

Appendix A: VGT Surface Sampling

HRT = Haematized F	Rhyolitic T	uff
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Grid			_				
Square	Transect						
No	No	Eastings	Northings	Sampling Comment	s Lithology	Specimen Comment	Transect
1	1	398825	6405175	FNS			
2	1	398875	6405175	FNS			
3	1	398925	6405175	FNS			
4	1	398975	6405175	FNS			
5	1	399025	6405175	FNS			
				Large boulder		Fresh. Creamy - white plagioclase laths, opaque - clear quartz, mica crystals set in a pink - red fine grained matrix. Possibly orthoclase	
6	2	398725	6405125	nearby too. Floats	HRT	and/or haematite.	
7	2	398775	6405125	Outcrop but float	HRT		
8	2	398825	6405125	FNS			
9	2	398875	6405125	FNS			
10	2	398925	6405125	FNS			
11	2	398975	6405125	FNS			
12	2	399025	6405125	FNS			
13	2	399075	6405125	FNS			
14	3	398675	6405075		HRT	Weathered. Plag, Qtz, Mica set in cream - brown matrix.	-
15	3	398725	6405075	Insitu. Hard	HRT	Fresh.	6 1 440
15	5	590725	0403073	Floats, Insitu too		116511.	
16	3	398775	6405075	hard	HRT	Slightly weathered.	
17	3	398825	6405075	FNS			Non-Alth
18	3	398875	6405075				
19	3	398925	6405075	FNS			
20	3	398975	6405075	FNS			
21	3	399025	6405075				
22	3	399075	6405075	FNS			

Grid							
Square	Transect						
No	No	Eastings	Northings	Sampling Comments	Lithology	Specimen Comment	Transect Photo
23	4	398625	6405025		HRT	Slightly weathered.	Addition
24	4	398675	6405025	No Sample. Hard			
25	4	398725	6405025	No Sample. Hard			
26	4	398775	6405025	FNS			
27	4	398825	6405025	FNS			ACCESSION AND A CONTRACTOR OF A
28	4	398875	6405025	No Sample. Float			
29	4	398925	6405025	Float	HRT	Fresh.	
30	4	398975	6405025	FNS			
31	4	399025	6405025	FNS			
32	4	399075	6405025	FNS			
33	4	399125	6405025	FNS			
34	5	398575	6404975		HRT	Weathered.	
35	5	398625	6404975		HRT	Weathered.	
36	5	398675	6404975		HRT	Weathered.	
				Outcrop too hard to			the state of the
				sample. Floaters		(1) Weathered. (2) Slightly	
37	5	398725	6404975	nearby	HRT	weathered, pink - brown matrix	
38	5	398775	6404975	Float	HRT	Fresh.	
				Outcrop too hard to			
				sample. Floaters			
39	5	398825	6404975	nearby	HRT	Fresh.	
40	5	398875	6404975		HRT	Fresh. Minor iron staining.	
						Slightly weathered. Pink - brown	
41	5	398925	6404975		HRT	matrix.	
				Lone sample in			
42	5	398975	6404975	paddock. Float	HRT	Fresh.	
43	5	399025	6404975		HRT	Slightly weathered.	
44	5	699075	6404975	FNS			

Grid							
Square	Transect						
No	No	Eastings	Northings	Sampling Comments	Lithology	Specimen Comment	Transect Photo
45	6	398475	6404925		HRT	Fresh.	
46	6	398525	6404925		HRT	Weathered.	All IN A REAL MARKED
47	6	398575	6404925		HRT	Slightly weathered.	46 CTONA 40 MA MO
48	6	398625	6404925		HRT	Slightly weathered.	C MARINE CONTRACTOR
49	6	398675	6404925		HRT	Weathered.	Alice to a a
						Fresh. Secondary iron staining (silver	
50	6	398725	6404925		HRT	grey)	A CALSON ADDITION
51	6	398775	6404925		HRT	Fresh.	
52	6	398825	6404925		HRT	Slightly weathered.	
53	6	398875	6404925	Floats (?)	HRT	Weathered.	
54	6	398925	6404925		HRT	Slightly weathered.	
				Paddock sample.			
55	6	398975	6404925	Float	HRT	Slightly weathered.	
56	6	399025	6404925	FNS			
57	6	399075	6404925	FNS			

HRT = Haematized Rhyolitic Tuff

Grid							
Square	Transect						
No	No	Eastings	Northings	Sampling Comments	5 Lithology	Specimen Comment	Transect Photo
58	7	398475	6404875	FNS			
59	7	398525	6404875		HRT	Weathered. HRT(?)	
60	7	398575	6404875	Insitu. Hard	HRT	Weathered.	CARE TO AND AND A SHI CORING
61	7	396625	6404875	Float	HRT	Fresh.	
				Float. Outcrop too			
62	7	398575	6404875	hard	HRT	Slightly weathered.	
				(Bit more sediments			
63	7	398726	6404875	in this area). Float	HRT	Fresh.	
				Float. A lot of float			
64	7	398775	6404875	around	HRT	Fresh.	
65	7	398825	6404875		HRT	Fresh.	
66	7	398875	6404875		HRT	Slightly weathered.	
				Float. Steep			
67	7	398925	6404875	paddock.	HRT	Fresh.	
68	7	398975	6404875	FNS			
69	7	399025	6404875	One single boulder	HRT	Slightly weathered.	
70	7	399075	6404875	FNS			
71	7	399125	6404875	FNS			

HRT = Haematized Rhyolitic Tuff

Grid			J				
Square	Transect						
No	No	Eastings	Northings	Sampling Comment	s Lithology	Specimen Comment	Transect Photo
72	8	398475	6404825		HRT	Weathered.	
				(Bit more sediments			
73	8	398525	6404825	in this area). Float	HRT	Slightly weathered.	
74	8	398575	6404825	Insitu. Hard	HRT	Fresh.	
75	8	398625	6404825		HRT	Fresh.	
76	8	398675	6404825		HRT	Fresh.	
				Outcrop Insitu.			Market Market
				Sample is edge off			
77	8	398725	6404825	larger rock	HRT	Slightly weathered.	
				Float. Lots of float			
78	8	398775	6404825	here	HRT	Slightly weathered.	
79	8	398825	6404825		HRT	Slightly weathered.	THE SALAN OF STREET
80	8	398875	6404825		HRT	Slightly weathered.	
81	8	398925	6404825	FNS			
82	8	398975	6404825	Float	HRT	Slightly weathered.	
83	8	399025	6404825	FNS			
84	8	399075	6404825	FNS			
85	8	399125	6404825	FNS			
86	8	399175	6404825	FNS			

HRT = Haematized Rhyolitic Tuff

Grid							
Square	Transect						
No	No	Eastings	Northings	Sampling Comments	Lithology	Specimen Comment	Transect Photo
				Float on grazed			
87	9	398475	6404775	paddock	HRT	Fresh.	
88	9	398525	6404775	Insitu. Hard	HRT	Slightly weathered.	
89	9	398575	6404775		HRT	Fresh.	
							A CALLER AND A CALL YOU
90	9	398625	6404775		HRT	Fresh. With qtz lithic fragment ~2cm	
91	9	398675	6404775		HRT	Fresh. Dense. Silver - grey iron	
92	9	398725	6404775	Hard outcrop. Insitu	HRT	Slightly weathered.	
93	9	398775	6404775		HRT	Slightly weathered.	A TAT 94 AB
94	9	398825	6404775		HRT	Fresh.	
95	9	398875	6404775	FNS			
96	9	398925	6404775		HRT	Fresh.	
97	9	398975	6404775	FNS			
98	9	399025	6404775	FNS			
99	9	399075	6404775	FNS			
100	9	399125	6404775	FNS			
101	9	399175	6404775	FNS			

