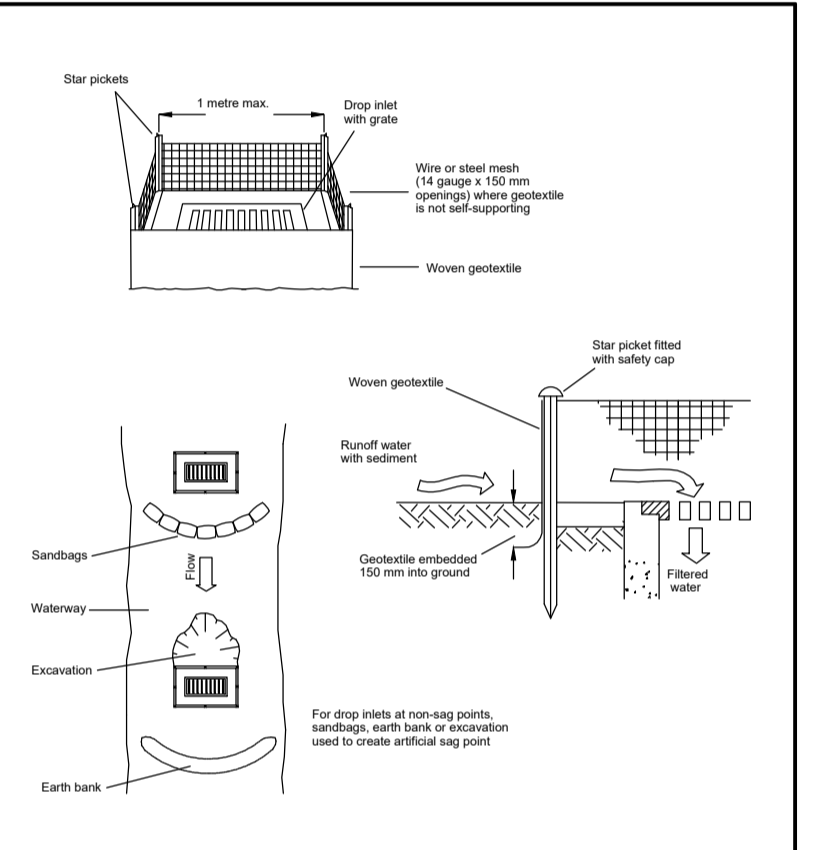
SEDIMENT FENCE SD 6-8



Construction Notes

- Install filters to kerb inlets only at sag points.
- Fabricate a sleeve made from geotextile or wire mesh longer than the length of the inlet pit and fill it with 25 mm to 50 mm gravel.
- Form an elliptical cross-section about 150 mm high x 400 mm wide.
- Place the filter at the opening leaving at least a 100-mm space between it and the kerb inlet. Maintain the opening with spacer blocks.
- Form a seal with the kerb to prevent sediment bypassing the filter.
- Sandbags filled with gravel can substitute for the mesh or geotextile providing they are placed so that they firmly abut each other and sediment-laden waters cannot pass between.

