Douglas PARTNERS

GROUNDED EXPERTISE

Groundwater Impact Assessment

Green Square DC SYDGPU1

3 Brookhollow Avenue, Norwest NSW

Prepared for Project Garden 1 Pty Ltd ATF The Norwest Property Trust

Project 233937.00

15 December 2025

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The undersigned, on behalf of Douglas Partners Pty Ltd, confirm that this document and all attached drawings, logs and test results have been checked and reviewed for errors, omissions and inaccuracies.

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Douglas Partners acknowledges Australia's First Peoples as the Traditional Owners of the Land and Sea on which we operate. We pay our respects to Elders past and present and to all Aboriginal and Torres Strait Islander peoples across the many communities in which we live, visit and work. We recognise and respect their ongoing cultural and spiritual connection to Country.

Table of Contents

Page No

1. Introduction 3
   - 1.1 Purpose of this report 3
   - 1.2 Regulatory framework and guidance 4
2. Site description 4
3. Hydrogeological setting 5
   - 3.1 Topography and hydrology 5
   - 3.2 Climate 5
   - 3.3 Geology 5
   - 3.4 Hydrogeology and groundwater source 6
   - 3.5 Groundwater receptors 6
   - 3.6 Acid sulphate soils and salinity 7
4. Investigation results 7
   - 4.1 Geotechnical 7
   - 4.2 Groundwater 8
     - 4.2.1 Groundwater levels 8
     - 4.2.2 Hydraulic conductivity 9
5. Groundwater quality 10
   - 5.1 Groundwater sampling methodology 10
   - 5.2 Field results 10
   - 5.3 Water screening levels 11
   - 5.4 Laboratory testing results 11
6. Conceptual hydrogeological model 12
7. Groundwater inflow and drawdown assessment 12
   - 7.1 Methodology 12
     - 7.1.1 Model geometry 13
     - 7.1.2 Model parameters 13
     - 7.1.1 Basement design and dewatering 14
     - 7.1.1 Simulation 14
     - 7.1.2 Sensitivity analyses 15
   - 7.2 Modelling results 15
     - 7.2.1 Groundwater inflow 15
     - 7.2.1 Drawdown and settlement 16

8. Impact assessment 17
8.1 Aquifer interference policy considerations 17
8.2 Risk assessment 17
9. Recommended groundwater management 18
9.1 General 18
9.2 Groundwater inflow control 18
9.2.1 Control measures and monitoring 18
9.2.2 Trigger levels 18
9.3 Discharge options and contingency measures 19
9.4 Suggested discharge criteria 20
9.5 Stormwater disposal treatment options 20
9.6 Batch monitoring 20
9.7 Analytes 21
9.8 Monitoring and reporting requirements 22
9.9 Personnel and responsibilities 23
9.10 Contingency plan 24
10. Approvals and licensing 25
10.1 Water Access Licence (WAL) and entitlements 25
10.2 Water Supply Works Approval (WSWA) 25
10.3 Council and EPA approvals 25
11. Conclusion 26
12. References 27
13. Limitations 28

Appendix A: Drawings

Appendix B: About This Report

Appendix C: Borehole logs

Appendix D: Hydrographs

Appendix E: Hydraulic testing results

Appendix F: Water quality, discharge criteria summary table and laboratory reports

Dewatering Management Plan

Green Square DC SYDGPU1
3 Brookhollow Avenue, Norwest NSW

1. Introduction

1.1 Purpose of this report

This report presents the results of a groundwater impact assessment (GIA) for the Stage 1 and 2 works, including upgrades to the existing data centre building, construction of new data centre buildings, a generator and electrical plant room, and a new substation and transformer yard. This assessment was commissioned by Juan Lewis of Aurecon Australasia (NSW) Pty Ltd as agents on behalf of Project Garden 1 Pty Ltd ATF Norwest Property Trust in accordance with Douglas Partners Pty Ltd (Douglas) proposal 233937.00.P.003.Rev0, dated 25 June 2025.

The purpose of this GIA is to support the submission of a State Significant Development Application (SSD-81928961), addressing the assessment requirements specified in Section 13. Additionally, this document aims to assist in obtaining the necessary approvals for the disposal of extracted groundwater, where applicable. The site location is shown on Drawing 1, Appendix A.

This GIA is based on the findings of site-specific investigations and monitoring and management requirements, including:

- Geotechnical investigation (Douglas 2025a); and,
- Acid sulfate soil management plan (Douglas 2025b).

The following key information is relevant to the preparation of this GIA:

- Parts of the proposed works will likely intercept the groundwater table;
- Groundwater levels on site have been observed at a maximum height of 93.8 m AHD;
- The finished floor levels vary across site, the deepest being the hydraulic plant room (Building B), with a proposed RL of 86.8 m AHD;
- The basement is intended to be drained; and,
- The extracted groundwater will be disposed of via the stormwater network.

This GIA is intended as a preliminary document and will be superseded by a detailed dewatering management plan that includes detailed water treatment and discharge schematics, monitoring plans, and triggers to be implemented during construction.

As the basement is proposed to be drained, a post-construction, long-term discharge treatment plan will be required, which is outside the scope of this report.

This report must be read in conjunction with all appendices including the notes provided in Appendix B.

1.2 Regulatory framework and guidance

The following key guidelines and documents were consulted in preparation of this report:

- *Guidelines for Groundwater Protection in Australia*, National Water Quality Management Strategy (Australian Government, 2013);
- Australian and New Zealand Guidelines for Fresh and Marine Water Quality (ANZG, 2018);
- Minimum requirements for building site groundwater investigations and reporting (NSW DPE, 2022);
- *NSW Aquifer Interference Policy* (DPI-NSW Office of Water, 2012); and
- *NSW Aquifer Interference Policy* (NSW DPI, 2013), supplementary version.
- Groundwater assessment toolbox for Major Projects in NSW (NSW DPE, 2022);
- Guidelines for Groundwater Documentation for SSD/SSI Projects (NSW DPE, 2022);
- Minimum Groundwater Modelling Requirements for SSD/SSI Projects (NSW DPE, 2022); and
- Cumulative Groundwater Impact Assessment Approaches (NSW DPE, 2022).

Additional regulatory requirements may also apply, including WaterNSW approvals for water take and disposal (see Section 9), and local council or EPA conditions depending on the nature and location of discharge.

2. Site description

The property at 3 Brookhollow Avenue, Norwest (Lot 2021 in Deposited Plan (DP) 831173), is located at the corner of Norwest Boulevard and Brookhollow Avenue. The lot forms an irregular shape with a total area of approximately 5.2 hectares. The site is situated within a commercial precinct, with the surrounding area predominantly occupied by commercial buildings ranging between two and four storeys in height.

Currently, the property is occupied by a multi-storey data centre building situated on a southwest facing hill, with at-grade parking areas located to the east of the structure. The remaining parts of the lot are generally grassed, with mature trees along the boundaries.

Based on historical knowledge of the site, the existing data centre building is constructed on a levelled platform at approximately RL 89 m AHD. This pad was likely formed by cutting into the hillside by approximately 2 to 3 m at the northern end of the existing building and placing fill of up to approximately 5 m depth at the southern end, in the area of the existing loading dock.

The twin tunnels of the Sydney Metro Northwest rail line are understood to pass below the north-western part of the site. The tunnels are currently indicated on supplied plan (Drawing 230062-DA010 Issue 6 by Greenbox Architecture) to occupy a width of approximately 14 m, while the publicly available TfNSW report “Northwest Rail Link. EIS Stage 1 – Major Civil Construction Works” report indicates that the tunnel invert levels range from RL 75 m AHD to RL 70 m AHD. The approximate alignment of the Metro tunnels is shown in Drawing 1 in Appendix A as a guide, but this should be confirmed by Sydney Metro provided details.

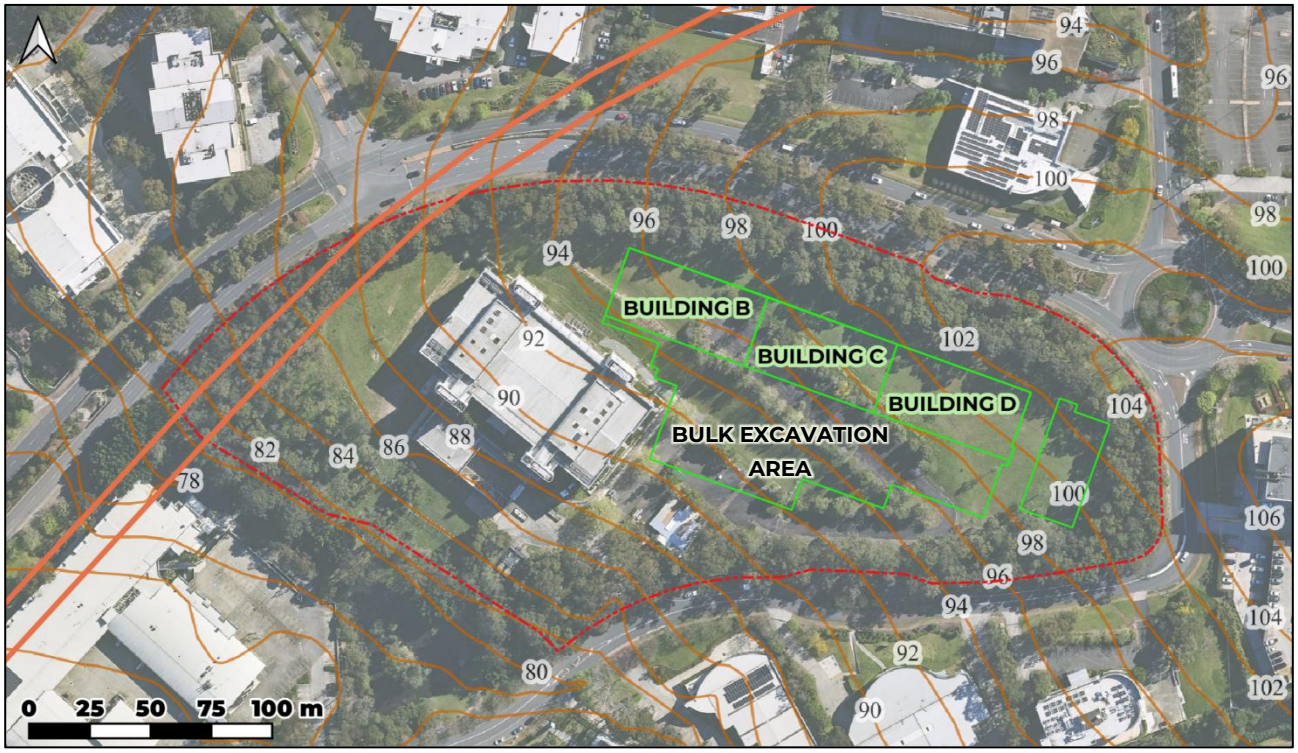


Figure 1: Aerial image of the site with 2 m AHD contours overlain, Stage 2 development are shown in green, approximate alignment of Sydney Metro shown in orange.

3. Hydrogeological setting

3.1 Topography and hydrology

The site surfaces slopes southwest from approximately 102 m to 80 m AHD, at a maximum 8 % gradient. The site is also situated 300 m east of a local ridgeline. Along the western boundary at 80 m AHD is Strangers Creek, which flows to Norwest Lake, an artificial lake situated at approximately 76 m AHD, 250 m west of the site (Figure 2). From Norwest Lake, Strangers Creek continues and ultimately discharges into the Hawkesbury River.

3.2 Climate

The average annual rainfall in Norwest is 900mm/year (www.bom.gov.au), in a temperate climate with warm summers, mild winters, and no dry seasons.

3.3 Geology

Reference to the Penrith 1:100 000 Geology Sheet indicates that the site is underlain by Ashfield Shale, which typically comprises black to dark grey shale and laminite (interlaminated siltstone and sandstone), overlain by residual clays of medium to high plasticity. The results of the field work for the present investigation were consistent with the published geological mapping with the exception of deeper fill in some areas due to past earthworks.

3.4 Hydrogeology and groundwater source

Based on the hydrogeological setting, the site is assumed to underlain by the following groundwater systems:

- Aquitard within the clay regolith profile where perched groundwater could be present;
- Minor fractured aquifers hosted by the underlying Ashfield Shale bedrock. These formations typically act as aquitards or aquicludes, although minor secondary permeability can occur where defects are present.

Reference to the Water Sharing Plan for the Greater Metropolitan Region Groundwater Sources (DPE Water, 2023) indicates that the site is underlain by the Sydney Basin Central Groundwater Source (i.e. the underlying siltstone bedrock), considered a porous rock groundwater source.

3.5 Groundwater receptors

A review of the Groundwater Dependent Ecosystems Atlas from the Australian Bureau of Meteorology indicates that there are no listed groundwater dependent ecosystems (GDE) within 500 m of the site. Located 580 m northwest is Cumberland Shale Plains Woodland, mapped as being a high potential GDE.

A review of the BoM national groundwater information system (NGIS) shows 5 water bores are registered within 500 m of the site (Figure 2). Note these bores were registered in 2007 for monitoring purposes only.

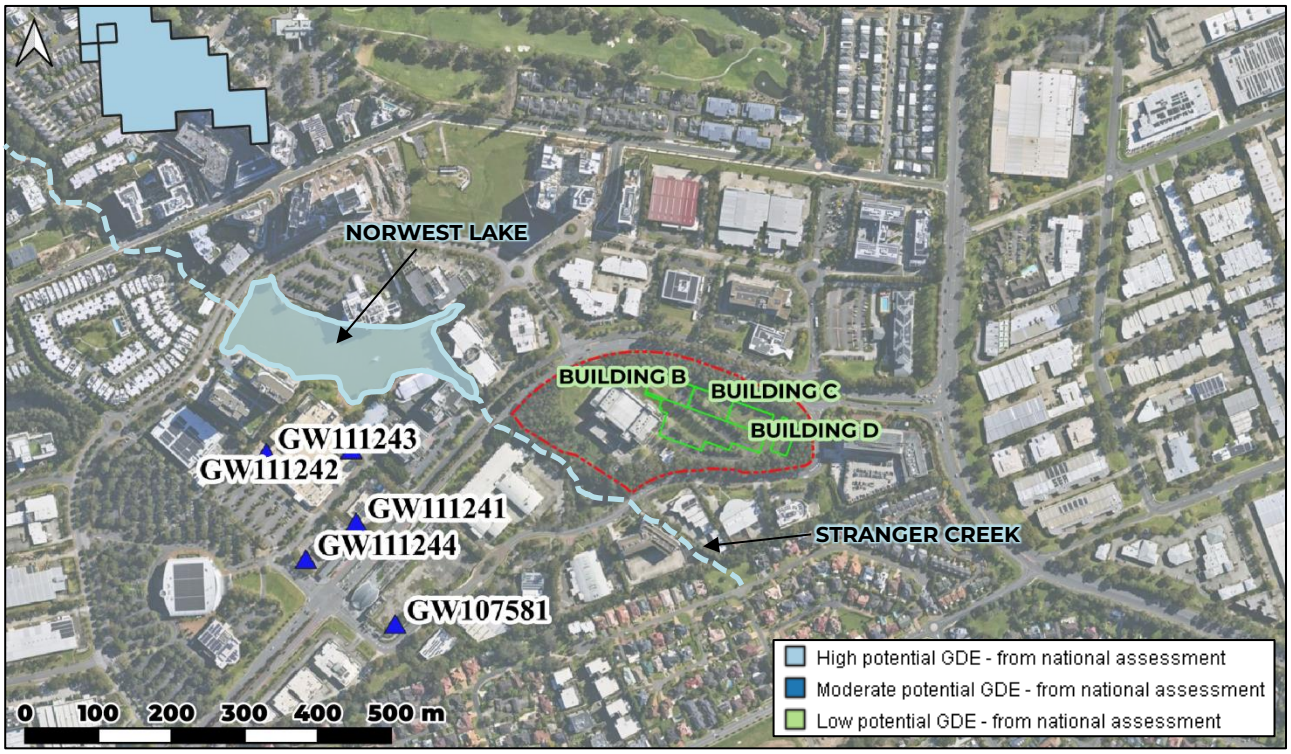


Figure 2: Registered groundwater bores and groundwater dependent ecosystems within the vicinity of the site

3.6 Acid sulphate soils and salinity

The site was reviewed for the potential presence of acid sulphate soils (ASS) and salinity risks based on regional mapping, soil classification and available site investigation data (Douglas, 2025b). Reference to the NSW Acid sulphate soil risk map indicates that the site is not within an area of known acid sulphate soil risk. Given that the site is neither located in a coastal nor alluvial region and is underlain by residual and erosional soil landscapes above RL 80 AHD, the presence of ASS within the underlying soils is unlikely. The risk of oxidising off-site ASS due to any dewatering related drawdown is also considered to be low.

The NSW Salinity Potential map for Western Sydney indicates that a moderate salinity potential category is applicable to the site and its immediate surrounds. A total of 12 laboratory tests were undertaken on samples from eight boreholes. The samples of the natural soil layers indicate non-saline values. Given the results obtained (i.e. non-saline soils), the provision of a salinity management plan is generally not required to address planning requirements.

4. Investigation results

4.1 Geotechnical

Previous geotechnical investigations by Douglas have been conducted on site, including in 1993, 2003 and May 2025. Refer to Douglas report 233937.00.R.002.Rev0 (2025a) for more information.

The current investigation was carried out in July 2025, and comprised the following:

- Ten additional boreholes (BH201 to BH210) drilled to depths of between 10.2 m to 20.5 m, using a truck-mounted drilling rig;
- Installation of monitoring wells within BH202 and BH205 located within the proposed Stage 2 development area, in addition to the monitoring well BH110 previously installed in May 2025 for Stage 1.

The test locations and geotechnical cross sections are shown in Appendix A. The detailed bore logs and rock core photographs are included in Appendix C, together with notes defining classifications methods and descriptive terms.

The interpreted geotechnical model can be summarised as follows:

Pavement / Slabs/ Fill	Fill material was generally encountered underlying the concrete, asphaltic concrete and topsoil surfaces at the majority of boreholes (excluding BH102, BH103, BH105, BH201 to BH202, BH205, BH207 and BH209 to BH210) to depths of between 0.5 m and 4.1 m. The general fill typically consists of poorly to moderately compacted brown clay fill with minor constituents of sand and igneous, siltstone and sandstone gravel.
Natural Clay (Residual)	Encountered below the fill within all test locations (excluding BH103 and BH104) to depths of between 0.6 m and 6.4 m. The consistencies of the clay were generally stiff to very stiff, grading to very stiff and hard with depth.

Siltstone (Very Low to Low Strength)	Generally consisting of extremely to moderately weathered, very low to low strength, dark grey to grey-brown, siltstone with some sandstone laminations, hard clay bands and weathered seams of medium strength to depths of between 3.9 m and 7.9 m.
Siltstone (Medium Strength or Stronger)	Generally consisting of slightly weathered to fresh, medium strength and stronger, dark grey siltstone with some sandstone laminations. Very high strength siltstone bands were encountered within BH107, BH201 to BH203, BH205 and BH209 at depths ranging from 6.7 m to 10.1 m, between 100 mm and 400 mm thick.

4.2 Groundwater

Table 1 summarises the three monitoring wells' attributes. Their location is shown on Drawing 1 (Appendix A).

Table 1: Monitoring well details

Well ID	Installation date	Surface (m AHD)	Screen depth (m bgl)	Screened lithology
BH110	20/05/25	89.0	7.0 – 10.5	Siltstone, extremely to slightly weathered
BH202	17/07/25	96.0	10.8 – 15.1	Siltstone, fresh, slightly fractured
BH205	16/07/25	100.0	16.3 – 20.2	Siltstone, fresh, unaltered

Notes: m bgl: metres below ground level

4.2.1 Groundwater levels

Groundwater seepage was observed within the clay fill during augering at BH105 and BH110, at depths of approximately 0.7 m and 2.5 m, respectively. Groundwater was not observed during auger drilling at the other locations. The use of water as a drilling fluid during rock coring precluded any further groundwater observations.

Groundwater levels have been continuously recorded at hourly intervals since 28 July 2025 using data loggers installed in the monitoring wells. Hydrographs showing recorded levels in comparison with rainfall data over time are presented in Appendix D. Groundwater levels were steady throughout the monitoring period, exhibiting little response to rainfall.

Maximum, minimum and average groundwater levels recorded by the dataloggers are presented in Table 2.

Table 2: Summary of logger data*

Well ID	Depth to groundwater (m bgl)			Groundwater elevation (m AHD)		
	Lowest	Highest	Average	Lowest	Highest	Average
BH110	Dry	Dry	Dry	<78.5	<78.5	<78.5
BH202	8.22	7.82	8.02	87.8	88.2	88.0
BH205	6.91	6.15	6.79	93.1	93.8	93.2

Notes: m bgl: metres below ground level
* Excluding data monitored during groundwater level recovery after hydraulic conductivity testing

4.2.2 Hydraulic conductivity

Rising head hydraulic conductivity tests were carried out at the monitoring wells on 28 July, 8 August and 9 September 2025. The tests were undertaken by removing a volume of water from the monitoring wells to lower the water level and measurement of the subsequent recovery rate. Testing was not carried out in BH110 due to it being dry throughout the monitoring period.

Hydraulic conductivity tests were analysed using the Hvorslev (1951) solution for slug testing interpretation. Results are presented in Table 3. The detailed results of the tests are attached in Appendix E.

Table 3: Interpreted hydraulic conductivity results

Well ID	Surface (m AHD)	Screen depth (m bgl)	Test No.	Hydraulic conductivity, K (m/s)
BH202	96.0	10.8 – 15.1	1	1.4×10^{-7}
			2	1.5×10^{-7}
BH205	100.0	16.3 – 20.2	1	7.2×10^{-10}
			2	1.7×10^{-9}

5. Groundwater quality

5.1 Groundwater sampling methodology

Groundwater quality testing was undertaken in monitoring wells BH202 and BH205 (BH110 could not be sampled). The results provide an indication of baseline groundwater quality at the site and inform potential disposal options and management requirements (further discussed in Section 9.3).

The general sampling procedure and subsequent laboratory analysis comprised of:

- Prior to sampling, measuring the groundwater monitoring wells using a dip meter which can detect phase separated hydrocarbons;
- Sampling using a bailer to visually inspect for oil and grease at the top of the water column;
- Following stabilisation of the field parameters, groundwater samples were collected in laboratory prepared bottles and vials. Samples collected for dissolved metals analysis were filtered using a 0.45 µm filter;
- Labelling each water sample container with an individual and unique identification, including project number and sample location. The containers were then placed into an ice cooled, insulated and sealed container for transport to the laboratory (with chain-of-custody); and
- Dispatching samples to Envirolab Services, which is National Authorities Testing Australia (NATA) accredited laboratory for the tests performed. The samples were analysed for a combination of the following common contaminants and parameters: total and dissolved metals; volatile organic compounds (VOC); benzene, toluene, ethylbenzene, xylene (BTEX); total recoverable hydrocarbons (TRH); polycyclic aromatic hydrocarbons (PAH); organochlorine pesticides (OCP); organo-phosphorus pesticides (OPP); polychlorinated biphenyls (PCB); per- and polyfluoroalkyl substances (PFAS); phenols; total dissolved solids (TDS); total suspended solid (TSS); pH; nutrients; cations, anions, and ionic balance.

5.2 Field results

The physical parameters recorded during sampling are summarised in Table 4.

Table 4: Physical Parameters from sampling on 14 October 2025

Borehole	Temperature (°C)	Dissolved oxygen (mg/L)	Electrical conductivity (µS/cm)	pH	Redox (mV)	Turbidity (NTU)
BH202	19.6	0.00	3787	6.85	113.6	10
BH205	18.9	0.16	6625	6.64	152.3	11

5.3 Water screening levels

The water screening levels were based on the following:

- ANZECC (2000) default trigger values for physical parameters (freshwater NSW lowland river systems);
- ANZG (2018) 95% level of protection (LOP) of species, freshwater water quality guidelines (i.e. as protective of the receiving water body), 99% LOP used for bioaccumulative contaminants;
- HEPA (2025) PFAS National Environmental Management Plan (NEMP);
- NHMRC (2008) guidelines for recreational waters, based on a relative consumption factors from the NHMRC (2018) drinking water guidelines; and
- NSW DPIE (2021) requirements.

These adopted screening levels form the basis of the dewatering discharge criteria further discussed in Section 9.4.

5.4 Laboratory testing results

The groundwater quality laboratory results have been compared to the adopted screening levels and are summarised in Appendix F, along with the supporting laboratory documentation.

The groundwater quality results are summarised as follows:

- pH and electrical conductivity (EC) values indicate brackish, slightly acidic to neutral conditions, expected of the site geology.
- No non-aqueous phase liquids or oily sheens were recorded.
- The following analytes were recorded above the discharge criteria:
    - Copper ranging in concentration from 5 µg/L to 78 µg/L (ANZG 95% DGV of 1.4 µg/L);
    - Nickel concentration in BH205 of 140 µg/L (ANZG 95% DGV of 11 µg/L);
    - Zinc ranging in concentrations from 52 µg/L to 110 µg/L (ANZG 95% DGV of 8 µg/L);
    - Ammonia concentration in BH205 of 1.4 mg/L (ANZG 95% DGV of 0.9 mg/L).
- PFOS was above the detection limits in both wells and exceeded the 99% LOP (0.00023 µg/L) in BH202 (0.0004 µg/L). However, total PFAS was within the 95% LOP in both locations.

As both BH202 and BH205 are situated near the top of the site, up-gradient of current site use, it is highly likely that elevated metal, nutrient, and PFAS concentrations are attributable to urban background levels.

6. Conceptual hydrogeological model

The geotechnical investigation identified that the Stage 2 site area is generally underlain by residual clay, grading to highly weathered to fresh siltstone. Groundwater levels were steady throughout the monitoring period, ranging between 93.8 and 78.5 m AHD, sloping approximately southwest, which is consistent with the local surface topography.

High groundwater water levels originating from monitoring wells screened within the lower siltstone strata indicate the fractured rock aquifer is (at least partially) confined by the residual clay aquitard above. Groundwater also likely exists near the soil-rock interface, particularly after rainfall, as indicated by seepage observed at 0.7 m and at 2.5 m depth in BH105 and BH110.

It is expected that water inputs to the ground profile beneath the site include:

- Rainfall infiltration on the site and surrounds, including water flowing downslope laterally through the natural soils and bedrock from areas of higher topography (Windsor Road); and
- Possible anthropogenic recharge from seepage from nearby leaking water mains and / or stormwater pipes, landscape irrigation etc.

Strangers Creek located at the bottom boundary of the site likely provides a path of groundwater discharge. The creek then feeds into 'Norwest Lake', situated at approximately 76 m AHD, which may act as a constant head boundary down gradient of the site.

At RL 75 to 70 m AHD, 50 m west of the site, the Metro tunnel may act as a slight hydraulic barrier, but it otherwise expected to have little interaction with the site hydrogeology.

It should be noted that groundwater levels are transient and fluctuate with climatic variations and other factors (e.g. possible adjacent basements). Therefore, the water levels will temporarily rise during periods of heavy or prolonged rainfall and fall during dry periods.

7. Groundwater inflow and drawdown assessment

7.1 Methodology

Seepage modelling was undertaken to assess the potential inflow rates into the proposed basement during and after construction, as well as induced groundwater level changes in adjacent and surrounding areas.

A 2-dimensional (2D) numerical groundwater model was developed. The modelling was carried out using the 2D finite element hydrogeological software SEEP/W (a component of GeoStudio) developed by GEOSLOPE International Ltd. The transient conditions during construction dewatering and the long-term 'steady-state' flow to the 'drained' excavation were modelled in the analysis.

7.1.1 Model geometry

A cross section in the north to south direction intercepting the basement footprint was selected to run the 2D analysis.

The aquifer surrounding the proposed development was simulated as a two-layer numerical model to represent the subsurface conditions beneath the site and allow simulation of the vertical flow components. The SEEP/W results are based on a 2D model and therefore provide seepage rates per unit metre. Inflow volumes were calculated through use of a shape factor based on Kavvadas (1992).

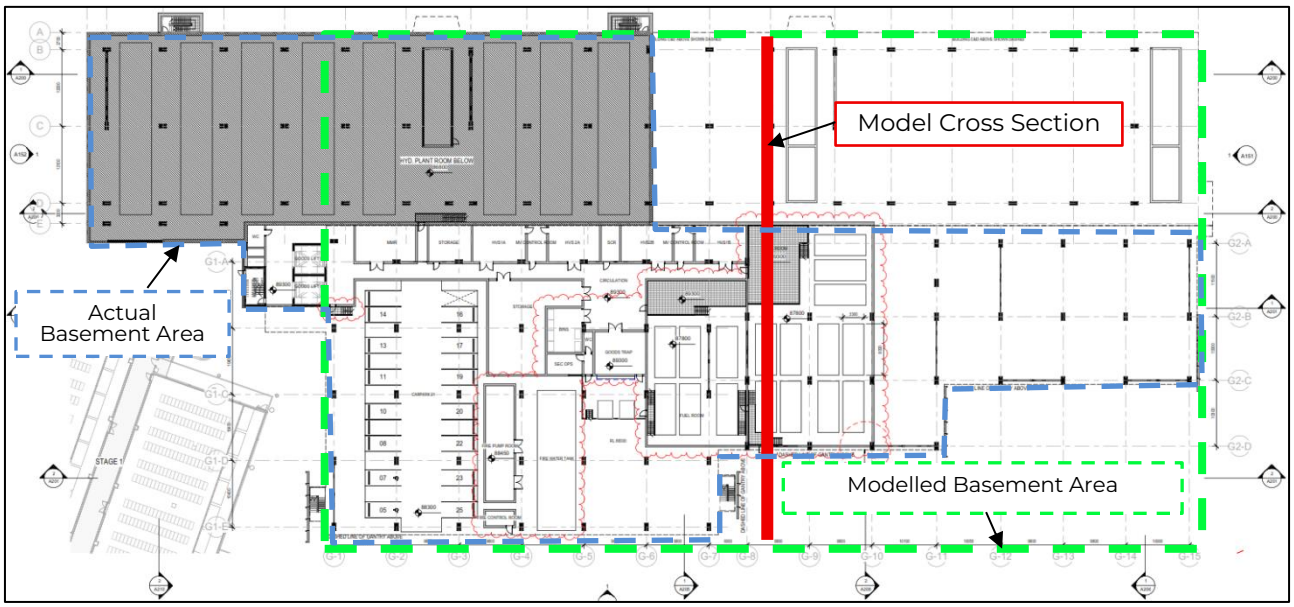


Figure 3: Assumed basement area and modelled cross-section (adapted from provided draft basement plan, Drawing 230062-A101 Issue 4 dated 28 November 2025, Greenbox Architecture)

The proposed excavation was measured to cover an approximate area of 8,000 m^2 and a perimeter of 500 m. For modelling purposes this was simplified to a rectangular configuration with an effective width of 80 m (B) and length of 100 m (L).

7.1.2 Model parameters

The adopted hydraulic properties (base case) are presented in Table 5. Parameters were selected based on the results of the hydraulic conductivity testing (Section 5.2.2), ranges of values documented in the available literature for similar lithologies (e.g. Fetter 1994, Hoek & Bray 1981, Pells et al 2019) and previous experience with similar materials.

Table 5: Model layers and adopted hydraulic properties

Layer	Material	Saturated Kh (m/s)	Anisotropy Ratio (Kv/Kh)	Porosity (%)	Material Model Adopted
1	Residual clay, extremely weathered siltstone	3.0×10^{-8}	1.0	30	Unsaturated / saturated
2	Fractured siltstone	1.5×10^{-7}	1.0	10	Unsaturated / saturated

A net infiltration percentage of 2% of the average annual rainfall (Section 3.2) was assigned as water influx over the ground surface, due to the sites urban setting and low permeability clay regolith.

The model adopted an upstream constant head boundary at RL 97 m AHD on the northern edge and a downstream constant head boundary at RL 72 m AHD on the southern edge to replicate the maximum groundwater levels observed within the site, as shown on Figure 4. The model boundary was extended 140 m from the northern and southern excavation boundaries.

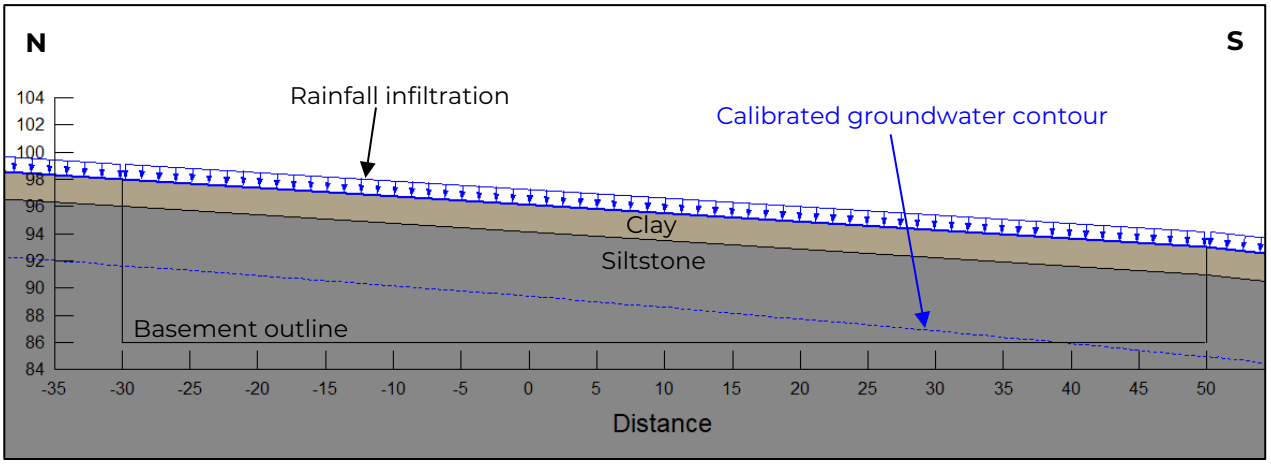


Figure 4: Model cross section, showing calibrated groundwater contour

7.1.1 Basement design and dewatering

The basement walls and sub-floor drainage were simulated as 'Seepage Faces' in SEEP/W with zero hydraulic pressure head along the full perimeter. The proposed finished floor level for the basement is understood to vary from 86.8 to 94.3m AHD. A dewatering level of 86.0 m AHD was used throughout the modelled basement area, which is considered conservative.

7.1.1 Simulation

To simulate the temporary dewatering required during construction, transient flow conditions were modelled in the analysis. For the purpose of the analysis, excavation was assumed to be

undertaken in a single stage (i.e. instantaneously) and the subsequent temporary dewatering period was assumed to be up to one year, subdivided into multiple time intervals.

The analysis was also run under 'steady state' conditions to assess long term groundwater inflows (i.e. assuming permanent drained basement, after construction).

7.1.2 Sensitivity analyses

Sensitivity analyses were undertaken to assess the impact of intersecting more permeable ground conditions or shallower water table:

- **Sensitivity 1**: assuming the groundwater table is 2 m higher than observed during monitoring.
- **Sensitivity 2**: assuming the siltstone has a hydraulic conductivity twice as conductive than baseline conditions (i.e. 3.0×10^{-7} m/s).

7.2 Modelling results

7.2.1 Groundwater inflow

The inflow rates obtained from the modelling represent the estimated total rate of groundwater flowing into the excavation and the volume (per unit time) requiring extraction via the dewatering system, in order to dewater the basement during and after construction.

The estimated inflows from the analysis for the first year (i.e. during construction) and long term are summarised in:

- Table 6, for the base case scenarios as detailed in previous sections; and
- Table 7, for the sensitivity analysis.

Note that the precision to which the results are presented is to allow comparisons of scenarios and does not represent the accuracy of the inflow estimates.

Table 6: Simulated Inflow Results for Baseline Cases

Elapsed Time	Baseline case – predicted inflow	
	kL/day	Cumulative (ML)
1 day	16.2	0.02
3 days	13.6	0.05
7 days	11.5	0.10
14 days	10.2	0.17
21 days	9.5	0.24
30 days	8.9	0.32
60 days	7.8	0.58
90 days	7.3	0.80

180 days	6.3	1.41
365 days	5.4	2.50
Long term	**kL/day**	**ML/year**
	4.24	1.55

Table 7: Simulated Long Term Inflow Results for Sensitivity Cases

Long term inflow	**Case 1 – higher GWL**		**Case 2 – higher K**	
	kL/day	**ML/year**	**kL/day**	**ML/year**
	6.66	2.43	8.09	2.95

Notes: K: hydraulic conductivity; GWL: groundwater level

The results of the sensitivity cases for groundwater inflow analysis indicate that:

- Inflow increases will be significant if higher permeability siltstone is intersected by the excavation; and,
- Inflow increases will be minor in the event of heavy rain and elevated groundwater levels.

It should be noted that these volumes are 'estimates' of the average inflows. It is possible that localised zones of higher permeability may be present within the site, through which the rate of inflow could be significantly higher, and considering the subsurface heterogeneity and fractured aquifer system, a safety margin for application in the field should be considered.

It should also be noted that the inflow analysis is conservative, as it assumes that the groundwater table is consistent in the north the south direction (i.e., not perched) and not all portions of the varying level basement excavation will intercept groundwater at 86 m AHD.

7.2.1 Drawdown and settlement

The simulated lowered groundwater levels, or impact to the water table, outside the basement footprint over the long term are summarised in Table 8. The model results indicate a maximum drawdown close to the excavation of about 5.6 m, reducing to 0.6 m at a distance of 80 m from the excavation. As the groundwater drawdown is expected to occur within the Ashfield Shale, settlements on adjacent structures due to dewatering are expected to be negligible.

Table 8: Predictive model simulated drawdown

	Distance from Excavation				
	0 m	**10 m**	**20 m**	**40 m**	**80 m**
Drawdown Depth (m) Upstream	5.6	4.0	3.1	2.3	0.3
Drawdown Depth (m) Downstream	3.2	3.0	2.6	2.2	0.6

8. Impact assessment

8.1 Aquifer interference policy considerations

The NSW Aquifer Interference Policy (AIP) indicates that the term "aquifer" is commonly understood to mean a groundwater system that is sufficiently permeable to allow water to move within it, and which can yield productive volumes of groundwater. A groundwater system is defined as any type of saturated geological formation that can yield low or high volumes of water. However, for the purpose of the AIP, the term aquifer has the same meaning as groundwater system and includes low yielding and saline systems.

Table 1 in Section 3.2.1 of the AIP outlines minimal impact considerations. The AIP indicates that "if predicted impacts are less than the Level 1 minimal impact considerations, then these impacts will be considered as acceptable". The following minimal impact considerations are outlined for less productive porous and fractured rock groundwater sources:

- Less than or equal to 10% cumulative variation in water table 40 m from any high priority GDE or high priority culturally significant site;
- A cumulative pressure head decline of no more than a 2 m at any water supply work; and
- Any change in groundwater quality should not lower the beneficial use category of the groundwater source beyond 40 m from the activity.

8.2 Risk assessment

An assessment of the potential effects of dewatering on neighbouring properties and groundwater receptors has been summarised in Table 9.

Table 9: Assessment of potential effects of dewatering

Item	Comment
Impacts on potential GDEs	There are no listed groundwater dependent ecosystems within 500 m of the site.
Water supply losses by neighbouring groundwater users	The nearest registered bores are monitoring wells, approximately 400 m away from the Stage 2 site area. There are no registered groundwater production bores within 1 km of the site.
Potential subsidence of neighbouring structures	Groundwater drawdown is predicted to occur within the siltstone. Therefore, risk of settlements due to lowering of the water table is expected to be negligible.
Mounding of water upgradient of structure	As the basement is proposed to be drained, mounding of groundwater is not expected upgradient of the site.

Therefore, the predicted impacts of dewatering are less than the Level 1 minimal impact considerations with reference to the NSW AIP.

9. Recommended groundwater management

9.1 General

A drained basement (i.e. non-hydrostatic) is considered manageable for this site due to the low inflows predicted, the site geology and the site topography. Drainage measures within the basement walls and below the basement slab will be required to collect and store water inflows for periodic discharge.

This section outlines the proposed management strategy based on the preferred methodology for dewatering and disposal.

9.2 Groundwater inflow control

9.2.1 Control measures and monitoring

Effective management and monitoring of groundwater inflow should be undertaken to support safe excavation, minimise impacts and satisfy the regulatory requirements. The proposed control measures will be selected based on site-specific hydrogeological conditions, construction methodology and relevant guidelines, and will be detailed in construction drawings and specifications.

Groundwater inflow control measures may include:

- Perimeter drainage to capture and divert groundwater;
- Inflow reduction measures. Such as grouting or ground improvement.

Groundwater level and inflow monitoring should be undertaken during construction to:

- Validate inflow predictions and performance of control measures (if any are decided to be necessary during construction);
- Ensure drawdown remains within acceptable limits to prevent environmental or structural impacts; and
- Implement trigger contingency responses if unexpected inflow or impacts occur.

More information on recommended monitoring is provided in Section 9.5.

9.2.2 Trigger levels

Control measures will be monitored against two key thresholds:

- Volume trigger: inflow exceeds 1.6 ML/year (maximum predicted by sensitivity modelling, Section 7); or
- Drawdown trigger: groundwater drawdown exceeds predictions by 2 m at any monitoring well.

If either trigger is reached, dewatering and excavation must be halted and a mitigation strategy (e.g., grouting) be implemented before work resumes.

9.3 Discharge options and contingency measures

Management options for groundwater disposal are presented in Table 10.

Based on groundwater quality testing and predicted dewatering rates, Option 1 (onsite dispersal) is considered feasible. The other two options listed in Table 10 are provided as contingency plans.

Table 10: Summary of possible management options

Management Option	Comments
Option 1: Batch monitoring and on-site dispersal.	As discussed in Section 5.2, the groundwater contamination levels are typical of an urban setting, and does not indicate a significant on-site source of contamination. Given the relatively small volume of water to potentially be irrigated (< 0.5 L/s, Sensitivity Case 2), on-site re-use is a preferred option. Preferentially used for dust suppression/washdown, or via irrigation. Batch monitoring implemented to ensure baseline water quality (Section 9.6). Considerations include vegetation salinity tolerance, limiting irrigation on slopes (< 6 % grade[for sprinklers] or <10% grade [trickle/micro spray]). Should signs of erosion be spotted then irrigation is to be halted, with the area inspected by a Geotechnical Engineer.
Option 2: On-site treatment and disposal to stormwater.	If high inflows occur, for example after rainfall events. Inflows associated with rainfall should have lower contamination levels, requiring only basic treatment, e.g. solids removal, alum dosing and pH adjustment. Water disposed to stormwater is typically required to meet general NSW DPE requirements (NSW DPE 2021 & 2022), ANZG (2018) water quality standards for the relevant receiving water body and any associated uses. Further requirements may be enforced depending on the specific approval documentation.
Option 3: * Tanker off-site for disposal at a licensed liquid waste treatment facility.	May be suitable for more heavily contaminated liquids where on-site treatment is not practicable. Appropriate as a contingency strategy. Can be limited in applicability for larger volumes of water. Water quality screening levels will depend on specific requirements of the waste facility (including their Environmental Protection Licence conditions)

Notes: *Off-site disposal of water by a tanker is generally only considered suitable in cases where periodic / batch disposal of groundwater is required (e.g., ephemeral water sources / rainfall / minor seepage only). Where continuous discharge is anticipated, this option is not feasible from an economic and / or environmental (i.e., emissions) perspective given the requirement to transport large volumes via truck at distance. It may be considered as a contingency strategy if any notable contamination which may be outside the operating capacity of the on-site treatment system is identified prior to disposal.

9.4 Suggested discharge criteria

The suggested groundwater discharge thresholds are provided in Appendix F. The adopted discharge criteria are based on the stormwater system discharging to a freshwater body. The discharge criteria include levels adopted from the Australian and New Zealand Guidelines for Fresh and Marine Water Quality (ANZG, 2018), with a 95% level of protection (LOP) for freshwater ecosystems.

The discharge criteria are subject to the approval of the Council or other consent authority responsible for the receiving water body and stormwater network.

9.5 Stormwater disposal treatment options

Potential treatment options for managing groundwater quality prior to discharge may include:

- Sediment control: Settling tanks or flocculation to reduce turbidity and suspended solids.
- pH adjustment: Dosing with lime or acid to achieve acceptable discharge pH.
- Metals removal: Filtration or chemical precipitation, if metal concentrations exceed guidelines following flocculation.

The treatment system will be designed by the dewatering contractor based on construction specifications, groundwater quality results and the discharge criteria. A detailed treatment schematic will be provided in the detailed DMP prior to construction.

9.6 Batch monitoring

The purpose of the monitoring is to assess ongoing compliance with the discharge criteria. Changes in the water quality post treatment may occur due to changes in the extracted / input water quality or changes during treatment (e.g., filters nearing end of life).

The base requirements for routine monitoring are provided in Section 9.8. The sampling frequency and analytical suite should be reviewed by the Environmental Consultant during the dewatering period. Based on this review the Environmental Consultant may recommend:

- Removing some analytes, due to sufficient data being available to assess that they are not of concern;
- A reduction in the frequency of analysis of some analytes, due to sufficient data being available to show consistency in water quality;
- Analysis of additional analytes, or more frequent analysis of some or all analytes, due to variability in the results or additional risks being identified based on field observations of laboratory results.

Recommendations for reductions in the analytical programme should be approved by Council prior to being implemented.

9.7 Analytes

The batch testing suite has been adopted based on the following information:

- The requirements of DPE (2022);
- The baseline water quality data (refer to Section 5.4);
- The potential for spills / leaks during excavation works; and
- General parameters to inform suitability of the water for discharge and changes in groundwater source.

Table 11: Proposed suite of analytes for water quality monitoring

Category	Analytes	Minimum frequency
Field parameters	pH, visible / olfactory signs of turbidity, oil / grease, chemical / noxious odours	Daily
	EC, REDOX potential (Eh), temperature, dissolved oxygen	Monthly[1]
Physical parameters	Alkalinity (total, carbonate, hydroxide bicarbonate), TDS, total hardness, TSS, total organic carbon (TOC), sodium absorption ratio (SAR)	Monthly[1]
Major ions	Sulfate (SO4), chloride (Cl), carbonates (CO3), bromide (Br), fluoride (F), Calcium (Ca), magnesium (Mg), sodium (Na), potassium (K) Cation/anion balance (as a percentage)	Monthly[1]
Metals (total and dissolved)	As, Cd, Cr, Cu, Hg, Pb, Mn, Ni and Zn	Every routine sampling event[2]
Nutrients	Ammonia (NH3), nitrate (NO3), total nitrogen (N), oxidised nitrogen (N), total phosphorus (P), reactive phosphorus (P)	Every routine sampling event[2]
Potential contaminants	TRH, BTEX, PAH, PFAS	Every routine sampling event[2]

Notes to table:
1. Based on requirements of DPE (2022)
2. Based on available data. Sampling requirements and frequencies may be reviewed as discussed in Section 9.6

Acronyms EC: electrical conductivity | TDS: total dissolved solids | TSS: total suspended solids
TRH: total recoverable hydrocarbons | TPH: total petroleum hydrocarbons
BTEX: benzene, toluene, ethylbenzene and xylenes
PAH: polycyclic aromatic hydrocarbons
VOC: volatile organic compounds
sVOC: semi-volatile organic compounds
PFAS: per- and polyfluorinated substances

9.8 Monitoring and reporting requirements

The following monitoring program and associated reporting is to be adopted until the end of excavation and construction works on-site. Groundwater level monitoring should be continued one month after construction. Groundwater level monitoring should be undertaken in a minimum of three monitoring wells. It is recommended that the existing monitoring wells used for the groundwater investigation be used during and after construction. Any wells damaged during construction should be replaced in a timely manner.

Table 12: Monitoring and reporting requirements

Item	Monitoring requirements	Methodology
Visual inspection	No visible oil and grease, sheen, significant discolouration or odours.	Daily inspections (by contractor). **HOLD POINT -** If indicators are observed, discharge must be suspended pending analytical confirmation.
Field parameter assessment	pH, daily, preferably continuously Visible / olfactory signs of turbidity, oil / grease, chemical / noxious odours: daily	• Preferably automated measurement as part of treatment system or measure with field pH meter of treated water. • Daily measurement (by contractor).
Groundwater level monitoring	Monitor drawdown at a minimum of three wells in the vicinity of the site.	• Continuous groundwater level monitoring using data logger (minimum six-hourly frequency). • Quarterly manual readings. • Monitoring to continue for one-month post-construction dewatering. **HOLD POINT -** If drawdown exceeds predictions by >2 m, construction must be halted and contingency measures activated.
Water quality sampling and testing	Samples from treated water to assess compliance with discharge criteria (Appendix E). As per analytical requirements in Section 9.7 Samples from untreated water as required to inform treatment requirements	All monitoring: • Sampling by suitably experienced environmental engineer / scientist. • Field parameters (pH, EC, turbidity) using field probes. • Chain of Custody (COC) documentation to accompany all samples. • Analysis by a NATA-accredited laboratory. Baseline and treatment trial: • As per Section 9.4.3

Item	Monitoring requirements	Methodology
		Routine: • Sampling pre-discharge (batch discharge). • Sampling twice weekly (continuous discharge). • Review and update frequency in accordance with Section 9.7 **HOLD POINT -** If water exceeds assessment criteria (SAC), manage as per contingency strategy (Section 9.7)
Quality control sampling	Replicate sampling to verify lab result accuracy.	• Conducted during above sampling events. • Analyse at a rate of 10% of samples. • Testing suite: Metals, TRH, BTEX • Results to be included in final dewatering completion report.
Quantity of groundwater inflows	Measurements of groundwater volumes (as per Section 9.2	• Weekly monitoring and reporting of groundwater abstraction volumes. • Results to be included in final dewatering completion report. **HOLD POINT -** If volumes exceed trigger levels or Council discharge allowance, works must be halted to minimise inflow (e.g., via grouting).
Dewatering completion report	Final documentation of the dewatering program.	Prepared by a suitably qualified consultant and to include: • Summary of contractor records (e.g. visual inspections, unexpected finds) • Full analytical and quality control results. • Records of dewatering volumes. • Commentary on compliance and any non-conformances with the DMP.

9.9 Personnel and responsibilities

Table 13 below outlines the proposed project personnel and relevant responsibilities as part of the management plan.

Table 13: Personnel and Responsibilities

Role	Responsibilities
Site Manager / Contractor	Routine visual inspection. Monitoring / recording of dewatering / discharge volumes. Maintaining any unexpected / contingency records.
Dewatering Contractor	Design / specification and ongoing maintenance of the dewatering system.
Geotechnical Consultant	Groundwater level monitoring and review of disposal volumes. Assist in preparation of the dewatering completion report.
Environmental Consultant	Water quality sampling (analysis using NATA accredited laboratories). Interim advice for each sampling event to confirm (or otherwise) compliance with discharge criteria. Quality control sampling. Assist in preparation of the dewatering completion report.

9.10 Contingency plan

As per Section 9.8, at any hold point if any non-conformance is encountered then the following general contingency plan will be enacted:

- Entity recording the non-conformance to notify the Site Manager / Contractor and other relevant parties;
- Dewatering volumes:
  - Should dewatering volumes be higher than predicted or higher than discharge limits provided by relevant authorities, suspend construction and reduce pumping rates. Options could include grouting to reduce groundwater inflows;
  - Suspend discharge;
- Water quality discharge criteria non-conformance:
  - Review of results by Environmental Consultant to assess potential risk to environment;
  - Should water quality be deemed unsuitable for disposal, suspend dewatering and (further) treat water prior to discharge;
  - If relevant, Environmental Consultant to inspect the site / unexpected finds and collect additional water quality samples to assess issue of concern;
  - Review treatment procedures and update methodology as required to improve treatment outcomes;
  - Written advice by the Environmental Consultant that additional laboratory analytical results meet the discharge criteria and / or that recommending that continued discharge is not considered to pose an unacceptable environmental risk and / or recommending additional work (as applicable).
- Off-site tankering may be adopted to meet disposal requirements; and
- Notification to relevant regulatory authorities, as required under approvals.

10. Approvals and licensing

This section outlines the approvals, licences and other authorisations required for the proposed dewatering activities, consistent with the *Water Management Act 2000*, WaterNSW requirements, and any relevant local council or EPA obligations.

10.1 Water Access Licence (WAL) and entitlements

Dewatering of groundwater for drained basement operations constitutes an aquifer interference activity under the Water Management Act 2000 and may require a Water Access Licence (WAL) and appropriate water entitlement units (share and extraction components), depending on the water source and proposed take.