HRT = Haematized Rhyolitic Tuff

HRT = Haematized Rhyolitic Tuff
Legend: FNS = Found no sample

				HRT = Haematized Rh	yolitic Tuff		
			Legend:	FNS = Found no samp	le		
Grid							
Square	Transect						
No	No	Eastings	Northings	Sampling Comments	Lithology	Specimen Comment	Transect Photo
102	10	398475	6404725	Float in paddock	HRT	Fresh.	
				Float in paddock			
103	10	398525	6404725	gully	HRT	Slightly weathered	
104	10	398575	6404725		HRT	Fresh. Slightly brown, hard	
105	10	398625	6404725		HRT	Slightly weathered	
106	10	398675	6404725		HRT	Fresh. Brown matrix	103
107	10	398725	6404725	Insitu. Steep outcrop	HRT	Fresh. Pink - brown matrix	
108	10	398775	6404725		HRT	Fresh.	
				Insitu. Flake off			
109	10	398825	6404725	edge.	HRT	Fresh.	
				Altered			
110	10	398875	6404725	conglomerate	HRT (?)	Very weathered.	State - A - AL
				Altered sheet flow in			
111	10	398925	6404725	gully, spring	HRT (a) / Qt	z (a) Fresh. (b) Highly weathered Qtzite	116.
112	10	398975	6404725	FNS			
113	10	399025	6404725	FNS			Kangananga Kur Shi Tangananga Kur S
114	10	399075	6404725	FNS			
115	10	399125	6404725	FNS			

Grid No No Eastings Northings Sampling Comments Lthology Specimen Comment Tansect Photo 116 11 398475 6404675 Uttrop HRT Slightly weathered. Brown matrix Tansect Photo 117 11 398575 6404675 HRT Fresh. Brown matrix Fresh. Brown matr				Legena		ріс		
No Eastings Northings Sampling Comments Lithology Specimen Comment Transect Photo 116 11 398475 6404675 Outcrop H RT Slightly weathered. Brown matrix 117 113 398525 6404675 H RT Fresh. Frey - brown matrix 118 11 398625 6404675 H RT Fresh. Rev matrix 120 11 398675 6404675 H RT Fresh. Red matrix 121 13 398675 6404675 H RT Fresh. Tersh. It brown matrix 122 11 398725 6404675 H RT Weathered. 122 11 39875 6404675 H RT Slightly weathered. Red matrix 123 11 39825 6404675 Float. No sample	Grid							
116 11 398475 6404675 Outcrop HRT Slightly weathered. Brown matrix 117 11 398525 6404675 HRT Fresh. Grey - Brown matrix 119 11 398625 6404675 HRT Fresh. Brown matrix 119 11 398625 6404675 HRT Fresh. Norm matrix 120 11 398675 6404675 HRT Fresh. It brown matrix 121 11 39875 6404675 HRT Weathered. 122 11 39875 6404675 FIAT Weathered. Red matrix 122 11 39825 6404675 FIAT Weathered. Red matrix 123 11 39825 6404675 FIAT Slightly weathered. Red matrix 123 11 39825 6404675 FIAT Bightly weathered. Green matrix 126 11 398975 6404675 FNS Image: State S								
117 11 398525 6404675 HRT Fresh. Grey - brown matrix 118 11 398575 6404675 HRT Fresh. Brown matrix 120 11 398675 6404675 HRT Fresh. Brown matrix 121 11 398675 6404675 HRT Fresh. Lt brown matrix 121 11 398725 6404675 HRT Fresh. Lt brown matrix 122 11 398725 6404675 FIOat. No sample	No	No	Eastings	Northings	Sampling Comments	Lithology	Specimen Comment	Transect Photo
118 11 398575 6404675 HRT Fresh. Brown matrix 119 11 398625 6404675 HRT Fresh. Red matrix 120 11 398675 6404675 HRT Fresh. Lt brown matrix 121 11 39875 6404675 HRT Weathered. 122 11 39875 6404675 HRT Weathered. 123 11 398825 6404675 HRT Slightly weathered. Red matrix 124 11 398875 6404675 Float. No sample HRT Slightly weathered. Green matrix 125 11 398925 6404675 Float. No sample HRT Slightly weathered. Green matrix 126 11 399075 6404675 FNS HRT Fresh. 130 12 398475 6404625 FNS HRT Fresh. 131 12 398255 6404625 HRT Fresh. HRT 131 12 398675 6404625 HRT Fresh. HRT 132 12 398675	116	11	398475	6404675	Outcrop	HRT	Slightly weathered. Brown matrix	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	117	11	398525	6404675		HRT	Fresh. Grey - brown matrix	
120 11 398675 6404675 HRT Fresh. Lt brown matrix 121 11 398725 6404675 HRT Weathered. 122 11 398775 6404675 FNS 123 11 398825 6404675 FNS 124 11 398875 6404675 Float. No sample 125 11 398825 6404675 Float. No sample 126 11 398975 6404675 Float. No sample 127 1399025 6404675 FNS 128 11 399025 6404675 FNS 129 11 399025 6404675 FNS 130 12 398525 6404625 FNS 131 12 398525 6404625 HRT Fresh. 132 12 398575 6404625 HRT Fresh. 133 12 398575 6404625 HRT Fresh. 133 12 398675 6404625 HRT Fresh. 133 12 3986	118	11	398575	6404675		HRT	Fresh. Brown matrix	
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	119	11	398625	6404675		HRT	Fresh. Red matrix	COMPANY AND
122 11 398775 6404675 FNS 123 11 398825 6404675 HRT Slightly weathered. Red matrix 124 11 398875 6404675 Float. No sample 125 11 398975 6404675 Float. No sample 126 11 398975 6404675 Float. No sample 127 11 399925 6404675 FNS 128 11 399075 6404675 FNS 129 11 399125 6404675 FNS 130 12 398475 6404625 Float HRT Fresh. 131 12 398525 6404625 Float HRT Fresh. 133 12 398525 6404625 HRT Fresh. 133 12 398575 6404625 HRT Fresh. 133 12 398575 6404625 HRT Fresh. 135 12 398775 6404625 HRT Fresh. 135 12 39875 6404625	120	11	398675	6404675		HRT	Fresh. Lt brown matrix	
123 11 398825 6404675 HRT Slightly weathered. Red matrix 124 11 398875 6404675 Float. No sample 125 11 398925 6404675 Float. No sample 126 11 398925 6404675 Float. No sample 127 11 399025 6404675 FNS 128 11 399025 6404675 FNS 129 11 399125 6404675 FNS 129 11 399125 6404675 FNS 130 12 39825 6404625 FNS 131 12 398575 6404625 HRT Fresh. 133 12 398575 6404625 HRT Fresh. 133 12 398675 6404625 HRT Fresh. 134 12 398675 6404625 HRT Fresh. 135 12 398775 6404625 FNS Image: Fresh. 136 12 398756 6404625 FNS (a) Iron rich Qtz vein (?). (c) I	121	11	398725	6404675		HRT	Weathered.	
124 11 398875 6404675 Float. No sample 125 11 398925 6404675 Float. No sample 126 11 398975 6404675 Float HRT Slightly weathered. Green matrix 127 11 399075 6404675 FNS Image: Stress of St	122	11	398775	6404675	FNS			
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	123	11	398825	6404675		HRT	Slightly weathered. Red matrix	
126 11 398975 6404675 Float HRT Slightly weathered. Green matrix 127 11 399025 6404675 FNS 128 11 399075 6404675 FNS 129 11 399125 6404675 FNS 129 11 399125 6404675 FNS 130 12 39875 6404625 Float HRT Fresh. 131 12 398575 6404625 HRT Fresh. Fresh. 132 12 398575 6404625 HRT Fresh. Fresh. 133 12 398675 6404625 HRT Fresh. Fresh. 134 12 398675 6404625 to sample HRT Fresh. 135 12 398775 6404625 FNS Foat. Insitu too hard Fresh. 135 12 398775 6404625 FNS Foat. No outcrop HRT Fresh. 136 12 398875 6404625 Float. No outcrop HRT Fresh. <td>124</td> <td>11</td> <td>398875</td> <td>6404675</td> <td>Float. No sample</td> <td></td> <td></td> <td></td>	124	11	398875	6404675	Float. No sample			
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	125	11	398925	6404675	Float. No sample			
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	126	11	398975	6404675	Float	HRT	Slightly weathered. Green matrix	
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	127	11	399025	6404675	FNS			
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	128	11	399075	6404675	FNS			
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	129	11	399125	6404675	FNS			The second secon
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	130	12	398475	6404625	Float	HRT	Fresh.	
133 12 398625 6404625 HRT Fresh. 134 12 398675 6404625 to sample HRT Fresh. 134 12 39875 6404625 to sample HRT Fresh. 135 12 39875 6404625 FNS HRT Fresh. 136 12 39875 6404625 Float. No outcrop HRT Fresh. 137 12 398875 6404625 Float. No outcrop HRT Fresh. 138 12 398875 6404625 altered V? HRT (?) 139 12 398975 6404625 Float HRT Fresh. 140 12 398975 6404625 Float HRT Fresh.	131	12	398525	6404625		HRT	Fresh.	Non and A set
134 12 398675 6404625 to sample HRT Fresh. 135 12 398725 6404625 FNS 136 12 398775 6404625 FNS 137 12 398825 6404625 Float. No outcrop HRT Fresh. 138 12 398875 6404625 Float. No outcrop HRT Fresh. 138 12 398875 6404625 altered V? HRT / Qtz HRT (?) 139 12 398925 6404625 Float HRT Fresh. 140 12 398975 6404625 Float HRT Fresh.	132	12	398575	6404625		HRT	Fresh.	
134 12 398675 6404625 to sample HRT Fresh. 135 12 398725 6404625 FNS HRT Fresh. 136 12 39875 6404625 FNS Steep gully. Dull hit / (a) Iron rich Qtz vein (?). (c) Iron rich 138 12 398875 6404625 Float. No outcrop HRT Fresh. 138 12 398875 6404625 altered V? HRT / Qtz HRT (?) 139 12 398975 6404625 Float HRT Fresh. 140 12 398975 6404625 Float HRT Fresh.	133	12	398625	6404625		HRT	Fresh.	Children Children (Children Children Ch
135 12 398725 6404625 HRT Fresh. 136 12 398775 6404625 FNS 137 12 398825 6404625 Float. No outcrop HRT Fresh. 137 12 398875 6404625 Float. No outcrop HRT Fresh. 138 12 398875 6404625 altered V? HRT / Qtz HRT (?) 139 12 398925 6404625 Float HRT Fresh. 140 12 398975 6404625 Float HRT Fresh.					Float. Insitu too haro	1		130 Star to the star
136 12 398775 6404625 FNS 137 12 398825 6404625 Float. No outcrop HRT Fresh. 138 12 398875 6404625 altered V? HRT / Qtz HRT (?) 139 12 398925 6404625 Float HRT Fresh. 140 12 398975 6404625 Float HRT Fresh.	134	12	398675	6404625	to sample	HRT	Fresh.	
137 12 398825 6404625 Float. No outcrop HRT Fresh. 138 12 398875 6404625 altered V? HRT / Qtz HRT (?) 139 12 398925 6404625 Float HRT Fresh. 140 12 398975 6404625 Float HRT Fresh.	135	12	398725	6404625		HRT	Fresh.	
Steep gully. Dull hit / (a) Iron rich Qtz vein (?). (c) Iron rich 138 12 398875 6404625 altered V? HRT / Qtz HRT (?) 139 12 398925 6404625 Float HRT Fresh. 140 12 398975 6404625 Float HRT Fresh.	136	12	398775	6404625	FNS			
138 12 398875 6404625 altered V? HRT / Qtz HRT (?) 139 12 398925 6404625 Float HRT Fresh. 140 12 398975 6404625 Float HRT Fresh.	137	12	398825	6404625	Float. No outcrop	HRT	Fresh.	
139 12 398925 6404625 Float HRT Fresh. 140 12 398975 6404625 Float HRT Fresh.					Steep gully. Dull hit /	/	(a) Iron rich Qtz vein (?). (c) Iron rich	
140 12 398975 6404625 Float HRT Fresh.	138	12	398875	6404625	altered V?	HRT / Qtz	HRT (?)	
	139	12	398925	6404625	Float	HRT	Fresh.	
141 12 399025 6404625 Float HRT Weathered. Green matrix	140	12	398975	6404625	Float	HRT	Fresh.	
	141	12	399025	6404625	Float	HRT	Weathered. Green matrix	
142 12 399075 6404625 Float HRT Slightly weathered	142	12	399075	6404625	Float	HRT	Slightly weathered	
143 12 399125 6404625 Insitu. HRT Fresh. Brown matrix	143	12			Insitu.	HRT		
144 12 399175 6404625 Float HRT Fresh	144	12	399175	6404625	Float	HRT	Fresh	