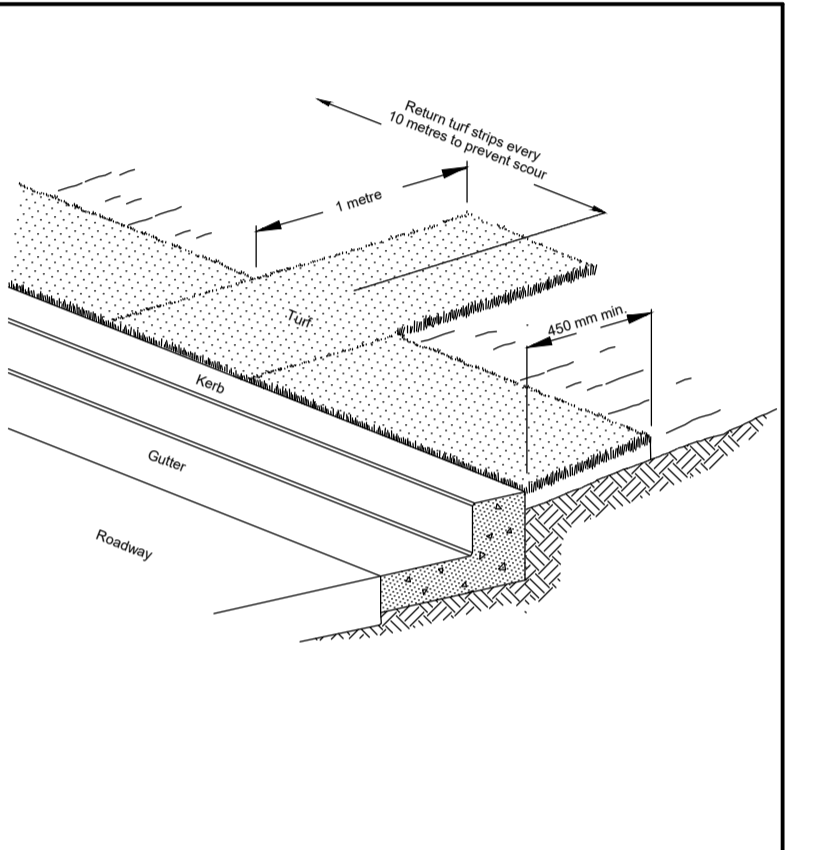
MESH AND GRAVEL INLET FILTER SD 6-11



Construction Notes

- Fabricate a sediment barrier made from geotextile or straw bales.
- Follow Standard Drawing 6-8 for installation procedures for the straw bales or geotextile. Reduce the picket spacing to 1 metre centres.
- In waterways, artificial sag points can be created with sandbags or earth banks as shown in the drawing.
- Do not cover the inlet with geotextile unless the design is adequate to allow for all waters to bypass it.

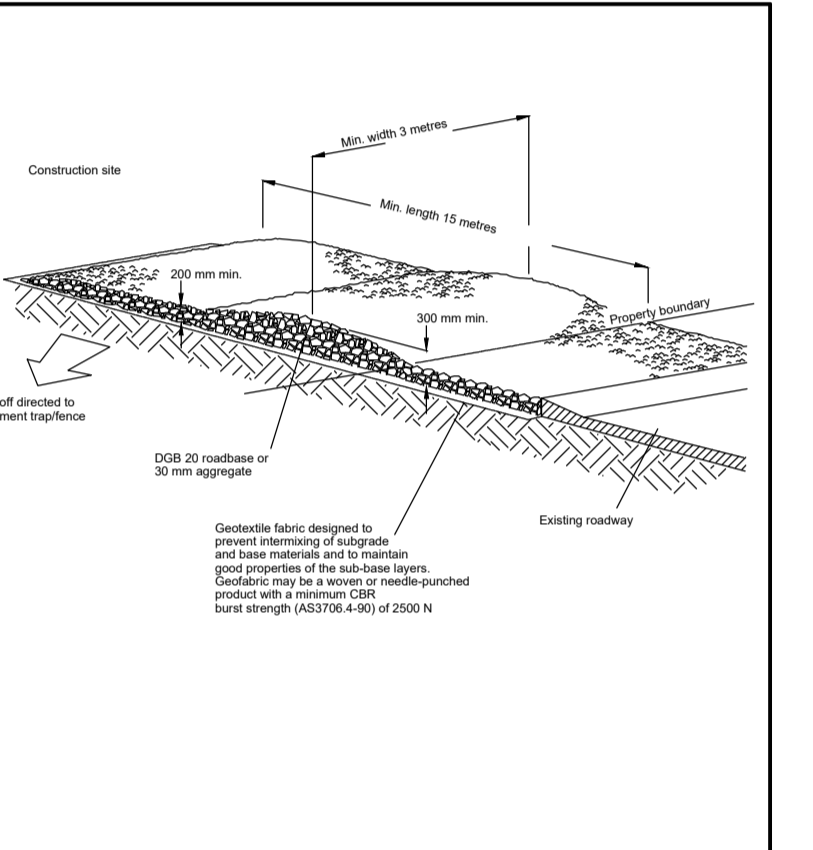
GEOTEXTILE INLET FILTER SD 6-12



Construction Notes

- Install a 450 mm minimum wide roll of turf on the footpath next to the kerb and at the same level as the top of the kerb.
- Lay 1.4 metre long turf strips normal to the kerb every 10 metres.
- Rehabilitate disturbed soil behind the turf strip following the ESCP/SWMP.

KERBSIDE TURF STRIP SD 6-13



Construction Notes

- Strip the topsoil, level the site and compact the subgrade.
- Cover the area with needle-punched geotextile.
- Construct a 200 mm thick pad over the geotextile using road base or 30 mm aggregate.
- Ensure the structure is at least 15 metres long to building alignment and at least 3 metres wide.
- Where a sediment fence joins onto the stabilised access, construct a hump in the stabilised access to divert water to the sediment fence.

