




Scale: 1 : 50



NTS

- NOTES:**
1. SHOWN IN ISOMETRIC VIEW.
 2. BAR CHAIRS AT 2000 CRS. EACH WAY UNO.
 3. FOR ≤ 750 THICK SLAB BAR SIZE TO BE N16
 4. FOR > 750 THICK SLAB BAR SIZE TO BE N20

 DENOTES EXISTING STRUCTURE

 DENOTES SURVEY TARGET

- NOTES:**
1. REFER DRAWING No. S001 & S002 FOR STRUCTURAL GENERAL NOTES.
 2. PLAN GRID AND R.L. DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE INDICATIVE ONLY AND REFLECT THE SET-OUT INFORMATION USED DURING DESIGN. REFER GENERAL NOTES AND ARCHITECT'S DRAWINGS FOR FINAL SET-OUT.

GEOTECHNICAL NOTES:

1. THRUST BLOCK EXCAVATIONS SHALL BE DEWATERED. ALL ROCK SURFACES SHALL BE CLEANED OF ALL DEBRIS AND ROUGHENED TO AT LEAST R4 – DEFINED AS “GROOVES OR UNDULATIONS GREATER THAN 10, WIDTH GREATER THAN 10 AT SPACING 50 TO 200”, ORIENTATED PERPENDICULAR TO THE BEAMS.
2. ROCK SHALL BE INSPECTED BY THE AECOM GEOTECHNICAL ENGINEER TO CONFIRM MINIMUM ROUGHNESS REQUIREMENTS HAVE BEEN MET PRIOR TO PLACING CONCRETE.

GROUND ANCHORING & MONITORING NOTES:

1. REFER TO SPECIFICATION FOR GROUND ANCHORING INFORMATION.
2. SURVEY DATA SHALL BE PROVIDED TO THE AECOM GEOTECHNICAL AND STRUCTURAL ENGINEERS TO MONITOR STRUCTURAL MOVEMENTS PRIOR, DURING AND FOLLOWING CONSTRUCTION.
3. THE DESIGN ASSUMES A MINIMUM ANCHOR HOLE DIAMETER IN ROCK OF 210.
4. ALL GROUND ANCHORS SHALL BE LOAD TESTED IN ACCORDANCE WITH SECTION 1.4 OF THE STRUCTURAL ENGINEERING SPECIFICATION WITH THE FOLLOWING TEST LOAD MODIFICATION. ANCHORS SHALL BE STRESSED TO 140% OF THE NOMINATED WORKING LOAD OF 1500KN OR 75% OF THE ULTIMATE ANCHOR CAPACITY, WHICHEVER IS LOWER.
5. AFTER TESTING ANCHORS SHALL BE LOCKED OFF AT 90% OF THE ANCHOR DESIGN WORKING LOAD OF 1500KN

DESIGN ASSUMPTIONS:

1. ULTIMATE DESIGN THRUST FROM EXISTING TIE BEAMS, 7000KN (3500KN PER TIE BEAM).
2. WHERE STAGED CONCRETE WORKS ARE UNDERTAKEN, CONCRETE SURFACE IS TO BE CLEARED OF ALL WATER & DEBRIS AND DELIBERATELY ROUGHENED BY TEXTURING THE CONCRETE TO GIVE A PRONOUNCED PROFILE. MUST ACHIEVE A FRICTION COEFFICIENT OF 0.9 & A COHESION COEFFICIENT OF 0.4 AS PER TABLE 8.4.3 AS3600-2009.
3. THE CONTRACTOR SHALL ENGAGE THE AECOM GEOTECHNICAL ENGINEER TO VERIFY THAT THE ROCK EXPOSED IN THE EXCAVATION IS CLASS II SANDSTONE OR BETTER, (CLASSIFIED IN ACCORDANCE WITH PELLIS ET AL 1998).

FLATJACK NOTES:

1. TO BE UTILISED INITIALLY TO PRELOAD THRUST BLOCK PRIOR TO CUTTING TIE BEAM. TEMPORARY 4X MIN 500DIA FLAT JACKS TO BE INSTALLED INTO 4X POCKETS AND PRELOADED TO 1000KN MAX PER JACK (MAX TOTAL THRUST 4000KN).
2. TO BE UTILISED IN CONJUNCTION WITH MONITORING DATA WHERE REQUIRED BY ADVICE OF STRUCTURAL ENGINEER.