The site is located within the Sydney Basin Central Groundwater Source and is subject to the Greater Metropolitan Region Groundwater Sources 2023 water sharing plan.

Groundwater modelling undertaken for the project predicts that groundwater inflow rates will be low (1.5 ML/year estimated under baseline conditions). Results of the sensitivity scenarios also suggest results below 3 ML/year although higher flow rates could occur during prolonged wetter periods. These predictions are estimates based on the information available; actual flow rates may vary from these and will only be known once the excavation is complete and inflow rates can be measured.

Given that the long-term predicted inflow is less than 3 ML/year, the proposed drained basement may also be eligible for an ongoing Water Access Licence exemption. However, this will be subject to review and approval by WaterNSW.

10.2 Water Supply Works Approval (WSWA)

An approval to construct and operate a water supply work (e.g., dewatering system) is typically required under the *Water Management Act 2000*, unless exempt. For major projects, the WSWA is usually granted as part of the SSD approvals process and does not require separate application.

10.3 Council and EPA approvals

Depending on the proposed discharge route and water quality:

- Discharge to stormwater: Council approval may be required. Acceptance criteria typically align with ANZG guidelines and site-specific conditions – refer to Section 9.4.1.
- Discharge to sewer: A trade waste agreement may be required from the relevant water utility – refer to Section 9.3
- Discharge off-site: A licensed contractor must be engaged, and records of transport and disposal must be maintained – refer to Section 9.3
- Environmental Protection Licence (EPL): May be required if discharge volumes or quality trigger thresholds under the Protection of the Environment Operations Act 1997.

11. Conclusion

This hydrogeological assessment considers that the proposed drained basement is feasible without any significant impact to surrounding properties. In addition, the use of a drained basement is not considered likely to influence any GDEs or groundwater supply works in accordance with the AIP Level 1 minimal impact considerations.

Groundwater quality data suggests that some heavy metals and nutrients exceed the adopted site assessment guidelines for disposal to surface water bodies. The detected concentrations were assessed to be the result of regional background groundwater quality and not related to site specific sources of contamination.

Supplementary groundwater quality assessment is recommended to confirm the groundwater quality closer to the commencement of dewatering. It is noted that the guidelines / LOP adopted for the assessment of groundwater quality and site contamination should not necessarily be adopted as discharge criteria for groundwater encountered during basement dewatering. Discharge criteria should consider assessment of the dewatering process and discharge destinations to assign more appropriate discharge criteria. Should the recommended discharge criteria be exceeded, treatment may be required prior to disposal. Long term disposal options for a drained basement should be considered.

Construction of a drained basement will be subject to review and approval from the DCCEEW and other relevant authorities.

12. References

ANZECC. (2000). *Australian and New Zealand Guidelines for Fresh and Marine Water Quality*. Australia New Zealand Environment Conservation Council.

ANZG. (2018). *Australian and New Zealand Guidelines for Fresh and Marine Water Quality*. Canberra, ACT: Australian and New Zealand Governments and Australian state and territory governments.

Australian Government (2013). *Guidelines for Groundwater Protection in Australia – National Water Quality Management Strategy*. Department of Agriculture, Water and the Environment, Canberra.

Douglas Partners. (2025a). *Report on Stage 1 & 2 Geotechnical Investigation – Green Square DC SYDGPU1* at 3 Brookhollow Avenue, Norwest, NSW. GreenSquareDC Pty Ltd Reference 233937.00.R.002.Rev0.

Douglas Partners. (2025b). *Report on Acid Sulphate and Salinity Assessment– Green Square DC SYDGPU1* at 3 Brookhollow Avenue, Norwest, NSW. GreenSquareDC Pty Ltd Reference 233937.00.R.003.Rev0.

Geoscience Australia. (2009). *Groundwater Sampling and Analysis - A Field Guide*. Commonwealth of Australia.

NSW DPE. (2021). *Minimum requirements for building site groundwater investigations and reporting*.

NSW DPI – Office of Water. (2012). *NSW Aquifer Interference Policy*.

NSW DPI – Water. (2013). *Aquifer Interference Policy: NSW Policy for Managing the Impacts of Groundwater Extraction*.

13. Limitations

Douglas Partners Pty Ltd (Douglas) has prepared this report for this project at 3 Brookhollow Avenue, Norwest NSW in line with Douglas' proposal 233937.00.P.003.Rev0 dated 25 June 2025 and acceptance received by Niel Donald of Area3 acting on as agents on behalf of Project Garden 1 Pty Ltd ATF Norwest Property Trust. The work was carried out under Douglas' Engagement Terms. This report is provided for the exclusive use of Project Garden 1 Pty Ltd ATF Norwest Property Trust for this project only and for the purposes described in the report. It should not be used by or relied upon for other projects or purposes on the same or other site or by a third party. Any party so relying upon this report beyond its exclusive use and purpose as stated above, and without the express written consent of Douglas, does so entirely at its own risk and without recourse to Douglas for any loss or damage. In preparing this report Douglas has necessarily relied upon information provided by the client and their agents.

The results provided in the report are indicative of the sub-surface conditions on the site only at the specific sampling and/or testing locations, and then only to the depths investigated and at the time the work was carried out. Sub-surface conditions can change abruptly due to variable geological processes and also as a result of human influences. Such changes may occur after Douglas' field testing has been completed.

Douglas' advice is based upon the conditions encountered during this investigation and previous investigations. The accuracy of the advice provided by Douglas in this report may be affected by undetected variations in ground conditions across the site between and beyond the sampling and/or testing locations.

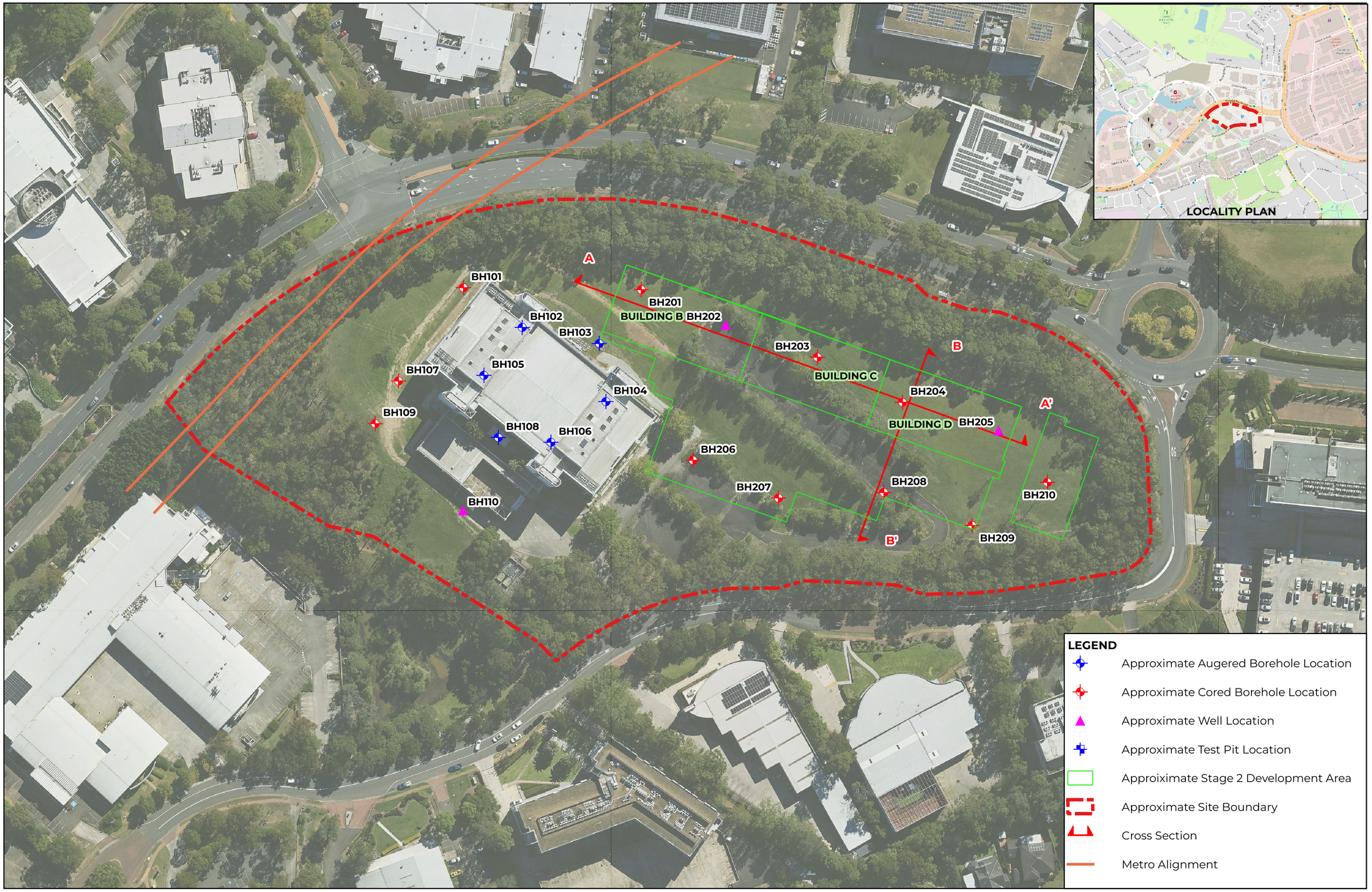
The assessment of atypical safety hazards arising from this advice is restricted to the geotechnical and groundwater components set out in this report and based on known project conditions and stated design advice and assumptions. While some recommendations for safe controls may be provided, detailed ‘safety in design’ assessment is outside the current scope of this report and requires additional project data and assessment.

This report must be read in conjunction with all of the attached and should be kept in its entirety without separation of individual pages or sections. Douglas cannot be held responsible for interpretations or conclusions made by others unless they are supported by an expressed statement, interpretation, outcome or conclusion stated in this report.

This report, or sections from this report, should not be used as part of a specification for a project, without review and agreement by Douglas. This is because this report has been written as advice and opinion rather than instructions for construction.

Appendix A

Drawings



REV	DESCRIPTION/COMMENT	DATE	DRAWN BY
0	Initial Issue	10.09.2025	MN

SCALE: 0 15 30 45 60 m — 1:1500 @ A3

Douglas PARTNERS
OFFICE: SYDNEY
96-98 Hermitage Rd, West Ryde NSW 2114
(02)9809 0666

CLIENT:
Project Garden 1 Pty Ltd ATF The Norwest Property Trust

NOTE:
1: Basemap from -Metromap (Dated 06.04.2025)
2: Development outlines from Greenbox Architects, Job No. 230062, Drawing No.0033, Issue 00 (Dtaed 06.09.2025)

COORDINATE REFERENCE SYSTEM: GDA2020 / MGA zone 56

PROJECT NAME:
Green Square DC SYDGPU1

PROJECT ADRESS:
3 Brookhollow Ave, Norwest

DRAWING TITLE:
Test Location Plan

PROJECT NO: **233937.00**
DRAWING NO: **1**
REVISION: **1**

\\dpsydnas02\Projects\233937.00 - BAULKHAM HILLS\3 Brookhollow Avenue\7.0 Drawings\7.3 QGIS\QGIS.qgz

A

A'

ELEVATION (AHD)

BH201

BH202

BH203

BH204

BH20 5

PROPOSED LOWEST FLOOR LEVEL
(BUILDING D) RL 100.8m

PROPOSED LOWEST FLOOR LEVEL
(BUILDING C) RL 94.3m

PROPOSED LOWEST FLOOR LEVEL
(BUILDING B & C) RL 86.8m

UNIT 1: FILL

UNIT 2: NATURAL CLAY
(RESIDUAL)

UNIT 3: VERY LOW TO LOW
STRENGTH SILTSTONE

UNIT 4: MEDIUM STRENGTH
OR STRONGER SILTSTONE

DISTANCE ALONG PROFILE (m)

LEGEND

- ASPHALTIC CONCRETE
- CH - High Plasticity CLAY
- CI - Medium Plasticity CLAY
- CI-CH - Medium to High Plasticity CLAY
- CLAY
- NO CORE
- FILL
- SILTSTONE
- TOPSOIL

TESTS / OTHER

- N - Standard penetration test value
- — ? — — - Interpreted geotechnical boundary
- ▼ - Groundwater Level

ROCK STRENGTH

- VL - Very Low
- L - Low
- M - Medium
- H - High
- VH - Very High
- EH - Extremely High

SOIL CONSISTENCY

- vs - Very Soft
- s - Soft
- f - Firm
- st - Stiff
- vst - Very Stiff
- h - Hard

REV	DESCRIPTION/COMMENT	DATE	DRAWN BY
0	Initial Issue	11.08.2025	MN
1	Updated with new Architectural Levels	10.09.2025	MN
2	Updated with new Basement Levels	04.12.2025	EC

SCALE: 0 10

Horizontal Scale 1:500
Vertical Exaggeration = 2.0

Douglas PARTNERS
OFFICE: SYDNEY
96-98 Hermitage Rd, West Ryde NSW 2114
(02) 9809 0666

CLIENT:
Project Garden 1 Pty Ltd ATF The Norwest Property Trust

NOTES
1. Subsurface conditions are accurate at the borehole locations only. Variations in subsurface conditions may occur between borehole locations. Interpreted strata boundaries are approximate and should be used as a guide only.
2. Summary logs only and should be read in conjunction with detailed logs.
3. Horizontal and vertical scales are not equal.

PROJECT NAME:
Green Square DC SYDGPU1

PROJECT ADDRESS:
3 Brookhollow Avenue, Norwest

DRAWING TITLE:
Geological Cross Section A-A'

PROJECT No: **233937.00**

DRAWING No: **2**

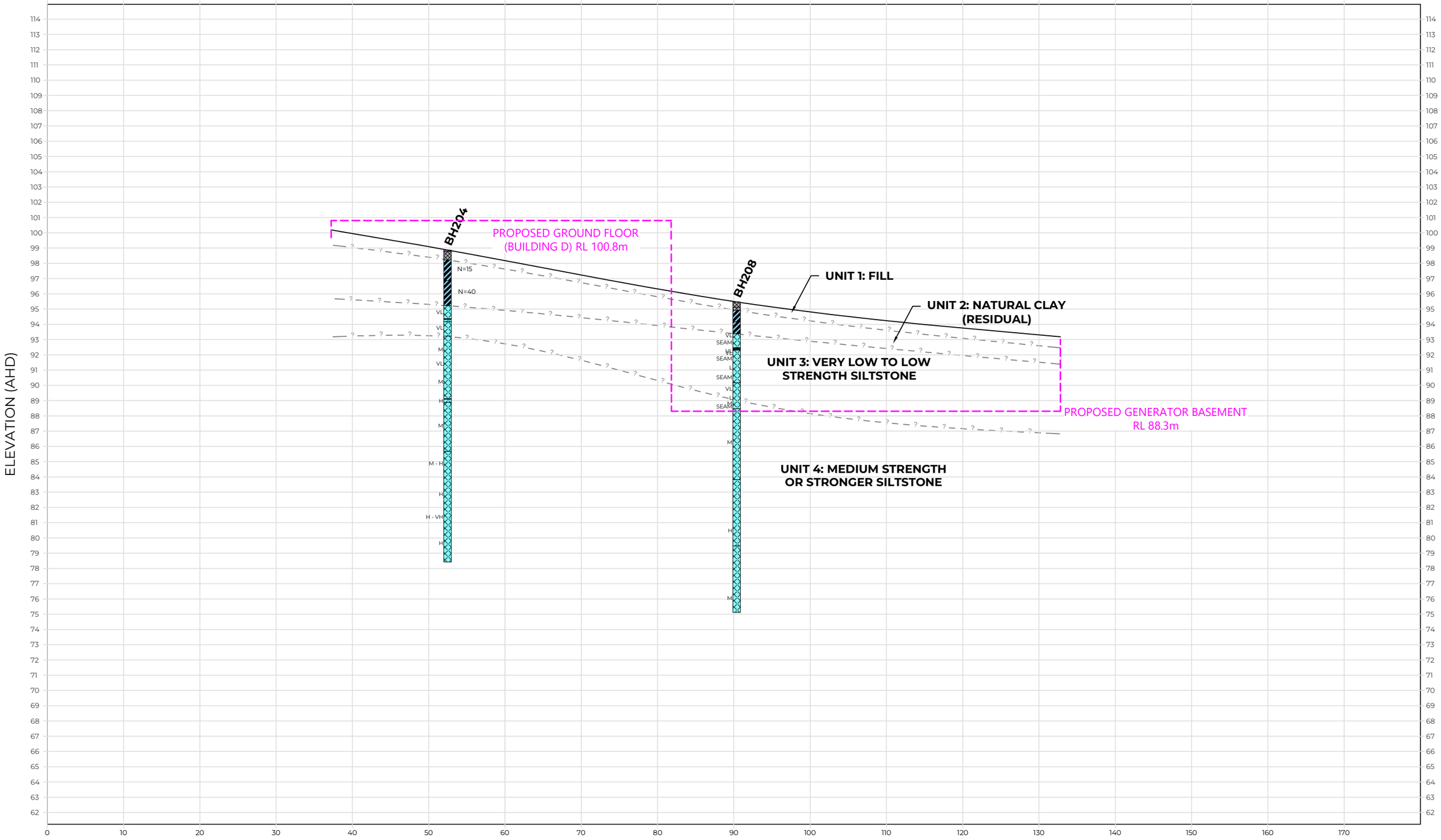
REVISION: **2**

\\dpsydnas02\Projects\233937.00 - BAU\KHAM HILLS\3 Brookhollow Avenue\7.0 Drawings\7.2 Out\233937.00.D.002.3.Rev0_Sections.dwg

B

B'

ELEVATION (AHD)



LEGEND

- ASPHALTIC CONCRETE
- CH - High Plasticity CLAY
- NO CORE
- FILL
- TOPSOIL
- SILTSTONE

TESTS / OTHER

N - Standard penetration test value

— ? — — - Interpreted geotechnical boundary

DISTANCE ALONG PROFILE (m)

ROCK STRENGTH

VL - Very Low
L - Low
M - Medium
H - High
VH - Very High
EH - Extremely High

SOIL CONSISTENCY

vs - Very Soft
s - Soft
f - Firm
st - Stiff
vst - Very Stiff
h - Hard

REV	DESCRIPTION/COMMENT	DATE	DRAWN BY
0	Initial Issue	11.08.2025	MN
1	Updated with new Architectural Levels	10.09.2025	MN
2	Updated with new Basement Levels	04.12.2025	EC

SCALE: 0 10

Horizontal Scale 1:500
Vertical Exaggeration = 2.0

Douglas PARTNERS

OFFICE: SYDNEY

96-98 Hermitage Rd, West Ryde NSW 2114

(02) 9809 0666

CLIENT:

Project Garden 1 Pty Ltd ATF The Norwest Property Trust

NOTES

1. Subsurface conditions are accurate at the borehole locations only. Variations in subsurface conditions may occur between borehole locations. Interpreted strata boundaries are approximate and should be used as a guide only.
2. Summary logs only and should be read in conjunction with detailed logs.
3. Horizontal and vertical scales are not equal.

PROJECT NAME:

Green Square DC SYDGPU1

PROJECT ADDRESS:

3 Brookhollow Avenue, Norwest

DRAWING TITLE:

Geological Cross Section B-B'

PROJECT No: **233937.00**

DRAWING No: **3**

REVISION: **2**

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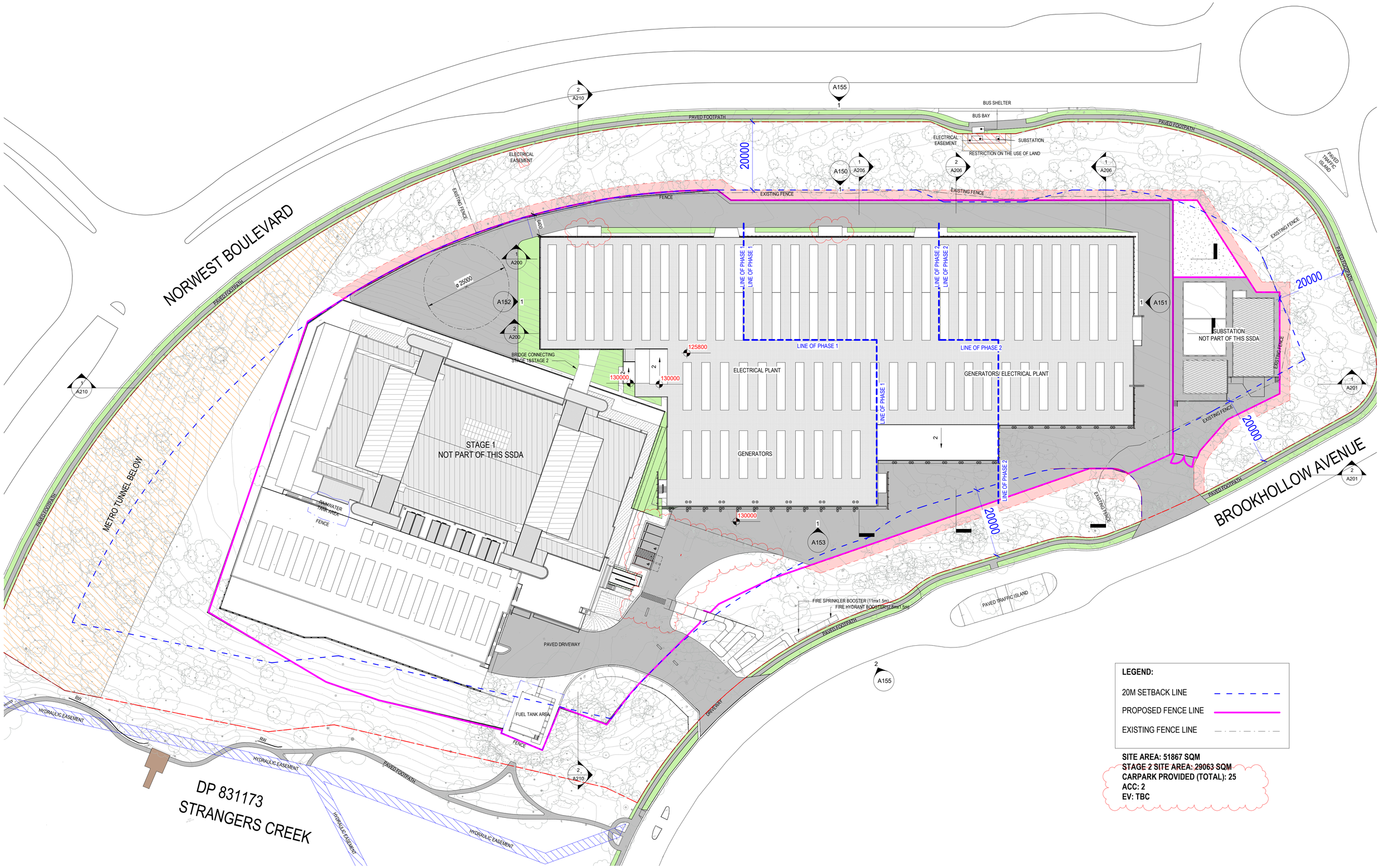


LEGEND:

20M SETBACK LINE

PROPOSED FENCE LINE

EXISTING FENCE LINE

SITE AREA: 51867 SQM
STAGE 2 SITE AREA: 29063 SQM
CARPARK PROVIDED (TOTAL): 25
ACC: 2
EV: TBC

N

A1

Issue	Date	Description
1	08/10/2025	ISSUED FOR INFORMATION
2	20/10/2025	ISSUED FOR INFORMATION
3	21/10/2025	INFORMATION
4	24/10/2025	INFORMATION
5	24/10/2025	INFORMATION
6	27/10/2024	INFORMATION
7	21/11/2025	ISSUE FOR INFORMATION
8	28/11/2025	ISSUE FOR INFORMATION

Key Plan

Project Manager: area3 — A BGIS Company

Client: Green SquareDC — HOME OF AI

Services and Lead Consultant: aurecon — AURECON

Structural and Civil: TTW

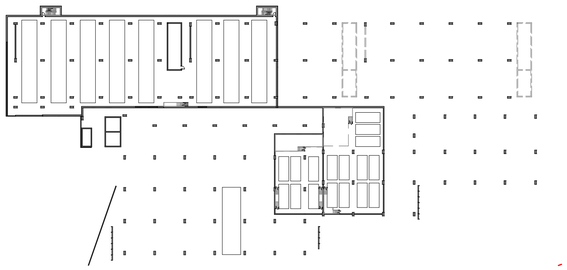
Greenbox — A WOOLPERT COMPANY
+61 2 8069 8930
LEVEL 22
68 PITT STREET
SYDNEY NSW 2000 AUSTRALIA
GREENBOX ARCHITECTURE PTY LTD
ABN: 79 139 779 098
ISO 9001 CERTIFIED QUALITY SYSTEM

Notes
- Use written dimensions only
- Do not scale from drawing
- Contractors shall confirm all dimensions on-site prior to commencing any work or producing any shop drawings
- All materials to be used in accordance with the manufacturer's specifications and instructions and shall comply with the relevant Australian Standards
- Copyright of this drawing and design remain the property of Greenbox Architecture Pty Ltd
- Nominated Architect - Gerard Page; NSW reg No.7247, Vic reg No.17664, QLD reg No.4538

Client: GreenSquareDC™

Project: Norwest DC
3 Brookhollow Avenue
Baulkham Hills
NSW

Scale	1 : 500
Sheet	A1
Scale Bar	1:500 0 5 10 15 25m
Drawn By	SP, BC
Project Status	DRAFT
Checked By	EX
Drawing Title	SITE PLAN
Approved By	GP
Job Number	230062
Drawing Number	A012
Issue	8



1 L01 - SSDA
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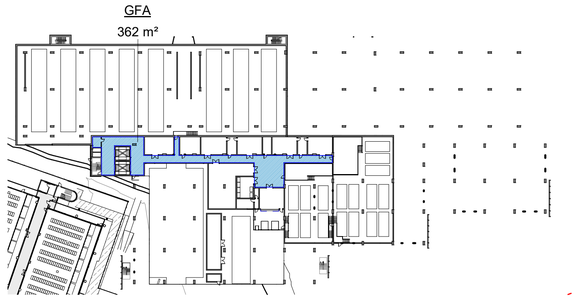

2 L02 - SSDA
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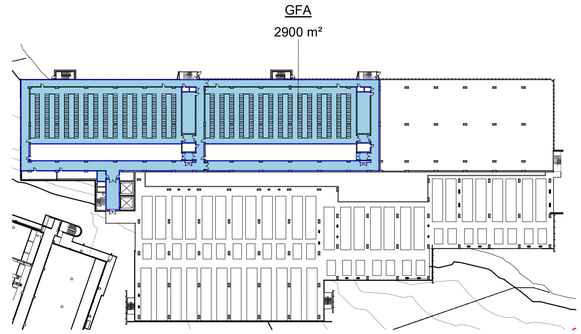

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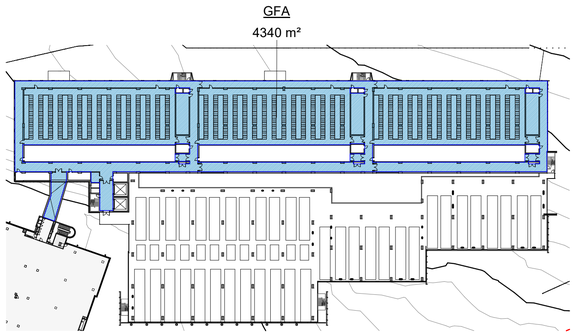

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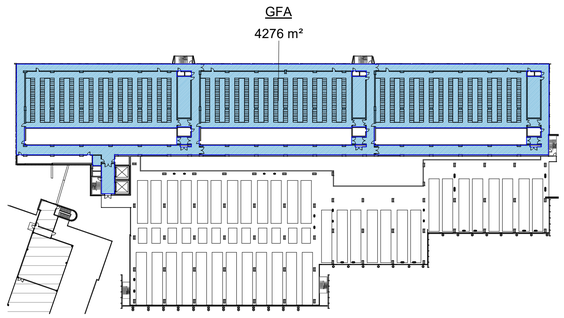

5 L05 - SSDA
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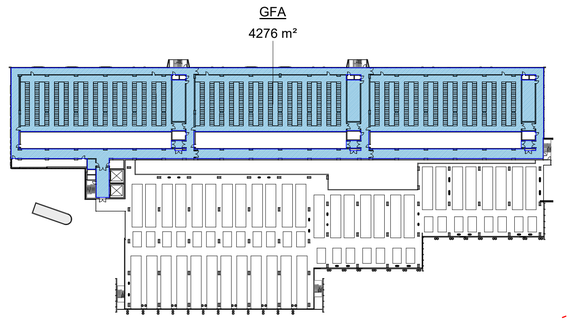

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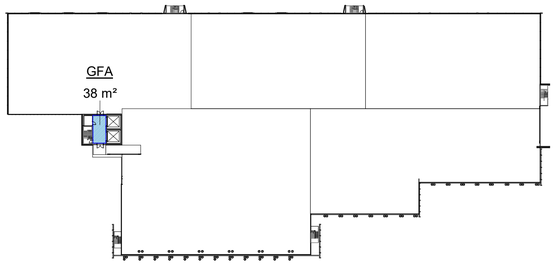

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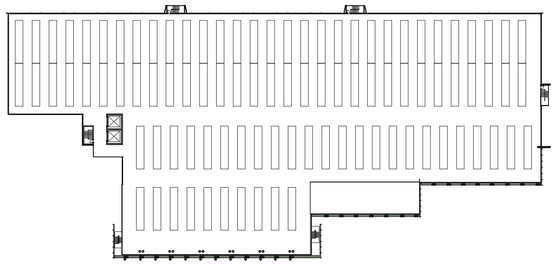

8 L08 - SSDA
1 : 1000

GFA SSDA	
Level	Area
L02	362 m²
L03	2900 m²
L04	4340 m²
L05	4276 m²
L06	4276 m²
L07	38 m²
	16191 m²

gross floor area (GFA) means the sum of the floor area of each floor of a building measured from the internal face of external walls, or from the internal face of walls separating the building from any other building, measured at a height of 1.4 metres above the floor, and
includes—
(a) the area of a mezzanine, and
(b) habitable rooms in a basement or an attic, and
(c) any shop, auditorium, cinema, and the like, in a basement or attic,
but excludes—
(d) any area for common vertical circulation, such as lifts and stairs, and
(e) any basement—
(i) storage, and
(ii) vehicular access, loading areas, garbage and services, and
(f) plant rooms, lift towers and other areas used exclusively for mechanical services or ducting, and
(g) car parking to meet any requirements of the consent authority (including access to that car parking), and
(h) any space used for the loading or unloading of goods (including access to it), and
(i) terraces and balconies with outer walls less than 1.4 metres high, and
(j) voids above a floor at the level of a storey or storey above.

N

A1

Issue	Date	Description
1	28/11/2025	ISSUE FOR INFORMATION

Key Plan

Project Manager
area3 A BGIS Company

Services and Lead Consultant
aurecon
AURECON

Client
Green SquareDC HOME OF AI

Structural and Civil
TTW

Greenbox A WOOLPERT COMPANY
+61 2 8069 8930
LEVEL 22
68 PITT STREET
SYDNEY NSW 2000 AUSTRALIA
GREENBOX ARCHITECTURE PTY LTD
ABN: 79 139 779 098
ISO 9001 CERTIFIED QUALITY SYSTEM

Notes
- Use written dimensions only
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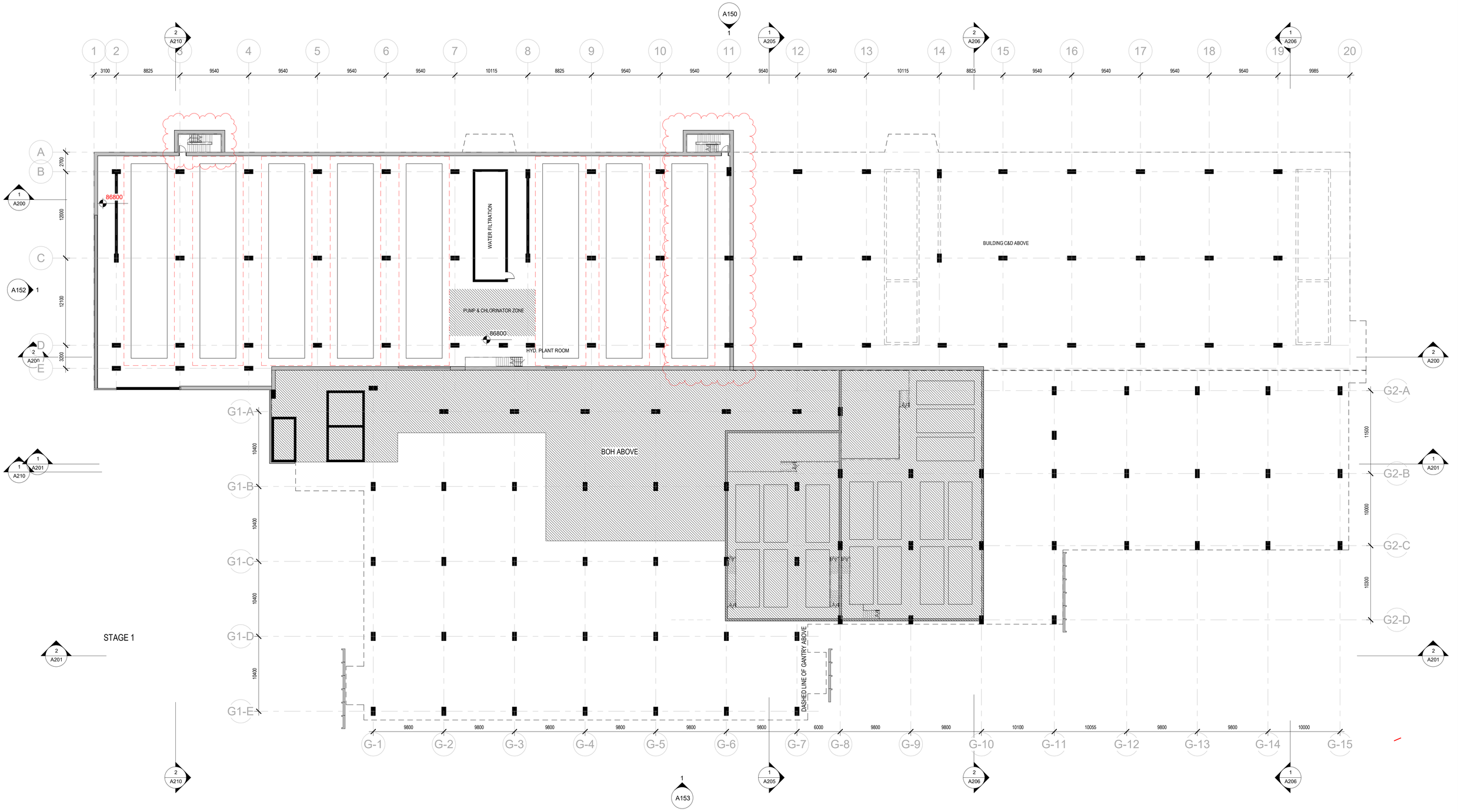
Client
GreenSquareDC™

Project
Norwest DC
3 Brookhollow Avenue
Baulkham Hills
NSW

Scale	1 : 1000
Sheet	A1
Scale Bar	0 10 20 30 50m
Drawn By	SP, BC
Project Status	DRAFT
Checked By	EX
Drawing Title	GFA CALCULATION DIAGRAMS
Approved By	GP
Job Number	230062
Drawing Number	A062
Issue	1



Issue	Date	Description
1	03/10/2025	ISSUED FOR INFORMATION
2	20/10/2025	ISSUED FOR INFORMATION
3	21/11/2025	ISSUE FOR INFORMATION
4	28/11/2025	ISSUE FOR INFORMATION

Key Plan

Project Manager: area3 — A BGIS Company

Client: Green SquareDC — HOME OF AI

Services and Lead Consultant: aurecon — AURECON

Structural and Civil: TTW

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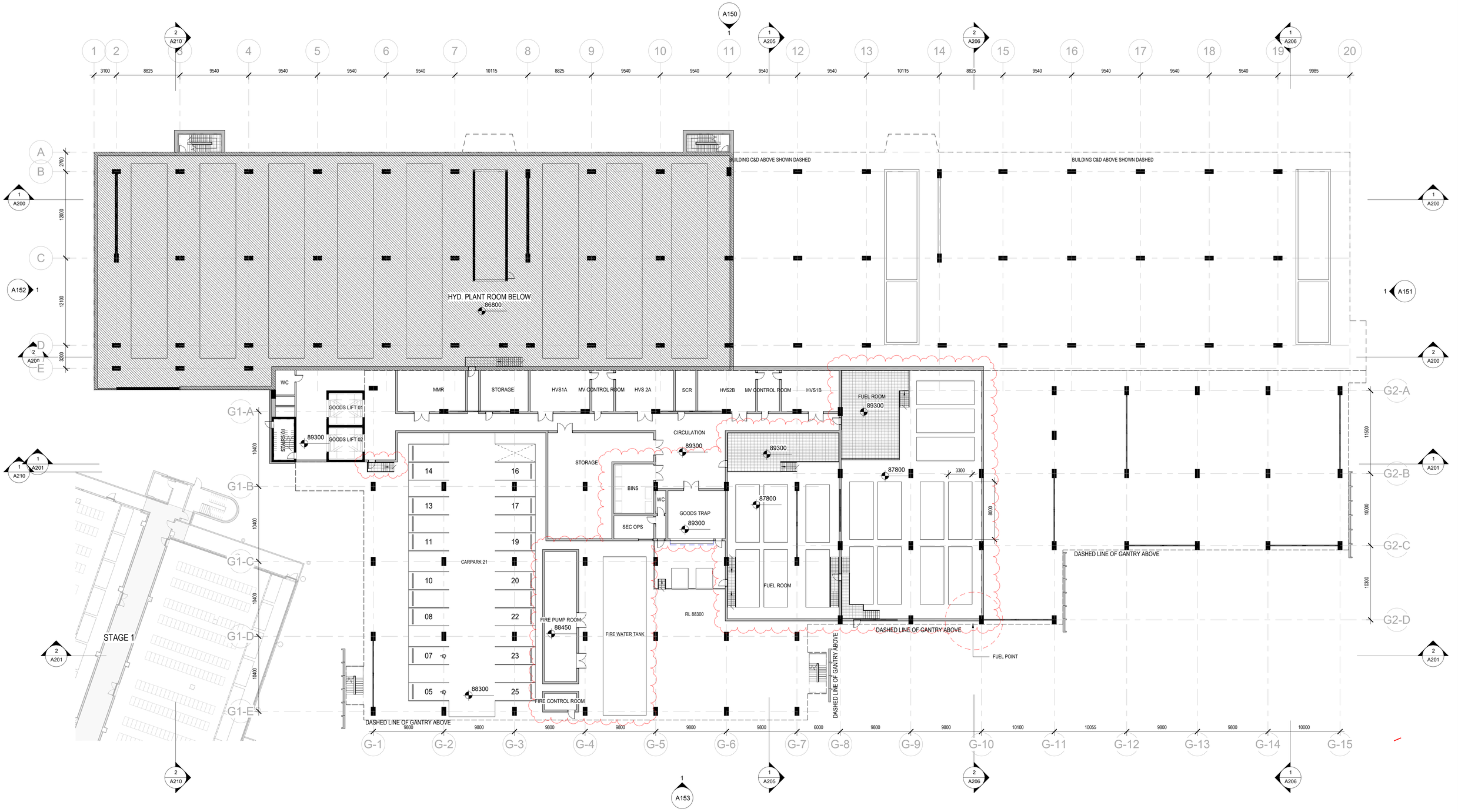
Client: GreenSquareDC™

Project: Norwest DC
3 Brookhollow Avenue
Baulkham Hills
NSW

Scale	1 : 250
Sheet	A1
Drawn By	SP, BC
Checked By	EX
Approved By	GP
Job Number	230062
Scale Bar	1:250 0 3.75 6.25 12.5m
Project Status	DRAFT
Drawing Title	LEVEL 01 - HYD PLANT
Drawing Number	A100
Issue	4

A1



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A1

Issue	Date	Description
1	03/10/2025	ISSUED FOR INFORMATION
2	20/10/2025	ISSUED FOR INFORMATION
3	21/11/2025	ISSUE FOR INFORMATION
4	28/11/2025	ISSUE FOR INFORMATION

Key Plan

Project Manager
areа3
A BGIS Company

Client
GreenSquareDC
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Client
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Project
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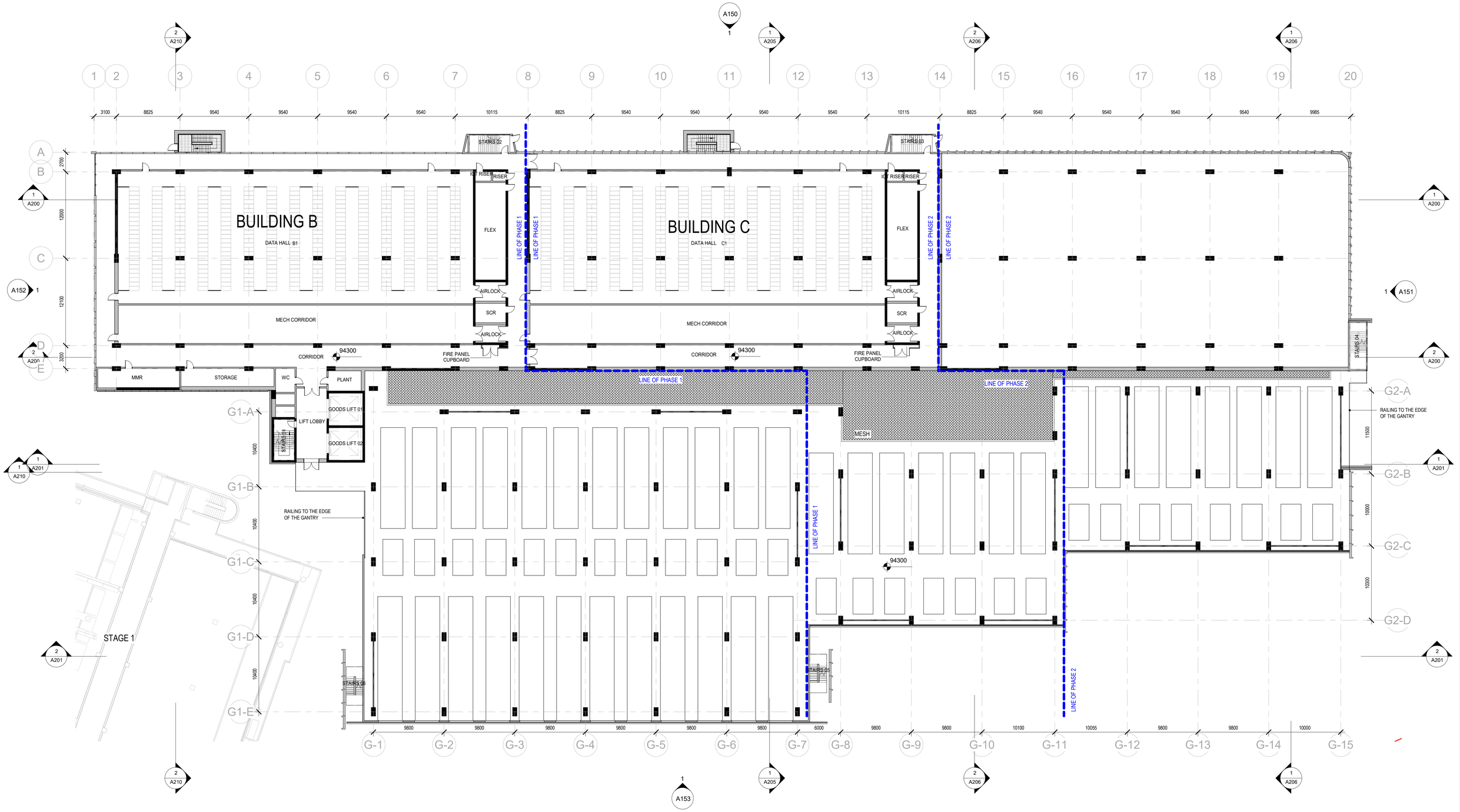
Baulkham Hills
NSW

Scale	1 : 250
Sheet	A1
Drawn By	SP, BC
Checked By	EX
Approved By	GP
Job Number	230062
Scale Bar	1:250 0 3.75 6.25 12.5m
Project Status	DRAFT
Drawing Title	LEVEL 02 - BOH FLOOR PLAN
Drawing Number	A101
Issue	4



Issue	Date	Description
1	03/10/2025	ISSUED FOR INFORMATION
2	20/10/2025	ISSUED FOR INFORMATION
3	28/11/2025	ISSUE FOR INFORMATION

Key Plan

Project Manager
area3 — A BGIS Company

Client
GreenSquareDC — HOME OF AI

Services and Lead Consultant
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Structural and Civil
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Client: GreenSquareDC™

Project: Norwest DC
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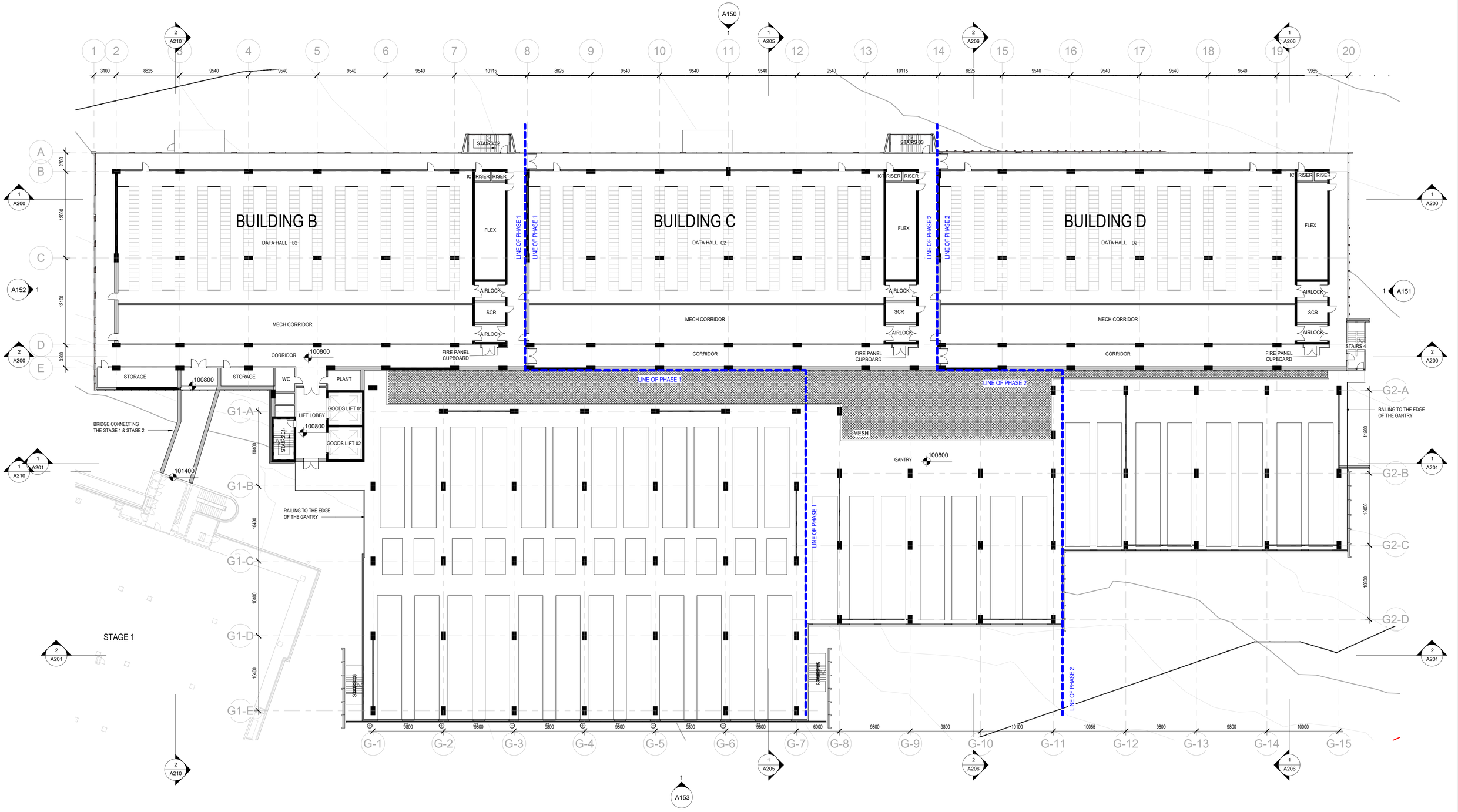
Baulkham Hills
NSW

Scale	1 : 250
Sheet	A1
Scale Bar	1:250 0 3.75 6.25 12.5m
Drawn By	SP, BC
Project Status	DRAFT
Checked By	EX
Drawing Title	LEVEL 03 - FLOOR PLAN
Approved By	GP
Job Number	230062
Drawing Number	A102
Issue	3

A1



Issue	Date	Description
1	03/10/2025	ISSUED FOR INFORMATION
2	20/10/2025	ISSUED FOR INFORMATION
3	28/11/2025	ISSUE FOR INFORMATION

Key Plan

Project Manager: area3 — A BGIS Company

Client: GreenSquareDC — HOME OF AI

Services and Lead Consultant: aurecon — AURECON

Structural and Civil: TTW

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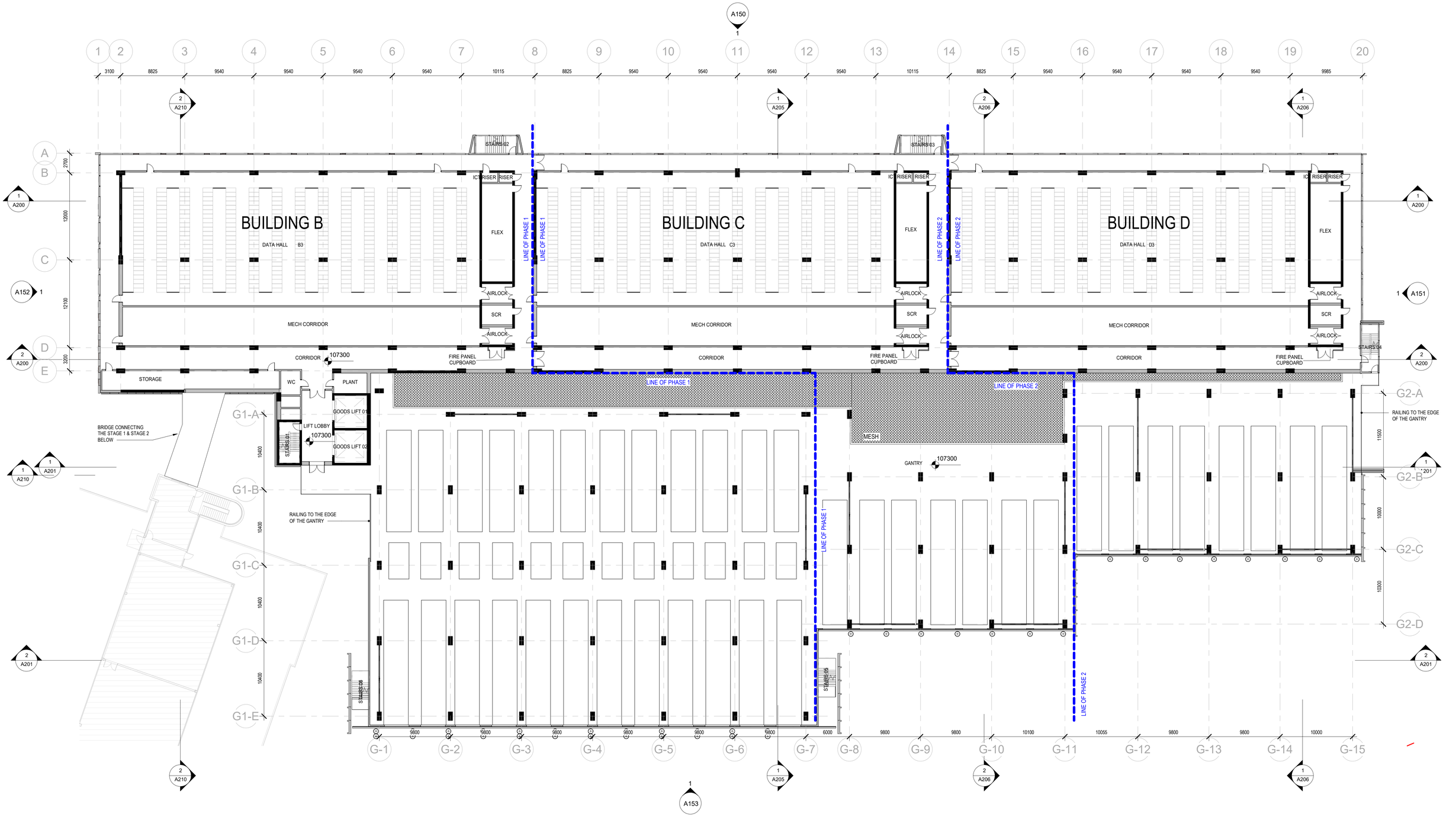
Client: GreenSquareDC™