STABILISED SITE ACCESS SD 6-14

SEDIMENTATION AND EROSION CONTROL DETAILS

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2	DEVELOPMENT APPLICATION	17.5.10	RESPONSIBLE PRINCIPAL SIGNATURE			TITLE SEDIMENTATION AND EROSION CONTROL DETAILS			SCALES N.T.S.			JOB No 17-828 DRAWING No C11 ISSUE 2		
1	DEVELOPMENT APPLICATION	15.5.10												
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ISSUE	REASON FOR ISSUE	DATE	DATE OF RELEASE				FULL SIZE ON ORIGINAL			0 1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 cm				



Suite 3, Level 1,
16 Telford Street,
NEWCASTLE EAST, NSW 2300
PO BOX 553
THE JUNCTION, NSW 2291
Tel: (02) 4927 5566
Fax: (02) 4927 5577
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Appendix E

Water Quality (MUSIC) Model



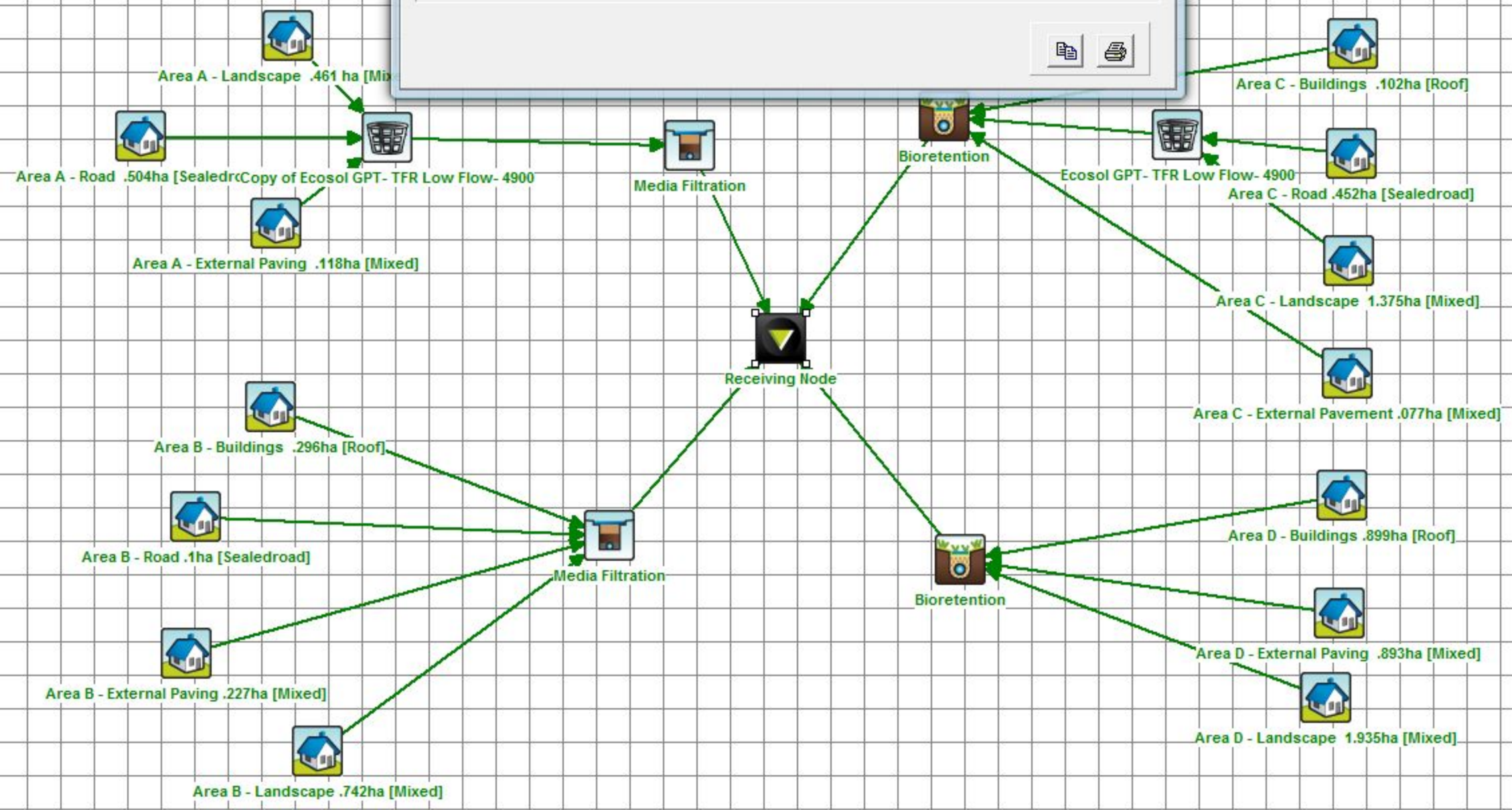
Treatment Train Effectiveness - Receiving Node