HRT = Haematized Rhyolitic Tuff

Grid						
	Transect					
No	No	Eastings	Northings	Sampling Comments	Lithology	Specimen Comment
145	13	398475	6404575	Float	HRT	Fresh. Brown matrix
146	13	398525	6404575		HRT	Fresh. Brown matrix
147	13	398575	6404575		HRT	Fresh.
148	13	398625	6404575		HRT	Slightly weathered.
149	13	398675	6404575	Outcrop. Insitu	HRT	Fresh
150	13	398725	6404575		HRT	Fresh
151	13	398775	6404575	FNS		
152	13	398825	6404575	FNS		
153	13	398875	6404575	FNS		
154	13	398925	6404575	FNS		
155	13	398975	6404575	FNS		
156	13	399025	6404575	Large outcrop	HRT	Fresh
157	13	399075	6404575	Outcrop and float	HRT	Fresh
				Float - ridge top /		
158	13	399125	6404575	saddle	HRT	Fresh
159	14	398475	6404525	Float	HRT	Slightly weathered. Brown matrix
160	14	398525	6404525	Float	HRT	Fresh. Brown matrix
161	14	398575	6404525		HRT	Fresh.
162	14	398625	6404525	Float. No sample		
163	14	398675	6404525	Outcrop. Insitu	HRT	Fresh. Brown matrix
164	14	398725	6404525		HRT (?)	Very weathered. Volcanic (?)
165	14	398775	6404525		HRT	Very - extreme weathered
166	14	398825	6404525	FNS		
				Float. No sample		
167	14	398875	6404525	(Volcanics)		
				Insitu. Volcanic		
168	14	398925	6404525	conglomerate	VCG	Volcanoclastic conglomerate
169	14	398975	6404525	FNS		
						(a) Fresh. Brown matrix (b) Fine
						grained volcanic, qtz crystals in grey
170	14	399025	6404525	Insitu	HRT	matrix. (c) Weathered
1/0	14	222072	0404525	IIISILU		

			Legena		pic		
Grid							
Square	Transect						
No	No	Eastings	Northings	Sampling Comments	Lithology	Specimen Comment	Transect Photo
171	15	398525	6404475		HRT	Weathered. Light brown matrix	
172	15	398575	6404475		HRT	Fresh. Brown matrix	A
173	15	398625	6404475		HRT	Slightly weathered. Brown matrix	
174	15	398675	6404475		HRT	Fresh. Brown matrix	A ASA ASA DO DO DO
175	15	398725	6404475	Float (?)	HRT	Fresh.	
176	15	398775	6404475		HRT	Weathered. Brown matrix	2100 100
177	15	398825	6404475	FNS			
178	15	398875	6404475	FNS			
179	15	398925	6404475	FNS			The second second
180	16	398625	6404425	FNS			
181	16	398675	6404425	Float, no outcrop	HRT	Weathered. Brown matrix	10
182	16	398725	6404425	Insitu	HRT	Fresh.	
183	16	398875	6404475	FNS			

HRT = Haematized Rhyolitic Tuff



Appendix B: VGT Test Pit Logs

Hillview Hard Rock Quarry: Test Pit Sampling Tables

Test Pit 2							
Eastings	399060.57		Elevation	108m			
Northings	6404597.77	7	Date	17.07.2024			
Logged By	TF		Checked	RG			
Depth	1.2		Rock at base?	Volcanic, extremely weathered			
From	То	Thickness	Description				
0	0.3m	0.3m	A1 soil horizon, brown clay				
0.3m	1.2m	0.9m	B1 soil horizon, grey brown sandy clay				
1.2m		Extremely weathered volcanic rock at base, end of hole.					