Project: Norwest DC
3 Brookhollow Avenue
Baulkham Hills
NSW

Scale	1 : 250
Sheet	A1
Drawn By	SP, BC
Checked By	EX
Approved By	GP
Job Number	230062
Scale Bar	1:250 0 3.75 6.25 12.5m
Project Status	DRAFT
Drawing Title	LEVEL 04 - FLOOR PLAN
Drawing Number	A103
Issue	3

A1



Issue	Date	Description
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3	28/11/2025	ISSUE FOR INFORMATION

Key Plan

Project Manager: area3 — A BGIS Company

Client: GreenSquareDC — HOME OF AI

Services and Lead Consultant: aurecon — AURECON

Structural and Civil: TTW

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Client: GreenSquareDC™

Project: Norwest DC
3 Brookhollow Avenue
Baulkham Hills
NSW

Scale	1 : 250
Sheet	A1
Drawn By	SP, BC
Checked By	EX
Approved By	GP
Job Number	230062
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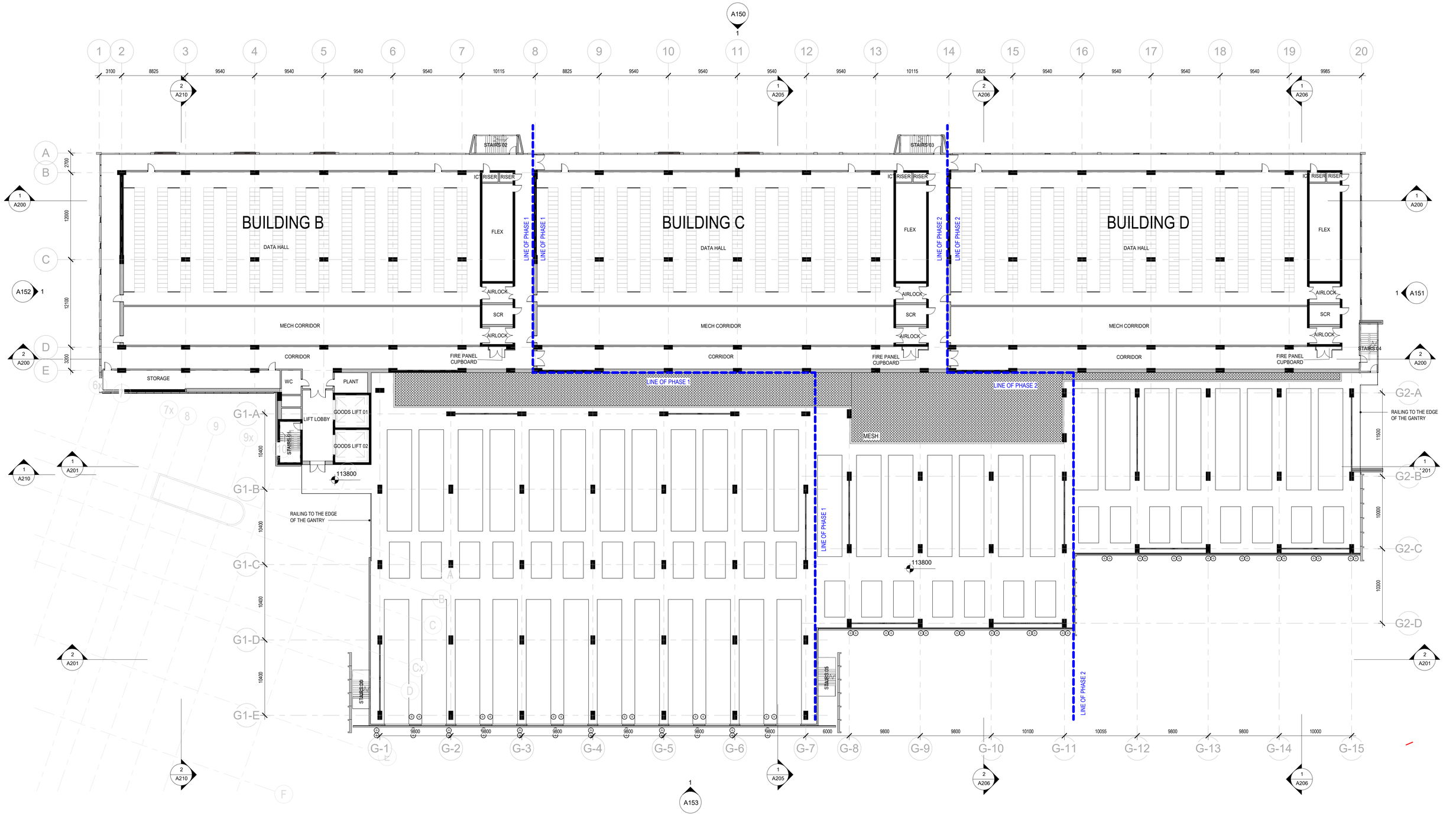
Project Status	DRAFT
Drawing Title	LEVEL 05 - FLOOR PLAN
Drawing Number	A104
Issue	3

A1



Issue	Date	Description
1	03/10/2025	ISSUED FOR INFORMATION
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3	28/11/2025	ISSUE FOR INFORMATION

Key Plan

Project Manager: area3 — A BGIS Company

Client: GreenSquareDC — HOME OF AI

Services and Lead Consultant: aurecon — AURECON

Structural and Civil: TTW

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Client: GreenSquareDC™

Project: Norwest DC
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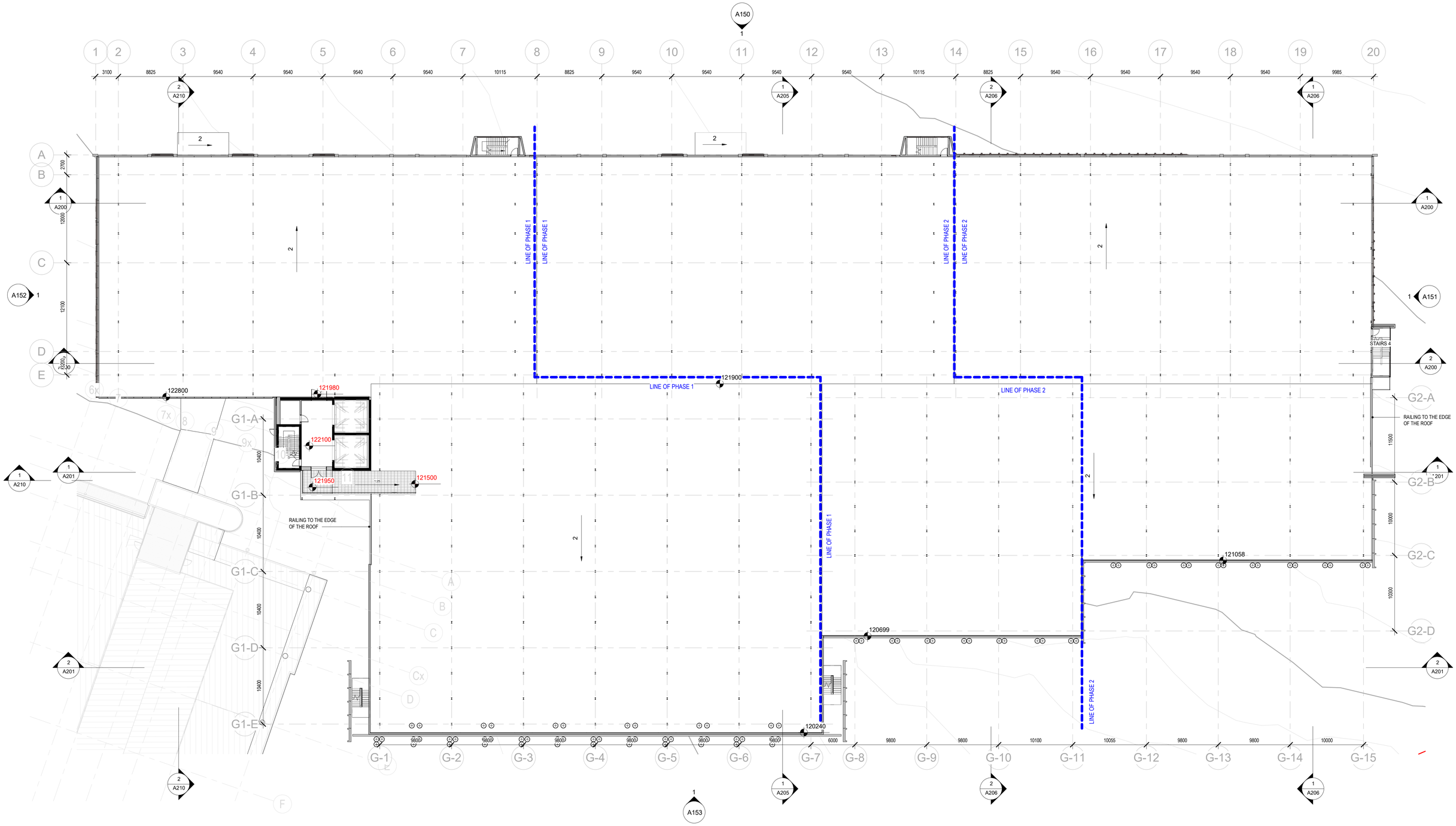
Baulkham Hills
NSW

Scale	1 : 250
Sheet	A1
Drawn By	SP, BC
Checked By	EX
Approved By	GP
Job Number	230062
Scale Bar	1:250 0 3.75 6.25 12.5m
Project Status	DRAFT
Drawing Title	LEVEL 06 - FLOOR PLAN
Drawing Number	A105
Issue	3

A1



Issue	Date	Description
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3	28/11/2025	ISSUE FOR INFORMATION

Key Plan

Project Manager
areа3 A BGIS Company

Client
Green SquareDC HOME OF AI

Services and Lead Consultant
aurecon
AURECON

Structural and Civil
TTW

Greenbox A WOOLPERT COMPANY
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68 PITT STREET
SYDNEY NSW 2000 AUSTRALIA
GREENBOX ARCHITECTURE PTY LTD
ABN: 79 139 779 098
ISO 9001 CERTIFIED QUALITY SYSTEM

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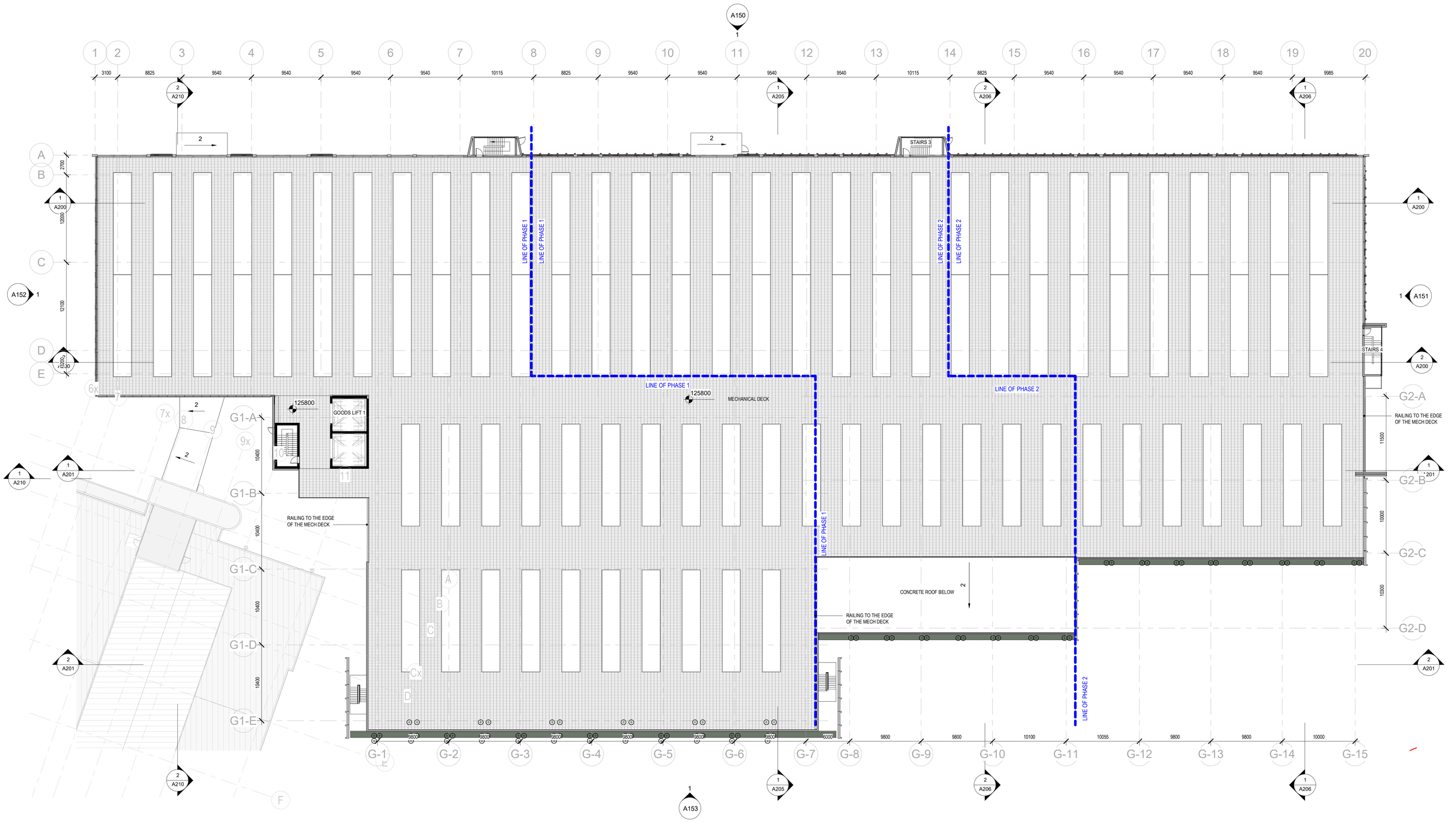
Client: GreenSquareDC™

Project: Norwest DC
3 Brookhollow Avenue
Baulkham Hills
NSW

Scale	1 : 250
Sheet	A1
Drawn By	SP, BC
Checked By	EX
Approved By	GP
Job Number	230062
Scale Bar	1:250 0 3.75 6.25 12.5m
Project Status	DRAFT
Drawing Title	LEVEL 07 - ROOF PLAN
Drawing Number	A106
Issue	3

A1



Issue	Date	Description
1	03/10/2025	ISSUED FOR INFORMATION
2	20/10/2025	ISSUED FOR INFORMATION
3	28/11/2025	ISSUE FOR INFORMATION

A1

Key Plan

Project Manager
area3
A BGIS Company

Client
GreenSquareDC
HOME OF AI

Services and Lead Consultant
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Structural and Civil
TTW

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Client
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Project
Norwest DC
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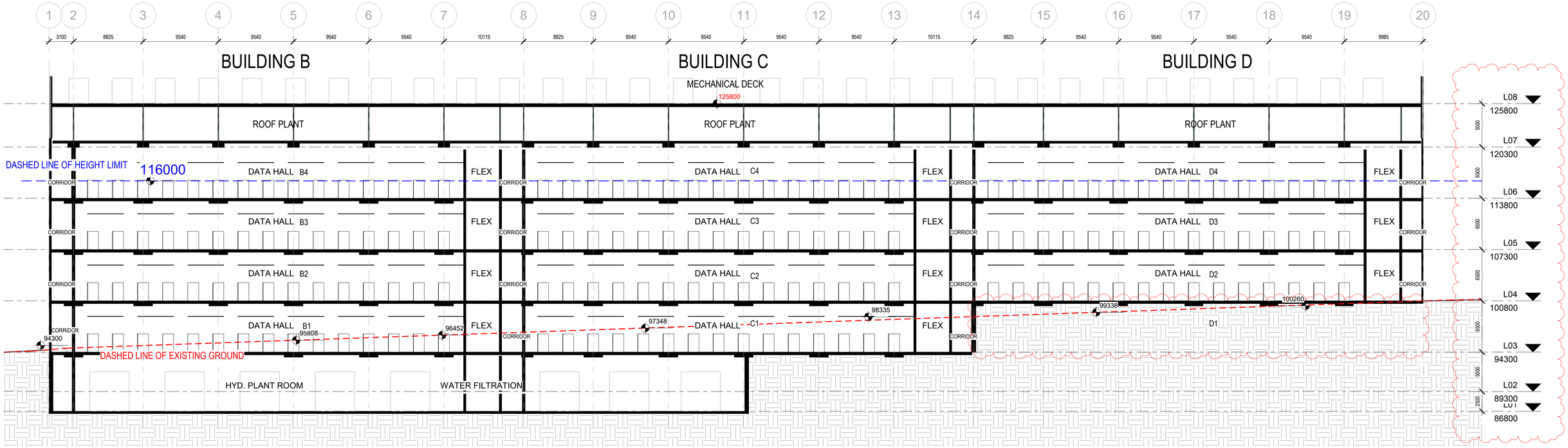
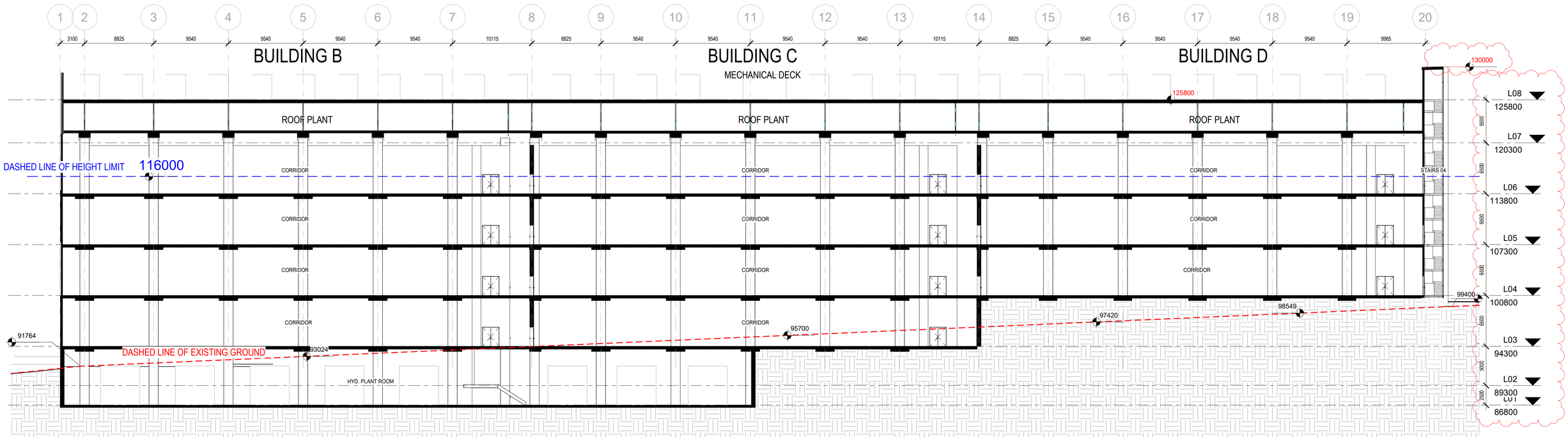
Baulkham Hills
NSW

Scale	1 : 250
Sheet	A1
Scale Bar	1:250 0 3.75 6.25 12.5m
Drawn By	SP, BC
Project Status	DRAFT
Checked By	EX
Drawing Title	LEVEL 08 - MECH DECK - ROOF PLAN
Approved By	GP
Job Number	230062
Drawing Number	A107
Issue	3



1 LONG SECTION AA
1 : 250



2 LONG SECTION BB
1 : 250

Issue	Date	Description
1	03/10/2025	ISSUED FOR INFORMATION
2	28/11/2025	ISSUE FOR INFORMATION

Key Plan

Project Manager: area3 — A BGIS Company

Client: GreenSquareDC — HOME OF AI

Services and Lead Consultant: aurecon — AURECON

Structural and Civil: TTW

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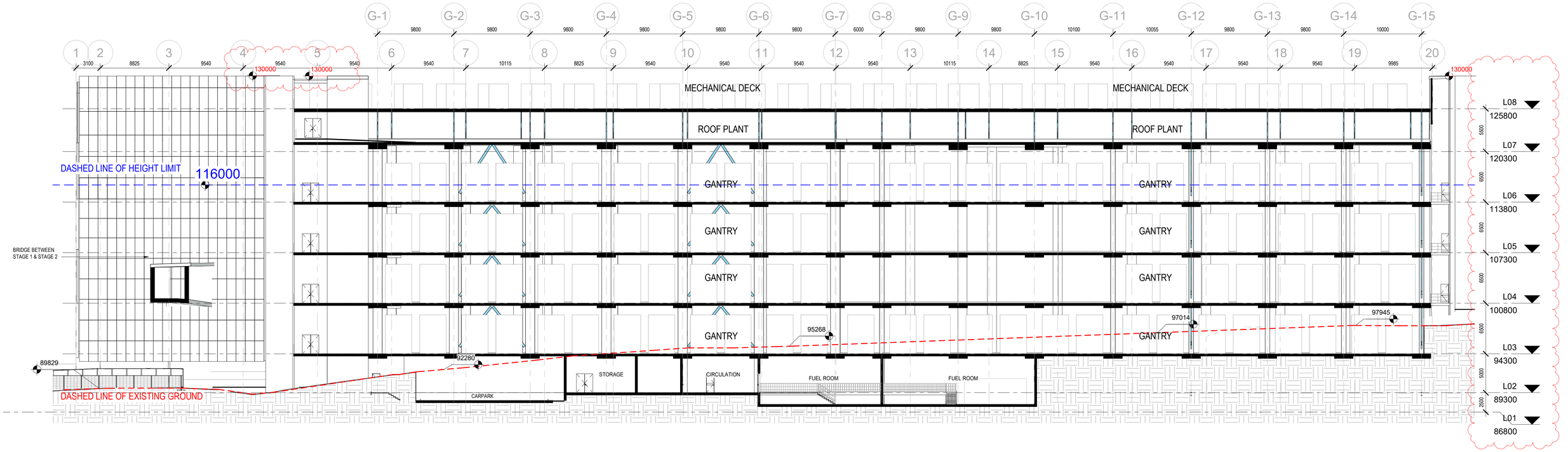
Client: GreenSquareDC™

Project: Norwest DC
3 Brookhollow Avenue
Baulkham Hills
NSW

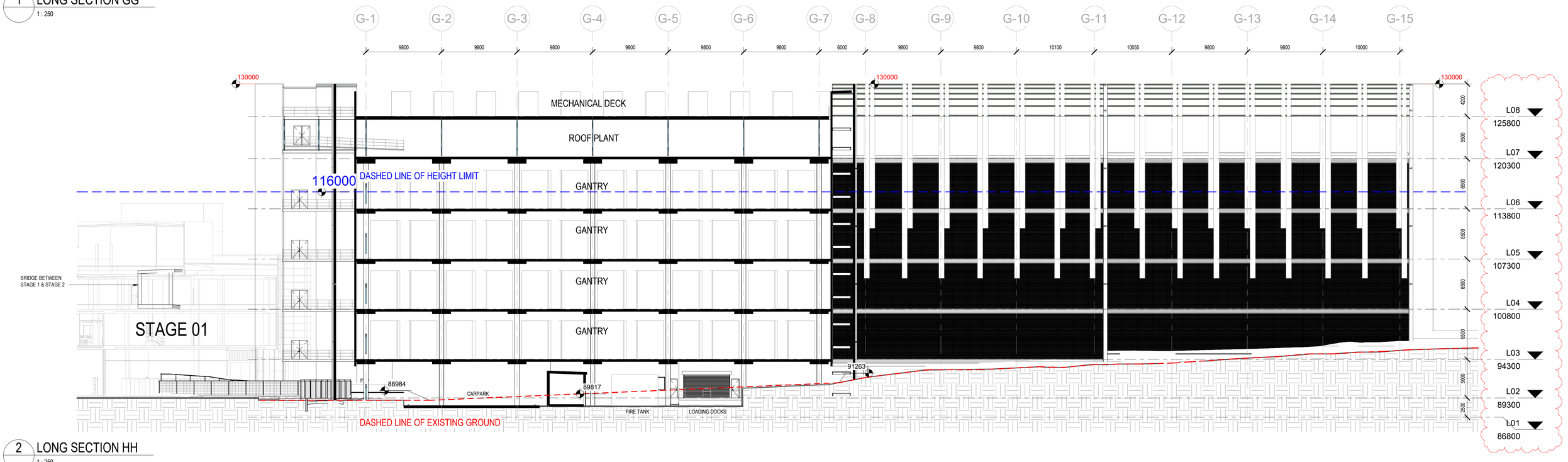
Scale	1 : 250
Sheet	A1
Scale Bar	1:250 0 3.75 6.25 12.5m
Drawn By	SP, BC
Project Status	DRAFT
Checked By	EX
Drawing Title	LONG SECTIONS SHEET 1
Approved By	GP
Job Number	230062
Drawing Number	A200
Issue	2

A1



1 LONG SECTION GG

1 : 250



2 LONG SECTION HH

1 : 250

Issue	Date	Description
1	03/10/2025	ISSUED FOR INFORMATION
2	10/10/2025	ISSUED FOR INFORMATION
3	28/11/2025	ISSUE FOR INFORMATION

Key Plan

Project Manager: area3 — A BGIS Company

Client: Green SquareDC — HOME OF AI

Services and Lead Consultant: aurecon — AURECON

Structural and Civil: TTW

Greenbox — A WOOLPERT COMPANY

+61 2 8069 8930
LEVEL 22
68 PITT STREET
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Client: GreenSquareDC™

Project: Norwest DC
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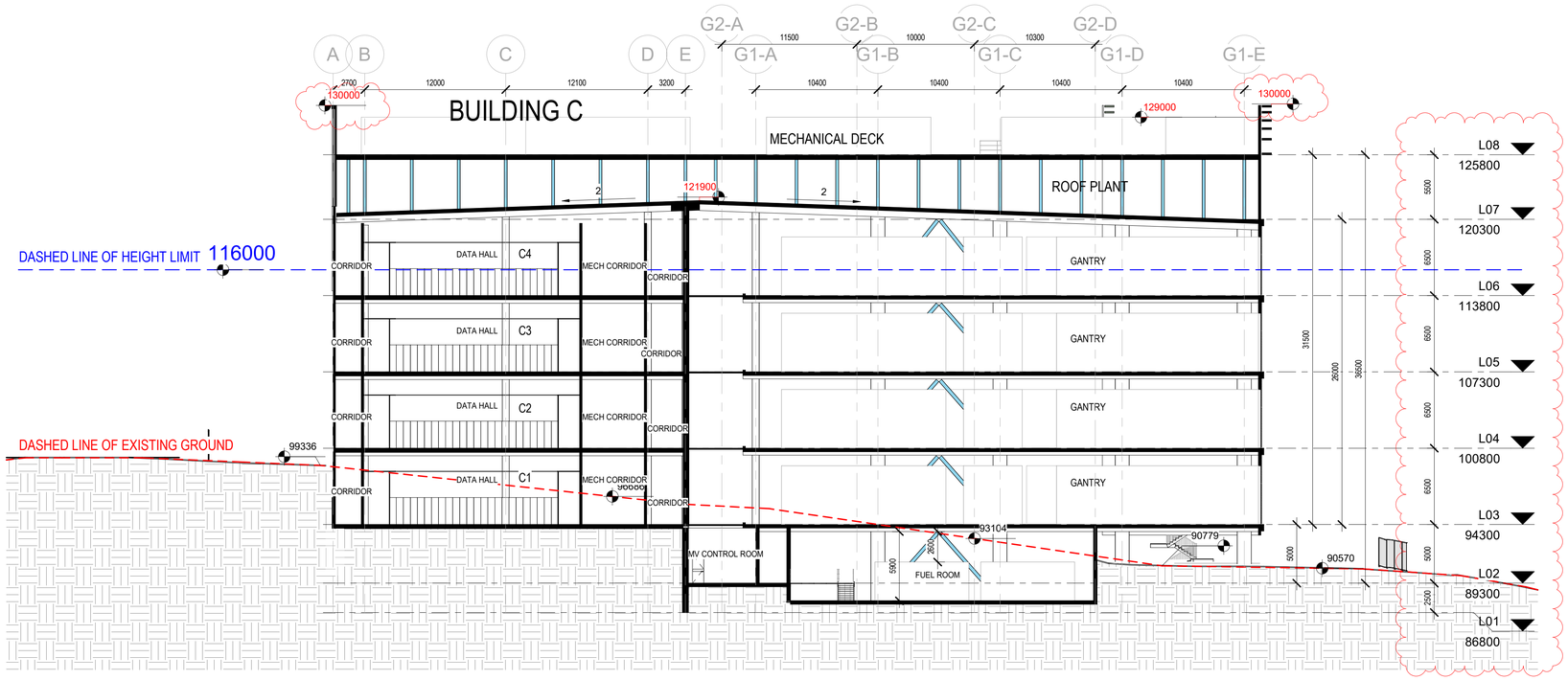
Baulkham Hills
NSW

Scale	1 : 250
Sheet	A1
Scale Bar	1:250 — 0 3.75 6.25 12.5m
Drawn By	SP, BC
Project Status	DRAFT
Checked By	EX
Drawing Title	LONG SECTIONS SHEET 2
Approved By	GP
Job Number	230062
Drawing Number	A201
Issue	3

A1



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Issue	Date	Description
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2	10/10/2025	ISSUED FOR INFORMATION
3	28/11/2025	ISSUE FOR INFORMATION

A1

Key Plan

Project Manager
area3 A BGIS Company

Client
Green SquareDC HOME OF AI

Services and Lead Consultant
aurecon
AURECON

Structural and Civil
TTW

Greenbox A WOOLPERT COMPANY
+61 2 8069 8930
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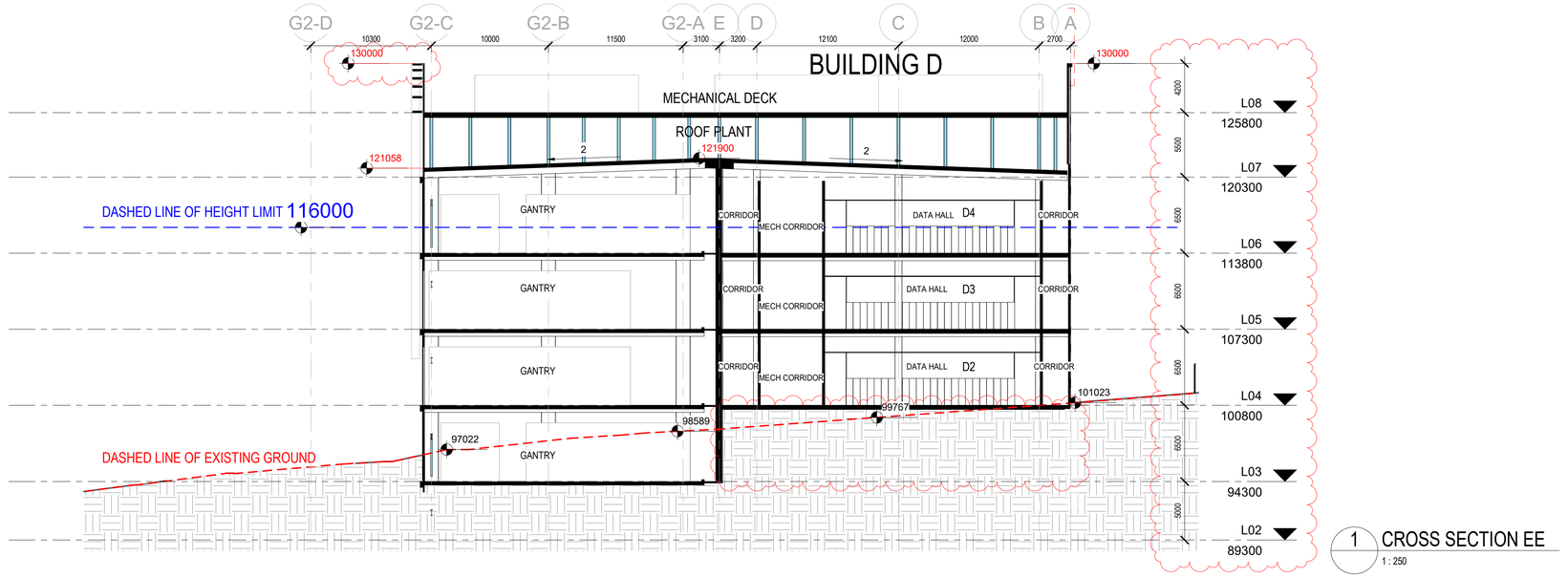
68 PITT STREET
SYDNEY NSW 2000 AUSTRALIA
GREENBOX ARCHITECTURE PTY LTD
ABN: 79 139 779 098
ISO 9001 CERTIFIED QUALITY SYSTEM

Notes
- Use written dimensions only
- Do not scale from drawing
- Contractors shall confirm all dimensions on-site prior to commencing any work or producing any shop drawings
- All materials to be used in accordance with the manufacturer's specifications and instructions and shall comply with the relevant Australian Standards
- Copyright of this drawing and design remain the property of Greenbox Architecture Pty Ltd
- Nominated Architect - Gerard Page; NSW reg No.7247, Vic reg No.17664, QLD reg No.4538

Client
GreenSquareDC™

Project
Norwest DC
3 Brookhollow Avenue
Baulkham Hills
NSW

Scale	1 : 250
Sheet	A1
Scale Bar	1:250 0 3.75 6.25 12.5m
Drawn By	SP, BC
Project Status	DRAFT
Checked By	EX
Drawing Title	CROSS SECTIONS SHEET 1
Approved By	GP
Job Number	230062
Drawing Number	A205
Issue	3



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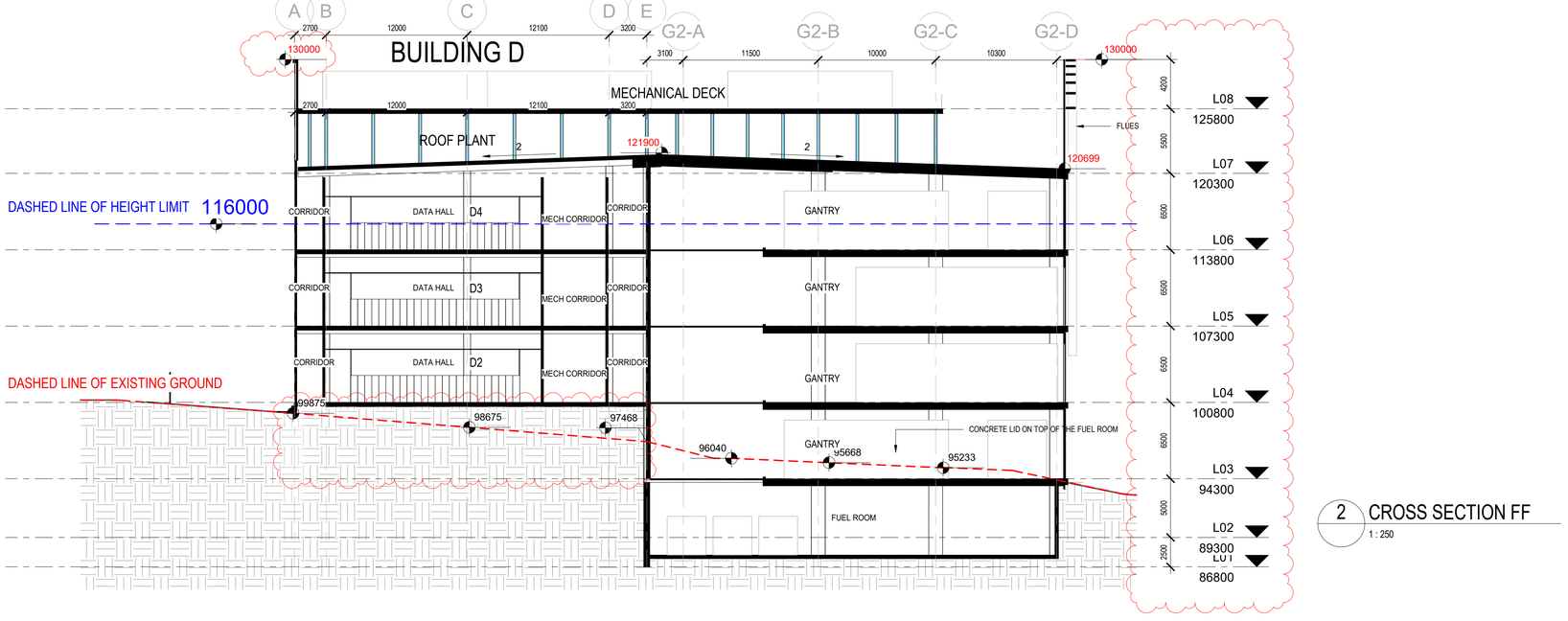

2 CROSS SECTION FF
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Issue	Date	Description
1	03/10/2025	ISSUED FOR INFORMATION
2	10/10/2025	ISSUED FOR INFORMATION
3	28/11/2025	ISSUE FOR INFORMATION

A1

Key Plan

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Client
GreenSquareDC™

Project
Norwest DC
3 Brookhollow Avenue
Baulkham Hills
NSW

Scale	1 : 250
Sheet	A1
Drawn By	SP, BC
Checked By	EX
Approved By	GP
Job Number	230062
Scale Bar	1:250 0 3.75 6.25 12.5m
Project Status	DRAFT
Drawing Title	CROSS SECTIONS SHEET 2
Drawing Number	A206
Issue	3

Appendix B

About this Report

About this Report

October 2024

Introduction

These notes have been provided to amplify Douglas' report in regard to classification methods, field procedures and the comments section. Not all are necessarily relevant to all reports.

Douglas' reports are based on information gained from limited subsurface excavations and sampling, supplemented by knowledge of local geology and experience. For this reason, they must be regarded as interpretive rather than factual documents, limited to some extent by the scope of information on which they rely.

Copyright

This report is the property of Douglas Partners Pty Ltd. The report may only be used for the purpose for which it was commissioned and in accordance with the Engagement Terms for the commission supplied at the time of proposal. Unauthorised use of this report in any form whatsoever is prohibited.

Borehole and Test Pit Logs

The borehole and test pit logs presented in this report are an engineering and/or geological interpretation of the subsurface conditions, and their reliability will depend to some extent on frequency of sampling and the method of drilling or excavation. Ideally, continuous undisturbed sampling or core drilling will provide the most reliable assessment, but this is not always practicable or possible to justify on economic grounds. In any case the boreholes and test pits represent only a very small sample of the total subsurface profile.

Interpretation of the information and its application to design and construction should therefore take into account the spacing of boreholes or pits, the frequency of sampling, and the possibility of other than 'straight line' variations between the test locations.

Groundwater

Where groundwater levels are measured in boreholes there are several potential problems, namely:

- In low permeability soils groundwater may enter the hole very slowly or perhaps not at all during the time the hole is left open;
- A localised, perched water table may lead to an erroneous indication of the true water table;
- Water table levels will vary from time to time with seasons or recent weather changes. They may not be the same at the time of construction as are indicated in the report; and
- The use of water or mud as a drilling fluid will mask any groundwater inflow. Water has to be blown out of the hole and drilling mud must first be washed out of the hole if water measurements are to be made.

More reliable measurements can be made by installing standpipes which are read at intervals over several days, or perhaps weeks for low permeability soils. Piezometers, sealed in a particular stratum, may be advisable in low permeability soils or where there may be interference from a perched water table.

Reports

The report has been prepared by qualified personnel, is based on the information obtained from field and laboratory testing, and has been undertaken to current engineering standards of interpretation and analysis. Where the report has been prepared for a specific design proposal, the information and interpretation may not be relevant if the design proposal is changed. If this happens, Douglas will be pleased to review the report and the sufficiency of the investigation work.

Every care is taken with the report as it relates to interpretation of subsurface conditions, discussion of geotechnical and environmental aspects, and recommendations or suggestions for design and construction. However, Douglas cannot always anticipate or assume responsibility for:

- Unexpected variations in ground conditions. The potential for this will depend partly on borehole or pit spacing and sampling frequency;
- Changes in policy or interpretations of policy by statutory authorities; or
- The actions of contractors responding to commercial pressures.

If these occur, Douglas will be pleased to assist with investigations or advice to resolve the matter.

About this Report

Site Anomalies

In the event that conditions encountered on site during construction appear to vary from those which were expected from the information contained in the report, Douglas requests that it be immediately notified. Most problems are much more readily resolved when conditions are exposed rather than at some later stage, well after the event.

Information for Contractual Purposes

Where information obtained from this report is provided for tendering purposes, it is recommended that all information, including the written report and discussion, be made available. In circumstances where the discussion or comments section is not relevant to the contractual situation, it may be appropriate to prepare a specially edited document. Douglas would be pleased to assist in this regard and/or to make additional report copies available for contract purposes at a nominal charge.

Site Inspection

The company will always be pleased to provide engineering inspection services for geotechnical and environmental aspects of work to which this report is related. This could range from a site visit to confirm that conditions exposed are as expected, to full time engineering presence on site.

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Appendix C

Bore logs

Terminology, Symbols and Abbreviations

November 2023

Introduction to Terminology, Symbols and Abbreviations

Douglas Partners' reports, investigation logs, and other correspondence may use terminology which has quantitative or qualitative connotations. To remove ambiguity or uncertainty surrounding the use of such terms, the following sets of notes pages may be attached Douglas Partners' reports, depending on the work performed and conditions encountered:

- Soil Descriptions;
- Rock Descriptions; and
- Sampling, insitu testing, and drilling methodologies

In addition to these pages, the following notes generally apply to most documents.

Abbreviation Codes

Site conditions may also be presented in a number of different formats, such as investigation logs, field mapping, or as a written summary. In some of these formats textual or symbolic terminology may be presented using textual abbreviation codes or graphic symbols, and, where commonly used, these are listed alongside the terminology definition. For ease of identification in these note pages, textual codes are presented in these notes in the following style `XW`. Code usage conforms with the following guidelines:

- Textual codes are case insensitive, although herein they are generally presented in upper case; and
- Textual codes are contextual (i.e. the same or similar combinations of characters may be used in different contexts with different meanings (for example `PL` is used for plastic limit in the context of soil moisture condition, as well as in `PL(A)` for point load test result in the testing results column)).

Data Integrity Codes

Subsurface investigation data recorded by Douglas Partners is generally managed in a highly structured database environment, where records "span" between a top and bottom depth interval. Depth interval "gaps" between records are considered to introduce ambiguity, and, where appropriate, our practice guidelines may require contiguous data sets. Recording meaningful data is not always appropriate (for example assigning a "strength" to a concrete pavement) and the following codes may be used to maintain contiguity in such circumstances.

Term	Description	Abbreviation Code
Core loss	No core recovery	`KL`
Unknown	Information was not available to allow classification of the property. For example, when auguring in loose, saturated sand auger cuttings may not be returned.	`UK`
No data	Information required to allow classification of the property was not available. For example, if drilling is commenced from the base of a hole predrilled by others	`ND`
Not Applicable	Derivation of the properties not appropriate or beyond the scope of the investigation. For example, providing a description of the strength of a concrete pavement	`NA`

Graphic Symbols

Douglas Partners' logs contain a "graphic" column which provides a pictorial representation of the basic composition of the material. The symbols used are directly representing the material name stated in the adjacent "Description of Strata" column, and as such no specific graphic symbology legend has been provided in these notes.

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Soil Descriptions

Terminology
Symbols
Abbreviations

November 2023

Introduction

All materials which are not considered to be "in-situ rock" are described in general accordance with the soil description model of AS 1726-2017 Part 6.1.3, and can be broken down into the following description structure:

The "classification" comprises a two character "group symbol" providing a general summary of dominant soil characteristics. The "name" summarises the particle sizes within the soil which most influence its behaviour. The detailed description presents more information about composition, condition, structure, and origin of the soil.

Classification, naming and description of soils require the relative proportion of particles of different sizes within the whole soil mixture to be considered.

Particle size designation and Behaviour Model

Solid particles within a soil are differentiated on the basis of size.

The engineering behaviour properties of a soil can subsequently be modelled to be either "fine grained" (also known as "cohesive" behaviour) or "coarse grained" ("non cohesive" behaviour), depending on the relative proportion of fine or coarse fractions in the soil mixture.

Particle Size Designation	Particle Size (mm)	Behaviour Model	
		Behaviour	Approximate Dry Mass
Boulder	>200	Excluded from particle behaviour model as "oversize"	
Cobble	63 - 200		
Gravel[1]	2.36 - 63	Coarse	>65%
Sand[1]	0.075 - 2.36		
Silt	0.002 - 0.075	Fine	>35%
Clay	<0.002		

[1] – refer grain size subdivision descriptions below

The behaviour model boundaries defined above are not precise, and the material behaviour should be assumed from the name given to the material (which considers the particle fraction which dominates the behaviour, refer "component proportions" below), rather than strict observance of the proportions of particle sizes. For example, if a material is named a "Sandy CLAY", this is indicative that the material exhibits fine grained behaviour, even if the dry mass of coarse grained material may exceed 65%.

Component proportions

The relative proportion of the dry mass of each particle size fraction is assessed to be a "primary", "secondary", or "minor" component of the soil mixture, depending on its influence over the soil behaviour.

Component Proportion Designation	Definition[1]	Relative Proportion	
		In Fine Grained Soil	In Coarse Grained Soil
Primary	The component (particle size designation, refer above) which dominates the engineering behaviour of the soil	The clay/silt component with the greater proportion	The sand/gravel component with the greater proportion
Secondary	Any component which is not the primary, but is significant to the engineering properties of the soil	Any component with greater than 30% proportion	Any granular component with greater than 30%; or Any fine component with greater than 12%
Minor[2]	Present in the soil, but not significant to its engineering properties	All other components	All other components

[1] As defined in AS1726-2017 6.1.4.4

[2] In the detailed material description, minor components are split into two further sub-categories. Refer "identification of minor components" below.

Composite Materials

In certain situations, a lithology description may describe more than one material, for example, collectively describing a layer of interbedded sand and clay. In such a scenario, the two materials would be described independently, with the names preceded or followed by a statement describing the arrangement by which the materials co-exist. For example, "INTERBEDDED Silty CLAY AND SAND".

Classification

The soil classification comprises a two character group symbol. The first character identifies the primary component. The second character identifies either the grading or presence of fines in a coarse grained soil, or the plasticity in a fine grained soil. Refer AS1726-2017 6.1.6 for further clarification.

Soil Name

For most soils, the name is derived with the primary component included as the noun (in upper case), preceded by any secondary components stated in an adjective form. In this way, the soil name also describes the general composition and indicates the dominant behaviour of the material.

Component [1]	Prominence in Soil Name
Primary	Noun (eg "CLAY")
Secondary	Adjective modifier (eg "Sandy")
Minor	No influence

[1] – for determination of component proportions, refer component proportions on previous page

For materials which cannot be disaggregated, or which are not comprised of rock or mineral fragments, the names "ORGANIC MATTER" or "ARTIFICIAL MATERIAL" may be used, in accordance with AS1726-2017 Table 14.

Commercial or colloquial names are not used for the soil name where a component derived name is possible (for example "Gravelly SAND" rather than "CRACKER DUST").

Materials of "fill" or "topsoil" origin are generally assigned a name derived from the primary/secondary component (where appropriate). In log descriptions this is preceded by uppercase "FILL" or "TOPSOIL". Origin uncertainty is indicated in the description by the characters (?), with the degree of uncertainty described (using the terms "probably" or "possibly" in the origin column, or at the end of the description).

Identification of minor components

Minor components are identified in the soil description immediately following the soil name. The minor component fraction is usually preceded with a term indicating the relative proportion of the component.

Minor Component Proportion Term	Relative Proportion	
	In Fine Grained Soil	In Coarse Grained Soil
With	All fractions: 15-30%	Clay/silt: 5-12% sand/gravel: 15-30%
Trace	All fractions: 0-15%	Clay/silt: 0-5% sand/gravel: 0-15%

The terms "with" and "trace" generally apply only to gravel or fine particle fractions. Where cobbles/boulders are encountered in minor proportions (generally less than about 12%) the term "occasional" may be used. This term describes the sporadic distribution of the material within the confines of the investigation excavation only, and there may be considerable variation in proportion over a wider area which is difficult to factually characterise due to the relative size of the particles and the investigation methods.

Soil Composition

Plasticity

Descriptive Term	Laboratory liquid limit range	
	Silt	Clay
Non-plastic materials	Not applicable	Not applicable
Low plasticity	≤50	≤35
Medium plasticity	Not applicable	>35 and ≤50
High plasticity	>50	>50

Note, Plasticity descriptions generally describe the plasticity behaviour of the whole of the fine grained soil, not individual fine grained fractions.

Grain Size

Type		Particle size (mm)
Gravel	Coarse	19 - 63
	Medium	6.7 - 19
	Fine	2.36 – 6.7
Sand	Coarse	0.6 - 2.36
	Medium	0.21 - 0.6
	Fine	0.075 - 0.21

Grading

Grading Term	Particle size (mm)
Well	A good representation of all particle sizes
Poorly	An excess or deficiency of particular sizes within the specified range
Uniformly	Essentially of one size
Gap	A deficiency of a particular size or size range within the total range

Note, AS1726-2017 provides terminology for additional attributes not listed here.

Soil Condition

Moisture

The moisture condition of soils is assessed relative to the plastic limit for fine grained soils, while for coarse grained soils it is assessed based on the appearance and feel of the material. The moisture condition of a material is considered to be independent of stratigraphy (although commonly these are related), and this data is presented in its own column on logs.

Applicability	Term	Tactile Assessment	Abbreviation code
Fine	Dry of plastic limit	Hard and friable or powdery	w<PL
	Near plastic limit	Can be moulded	w=PL
	Wet of plastic limit	Water residue remains on hands when handling	w>PL
	Near liquid limit	"oozes" when agitated	w=LL
	Wet of liquid limit	"oozes"	w>LL
Coarse	Dry	Non-cohesive and free running	D
	Moist	Feels cool, darkened in colour, particles may stick together	M
	Wet	Feels cool, darkened in colour, particles may stick together, free water forms when handling	W

The abbreviation code NDF, meaning "not-assessable due to drilling fluid use" may also be used.

Note, observations relating to free ground water or drilling fluids are provided independent of soil moisture condition.

Consistency/Density/Compaction/Cementation/Extremely Weathered Material

These concepts give an indication of how the material may respond to applied forces (when considered in conjunction with other attributes of the soil). This behaviour can vary independent of the composition of the material, and on logs these are described in an independent column and are generally mutually exclusive (i.e it is inappropriate to describe both consistency and compaction at the same time). The method by which the behaviour is described depends on the behaviour model and other characteristics of the soil as follows:

- In fine grained soils, the "consistency" describes the ease with which the soil can be remoulded, and is generally correlated against the materials undrained shear strength;
- In granular materials, the relative density describes how tightly packed the particles are, and is generally correlated against the density index;
- In anthropogenically modified materials, the compaction of the material is described qualitatively;
- In cemented soils (both natural and anthropogenic), the cemented "strength" is described qualitatively, relative to the difficulty with which the material is disaggregated; and
- In soils of extremely weathered material origin, the engineering behaviour may be governed by relic rock features, and expected behaviour needs to be assessed based the overall material description.

Quantitative engineering performance of these materials may be determined by laboratory testing or estimated by correlated field tests (for example penetration or shear vane testing). In some cases, performance may be assessed by tactile or other subjective methods, in which case investigation logs will show the estimated value enclosed in round brackets, for example (VS).

Consistency (fine grained soils)

Consistency Term	Tactile Assessment	Undrained Shear Strength (kPa)	Abbreviation Code
Very soft	Extrudes between fingers when squeezed	<12	VS
Soft	Mouldable with light finger pressure	>12 - ≤25	S
Firm	Mouldable with strong finger pressure	>25 - ≤50	F
Stiff	Cannot be moulded by fingers	>50 - ≤100	St
Very stiff	Indented by thumbnail	>100 - ≤200	VSt
Hard	Indented by thumbnail with difficulty	>200	H
Friable	Easily crumbled or broken into small pieces by hand	-	Fr

Relative Density (coarse grained soils)

Relative Density Term	Density Index	Abbreviation Code
Very loose	<15	VL
Loose	>15 - ≤35	L
Medium dense	>35 - ≤65	MD
Dense	>65 - ≤85	D
Very dense	>85	VD

Note, tactile assessment of relative density is difficult, and generally requires penetration testing, hence a tactile assessment guide is not provided.