	Sources	Residual Load	% Reduction
Flow (ML/yr)	24.3	24.3	0
Total Suspended Solids (kg/yr)	2390	2390	0
Total Phosphorus (kg/yr)	6.82	6.82	0
Total Nitrogen (kg/yr)	60.4	60.4	0
Gross Pollutants (kg/yr)	29.8	29.8	0



Treatment Train Effectiveness - Receiving Node

	Sources	Residual Load	% Reduction
Flow (ML/yr)	64	62.4	2.6
Total Suspended Solids (kg/yr)	7400	608	91.8
Total Phosphorus (kg/yr)	16.7	6.08	63.5
Total Nitrogen (kg/yr)	128	60.2	53
Gross Pollutants (kg/yr)	1110	0	100



MUSIC-*link* Report

Project Details		Company Details	
Project:	Developed Site	Company:	MPC Consulting Engineers
Report Export Date:	23/08/2018	Contact:	Matthew Snelson
Catchment Name:	Music 2 Latest Model	Address:	16 Telford Street, Newcastle East NSW
Catchment Area:	8.885ha	Phone:	02 4927 5566
Impervious Area*:	49.20%	Email:	matthews@mpceng.com.au
Rainfall Station:	WILLIAMTOWN RAAF - Station 061078 - Zone B		
Modelling Time-step:	6 Minutes		
Modelling Period:	1/01/1998 - 31/12/2007 11:54:00 PM		
Mean Annual Rainfall:	1125mm		
Evapotranspiration:	1394mm		
MUSIC Version:	6.3.0		
MUSIC-link data Version:	6.31		
Study Area:	Williamtown		
Scenario:	Sensitive Catchment - Sandy soils		

* takes into account area from all source nodes that link to the chosen reporting node, excluding Import Data Nodes

Treatment Train Effectiveness		Treatment Nodes		Source Nodes	
Node: Receiving Node	Reduction	Node Type	Number	Node Type	Number
Flow	2.58%	Media Filtration Node	2	Urban Source Node	14
TSS	91.8%	Bio Retention Node	2		
TP	63.5%	GPT Node	2		
TN	53%				
GP	100%				

Comments

Passing Parameters

Node Type	Node Name	Parameter	Min	Max	Actual
Bio	Bioretention	Hi-flow bypass rate (cum/sec)	None	None	100
Bio	Bioretention	Hi-flow bypass rate (cum/sec)	None	None	100
Bio	Bioretention	PET Scaling Factor	2.1	2.1	2.1
Bio	Bioretention	PET Scaling Factor	2.1	2.1	2.1
GPT	Copy of Ecosol GPT- TFR Low Flow- 4900	Hi-flow bypass rate (cum/sec)	None	99	0.315
GPT	Ecosol GPT- TFR Low Flow- 4900	Hi-flow bypass rate (cum/sec)	None	99	0.315
Receiving	Receiving Node	% Load Reduction	None	None	2.58
Receiving	Receiving Node	GP % Load Reduction	90	None	100
Receiving	Receiving Node	TN % Load Reduction	45	None	53
Receiving	Receiving Node	TP % Load Reduction	60	None	63.5
Receiving	Receiving Node	TSS % Load Reduction	90	None	91.8
Urban	Area A - External Paving .118ha	Area Impervious (ha)	None	None	0.118
Urban	Area A - External Paving .118ha	Area Pervious (ha)	None	None	0
Urban	Area A - External Paving .118ha	Total Area (ha)	None	None	0.118
Urban	Area A - Landscape .461 ha	Area Impervious (ha)	None	None	0
Urban	Area A - Landscape .461 ha	Area Pervious (ha)	None	None	0.461
Urban	Area A - Landscape .461 ha	Total Area (ha)	None	None	0.461
Urban	Area A - Road .504ha	Area Impervious (ha)	None	None	0.504
Urban	Area A - Road .504ha	Area Pervious (ha)	None	None	0
Urban	Area A - Road .504ha	Total Area (ha)	None	None	0.504
Urban	Area B - Buildings .296ha	Area Impervious (ha)	None	None	1
Urban	Area B - Buildings .296ha	Area Pervious (ha)	None	None	0
Urban	Area B - Buildings .296ha	Total Area (ha)	None	None	1
Urban	Area B - External Paving .227ha	Area Impervious (ha)	None	None	0.227
Urban	Area B - External Paving .227ha	Area Pervious (ha)	None	None	0
Urban	Area B - External Paving .227ha	Total Area (ha)	None	None	0.227
Urban	Area B - Landscape .742ha	Area Impervious (ha)	None	None	0
Urban	Area B - Landscape .742ha	Area Pervious (ha)	None	None	0.742
Urban	Area B - Landscape .742ha	Total Area (ha)	None	None	0.742
Urban	Area B - Road .1ha	Area Impervious (ha)	None	None	0.1
Urban	Area B - Road .1ha	Area Pervious (ha)	None	None	0
Urban	Area B - Road .1ha	Total Area (ha)	None	None	0.1
Urban	Area C - Buildings .102ha	Area Impervious (ha)	None	None	0.102
Urban	Area C - Buildings .102ha	Area Pervious (ha)	None	None	0
Urban	Area C - Buildings .102ha	Total Area (ha)	None	None	0.102
Urban	Area C - External Pavement .077ha	Area Impervious (ha)	None	None	0.077
Urban	Area C - External Pavement .077ha	Area Pervious (ha)	None	None	0
Urban	Area C - External Pavement .077ha	Total Area (ha)	None	None	0.077
Urban	Area C - Landscape 1.375ha	Area Impervious (ha)	None	None	0
Urban	Area C - Landscape 1.375ha	Area Pervious (ha)	None	None	1.375