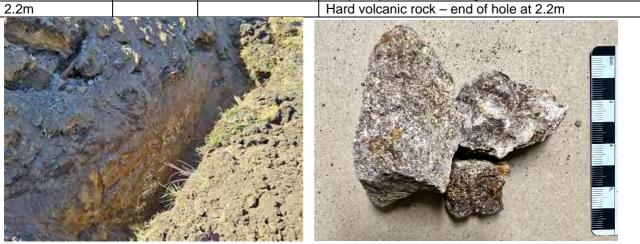
Test pit 2

Tost Dit 5

Test Fit 5					
Eastings	398915.09	9	Elevation	106.5m	
Northings	6404611.8	38	Date	17.07.2024	
Logged By	TF		Checked	RG	
Depth	0.5		Rock at base?	Volcanic, fresh	
From	To	Thickness	Description		
0	0.13m	0.13m	Dark brown clay/sand		
0.13m	0.5m	0.27m	B1 soil horizon, grey brown sandy clay		
0.5m			Fresh, red volcan	ic rock – end of hole 0.5m	



Test Pit 6						
Eastings	398954.77	7	Elevation	101m		
Northings	6404643.3	31	Date	17.07.2024		
Logged By	TF		Checked	RG		
Depth	2.2		Rock at base?	Volcanic, slightly weathered		
From	То	Thickness	Description			
0	1.0m	1.0m	A1 soil horizon, d	A1 soil horizon, dark brown sandy clay		
1.0m	2.2m	1.2m	Grey, silty and ha	Grey, silty and hard with green rock		
2.2m			Hard volcanic roc	Hard volcanic rock – end of hole at 2.2m		



Test Pit 7					
Eastings	398921.42	2	Elevation	108.2m	
Northings	6404698.4	42	Date	17.07.2024	
Logged By	TF		Checked	RG	
Depth	1.4		Rock at base?	Volcanic, moderately weathered	
From	То	Thickness	Description		
0	0.7m	0.7m	Dark brown high quality soil		
0.7m	1.4m	0.7mm	Yellow/grey clay		
1.4m			Hard volcanic rock – end of hole 1.4m		



Test Pit 8				
Eastings	398961.0	9	Elevation	101.8m
Northings	6404690.	73	Date	17.07.2024
Logged By	TF		Checked	RG
Depth	1.5		Rock at base?	Volcanic, highly weathered
From	То	Thickness	Description	
0	0.3m	0.3m	Dark brown muddy soil	
0.3m	1.5m	1.2m	Yellow/grey clay	
1.5m		Weathered rock, End of hole at 1.5m		





Test Pit 10						
Eastings	398978.86		Elevation	106.43m		
Northings	6404783.10		Date	17.07.2024		
Logged By	TF		Checked	RG		
Depth	1.8		Rock at base?	Volcanic, slightly weathered		
			·			
From	То	Thickness	Description			
0	0.3m	0.3m	Dark brown mud, A1 soil horizon			
0.3m	1.5m 1.2m		Yellow/grey clay			
1.5m	1.8m 0.3m		Hard volcanic rock, end of hole 1.8m			



Test Pit 13					
Eastings	399026.2	2	Elevation	107.04m	
Northings	6404872.	50	Date	17.07.2024	
Logged By	TF		Checked	RG	
Depth	1.1m		Rock at base?	Volcanic, highly weathered	
From	То	Thickness	Description		
0	1.1m	1.1m	Grey / brown clay		
1.1m		Weathered rock, End of hole			





Test Pit 17					
Eastings	399100.70)	Elevation	103.41m	
Northings	6404645.2	24	Date	17.07.2024	
Logged By	TF		Checked	RG	
Depth	0.6m		Rock at base?	Volcanic, moderately weathered	
_					
From	То	Thickness	Description		
0	0.15m	0.15m	A1 soil horizon, d	lark brown clay	
0.15m	0.35m	0.2m	A2 – grey brown clay		
0.35m	0.55m	0.2m	B1 horizon, orange sandy clay		
0.55m	0.6m	0.05m	Rock at base, en	d of hole 0.6m	
	1				



Test Pit 20					
Eastings	398983.2	8	Elevation	110m	
Northings	6404521.	67	Date	17.07.2024	
Logged By	TF		Checked	RG	
Depth	3.2m		Rock at base?	Volcanic, extremely weathered	
	-				
From	То	Thickness	Description		
0	0.5m	0.5m	A1 soil horizon, d	lark brown clay	
0. 5m	2.0m	1.5m	Sandy orange bro	own clay	
2.0m	3.2m	1.2m	Increasing sand in clay		
3.2m			Equipment refusal, extremely weathered rock at		
			base, end of hole	e 3.2m	



Test Pit 21

I COL FIL ZI						
Eastings	399013.0	5	Elevation	106.55m		
Northings	6404564.0	65	Date	17.07.2024		
Logged By	TF		Checked	RG		
Depth	1.6m		Rock at base?	Volcanic, extremely weathered		
From	То	Thickness	Description			
0	0.4m	0.4m	Brown sandy clay, A1 soil horizon			
0.4m	1.4m	1.0m	Brown/grey streaky clay			
1.4m	1.6m	0.2m	Pale grey clay			
1.6m			Extremely weather	Extremely weathered rock at base, end of hole 1.6m		



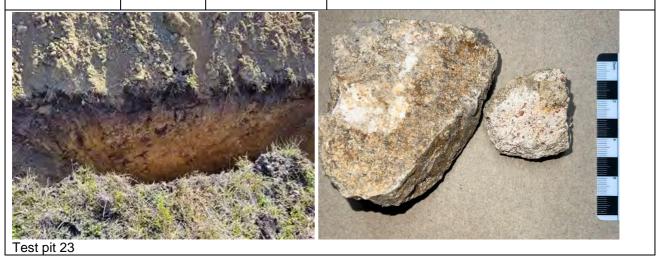
Test Pit 22					
Eastings	399006.47		Elevation	100.34m	
Northings	6404627.59		Date	17.07.2024	
Logged By	TF		Checked	RG	
Depth	2.4m		Rock at base?	Volcanic, extremely weathered	
-	- -			· · ·	
From	То	Thickness	Description		
0	0.3m	0.3m	Brown sandy clay, A1 soil horizon		
0.3m	1.9m	1.6m	Brown / orange clay	Brown / orange clay	
1.9m	2.4m	0.5m	Extremely weathered	ed & soft volcanic rock	
2.4m			Equipment refusal, end of hole		
			6		



Test Pit 23	
-------------	--

Eastings	398918.75	Elevation	110.33m
Northings	6404556.60	Date	17.07.2024
Logged By	TF	Checked	RG
Depth	2.5m	Rock at base?	Volcanic, Moderately
			weathered

From	То	Thickness	Description
0	0.4m	0.4m	Dark brown sandy soil, A1 soil horizon
0.4m	2.5m	2.1m	Brown / orange clay
2.5m			Volcanic rock at 2.5m, end of hole



Test Pit 24				
Eastings	399064.2	9	Elevation	103.75m
Northings	6404939.	68	Date	17.07.2024
Logged By	TF		Checked	RG
Depth	1.2m		Rock at base?	Volcanic, slightly weathered
From	То	Thickness	Description	
0	0.5m	0.5m	Dark brown good quality soil, A1 soil horizon	
0.5m	1.2m	1.2m 0.7m Red / brown clay		
1.2m			Very hard volcani	ic rock at 1.2m, end of hole



Test Pit 25					
Eastings	399185.14	1	Elevation	95.82m	
Northings	6404861.9	94	Date	17.07.2024	
Logged By	TF		Checked	RG	
Depth	1.1m		Rock at base?	Volcanic, fresh	
From	To	Thickness	Description		
0	0.5m	0.5m	Dark brown soil, /	Dark brown soil, A1 soil horizon	
0.5m	1.0m	0.5m	Red / brown clay	Red / brown clay	
1.0m	1.1m	0.1m	Hard, red volcani	c rock at 1.1m, end of hole	



Test Pit 26				
Eastings	399164.73	3	Elevation	102.22m
Northings	6404798.7	70	Date	17.07.2024
Logged By	TF		Checked	RG
Depth	1.05m		Rock at base?	Volcanic, moderate weathering
From	То	Thickness	Description	
0	0.3m	0.3m	Dark brown soil, A1 soil horizon	
0.3m	0.6m	0.3m	Brown clay	
0.6m	1.05m	0.45m	Sandy clay	
1.05m		Hard rock at 1.05m, end of hole		





Test Pit 27

Test Pit 27				
Eastings	399164.1	8	Elevation	111.31m
Northings	6404635.2	28	Date	17.07.2024
Logged By	TF		Checked	RG
Depth	0.9m		Rock at base?	Volcanic, fresh
From	То	Thickness	Description	
0	0.3m	0.3m	Dark brown soil, /	A1 soil horizon
0.3m	0.7m	0.4m	Red/brown streaky clay	
0.7m	0.9m	0.2m	Weathered red vo	olcanic rock
0.0			المعاممة المعال	



Test Pit 28					
Eastings	399136.57	,	Elevation	114.79m	
Northings	6404585.8	8	Date	17.07.2024	
Logged By	TF		Checked	RG	
Depth	0.9m		Rock at base?	Volcanic, slightly weathered	
From	То	Thickness	Description		
0	0.3m	0.3m	Dark brown soil, A1 soil horizon		
0.3m	0.7m	0.4m	Grey clay	Grey clay	
0.7m	0.9m	0.2m	Weathered red volcanic rock		
0.9m			Hard red rock at 0.9m, end of hole		