Compaction (anthropogenically modified soil)

Compaction Term	Abbreviation Code
Well compacted	WC
Poorly compacted	PC
Moderately compacted	MC
Variably compacted	VC

Cementation (natural and anthropogenic)

Cementation Term	Abbreviation Code
Moderately cemented	MOD
Weakly cemented	WEK

Extremely Weathered Material

AS1726-2017 considers weathered material to be soil if the unconfined compressive strength is less than 0.6 MPa (i.e. less than very low strength rock). These materials may be identified as "extremely weathered material" in reports and by the abbreviation code XWM on log sheets. This identification is not correlated to any specific qualitative or quantitative behaviour, and the engineering properties of this material must therefore be assessed according to engineering principles with reference to any relic rock structure, fabric, or texture described in the description.

Soil Origin

Term	Description	Abbreviation Code
Residual	Derived from in-situ weathering of the underlying rock	RS
Extremely weathered material	Formed from in-situ weathering of geological formations. Has strength of less than 'very low' as per as1726 but retains the structure or fabric of the parent rock.	XWM
Alluvial	Deposited by streams and rivers	ALV
Estuarine	Deposited in coastal estuaries	EST
Marine	Deposited in a marine environment	MAR
Lacustrine	Deposited in freshwater lakes	LAC
Aeolian	Carried and deposited by wind	AEO
Colluvial	Soil and rock debris transported down slopes by gravity	COL
Slopewash	Thin layers of soil and rock debris gradually and slowly deposited by gravity and possibly water	SW
Topsoil	Mantle of surface soil, often with high levels of organic material	TOP
Fill	Any material which has been moved by man	FILL
Littoral	Deposited on the lake or seashore	LIT
Unidentifiable	Not able to be identified	UID

Cobbles and Boulders

The presence of particles considered to be "oversize" may be described using one of the following strategies:

- Oversize encountered in a minor proportion (when considered relative to the wider area) are noted in the soil description; or
- Where a significant proportion of oversize is encountered, the cobbles/boulders are described independent of the soil description, in a similar manner to composite soils (described above) but qualified with "MIXTURE OF".

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Rock Strength

Rock strength is defined by the unconfined compressive strength, and it refers to the strength of the rock substance and not the strength of the overall rock mass, which may be considerably weaker due to defects.

The Point Load Strength Index $I_{s(50)}$ is commonly used to provide an estimate of the rock strength and site specific correlations should be developed to allow UCS values to be determined. The point load strength test procedure is described by Australian Standard AS4133.4.1-2007. The terms used to describe rock strength are as follows:

Strength Term	Unconfined Compressive Strength (MPa)	Point Load Index[1] $I_{s(50)}$ MPa	Abbreviation Code
Very low	0.6 - 2	0.03 - 0.1	VL
Low	2 - 6	0.1 - 0.3	L
Medium	6 - 20	0.3 - 1.0	M
High	20 - 60	1 - 3	H
Very high	60 - 200	3 - 10	VH
Extremely high	>200	>10	EH

[1] Rock strength classification is based on UCS. The UCS to $I_{s(50)}$ ratio varies significantly for different rock types and specific ratios may be required for each site. The point load Index ranges shown above are as suggested in AS1726 and should not be relied upon without supporting evidence.

The following abbreviation codes are used for soil layers or seams of material "within rock" but for which the equivalent UCS strength is less than 0.6 MPa.

Scenario	Abbreviation Code
The material encountered has an equivalent UCS strength of less than 0.6 MPa, and therefore is considered to be soil (as per Note 1 of Table 20 of AS 1726-2017). The properties of the material encountered over this interval are described in the "Description of Strata" and soil properties columns.	SOIL
The material encountered has an equivalent UCS strength of less than 0.6 MPa, and therefore is considered to be soil (as per Note 1 of Table 20 of AS 1726-2017). The prominence of the material is such that it can be considered to be a seam (as defined in Table 22 of AS1726-2017) and the properties of the material are described in the defect column.	SEAM

Degree of Weathering

The degree of weathering of rock is classified as follows:

Weathering Term	Description	Abbreviation Code
Residual Soil[1]	Material is weathered to such an extent that it has soil properties. Mass structure and material texture and fabric of original rock are no longer visible, but the soil has not been significantly transported.	RS
Extremely weathered[1]	Material is weathered to such an extent that it has soil properties. Mass structure and material texture and fabric of original rock are still visible	XW
Highly weathered	The whole of the rock material is discoloured, usually by iron staining or bleaching to the extent that the colour of the original rock is not recognisable. Rock strength is significantly changed by weathering. Some primary minerals have weathered to clay minerals. Porosity may be increased by leaching or may be decreased due to deposition of weathering products in pores.	HW
Moderately weathered	The whole of the rock material is discoloured, usually by iron staining or bleaching to the extent that the colour of the original rock is not recognisable but shows little or no change of strength from fresh rock.	MW
Slightly weathered	Rock is partially discoloured with staining or bleaching along joints but shows little or no change of strength from fresh rock.	SW
Fresh	No signs of decomposition or staining.	FR
Note: If HW and MW cannot be differentiated use DW (see below)		
Distinctly weathered	Rock strength usually changed by weathering. The rock may be highly discoloured, usually by iron staining. Porosity may be increased by leaching or may be decreased due to deposition of weathered products in pores.	DW

[1] The parent rock type, of which the residual/extremely weathered material is a derivative, will be stated in the description (where discernible).

Degree of Alteration

The degree of alteration of the rock material (physical or chemical changes caused by hot gasses or liquids at depth) is classified as follows:

Term	Description	Abbreviation Code
Extremely altered	Material is altered to such an extent that it has soil properties. Mass structure and material texture and fabric of original rock are still visible.	XA
Highly altered	The whole of the rock material is discoloured, usually by staining or bleaching to the extent that the colour of the original rock is not recognisable. Rock strength is changed by alteration. Some primary minerals are altered to clay minerals. Porosity may be increased by leaching or may be decreased due to precipitation of secondary materials in pores.	HA
Moderately altered	The whole of the rock material is discoloured, usually by staining or bleaching to the extent that the colour of the original rock is not recognisable but shows little or no change of strength from fresh rock.	MA
Slightly altered	Rock is slightly discoloured but shows little or no change of strength from fresh rock	SA
Note: If HA and MA cannot be differentiated use DA (see below)		
Distinctly altered	Rock strength usually changed by alteration. The rock may be highly discoloured, usually by staining or bleaching. Porosity may be increased by leaching or may be decreased due to precipitation of secondary minerals in pores.	DA

Degree of Fracturing

The following descriptive classification apply to the spacing of natural occurring fractures in the rock mass. It includes bedding plane partings, joints and other defects, but excludes drilling breaks. These terms are generally not required on investigation logs where fracture spacing is presented as a histogram, and where used are presented in an unabbreviated format.

Term	Description
Fragmented	Fragments of <20 mm
Highly Fractured	Core lengths of 20-40 mm with occasional fragments
Fractured	Core lengths of 30-100 mm with occasional shorter and longer sections
Slightly Fractured	Core lengths of 300 mm or longer with occasional sections of 100-300 mm
Unbroken	Core contains very few fractures

Rock Quality Designation

The quality of the cored rock can be measured using the Rock Quality Designation (RQD) index, defined as:

$$\text{RQD \%} = \frac{\text{cumulative length of 'sound' core sections > 100 mm long}}{\text{total drilled length of section being assessed}}$$

where 'sound' rock is assessed to be rock of low strength or stronger. The RQD applies only to natural fractures. If the core is broken by drilling or handling (i.e., drilling breaks) then the broken pieces are fitted back together and are not included in the calculation of RQD.

Stratification Spacing

These terms may be used to describe the spacing of bedding partings in sedimentary rocks. Where used, these terms are generally presented in an unabbreviated format

Term	Separation of Stratification Planes
Thinly laminated	< 6 mm
Laminated	6 mm to 20 mm
Very thinly bedded	20 mm to 60 mm
Thinly bedded	60 mm to 0.2 m
Medium bedded	0.2 m to 0.6 m
Thickly bedded	0.6 m to 2 m
Very thickly bedded	> 2 m

Defect Descriptions

Defect Type

Term	Abbreviation Code
Bedding plane	B
Infilled seam	IS
Cleavage	CV
Crushed zone	CZ
Decomposed seam	DS
Fault	F
Joint	JT
Lamination	LAM
Parting	P
Shear zone	SZ
Vein	VN
Drilling/handling break	DB , HB
Fracture	FC

Rock Defect Orientation

Term	Abbreviation Code
Horizontal	H
Vertical	V
Sub-horizontal	SH
Sub-vertical	SV

Rock Defect Coating

Term	Abbreviation Code
Clean	CN
Coating	CT
Healed	HE
Infilled	INF
Stained	SN
Tight	TI
Veneer	VNR

Rock Defect Infill

Term	Abbreviation Code
Calcite	CA
Carbonaceous	CBS
Clay	CLAY
Iron oxide	FE
Manganese	MN

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Rock Defect Shape/Planarity

Term	Abbreviation Code
Curved	CU
Irregular	IR
Planar	PR
Stepped	ST
Undulating	UN

Rock Defect Roughness

Term	Abbreviation Code
Polished	PO
Rough	RF
Slickensided	SL
Smooth	SM
Very rough	VR

Defect Orientation

The inclination of defects is always measured from the perpendicular to the core axis.

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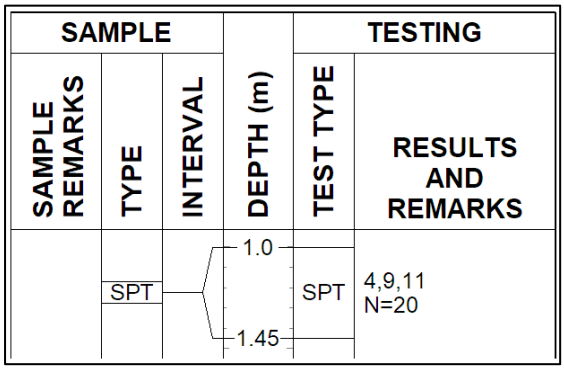
Sampling, Testing and Excavation Methodology

Terminology
Symbols
Abbreviations

November 2023

Sampling and Testing

A record of samples retained, and field testing performed is usually shown on a Douglas Partners' log with samples appearing to the left of a depth scale, and selected field and laboratory testing (including results, where relevant) appearing to the right of the scale, as illustrated below:



Sampling

The type or intended purpose for which a sample was taken is indicated by the following abbreviation codes.

Sample Type	Code
Auger sample	A
Bulk sample	B
Core sample	C
Disturbed sample	D
Sample from SPT test	SPT
Environmental sample	ES
Gas sample	G
Undisturbed tube sample	U[1]
Water sample	W
Piston sample	P
Core sample for unconfined compressive strength testing	UCS
Material Sample	MT

[1] – numeric suffixes indicate tube diameter/width in mm

The above codes only indicate that a sample was retained, and not that testing was scheduled or performed.

Field and Laboratory Testing

A record that field and laboratory testing was performed is indicated by the following abbreviation codes.

Test Type	Code
Pocket penetrometer (kPa)	PP
Photo ionisation detector (ppm)	PID
Standard Penetration Test x/y =x blows for y mm penetration HB = hammer bouncing HW = fell under weight of hammer	SPT
Shear vane (kPa)	V
Unconfined compressive strength, (MPa)	UCS

Field and laboratory testing (continued)

Test Type	Code
Point load test, (MPa), axial (A), diametric (D), irregular (I)	PLT(_)
Dynamic cone penetrometer, followed by blow count penetration increment in mm (cone tip, generally in accordance with AS1289.6.3.2)	DCP/150
Perth sand penetrometer, followed by blow count penetration increment in mm (flat tip, generally in accordance with AS1289.6.3.3)	PSP/150

Groundwater Observations

▷	seepage/inflow
▽	standing or observed water level
NFGWO	no free groundwater observed
OBS	observations obscured by drilling fluids

Drilling or Excavation Methods/Tools

The drilling/excavation methods used to perform the investigation may be shown either in a dedicated column down the left-hand edge of the log, or stated in the log footer. In some circumstances abbreviation codes may be used.

Method	Abbreviation Code
Toothed bucket	TB[1]
Mud/blade bucket	MB[1]
Ripping tyne/ripper	R
Rock breaker/hydraulic hammer	RB
Hand auger	HA[1]
NMLC series coring	NMLC
HMLC series coring	HMLC
NQ coring	NQ3
HQ coring	HQ3
PQ coring	PQ3
Push tube	PT[1]
Rock roller	RR[1]
Solid flight auger. Suffixes: /T = tungsten carbide tip, /V = v-shaped tip	AD[1]
Sonic drilling	SON[1]
Vibrocore	VC[1]
Wash bore (unspecified bit type)	WB[1]
Existing exposure	X
Hand tools (unspecified)	HAND
Predrilled	PD
Diatube	DT[1]
Hollow flight auger	HSA[1]
Vacuum excavation	VE

[1] – numeric suffixes indicate tool diameter/width in mm

Douglas PARTNERS

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.1 AHD
COORDINATE: E:311663.0, N:6265701.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH101
PROJECT No: 233937.00
DATE: 22/05/25
SHEET: 1 of 2

CONDITIONS ENCOUNTERED								SAMPLE				TESTING AND REMARKS	
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN[#]	CONSIS.[*] / DENSITY.[*]	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
22/05/25 No Free Groundwater Observed Whilst Augering	89	0.15	TOPSOIL / Silty CLAY trace rootlets trace gravel: dark brown; medium plasticity; fine to coarse, igneous and siltstone gravel; Topsoil Fill.	TS	TOP	ND			D	0.40 - 0.50	0.40		
	88	1	FILL / CLAY with gravel: brown; medium plasticity; fine to coarse, igneous and siltstone gravel.		FILL	PC to MC	w=PL				1	SPT	3,2,3 N=5
		1.60							D	1.40 - 1.50	1.40		
		1.80	CLAY (CI): pale grey and orange-brown; medium plasticity.		XWM	H					1.50	SPT	12,18,27 N=45
	87	2				NA	NA				2	SPT	100 (HB)
		2.10	SILTSTONE: dark grey and orange-brown; inferred very low strength, generally iron stained. Ashfield shale.										
			Continued as rock log										
	86	3									3		
	85	4									4		
	84	5									5		
	83	6									6		
	82	7									7		
	81	8									8		
	80	9									9		

NOTES: [#] Soil origin is "probable" unless otherwise stated. [*] Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: Geo205
METHOD: SFA (115mm) to 2.0m, NMLC to 7.2m
REMARKS: Grass cover at surface

OPERATOR: Terratest (K.Mccorron)

LOGGED: R.Muller
CASING: HWT to 2.3m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

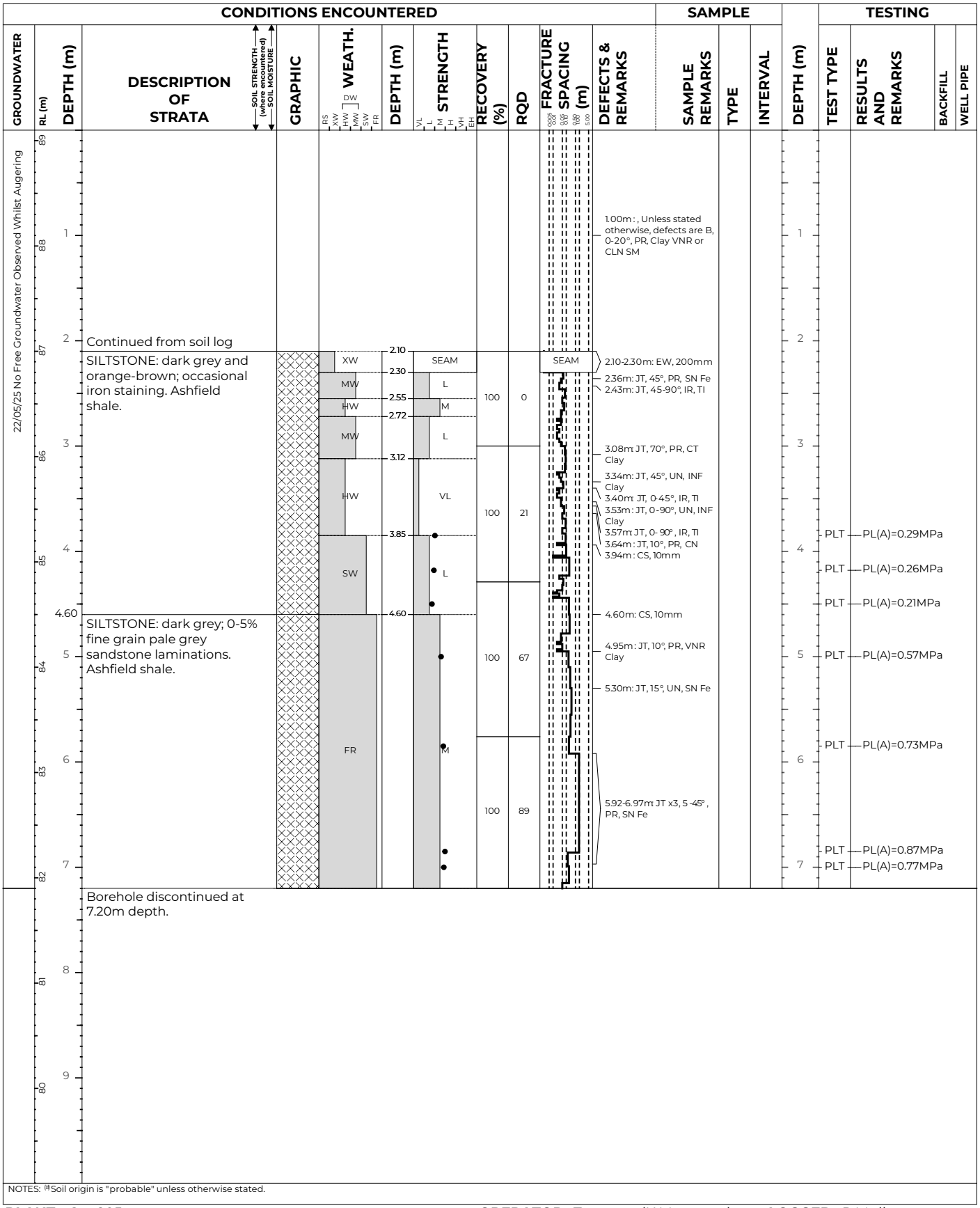
Douglas PARTNERS

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.1 AHD
COORDINATE: E:311663.0, N:6265701.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH101
PROJECT No: 233937.00
DATE: 22/05/25
SHEET: 2 of 2



NOTES: [a] Soil origin is "probable" unless otherwise stated.

PLANT: Geo205
METHOD: SFA (115mm) to 2.0m, NMLC to 7.2m
REMARKS: Grass cover at surface

OPERATOR: Terratest (K.Mccorron)

LOGGED: R.Muller
CASING: HWT to 2.3m

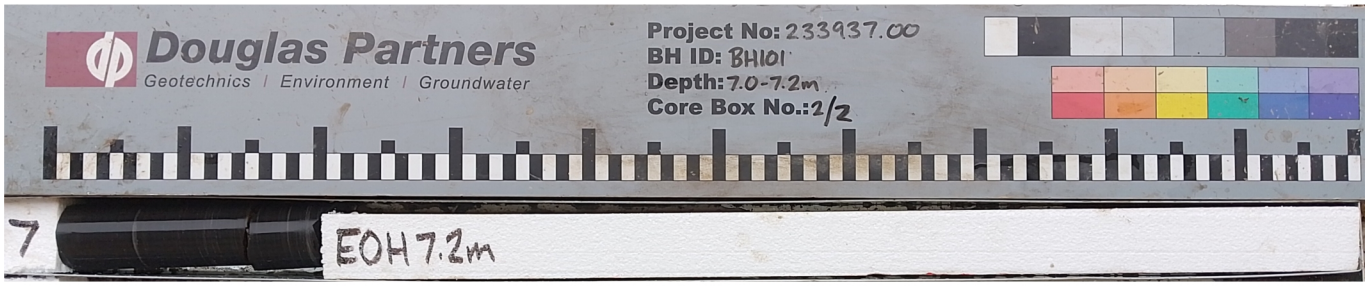
Douglas PARTNERS

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.1 AHD
COORDINATE: E:311663.0, N:6265701.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH101
PROJECT No: 233937.00
DATE: 22/05/25
SHEET: 1 of 1



Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.5 AHD
COORDINATE: E:311688.0, N:6265684.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH102
PROJECT No: 233937.00
DATE: 23/05/25
SHEET: 1 of 1

CONDITIONS ENCOUNTERED								SAMPLE			TESTING AND REMARKS	
GROUNDWATER	RL (m) / DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*) / DENSITY.(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
23/05/25 No Free Groundwater Observed Whilst Augering	89 / 0.15	CONCRETE: 150mm thick. 8mm diameter biaxial mesh at 75mm depth.			NA	NA		C		0.15	DCP9/150	5 10 15
	0.20	FILL / SAND: yellow-brown; fine to medium.		FILL	MC	D		D		0.20		
	0.40	FILL / Gravelly SAND: dark grey; fine to coarse; fine to coarse, igneous gravel.		FILL		M		D		0.30 0.40		
	88	CLAY (CI): pale grey; medium plasticity.		XWM	H	w=PL		D		0.50		
	0.60	SILTSTONE: grey; inferred very low strength, with clay seams. Ashfield Shale.			NA	NA		D		0.60 0.65 0.70		41/100mm
	1	Borehole discontinued at 0.70m depth. Hand Auger Refusal.										
	87	2										
	86	3										
	85	4										

NOTES: (#) Soil origin is "probable" unless otherwise stated. (*) Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT:
OPERATOR: R.Muller
LOGGED: R.Muller
METHOD: Diatube (175mm) to 0.15m, Hand Auger (75mm) to 0.7m
CASING: Uncased
REMARKS:

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

Generated with CORE-GS by Geroc - Soil Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.3 AHD
COORDINATE: E:311720.0, N:6265677.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH103
PROJECT No: 233937.00
DATE: 23/05/25
SHEET: 1 of 1

CONDITIONS ENCOUNTERED								SAMPLE				TESTING AND REMARKS	
GROUNDWATER	RL (m) / DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*) / DENSITY.(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
23/05/25 No Free Groundwater Observed Whilst Augering	89	TOPSOIL / Silty CLAY trace rootlets: brown; medium plasticity; Topsoil Fill.	TS	TOP	ND	w=PL		D		0.20		
	0.30	SILTSTONE: dark grey; inferred very low strength. Ashfield Shale.			NA	NA		D		0.30 0.40		
		Borehole discontinued at 0.50m depth. Hand Auger Refusal.								0.50		
	1 / 88											
	2 / 87											
	3 / 86											
	4 / 85											

NOTES: (#) Soil origin is "probable" unless otherwise stated. (*) Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT:
METHOD: Hand Auger (75mm) to 0.5m
REMARKS: Grass cover at surface

OPERATOR: R.Muller

LOGGED: R.Muller
CASING: Uncased

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

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BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.5 AHD
COORDINATE: E:311723.0, N:6265653.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH104
PROJECT No: 233937.00
DATE: 23/05/25
SHEET: 1 of 1

CONDITIONS ENCOUNTERED								SAMPLE				TESTING AND REMARKS	
GROUNDWATER	RL (m) / DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*) / DENSITY.(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
23/05/25 No Free Groundwater Observed Whilst Augering	0.15	CONCRETE: 150mm thick. 8mm diameter biaxial mesh at 90mm depth.			NA	NA		C		0.15		
	0.20	FILL / SAND: yellow-brown; fine to medium.		FILL	ND	D		D		0.20		
	0.40	FILL / Gravelly SAND: dark grey; fine to coarse; fine to coarse, igneous gravel.		FILL		M		D		0.30		
	0.50	FILL / CLAY with gravel: brown; medium plasticity; fine to coarse, igneous and siltstone gravel.		FILL		w=PL		D		0.40		
	89	SILTSTONE: grey and red-brown; inferred very low to low strength. Ashfield Shale.		NA	NA	NA		D		0.50		
		Borehole discontinued at 0.53m depth. Hand Auger Refusal.								0.53		
	1											
	88											
	2											
	87											
	3											
	86											
	4											
	85											

NOTES: (#) Soil origin is "probable" unless otherwise stated. (*) Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT:
OPERATOR: R.Muller
LOGGED: R.Muller
METHOD: Diatube (175mm) to 0.15m, Hand Auger (75mm) to 0.53m
CASING: Uncased
REMARKS:

Refer to explanatory notes for symbol and abbreviation definitions

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BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.5 AHD
COORDINATE: E:311672.0, N:6265664.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH105
PROJECT No: 233937.00
DATE: 23/05/25
SHEET: 1 of 1

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*) / DENSITY.(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
		0.22	TILE: 5mm thick. CONCRETE: 215mm thick. 8mm diameter biaxial mesh at 120mm depth.			NA	NA		C		0.22	DCP9/150	5 10 15
	89		FILL / Gravelly SAND: dark grey; fine to coarse; fine to coarse, igneous gravel.			MC	M		D		0.30 0.40		
23/05/25		1			FILL				D		0.50 0.90		
						PC	W		D		1.00 1.40		
	88	1.60	CLAY: pale grey; medium to high plasticity.								1.50 1.60		
		2			RS	(St) to (VSt)	w=PL		B		2		
	87	3									3		
		3.20	SILTSTONE: dark grey and orange-brown; inferred very low strength.			NA	NA		D		3.20 3.40 3.50		
	86	4							D		3.90 4.00		
	85		Borehole discontinued at 4.00m depth.										

NOTES: (#) Soil origin is "probable" unless otherwise stated. (*) Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: XC
METHOD: Diatube (175mm) to 0.22m, SFA (115mm) to 4.0m
REMARKS:

OPERATOR: Terratest (J.Wright)

LOGGED: R.Muller
CASING: Uncased

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Soil Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.5 AHD
COORDINATE: E:311700.0, N:6265636.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH106
PROJECT No: 233937.00
DATE: 23/05/25
SHEET: 1 of 1

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*) / DENSITY.(*)	MOISTURE	REMARKS	SAMPLE TYPE	SAMPLE INTERVAL	DEPTH (m)	TESTING AND REMARKS: TEST TYPE	RESULTS AND REMARKS
23/05/25 No Free Groundwater Observed Whilst Augering	89	0.17	TILE: 5mm thick. CONCRETE: 165mm thick. 10mm biaxial mesh at 70mm depth.			NA	NA		C		0.17	DCP9/150	5 10 15
		0.30	FILL / Gravelly SAND: dark grey; fine to coarse; fine to coarse, igneous gravel.		FILL	(MC)	M		D		0.30		
			FILL / CLAY with gravel: brown; medium plasticity; fine to coarse, igneous and siltstone gravel.						D		0.40 0.50		
	88	1			FILL	MC			D		0.90 1.00		
									D		1.40 1.50		
		1.60	CLAY (CI-CH): pale grey mottled yellow-brown; medium to high plasticity.		RS	(St) to (VSt)	w=PL		B		1.60		
	87	2									2		
		3									3		
		3.10	SILTSTONE: dark grey and orange-brown; apparently very low strength, generally iron stained. Ashfield Shale.			NA	NA				3.10		
	86								D		3.40 3.50		
			Borehole discontinued at 3.50m depth.										
	85	4											

NOTES: (#) Soil origin is "probable" unless otherwise stated. (*) Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: XC
METHOD: Diatube (175mm) to 0.17m, SFA (115mm) to 3.5m
REMARKS:

OPERATOR: Terratest (J.Wright)

LOGGED: R.Muller
CASING: Uncased

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Soil Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.1 AHD
COORDINATE: E:311636.0, N:6265662.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH107
PROJECT No: 233937.00
DATE: 21/05/25
SHEET: 1 of 3

CONDITIONS ENCOUNTERED								SAMPLE				TESTING AND REMARKS	
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN[#]	CONSIS.[*] / DENSITY.[*]	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
21/05/25 No Free Groundwater Observed Whilst Augering	89	0.15	TOPSOIL / Silty CLAY trace rootlets trace gravel: dark brown; medium plasticity; fine to coarse, igneous and siltstone gravel; Topsoil Fill.	TS	TOP	ND	w=PL		D	0.10	0.10		
			FILL / CLAY with gravel: brown; medium plasticity; fine to coarse, igneous and siltstone gravel.			(MC)			D	0.40 – 0.50	0.40		
	88	1							D	0.90 – 1.00	0.90		
											1.00	SPT	3,3,6 N=9
									D	1.40 – 1.50	1.40		
	87	2			FILL		w<PL to w=PL		D	1.90 – 2.00	1.90		
						MC					2.00		
												SPT	4,6,7 N=13
	86	3							D	3.40 – 3.50	3		
		4									4		
	85	4.10	CLAY (CI-CH): pale grey mottled yellow-brown; medium to high plasticity.		RS	VSt	w=PL					SPT	4,7,13 N=20
		4.70	SILTSTONE: orange-brown and red-brown; iron stained, with clay seams, inferred very low strength. Ashfield shale.			NA	NA					SPT	4,16,27 N=43
	84	4.95	Continued as rock log								5		
	83	6									6		
	82	7									7		
	81	8									8		
	80	9									9		

NOTES: [#] Soil origin is "probable" unless otherwise stated. [*] Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: Geo205
METHOD: SFA (115mm) to 4.95m, NMLC to 10.42m
REMARKS: Grass cover at surface

OPERATOR: Terratest (K.Mccorron)

LOGGED: R.Muller
CASING: Uncased

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.1 AHD
COORDINATE: E:311636.0, N:6265662.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH107
PROJECT No: 233937.00
DATE: 21/05/25
SHEET: 2 of 3

CONDITIONS ENCOUNTERED														SAMPLE				TESTING			
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA (SOIL STRENGTH (where encountered) / SOIL MOISTURE)	GRAPHIC	WEATH. (PS, XW, HW, DW, MW, SW, FR)	DEPTH (m)	STRENGTH (VL, L, M, H, VH, EH)	RECOVERY (%)	RQD	FRACTURE SPACING (m) (0.03, 0.06, 0.20, 0.60, 2.00, >2.00)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
21/05/25 No Free Groundwater Observed Whilst Augering	89	0																			
	88	1																			
	87	2																			
	86	3										3.00m , Unless stated otherwise, defects are B, 0-20°, PR, Clay VNR or CLN SM								
	85	4																			
	84	5	Continued from soil log			4.95															
			SILTSTONE: dark grey; 1-5% fine grain pale grey sandstone laminations, occasional iron staining. Ashfield shale.		XW	4.98	SEAM			SEAM	4.98-5.30m : EW								
						5.30		100	0							PLT	PL(A)=0.21MPa		
					SW		L									PLT	PL(A)=0.28MPa		
	83	6									5.68m : JT, 70°, PR, TI 5.77m: JT, 20°, UN, VNR Clay 5.80m: JT, 60°, TI 5.88m : JT, 60° , UN, SN Fe 5.97m: JT, 20°, PR, SN Fe 6.09-6.26m: JT x5 , 30°, PR, INF Clay, 1mm 6.30m : JT, 10°, UN, VNR Clay 6.36-6.42m: JT x3, 0-50°, CU, INF Clay, 2mm 6.53m: JT, 10°, PR, CN					PLT	PL(A)=0.14MPa		
					HW	6.00	VL												
						6.30													
					MW		L	100	40							PLT	PL(A)=0.21MPa		
	82	7																	
						7.30										PLT	PL(A)=1.1MPa		
			SILTSTONE: dark grey; 15-30% fine grain pale grey sandstone laminations. Ashfield shale.		SW		H												
	81	8														PLT	PL(A)=1.1MPa		
						8.20		100	81		8.10-8.29m: JT x5, 0-90°, ST, SN Fe								
						8.50	VH									PLT	PL(A)=2.6MPa		
						8.90					8.90m : JT, 45°, PR, SN Fe								
	80	9	SILTSTONE: dark grey; 1-5% fine grain pale grey sandstone laminations. Ashfield shale.		FR		H	100	98							PLT	PL(A)=3.5MPa		
											9.70m : JT, 15°, PR, CN								
																PLT	PL(A)=2.7MPa		

NOTES: [a] Soil origin is "probable" unless otherwise stated.

Generated with CORE-GS by Geroc - Split Soil-Rock Log

PLANT: Geo205
METHOD: SFA (115mm) to 4.95m, NMLC to 10.42m
REMARKS: Grass cover at surface

OPERATOR: Terratest (K.Mccorron)

LOGGED: R.Muller
CASING: Uncased

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.1 AHD
COORDINATE: E:311636.0, N:6265662.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH107
PROJECT No: 233937.00
DATE: 21/05/25
SHEET: 3 of 3

CONDITIONS ENCOUNTERED														SAMPLE				TESTING			
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA (SOIL STRENGTH (where encountered) / SOIL MOISTURE)	GRAPHIC	WEATH. (DW: PS, XW, HW, MW, SW, FR)	DEPTH (m)	STRENGTH (VL, L, M, H, VH, EH)	RECOVERY (%)	RQD	FRACTURE SPACING (m) (0.03, 0.06, 0.2, 0.6, 2.0)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
	79	10	[CONT] SILTSTONE: dark grey; 1-5% fine grain pale grey sandstone laminations. Ashfield shale.		FR		H	100	98		> 10.03-10.06m: JT x2, 15°, UN, CN Fe					PLT	PL(A)=2.3MPa		
	78	11	Borehole discontinued at 10.42m depth.																
	77	12																	
	76	13																	
	75	14																	
	74	15																	
	73	16																	
	72	17																	
	71	18																	
	70	19																	

NOTES: [a] Soil origin is "probable" unless otherwise stated.

PLANT: Geo205
METHOD: SFA (115mm) to 4.95m, NMLC to 10.42m
REMARKS: Grass cover at surface

OPERATOR: Terratest (K.Mccorron)

LOGGED: R.Muller
CASING: Uncased

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

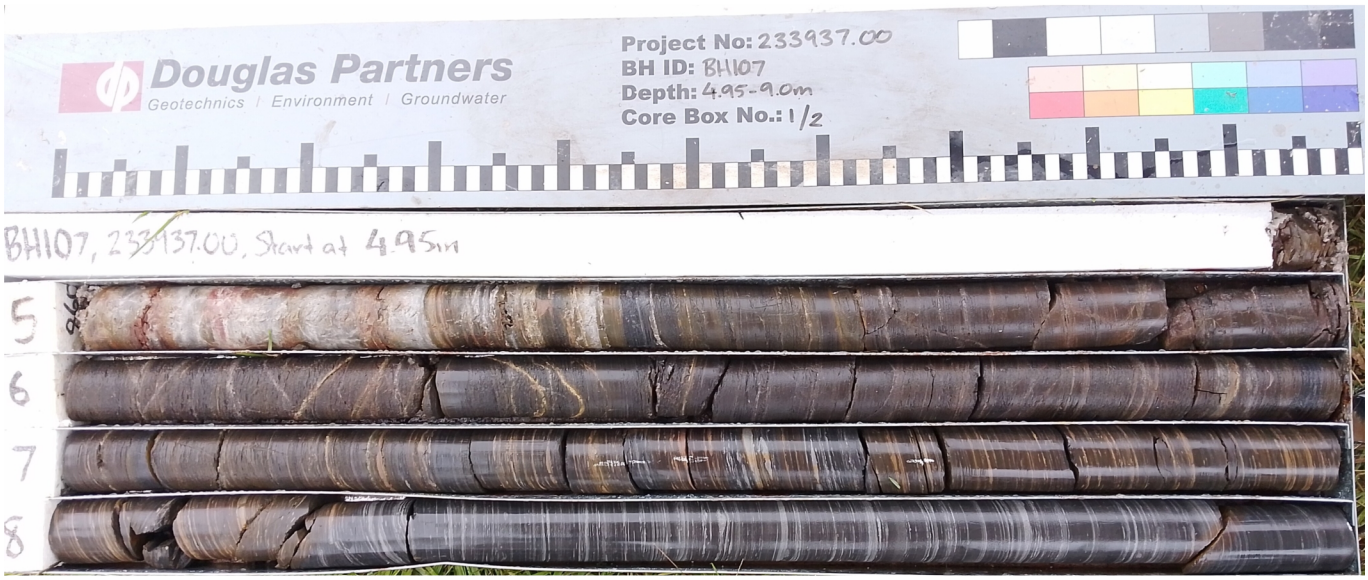
Generated with CORE-GS by Geroc - Split Soil-Rock Log

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.1 AHD
COORDINATE: E:311636.0, N:6265662.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH107
PROJECT No: 233937.00
DATE: 21/05/25
SHEET: 1 of 1



Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.5 AHD
COORDINATE: E:311678.0, N:6265638.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH108
PROJECT No: 233937.00
DATE: 23/05/25
SHEET: 1 of 1

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*) / DENSITY.(*)	MOISTURE	REMARKS	SAMPLE TYPE	SAMPLE INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
23/05/25 No Free Groundwater Observed Whilst Augering		0.05	PAVER: 50mm thick.			NA	NA				0.05		
			FILL / SAND: yellow-brown; fine to medium.		FILL		M		D		0.10		
	89	0.25	FILL / CLAY with gravel: brown; medium plasticity; fine to coarse, igneous and siltstone gravel.			PC			D		0.20		
											0.30		
									D		0.40		
											0.50	DCP9/150	
						MC							
	1				FILL				D		0.90		
											1.00		
						(MC)	w=PL						
	88								D		1.40		
											1.50		
		1.90	CLAY (CI-CH): pale grey mottled yellow-brown; medium to high plasticity.								1.90		
		2									2		
	87				RS	(St) to (VSt)			B				
		3.00	SILTSTONE: red-brown; inferred very low strength, iron stained. Ashfield shale.			NA	NA		D		3.00		
											3.10		
	86		Borehole discontinued at 3.50m depth.								3.40		
									D		3.50		
	85	4											

NOTES: (#) Soil origin is "probable" unless otherwise stated. (*) Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: XC
METHOD: Hand Tools to 0.05m, SFA (115mm) to 3.5m
REMARKS:

OPERATOR: Terratest (J.Wright)

LOGGED: R.Muller
CASING: Uncased

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

Generated with CORE-GS by Geroc - Soil Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 88.7 AHD
COORDINATE: E:311627.0, N:6265644.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH109
PROJECT No: 233937.00
DATE: 21/05/25
SHEET: 1 of 2

CONDITIONS ENCOUNTERED								SAMPLE				TESTING AND REMARKS	
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*) / DENSITY.(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
21/05/25 No Free Groundwater Observed Whilst Augering	88	0.15	TOPSOIL / Silty CLAY trace rootlets trace gravel: dark brown; medium plasticity; fine to coarse, igneous and siltstone gravel; Topsoil Fill.	TS	TOP	ND	w=PL		D		0.10		
									D		0.40 0.50		
	88	1	FILL / CLAY with gravel: brown; medium plasticity; fine to coarse, igneous and siltstone gravel.			(MC)	w=PL						
									D		0.90 1.00	SPT	4,7,9 N=16
	87	2			FILL				D		1.90 2.00		
	86	3				MC	w<PL				3	SPT	2,9,14 N=23
	85	4							D		3.90 4.00		
		4.20	CLAY (CI-CH): pale grey mottled yellow-brown; medium to high plasticity.		RS	St	w=PL					SPT	1,5,7 N=12
	84	5									5		
		5.20	SILTSTONE: grey; inferred very low strength. Ashfield shale.			NA	NA					SPT	7/10 (HB)
		5.51	Continued as rock log										
	83	6									6		
	82	7									7		
	81	8									8		
	80	9									9		
	79												

NOTES: (#) Soil origin is "probable" unless otherwise stated. (*) Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: Geo205
OPERATOR: Terratest (K.Mccorron)
LOGGED: R.Muller
METHOD: SFA (115mm) to 5.5m, NMLC to 9.96m
CASING: HWT to 5.6m
REMARKS: Water added to hole from 2.5m to aid with hole collapse
Grass cover at surface

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

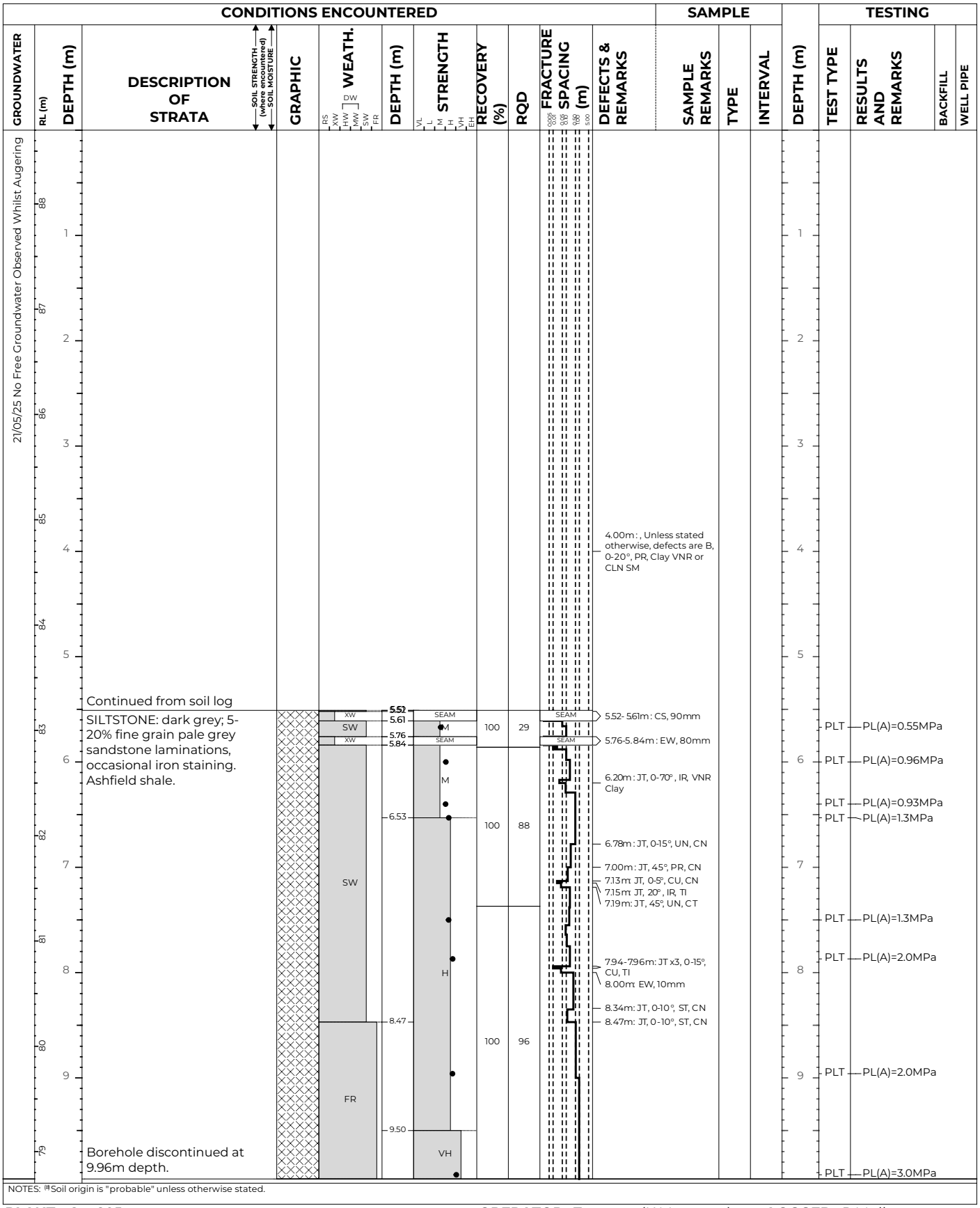
Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 88.7 AHD
COORDINATE: E:311627.0, N:6265644.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH109
PROJECT No: 233937.00
DATE: 21/05/25
SHEET: 2 of 2



NOTES: [a] Soil origin is "probable" unless otherwise stated.

PLANT: Geo205
OPERATOR: Terratest (K.Mccorron)
LOGGED: R.Muller
METHOD: SFA (115mm) to 5.5m, NMLC to 9.96m
CASING: HWT to 5.6m
REMARKS: Water added to hole from 2.5m to aid with hole collapse
Grass cover at surface

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 88.7 AHD
COORDINATE: E:311627.0, N:6265644.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH109
PROJECT No: 233937.00
DATE: 21/05/25
SHEET: 1 of 1

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.0 AHD
COORDINATE: E:311663.0, N:6265608.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH110
PROJECT No: 233937.00
DATE: 20/05/25
SHEET: 1 of 3

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN[#]	CONSIS.[*] / DENSITY.[*]	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
		0.15	TOPSOIL / Silty CLAY trace rootlets trace gravel: dark brown; medium plasticity; fine to coarse, igneous and siltstone gravel; Topsoil Fill.	TS	TOP	ND	w=PL		D	0.40–0.50	0.40				
			FILL / CLAY with gravel: brown; medium plasticity; fine to coarse, igneous and siltstone gravel.			(MC)					0.50				
	88	1									1	SPT	4,4,5 N=9		
						MC	w=PL		D	1.90–2.00	1.90				
▼ 20/05/25	87	2			FILL						2.00				
												SPT	0,1,2 N=3	Bentonite	
	86	3				PC	w>PL				3				
	85	4							D	4.40–4.50	4.40				
		4.50	CLAY (CI-CH): grey pale grey mottled yellow-brown; medium to high plasticity.								4.50	SPT	3,3,4 N=7		
	84	5				F to St					5				
					RS		w=PL		B						
	83	6				VSt					6	SPT	6,13,19 N=32		
		6.40	SILTSTONE: grey and red-brown; inferred very low strength, iron stained. Ashfield shale.								6.40				
	82	7				NA	NA				7	SPT	24,25,25 N=50	Gravel	
		7.50	Continued as rock log												
	81	8									8				
	80	9									9				

NOTES: [#] Soil origin is "probable" unless otherwise stated. [*] Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: Geo205
METHOD: SFA (115mm) to 7.0m, NMLC to 10.5m
REMARKS: Grass cover at surface

OPERATOR: Terratest (K.Mccorron)

LOGGED: R.Muller
CASING: HWT to 7.1m

Refer to explanatory notes for symbol and abbreviation definitions

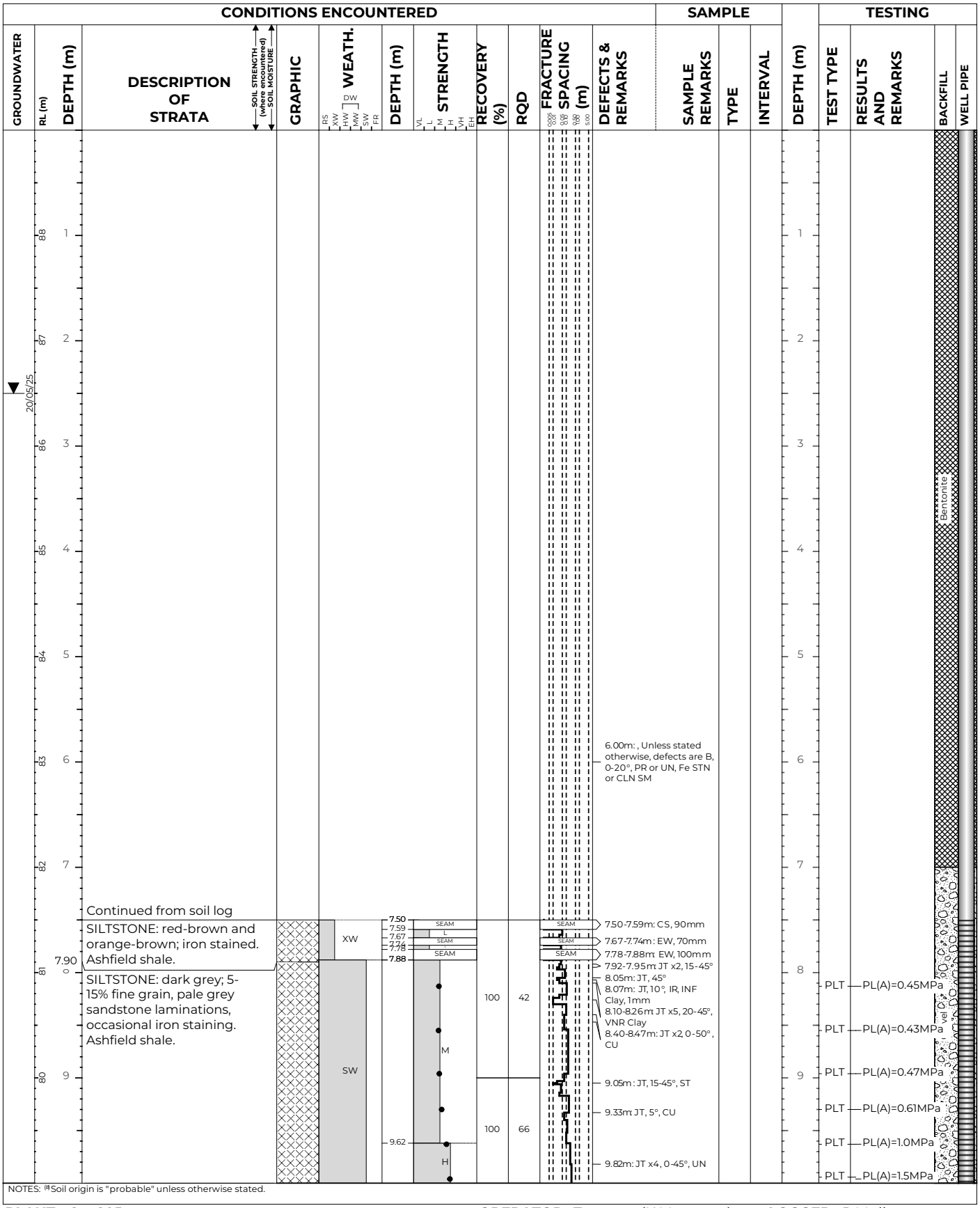
Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.0 AHD
COORDINATE: E:311663.0, N:6265608.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH110
PROJECT No: 233937.00
DATE: 20/05/25
SHEET: 2 of 3



NOTES: [a] Soil origin is "probable" unless otherwise stated.

Generated with CORE-GS by Geroc - Split Soil-Rock Log

PLANT: Geo205
METHOD: SFA (115mm) to 7.0m, NMLC to 10.5m
REMARKS: Grass cover at surface

OPERATOR: Terratest (K.Mccorron)

LOGGED: R.Muller
CASING: HWT to 7.1m

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.0 AHD
COORDINATE: E:311663.0, N:6265608.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH110
PROJECT No: 233937.00
DATE: 20/05/25
SHEET: 3 of 3

CONDITIONS ENCOUNTERED															SAMPLE				TESTING			
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA (SOIL STRENGTH (where encountered) / SOIL MOISTURE)	GRAPHIC	WEATH. (PS, XW, HW, DW, MW, SW, FR)	DEPTH (m)	STRENGTH (VL, L, M, H, VH, EH)	RECOVERY (%)	RQD	FRACTURE SPACING (m) (0.03, 0.06, 0.20, 0.60, 2.00)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
	78	11	[CONT] SILTSTONE: dark grey; 5-15% fine grain, pale grey sandstone laminations, occasional iron staining. Ashfield shale.		SW		H	100	66		10.07-10.15m: JT x3, 10° 10.21-10.25m: JT x3, 0-50°, CU					PLT	PL(A)=1.3MPa	Gravel	
			Borehole discontinued at 10.50m depth.																			
	77	12																				
	76	13																				
	75	14																				
	74	15																				
	73	16																				
	72	17																				
	71	18																				
	70	19																				

NOTES: [a] Soil origin is "probable" unless otherwise stated.