Only certain parameters are reported when they pass validation

Node Type	Node Name	Parameter	Min	Max	Actual
Urban	Area C - Landscape 1.375ha	Total Area (ha)	None	None	1.375
Urban	Area C - Road .452ha	Area Impervious (ha)	None	None	0.452
Urban	Area C - Road .452ha	Area Pervious (ha)	None	None	0
Urban	Area C - Road .452ha	Total Area (ha)	None	None	0.452
Urban	Area D - Buildings .899ha	Area Impervious (ha)	None	None	0.899
Urban	Area D - Buildings .899ha	Area Pervious (ha)	None	None	0
Urban	Area D - Buildings .899ha	Total Area (ha)	None	None	0.899
Urban	Area D - External Paving .893ha	Area Impervious (ha)	None	None	0.893
Urban	Area D - External Paving .893ha	Area Pervious (ha)	None	None	0
Urban	Area D - External Paving .893ha	Total Area (ha)	None	None	0.893
Urban	Area D - Landscape 1.935ha	Area Impervious (ha)	None	None	0
Urban	Area D - Landscape 1.935ha	Area Pervious (ha)	None	None	1.935
Urban	Area D - Landscape 1.935ha	Total Area (ha)	None	None	1.935

Only certain parameters are reported when they pass validation

Appendix F

Stormwater Maintenance Plan

MPC Ref: 17-828
March 2018

**PLAN OF MANAGEMENT
FOR
STORMWATER DRAINAGE SYSTEM**

**PROPOSED DEVELOPMENT AT:
507 MEDOWIE ROAD, MEDOWIE NSW**

The below schedules provide a timetable for various maintenance procedures which are relevant to the current stormwater system.

It also outlines the persons responsible and describes the actions required for each maintenance activity.

Regular checks for blockages should be undertaken after significant rainfall events outside the scheduled maintenance times.

Inspection of the Detention basin and Gross Pollutant Traps should be carried out by qualified personnel.



MPC Consulting Engineers
Suite 3, Level 1,
16 Telford Street,
NEWCASTLE EAST NSW 2300
PO Box 553
THE JUNCTION NSW 2291

Phone: (02) 4927 5566
Fax: (02) 4927 5577
Email: admin@mpceng.com.au
Web : mpcconsultingengineers.com.au

ACN 098 542 575
ABN 14 904 295 412

PITS/CARPARK/ROADWAY			
Maintenance Action	Frequency	Responsibility	Procedure
Inspect outlet pipe and remove any blockage	3 monthly	Owner	Remove grate and screen to inspect outlet
Inspect internal walls of pit for cracks or spalling	Annually	Maintenance Contractor	Remove grate to inspect internal walls. Repair as required.
Inspect grate for damage or blockage	3 monthly	Owner	Check both sides of grate for corrosion, damage or blockage
Inspect screen and clean	3 monthly	Owner	Remove grate and screen and clean if required
Check attachment of screen to wall of pit	Annually	Maintenance Contractor	Remove grate and screen. Ensure screen fixings are secure. Repair as required
Inspect sump and remove any sediment	3 monthly	Owner	Remove grate and screen. Remove sediment build up
Inspect car park for litter	Weekly	Owner	Remove any surface litter
Inspect car park for surface debris	3 monthly	Owner	Surface sweep and vacuum carpark pavement/hardstand to remove surface debris