Test Pit 29				
Eastings	399040.	60	Elevation	98.47m
Northings	6404662	2.08	Date	17.07.2024
Logged By	TF		Checked	RG
Depth	2.1m		Rock at base?	Volcanic, extremely weathered
From	То	Thickness	Description	
-				

From	10	Inickness	Description
0	0.4m	0.4m	Grey / brown sandy soil, A1 soil horizon
0.4m	2.1 m	1.7m	Red / orange and brown clay
1.7m			Weathered red volcanic rock, end of hole 1.7m



Test Pit 30				
Eastings	399025.27		Elevation	111.92m
Northings	6404526.61		Date	17.07.2024
Logged By	TF		Checked	RG
Depth	1.1m		Rock at base?	Volcanic, Fresh
From	То	Thickness	Description	
0	0.4m	0.4m	Dark brown soil, A1	
0.4m	1.1 m	0.7m	Sandy orange grey	clay
1.1m			Weathered red volc	anic rock, end of hole 1.1m
Test pit 30				



Appendix C: VGT Drill Logs

Plan of:		Hillvie	ew Qua	rry - Dr	rill Hole Lo	og (DDH2) 0-5m	L	_ocation:	Off M	aytoms Lane, Booral, NSW		Source:	Drill logs and Qual	test reports		Plan By:	то
Figure:		xxxx							Council:	Grea	Lakes Shire Council		Survey:	Not applicable			Project Manager:	GT
Version/ Date:	1	V4 3/0)4/2023					T	Fenure:	N/A			Projection:	Not applicable		l		
Our Ref	:	1195_	HV_DD	H2 Log	J_P1			C	Client:	Trico	n Mining Equipment Pty Ltd		Contour Interval:	Not applicable				
													50					
		OLE L lole: DI				Date	l lview e Logged: (<i>i</i> ct ¹						
-		ber: 3430				Date	e Commen e Complete	ed: 13/12	2/2016		v	gu						
		-	Equipme ssessme				face RL: 18 ordinates: E		HD 2.97 N:6404	803.67		Environmental Compliance Solutions						
	ion: Off ed by: N		s Lane, I	Booral		Drill	ing Contra	ctor: D a	and E Drilling	g		1 of21						
Drilli Meth	ng El	levation (RL)	Depth (m)	Core Recovery (%)	Diametral Strength (Mpa)	Axial Strength (Mpa)	UCS Strength (Mpa)	racture equency /m	Fracture Log	Graphic Log	Lithological Description	Additional Informatior						
			0	- B			<u> </u>	<u>ч</u> Е		D.D.	Hematized Rhyolitic Tuff-	Becoming						
A					0.74	0.76	-			D D.	brown,grey to pink, weathered, porphrytic	competer	t	1+0	A COLORED OF THE OWNER			
u g e				60				2		· ^ . 4				and to be		(C		
r	190									۵. d.				1 C			4	721
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				55				0		A . A .							and the second	
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										D . A .								
				100				3		D .A.								
				100				5		DA								
	186	_								A A A				<u>Qualtest R</u>	<u>esource Matei</u>	rial Testin	<mark>g Assessm</mark>	<u>ient Extra</u>
			4				-			·						_	Table 24 –	Material Suito
					8.95	8	215			A A				1.000				1.000
				95				3		DA	Hematized Rhyolitic Tuff-			Sample Number	Identification	Durable	Concrete Aggregate	Concrete Sand
	185		5							A A A A A	dark red to pink, porphrytic			DDH2	Hematized Rhyolitic Tuff/ Ryhodacite (ii)	Yes	Yes (i)	Yes (i)
		nation:		nave be	en sourced	from field	observatio	ons and	geotechnic		cted and provided by Qualtest Labo	pratories.		DDH2 S-2	Rhyolitic Tuff Trachyte (ii)	Yes	Yes (i)	Yes (i)
*: Sul	o Vertic	al Joint	n multiple								,							
Prep	ared B	y: MA/SI	K		rvey Data	Chec	ked By: S	K			Version: 3430_HQ_GA_DDH	I2_V3		(Provided that the app deleterious alkali-silica	reactivity.		
															The Rhyolitic Tuff/ Rhyo suitable boulders are p		ely to be suitable	for dimension





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Potential Product Suitability											
e	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill						
	Yes	Yes	Yes	Yes	Yes						
1	Yes	Yes	Yes	Yes	Yes						

engineering design to take into account the potential for mild or slow

on stone and marine armour rock, if defect spacing is minimal and

Plan	of:	Hillvi	ew Qua	rry - Dri	ill Hole Lo	og (DDH2)) 5-10m	Location:	Off	Maytoms Lane, Booral, NSW		Source:	D	rill logs and Qualt	est reports		Plan By:	то
Figur	e:	xxxx						Council:	Grea	at Lakes Shire Council		Survey:	N	lot applicable			Project Manager:	GT
Versi Date:		V4 4/0	04/2023					Tenure:	N/A		-	Projection	1: N	lot applicable				
Our R	Ref:	1195_	HV_DD	H2 Log	_P2			Client:	Trico	on Mining Equipment Pty Ltd		Contour Interval:		lot applicable				
Ni Pr Cli Pr Lo	OREH(ame of H oject Numb ient: Tricon oject: Geol ocation: Off ogged by: M rilling Ele	ole: D ber: 3430 Mining ogical A Maytom IA/SK	DH2) Equipme ssessme s Lane, I	nt	Diametral Strength (Mpa)	Date Date Date Surf Coo		08/12/2016 3/12/2016 5 AHD 672.97 N:6404 D and E Drilling		Lithological Description	Environmental Compliance Solutions 2 of 21 Additional Information							
	184 -		6 -	95	7.08	7.59	6		4 7 4 4 4 7 4 7 4 7 4 7 4 7 4 7 4 7 4 7					Part of the second seco				
	Diamond Drill 581		7 -	95					0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.									
	182 -		8 -	95	8.67	5.75	6		A A A A A A A A A A					Qualtast Pr	esource Mater	tial Tostin		Nont Extra
	181 -		9 -	95			6*		D.A.	Dyke- grey to dark grey, aphanitic	Pyrite crystallisatic	on	/	Sample Number	Identification	Durable		Material Suite Concrete Sand
	180 -	_	10						D .A.	Hematized Rhyolitic Tuff-				DDH2	Hematized Rhyolitic Tuff/ Ryhodacite (ii)	Yes	Yes (i)	Yes (i)
Da *: >1	Sub Vertica I: Rubble z	d within al Joint one with	multiple		en sourced			nd geotechnica		lected and provided by Qualtest Labor				DDH2 S-2	Rhyolitic Tuff Trachyte (ii) rovided that the app	Yes	Yes (i)	Yes (i)
Pr Lo	epared By g updated 03	: MA/SI 8/04/2023	∧ following	ADW Sur	vey Data	Chec	ked By: SK			Version: 3430_HQ_GA_DDH2	۷_۷3			d (ii) Ti	eleterious alkali-silica ne Rhyolitic Tuff/ Rhyo vitable boulders are p	reactivity. odacite are lik		





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Potential Product Suitability											
e	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill						
	Yes	Yes	Yes	Yes	Yes						
	Yes	Yes	Yes	Yes	Yes						

engineering design to take into account the potential for mild or slow

on stone and marine armour rock, if defect spacing is minimal and

Pla	n of:	Hillview Qua	arry - Dr	ill Hole L	.og (DDH	l2) 10-15m	Location:	Off Maytoms Lane, Booral, NSW		Source:	Drill logs and Qualtest reports	Plan By:	то
Fig	ure:	xxxx					Council:	Great Lakes Shire Council		Survey:	Not applicable	Project Manager:	GT
Ver Dat	sion/ e:	V4 4/04/2023	3				Tenure:	N/A		Projection:	Not applicable		_
Oui	Ref:	1195_HV_DI	DH2 Log	_P3			Client:	Tricon Mining Equipment Pty Ltd		Contour Interval:	Not applicable		
	Name of H Project Num Client: Tricor Project: Geo Location: Off Logged by: N	n Mining Equipmo ological Assessme ff Maytoms Lane, MA/SK	ent Booral		Date Date Date Surfa Coor Drilli	ing Contractor: D a	12/2016 2/2016 ID 2.97 N:6404803.67 nd E Drilling	V	Environmental Compliance Solutions 3 of 21				
	Drilling E Method	Elevation Depth (RL) (m)	Core Recovery (%)	Diametral Strength (Mpa)	Axial Strength (Mpa)	UCS Strength (Mpa) Fracture Frequency	Fracture Graphic Log Log	Lithological Description	Additional Information				
				9.54	5.28		DA	dark red to pink, porphrytic				and a stand of the	and and the
	179	11	95	4.51	2.85	4*		Hematized Rhyolitic Tuff- dark red to pink, porphrytic Hematized Rhyolitic Tuff-	Feldspathic veining Plagioclase alteration				
	178 III O 7	12						weathered, porphrvtic Dyke- grey to dark grey, aphanitic Hematized Rhyolitic Tuff- dark red to pink, porphrytic	Some pyrite crystallisation Some weathering on upper contact				ar (13) 1 m/ (
	Diamond Drill	— 13	100			7		Hematized Rhyolitic Tuff-	Feldspathic veining				