PLANT: Geo205
METHOD: SFA (115mm) to 7.0m, NMLC to 10.5m
REMARKS: Grass cover at surface

OPERATOR: Terratest (K.Mccorron)

LOGGED: R.Muller
CASING: HWT to 7.1m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.0 AHD
COORDINATE: E:311663.0, N:6265608.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH110
PROJECT No: 233937.00
DATE: 20/05/25
SHEET: 1 of 1

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 95.1 AHD
COORDINATE: E:311737.8, N:6265699.8
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH201
PROJECT No: 233937.00
DATE: 11/07/25
SHEET: 1 of 3

CONDITIONS ENCOUNTERED								SAMPLE				TESTING AND REMARKS	
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*) DENSITY.(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
11/07/25 No Free Groundwater Observed While Augering	95	0.20	TOPSOIL / Silty CLAY trace grass roots: grey-brown; medium plasticity.	TS	TOP	ND	M		A		0.10		
			CLAY (CH): brown; high plasticity.		RS	(St)	w=PL		A		0.40 0.50		
		0.55	CLAY (Cl): pale grey-brown; medium plasticity.		XWM	VSt to H	w<PL		A		0.90 1.00	SPT	11,18,25/120 R
	94	1							SPT		1.42		
	93	2.00	Continued as rock log								2		
	92	3									3		
	91	4									4		
	90	5									5		
	89	6									6		
	88	7									7		
	87	8									8		
	86	9									9		

NOTES: (#)Soil origin is "probable" unless otherwise stated. (*)Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.0m, NMLC to 15.67m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 1m

Refer to explanatory notes for symbol and abbreviation definitions

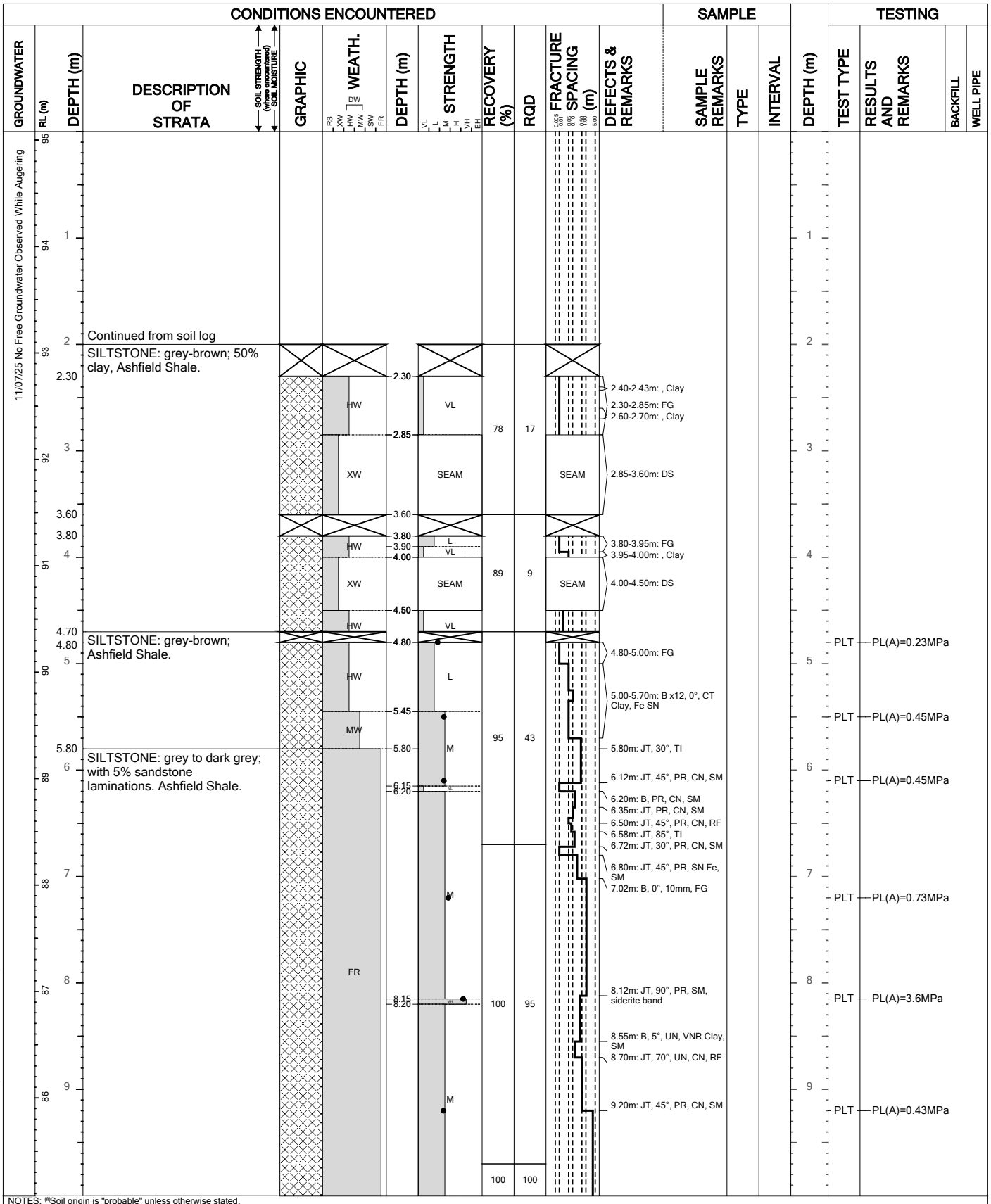
Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 95.1 AHD
COORDINATE: E:311737.8, N:6265699.8
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH201
PROJECT No: 233937.00
DATE: 11/07/25
SHEET: 2 of 3



NOTES: [#]Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.0m, NMLC to 15.67m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 95.1 AHD
COORDINATE: E:311737.8, N:6265699.8
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH201
PROJECT No: 233937.00
DATE: 11/07/25
SHEET: 3 of 3

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	WEATH.	DEPTH (m)	STRENGTH	RECOVERY (%)	RQD	FRACTURE SPACING (m)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
	85	0	[CONT] SILTSTONE: grey to dark grey; with 5% sandstone laminations. Ashfield Shale.												0	PLT	PL(A)=0.60MPa		
	84	11					M	100	100						11	PLT	PL(A)=0.65MPa		
	83	12									12.50m: JT, 60°, PR, 50mm, SM, FG, SZ				12	PLT	PL(A)=0.72MPa		
	82	13			FR	13.00					12.70-13.10m: JT, 85°, UN, CN, RF 13.15m: JT, 45°, PR, CN, SM				13	PLT	PL(A)=1.1MPa		
		13.30	SILTSTONE: dark grey; with 20% pale grey fine grained sandstone. Ashfield Shale.								13.30m: JT, 45°, PR, CN, SM 13.30-13.50m: , fault zone 13.45m: F, 30° 13.50m: JT, 45-70°, CU, TI, SM								
	81	14	13.30m-13.50m: Irregular / disturbed bedding				H	100	97						14	PLT	PL(A)=2.5MPa		
	80	15													15	PLT	PL(A)=2.5MPa		
	79	16	Borehole discontinued at 15.67m depth. Target depth reached.																
	78	17																	
	77	18																	
	76	19																	

NOTES: [d]Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.0m, NMLC to 15.67m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

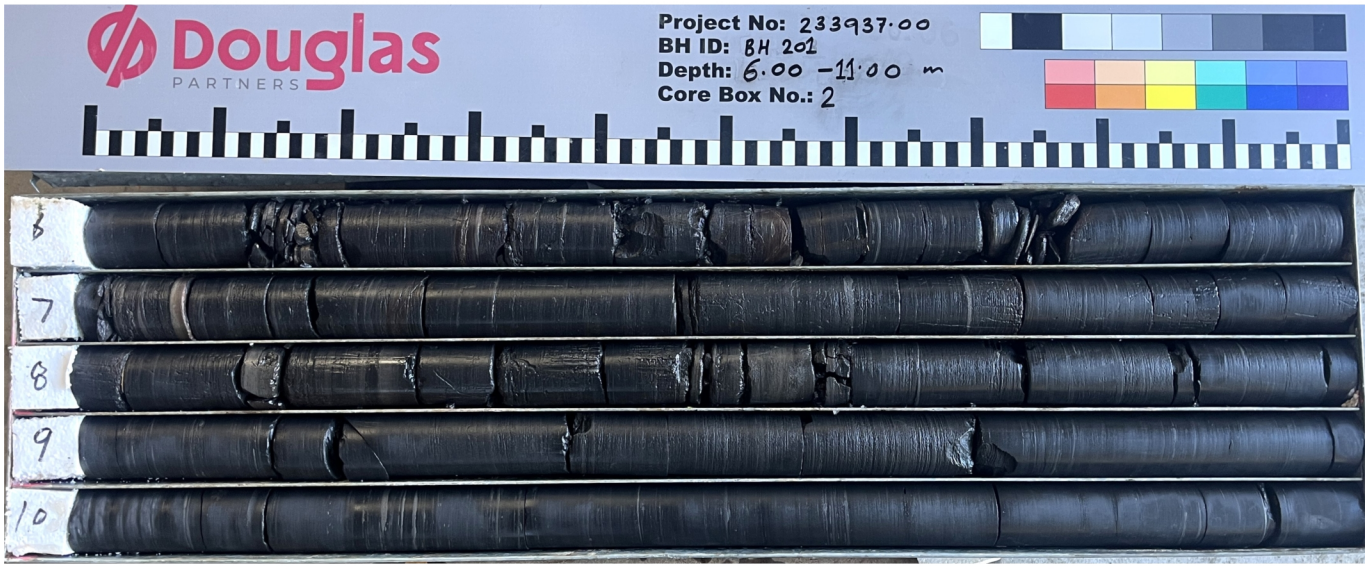
CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 95.1 AHD
COORDINATE: E:311737.8, N:6265699.8
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH201
PROJECT No: 233937.00
DATE: 11/07/25
SHEET: 1 of 2

2.00-6.00 m depth



6.00-11.00 m depth

Generated with CORE-GS by Geroc - Split Soil-Rock Log

Douglas PARTNERS

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 95.1 AHD
COORDINATE: E:311737.8, N:6265699.8
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH201
PROJECT No: 233937.00
DATE: 11/07/25
SHEET: 2 of 2

11.00-15.67 m depth

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 96.0 AHD
COORDINATE: E:311773.0, N:6265685.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH202
PROJECT No: 233937.00
DATE: 17/07/25
SHEET: 1 of 3

CONDITIONS ENCOUNTERED								SAMPLE				TESTING AND REMARKS			
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*) DENSITY.(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
17/07/25 No Free Groundwater Observed Whilst Augering	96	0.02	ASPHALTIC CONCRETE; 20 mm thick.		FILL	(WC)	M		A		0.10			Flush cover - Gatic Cover	
		0.25	FILL / GRAVEL; fine to medium, angular, igneous.											Backfill	
			CLAY (CH): orange-brown; high plasticity.		RS	(St)	w=PL		A		0.40 0.50				
		0.60	SILTSTONE: grey-brown; inferred very low strength. Ashfield Shale.						B						
	95	1			NA	NA	NA		A		0.90 1.00	SPT	25/130 R		
									SPT		1.13				
		1.60	Continued as rock log											Bentonite	
	94	2									2				
	93	3									3				
	92	4									4				
	91	5									5				
	90	6									6				
	89	7									7				
	88	8									8				
	87	9									9				

NOTES: (#)Soil origin is "probable" unless otherwise stated. (*)Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 1.6m, NMLC to 15.10m
REMARKS:

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 96.0 AHD
COORDINATE: E:311773.0, N:6265685.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH202
PROJECT No: 233937.00
DATE: 17/07/25
SHEET: 2 of 3

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	WEATH.	DEPTH (m)	STRENGTH	RECOVERY (%)	RQD	FRACTURE SPACING (m)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
17/07/25 No Free Groundwater Observed Whilst Augering	96	0	Continued from soil log												0			Flush cover - Gatic Cover	
	95	1													1				
	94	2.00	SILTSTONE: grey-brown. Ashfield Shale.		HW	1.60	VL				2.00-2.12m: FG				2				
		2.12			XW	2.00	SEAM			SEAM	2.12-2.35m: , Clay								
	93	2.35	SILTSTONE: grey-brown; 10% clay seams. Ashfield Shale.		HW	2.35	VL - L	61	55		2.35-2.95m: B x6, 0°, Clay, 2-5mm				3	PLT	PL(A)=0.11MPa		
		3.30	SILTSTONE: dark grey; with 5% fine sandstone laminations;. Ashfield Shale.		MW	3.30	L - M				3.70-3.85m: FG					PLT	PL(A)=0.35MPa		
	92	4									4.05m: JT, 70°, UN, SN Fe, RF				4			backfill	
						4.25					4.25-4.90m: B x3, 0°, CT Clay, 2-3mm					PLT	PL(A)=0.56MPa		
	91	5			SW			100	95						5	PLT	PL(A)=0.82MPa		
	90	6					M				5.75m: JT, 45°, PR, CN, SM				6				
											5.95m: B, 0°, Clay 2mm								
						6.40					6.22m: JT, 45°, PR, Clay, SM								
											6.40m: JT, 70°, PR, SN Fe, RF, Clay, 10mm								
											6.70-6.73m: FG, CBS, shale					PLT	PL(A)=0.67MPa		
	89	7									6.75m: JT, 45°, PR, CN, SM				7				
											7.50m: JT, 45°, PR, CN, SM								
	88	8			FR			100	97						8	PLT	PL(A)=0.68MPa		
											8.40-8.45m: FG					PLT	PL(A)=0.69MPa		
						8.85	VH				8.85m: JT, 30°, PR, 150mm, SM, siderite band					PLT	PL(A)=3.2MPa		
	87	9	8.85m-9.00m: fine grained sandstone bed			9.00									9	PLT	PL(A)=0.72MPa	bentonite	
							M	100	100										

NOTES: (R)Soil origin is "probable" unless otherwise stated.

Generated with CORE-GS by Geroc - Split Soil-Rock Log

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 1.6m, NMLC to 15.10m
REMARKS:

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 96.0 AHD
COORDINATE: E:311773.0, N:6265685.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH202
PROJECT No: 233937.00
DATE: 17/07/25
SHEET: 3 of 3

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	WEATH.	DEPTH (m)	STRENGTH	RECOVERY (%)	RQD	FRACTURE SPACING (m)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
	85	11	[CONT] SILTSTONE: dark grey; with 5% fine sandstone laminations;. Ashfield Shale.		FR		M	100	100		10.50m: FG, 0°, 10mm, B				11	PLT	PL(A)=0.66MPa	Bentonite	
	84	12									11.30m: FG, 0°, 5mm, B				12	PLT	PL(A)=0.63MPa		
	83	13				13.00	M + H				12.40m: JT, 45°, UN, CN, RF				13	PLT	PL(A)=0.38MPa	Gravel	
	82	13.80 14	SILTSTONE: dark grey; with 10-20% pale grey fine grained sandstone. Ashfield Shale.			13.80	H	100	100						14	PLT	PL(A)=1.0MPa		
	81	15	Borehole discontinued at 15.10m depth. Target depth reached.								14.85m: JT, 45°, PR, CN, SM				15	PLT	PL(A)=3.0MPa		
	80	16														PLT	PL(A)=2.0MPa		
	79	17																	
	78	18																	
	77	19																	

NOTES: (R)Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 1.6m, NMLC to 15.10m
REMARKS:

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 96.0 AHD
COORDINATE: E:311773.0, N:6265685.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH202
PROJECT No: 233937.00
DATE: 17/07/25
SHEET: 1 of 2

1.60-6.00 m depth

6.00-11.00 m depth

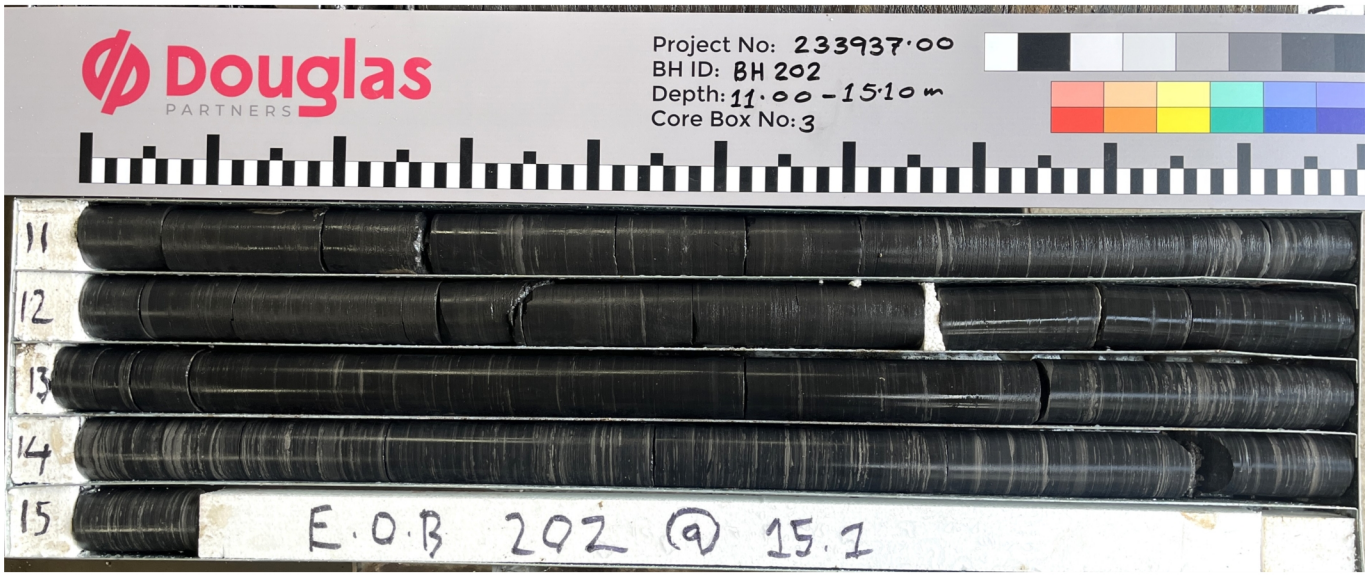
Generated with CORE-GS by Geroc - Split Soil-Rock Log

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 96.0 AHD
COORDINATE: E:311773.0, N:6265685.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH202
PROJECT No: 233937.00
DATE: 17/07/25
SHEET: 2 of 2



11.00-15.10 m depth

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 99.2 AHD
COORDINATE: E:311811.4, N:6265671.6
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH203
PROJECT No: 233937.00
DATE: 14/07/25
SHEET: 1 of 3

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*) DENSITY.(*)	MOISTURE	REMARKS	SAMPLE TYPE	SAMPLE INTERVAL	DEPTH (m)	TESTING AND REMARKS: TEST TYPE	TESTING AND REMARKS: RESULTS AND REMARKS
14/07/25 No Free Groundwater Observed	99	0.20	TOPSOIL / Silty CLAY trace grass roots: grey-brown; low plasticity.	TS	TOP	ND	w<PL		A	0.10			
			FILL / CLAY trace silt trace sand: grey-brown; medium plasticity; fine sand.						A	0.40 - 0.50			
	98	1			FILL	(MC)	w=PL		A	0.90 - 1.00			
									SPT	1.00 - 1.45		SPT	5,7,8 N=15
	97	2.00	CLAY (CL): pale grey-brown; low plasticity.		XWM	H	w<PL		SPT	2.50 - 2.78		SPT	14,25/130
	96	3.00	Continued as rock log										
	95	4											
	94	5											
	93	6											
	92	7											
	91	8											
	90	9											

NOTES: (#)Soil origin is "probable" unless otherwise stated. (*)Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: Explorer
METHOD: AD/T to 2.5m, WB to 3.0m, NMLC to 15.50m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 2.5m, then HQ to 3m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 99.2 AHD
COORDINATE: E:311811.4, N:6265671.6
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH203
PROJECT No: 233937.00
DATE: 14/07/25
SHEET: 2 of 3

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	WEATH.	DEPTH (m)	STRENGTH	RECOVERY (%)	RQD	FRACTURE SPACING (m)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
14/07/25 No Free Groundwater Observed	99	0	Continued from soil log												0				
	98	1													1				
	97	2													2				
	96	3													3				
		3.40	SILTSTONE: grey-brown; 10% clay seams, Ashfield Shale.		HW	3.40	VL	60	33		3.80m: JT, 80°, PR, CN, RF 3.40-4.40m: FG 3.92m: B, 0°, Clay 10mm 3.95m: JT, 70°, UN, CN, RF				4	PLT	PL(A)=0.13MPa		
	95	4																	
		4.45	SILTSTONE: pale grey and grey-brown; 25% clay, Ashfield Shale.		XW	4.40	SEAM			SEAM	4.40-4.80m: DS								
						4.80	VL				4.90m: JT, 45°, SN Fe, TI								
	94	5			HW	5.00	L	100	45		5.00-5.55m: B x12, 0°, SN Fe, FG, Clay				5	PLT	PL(A)=0.21MPa		
					MW	5.55	L M				5.60-5.95m: B x5, 0°, SN Fe, Clay CT				6	PLT	PL(A)=0.52MPa		
	93	6			HW	6.00	L				6.00-6.30m: B x3, 0°, Clay 5mm								
		6.55	SILTSTONE: grey and dark grey; with 5% sandstone laminations, Ashfield Shale.			6.55					6.45m: JT, 45-80°, CU, CN, RF 6.50-6.60m: FG, Clay				7	PLT	PL(A)=0.56MPa		
	92	7					M				6.80m: B, 0°, Fe 7.05-7.10m: B x2, 0-5°, SN Fe					PLT	PL(A)=0.48MPa		
						7.55	VH				7.55m: JT, 85°, CU, CN, RF, siderite band					PLT	PL(A)=3.0MPa		
	91	8			SW	7.60		97	97		8.20-8.23m: DS				8	PLT	PL(A)=0.67MPa		
							M				8.65-8.80m: JT x2, 45°, PR, CN, SM								
	90	9									9.00m: JT, 45°, PR, SN Fe, RF				9				
		9.25																	
		9.33				9.72	VH	100	93		9.50-9.65m: JT, 85°, UN, SN Fe, RF 9.65-9.72m: FG, SN Fe					PLT	PL(A)=0.73MPa		
						9.80	M				9.72m: JT, 30°, UN, SN Fe, RF								

NOTES: [#]Soil origin is "probable" unless otherwise stated.

Generated with CORE-GS by Geroc - Split Soil-Rock Log

PLANT: Explorer
METHOD: AD/T to 2.5m, WB to 3.0m, NMLC to 15.50m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 2.5m, then HQ to 3m

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 99.2 AHD
COORDINATE: E:311811.4, N:6265671.6
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH203
PROJECT No: 233937.00
DATE: 14/07/25
SHEET: 3 of 3

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	WEATH.	DEPTH (m)	STRENGTH	RECOVERY (%)	RQD	FRACTURE SPACING (m)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
	89	10.00	SILTSTONE: grey and dark grey; <1% fine grained sandstone laminations, Ashfield Shale.		FR	10.00	M				9.80m: JT, 45°, UN, SN Fe, RF				11	PLT	PL(A)=0.76MPa		
						10.70	VH				9.85-10.00m: JT, 85°, UN, SN Fe, RF, siderite band					PLT	PL(A)=3.4MPa		
						10.80					10.10m: JT, 45°, PR, SN Fe, RF								
	88	11									10.25m: JT, 45°, PR, CN, SM								
											10.50m: B, 0°, 10mm, FG								
											10.52m: JT, 85°, PR, CN, SM								
							M	100	93		10.75m: JT, 90°, TI, siderite band					PLT	PL(A)=3.4MPa		
							H				10.85m: JT, 20°, PR, TI, SM					PLT	PL(A)=0.70MPa		
	87	12				11.80					11.10m: JT, 70°, 20mm, FG, SZ				12	PLT	PL(A)=0.66MPa		
											11.28m: JT, 90°, PR, CN, SM								
											11.50m: FG, 30mm								
											12.40m: JT, 80°, PR, CN, RF								
	86	13													13	PLT	PL(A)=0.65MPa		
							M				13.45m: JT, 90°, PR, CN, SM								
											13.75m: B, 0°, Clay 5mm, FG								
	85	14						100	100						14	PLT	PL(A)=0.57MPa		
	84	15													15	PLT	PL(A)=0.62MPa		
			Borehole discontinued at 15.50m depth. Target depth reached.																
	83	16																	
	82	17																	
	81	18																	
	80	19																	

NOTES: (R)Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 2.5m, WB to 3.0m, NMLC to 15.50m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 2.5m, then HQ to 3m

Refer to explanatory notes for symbol and abbreviation definitions

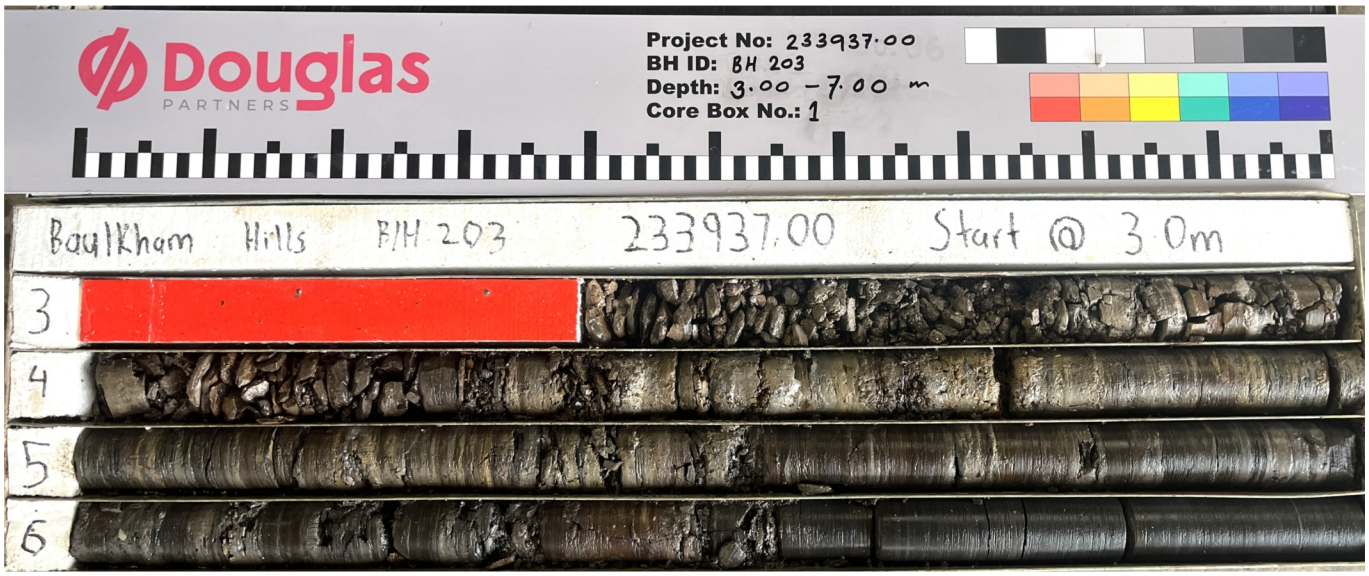
Generated with CORE-GS by Geroc - Split Soil-Rock Log

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 99.2 AHD
COORDINATE: E:311811.4, N:6265671.6
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH203
PROJECT No: 233937.00
DATE: 14/07/25
SHEET: 1 of 2



3.00-7.00 m depth

7.00-12.00 m depth

Generated with CORE-GS by Geroc - Split Soil-Rock Log

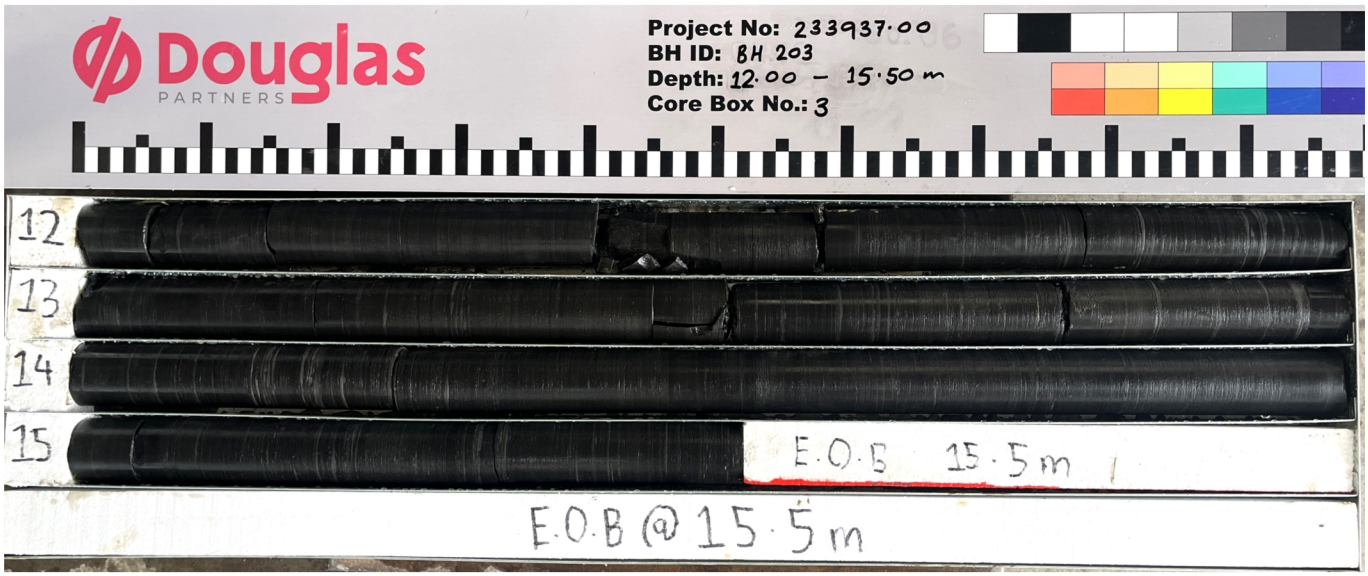
Douglas PARTNERS

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 99.2 AHD
COORDINATE: E:311811.4, N:6265671.6
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH203
PROJECT No: 233937.00
DATE: 14/07/25
SHEET: 2 of 2



12.00-15.50 m depth

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 98.9 AHD
COORDINATE: E:311847.0, N:6265652.7
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH204
PROJECT No: 233937.00
DATE: 18/07/25
SHEET: 1 of 4

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*) DENSITY.(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
18/07/25 No free groundwater observed while augering	98	0.10	TOPSOIL / Silty CLAY trace rootlets: grey-brown; low plasticity.	TS	TOP	ND	w<PL		A		0.10		
			FILL / Silty CLAY trace gravel: dark grey-brown; low to medium plasticity; concrete gravel; trace organic debris.		FILL	(PC)	w=PL		A		0.40 0.50		
		0.65	CLAY (CH) trace gravel: pale grey and orange-brown; high plasticity; ironstone gravel.						A		0.90		
	97	1			RS	St					1.00		
									SPT			SPT	7,7,8 N=15
											1.45		
		2.00	CLAY (CI): grey-brown; medium plasticity.				w<PL				2		
	96	3			XWM	H					2.50		
									SPT			SPT	15,17,23 N=40
		3.65	Continued as rock log								2.95		
	95	4									4		
	94	5									5		
	93	6									6		
	92	7									7		
	91	8									8		
	90	9									9		
	89												

NOTES: (#)Soil origin is "probable" unless otherwise stated. (*)Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: Explorer
METHOD: AD/T to 2.5m, WB to 3.65m, NMLC to 20.45m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 3.65m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 98.9 AHD
COORDINATE: E:311847.0, N:6265652.7
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH204
PROJECT No: 233937.00
DATE: 18/07/25
SHEET: 2 of 4

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	WEATH.	DEPTH (m)	STRENGTH	RECOVERY (%)	RQD	FRACTURE SPACING (m)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
18/07/25 No free groundwater observed while augering	98	0													0				
	97	1													1				
	96	2													2				
	95	3	Continued from soil log												3				
	94	4	SILTSTONE: pale grey-brown; 50% clay, fractured. Ashfield Shale.		XW to HW	3.65	VL	94	0		3.65-4.50m: FG, Clay, DS				4				
		4.50				4.55					4.55-4.70m: FG, Clay								
		4.55																	
	94	4.70	SILTSTONE: grey-brown; with 5-10% sandstone laminations. Ashfield Shale.		HW		VL				5.15m: JT, 40°, PR, TI, RF				5				
											5.40-5.55m: FG								
		5.65	SILTSTONE: grey; with 5% sandstone laminations. Ashfield Shale.			5.65		100	58		5.70m: JT, 30°, PR, Clay, SM				6	PLT	PL(A)=0.58MPa		
	93	6			SW		M				6.45m: JT, 45-65°, ST, CN, SM, 2								
	92	7			FR	7.00					6.70-6.85m: JT, 90°, PR, CN, SM				7	PLT	PL(A)=0.56MPa		
											6.85m: JT, 45°, PR, CN, SM								
											7.15m: JT, 45°, PR, CN, SM								
					HW	7.40	VL				7.40m: JT, 25°, PR, Clay 30mm, SM, FG								
						7.50													
	91	8									7.80m: JT, 30°, PR, CN, SM				8	PLT	PL(A)=0.53MPa		
								100	94										
	90	9			FR		M				8.90m: JT, 45°, PR, SN Fe, SM				9	PLT	PL(A)=0.87MPa		
											9.35m: JT, 70°, PR, CN, SM								
											9.42m: JT, 65°, PR, CN, SM								
	89	9.75				9.75													
		9.78			FR	9.78	H	99	97		9.80-9.85m: , siderite bands					PLT	PL(A)=2.2MPa		

NOTES: (R)Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 2.5m, WB to 3.65m, NMLC to 20.45m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 3.65m

Generated with CORE-GS by Geroc - Split Soil-Rock Log

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 98.9 AHD
COORDINATE: E:311847.0, N:6265652.7
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH204
PROJECT No: 233937.00
DATE: 18/07/25
SHEET: 3 of 4

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	WEATH.	DEPTH (m)	STRENGTH	RECOVERY (%)	RQD	FRACTURE SPACING (m)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
	89	10.00	[CONT] SILTSTONE: grey, with 5% sandstone laminations. Ashfield Shale.			10.00	M	99	97		10.25m: , 30°, TI, micro fault				10	PLT	PL(A)=0.52MPa		
	88	11													11				
	87	12													12	PLT	PL(A)=0.76MPa		
	86	13				13.00					12.85m: B, 0°, Clay				13	PLT	PL(A)=0.82MPa		
		13.20	SILTSTONE: dark grey; with 10-20% pale grey fine grained sandstone laminations. Ashfield Shale.								13.20m: JT, 20°, PR, Clay, SM								
	85	14					M - H	100	100		13.60m: JT, 45°, PR, Clay, SM				14	PLT	PL(A)=1.2MPa		
	84	15			FR	15.00									15	PLT	PL(A)=0.80MPa		
	83	16					H								16	PLT	PL(A)=1.6MPa		
	82	17				17.00	H - VH	100	100						17	PLT	PL(A)=2.1MPa		
	81	18				18.00									18	PLT	PL(A)=3.1MPa		
	80	19					H	100	100						19	PLT	PL(A)=2.3MPa		
	79															PLT	PL(A)=2.0MPa		

NOTES: [#]Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 2.5m, WB to 3.65m, NMLC to 20.45m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 3.65m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 98.9 AHD
COORDINATE: E:311847.0, N:6265652.7
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH204
PROJECT No: 233937.00
DATE: 18/07/25
SHEET: 4 of 4

CONDITIONS ENCOUNTERED														SAMPLE				TESTING			
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA (SOIL STRENGTH (where encountered) SOIL MOISTURE)	GRAPHIC	WEATH. (RS XW HW MW SW FR; DW)	DEPTH (m)	STRENGTH (VL L M H VH EH)	RECOVERY (%)	RQD	FRACTURE SPACING (m) (0.005 0.06 0.2 0.6 2.00)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
	78	21	[CONT] SILTSTONE: dark grey; with 10-20% pale grey fine grained sandstone laminations. Ashfield Shale.		FR		H	100	100							PLT	PL(A)=2.7MPa		
			Borehole discontinued at 20.45m depth. Target depth reached.																
	77	22																	
	76	23																	
	75	24																	
	74	25																	
	73	26																	
	72	27																	
	71	28																	
	70	29																	
	69																		

NOTES: (R)Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 2.5m, WB to 3.65m, NMLC to 20.45m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 3.65m

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

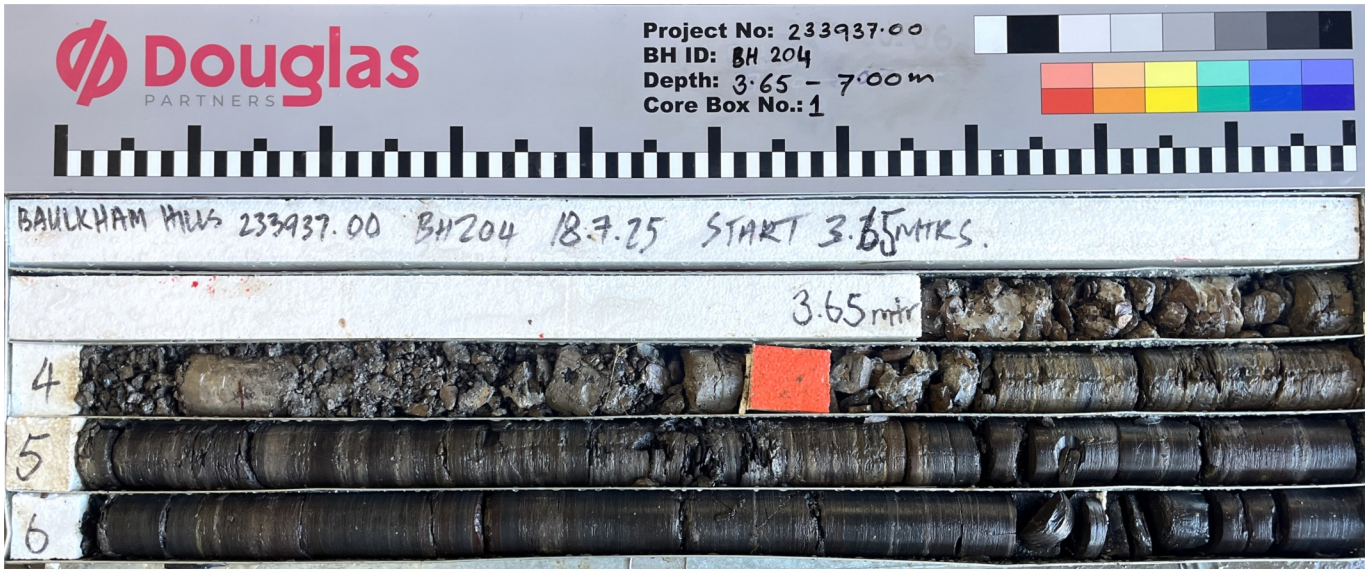
Generated with CORE-GS by Geroc - Split Soil-Rock Log

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 98.9 AHD
COORDINATE: E:311847.0, N:6265652.7
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH204
PROJECT No: 233937.00
DATE: 18/07/25
SHEET: 1 of 2



3.65-7.00 m depth

7.00-12.00 m depth

Generated with CORE-GS by Geroc - Split Soil-Rock Log

Douglas PARTNERS

CORE PHOTO LOG

CLIENT:	GreenSquareDC Pty Ltd	**SURFACE LEVEL:**	98.9 AHD	**LOCATION ID:**	BH204
PROJECT:	Green Square DC SYDGPU1	**COORDINATE:**	E:311847.0, N:6265652.7	**PROJECT No:**	233937.00
LOCATION:	3 Brookhollow Avenue, Baulkham Hills, NSW 2153	**DATUM/GRID:**	MGA2020 Zone 56	**DATE:**	18/07/25
		DIP/AZIMUTH:	90°/---°	**SHEET:**	2 of 2

12.00-17.00 m depth

17.00-20.45 m depth

Generated with CORE-GS by Geroc - Split Soil-Rock Log

Douglas PARTNERS

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 100.0 AHD
COORDINATE: E:311887.1, N:6265641.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH205
PROJECT No: 233937.00
DATE: 16/07/25
SHEET: 1 of 4

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*)	DENSITY.(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
16/07/25 No Free Groundwater Observed While Augering	100	0.20	TOPSOIL / Silty CLAY trace grass roots: grey-brown; low plasticity.	TS	TOP	ND		M		A		0.10			Flush cover - Gatic cover	
			CLAY (CI-CH): brown; medium to high plasticity.		RS	(St)		w=PL		A		0.40 0.50				
	99	0.75 1	SILTSTONE: pale grey; trace ironstone band, inferred very low strength, Ashfield Shale.			NA		NA		A SPT		0.90 1.00 1.15	SPT	25/150	Backfill	
	98	2.00	Continued as rock log									2				
	97	3										3				
	96	4										4				
	95	5										5				
	94	6										6				
	93	7										7				
	92	8										8				
	91	9										9				

NOTES: (#)Soil origin is "probable" unless otherwise stated. (*)Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.0m, NMLC to 20.20m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 1m

Refer to explanatory notes for symbol and abbreviation definitions

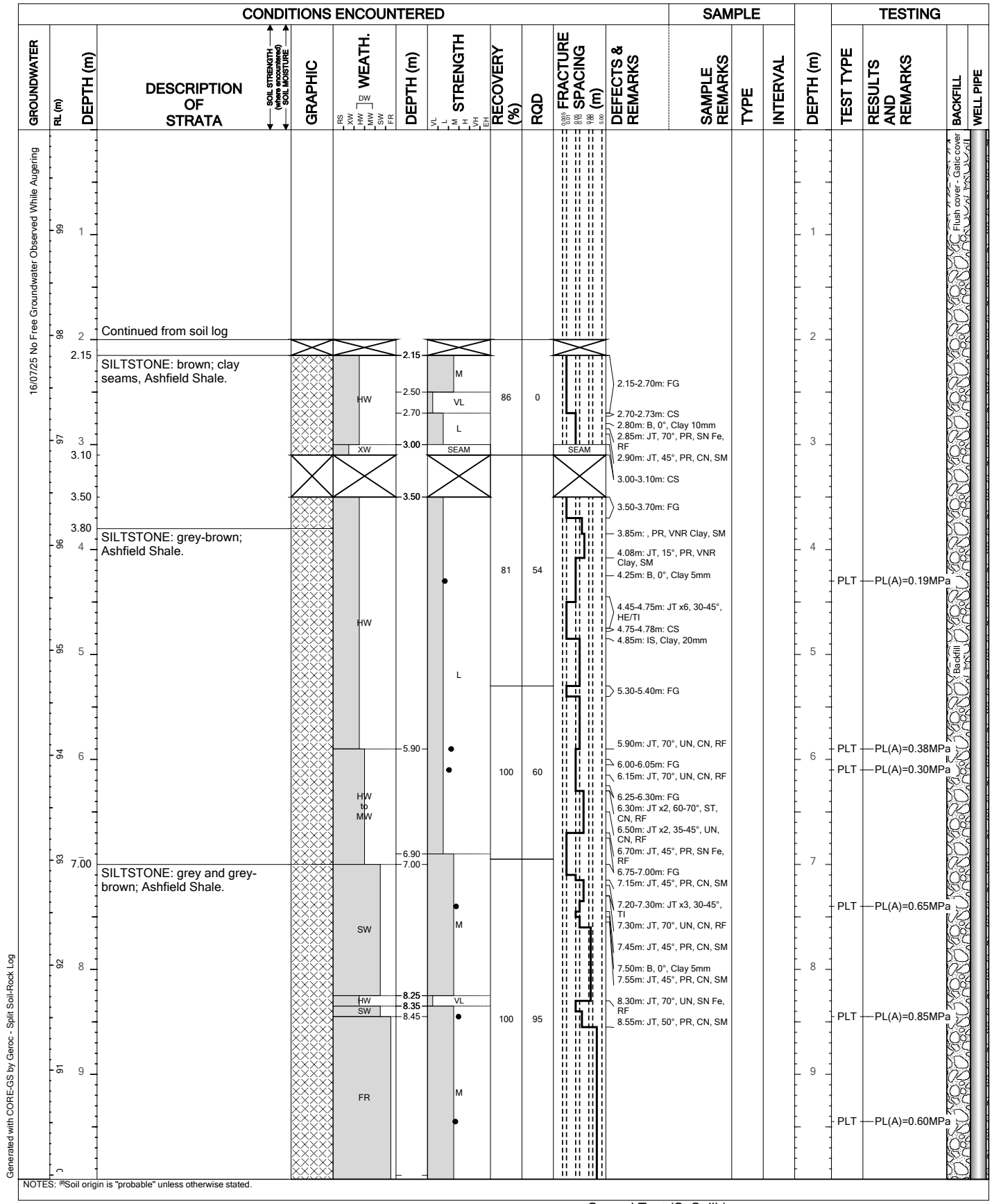
Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 100.0 AHD
COORDINATE: E:311887.1, N:6265641.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH205
PROJECT No: 233937.00
DATE: 16/07/25
SHEET: 2 of 4



NOTES: (#)Soil origin is "probable" unless otherwise stated.

Generated with CORE-GS by Geroc - Split Soil-Rock Log

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.0m, NMLC to 20.20m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 1m

Refer to explanatory notes for symbol and abbreviation definitions

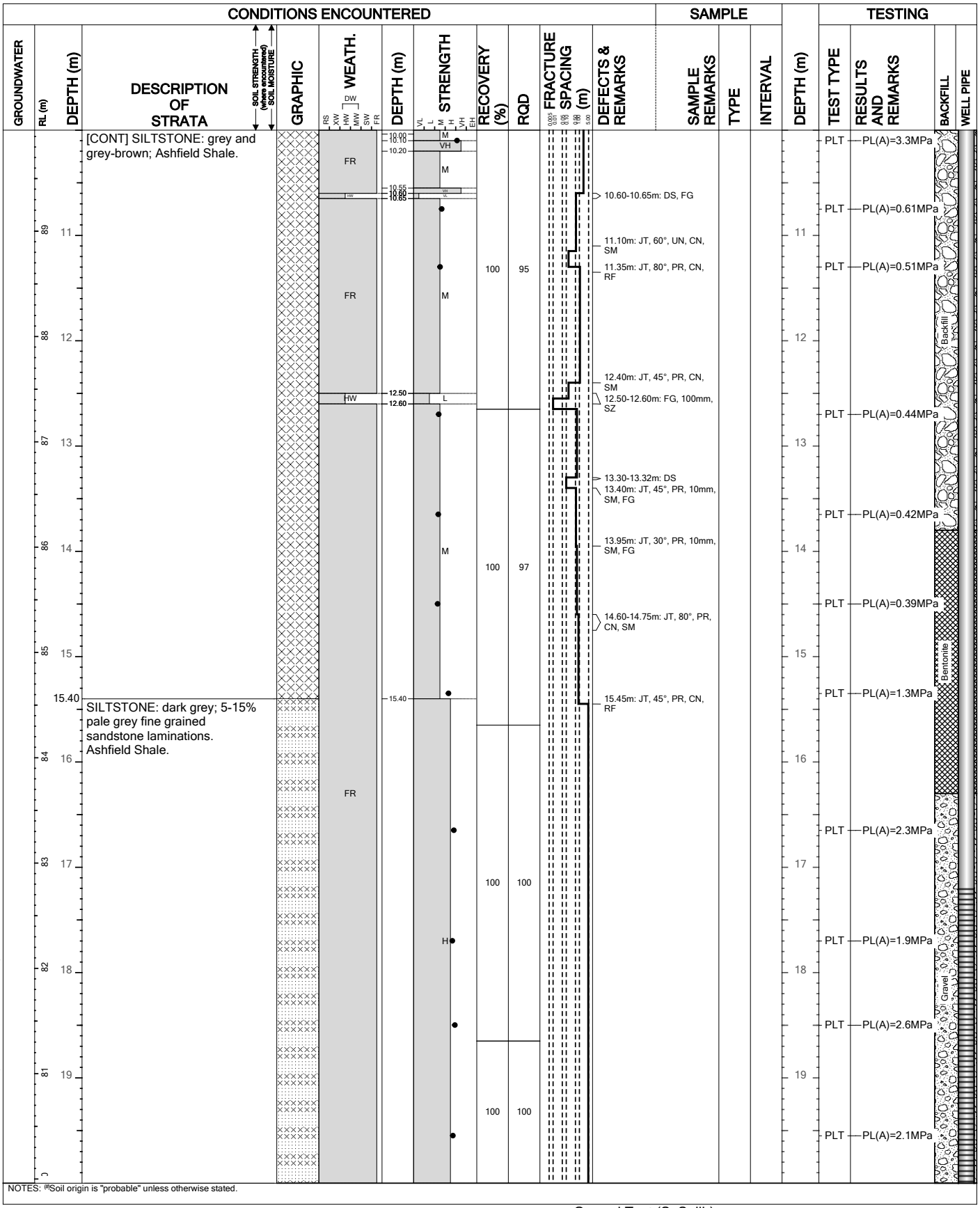
Douglas PARTNERS

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 100.0 AHD
COORDINATE: E:311887.1, N:6265641.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH205
PROJECT No: 233937.00
DATE: 16/07/25
SHEET: 3 of 4



NOTES: (#)Soil origin is "probable" unless otherwise stated.