CONSTRUCTION NOTES:

1. CONTRACTOR TO MITIGATE THE EFFECTS OF SHRINKAGE AND THERMAL EFFECTS DUE TO HEAT OF HYDRATION
CONTRACTOR TO SUBMIT PROPOSAL FOR REVIEW AND APPROVAL OF MIX DESIGN, POUR SEQUENCE AND
METHODODOLOGY.

REPORTING & SEQUENCING:

1. BASELINE MONITORING SURVEY.
2. EXCAVATE TO 1.2m.
3. READ & REPORT SURVEY DATA.
4. EXCAVATE TO FINAL LEVEL (2.4m).
5. READ & REPORT SURVEY DATA.
6. READ & REPORT SURVEY DATA EVERY 2 DAYS UNTIL PRESTRESSING HAS BEEN COMPLETE.
7. PREPARE GROUND SURFACE. THE AECOM GEOTECHNICAL ENGINEER IS TO REVIEW SURFACE ROUGHNESS AND TO ADVISE SUITABILITY BEFORE CONTINUATION OF CONSTRUCTION.
8. PLACE REINFORCEMENT.
9. CONCRETING WORKS (POSSIBLY STAGED).
10. INSTALL UPL ANCHORS. BOND LENGTH TO BE INSTALLED. FREE LENGTH TO BE LEFT UNGROUTED.
11. FORMING, COMPACTION & INTERFACE OF THRUST BLOCK UP AGAINST EXISTING TIE BEAM IS CRITICAL. AECOM ENGINEER TO INSPECT AND APPROVE CONCRETE WORKS IN THIS ZONE PRIOR TO PROCEEDING
12. VSL ANCHORS TO BE STRESSED FOLLOWING THRUST BLOCK ACHIEVING 28 DAY STRENGTH. NOTE : ADMIXTURES CAN BE ADDED TO MIX DESIGN TO REDUCE CURING TIME IF REQUIRED. CONTRACTOR TO SUBMIT PROPOSAL TO AECOM FOR REVIEW. PROOF LOAD TEST TO BE COMPLETED AND LOADING REPORT TO BE SENT TO THE AECOM GEOTECHNICAL AND STRUCTURAL ENGINEERS.
13. INJECT TEMPORARY FLAT JACKS INTO FLAT JACK POCKETS AND PRELOAD TO 1000KN MAX PER JACK (MAX TOTAL THRUST 4000KN) OR UNTIL ANY MOVEMENT TRIGGERED OF EXISTING TIE BEAM IN NORTH DIRECTION FROM MONITORING SURVEY TARGET POINTS.
14. READ & REPORT SURVEY DATA.
15. CUT FIRST EXISTING PT BEAM.
16. READ & REPORT SURVEY DATA.
17. CUT SECOND EXISTING PT BEAM.
18. READ & REPORT SURVEY DATA.
19. GROUT FREE END OF PT DUCT IN THRUST BLOCK.
20. GROUT PACK INTERFACE ZONE BETWEEN THRUST BLOCK AND EXISTING TIE BEAMS IF ANY LATERAL MOVEMENT HAS OCCURRED. ENGINEER TO ADVISE AND INSPECT GROUT PACKING LOCATIONS FOLLOWING SITE INSPECTION.
21. CONTINUE REPORTING SURVEY DATA UNTIL CONSTRUCTION WORKS ARE COMPLETE AND THE AECOM GEOTECHNICAL ENGINEER CONCLUDES THAT MAJOR LATERAL MOVEMENT HAS CEASED.
22. REMOVE FLAT JACKS SEQUENTIALLY.
23. ON COMPLETION OF WORKS CONTRACTOR TO INSPECT SUBSTATION AND PROVIDE CONDITION REPORT TO STRUCTURAL ENGINEER FOR REVIEW

ANCHOR MONITORING PROCEDURE:

1. ELECTRICALLY ISOLATED ANCHORS TO BE REMOTE MONITORED.
2. MONITORING CONDUITS TO BE INSTALLED TO ANCHOR HEADS AND LINKED TO A CENTRAL MONITORING TERMINAL LOCATED AT HATCH H2 (ALTERNATE LOCATION AT ANY HATCH H1). CONDUITS TO BE CAST INTO NEW SLAB AND RUN TO THE SAND PUMP HOUSE. LATERAL JACKING MONITORING CONDUITS TO BE INSTALLED TO ELECTRICALLY ISOLATED ANCHORS TO OCCUR MINIMUM EVERY 5 YEARS, AND DATA SUBMITTED TO VSL AND AECOM FOR REVIEW.
3. INSTALL SURVEY POINTS TO MONITOR THRUST BLOCK LATERAL MOVEMENT WITHIN H1 HATCHES. LATERAL MOVEMENT TO BE SURVEYED MINIMUM EVERY 5 YEARS AND DATA SUBMITTED TO AECOM FOR REVIEW. FUTURE PLAT JACKING AND LATERAL JACKING MAYBE REQUIRED IF LATERAL MOVEMENT HAS OCCURRED. ENGINEER TO ADVISE BASED ON 5 YEAR REPORTING DATA.
4. ANCHOR DESIGN LIFE TO BE REVIEWED BY VSL AFTER INITIAL 95 YEARS HAS EXPIRED.

LATERAL MOVEMENT AT INTERFACE OF THRUST BLOCK AND EXISTING STRUCTURE

<3mm	3mm	5-10mm	>10mm
A	B	C	D
-	INFORM THE AECOM GEOTECHNICAL AND STRUCTURAL ENGINEERS AND SEEK INSTRUCTION.	PROCEED WORK UNDER ADVICE FROM THE AECOM GEOTECHNICAL AND STRUCTURAL ENGINEERS AND INCREASE SURVEY REPORTING TO TWICE A DAY.	FLAT JACKING TO BEGIN.



NOT FOR CONSTRUCTION

Issue	Description	Drawn	Apvd	Date
05	ISSUED FOR TENDER	J.T.	J.O.	10.08.2018
04	ISSUED FOR TENDER	E.C.	J.O.	13.07.2018
03	ISSUED FOR TENDER	J.T.	J.O.	26.06.2018
02	ISSUED FOR TENDER	HG	J.O.	02.05.2018
01	ISSUED FOR TENDER	J.T.	J.O.	15.12.2017

SAFETY IN DESIGN INFORMATION

SAFETY IN DESIGN ISSUES HAVE BEEN ASSESSED AS PART OF THE DESIGN PROCESS. ALL REASONABLE STEPS HAVE BEEN TAKEN TO ENSURE HAZARDS AND RISKS NORMALLY ASSOCIATED WITH THIS TYPE OF DESIGN HAVE BEEN MITIGATED AND/OR COMMUNICATED. RESIDUAL HAZARDS AND RISKS AND/OR HAZARDS AND RISKS NOT NORMALLY ASSOCIATED WITH THIS TYPE OF WORK WHICH MAY REQUIRE SUBSEQUENT CONSIDERATION AND/OR ACTION ARE DESCRIBED IN - SOH-FOH SAFETY DESIGN REGISTER MATRIX DOCUMENT

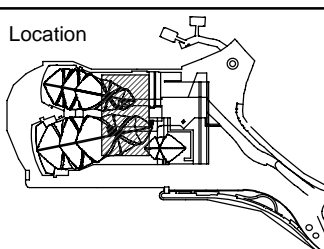
CONSULTANT

AECOM Australia Pty. Ltd.
A.B.N. 20 093 846 925

Level 21, 420 George Street
Sydney, NSW 2000
+61 2 8934 0000 tel +61 2 8934 0001 fax
www.aecom.com

TONKIN ZULAIKHA GREER ARCHITECTS
117 Reservoir Street
ABN: 46002722349
P: (02) 9215 4900
F: (02) 9215 4901
EMAIL info@tztg.com.au
WEB www.tztg.com.au

tonkin
greer
zulaikha
ARCHITECTS



Status	FOR TENDER
--------	------------

Contract	COLL 510
----------	----------

--	--

2011	2012
2013	2014

Project **BUILDING RENEWAL PROGRAM**
ENTRY FOYER ESCALATORS

Title STRUCTURAL

TYPICAL BLOCK DETAILS - QUEST 4

Sydney Opera House Trust
GPO Box 4274 Sydney NSW Australia 200

Phone: +61 2 9250 7541
email: info@sydneyoperahouse.com

Location	Drawing No:
----------	-------------

BX VA | 29 BR AEC

Sheet:	Rev:
--------	------

S015	05	A1
------	----	----