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Hematized Rhyolitic Tuff-dark red, porphrytic

Hematized Rhyolitic Tuff-

Version: 3430_HQ_GA_DDH2_V3

Δ. 4. D ·D - 0 4.

D

1*

>6

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories.

Checked By: SK

95

95

9.65

8.76

14

15

176

175

Other information:

*: Sub Vertical Joint

Prepared By: MA/SK

>1: Rubble zone with multiple joints

Log updated 03/04/2023 following ADW Survey Data

Qualtest Resource Material Testing Assessment Extract

			J		Potenti	al Product Suite	ability		
Sample Number	Identification	Durable	Concrete Aggregate	Concrete Sand	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill
DDH2	Hematized Rhyolitic Tuff/ Ryhodacite (ii)	Yes	Yes (i)	Yes (i)	Yes	Yes	Yes	Yes	Yes
DDH2 S-2	Rhyolitic Tuff Trachyte (ii)	Yes	Yes (i)	Yes (i)	Yes	Yes	Yes	Yes	Yes

(i) Provided that the appropriate precautions are taken in mix and engineering design to take into account the potential for mild or slow

deleterious alkali-silica reactivity.

suitable boulders are procured.

4/30 Glenwood Drive, Thornton NSW 2322 PO Box 2335, Greenhills NSW 2323 ph: (02) 4028 6412 email: mail@vgt.com.au www.vgt.com.au ABN: 26 621 943 888 VGT Environmental Compliance Solutions Pty Ltd



This figure may be based on third party data which has not been verified by vgt and may not be to scale. Unless expressly agreed otherwise, this figure is intended as a guide only and vgt does not warrant its accuracy.



(ii) The Rhyolitic Tuff/ Rhyodacite are likely to be suitable for dimension stone and marine armour rock, if defect spacing is minimal and

lan of	:	Hill	view Q	uarry -	Drill Hole	e Log (DI	OH2) 15-20m	Loca	tion:	Off Maytoms Lane, Booral, N	ISW		Source:	Drill logs and Qualt	est reports		Plan By:	то
igure:		xx	xx					Cou	ncil:	Great Lakes Shire Council			Survey:	Not applicable			Project Manager:	GT
ersior ate:	ר/	V4 -	4/04/20	23				Tenu	re:	 N/A			Projection:	Not applicable		L		
ur Re	f:	119	5_HV_	DDH2 L	og_P4			Clier	it:	Tricon Mining Equipment Pty	[,] Ltd		Contour Interval:	Not applicable				
Nal Proj Clie Proj Loc Log	me of ect Nui nt: Tric ect: Ge ation: C ged by	Hole: E mber: 343 on Mining cological J Off Maytor : MA/SK Elevation	DDH2 30 g Equipm Assessm ms Lane,	ent Booral	Diametral Strength (Mpa)	Date Date Date Surf Coo Drilli	Iview Logged: 03/01/20 Commenced: 08/ Completed: 13/12 acce RL: 189.85 AF rdinates: E:398672 ng Contractor: D a Son the form of the form of the form Son the form of the form of the form Son the form of the form of the form of the form of the form Son the form of the form	12/2016 2/2016 ID 2.97 N:6404 nd E Drilling Fracture	Graphic	Lithological Description	Environmental Compliance Solutions 4 of 21 Additional							
wier	thod	(RL)	(m)				Stre Stre (N Frec	Log	Log	orange, weathered, friable,	Information							
					3.54	3.09			A 4 D A	porphrytic				Call of	4	Res Marries		
	174		10	95			6		0 0 0 0 0	Hematized Rhyolitic Tuff- dark red to pink, porphrytic,			\backslash					131 M (C
			- 16	85			>6		Q. D. D.	Hematized Rhyolitic Tuff- orange, weathered, friable, porphrytic								
	173		- 17		6.79	5.54				Dyke- grey to dark grey, aphanitic	Some pyrite crystallisation							t
Diamond Drill	172			95			7		D. D					e I+				Le in
			- 18	85	10.39	7.26	6		00000	Hematized Rhyolitic Tuff- dark red to pink, porphrytic	Some feldspathic veining		/		E. L.			Anna an
	171		- 19						0.0.0			-		Qualtest R	esource Mater	rial Testin	-	nent Extra Material Suita
				95			7		D D D	Hematized Rhyolitic Tuff- grey to dark red, porphrytic			/					
	170		_ 20						0 0 0 0	Hematized Rhyolitic Tuff- dark red to pink, porphrytic				Sample Number	Identification Hematized Rhyolitic Tuff/	Durable	Concrete Aggregate	Concrete Sand
Data	a inclu	rmation ded within	: n this log	have bee	en sourced	from field of	bservations and	geotechnica		cted and provided by Qualtest Labora	atories.			DDH2	Ryhodacite (ii) Rhyolitic Tuff	Yes	Yes (i)	Yes (i)
*: S >1: Pre	ub Ver Rubble pared	tical Joint zone wi By: MA /S	t th multip SK				ked By: SK			Version: 3430_HQ_GA_DDH2				d (ii) T	Trachyte (ii) rovided that the app eleterious alkali-silica he Rhyolitic Tuff/ Rhyo uitable boulders are p	a reactivity. odacite are like		





<u>act</u>

Potential Product Suitability											
Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fil							
Yes	Yes	Yes	Yes	Yes							
Yes	Yes	Yes	Yes	Yes							

engineering design to take into account the potential for mild or slow

on stone and marine armour rock, if defect spacing is minimal and

:	Hillvie	w Quar	ry - Dril	l Hole	Log (DDH	l2) 20-25m	Lo	cation:	Off Maytoms Lane, Booral	, NSW	Source:	Drill logs and Qual	test reports		Plan By:	то
	xxxx						Co	uncil:	Great Lakes Shire Council	I	Survey:	Not applicable			Project Manager:	GT
ı/	V1 4/04	4/2023					Тег	nure:	N/A		Projection:	Not applicable		l		
f:	1195_⊦	IV_DDI	H2 Log_	_P5			Cli	ent:	Tricon Mining Equipment F	^S ty Ltd	Contour Interval:	Not applicable				
OREHO		DG			Hillv	iew										
ame of H oject Numb ent: Tricon oject: Geolo cation: Off gged by: M	ole: DD ber: 3430 Mining Ec ogical Ass Maytoms	H2 quipment essment	oral		Date Lo Date Co Date Co Surface Coordir	ogged: 03/01/ ommenced: 0 ompleted: 13/ e RL: 189.85 / nates: E:3986 Contractor: D	8/12/2016 12/2016 AHD 72.97 N:640		V	Environmental Compliance Solutions 5 of 21		C Lag				Antopolicies
	evation [(RL)	epth e (m) S	Recovery (%) Diametral	Strength (Mpa)	Axial Strength (Mpa) IICS	Strength (Mpa) Fracture Frequency	Fracture	Graphic Log	Lithological Description	Additional Information		<u> </u> c +		4		le le
		8	30 ().29	0.17	8	VVV		Dyke- grey to dark grey, aphanitic	Dyke, some pyrite crystallisation, weathered at base			2. 1			
169 -		21 —												× (0)		
168 -		22 —	90			7*		0 0 0 0 0	Hematized Rhyolitic Tuff-							
iond Drill		9	90	1.82	9.71	4*		0 0 0 0 0 0	dark red to pink. porphrytic					7	24,4	
Diamond Diamond		23 —									/			1		X
166 -		24 —	95			11			Hematized Rhyolitic Tuff- grey to dark red, porphrytic			Qualtest R	esource Mater	rial Testin		
		9	95			4*			Hematized Rhyolitic Tuff- dark red to pink, porphrytic			Sample Number	Identification	Durable	Table 24 – Concrete Aggregate	Material Suito
165 -		25	<u></u>	9.44	9.72 2	209					/	DDH2	Hematized Rhyolitic Tuff/ Ryhodacite (ii)	Yes	Yes (i)	Yes (i)
ta includeo Sub Vertica : Rubble zo	d within th al Joint one with r	•		ourced fr			d geotechnie	cal data collec	ted and provided by Qualtest Labo			DDH2 S-2	Rhyolitic Tuff Trachyte (ii)	Yes	Yes (i)	Yes (i)
epared By g updated 03	7: MA/SK 8/04/2023 fo	llowing AL	W Survey	Data	Checked	1 By: SK			Version: 3430_HQ_GA_DDH	2_V3		c (ii) T	Provided that the app deleterious alkali-silica he Rhyolitic Tuff/ Rhyo uitable boulders are p	reactivity. odacite are like		