Generated with CORE-GS by Geroc - Split Soil-Rock Log

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.0m, NMLC to 20.20m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 100.0 AHD
COORDINATE: E:311887.1, N:6265641.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH205
PROJECT No: 233937.00
DATE: 16/07/25
SHEET: 4 of 4

CONDITIONS ENCOUNTERED														SAMPLE				TESTING			
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA (SOIL STRENGTH (where encountered) SOIL MOISTURE)	GRAPHIC	WEATH. (RS XW HW MW SW FR; DW)	DEPTH (m)	STRENGTH (VL L M H VH EH)	RECOVERY (%)	RQD	FRACTURE SPACING (m) (0.05 0.06 0.08 0.60)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
			[CONT] SILTSTONE: dark grey; 5-15% pale grey fine grained sandstone laminations. Ashfield Shale.		FR		H	100	100							PLT	PL(A)=1.6MPa	Gravel	
	79	21	Borehole discontinued at 20.20m depth. Target depth reached.																
	78	22																	
	77	23																	
	76	24																	
	75	25																	
	74	26																	
	73	27																	
	72	28																	
	71	29																	
	0																		

NOTES: (R)Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.0m, NMLC to 20.20m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 100.0 AHD
COORDINATE: E:311887.1, N:6265641.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH205
PROJECT No: 233937.00
DATE: 16/07/25
SHEET: 1 of 2

2.00-6.00 m depth

6.00-11.00 m depth

Generated with CORE-GS by Geroc - Split Soil-Rock Log

Douglas PARTNERS

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 100.0 AHD
COORDINATE: E:311887.1, N:6265641.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH205
PROJECT No: 233937.00
DATE: 16/07/25
SHEET: 2 of 2

11.00-16.00 m depth

16.00-20.20 m depth

Generated with CORE-GS by Geroc - Split Soil-Rock Log

Douglas PARTNERS

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.2 AHD
COORDINATE: E:311759.4, N:6265628.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH206
PROJECT No: 233937.00
DATE: 07/07/25 - 08/07/25
SHEET: 1 of 3

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*) DENSITY.(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
07/07/25 No Free Groundwater Observed Whilst Augering	89	0.03	ASPHALTIC CONCRETE.						A		0.10		
			FILL / CLAY with gravel: grey to brown; medium plasticity; medium to coarse, igneous and siltstone gravel.		FILL	ND	w=PL	X2	A		0.20 0.50		
											0.60		
		0.90			RS				A		0.90		
	88	1.00	CLAY (CI-CH) with gravel: brown; medium to high plasticity; fine, siltstone gravel.								1.00		
			CLAY (CI-CH) with gravel: dark brown to orange; medium to high plasticity; fine, siltstone gravel.		XWM	H	w<PL		SPT		1.45	SPT	7,13,20 N=33
		2.00	Continued as rock log										
	87										2		
	86	3									3		
	85	4									4		
	84	5									5		
	83	6									6		
	82	7									7		
	81	8									8		
	80	9									9		

NOTES: (#)Soil origin is "probable" unless otherwise stated. (*)Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.0m, NMLC to 15.25m
REMARKS:

OPERATOR: Ground Test (S. Salib)

LOGGED: C. Lambert-Bridges
CASING: HWT to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.2 AHD
COORDINATE: E:311759.4, N:6265628.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH206
PROJECT No: 233937.00
DATE: 07/07/25 - 08/07/25
SHEET: 2 of 3

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	SOIL STRENGTH (Where encountered) / SOIL MOISTURE	GRAPHIC	WEATH. (RS, XW, HW, DW, MW, SW, FR)	DEPTH (m)	STRENGTH (VL, L, M, H, VH, EH)	RECOVERY (%)	RQD	FRACTURE SPACING (m) (0.05, 0.06, 0.08, 0.10, 0.20)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
07/07/25 No Free Groundwater Observed Whilst Augering	89	0														0				
	88	1														1				
	87	2	Continued from soil log													2				
		2.20	SILTSTONE: dark grey. Ashfield Shale.				2.20	SEAM			SEAM	2.20-2.30m: EW, Clay								
							2.30													
							2.40													
								SEAM	83	0	SEAM	2.40-2.86m: IS x6, 0-20°, UN, SN Fe, SM, TI				PLT	PL(A)=0.02MPa		
	86	3				XW - HW	2.86									3				
							2.95					2.95-3.00m: EW								
							3.00					3.00-3.29m: B x9, 0°, PR, CT Clay, SM, 1-2mm								
		3.30						VL												
		3.45	SILTSTONE: dark grey. Ashfield Shale.																	
	85	4					3.60	SEAM			SEAM	3.62-3.66m: JT, 60°, IR 3mm, SM				4				
												3.60-3.87m: EW								
							3.87					3.92m: B, PR, CT Clay 5mm, SM								
							3.93					4.09m: B, PR, CN, SM								
									75	69		4.19m: B, PR, CN, SM					PLT	PL(A)=0.49MPa		
	84	5						M				4.92m: B, PR, VNR Clay, SM				5	PLT	PL(A)=0.72MPa		
												5.09m: JT, 5°, UN, CN, SM					PLT	PL(A)=0.77MPa		
												5.20m: JT, 30°, UN, CN, SM								
												5.42m: B, PR, VNR Clay, SM								
	83	6					6.00									6				
		6.10	SILTSTONE: dark grey; with 10-20% pale grey fine grained sandstone. Ashfield Shale.														PLT	PL(A)=1.9MPa		
									100	100		6.70m: B, PR, VNR Clay, SM								
	82	7				FR										7	PLT	PL(A)=1.6MPa		
								H												
	81	8										8.35m: B, PR, CN, SM				8	PLT	PL(A)=1.8MPa		
									100	100										
	80	9										9.50m: B, PR, VNR Clay, SM				9	PLT	PL(A)=2.4MPa		
																	PLT	PL(A)=2.5MPa		

NOTES: (R)Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.0m, NMLC to 15.25m
REMARKS:

OPERATOR: Ground Test (S. Salib)

LOGGED: C. Lambert-Bridges
CASING: HWT to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.2 AHD
COORDINATE: E:311759.4, N:6265628.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH206
PROJECT No: 233937.00
DATE: 07/07/25 - 08/07/25
SHEET: 3 of 3

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA (SOIL STRENGTH (where encountered) / SOIL MOISTURE)	GRAPHIC	WEATH. (RS, XW, HW, MW, SW, FR; DW)	DEPTH (m)	STRENGTH (VL, L, M, H, VH, EH)	RECOVERY (%)	RQD	FRACTURE SPACING (m) (0.05, 0.06, 0.08, 0.10, 0.20)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
	79	11	[CONT] SILTSTONE: dark grey; with 10-20% pale grey fine grained sandstone. Ashfield Shale.					100	100		10.90m: B, PR, VNR Clay, SM				11	PLT	PL(A)=2.2MPa		
	11.00 78		SILTSTONE: dark grey; with <5% pale grey fine grained sandstone. Ashfield Shale.								11.82m: B, PR, VNR Clay, SM								
	77	12			FR		H	100	100						12	PLT	PL(A)=1.9MPa		
	76	13													13	PLT	PL(A)=1.2MPa		
	75	14									13.90m: B, PR, VNR Clay, SM				14	PLT	PL(A)=1.5MPa		
	74	15				14.80	M H	100	100		14.86m: JT, 45°, PR, VNR Clay, SM 15.00m: JT, 45°, PR, CN, SM 15.20m: B, PR, VNR Clay, SM				15	PLT	PL(A)=0.75MPa		
			Borehole discontinued at 15.25m depth. Target depth reached.																
	73	16																	
	72	17																	
	71	18																	
	70	19																	

NOTES: (R)Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.0m, NMLC to 15.25m
REMARKS:

OPERATOR: Ground Test (S. Salib)

LOGGED: C. Lambert-Bridges
CASING: HWT to 1m

Refer to explanatory notes for symbol and abbreviation definitions

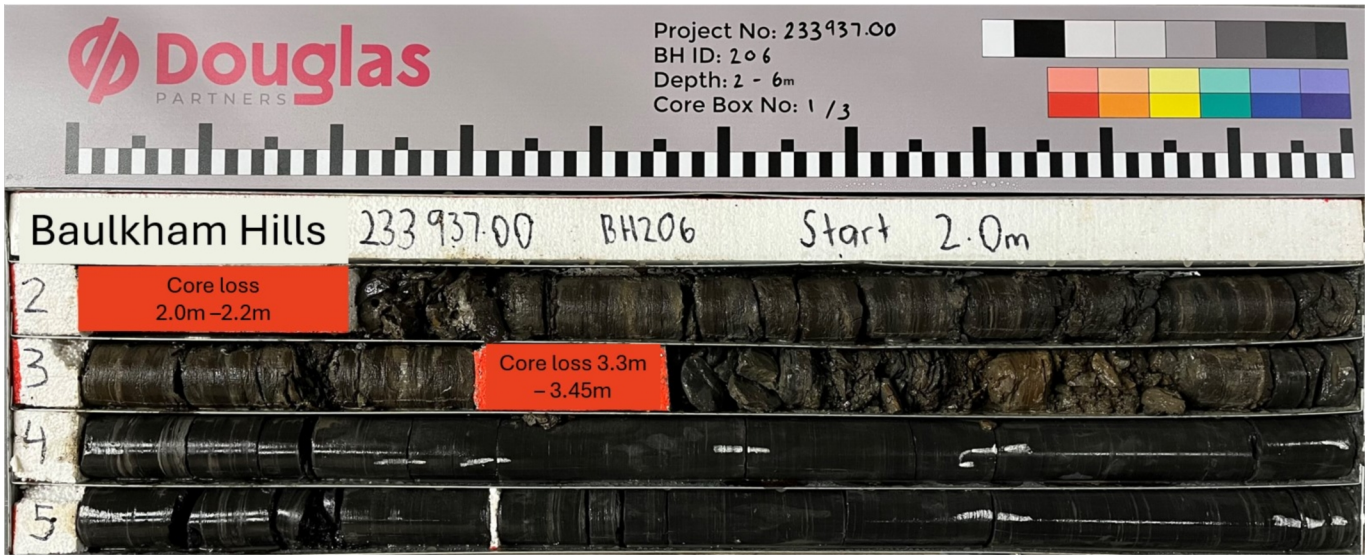
Generated with CORE-GS by Geroc - Split Soil-Rock Log

CORE PHOTO LOG

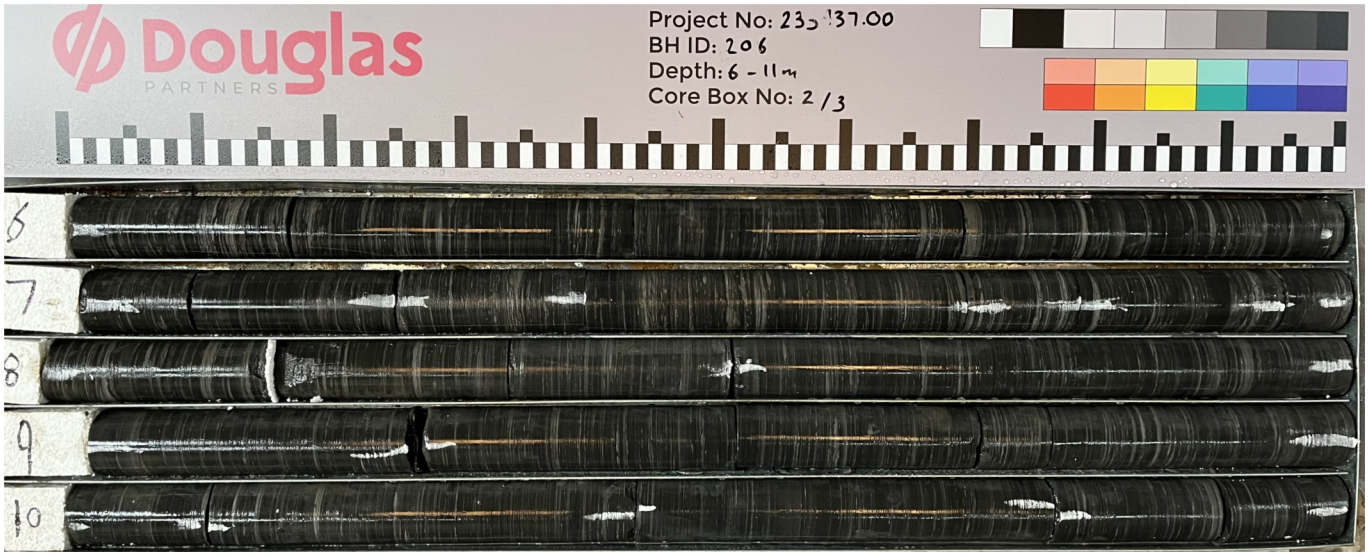
CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.2 AHD
COORDINATE: E:311759.4, N:6265628.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH206
PROJECT No: 233937.00
DATE: 07/07/25 - 08/07/25
SHEET: 1 of 2



2.00-6.00 m depth



6.00-11.00 m depth

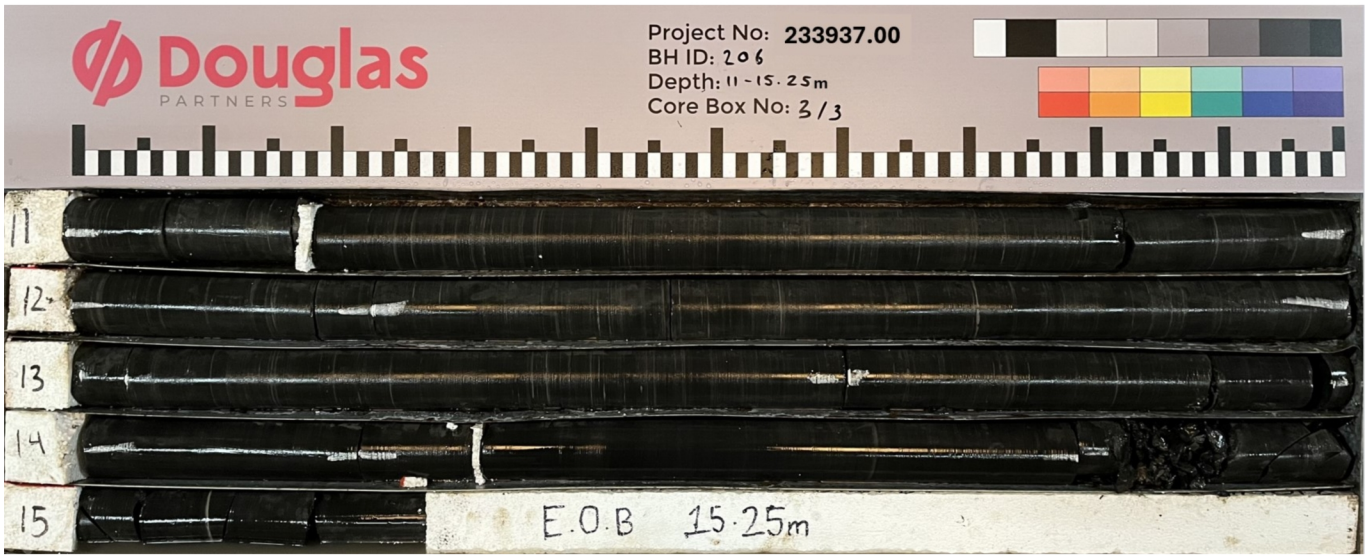
Generated with CORE-GS by Geroc - Split Soil-Rock Log

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 89.2 AHD
COORDINATE: E:311759.4, N:6265628.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH206
PROJECT No: 233937.00
DATE: 07/07/25 - 08/07/25
SHEET: 2 of 2



11.00-15.25 m depth

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 90.6 AHD
COORDINATE: E:311795.2, N:6265613.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH207
PROJECT No: 233937.00
DATE: 08/07/25
SHEET: 1 of 3

CONDITIONS ENCOUNTERED								SAMPLE				TESTING AND REMARKS	
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*) DENSITY.(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
08/07/25 No Free Groundwater Observed Whilst Augering	90	0.30	TOPSOIL / Sandy CLAY trace gravel: dark brown; fine, igneous and siltstone gravel; with roots.	TS TS T	TOP / TOP	NA	w=PL		A		0.10		
		0.70	CLAY (CI-CH) with gravel: brown; medium to high plasticity; weathered siltstone gravel.		RS	(St) to (VSt)			A		0.50		
	89	1	CLAY (CI) with gravel: pale brown; medium plasticity.		XWM	H	w<PL		A SPT		1.00 1.25	SPT	20,25/100 refusal
	88	2.00	Continued as rock log								2		
	87	3									3		
	86	4									4		
	85	5									5		
	84	6									6		
	83	7									7		
	82	8									8		
	81	9									9		

NOTES: (#)Soil origin is "probable" unless otherwise stated. (*)Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.0m, NMLC to 15.38m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: C. Lambert-Bridges
CASING: HW to 1m

Refer to explanatory notes for symbol and abbreviation definitions

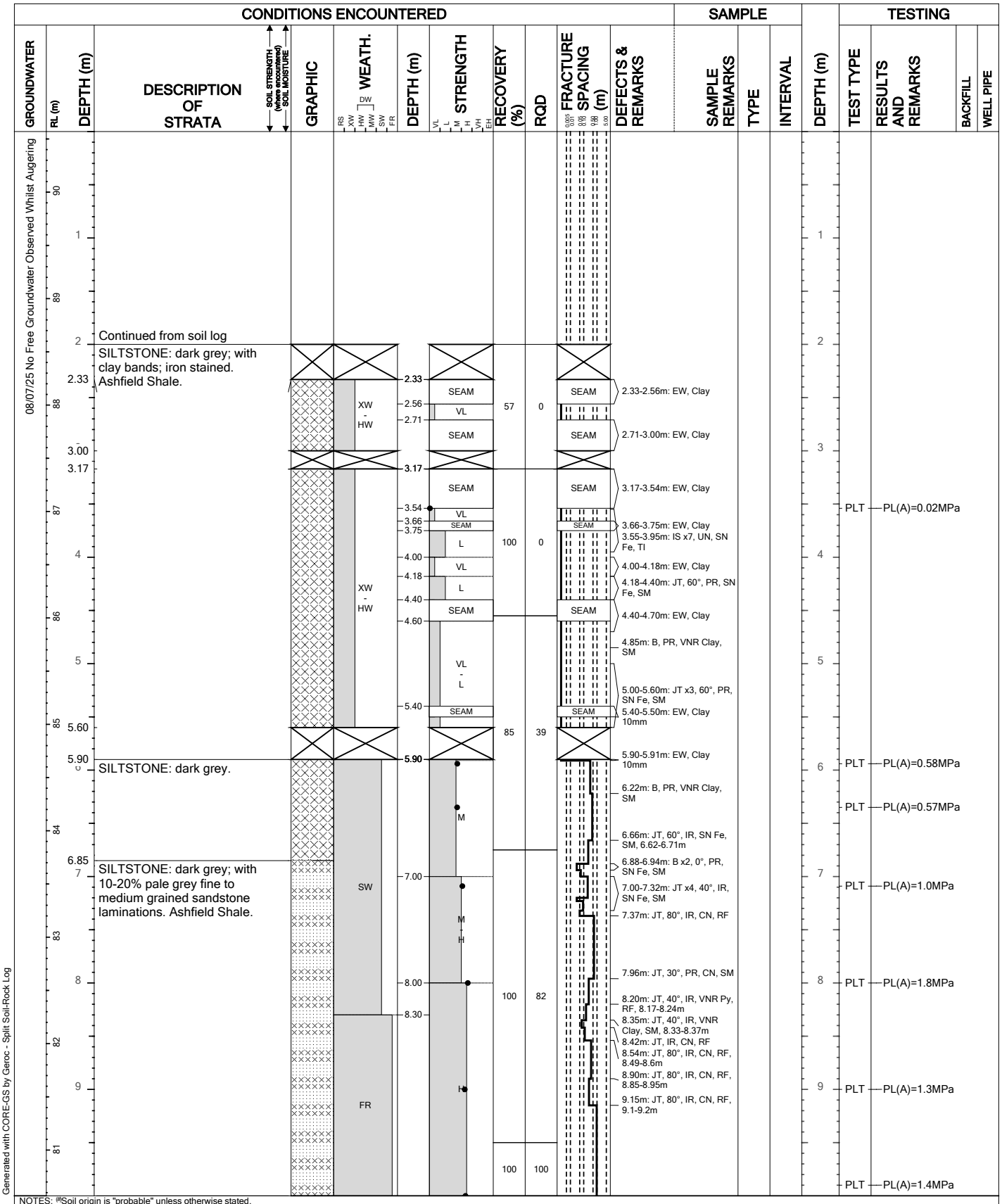
Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 90.6 AHD
COORDINATE: E:311795.2, N:6265613.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH207
PROJECT No: 233937.00
DATE: 08/07/25
SHEET: 2 of 3



NOTES: [#]Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.0m, NMLC to 15.38m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: C. Lambert-Bridges
CASING: HW to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 90.6 AHD
COORDINATE: E:311795.2, N:6265613.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH207
PROJECT No: 233937.00
DATE: 08/07/25
SHEET: 3 of 3

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA (SOIL STRENGTH (where encountered) / SOIL MOISTURE)	GRAPHIC	WEATH. (RS, XW, HW, MW, SW, FR; DW)	DEPTH (m)	STRENGTH (VL, L, M, H, VH, EH)	RECOVERY (%)	RQD	FRACTURE SPACING (m) (0.03, 0.06, 0.09, 0.60, 2.00)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
	80	11	[CONT] SILTSTONE: dark grey; with 10-20% pale grey fine to medium grained sandstone laminations. Ashfield Shale.								10.10m: JT, 30°, IR, CN, RF				11	PLT	PL(A)=2.7MPa		
		11.20									11.12m: JT, 50°, IR, CN, RF								
	79		SILTSTONE: dark grey; <5% pale grey fine to medium grained sandstone laminations. Ashfield Shale.					100	100		11.53m: JT, 50°, IR, CN, RF								
		12									12.06m: JT, 20°, IR, CN, RF				12	PLT	PL(A)=2.5MPa		
	78				FR		H												
		13													13	PLT	PL(A)=1.4MPa		
	77																		
		14						100	100						14	PLT	PL(A)=1.3MPa		
	76																		
		15													15	PLT	PL(A)=1.0MPa		
	75		Borehole discontinued at 15.38m depth. Target depth reached.																
		16																	
	74																		
		17																	
	73																		
		18																	
	72																		
		19																	
	71																		

NOTES: (R)Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.0m, NMLC to 15.38m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: C. Lambert-Bridges
CASING: HW to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 90.6 AHD
COORDINATE: E:311795.2, N:6265613.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH207
PROJECT No: 233937.00
DATE: 08/07/25
SHEET: 1 of 2

2.00-6.00 m depth

6.00-11.00 m depth

Generated with CORE-GS by Geroc - Split Soil-Rock Log

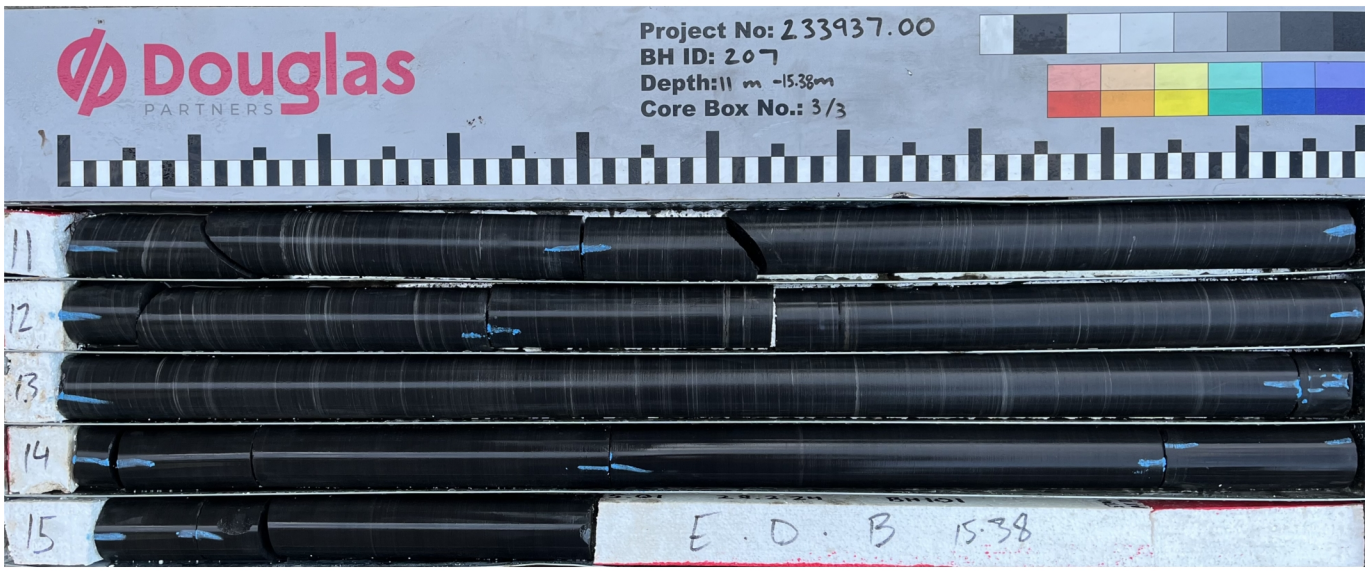
Douglas PARTNERS

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 90.6 AHD
COORDINATE: E:311795.2, N:6265613.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH207
PROJECT No: 233937.00
DATE: 08/07/25
SHEET: 2 of 2



11.00-15.38 m depth

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 95.5 AHD
COORDINATE: E:311839.2, N:6265615.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH208
PROJECT No: 233937.00
DATE: 09/07/25
SHEET: 1 of 4

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*) DENSITY.(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
09/07/25 No Free Groundwater Observed Whilst Augering	95	0.02	ASPHALTIC CONCRETE.		FILL	ND	D		A		0.10		
		0.35	FILL / Sandy GRAVEL: pale brown; igneous and siltstone gravel.		FILL		M						
		0.55	FILL / Sandy CLAY trace gravel: dark brown; medium plasticity; coarse, siltstone and sandstone gravel.						A		0.50		
		1	CLAY (CH): orange brown; high plasticity.		RS	VSt			A		1.00		
	94	1.30	CLAY (CH) with gravel: orange-brown; high plasticity.		XWM	H	w<PL		SPT		1.40	SPT	9,15,25/100
		2									2		
	93	2.10	Continued as rock log										
		3									3		
	92	4									4		
	91	5									5		
	90	6									6		
	89	7									7		
	88	8									8		
	87	9									9		
	86												

NOTES: (#)Soil origin is "probable" unless otherwise stated. (*)Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.10m, NMLC to 20.35m
REMARKS:

OPERATOR: Ground Test (S. Salib)

LOGGED: C. Lambert-Bridges
CASING: HWT to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 95.5 AHD
COORDINATE: E:311839.2, N:6265615.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH208
PROJECT No: 233937.00
DATE: 09/07/25
SHEET: 2 of 4

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	WEATH.	DEPTH (m)	STRENGTH	RECOVERY (%)	RQD	FRACTURE SPACING (m)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
09/07/25 No Free Groundwater Observed Whilst Augering	95	0	Continued from soil log								2.10m: , Unless otherwise stated, all defects are B or P, 0-20°, PR, IR, SM and CN				0				
	94	1	SILTSTONE: pale brown to pale grey; frequent clay seams; iron stained. Ashfield Shale.		HW	2.10	VL								1				
	93	2			XW	2.30	SEAM	90	0	SEAM	2.30-3.00m: EW, Clay				2				
		3.00																	
		3.17	SILTSTONE: pale brown and dark grey; frequent decomposed seams and clay seams; iron stained. Ashfield Shale.		HW	3.17	VL												
					XW	3.25	SEAM			SEAM	3.25-3.31m: EW, Clay								
					HW	3.31	VL												
	92	4			XW	3.37	SEAM			SEAM	3.37-4.06m: EW, Clay				3				
					HW	4.06	L	96	0		4.06-4.56m: IS x10, UN, SN Fe, TI				4				
	91	5			XW	4.56	SEAM			SEAM	4.56-5.30m: EW, Clay				5				
		5.30	SILTSTONE: brown to black; crushed seams potentially from faulting; occasional iron staining. Ashfield Shale.		HW	5.30	VL				5.35m: B, PR, VNR Clay, SM								
	90	6									5.47m: JT, 30°, IR, SN Fe, RF				6				
											5.50m: B, 0°, PR, CT Clay 5mm, TI								
											5.55m: B, 0°, PR, CT Clay 5mm, SM								
											5.62m: CS, 40mm								
											5.72m: CS, 40mm								
											5.86m: CS, 30mm					PLT	PL(A)=0.22MPa		
											6.07m: B, PR, CN, SM								
											6.13m: JT, 30°, IR, SN Fe, TI					PLT	PL(A)=0.31MPa		
						6.10	L				6.14m: EW, Clay								
					SW	6.50	M	100	65		6.16-6.31m: B x5, PR, VNR Clay, SM								
					XW	6.83	SEAM			SEAM	6.34m: B, 0°, IR, SN Fe, TI, CT, Clay								
		7.00	SILTSTONE: dark grey; <5% pale grey fine to medium grained sandstone laminations. Ashfield Shale.			6.92					6.42m: B, PR, SN Fe, SM				7	PLT	PL(A)=0.45MPa		
						6.95					6.45m: EW, 10mm								
											6.53m: JT, 15°, PR, CN, SM								
											6.59m: JT, 30°, PR, CN, SM								
											6.68m: JT, 80°, PR, CN								
	88	8			SW		M				6.75m: B, 0°, PR, CT Clay, SM				8				
											6.83-6.92m: SS, CT Clay								
											7.03m: JT, 30°, IR, CN, SM								
											7.10m: JT, 60°, PR, CN, SM					PLT	PL(A)=0.40MPa		
	87	9									7.24m: JT, 70°, UN, CN, SM, 7.19-7.3				9				
											7.42m: JT, 7.35-7.5m								
											7.60m: B, PR, CT Clay, SM, 3mm								
								100	91		7.72m: JT, 45°, PR, CN, SM					PLT	PL(A)=0.48MPa		
	86										7.90m: JT, 80°, UN, CN, SM								
											8.13m: JT, PR, CN, SM, 8.11-8.16m								
											8.31m: B, 0°, PR, CT Clay, SM								
					FR	9.76					8.40m: B, PR, CT Clay,								

NOTES: [#]Soil origin is "probable" unless otherwise stated.

Generated with CORE-GS by Geroc - Split Soil-Rock Log

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.10m, NMLC to 20.35m
REMARKS:

OPERATOR: Ground Test (S. Salib)

LOGGED: C. Lambert-Bridges
CASING: HWT to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 95.5 AHD
COORDINATE: E:311839.2, N:6265615.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH208
PROJECT No: 233937.00
DATE: 09/07/25
SHEET: 3 of 4

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA (SOIL STRENGTH (where encountered) / SOIL MOISTURE)	GRAPHIC	WEATH. (RS, XW, HW, DW, MW, SW, FR)	DEPTH (m)	STRENGTH (VL, L, M, H, VH, EH)	RECOVERY (%)	RQD	FRACTURE SPACING (m) (0.05, 0.06, 0.08, 0.10)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
	85	11	[CONT] SILTSTONE: dark grey; <5% pale grey fine to medium grained sandstone laminations. Ashfield Shale.				M	100	91		SM, 1mm 8.57m: JT, 40°, IR, CN, RF 8.62m: JT, 40°, IR, CN 8.64-8.66m: JT, 50°, IR, CN, RF 9.32m: B, 0°, PR, CT Clay, SM, 1mm				11	PLT	PL(A)=0.66MPa		
	84	11.65	SILTSTONE: dark grey; with 10-20% pale grey fine to medium grained sandstone laminations. Ashfield Shale.			11.50					11.10m: JT, 40°, PR, CN, SM 11.39m: JT x2, 80°, PR, CN, RF, 11.29-11.5m								
	83	12													12	PLT	PL(A)=1.5MPa		
	82	13						100	100						13	PLT	PL(A)=1.3MPa		
	81	14									14.03m: JT, 40°, PR, CN, SM				14	PLT	PL(A)=1.8MPa		
	80	15			FR		H				14.83m: JT, 20°, PR, CN, SM 15.45m: JT, 40°, PR, CN, RF				15	PLT	PL(A)=2.9MPa		
	79	16	16.00 SILTSTONE: dark grey; with <5% pale grey fine to medium grained sandstone laminations. Ashfield shale.					100	100						16	PLT	PL(A)=1.7MPa		
	78	17													17	PLT	PL(A)=1.2MPa		
	77	18				18.50		100	100		17.70m: JT, 50°, PR, CN, SM				18	PLT	PL(A)=2.1MPa		
	76	19					M								19	PLT	PL(A)=0.95MPa		
																PLT	PL(A)=0.79MPa		

NOTES: [R]Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.10m, NMLC to 20.35m
REMARKS:

OPERATOR: Ground Test (S. Salib)

LOGGED: C. Lambert-Bridges
CASING: HWT to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 95.5 AHD
COORDINATE: E:311839.2, N:6265615.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH208
PROJECT No: 233937.00
DATE: 09/07/25
SHEET: 4 of 4

CONDITIONS ENCOUNTERED														SAMPLE				TESTING			
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA (SOIL STRENGTH (Where encountered) SOIL MOISTURE)	GRAPHIC	WEATH. (RS, XW, DW, HW, MW, SW, FR)	DEPTH (m)	STRENGTH (VL, L, M, H, VH, EH)	RECOVERY (%)	RQD	FRACTURE SPACING (m) (0.005, 0.05, 0.06, 0.2, 0.6, 2.00)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
	75	20	[CONT] SILTSTONE: dark grey; with <5% pale grey fine to medium grained sandstone laminations. Ashfield shale.		FR		M	100	100										
			Borehole discontinued at 20.35m depth. Target depth reached.																
	74	21																	
	73	22																	
	72	23																	
	71	24																	
	70	25																	
	69	26																	
	68	27																	
	67	28																	
	66	29																	

NOTES: [R]Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.10m, NMLC to 20.35m
REMARKS:

OPERATOR: Ground Test (S. Salib)

LOGGED: C. Lambert-Bridges
CASING: HWT to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 95.5 AHD
COORDINATE: E:311839.2, N:6265615.0
DATUM/GRID: MGA2020 Zone 56
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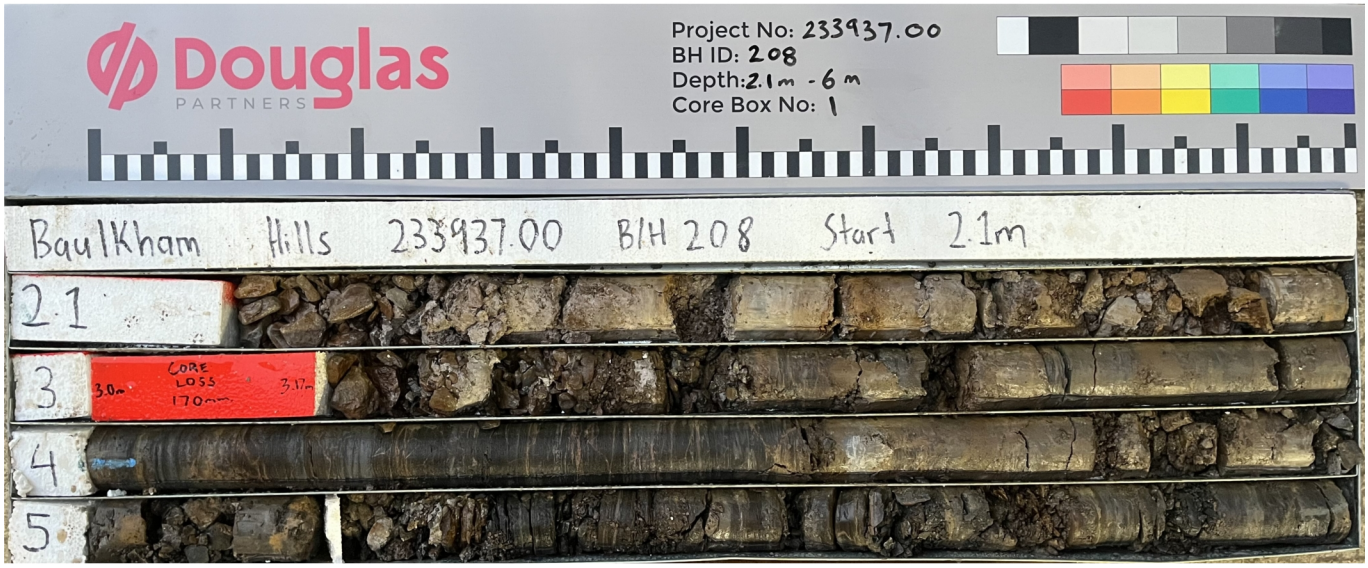
LOCATION ID: BH208
PROJECT No: 233937.00
DATE: 09/07/25
SHEET: 1 of 2



2.10-6.00 m depth

6.00-11.00 m depth

Generated with CORE-GS by Geroc - Split Soil-Rock Log

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 95.5 AHD
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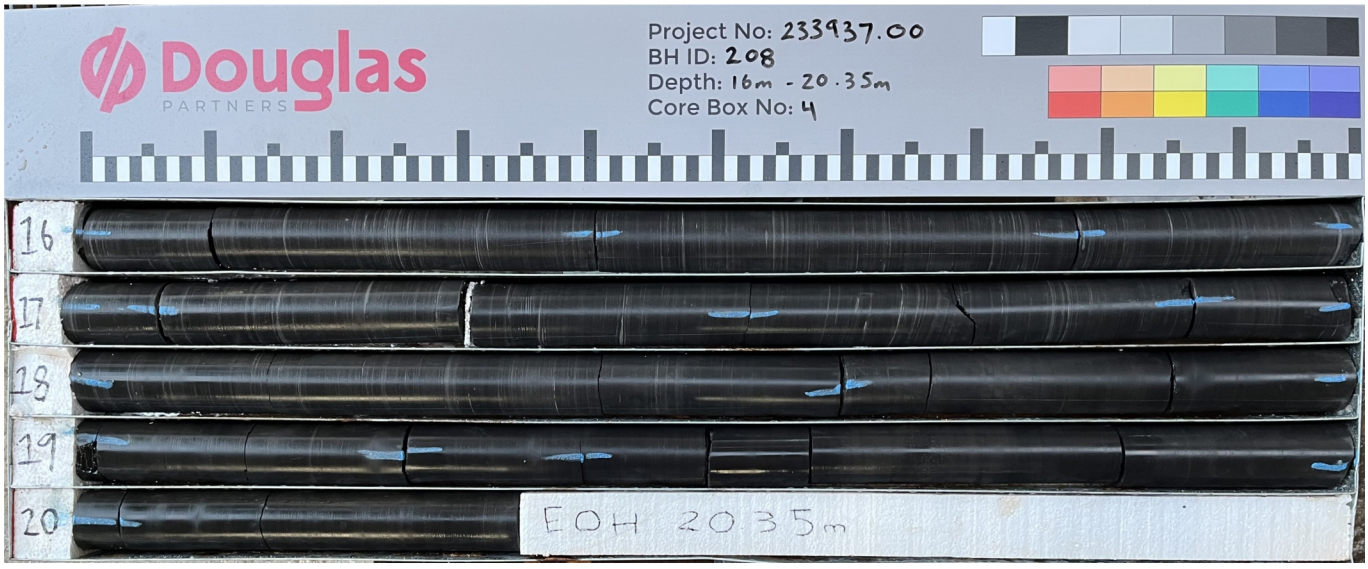
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH208
PROJECT No: 233937.00
DATE: 09/07/25
SHEET: 2 of 2

11.00-16.00 m depth



16.00-20.35 m depth

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 96.7 AHD
COORDINATE: E:311876.1, N:6265601.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH209
PROJECT No: 233937.00
DATE: 10/07/25
SHEET: 1 of 4

CONDITIONS ENCOUNTERED								SAMPLE				TESTING AND REMARKS	
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*) DENSITY.(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
09/07/25 No Free Groundwater Observed Whilst Augering	96	0.25	TOPSOIL / CLAY: dark brown; medium plasticity; with rootlets.		TOP / TOP	ND	w=PL		A		0.10		
			CLAY (CH): red-brown; high plasticity.								0.30		
			0.65m-1.40m: Orange-brown		RS	St to VSt			SPT A B		0.50 0.70	SPT	3,6,11 N=17
	95	1.40	CLAY (CH): pale grey; high plasticity; with pockets of highly weathered very low strength siltstone.		XWM	(H)	w<PL	X2	A		1.00		
	94	2.20	Continued as rock log										

NOTES: (#)Soil origin is "probable" unless otherwise stated. (*)Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.20m, NMLC to 20.25m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: C. Lambert-Bridges /
CASING: HWT to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 96.7 AHD
COORDINATE: E:311876.1, N:6265601.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH209
PROJECT No: 233937.00
DATE: 10/07/25
SHEET: 2 of 4

09/07/25 No Free Groundwater Observed Whilst Augering

Continued from soil log

2.32 — SILTSTONE: grey-brown; 10% clay, Ashfield Shale.

4.00 — SILTSTONE: pale grey and grey; Ashfield Shale.

6.60 — SILTSTONE: grey and dark grey; with 5% sandstone laminations, Ashfield Shale.

DEPTH (m)	RECOVERY (%)	RQD
0.00–2.32	90	0
2.32–6.60	100	17
6.60–6.75	100	98
6.75–9.80	100	100

DEPTH (m)	STRENGTH
2.32	L - M
2.60	VL
2.80	L
3.00	VL
3.35	L
6.55	M
6.60	M
6.75	VH
6.85	M

WEATH.: HW (2.32–6.55), FR (6.60–9.80)

DEPTH (m)	DEFECTS & REMARKS
2.32–3.35	FG
3.35–3.55	B x4, 0-5°, CT Clay, 1-3 mm
3.55–3.80	FG
3.80	EW, 10mm
3.88	JT, 30°, PR, CN, SM
4.05	B, PR, VNR Clay, SM
4.15	JT, 30°, PR, CN, SM
4.40	B, PR, VNR Clay, SM
4.55	B, 0°, Clay 20mm
4.81	B, PR, VNR Clay, SM
5.14	B, PR, SN Fe, SM
5.36	JT, 45°, PR, CN, SM
5.60	JT, 45°, PR, CN, SM
5.65	JT, 80°, PR, CN, SM
5.75	B, PR, VNR Clay, SM
5.85	JT, 80°, TI
6.00	JT, 85°, UN, CN, RF
6.25	JT, 80°, UN, CN, RF
6.40	JT, 45°, PR, CN, SM
6.55–6.60	EW, 50mm
7.18	B, PR, VNR Clay, SM
7.28	B, PR, CN, SM
7.55	EW, 20mm
8.70	JT, 30°, PR, CN, SM
8.88	JT, 15°, PR, CN, SM
9.80	JT, 45°, PR, CN, SM

DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
4.00	PLT	PL(A)=0.20MPa
4.50	PLT	PL(A)=0.20MPa
5.50	PLT	PL(A)=0.17MPa
6.50	PLT	PL(A)=0.23MPa
6.60	PLT	PL(A)=3.5MPa
8.00	PLT	PL(A)=0.43MPa
8.70	PLT	PL(A)=0.51MPa
9.80	PLT	PL(A)=0.41MPa

NOTES: (R)Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.20m, NMLC to 20.25m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: C. Lambert-Bridges /
CASING: HWT to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 96.7 AHD
COORDINATE: E:311876.1, N:6265601.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH209
PROJECT No: 233937.00
DATE: 10/07/25
SHEET: 3 of 4

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	SOIL STRENGTH (Where encountered) / SOIL MOISTURE	GRAPHIC	WEATH. (RS, XW, HW, DW, MW, SW, FR)	DEPTH (m)	STRENGTH (VL, L, M, H, VH, EH)	RECOVERY (%)	RQD	FRACTURE SPACING (m)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
	86	11	[CONT] SILTSTONE: grey and dark grey; with 5% sandstone laminations, Ashfield Shale.				10.00	M	100	100		10.90m: B, 0°, PR, CN, SM				11	PLT	PL(A)=0.42MPa		
	85																PLT	PL(A)=0.64MPa		
		12.00	INTERBEDDED SILTSTONE AND SANDSTONE: : 30-70%, pale grey, fine to medium grained sandstone interbedded and interlaminated with grey and dark grey siltstone. Ashfield Shale.				12.00									12				
	84	13							100	100		13.15m: JT, 45°, PR, CN, SM				13	PLT	PL(A)=2.1MPa		
	83																PLT	PL(A)=2.1MPa		
		14						H								14				
	82																PLT	PL(A)=1.8MPa		
		15				FR										15				
	81								100	100							PLT	PL(A)=3.1MPa		
		16.00	SILTSTONE: dark grey; with 5% sandstone laminations, Ashfield Shale.													16	PLT	PL(A)=2.1MPa		
	80	17					17.00					16.80-17.00m: JT, 85°, PR, CN, SM				17	PLT	PL(A)=1.3MPa		
	79											17.65m: JT, 45°, PR, CN, SM, TI								
		18						H								18	PLT	PL(A)=1.5MPa		
	78								100	100										
		19					19.50	M H								19	PLT	PL(A)=1.5MPa		
	77																			

NOTES: [R]Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.20m, NMLC to 20.25m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: C. Lambert-Bridges /
CASING: HWT to 1m

Generated with CORE-GS by Geroc - Split Soil-Rock Log

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 96.7 AHD
COORDINATE: E:311876.1, N:6265601.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH209
PROJECT No: 233937.00
DATE: 10/07/25
SHEET: 4 of 4

CONDITIONS ENCOUNTERED														SAMPLE				TESTING			
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA (SOIL STRENGTH (where encountered) SOIL MOISTURE)	GRAPHIC	WEATH. (RS XW HW MW SW FR; DW)	DEPTH (m)	STRENGTH (VL L M H VH EH)	RECOVERY (%)	RQD	FRACTURE SPACING (m) (0.05 0.06 0.20 0.60 2.00)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
			[CONT] SILTSTONE: dark grey; with 5% sandstone laminations, Ashfield Shale.		FR	20.00	M	100	100							PLT	PL(A)=0.99MPa		
	76	21	Borehole discontinued at 20.25m depth. Target depth reached.																
	75	22																	
	74	23																	
	73	24																	
	72	25																	
	71	26																	
	70	27																	
	69	28																	
	68	29																	
	67																		

NOTES: [R]Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 1.0m, WB to 2.20m, NMLC to 20.25m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: C. Lambert-Bridges /
CASING: HWT to 1m

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

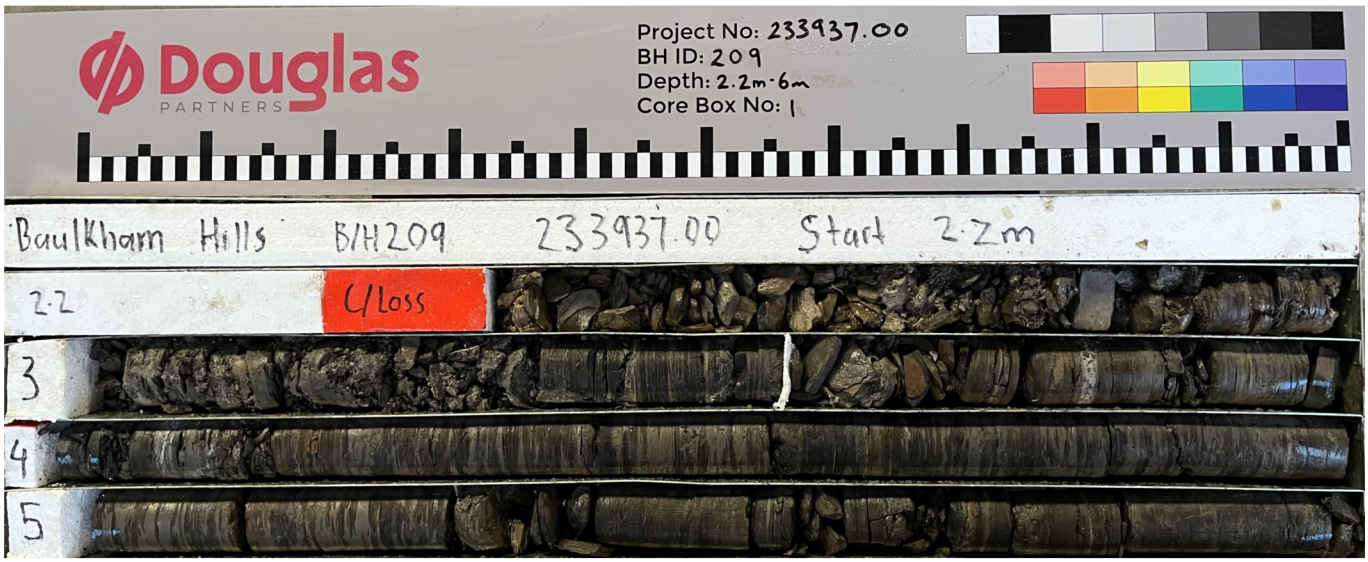
Generated with CORE-GS by Geroc - Split Soil-Rock Log

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 96.7 AHD
COORDINATE: E:311876.1, N:6265601.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH209
PROJECT No: 233937.00
DATE: 10/07/25
SHEET: 1 of 2



2.20-6.00 m depth

6.00-11.00 m depth

Generated with CORE-GS by Geroc - Split Soil-Rock Log

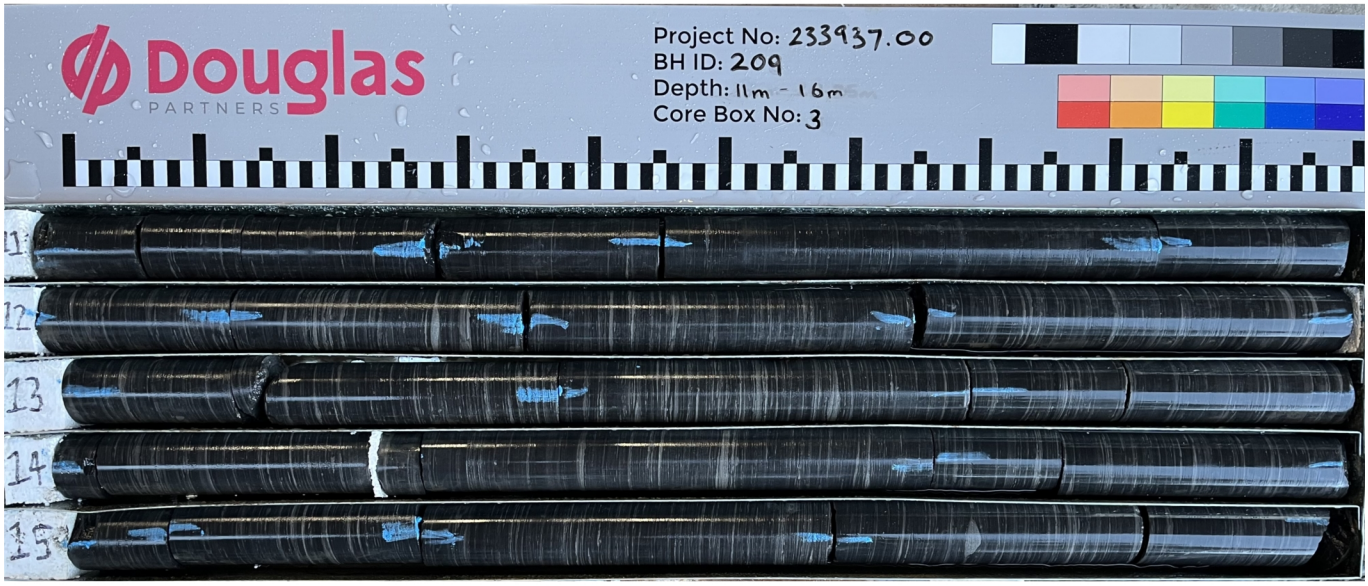
Douglas PARTNERS

CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 96.7 AHD
COORDINATE: E:311876.1, N:6265601.0
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH209
PROJECT No: 233937.00
DATE: 10/07/25
SHEET: 2 of 2



11.00-16.00 m depth

16.00-20.25 m depth

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 100.7 AHD
COORDINATE: E:311907.6, N:6265619.2
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH210
PROJECT No: 233937.00
DATE: 18/07/25
SHEET: 1 of 3

CONDITIONS ENCOUNTERED								SAMPLE				TESTING AND REMARKS	
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	ORIGIN(#)	CONSIS.(*) DENSITY.(*)	MOISTURE	REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS
		0.20	TOPSOIL / Silty CLAY trace grass roots: grey; low plasticity.		TOP FILL	ND	w<PL		A		0.10		
			CLAY (CH): grey brown; high plasticity.		RS	(St)	w=PL		A		0.40 0.50		
	100	1							A		0.90 1.00		
		1.20	CLAY (CI): brown; medium plasticity.		RS	VSt - H	w<PL		SPT		1.45	SPT	12,15,18 N=33
	99	2.00	Continued as rock log								2		
	98	3									3		
	97	4									4		
	96	5									5		
	95	6									6		
	94	7									7		
	93	8									8		
	92	9									9		
	91												

NOTES: (#)Soil origin is "probable" unless otherwise stated. (*)Consistency/Relative density shading is for visual reference only - no correlation between cohesive and granular materials is implied.

PLANT: Explorer
METHOD: AD/T to 2.0m, NMLC to 10.20m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 2m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 100.7 AHD
COORDINATE: E:311907.6, N:6265619.2
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH210
PROJECT No: 233937.00
DATE: 18/07/25
SHEET: 2 of 3

GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA	GRAPHIC	WEATH.	DEPTH (m)	STRENGTH	RECOVERY (%)	RQD	FRACTURE SPACING (m)	DEFECTS & REMARKS	SAMPLE REMARKS	SAMPLE TYPE	SAMPLE INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
	100	0													0				
	99	1													1				
	98	2	Continued from soil log												2				
		2.55	SILTSTONE: grey-brown. Ashfield Shale.		MW	2.55	M				2.55-3.10m: FG, SN Fe								
	97	3			XW	3.25	SEAM	83	32	SEAM	3.25-3.55m: DS, Clay				3				
					HW	3.55	VL												
					MW	3.75	L									PLT	PL(A)=0.22MPa		
		4.00	SILTSTONE: grey-brown; 10-20% clay. Ashfield Shale.			4.00					3.80-4.20m: FG				4				
	96	5					VL				4.40-4.55m: FG, Clay				5				
						4.80	SEAM			SEAM	4.80-4.85m: CS								
						4.85	VL												
						5.15	SEAM			SEAM	5.15-5.20m: CS								
					XW - HW	5.20	VL												
	95	6				5.50	SEAM			SEAM	5.50-5.65m: DS				6				
						5.65	VL												
						5.85	SEAM			SEAM	5.85-5.95m: CS								
						5.95	VL												
						6.00	SEAM	100	42	SEAM	6.00-6.70m: DS								
	94	6.70	SILTSTONE: grey and brown. Ashfield Shale.			6.70					6.70-6.90m: FG, SN Fe				7	PLT	PL(A)=0.30MPa		
		7			MW		L - M				7.25m: JT, 45°, PR, RF, FG								
	93					7.65					7.50-7.70m: FG, Clay					PLT	PL(A)=0.80MPa		
											7.70m: JT, 85°, PR, SN Fe, RF					PLT	PL(A)=0.52MPa		
		8				7.85					7.80m: JT, 30°, PR, SN Fe, RF				8				
	92						M				7.85m: JT, 45°, PR, CN, SM					PLT	PL(A)=0.54MPa		
											8.10m: JT, 80°, UN, CN, RF								
											8.35m: JT, 35°, UN, Fe, RF								
											8.60m: JT, 45°, PR, CN, SM								
					FR						8.92m: JT, 85°, PR, CN, SM								
		9.00	SILTSTONE: grey; 1% fine grained sandstone laminations. Ashfield Shale.			9.00		100	100		9.10m: JT, 80°, UN, CN, RF				9	PLT	PL(A)=1.2MPa		
	91						H									PLT	PL(A)=1.2MPa		

NOTES: (R)Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 2.0m, NMLC to 10.20m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 2m

Generated with CORE-GS by Geroc - Split Soil-Rock Log

Refer to explanatory notes for symbol and abbreviation definitions

Douglas PARTNERS

BOREHOLE LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 100.7 AHD
COORDINATE: E:311907.6, N:6265619.2
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH210
PROJECT No: 233937.00
DATE: 18/07/25
SHEET: 3 of 3

CONDITIONS ENCOUNTERED														SAMPLE				TESTING			
GROUNDWATER	RL (m)	DEPTH (m)	DESCRIPTION OF STRATA (SOIL STRENGTH (where encountered) SOIL MOISTURE)	GRAPHIC	WEATH. (RS XW HW MW SW FR; DW)	DEPTH (m)	STRENGTH (VL L M H VH EH)	RECOVERY (%)	RQD	FRACTURE SPACING (m) (0.05 0.06 0.08 0.60 2.00)	DEFECTS & REMARKS	SAMPLE REMARKS	TYPE	INTERVAL	DEPTH (m)	TEST TYPE	RESULTS AND REMARKS	BACKFILL	WELL PIPE
	90	10	[CONT] SILTSTONE: grey; 1% fine grained sandstone laminations. Ashfield Shale.		FR		H	100	100		10.20m: JT, 45°, PR, CN, SM								
			Borehole discontinued at 10.20m depth. Target depth reached.																
	89	11																	
	88	12																	
	87	13																	
	86	14																	
	85	15																	
	84	16																	
	83	17																	
	82	18																	
	81	19																	

NOTES: [#]Soil origin is "probable" unless otherwise stated.

PLANT: Explorer
METHOD: AD/T to 2.0m, NMLC to 10.20m
REMARKS: Grass cover at surface

OPERATOR: Ground Test (S. Salib)

LOGGED: S. Islam
CASING: HW to 2m

Refer to explanatory notes for symbol and abbreviation definitions

Generated with CORE-GS by Geroc - Split Soil-Rock Log

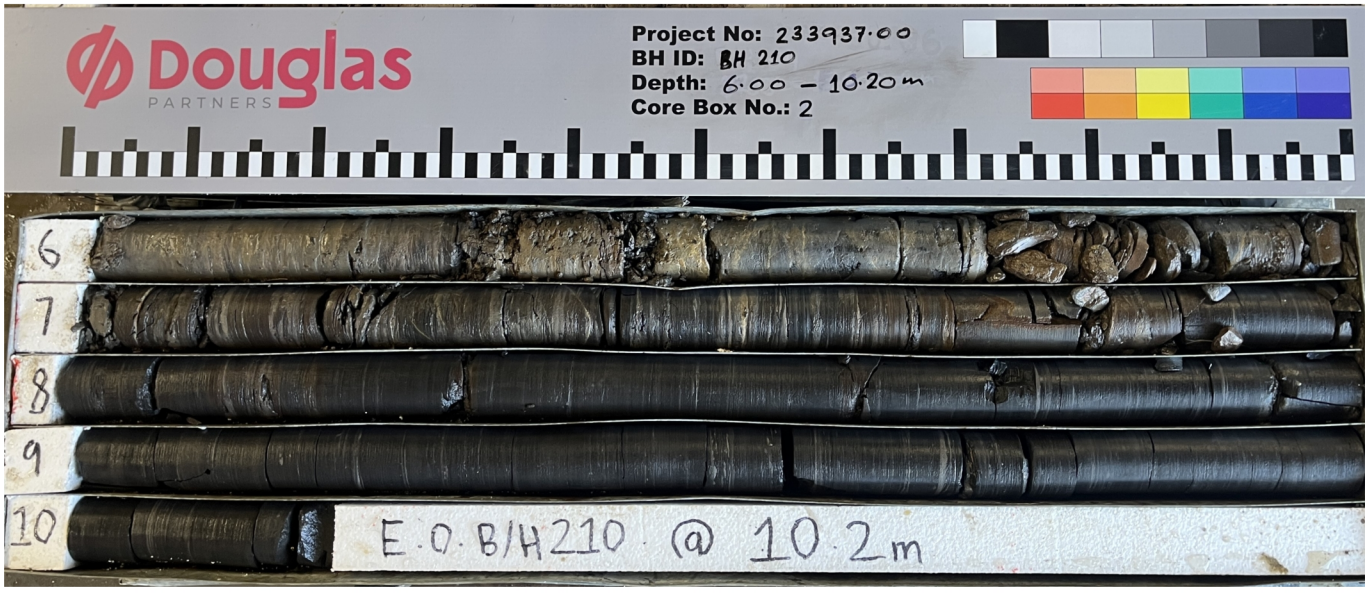
CORE PHOTO LOG

CLIENT: GreenSquareDC Pty Ltd
PROJECT: Green Square DC SYDGPU1
LOCATION: 3 Brookhollow Avenue, Baulkham Hills, NSW 2153

SURFACE LEVEL: 100.7 AHD
COORDINATE: E:311907.6, N:6265619.2
DATUM/GRID: MGA2020 Zone 56
DIP/AZIMUTH: 90°/---°

LOCATION ID: BH210
PROJECT No: 233937.00
DATE: 18/07/25
SHEET: 1 of 1

2.00-6.00 m depth



6.00-10.20 m depth

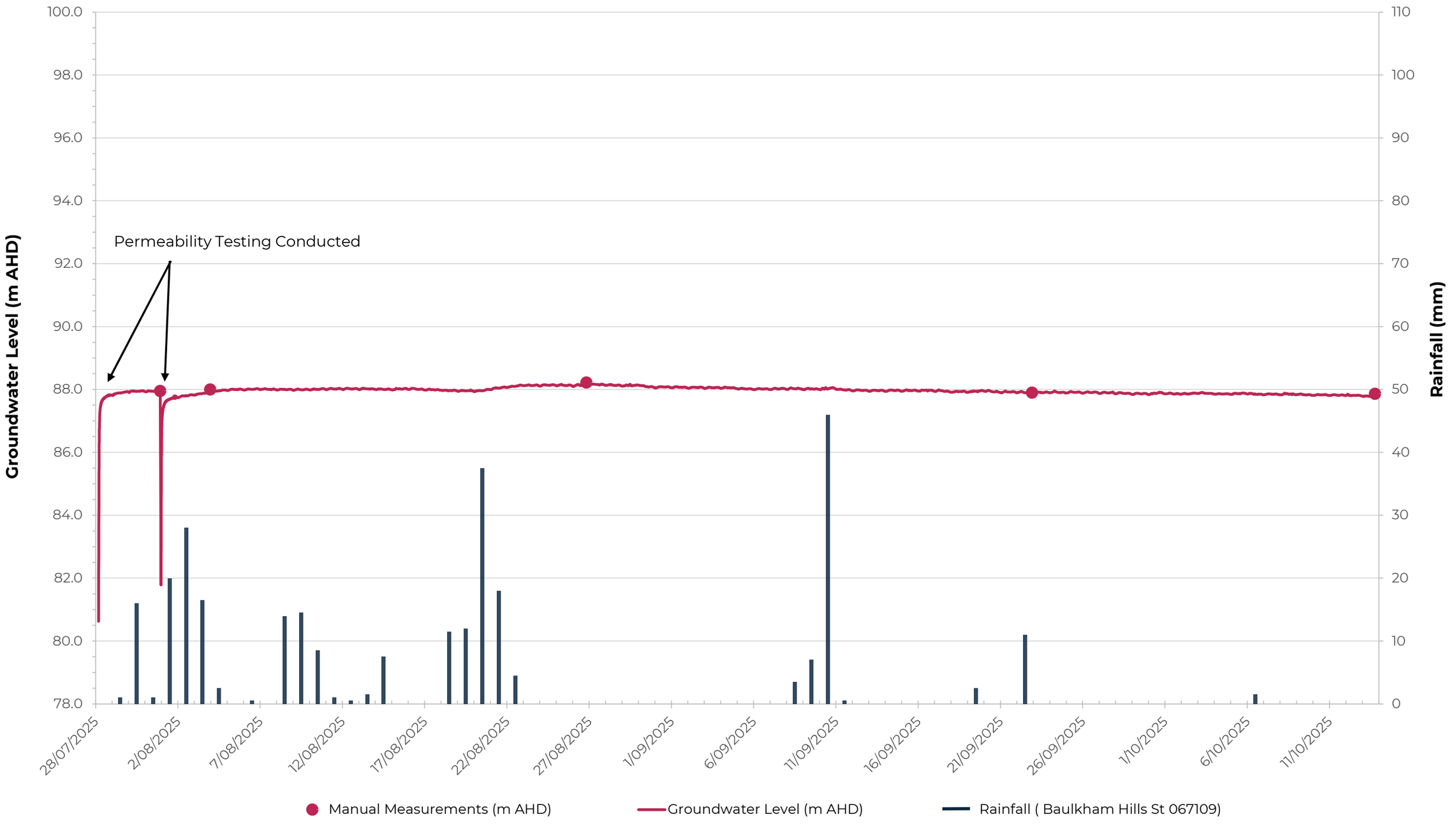
Generated with CORE-GS by Geroc - Split Soil-Rock Log

Douglas PARTNERS

Appendix D

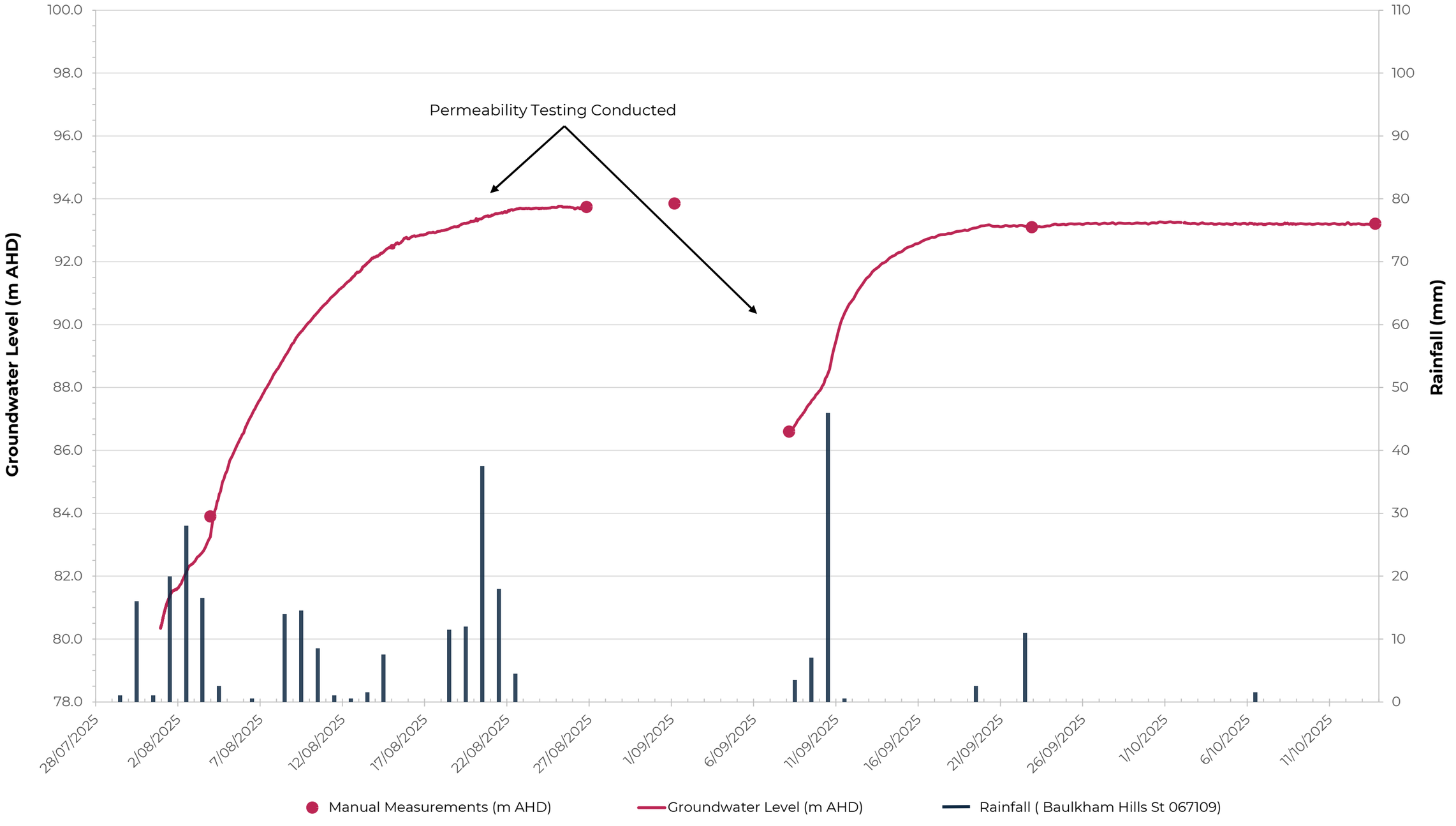
Hydrographs

BH202



Douglas PARTNERS	CLIENT: Project Garden 1 Pty Ltd ATF The Norwest Property Trust	**Groundwater Monitoring 28/07/2025 to 14/10/25** **Green Square DC SYDGPU1** **3 Brookhollow Avenue, Norwest, NSW 2153**	PROJECT NO: 233937.00
	OFFICE: Sydney / DRAWN BY: E. Miler		FIGURE NO: 1 of 2
	SURFACE LEVEL: 96.0 m AHD / DATE: 14 October 2025		REVISION: Rev1

BH205



Douglas PARTNERS	CLIENT: Project Garden 1 Pty Ltd ATF The Norwest Property Trust	**Groundwater Monitoring 28/07/2025 to 14/10/25** **Green Square DC SYDGPU1** **3 Brookhollow Avenue, Norwest, NSW 2153**	PROJECT NO: 233937.00
	OFFICE: Sydney / DRAWN BY: E. Miler		FIGURE NO: 2 of 2
	SURFACE LEVEL: 100.0 m AHD / DATE: 14 October 2025		REVISION: Rev1

Appendix E

Hydraulic Testing Results

Douglas Partners Pty Ltd
ABN 75 053 980 117
www.douglaspartners.com.au

Permeability Testing - Rising or Falling Head Test Report

Client:	Project Garden 1 Pty Ltd ATF The Norwest Property Trust	Project No:	233937.00
Project:	Green Square DC SYDGPU1	Test date:	28.07.2025
Location:	3 Brookhollow Avenue, Norwest, NSW	Tested by:	LDS

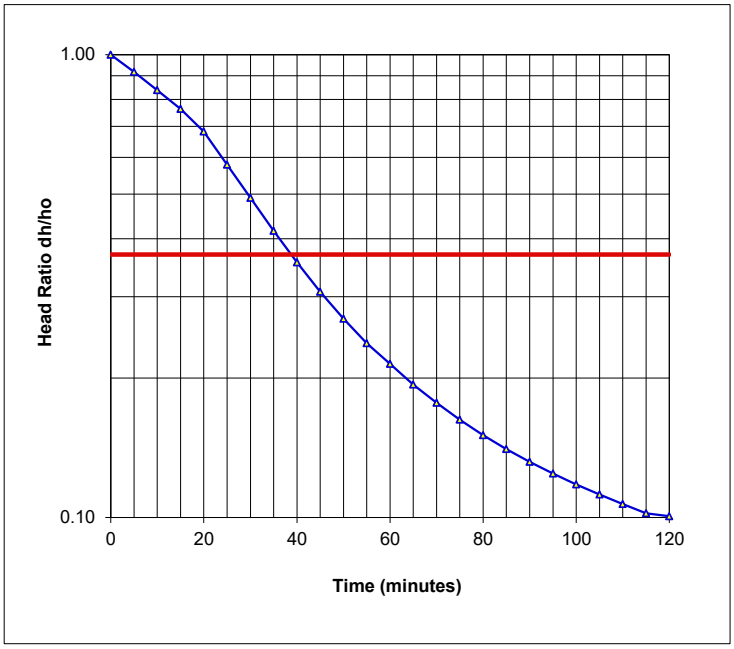
Test Location:	BH202	**Test No.**	1	
Description:	Standpipe in borehole	Easting:	311773.0	m
Material type:	Ashfield shale	Northing	6265685.0	m
		Surface Level:	96	m AHD

Details of Well Installation

Effective diameter (2re)	50	mm	Depth to water before test	8.05	m
borehole diameter (2R)	96	mm	Depth to water at start of test	14.34	m
Effective Length of well screen (Le)	4.30	m	Depth to top of gravel pack	10.80	m
			Depth of base of PVC standpipe	15.10	m

Test Results

Time (min)	Depth (m)	Change in Head δH (m)	δH/Ho
0.00	14.34	6.29	1.000
5.00	13.82	5.77	0.917
10.00	13.33	5.28	0.839
15.00	12.85	4.80	0.763
20.00	12.34	4.29	0.682
25.00	11.69	3.64	0.579
30.00	11.13	3.08	0.490
35.00	10.67	2.62	0.416
40.00	10.29	2.24	0.356
45.00	9.98	1.93	0.308
50.00	9.74	1.69	0.269
55.00	9.55	1.50	0.238
60.00	9.40	1.35	0.215
65.00	9.27	1.22	0.194
70.00	9.16	1.11	0.177
75.00	9.07	1.02	0.163
80.00	9.00	0.95	0.151
85.00	8.93	0.88	0.141
90.00	8.88	0.83	0.132
95.00	8.83	0.78	0.124
100.00	8.79	0.74	0.118
105.00	8.76	0.71	0.112
110.00	8.72	0.67	0.107
115.00	8.69	0.64	0.102
120.00	8.68	0.63	0.101



To = 39 mins
2340 secs

Theory: Falling Head Permeability calculated using equation by Hvorslev

$k = [r^2 \ln(Le/R)]/2Le\ To$

where r = radius of casing
R = radius of well screen
Le = length of well screen
To = time taken to rise or fall to 37% of initial change

Hydraulic Conductivity	**k =**	**1.4E-07**	m/sec
	=	**0.012**	m/day

Douglas Partners Pty Ltd
ABN 75 053 980 117
www.douglaspartners.com.au

Permeability Testing - Rising or Falling Head Test Report

Client:	Project Garden 1 Pty Ltd ATF The Norwest Property Trust	Project No:	233937.00
Project:	Green Square DC SYDGPU1	Test date:	1-Aug-25
Location:	3 Brookhollow Avenue, Norwest, NSW	Tested by:	LDS

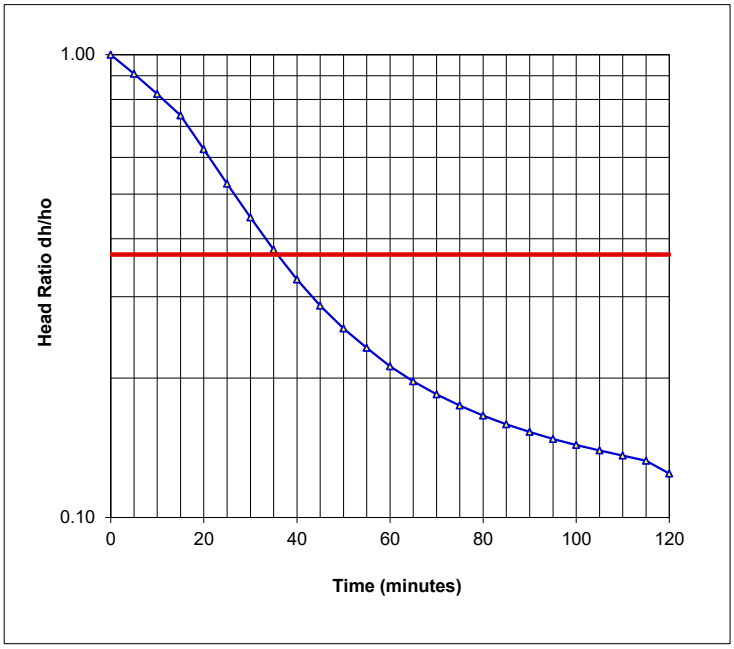
Test Location:	BH202	**Test No.**	2	
Description:	Standpipe in borehole	Easting:	311773.0	m
Material type:	Ashfield shale	Northing	6265685.0	m
		Surface Level:	96	m AHD

Details of Well Installation

Effective diameter (2re)	50	mm	Depth to water before test	8	m
borehole diameter (2R)	96	mm	Depth to water at start of test	14.15	m
Effective Length of well screen (Le)	4.30	m	Depth to top of gravel pack	10.80	m
			Depth of base of PVC standpipe	15.10	m

Test Results

Time (min)	Depth (m)	Change in Head δH (m)	δH/Ho
0.00	14.15	6.15	1.000
5.00	13.59	5.59	0.909
10.00	13.06	5.06	0.822
15.00	12.55	4.55	0.739
20.00	11.85	3.85	0.625
25.00	11.24	3.24	0.526
30.00	10.74	2.74	0.445
35.00	10.33	2.33	0.379
40.00	10.01	2.01	0.327
45.00	9.76	1.76	0.287
50.00	9.58	1.58	0.256
55.00	9.43	1.43	0.232
60.00	9.31	1.31	0.212
65.00	9.21	1.21	0.197
70.00	9.14	1.14	0.185
75.00	9.07	1.07	0.174
80.00	9.02	1.02	0.166
85.00	8.98	0.98	0.159
90.00	8.94	0.94	0.153
95.00	8.91	0.91	0.148
100.00	8.88	0.88	0.144
105.00	8.86	0.86	0.140
110.00	8.84	0.84	0.136
115.00	8.82	0.82	0.133
120.00	8.77	0.77	0.124



To = 36 mins
2160 secs

Theory: Falling Head Permeability calculated using equation by Hvorslev

$k = [r^2 \ln(Le/R)]/2Le\ To$

where r = radius of casing
R = radius of well screen
Le = length of well screen
To = time taken to rise or fall to 37% of initial change

Hydraulic Conductivity	**k =**	**1.5E-07**	m/sec
	=	**0.013**	m/day

Douglas Partners Pty Ltd
ABN 75 053 980 117
www.douglaspartners.com.au

Permeability Testing - Rising or Falling Head Test Report

Client:	Project Garden 1 Pty Ltd ATF The Norwest Property Trust	Project No:	233937.00
Project:	Green Square DC SYDGPU1	Test date:	28.07.2025
Location:	3 Brookhollow Avenue, Norwest, NSW	Tested by:	LDS

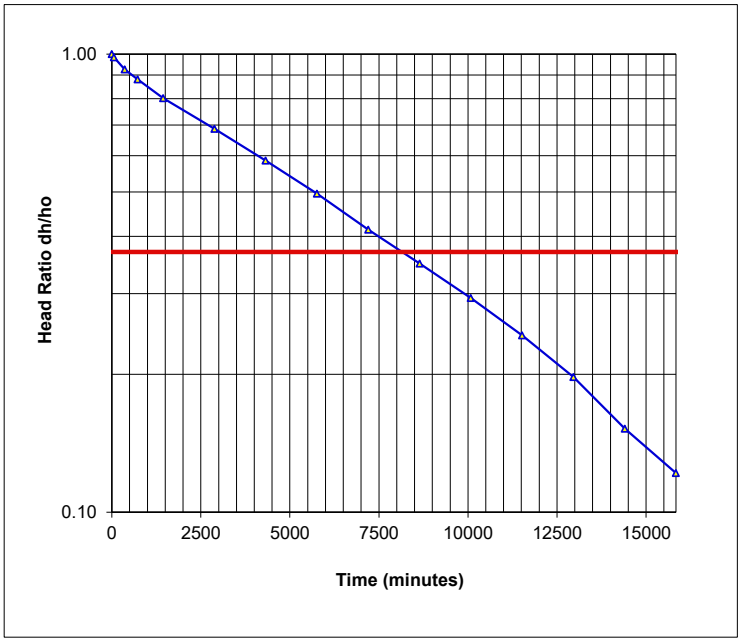
Test Location:	BH205	**Test No.**	1	
Description:	Standpipe in borehole	Easting:	311887.1	m
Material type:	Ashfield shale	Northing	6265641.0	m
		Surface Level:	100	m AHD

Details of Well Installation

Effective diameter (2re)	50	mm	Depth to water before test	6.26	m
borehole diameter (2R)	96	mm	Depth to water at start of test	16.74	m
Effective Length of well screen (Le)	3.90	m	Depth to top of gravel pack	16.30	m
			Depth of base of PVC standpipe	20.20	m

Test Results

Time (min)	Depth (m)	Change in Head δH (m)	δH/Ho
0	16.74	10.48	1.000
60	16.58	10.32	0.985
360	15.97	9.71	0.926
720	15.50	9.24	0.881
1440	14.66	8.40	0.802
2880	13.47	7.21	0.688
4320	12.40	6.14	0.586
5760	11.46	5.20	0.496
7200	10.60	4.34	0.414
8640	9.92	3.66	0.349
10080	9.34	3.08	0.294
11520	8.81	2.55	0.243
12960	8.33	2.07	0.197
14400	7.86	1.60	0.152
15840	7.54	1.28	0.122
17280	7.27	1.01	0.096
18720	7.14	0.88	0.084
20160	7.02	0.76	0.073
21600	6.88	0.62	0.059
23040	6.70	0.44	0.042
24480	6.55	0.29	0.028
25920	6.42	0.16	0.015
27360	6.31	0.05	0.005
28800	6.30	0.04	0.004
30240	6.27	0.01	0.001



To = 8200 mins
492000 secs

Theory: Falling Head Permeability calculated using equation by Hvorslev

$k = [r^2 \ln(Le/R)]/2Le\ To$

where r = radius of casing
R = radius of well screen
Le = length of well screen
To = time taken to rise or fall to 37% of initial change

Hydraulic Conductivity	**k =**	**7.2E-10**	m/sec
	=	**0.00006**	m/day

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Permeability Testing - Rising or Falling Head Test Report

Client:	Project Garden 1 Pty Ltd ATF The Norwest Property Trust	Project No:	233937.00
Project:	Green Square DC SYDGPU1	Test date:	8-Sep-25
Location:	3 Brookhollow Avenue, Norwest, NSW	Tested by:	EM

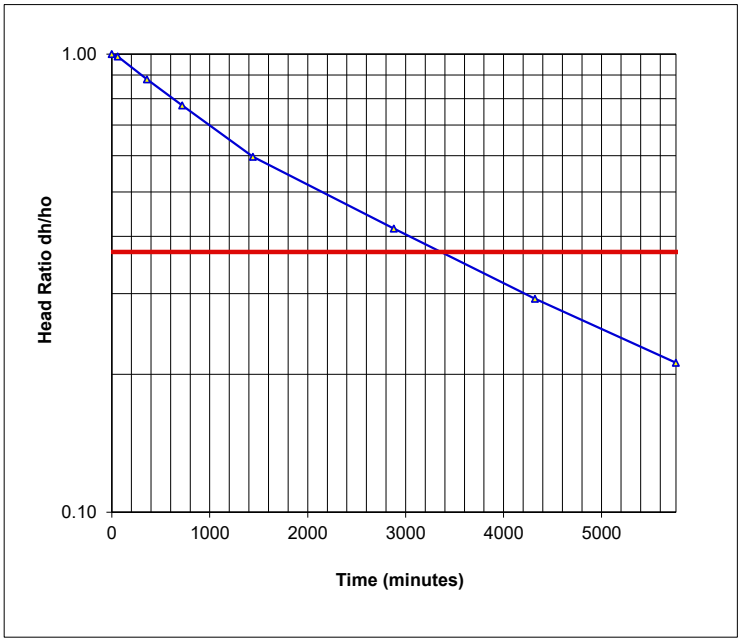
Test Location:	BH205	**Test No.**	2	
Description:	Standpipe in borehole	Easting:	311887.1	m
Material type:	Ashfield shale	Northing	6265641.0	m
		Surface Level:	100	m AHD

Details of Well Installation

Effective diameter (2re)	50	mm	Depth to water before test	6.80	m
borehole diameter (2R)	96	mm	Depth to water at start of test	11.52	m
Effective Length of well screen (Le)	3.90	m	Depth to top of gravel pack	16.30	m
			Depth of base of PVC standpipe	20.20	m

Test Results

Time (min)	Depth (m)	Change in Head δH (m)	δH/Ho
0	11.52	4.72	1.000
60	11.47	4.67	0.989
360	10.96	4.16	0.881
720	10.45	3.65	0.773
1440	9.62	2.82	0.597
2880	8.77	1.97	0.416
4320	8.18	1.38	0.293
5760	7.80	1.00	0.212
7200	7.26	0.46	0.097
8640	6.88	0.08	0.016
10080	6.82	0.02	0.005



To = 3380 mins
202800 secs

Theory: Falling Head Permeability calculated using equation by Hvorslev

$k = [r^2 \ln(Le/R)]/2Le\ To$

where r = radius of casing
R = radius of well screen
Le = length of well screen
To = time taken to rise or fall to 37% of initial change

Hydraulic Conductivity	**k =**	**1.7E-09**	m/sec
	=	**0.00015**	m/day

Appendix F

Groundwater Quality Summary Table

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Summary of Laboratory Results – Metals, TRH, BTEX, PAH, OCP, OPP, PCB, VOC (excluding BTEX), PFAS, Nutrients, Physical Parameters, Anions & Cations

Client:	Project Garden 1 Pty Ltd ATF The Norwest Property Trust	Project No:	233937.00
Project:	Green Square DC SYDGPU1	Test date:	14-Oct-25
Location:	3 Brookhollow Avenue, Norwest, NSW	Tested by:	SF

Sample ID	Analyte	PQL	Units	ANZG (2018) 95% LOP Fresh waters	HEPA (2020) 95% LOP Fresh waters	BH202	BH205
Sample Date						14/10/25	14/10/25
Metals	Total Arsenic	1	µg/L	13		<1	**3**
	Cadmium	0.1	µg/L	0.2		<0.1	<0.1
	Total Chromium	1	µg/L	1		<1	**1**
	Copper	1	µg/L	1.4		**5**	**78**
	Lead	1	µg/L	3.4		<1	<1
	Mercury (inorganic)	0.05	µg/L	0.06		<0.05	<0.05
	Nickel	1	µg/L	11		**4**	**140**
	Zinc	1	µg/L	8		**52**	**110**
Metals - Dissolved	Total Arsenic	1	µg/L	13		<1	**2**
	Cadmium	0.1	µg/L	0.2		<0.1	**0.1**
	Total Chromium	1	µg/L	1		<1	<1
	Copper	1	µg/L	1.4		<1	**100**
	Lead	1	µg/L	3.4		<1	<1
	Mercury (inorganic)	0.05	µg/L	0.06		<0.05	<0.05
	Nickel	1	µg/L	11		**6**	**120**
	Zinc	1	µg/L	8		**70**	**170**
TRH	F1 ((C6-C10)-BTEX)	10	µg/L			<10	<10
	F2 (>C10-C16 less Naphthalene)	50	µg/L			<50	<50
	F3 (>C16-C34)	100	µg/L			<100	<100
	F4 (>C34-C40)	100	µg/L			<100	<100
BTEX	Benzene	1	µg/L	950		<1	<1
	Toluene	1	µg/L	180		<1	<1
	Ethylbenzene	1	µg/L	80		<1	<1
	o-Xylene	1	µg/L	350		<1	<1
	m+p-Xylene	2	µg/L	75		<2	<2
	Total Xylenes	1	µg/L			<1	<1
PAH	Acenaphthene	0.01	µg/L			<0.01	<0.01
	Acenaphthylene	0.01	µg/L			<0.01	<0.01
	Anthracene	0.01	µg/L	0.01		<0.01	<0.01
	Benzo(a)anthracene	0.01	µg/L			<0.01	<0.01
	Naphthalene	1	µg/L	16		<1	<1
	Benzo(a)pyrene (B(a)P)	0.01	µg/L	0.1		<0.01	<0.01
	Benzo(b,j+k)fluoranthene	0.02	µg/L			<0.02	<0.02
	Benzo(g,h,i)perylene	0.01	µg/L			<0.01	<0.01
	Chrysene	0.01	µg/L			<0.01	<0.01
	Dibenzo(a,h)anthracene	0.01	µg/L			<0.01	<0.01
	Fluoranthene	0.01	µg/L	1		**0.02**	**0.02**
	Fluorene	0.01	µg/L			<0.01	<0.01
	Indeno(1,2,3-c,d)pyrene	0.01	µg/L			<0.01	<0.01
	Phenanthrene	0.01	µg/L	0.6		**0.02**	**0.04**

	Pyrene	0.01	µg/L			**0.02**	**0.02**
	Sum of detected PAH	0	µg/L			**0.061**	**0.081**
OCP	DDE	0.001	µg/L			<0.001	<0.001
	DDT	0.001	µg/L	0.006		<0.001	<0.001
	DDD	0.001	µg/L			<0.001	<0.001
	Aldrin	0.001	µg/L	0.001		<0.001	<0.001
	Dieldrin	0.001	µg/L	0.01		<0.001	<0.001
	Aldrin + Dieldrin (calculated)	0.001	µg/L			<0.001	<0.001
	alpha-chlordane	0.001	µg/L			<0.001	<0.001
	gamma-Chlordane	0.001	µg/L			<0.001	<0.001
	Endosulfan I	0.002	µg/L			<0.002	<0.002
	Endosulfan II	0.002	µg/L			<0.002	<0.002
	Endosulfan Sulphate	0.001	µg/L			<0.001	<0.001
	Endrin	0.001	µg/L	0.01		<0.001	<0.001
	Endrin Aldehyde	0.001	µg/L			<0.001	<0.001
	Heptachlor	0.001	µg/L	0.01		<0.001	<0.001
	Heptachlor Epoxide	0.001	µg/L			<0.001	<0.001
	Hexachlorobenzene	0.001	µg/L	0.1		<0.001	<0.001
	Methoxychlor	0.001	µg/L	0.005		<0.001	<0.001
	alpha-BHC	0.001	µg/L			<0.001	<0.001
	beta-BHC	0.001	µg/L			<0.001	<0.001
	delta-BHC	0.001	µg/L			<0.001	<0.001
	Lindane	0.001	µg/L	0.2		<0.001	<0.001
	Sum of detected OCP	0.001	µg/L			<0.001	<0.001
OPP	Azinphos methyl (Guthion)	0.02	µg/L	0.02		<0.02	<0.02
	Bromophos-ethyl	0.05	µg/L			<0.05	<0.05
	Chlorpyriphos	0.009	µg/L	0.00004		<0.009	<0.009
	Chlorpyriphos-methyl	0.05	µg/L			<0.05	<0.05
	Diazinon	0.01	µg/L	0.01		<0.01	<0.01
	Dichlorvos	0.05	µg/L			<0.05	<0.05
	Dimethoate	0.1	µg/L	0.15		<0.1	<0.1
	Ethion	0.05	µg/L			<0.05	<0.05
	Ronnel (fenchlorphos)	0.05	µg/L			<0.05	<0.05
	Fenitrothion	0.05	µg/L	0.2		<0.05	<0.05
	Fenthion	0.05	µg/L			<0.05	<0.05
	Malathion	0.05	µg/L	0.05		<0.05	<0.05
	Parathion	0.004	µg/L	0.004		<0.004	<0.004
	Parathion-methyl	0.05	µg/L			<0.05	<0.05
	Methidathion	0.05	µg/L			<0.05	<0.05
	Fenamiphos	0.05	µg/L			<0.05	<0.05
	Sum of detected OPP	0.004	µg/L			<0.004	<0.004
PCB	Arochlor 1016	0.01	µg/L			<0.01	<0.01
	Arochlor 1221	0.01	µg/L			<0.01	<0.01
	Arochlor 1232	0.01	µg/L			<0.01	<0.01
	Arochlor 1242	0.01	µg/L	0.3		<0.01	<0.01
	Arochlor 1248	0.01	µg/L			<0.01	<0.01
	Arochlor 1254	0.01	µg/L	0.01		<0.01	<0.01
	Aroclor 1260	0.01	µg/L			<0.01	<0.01
	Sum of detected PCB	0.01	µg/L			<0.01	<0.01
	1,1,1,2-tetrachloroethane	1	µg/L			<1	<1
	1,1,1-trichloroethane	1	µg/L	270		<1	<1
	1,1,2,2-tetrachloroethane	1	µg/L	400		<1	<1
	tetrachloroethene	1	µg/L	70		<1	<1
	1,1,2-trichloroethane	1	µg/L	6,500		<1	<1

VOC (excluding BTEX)	1,1,2-trichloroethylene	1	µg/L	330		<1	<1
	1,1-dichloroethane	1	µg/L			<1	<1
	1,1-Dichloroethene	1	µg/L	700		<1	<1
	1,1-dichloropropene	1	µg/L			<1	<1
	1,2,3-trichlorobenzene	1	µg/L	3		<1	<1
	1,2,3-trichloropropane	1	µg/L			<1	<1
	1,2,4-trichlorobenzene	1	µg/L	85		<1	<1
	1,2,4-trimethyl benzene	1	µg/L			<1	<1
	1,2-dibromo-3-chloropropane	1	µg/L			<1	<1
	1,2-dichlorobenzene	1	µg/L	160		<1	<1
	1,2-dichloroethane	1	µg/L	1,900		<1	<1
	1,2-dichloropropane	1	µg/L	900		<1	<1
	1,3,5-trimethyl benzene	1	µg/L			<1	<1
	1,3-dichlorobenzene	1	µg/L	260		<1	<1
	1,3-dichloropropane	1	µg/L	1,100		<1	<1
	1,4-dichlorobenzene	1	µg/L	60		<1	<1
	2,2-dichloropropane	1	µg/L			<1	<1
	2-chlorotoluene	1	µg/L			<1	<1
	4-chlorotoluene	1	µg/L			<1	<1
	4-isopropyl toluene	1	µg/L			<1	<1
	Bromobenzene	1	µg/L			<1	<1
	Bromochloromethane	1	µg/L			<1	<1
	Bromodichloromethane	1	µg/L			<1	<1
	Bromoform	1	µg/L			<1	<1
	carbon tetrachloride	1	µg/L	240		<1	<1
	Chloroethane	10	µg/L			<10	<10
	Vinyl Chloride	10	µg/L	100		<10	<10
	Chloroform	1	µg/L	770		<1	**5**
	Chloromethane	10	µg/L			<10	<10
	cis-1,2-dichloroethene	1	µg/L			<1	<1
	cis-1,3-dichloropropene	1	µg/L			<1	<1
	Isopropylbenzene (cumene)	1	µg/L	30		<1	<1
	Cyclohexane	1	µg/L			<1	<1
	dibromochloromethane	1	µg/L			<1	<1
	Dibromomethane	1	µg/L			<1	<1
	Dichlorodifluoromethane	10	µg/L			<10	<10
	1,2-dibromoethane	1	µg/L			<1	<1
	hexachlorobutadiene	1	µg/L			<1	<1
	Bromomethane	10	µg/L			<10	<10
	Monochlorobenzene	1	µg/L	55		<1	<1
	n-butyl benzene	1	µg/L			<1	<1
	n-propyl benzene	1	µg/L			<1	<1
	sec-butyl benzene	1	µg/L			<1	<1
	Styrene (vinylbenzene)	1	µg/L			<1	<1
	Tert-butyl benzene	1	µg/L			<1	<1
	trans-1,2-dichloroethene	1	µg/L			<1	<1
	trans-1,3-dichloropropene	1	µg/L			<1	<1
	Trichlorofluoromethane	10	µg/L			<10	<10
	Sum of detected VOC	1	µg/L			<1	**5**
PFAS	PFOS	0.0002	µg/L		0.13	**0.0004**	**0.0002**
	PFOA	0.0002	µg/L		220	<0.0002	**0.0004**
	PFHxS	0.0002	µg/L			<0.0002	<0.0002
	Perfluorobutanesulfonic acid (PFBS)	0.0004	µg/L			<0.0004	<0.0004
	6:2 FTS	0.0004	µg/L			**0.018**	**0.0074**

	8:2 FTS	0.0004	µg/L			<0.0004	<0.0004
	Sum of detected PFAS	0.0002	µg/L			**0.018**	**0.007**
Nutrients	Nitrogen	100	µg/L			**800**	**2,300**
	TKN in water	100	µg/L			**800**	**1,800**
	NOx as N	5	µg/L			<5	**540**
	Organic Nitrogen as N	200	µg/L			<200	**400**
	Phosphorous	50	µg/L			<50	<50
Nutrients - Dissolved	Ammonia as N	5	µg/L	900		**660**	**1,400**
	Nitrate as N	5	µg/L	1,100		<5	**30**
	Nitrite as N	5	µg/L			<5	**510**
Physical Parameters	Total dissolved solids	5,000	µg/L			**2,600,000**	**4,400,000**
Anions & Cations	Bicarbonate Alkalinity as CaCO3	5,000	µg/L			**890,000**	**800,000**
	Chloride	1000	µg/L			**510,000**	**2,100,000**
	Hydroxide Alkalinity (OH-) as CaCO3	5,000	µg/L			<5000	<5000
	Ionic Balance	0	mg/kg			**40,000**	**-210000**
	Sulphate	1000	µg/L			**120,000**	**220,000**
	Total Alkalinity as CaCO3	5,000	µg/L			**890,000**	**800,000**
Anions & Cations - Dissolved	Calcium	500	µg/L			**41,000**	**130,000**
	Magnesium	500	µg/L			**67,000**	**130,000**
	Potassium	500	µg/L			**24,000**	**27,000**
	Sodium	500	µg/L			**670,000**	**800,000**

Notes:

-	No criterion / not defined / not tested / not applicable
*	QA/QC replicate of sample listed directly below the primary sample
NL	Not limiting
PQL	Practical quantitation limit
	Shaded cell is exceedance of guideline value
	Where one or more guideline value is exceeded, the cell is shaded to the colour of the highest guideline value exceeded
	ANZG (2018) Australian and New Zealand Guidelines for Fresh and Marine Water Quality, 95% level of protection of species for Fresh aquatic ecosystems [NB: 99% level of protection adopted for bioaccumulative chemicals]
	HEPA (2025) PFAS National Environmental Management Plan, Version 3.0 99% level of protection for Fresh water aquatic ecosystems
	ANZG (2018) Australian and New Zealand Guidelines for Fresh and Marine Water Quality, orange text is 'unknown' level of protection
	Underlining of ANZG (2018) criteria indicates a criteria with an 'unknown' level of protection.
	ANZG (2018) DGV adopted for most conservative species of following analytes: DGV for xylene (m) adopted for xylene (m+p); DGV for CrVI adopted for total chromium; DGV for AsV adopted for total arsenic
	ANZG (2018) DGV adopted for aluminium in freshwater is for receiving waters with pH >6.5. For receiving waters with pH <6.5 suitability of the more conservative, low reliability DGV of unknown LOP should be considered
	ANZG (2018) Ammonia DGV is pH and temperature dependant. DGV for a pH of 8 provided in table.

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CERTIFICATE OF ANALYSIS 392905

Client Details

Client	Douglas Partners Pty Ltd
Attention	Adam Teoh
Address	96 Hermitage Rd, West Ryde, NSW, 2114

Sample Details

Your Reference	**233937.00, Baulkham Hills**
Number of Samples	2 Water
Date samples received	14/10/2025
Date completed instructions received	14/10/2025

Analysis Details

Please refer to the following pages for results, methodology summary and quality control data.

Samples were analysed as received from the client unless as indicated below in the method summaries. Results relate specifically to the samples as received.

Results are reported on a dry weight basis for solids and on an as received basis for other matrices.

Please refer to the last page of this report for any comments relating to the results.

Report Details

Date results requested by	21/10/2025
Date of Issue	21/10/2025

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Accredited for compliance with ISO/IEC 17025 - Testing. **Tests not covered by NATA are denoted with ***

Results Approved By
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Nancy Zhang, Laboratory Manager, Sydney
Nick Sarlamis, Assistant Operation Manager
Sean McAlary, Senior Chemist
Tabitha Roberts, Senior Chemist
Timothy Toll, Senior Chemist

Authorised By
Nancy Zhang, Laboratory Manager

VOCs in water			
Our Reference		392905-1	392905-2
Your Reference	UNITS	BH202	BH205
Date Sampled		14/10/2025	14/10/2025
Type of sample		Water	Water
Date Extracted	-	16/10/2025	16/10/2025
Date Analysed	-	17/10/2025	17/10/2025
Dichlorodifluoromethane	µg/L	<10	<10
Chloromethane	µg/L	<10	<10
Vinyl Chloride	µg/L	<10	<10
Bromomethane	µg/L	<10	<10
Chloroethane	µg/L	<10	<10
Trichlorofluoromethane	µg/L	<10	<10
1,1-Dichloroethene	µg/L	<1	<1
Trans-1,2-dichloroethene	µg/L	<1	<1
1,1-dichloroethane	µg/L	<1	<1
Cis-1,2-dichloroethene	µg/L	<1	<1
Bromochloromethane	µg/L	<1	<1
Chloroform	µg/L	<1	5
2,2-dichloropropane	µg/L	<1	<1
1,2-dichloroethane	µg/L	<1	<1
1,1,1-trichloroethane	µg/L	<1	<1
1,1-dichloropropene	µg/L	<1	<1
Cyclohexane	µg/L	<1	<1
Carbon tetrachloride	µg/L	<1	<1
Benzene	µg/L	<1	<1
Dibromomethane	µg/L	<1	<1
1,2-dichloropropane	µg/L	<1	<1
Trichloroethene	µg/L	<1	<1
Bromodichloromethane	µg/L	<1	<1
trans-1,3-dichloropropene	µg/L	<1	<1
cis-1,3-dichloropropene	µg/L	<1	<1
1,1,2-trichloroethane	µg/L	<1	<1
Toluene	µg/L	<1	<1
1,3-dichloropropane	µg/L	<1	<1
Dibromochloromethane	µg/L	<1	<1
1,2-dibromoethane	µg/L	<1	<1
Tetrachloroethene	µg/L	<1	<1
1,1,1,2-tetrachloroethane	µg/L	<1	<1
Chlorobenzene	µg/L	<1	<1
Ethylbenzene	µg/L	<1	<1

VOCs in water			
Our Reference		392905-1	392905-2
Your Reference	UNITS	BH202	BH205
Date Sampled		14/10/2025	14/10/2025
Type of sample		Water	Water
Bromoform	µg/L	<1	<1
m+p-xylene	µg/L	<2	<2
Styrene	µg/L	<1	<1
1,1,2,2-tetrachloroethane	µg/L	<1	<1
o-xylene	µg/L	<1	<1
1,2,3-trichloropropane	µg/L	<1	<1
Isopropylbenzene	µg/L	<1	<1
Bromobenzene	µg/L	<1	<1
n-propyl benzene	µg/L	<1	<1
2-chlorotoluene	µg/L	<1	<1
4-chlorotoluene	µg/L	<1	<1
1,3,5-trimethyl benzene	µg/L	<1	<1
Tert-butyl benzene	µg/L	<1	<1
1,2,4-trimethyl benzene	µg/L	<1	<1
1,3-dichlorobenzene	µg/L	<1	<1
Sec-butyl benzene	µg/L	<1	<1
1,4-dichlorobenzene	µg/L	<1	<1
4-isopropyl toluene	µg/L	<1	<1
1,2-dichlorobenzene	µg/L	<1	<1
n-butyl benzene	µg/L	<1	<1
1,2-dibromo-3-chloropropane	µg/L	<1	<1
1,2,4-trichlorobenzene	µg/L	<1	<1
Hexachlorobutadiene	µg/L	<1	<1
1,2,3-trichlorobenzene	µg/L	<1	<1
Surrogate Dibromofluoromethane	%	104	102
Surrogate Toluene-d8	%	104	102
Surrogate 4-Bromofluorobenzene	%	102	100

vTRH(C6-C10)/BTEXN in Water			
Our Reference	UNITS	392905-1	392905-2
Your Reference		BH202	BH205
Date Sampled		14/10/2025	14/10/2025
Type of sample		Water	Water
Date extracted	-	16/10/2025	16/10/2025
Date analysed	-	17/10/2025	17/10/2025
TRH C_6 - C_9	µg/L	<10	<10
TRH C_6 - C_{10}	µg/L	<10	<10
TRH C_6 - C_{10} less BTEX (F1)	µg/L	<10	<10
Benzene	µg/L	<1	<1
Toluene	µg/L	<1	<1
Ethylbenzene	µg/L	<1	<1
m+p-xylene	µg/L	<2	<2
o-xylene	µg/L	<1	<1
Naphthalene	µg/L	<1	<1
Surrogate Dibromofluoromethane	%	104	102
Surrogate Toluene-d8	%	104	102
Surrogate 4-Bromofluorobenzene	%	102	100

svTRH (C10-C40) in Water			
Our Reference	UNITS	392905-1	392905-2
Your Reference		BH202	BH205
Date Sampled		14/10/2025	14/10/2025
Type of sample		Water	Water
Date extracted	-	15/10/2025	15/10/2025
Date analysed	-	15/10/2025	15/10/2025
TRH C_{10} - C_{14}	µg/L	<50	<50
TRH C_{15} - C_{28}	µg/L	<100	<100
TRH C_{29} - C_{36}	µg/L	<100	<100
Total +ve TRH (C10-C36)	µg/L	<50	<50
TRH >C_{10} - C_{16}	µg/L	<50	<50
TRH >C_{10} - C_{16} less Naphthalene (F2)	µg/L	<50	<50
TRH >C_{16} - C_{34}	µg/L	<100	<100
TRH >C_{34} - C_{40}	µg/L	<100	<100
Total +ve TRH (>C10-C40)	µg/L	<50	<50
Surrogate o-Terphenyl	%	74	78

PAHs in Water - Trace Level			
Our Reference		392905-1	392905-2
Your Reference	UNITS	BH202	BH205
Date Sampled		14/10/2025	14/10/2025
Type of sample		Water	Water
Date extracted	-	15/10/2025	15/10/2025
Date analysed	-	17/10/2025	17/10/2025
Naphthalene	µg/L	<0.02	<0.02
Acenaphthylene	µg/L	<0.01	<0.01
Acenaphthene	µg/L	<0.01	<0.01
Fluorene	µg/L	<0.01	<0.01
Phenanthrene	µg/L	0.02	0.04
Anthracene	µg/L	<0.01	<0.01
Fluoranthene	µg/L	0.02	0.02
Pyrene	µg/L	0.02	0.02
Benzo(a)anthracene	µg/L	<0.01	<0.01
Chrysene	µg/L	<0.01	<0.01
Benzo(b,j+k)fluoranthene	µg/L	<0.02	<0.02
Benzo(a)pyrene	µg/L	<0.01	<0.01
Dibenzo(a,h)anthracene	µg/L	<0.01	<0.01
Indeno(1,2,3-c,d)pyrene	µg/L	<0.01	<0.01
Benzo(g,h,i)perylene	µg/L	<0.01	<0.01
Benzo(a)pyrene TEQ	µg/L	<0.05	<0.05
Total +ve PAH's	µg/L	0.061	0.081
Surrogate p-Terphenyl-d14	%	94	95

OCPs in Water - Trace Level			
Our Reference	UNITS	392905-1	392905-2
Your Reference		BH202	BH205
Date Sampled		14/10/2025	14/10/2025
Type of sample		Water	Water
Date extracted	-	15/10/2025	15/10/2025
Date analysed	-	17/10/2025	17/10/2025
alpha-BHC	µg/L	<0.001	<0.001
HCB	µg/L	<0.001	<0.001
beta-BHC	µg/L	<0.001	<0.001
gamma-BHC	µg/L	<0.001	<0.001
Heptachlor	µg/L	<0.001	<0.001
delta-BHC	µg/L	<0.001	<0.001
Aldrin	µg/L	<0.001	<0.001
Heptachlor Epoxide	µg/L	<0.001	<0.001
gamma-Chlordane	µg/L	<0.001	<0.001
alpha-Chlordane	µg/L	<0.001	<0.001
Endosulfan I	µg/L	<0.002	<0.002
pp-DDE	µg/L	<0.001	<0.001
Dieldrin	µg/L	<0.001	<0.001
Endrin	µg/L	<0.001	<0.001
Endosulfan II	µg/L	<0.002	<0.002
pp-DDD	µg/L	<0.001	<0.001
Endrin Aldehyde	µg/L	<0.001	<0.001
pp-DDT	µg/L	<0.001	<0.001
Endosulfan Sulphate	µg/L	<0.001	<0.001
Methoxychlor	µg/L	<0.001	<0.001
Surrogate 4-Chloro-3-NBTF	%	86	92

OP in water LL ANZECCF/ADWG			
Our Reference		392905-1	392905-2
Your Reference	UNITS	BH202	BH205
Date Sampled		14/10/2025	14/10/2025
Type of sample		Water	Water
Date extracted	-	15/10/2025	15/10/2025
Date analysed	-	17/10/2025	17/10/2025
Dichlorvos	µg/L	<0.05	<0.05
Mevinphos	µg/L	<0.05	<0.05
Phorate	µg/L	<0.05	<0.05
Dimethoate	µg/L	<0.1	<0.1
Diazinon	µg/L	<0.01	<0.01
Disulfoton	µg/L	<0.05	<0.05
Chlorpyriphos-methyl	µg/L	<0.05	<0.05
Parathion-Methyl	µg/L	<0.05	<0.05
Ronnel	µg/L	<0.05	<0.05
Fenitrothion	µg/L	<0.05	<0.05
Malathion	µg/L	<0.05	<0.05
Chlorpyriphos	µg/L	<0.009	<0.009
Fenthion	µg/L	<0.05	<0.05
Parathion	µg/L	<0.004	<0.004
Bromophos ethyl	µg/L	<0.05	<0.05
Methidathion	µg/L	<0.05	<0.05
Fenamiphos	µg/L	<0.05	<0.05
Ethion	µg/L	<0.05	<0.05
Phosalone	µg/L	<0.05	<0.05
Azinphos-methyl (Guthion)	µg/L	<0.02	<0.02
Coumaphos	µg/L	<0.05	<0.05
Surrogate 4-Chloro-3-NBTF	%	86	92