<u>ract</u>

Potential Product Suitability											
e	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill						
	Yes	Yes	Yes	Yes	Yes						
1	Yes	Yes	Yes	Yes	Yes						

engineering design to take into account the potential for mild or slow

on stone and marine armour rock, if defect spacing is minimal and

n of:	Hillvie	w Quar	ry - Dı	ill Hole I	.og (DDI	H2) 25-3(0m	Loca	ion:	Off Mayto	oms Lane, B	ooral, NS	SW		Source:	Drill logs and Qua	Itest reports		Plan By:	ТО
ure:	xxxx							Coun	cil:	Great Lak	kes Shire Co	ouncil			Survey:	Not applicable			Project Manager:	GT
sion/ e:	V4 4/0	4/2023						Tenu	e:	N/A					Projection:	Not applicable				
r Ref:	1195_1		H2 Log	_P6				Clien	t:	Tricon Mir	ning Equipm	nent Pty L	.td		Contour Interval:	Not applicable				
BOREH Name of Project Nul Client: Tric Project: Ge Location: C Logged by Drilling Method	Hole: Dl mber: 3430 on Mining cological A Off Maytom : MA/SK	DH2) Equipmer ssessmer s Lane, E	nt ooral	Diametral Strength (Mpa)	Date Date Date Surf Coo Drilli	Liview e Logged: 0 e Commence complete face RL: 18 rrdinates: E ing Contract S	ced: 08/1 ed: 13/12 39.85 AH E:398672 ctor: D ar	2/2016 /2016 D .97 N:6404			Lithological Description		6 of 21 Addition Calcite veining	n						
164		26 -	90				>5		7 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Hematiz dark red Dyke- g	red Rhyolitic to pink, por grey to dark aphanitic	phrytic	Dyke, sor pyrite crystallisat weathered base	ion,						
Diamond Drill		27	90	6.05	4.15		6												NY NY	
161		28 -	75 -	11.85	7.32		3			Hematiz dark red	red Rhyolitic to pink, por	: Tuff- phrytic	Weathere base	d at		Qualtest R	Resource Mater	rial Testir	ng Assessm	nent Extra
							-												Table 24 –	Material Sui
			90				7	<	0. D. 0.						/	Sample Number	Identification	Durable	Concrete Aggregate	Concrete Sand
160		30						<u> </u>	0.0							DDH2	Hematized Rhyolitic Tuff/ Ryhodacite (ii)	Yes	Yes (i)	Yes (i)
Other info Data incluo *: Sub Vert >1: Rubble	ded within tical Joint	-		n sourced	from field	observatio	ons and g	eotechnic	al data colle	ected and prov	vided by Qual	test Labora	atories.			DDH2 S-2	Rhyolitic Tuff Trachyte (ii)	Yes	Yes (i)	Yes (i)
Prepared Log updated	By: MA/SI 1 03/04/2023	K following /	ADW Sur	vey Data	Chec	ked By: Sl	К			Versior	n: 3430_HQ_(GA_DDH2_	_V3			(ii)	Provided that the app deleterious alkali-silico The Rhyolitic Tuff/ Rhy suitable boulders are	a reactivity. odacite are lik		

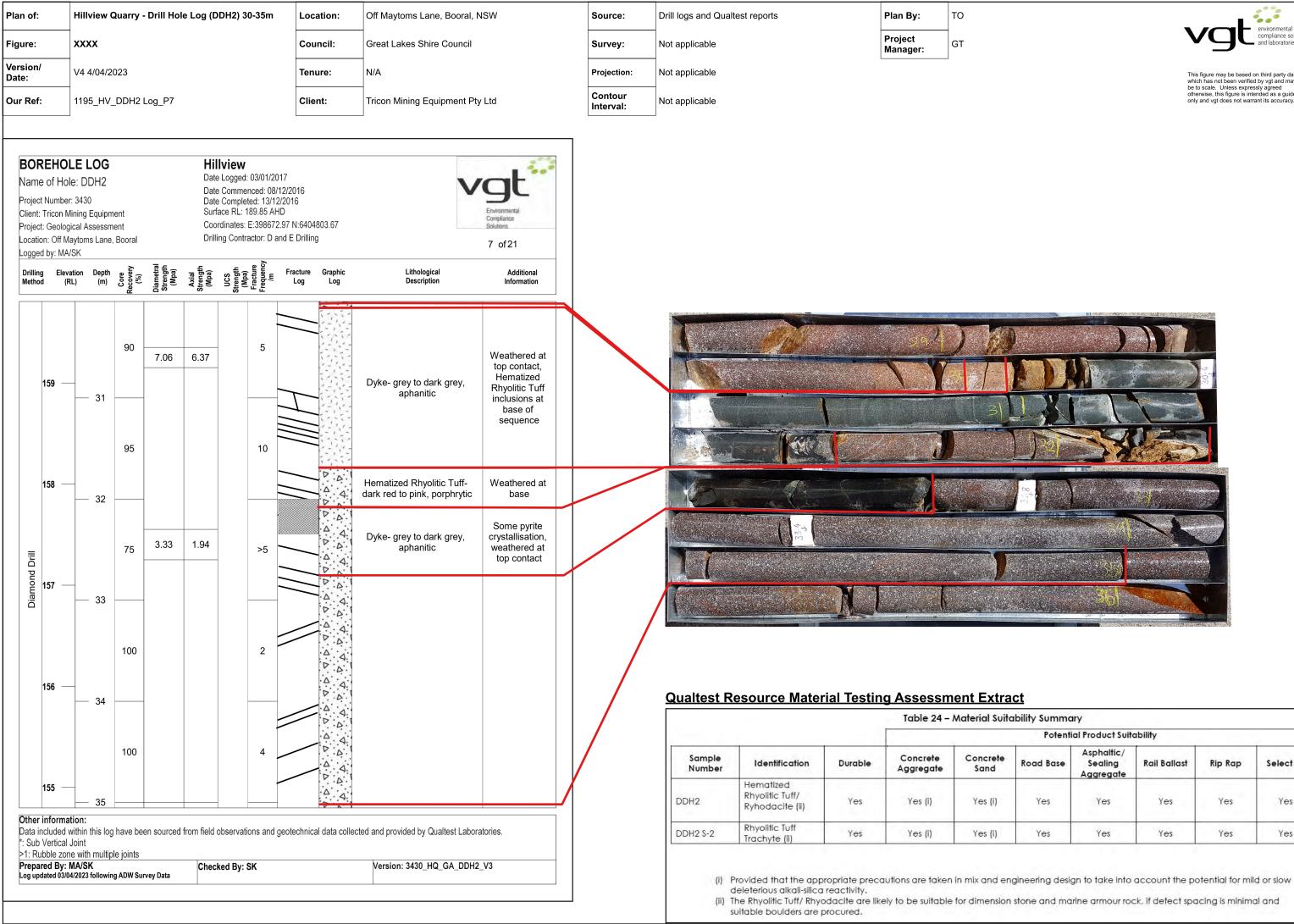




Suito	uitability Summary											
Potential Product Suitability												
te	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill							
)	Yes	Yes	Yes	Yes	Yes							
)	Yes	Yes	Yes	Yes	Yes							