PCBs in Water - Trace Level			
Our Reference		392905-1	392905-2
Your Reference	UNITS	BH202	BH205
Date Sampled		14/10/2025	14/10/2025
Type of sample		Water	Water
Date extracted	-	15/10/2025	15/10/2025
Date analysed	-	17/10/2025	17/10/2025
Aroclor 1016	µg/L	<0.01	<0.01
Aroclor 1221	µg/L	<0.01	<0.01
Aroclor 1232	µg/L	<0.01	<0.01
Aroclor 1242	µg/L	<0.01	<0.01
Aroclor 1248	µg/L	<0.01	<0.01
Aroclor 1254	µg/L	<0.01	<0.01
Aroclor 1260	µg/L	<0.01	<0.01
Surrogate 2-Fluorobiphenyl	%	79	81

HM in water - dissolved			
Our Reference	UNITS	392905-1	392905-2
Your Reference		BH202	BH205
Date Sampled		14/10/2025	14/10/2025
Type of sample		Water	Water
Date prepared	-	15/10/2025	15/10/2025
Date analysed	-	15/10/2025	15/10/2025
Arsenic-Dissolved	µg/L	<1	2
Cadmium-Dissolved	µg/L	<0.1	0.1
Chromium-Dissolved	µg/L	<1	<1
Copper-Dissolved	µg/L	<1	100
Lead-Dissolved	µg/L	<1	<1
Mercury-Dissolved	µg/L	<0.05	<0.05
Nickel-Dissolved	µg/L	6	120
Zinc-Dissolved	µg/L	70	170

HM in water - total			
Our Reference	UNITS	392905-1	392905-2
Your Reference		BH202	BH205
Date Sampled		14/10/2025	14/10/2025
Type of sample		Water	Water
Date prepared	-	15/10/2025	15/10/2025
Date analysed	-	15/10/2025	15/10/2025
Arsenic-Total	µg/L	<1	3
Cadmium-Total	µg/L	<0.1	<0.1
Chromium-Total	µg/L	<1	1
Copper-Total	µg/L	5	78
Lead-Total	µg/L	<1	<1
Mercury-Total	µg/L	<0.05	<0.05
Nickel-Total	µg/L	4	140
Zinc-Total	µg/L	52	110

Metals in Waters - Acid extractable			
Our Reference		392905-1	392905-2
Your Reference	UNITS	BH202	BH205
Date Sampled		14/10/2025	14/10/2025
Type of sample		Water	Water
Date prepared	-	15/10/2025	15/10/2025
Date analysed	-	15/10/2025	15/10/2025
Phosphorus - Total	mg/L	<0.05	<0.05

Total Phenolics in Water			
Our Reference		392905-1	392905-2
Your Reference	UNITS	BH202	BH205
Date Sampled		14/10/2025	14/10/2025
Type of sample		Water	Water
Date extracted	-	15/10/2025	15/10/2025
Date analysed	-	15/10/2025	15/10/2025
Total Phenolics (as Phenol)	mg/L	<0.05	<0.05

Miscellaneous Inorganics			
Our Reference		392905-1	392905-2
Your Reference	UNITS	BH202	BH205
Date Sampled		14/10/2025	14/10/2025
Type of sample		Water	Water
Date prepared	-	15/10/2025	15/10/2025
Date analysed	-	15/10/2025	15/10/2025
Total Dissolved Solids (grav)	mg/L	2,600	4,400
Total Suspended Solids	mg/L	9	9
Ammonia as N in water	mg/L	0.66	1.4
Nitrate as N in water	mg/L	<0.005	0.03
Nitrite as N in water	mg/L	<0.005	0.51
NOx as N in water	mg/L	<0.005	0.54
Total Nitrogen in water	mg/L	0.8	2.3
TKN in water	mg/L	0.8	1.8
Phosphate as P in water	mg/L	0.01	0.02
Organic Nitrogen as N	mg/L	<0.2	0.4

Ion Balance			
Our Reference	UNITS	392905-1	392905-2
Your Reference		BH202	BH205
Date Sampled		14/10/2025	14/10/2025
Type of sample		Water	Water
Date prepared	-	15/10/2025	15/10/2025
Date analysed	-	15/10/2025	15/10/2025
Calcium - Dissolved	mg/L	41	130
Potassium - Dissolved	mg/L	24	27
Sodium - Dissolved	mg/L	670	800
Magnesium - Dissolved	mg/L	67	130
Hardness (calc) equivalent $CaCO_3$	mg/L	380	880
Hydroxide Alkalinity (OH^-) as $CaCO_3$	mg/L	<5	<5
Bicarbonate Alkalinity as $CaCO_3$	mg/L	890	800
Carbonate Alkalinity as $CaCO_3$	mg/L	<5	<5
Total Alkalinity as $CaCO_3$	mg/L	890	800
Sulphate, SO4	mg/L	120	220
Chloride, Cl	mg/L	510	2,100
Ionic Balance	%	4.0	-21

PFAS in Water TRACE Short			
Our Reference		392905-1	392905-2
Your Reference	UNITS	BH202	BH205
Date Sampled		14/10/2025	14/10/2025
Type of sample		Water	Water
Date prepared	-	16/10/2025	16/10/2025
Date analysed	-	17/10/2025	17/10/2025
Perfluorobutanesulfonic acid	µg/L	<0.0004	<0.0004
Perfluorohexanesulfonic acid - PFHxS	µg/L	<0.0002	<0.0002
Perfluorooctanesulfonic acid PFOS	µg/L	0.0004	0.0002
Perfluorooctanoic acid PFOA	µg/L	<0.0002	0.0004
6:2 FTS	µg/L	0.018	0.0074
8:2 FTS	µg/L	<0.0004	<0.0004
Surrogate $^{13}C_8$ *PFOS*	%	102	100
Surrogate $^{13}C_2$ *PFOA*	%	107	105
Extracted ISTD $^{13}C_3$ *PFBS*	%	92	97
Extracted ISTD $^{18}O_2$ *PFHxS*	%	97	103
Extracted ISTD $^{13}C_4$ *PFOS*	%	84	90
Extracted ISTD $^{13}C_4$ *PFOA*	%	103	113
Extracted ISTD $^{13}C_2$ *6:2FTS*	%	153	141
Extracted ISTD $^{13}C_2$ *8:2FTS*	%	133	136
Total Positive PFHxS & PFOS	µg/L	0.0004	0.0002
Total Positive PFOS & PFOA	µg/L	0.0004	0.0006
Total Positive PFAS	µg/L	0.019	0.0080

Microbiological Testing			
Our Reference	UNITS	392905-1	392905-2
Your Reference		BH202	BH205
Date Sampled		14/10/2025	14/10/2025
Type of sample		Water	Water
Date of testing	-	15/10/2025	15/10/2025
Total Coliforms	MPN/100mL	>100 & <1000	5,000

Method ID	Methodology Summary
Ext-008	Subcontracted to Sonic Food & Water Testing. NATA Accreditation No. 4034.
Inorg-006	Alkalinity - determined titrimetrically in accordance with APHA latest edition, 2320-B.
Inorg-018	Total Dissolved Solids - determined gravimetrically. The solids are dried at 180+/-10°C. NOTE: Where the EC of the sample is <100µS/cm, the TDS will typically be below 70mg/L (as the sample is very likely to be at least drinking water quality). Therefore to ensure data quality for TDS, the TDS is typically calculated as per the equation below:- TDS = EC * 0.6
Inorg-019	Suspended Solids - determined gravimetricially by filtration of the sample. The samples are dried at 104+/-5°C.
Inorg-031	Total Phenolics by segmented flow analyser (in line distillation with colourimetric finish). Solids are extracted in a caustic media prior to analysis.
Inorg-040	The concentrations of the major ions (mg/L) are converted to milliequivalents and summed. The ionic balance should be within +/- 15% ie total anions = total cations +/-15%.
Inorg-055	Nitrate - determined colourimetrically. Waters samples are filtered on receipt prior to analysis. Soils are analysed following a water extraction.
Inorg-055	Nitrite - determined colourimetrically based on APHA latest edition NO2- B. Waters samples are filtered on receipt prior to analysis. Soils are analysed following a water extraction.
Inorg-055/062/127	Total Nitrogen - Calculation sum of TKN and oxidised Nitrogen. Alternatively analysed by combustion and chemiluminescence.
Inorg-057	Ammonia - determined colourimetrically, based on APHA latest edition 4500-NH3 F. Waters samples are filtered on receipt prior to analysis. Soils are analysed following a KCl extraction.
Inorg-060	Phosphate determined colourimetrically based on EPA365.1 and APHA latest edition 4500 P E. Waters samples are filtered on receipt prior to analysis. Soils are analysed following a water extraction.
Inorg-062	TKN - determined colourimetrically based on APHA latest edition 4500 Norg. Alternatively, TKN can be derived from calculation (Total N - NOx).
Inorg-081	Anions - a range of Anions are determined by Ion Chromatography, in accordance with APHA latest edition, 4110-B. Waters samples are filtered on receipt prior to analysis. Alternatively determined by colourimetry/turbidity using Discrete Analyser.

Method ID	Methodology Summary
Metals-020	Determination of various metals/elements by ICP-AES. Total Phosphate determined stochiometrically from Phosphorus (assumed to be present as Phosphate). Where salts (oxides, chlorides etc.) are calculated from the element concentration stoichiometrically there is no guarantee that the salt form is completely soluble in the acids used in the preparation. Submission of low masses of sample e.g. for dust samples, may result in raised PQLs. Where molecular anion forms are calculated from an element (e.g. SO4 from S or PO4 from P stoichiometrically), the assumption is that the element is only present in that molecular anion form.
Metals-021	Determination of Mercury by Cold Vapour AAS.
Metals-022	Determination of various metals by ICP-MS. Please note for Bromine and Iodine, any forms of these elements that are present are included together in the one result reported for each of these two elements. Where salts (oxides, chlorides etc.) are calculated from the element concentration stoichiometrically there is no guarantee that the salt form is completely soluble in the acids used in the preparation.
Org-020	Soil samples are extracted with Dichloromethane/Acetone and waters with Dichloromethane and analysed by GC-FID. F2 = (>C10-C16)-Naphthalene as per NEPM B1 Guideline on Investigation Levels for Soil and Groundwater (HSLs Tables 1A (3, 4)). Note Naphthalene is determined from the VOC analysis.
Org-021/022/025	Soil samples are extracted with dichloromethane/acetone and waters with dichloromethane and analysed by GC-ECD and/or GC-MS/GC-MSMS.
Org-021/022/025	Soil samples are extracted with dichloromethane/acetone and waters with dichloromethane and analysed by GC-ECD and/or GC-MS/GC-MSMS. Note, the Total +ve PCBs PQL is reflective of the lowest individual PQL and is therefore" Total +ve PCBs" is simply a sum of the positive individual PCBs.
Org-022/025	Soil samples are extracted with Dichloromethane/Acetone and waters with Dichloromethane and analysed by GC-MS/GC-MSMS.
Org-022/025	Soil samples are extracted with dichloromethane/acetone and waters with dichloromethane and analysed by GC-MS/GC-MSMS. Note, the Total +ve reported DDD+DDE+DDT PQL is reflective of the lowest individual PQL and is therefore simply a sum of the positive individually report DDD+DDE+DDT.
Org-022/025	Soil samples are extracted with Dichloromethane/Acetone and waters with Dichloromethane and analysed by GC-MS/GC-MSMS. Benzo(a)pyrene TEQ as per NEPM B1 Guideline on Investigation Levels for Soil and Groundwater - 2013.
Org-023	Water samples are analysed directly by purge and trap GC-MS.
Org-023	Soil samples are extracted with methanol and spiked into water prior to analysing by purge and trap GC-MS. Water samples are analysed directly by purge and trap GC-MS. F1 = (C6-C10)-BTEX as per NEPM B1 Guideline on Investigation Levels for Soil and Groundwater.
Org-023	Soil samples are extracted with methanol and spiked into water prior to analysing by purge and trap GC-MS. Water samples are analysed directly by purge and trap GC-MS. F1 = (C6-C10)-BTEX as per NEPM B1 Guideline on Investigation Levels for Soil and Groundwater. Note, the Total +ve Xylene PQL is reflective of the lowest individual PQL and is therefore "Total +ve Xylenes" is simply a sum of the positive individual Xylenes.

Method ID	Methodology Summary
Org-029	Soil samples are extracted with basified Methanol. Waters and soil extracts are directly injected and/or concentrated/extracted using SPE. TCLPs/ASLP leachates are centrifuged, the supernatant is then analysed (including amendment with solvent) - as per the option in AS4439.3. Analysis is undertaken with LC-MS/MS. PFAS results include the sum of branched and linear isomers where applicable. Please note that PFAS results are corrected for Extracted Internal Standards (QSM terminology), which are mass labelled analytes added prior to sample preparation to assess matrix effects and verify processing of the sample. PFAS analytes without a commercially available mass labelled analogue are corrected vs a closely eluting mass labelled PFAS compound. Surrogates are also reported, in this context they are mass labelled PFAS compounds added prior to extraction but are used as monitoring compounds only (not used for result correction). Envicarb (or similar) is used discretionally to remove interfering matrix components. Please contact the laboratory if estimates of Measurement Uncertainty are required as per WA DER.

QUALITY CONTROL: VOCs in water						Duplicate			Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W2	[NT]
Date Extracted	-			16/10/2025	1	16/10/2025	17/10/2025		16/10/2025	[NT]
Date Analysed	-			17/10/2025	1	17/10/2025	20/10/2025		17/10/2025	[NT]
Dichlorodifluoromethane	µg/L	10	Org-023	<10	1	<10	<10	0	[NT]	[NT]
Chloromethane	µg/L	10	Org-023	<10	1	<10	<10	0	[NT]	[NT]
Vinyl Chloride	µg/L	10	Org-023	<10	1	<10	<10	0	[NT]	[NT]
Bromomethane	µg/L	10	Org-023	<10	1	<10	<10	0	[NT]	[NT]
Chloroethane	µg/L	10	Org-023	<10	1	<10	<10	0	[NT]	[NT]
Trichlorofluoromethane	µg/L	10	Org-023	<10	1	<10	<10	0	[NT]	[NT]
1,1-Dichloroethene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Trans-1,2-dichloroethene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
1,1-dichloroethane	µg/L	1	Org-023	<1	1	<1	<1	0	117	[NT]
Cis-1,2-dichloroethene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Bromochloromethane	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Chloroform	µg/L	1	Org-023	<1	1	<1	<1	0	122	[NT]
2,2-dichloropropane	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
1,2-dichloroethane	µg/L	1	Org-023	<1	1	<1	<1	0	119	[NT]
1,1,1-trichloroethane	µg/L	1	Org-023	<1	1	<1	<1	0	118	[NT]
1,1-dichloropropene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Cyclohexane	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Carbon tetrachloride	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Benzene	µg/L	1	Org-023	<1	1	<1	<1	0	121	[NT]
Dibromomethane	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
1,2-dichloropropane	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Trichloroethene	µg/L	1	Org-023	<1	1	<1	<1	0	120	[NT]
Bromodichloromethane	µg/L	1	Org-023	<1	1	<1	<1	0	120	[NT]
trans-1,3-dichloropropene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
cis-1,3-dichloropropene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
1,1,2-trichloroethane	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Toluene	µg/L	1	Org-023	<1	1	<1	<1	0	120	[NT]
1,3-dichloropropane	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Dibromochloromethane	µg/L	1	Org-023	<1	1	<1	<1	0	115	[NT]
1,2-dibromoethane	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Tetrachloroethene	µg/L	1	Org-023	<1	1	<1	<1	0	120	[NT]
1,1,1,2-tetrachloroethane	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Chlorobenzene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Ethylbenzene	µg/L	1	Org-023	<1	1	<1	<1	0	118	[NT]
Bromoform	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
m+p-xylene	µg/L	2	Org-023	<2	1	<2	<2	0	119	[NT]
Styrene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
1,1,2,2-tetrachloroethane	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]

QUALITY CONTROL: VOCs in water						Duplicate			Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W2	[NT]
o-xylene	µg/L	1	Org-023	<1	1	<1	<1	0	120	[NT]
1,2,3-trichloropropane	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Isopropylbenzene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Bromobenzene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
n-propyl benzene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
2-chlorotoluene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
4-chlorotoluene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
1,3,5-trimethyl benzene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Tert-butyl benzene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
1,2,4-trimethyl benzene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
1,3-dichlorobenzene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Sec-butyl benzene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
1,4-dichlorobenzene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
4-isopropyl toluene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
1,2-dichlorobenzene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
n-butyl benzene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
1,2-dibromo-3-chloropropane	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
1,2,4-trichlorobenzene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Hexachlorobutadiene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
1,2,3-trichlorobenzene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Surrogate Dibromofluoromethane	%		Org-023	102	1	104	105	1	103	[NT]
Surrogate Toluene-d8	%		Org-023	104	1	104	105	1	103	[NT]
Surrogate 4-Bromofluorobenzene	%		Org-023	100	1	102	100	2	99	[NT]

QUALITY CONTROL: vTRH(C6-C10)/BTEXN in Water						Duplicate			Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W2	[NT]
Date extracted	-			16/10/2025	1	16/10/2025	17/10/2025		16/10/2025	[NT]
Date analysed	-			17/10/2025	1	17/10/2025	20/10/2025		17/10/2025	[NT]
TRH C_6 - C_9	µg/L	10	Org-023	<10	1	<10	<10	0	119	[NT]
TRH C_6 - C_{10}	µg/L	10	Org-023	<10	1	<10	<10	0	119	[NT]
Benzene	µg/L	1	Org-023	<1	1	<1	<1	0	121	[NT]
Toluene	µg/L	1	Org-023	<1	1	<1	<1	0	120	[NT]
Ethylbenzene	µg/L	1	Org-023	<1	1	<1	<1	0	118	[NT]
m+p-xylene	µg/L	2	Org-023	<2	1	<2	<2	0	119	[NT]
o-xylene	µg/L	1	Org-023	<1	1	<1	<1	0	120	[NT]
Naphthalene	µg/L	1	Org-023	<1	1	<1	<1	0	[NT]	[NT]
Surrogate Dibromofluoromethane	%		Org-023	102	1	104	105	1	103	[NT]
Surrogate Toluene-d8	%		Org-023	104	1	104	105	1	103	[NT]
Surrogate 4-Bromofluorobenzene	%		Org-023	100	1	102	100	2	99	[NT]

QUALITY CONTROL: svTRH (C10-C40) in Water					Duplicate				Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W1	[NT]
Date extracted	-			15/10/2025	[NT]	[NT]	[NT]	[NT]	15/10/2025	[NT]
Date analysed	-			15/10/2025	[NT]	[NT]	[NT]	[NT]	15/10/2025	[NT]
TRH C_{10} - C_{14}	µg/L	50	Org-020	<50	[NT]	[NT]	[NT]	[NT]	94	[NT]
TRH C_{15} - C_{28}	µg/L	100	Org-020	<100	[NT]	[NT]	[NT]	[NT]	109	[NT]
TRH C_{29} - C_{36}	µg/L	100	Org-020	<100	[NT]	[NT]	[NT]	[NT]	86	[NT]
TRH >C_{10} - C_{16}	µg/L	50	Org-020	<50	[NT]	[NT]	[NT]	[NT]	94	[NT]
TRH >C_{16} - C_{34}	µg/L	100	Org-020	<100	[NT]	[NT]	[NT]	[NT]	109	[NT]
TRH >C_{34} - C_{40}	µg/L	100	Org-020	<100	[NT]	[NT]	[NT]	[NT]	86	[NT]
Surrogate o-Terphenyl	%		Org-020	89	[NT]	[NT]	[NT]	[NT]	98	[NT]

QUALITY CONTROL: PAHs in Water - Trace Level					Duplicate				Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W1	[NT]
Date extracted	-			15/10/2025	1	15/10/2025	20/10/2025		15/10/2025	[NT]
Date analysed	-			17/10/2025	1	17/10/2025	21/10/2025		17/10/2025	[NT]
Naphthalene	µg/L	0.02	Org-022/025	<0.02	1	<0.02	<0.02	0	83	[NT]
Acenaphthylene	µg/L	0.01	Org-022/025	<0.01	1	<0.01	<0.01	0	[NT]	[NT]
Acenaphthene	µg/L	0.01	Org-022/025	<0.01	1	<0.01	<0.01	0	80	[NT]
Fluorene	µg/L	0.01	Org-022/025	<0.01	1	<0.01	<0.01	0	91	[NT]
Phenanthrene	µg/L	0.01	Org-022/025	<0.01	1	0.02	0.02	0	96	[NT]
Anthracene	µg/L	0.01	Org-022/025	<0.01	1	<0.01	<0.01	0	[NT]	[NT]
Fluoranthene	µg/L	0.01	Org-022/025	<0.01	1	0.02	0.02	0	102	[NT]
Pyrene	µg/L	0.01	Org-022/025	<0.01	1	0.02	0.02	0	103	[NT]
Benzo(a)anthracene	µg/L	0.01	Org-022/025	<0.01	1	<0.01	<0.01	0	[NT]	[NT]
Chrysene	µg/L	0.01	Org-022/025	<0.01	1	<0.01	<0.01	0	86	[NT]
Benzo(b,j+k)fluoranthene	µg/L	0.02	Org-022/025	<0.02	1	<0.02	<0.02	0	[NT]	[NT]
Benzo(a)pyrene	µg/L	0.01	Org-022/025	<0.01	1	<0.01	<0.01	0	93	[NT]
Dibenzo(a,h)anthracene	µg/L	0.01	Org-022/025	<0.01	1	<0.01	<0.01	0	[NT]	[NT]
Indeno(1,2,3-c,d)pyrene	µg/L	0.01	Org-022/025	<0.01	1	<0.01	<0.01	0	[NT]	[NT]
Benzo(g,h,i)perylene	µg/L	0.01	Org-022/025	<0.01	1	<0.01	<0.01	0	[NT]	[NT]
Surrogate p-Terphenyl-d14	%		Org-022/025	104	1	94	91	3	98	[NT]

QUALITY CONTROL: PAHs in Water - Trace Level					Duplicate				Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	[NT]	[NT]
Date extracted	-			[NT]	2	15/10/2025	20/10/2025		[NT]	[NT]
Date analysed	-			[NT]	2	17/10/2025	21/10/2025		[NT]	[NT]
Naphthalene	µg/L	0.02	Org-022/025	[NT]	2	<0.02	<0.02	0	[NT]	[NT]
Acenaphthylene	µg/L	0.01	Org-022/025	[NT]	2	<0.01	<0.01	0	[NT]	[NT]
Acenaphthene	µg/L	0.01	Org-022/025	[NT]	2	<0.01	<0.01	0	[NT]	[NT]
Fluorene	µg/L	0.01	Org-022/025	[NT]	2	<0.01	<0.01	0	[NT]	[NT]
Phenanthrene	µg/L	0.01	Org-022/025	[NT]	2	0.04	0.01	120	[NT]	[NT]
Anthracene	µg/L	0.01	Org-022/025	[NT]	2	<0.01	<0.01	0	[NT]	[NT]
Fluoranthene	µg/L	0.01	Org-022/025	[NT]	2	0.02	0.03	40	[NT]	[NT]
Pyrene	µg/L	0.01	Org-022/025	[NT]	2	0.02	0.02	0	[NT]	[NT]
Benzo(a)anthracene	µg/L	0.01	Org-022/025	[NT]	2	<0.01	<0.01	0	[NT]	[NT]
Chrysene	µg/L	0.01	Org-022/025	[NT]	2	<0.01	<0.01	0	[NT]	[NT]
Benzo(b,j+k)fluoranthene	µg/L	0.02	Org-022/025	[NT]	2	<0.02	<0.02	0	[NT]	[NT]
Benzo(a)pyrene	µg/L	0.01	Org-022/025	[NT]	2	<0.01	<0.01	0	[NT]	[NT]
Dibenzo(a,h)anthracene	µg/L	0.01	Org-022/025	[NT]	2	<0.01	<0.01	0	[NT]	[NT]
Indeno(1,2,3-c,d)pyrene	µg/L	0.01	Org-022/025	[NT]	2	<0.01	<0.01	0	[NT]	[NT]
Benzo(g,h,i)perylene	µg/L	0.01	Org-022/025	[NT]	2	<0.01	<0.01	0	[NT]	[NT]
Surrogate p-Terphenyl-d14	%		Org-022/025	[NT]	2	95	78	20	[NT]	[NT]

QUALITY CONTROL: OCPs in Water - Trace Level					Duplicate				Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W1	[NT]
Date extracted	-			15/10/2025	[NT]	[NT]	[NT]	[NT]	15/10/2025	[NT]
Date analysed	-			17/10/2025	[NT]	[NT]	[NT]	[NT]	17/10/2025	[NT]
alpha-BHC	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	98	[NT]
HCB	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
beta-BHC	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	96	[NT]
gamma-BHC	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Heptachlor	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	104	[NT]
delta-BHC	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Aldrin	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	100	[NT]
Heptachlor Epoxide	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	108	[NT]
gamma-Chlordane	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
alpha-Chlordane	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Endosulfan I	µg/L	0.002	Org-022/025	<0.002	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
pp-DDE	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	98	[NT]
Dieldrin	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	108	[NT]
Endrin	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	104	[NT]
Endosulfan II	µg/L	0.002	Org-022/025	<0.002	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
pp-DDD	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	112	[NT]
Endrin Aldehyde	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
pp-DDT	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Endosulfan Sulphate	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	104	[NT]
Methoxychlor	µg/L	0.001	Org-022/025	<0.001	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Surrogate 4-Chloro-3-NBTF	%		Org-022/025	76	[NT]	[NT]	[NT]	[NT]	84	[NT]

QUALITY CONTROL: OP in water LL ANZECCF/ADWG					Duplicate				Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W1	[NT]
Date extracted	-			15/10/2025	[NT]	[NT]	[NT]	[NT]	15/10/2025	[NT]
Date analysed	-			17/10/2025	[NT]	[NT]	[NT]	[NT]	17/10/2025	[NT]
Dichlorvos	µg/L	0.05	Org-022/025	<0.05	[NT]	[NT]	[NT]	[NT]	117	[NT]
Mevinphos	µg/L	0.05	Org-022/025	<0.05	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Phorate	µg/L	0.05	Org-022/025	<0.05	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Dimethoate	µg/L	0.1	Org-022/025	<0.1	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Diazinon	µg/L	0.01	Org-022/025	<0.01	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Disulfoton	µg/L	0.05	Org-022/025	<0.05	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Chlorpyriphos-methyl	µg/L	0.05	Org-022/025	<0.05	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Parathion-Methyl	µg/L	0.05	Org-022/025	<0.05	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Ronnel	µg/L	0.05	Org-022/025	<0.05	[NT]	[NT]	[NT]	[NT]	97	[NT]
Fenitrothion	µg/L	0.05	Org-022/025	<0.05	[NT]	[NT]	[NT]	[NT]	101	[NT]
Malathion	µg/L	0.05	Org-022/025	<0.05	[NT]	[NT]	[NT]	[NT]	113	[NT]
Chlorpyriphos	µg/L	0.009	Org-022/025	<0.009	[NT]	[NT]	[NT]	[NT]	114	[NT]
Fenthion	µg/L	0.05	Org-022/025	<0.05	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Parathion	µg/L	0.004	Org-022/025	<0.004	[NT]	[NT]	[NT]	[NT]	96	[NT]
Bromophos ethyl	µg/L	0.05	Org-022/025	<0.05	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Methidathion	µg/L	0.05	Org-022/025	<0.05	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Fenamiphos	µg/L	0.05	Org-022/025	<0.05	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Ethion	µg/L	0.05	Org-022/025	<0.05	[NT]	[NT]	[NT]	[NT]	109	[NT]
Phosalone	µg/L	0.05	Org-022/025	<0.05	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Azinphos-methyl (Guthion)	µg/L	0.02	Org-022/025	<0.02	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Coumaphos	µg/L	0.05	Org-022/025	<0.05	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Surrogate 4-Chloro-3-NBTF	%		Org-022/025	76	[NT]	[NT]	[NT]	[NT]	84	[NT]

QUALITY CONTROL: PCBs in Water - Trace Level						Duplicate			Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W1	[NT]
Date extracted	-			15/10/2025	[NT]	[NT]	[NT]	[NT]	15/10/2025	[NT]
Date analysed	-			17/10/2025	[NT]	[NT]	[NT]	[NT]	17/10/2025	[NT]
Aroclor 1016	µg/L	0.01	Org-021/022/025	<0.01	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Aroclor 1221	µg/L	0.01	Org-021/022/025	<0.01	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Aroclor 1232	µg/L	0.01	Org-021/022/025	<0.01	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Aroclor 1242	µg/L	0.01	Org-021/022/025	<0.01	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Aroclor 1248	µg/L	0.01	Org-021/022/025	<0.01	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Aroclor 1254	µg/L	0.01	Org-021/022/025	<0.01	[NT]	[NT]	[NT]	[NT]	115	[NT]
Aroclor 1260	µg/L	0.01	Org-021/022/025	<0.01	[NT]	[NT]	[NT]	[NT]	[NT]	[NT]
Surrogate 2-Fluorobiphenyl	%		Org-021/022/025	63	[NT]	[NT]	[NT]	[NT]	75	[NT]

QUALITY CONTROL: HM in water - dissolved						Duplicate			Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W2	392905-2
Date prepared	-			15/10/2025	1	15/10/2025	15/10/2025		15/10/2025	15/10/2025
Date analysed	-			15/10/2025	1	15/10/2025	15/10/2025		15/10/2025	15/10/2025
Arsenic-Dissolved	µg/L	1	Metals-022	<1	1	<1	<1	0	106	117
Cadmium-Dissolved	µg/L	0.1	Metals-022	<0.1	1	<0.1	<0.1	0	105	104
Chromium-Dissolved	µg/L	1	Metals-022	<1	1	<1	<1	0	102	119
Copper-Dissolved	µg/L	1	Metals-022	<1	1	<1	<1	0	106	108
Lead-Dissolved	µg/L	1	Metals-022	<1	1	<1	<1	0	110	98
Mercury-Dissolved	µg/L	0.05	Metals-021	<0.05	1	<0.05	[NT]		107	[NT]
Nickel-Dissolved	µg/L	1	Metals-022	<1	1	6	6	0	104	110
Zinc-Dissolved	µg/L	1	Metals-022	<1	1	70	72	3	104	105

QUALITY CONTROL: HM in water - total						Duplicate			Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W4	[NT]
Date prepared	-			15/10/2025	[NT]	[NT]	[NT]	[NT]	15/10/2025	[NT]
Date analysed	-			15/10/2025	[NT]	[NT]	[NT]	[NT]	15/10/2025	[NT]
Arsenic-Total	µg/L	1	Metals-022	<1	[NT]	[NT]	[NT]	[NT]	101	[NT]
Cadmium-Total	µg/L	0.1	Metals-022	<0.1	[NT]	[NT]	[NT]	[NT]	99	[NT]
Chromium-Total	µg/L	1	Metals-022	<1	[NT]	[NT]	[NT]	[NT]	100	[NT]
Copper-Total	µg/L	1	Metals-022	<1	[NT]	[NT]	[NT]	[NT]	103	[NT]
Lead-Total	µg/L	1	Metals-022	<1	[NT]	[NT]	[NT]	[NT]	102	[NT]
Mercury-Total	µg/L	0.05	Metals-021	<0.05	[NT]	[NT]	[NT]	[NT]	97	[NT]
Nickel-Total	µg/L	1	Metals-022	<1	[NT]	[NT]	[NT]	[NT]	101	[NT]
Zinc-Total	µg/L	1	Metals-022	<1	[NT]	[NT]	[NT]	[NT]	101	[NT]

QUALITY CONTROL: Metals in Waters - Acid extractable					Duplicate				Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W1	[NT]
Date prepared	-			15/10/2025	[NT]	[NT]	[NT]	[NT]	15/10/2025	[NT]
Date analysed	-			15/10/2025	[NT]	[NT]	[NT]	[NT]	15/10/2025	[NT]
Phosphorus - Total	mg/L	0.05	Metals-020	<0.05	[NT]	[NT]	[NT]	[NT]	93	[NT]

QUALITY CONTROL: Total Phenolics in Water						Duplicate			Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W1	392905-2
Date extracted	-			15/10/2025	1	15/10/2025	15/10/2025		15/10/2025	15/10/2025
Date analysed	-			15/10/2025	1	15/10/2025	15/10/2025		15/10/2025	15/10/2025
Total Phenolics (as Phenol)	mg/L	0.05	Inorg-031	<0.05	1	<0.05	<0.05	0	101	98

QUALITY CONTROL: Miscellaneous Inorganics						Duplicate			Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W1	392905-2
Date prepared	-			15/10/2025	1	15/10/2025	15/10/2025		15/10/2025	15/10/2025
Date analysed	-			15/10/2025	1	15/10/2025	15/10/2025		15/10/2025	15/10/2025
Total Dissolved Solids (grav)	mg/L	5	Inorg-018	<5	1	2600	[NT]		93	[NT]
Total Suspended Solids	mg/L	5	Inorg-019	<5	1	9	[NT]		93	[NT]
Ammonia as N in water	mg/L	0.005	Inorg-057	<0.005	1	0.66	0.66	0	95	93
Nitrate as N in water	mg/L	0.005	Inorg-055	<0.005	1	<0.005	<0.005	0	93	91
Nitrite as N in water	mg/L	0.005	Inorg-055	<0.005	1	<0.005	<0.005	0	96	#
NOx as N in water	mg/L	0.005	Inorg-055	<0.005	1	<0.005	<0.005	0	93	91
Total Nitrogen in water	mg/L	0.1	Inorg-055/062/127	<0.1	1	0.8	[NT]		107	[NT]
TKN in water	mg/L	0.1	Inorg-062	<0.1	1	0.8	[NT]		[NT]	[NT]
Phosphate as P in water	mg/L	0.005	Inorg-060	<0.005	1	0.01	0.01	0	102	91
Organic Nitrogen as N	mg/L	0.2	Inorg-055/062/127	<0.2	1	<0.2	[NT]		[NT]	[NT]

QUALITY CONTROL: Ion Balance						Duplicate			Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W1	392905-2
Date prepared	-			15/10/2025	1	15/10/2025	15/10/2025		15/10/2025	15/10/2025
Date analysed	-			15/10/2025	1	15/10/2025	15/10/2025		15/10/2025	15/10/2025
Calcium - Dissolved	mg/L	0.5	Metals-020	<0.5	1	41	40	2	93	#
Potassium - Dissolved	mg/L	0.5	Metals-020	<0.5	1	24	23	4	97	92
Sodium - Dissolved	mg/L	0.5	Metals-020	<0.5	1	670	680	1	88	#
Magnesium - Dissolved	mg/L	0.5	Metals-020	<0.5	1	67	66	2	93	#
Hardness (calc) equivalent $CaCO_3$	mg/L	3	Metals-020	[NT]	1	380	370	3	[NT]	[NT]
Hydroxide Alkalinity (OH^-) as $CaCO_3$	mg/L	5	Inorg-006	<5	1	<5	[NT]		[NT]	[NT]
Bicarbonate Alkalinity as $CaCO_3$	mg/L	5	Inorg-006	<5	1	890	[NT]		[NT]	[NT]
Carbonate Alkalinity as $CaCO_3$	mg/L	5	Inorg-006	<5	1	<5	[NT]		[NT]	[NT]
Total Alkalinity as $CaCO_3$	mg/L	5	Inorg-006	<5	1	890	[NT]		95	[NT]
Sulphate, SO4	mg/L	1	Inorg-081	<1	1	120	[NT]		110	[NT]
Chloride, Cl	mg/L	1	Inorg-081	<1	1	510	[NT]		117	[NT]
Ionic Balance	%		Inorg-040	[NT]	1	4.0	[NT]		[NT]	[NT]

QUALITY CONTROL: PFAS in Water TRACE Short					Duplicate				Spike Recovery %	
Test Description	Units	PQL	Method	Blank	#	Base	Dup.	RPD	LCS-W1	[NT]
Date prepared	-			16/10/2025	[NT]	[NT]	[NT]	[NT]	16/10/2025	[NT]
Date analysed	-			17/10/2025	[NT]	[NT]	[NT]	[NT]	17/10/2025	[NT]
Perfluorobutanesulfonic acid	µg/L	0.0004	Org-029	<0.0004	[NT]	[NT]	[NT]	[NT]	94	[NT]
Perfluorohexanesulfonic acid - PFHxS	µg/L	0.0002	Org-029	<0.0002	[NT]	[NT]	[NT]	[NT]	92	[NT]
Perfluorooctanesulfonic acid PFOS	µg/L	0.0002	Org-029	<0.0002	[NT]	[NT]	[NT]	[NT]	95	[NT]
Perfluorooctanoic acid PFOA	µg/L	0.0002	Org-029	<0.0002	[NT]	[NT]	[NT]	[NT]	96	[NT]
6:2 FTS	µg/L	0.0004	Org-029	<0.0004	[NT]	[NT]	[NT]	[NT]	109	[NT]
8:2 FTS	µg/L	0.0004	Org-029	<0.0004	[NT]	[NT]	[NT]	[NT]	106	[NT]
Surrogate $^{13}C_8$ *PFOS*	%		Org-029	107	[NT]	[NT]	[NT]	[NT]	98	[NT]
Surrogate $^{13}C_2$ *PFOA*	%		Org-029	101	[NT]	[NT]	[NT]	[NT]	103	[NT]
Extracted ISTD $^{13}C_3$ *PFBS*	%		Org-029	84	[NT]	[NT]	[NT]	[NT]	87	[NT]
Extracted ISTD $^{18}O_2$ *PFHxS*	%		Org-029	88	[NT]	[NT]	[NT]	[NT]	90	[NT]
Extracted ISTD $^{13}C_4$ *PFOS*	%		Org-029	73	[NT]	[NT]	[NT]	[NT]	79	[NT]
Extracted ISTD $^{13}C_4$ *PFOA*	%		Org-029	97	[NT]	[NT]	[NT]	[NT]	94	[NT]
Extracted ISTD $^{13}C_2$ *6:2FTS*	%		Org-029	129	[NT]	[NT]	[NT]	[NT]	117	[NT]
Extracted ISTD $^{13}C_2$ *8:2FTS*	%		Org-029	123	[NT]	[NT]	[NT]	[NT]	108	[NT]

Result Definitions

NT	Not tested
NA	Test not required
INS	Insufficient sample for this test
PQL	Practical Quantitation Limit
<	Less than
>	Greater than
RPD	Relative Percent Difference
LCS	Laboratory Control Sample
NS	Not specified
NEPM	National Environmental Protection Measure
NR	Not Reported

Quality Control Definitions

Blank	This is the component of the analytical signal which is not derived from the sample but from reagents, glassware etc, can be determined by processing solvents and reagents in exactly the same manner as for samples.
Duplicate	This is the complete duplicate analysis of a sample from the process batch. If possible, the sample selected should be one where the analyte concentration is easily measurable.
Matrix Spike	A portion of the sample is spiked with a known concentration of target analyte. The purpose of the matrix spike is to monitor the performance of the analytical method used and to determine whether matrix interferences exist.
LCS (Laboratory Control Sample)	This comprises either a standard reference material or a control matrix (such as a blank sand or water) fortified with analytes representative of the analyte class. It is simply a check sample.
Surrogate Spike	Surrogates are known additions to each sample, blank, matrix spike and LCS in a batch, of compounds which are similar to the analyte of interest, however are not expected to be found in real samples.

Laboratory Acceptance Criteria

Duplicate sample and matrix spike recoveries may not be reported on smaller jobs, however, were analysed at a frequency to meet or exceed NEPM requirements. All samples are tested in batches of 20. The duplicate sample RPD and matrix spike recoveries for the batch were within the laboratory acceptance criteria.

Filters, swabs, wipes, tubes and badges will not have duplicate data as the whole sample is generally extracted during sample extraction.

Spikes for Physical and Aggregate Tests are not applicable.

For VOCs in water samples, three vials are required for duplicate or spike analysis.

Duplicates: >10xPQL - RPD acceptance criteria will vary depending on the analytes and the analytical techniques but is typically in the range 20%-50% – see ELN-P05 QA/QC tables for details; <10xPQL - RPD are higher as the results approach PQL and the estimated measurement uncertainty will statistically increase.

Air volumes are typically provided by customers (often as flow rate(s) and sampling time(s) and/or simply volumes) sampled or exposure times (determines 'volume' passive badges are exposed to)). Hence in such circumstances the volume measurement is inevitably not covered by Envirolab's NATA accreditation. An exception may occur where Envirolab Newcastle does the sampling where accreditation exists for certain types of sampling and hence volume determination(s). Note air volumes are often used to determine concentrations for dust and/or analyses on filters, sorbents and in impingers. For canister sampling, the air volume is covered by Envirolab's NATA accreditation.

Matrix Spikes, LCS and Surrogate recoveries: Generally 70-130% for inorganics/metals (not SPOCAS); 60-140% for organics/SPOCAS (+/-50% surrogates) and 10-140% for labile SVOCs (including labile surrogates), ultra trace organics and speciated phenols is acceptable.

In circumstances where no duplicate and/or sample spike has been reported at 1 in 10 and/or 1 in 20 samples respectively, the sample volume submitted was insufficient in order to satisfy laboratory QA/QC protocols.

When samples are received where certain analytes are outside of recommended technical holding times (THTs), the analysis has proceeded. Where analytes are on the verge of breaching THTs, every effort will be made to analyse within the THT or as soon as practicable.

Where sampling dates are not provided, Envirolab are not in a position to comment on the validity of the analysis where recommended technical holding times may have been breached.

Measurement Uncertainty estimates are available for most tests upon request.

Analysis of aqueous samples typically involves the extraction/digestion and/or analysis of the liquid phase only (i.e. NOT any settled sediment phase but inclusive of suspended particles if present), unless stipulated on the Envirolab COC and/or by correspondence. Notable exceptions include certain Physical Tests (pH/EC/BOD/COD/Apparent Colour etc.), Solids testing, total recoverable metals and PFAS where solids are included by default.

For Dust Deposit Gauge (DDG) analysis the sampling, sampling period and funnel exposure area do not fall under Envirolab's NATA accreditation (unless the Newcastle laboratory where responsible for the sampling), hence the annotation on the DDG units of reporting.

Urine Analysis - The BEI values listed are taken from the 2022 edition of "TLVs and BEIs Threshold Limits" by ACGIH.

Report Comments

MISC_INORG: # Percent recovery not reported due to the high concentration of the analyte/s in the sample/s. However, an acceptable recovery was obtained for the LCS.

Ion Balance - # Percent recovery is not applicable due to the high concentration of the element/s in the sample/s. However an acceptable recovery was obtained for the LCS.
The mass inbalance for sample#2 may be caused by other ions that have not been measured.

PFAS in Water TRACE Short - For PFAS Extracted Internal Standards denoted with # or outside the 50-150% acceptance range, the respective target analyte results may be unaffected, in other circumstances the PQL has been raised to accommodate the outlier(s).

8 HM in water - TRACE METALS: In theory the total metal content should be higher than the dissolved metal content. However, in some samples this is not the case. The sample has been re-analysed for both Total and Dissolved metals and results have been confirmed.

Micro analysed by Sonic, report no W2523330.

Douglas PARTNERS

Groundwater (GW) Field Sheet

Project and Bore Installation Details						
Project Name / Site Location	3 Brookhollow Ave				Project Number	237937.00
Well Construction Details	Well ID	BH202	Drilling Method		Hole Diameter (m)~	
	Well Depth (m bgl)		Screened (m bgl)		Stick Up (m)	
Survey Information	Easting		Northing		Elevation RL	
GW Level During Drilling		m bgl				
Contaminants/Comments						
*Estimated Well Volume = H * F	Std. Drilling Diameter (m)~	NMLC (0.075)	HQ (0.096)	PQ (0.1226)	SFA (0.125)	HFA (0.194)
**Purge Target: min. 3 well volumes	Factor (F):	2.8	3.7	5.2	5.4	11.1

Well Development Details					
Date / Time / Weather Conditions				Purged By	
Purge Method / Equipment					
Product observed / Thickness		mm	Confirmed with Bailer? (Y / N)		
GW Level (pre-purge)		m bgl	Observed Well Depth		m bgl
Height of Water Column (H)		m bgl	Estimated Bore Volume*		L
GW Level (post-purge)		m bgl	Total Volume Purged**		L
Appearance/Comments					

Sampling Details					
Date / Time / Weather Conditions	14/10/25 7:28 Sunny			Sampled By	SHF
Sampling Method / Equipment	~~Pump~~ Bladder pump				
WQM Model	YSI		WQM Calibration Date		
Product observed / Thickness	-	mm	Confirmed with Bailer? (Y / N)	Y	
GW Level (pre-micropurge)	8.14	m bgl	Observed Well Depth	12.43	m bgl
Height of Water Column	4.29	m bgl	Estimated Bore Volume*	23.2	L
GW Level (post sample)	9.30	m bgl	Total Volume of Micro-Purged	13	L

Water Quality Parameters

Time	Cumulative Volume (L)	Temp (°C)	DO (mg/L)#	EC (Circle: µS or mS/cm)	pH	Redox (mV) (AgCl Probe)	Turbidity^
Stabilisation Target (3 readings)		0.2	+/- 10%	+/- 5%	+/- 0.1	+/- 10 mV	+/- 10%
7:49	1	19.5	1.01	3644	6.81	220.8	11
7:51	2	19.6	0.40	3721	6.86	189.8	14
7:53	3	19.6	0.24	3737	6.84	164.8	17
7:55	4	19.6	0.14	3762	6.84	145.1	12
7:57	5	19.6	0.11	3769	6.84	135.8	11
7:59	6	19.6	0.06	3771	6.84	129.1	12
8:01	7	19.6	0.02	3778	6.84	123.1	12
8:03	8	19.6	0.0	3786	6.85	118.7	10
8:05	9	19.6	0.0	3787	6.85	113.6	10

Notes: # Considered stabilised if three DO values are less than 0.5 mg/L ^ Considered stabilised if three Turbidity values are less than 5 NTU

Sample Details						
Sampling Depth (rationale)	10	m bgl,	mid column			
Sample Observations (e.g. colour, sediment, sheen, odour)	Clear, no silt, no sheen, no odour					
Sample ID	BH202					
QAQC Samples	Replicate		Triplicate		Other	
Sample Containers Quantity (fill in quantity) Preservation (as shown) Filtration (Circle Filtered or UnFiltered)	Amber glass		Plastic		PFAS (no teflon)	
	Metals (F/UF) (HNO3)		Phenols/COD/NH3 (H2SO4)		Vials (HCl)	
	Ferrous/Ferric Iron (HCl)		Cyanides/Chromium (NaOH)		Other	
Comments						

Groundwater (GW) Field Sheet

Project and Bore Installation Details						
Project Name / Site Location					Project Number	233937.00
Well Construction Details	Well ID	BH205	Drilling Method		Hole Diameter (m)~	
	Well Depth (m bgl)		Screened (m bgl)		Stick Up (m)	
Survey Information	Easting		Northing		Elevation RL	
GW Level During Drilling		m bgl				
Contaminants/Comments						
*Estimated Well Volume = H * F	Std. Drilling Diameter (m)~	NMLC (0.075)	HQ (0.096)	PQ (0.1226)	SFA (0.125)	HFA (0.194)
**Purge Target: min. 3 well volumes	Factor (F):	2.8	3.7	5.2	5.4	11.1

Well Development Details					
Date / Time / Weather Conditions				Purged By	
Purge Method / Equipment					
Product observed / Thickness		mm	Confirmed with Bailer? (Y / N)		
GW Level (pre-purge)		m bgl	Observed Well Depth		m bgl
Height of Water Column (H)		m bgl	Estimated Bore Volume*		L
GW Level (post-purge)		m bgl	Total Volume Purged**		L
Appearance/Comments					

Sampling Details							
Date / Time / Weather Conditions	14/10/25 8:44 Sunny					Sampled By	SAF
Sampling Method / Equipment	Bladder Pump						
WQM Model	YSI				WQM Calibration Date		
Product observed / Thickness	–	mm			Confirmed with Bailer? (Y / N)	X	
GW Level (pre-micropurge)	6.79	m bgl			Observed Well Depth	20.20	m bgl
Height of Water Column	13.41	m bgl			Estimated Bore Volume*	72.4	L
GW Level (post sample)	10.74	m bgl			Total Volume of Micro-Purged	10	L

Water Quality Parameters

Time	Cumulative Volume (L)	Temp (°C)	DO (mg/L)#	EC (Circle: µS or mS/cm)	pH	Redox (mV) (AgCl Probe)	Turbidity^
Stabilisation Target (3 readings)		0.2	+/- 10%	+/- 5%	+/- 0.1	+/- 10 mV	+/- 10%
9:16	1	18.9	1.35	6712	6.63	158.4	10
9:18	2	18.9	0.40	6778	6.68	156.6	11
9:20	3	18.9	0.25	6729	6.65	156.9	11
9:22	4	18.9	0.16	6625	6.64	157.3	11
Notes:	# Considered stabilised if three DO values are less than 0.5 mg/L			^ Considered stabilised if three Turbidity values are less than 5 NTU			

Sample Details						
Sampling Depth (rationale)	13	m bgl,	mid column			
Sample Observations (e.g. colour, sediment, sheen, odour)	Clear, no silt, no sheen, no odour					
Sample ID	BH205					
QAQC Samples	Replicate		Triplicate		Other	
Sample Containers Quantity (fill in quantity) Preservation (as shown) Filtration (Circle Filtered or UnFiltered)	Amber glass		Plastic		PFAS (no teflon)	
	Metals (F/UF) (HNO3)		Phenols/COD/NH3 (H2SO4)		Vials (HCl)	
	Ferrous/Ferric Iron (HCl)		Cyanides/Chromium (NaOH)		Other	
Comments						

Douglas PARTNERS | GROUNDED EXPERTISE

CHAIN OF CUSTODY DESPATCH SHEET

Project No:	233937.00	**Suburb:**	Baulkham Hills	**To:**	Envirolab Services
Project Manager:	Adam Teoh	**Order Number:**		**Sampler:**	SAF
					12 Ashley St, Chatswood NSW 2067
Email:	adam.teoh@douglaspartners.com.au			**Attn:**	Sample Receipt
Turnaround time:	☑ Standard ☐ 72 hour ☐ 48 hour ☐ 24 hour ☐ Same day				(02) 9910 6200 samplereceipt@envirolab.com
Prior Storage:	☐ Fridge ☐ Freezer ☑ Esky ☐ Shelf	**Do samples contain 'potential' HBM?**	☐ No ☐ Yes (If Yes, then handle, transport and store in accordance with FPM HAZID)		

Lab ID	Sample ID: Location / Other ID	Depth From	Depth To	Date Sampled	Sample Type: S - soil W - water M - Material	Container Type: G - glass P - plastic	Analytes: 8 Metals - dissolved and total	TRH, BTEX, Phenols	trace PAH	trace (OCP, OPP and PCB)	PFAS short suite (trace level)	TDS, TSS, nutrients, coliforms,	ionic balance suite	VOCs	Combo 8L			Notes/ Preservation/ Additional Requirements
1	BH202			14/10/25	W	G/P					x	x	x	x	x			Trace PAH, OCP, OPP, PCB
2	BH205			14/10/25	W	G/P					x	x	x	x	x			

Envirolab Services
12 Ashley St
Chatswood NSW 2067
Ph: (02) 9910 6200

Job No: 392905
Date Received: 14/10/25
Time Received: 1610
Received By: TTV
Temp: Cool/Ambient
Cooling: Ice/Icepack
Security: Intact/Broken/None

Metals to analyse:	8 Metals - dissolved and total		
Number of samples in container:		**Transported to laboratory by:**	
Send results to:	Douglas Partners Pty Ltd		
Address:	96 Hermitage Road, West Ryde NSW 211	**Phone:**	(02) 9809 0666
Relinquished by:		**Date:**	**Signed:**

LAB RECEIPT

Lab Ref. No:	392905
Received by:	Tony T.
Date & Time:	14/10/25 1610
Signed:	TTV