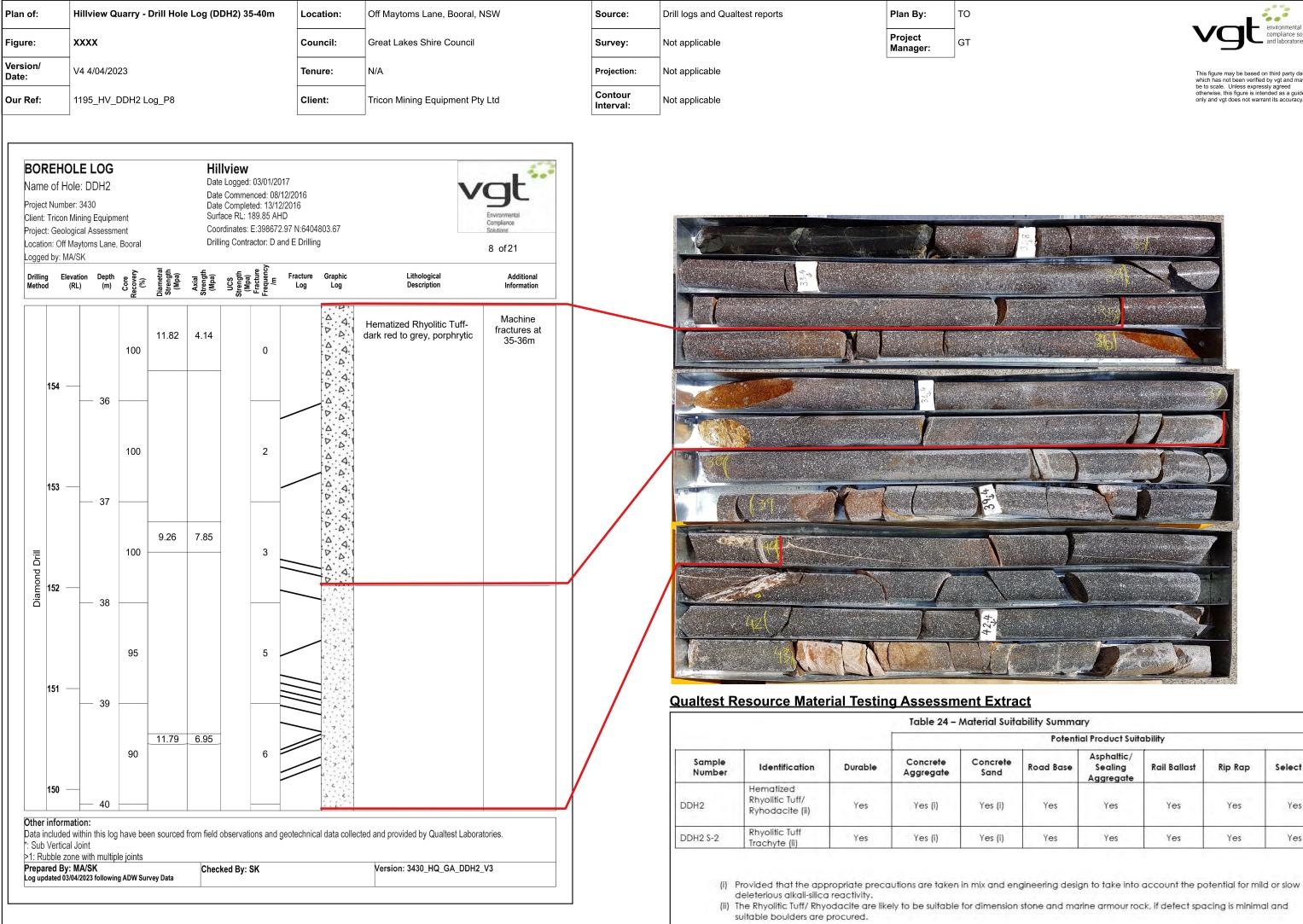
engineering design to take into account the potential for mild or slow

ion stone and marine armour rock, if defect spacing is minimal and



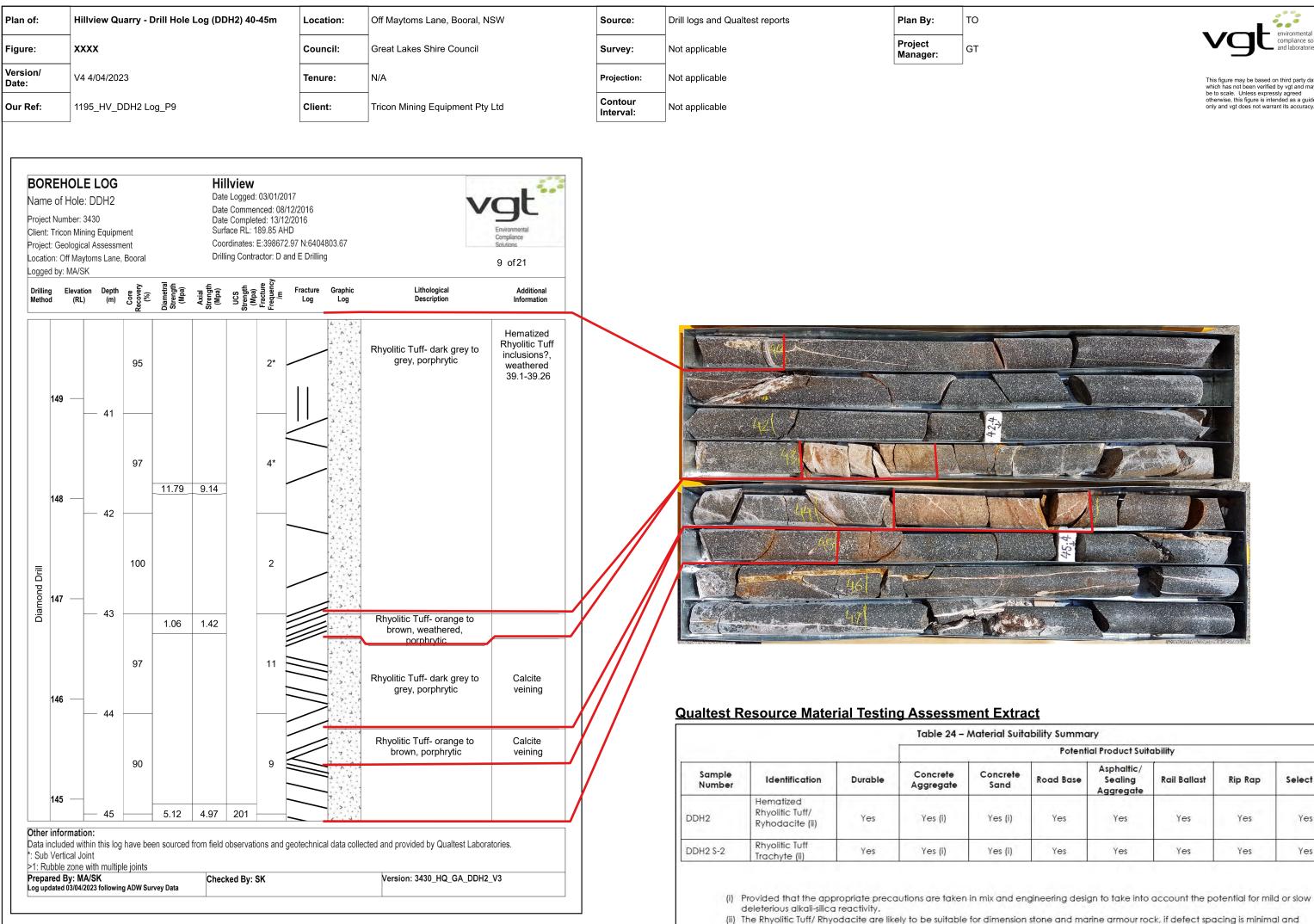


Potential Product Suitability							
1	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill		
	Yes	Yes	Yes	Yes	Yes		
1	Yes	Yes	Yes	Yes	Yes		





Potential Product Suitability								
1	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill			
	Yes	Yes	Yes	Yes	Yes			
1	Yes	Yes	Yes	Yes	Yes			



suitable boulders are procured.



Potential Product Suitability								
	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill			
	Yes	Yes	Yes	Yes	Yes			
1	Yes	Yes	Yes	Yes	Yes			

Plan of:	Hillview C	uarry - D	rill Hole	Log (D	DH2) 45-50	Dm L	ocation:	Off Maytoms Lane,	Booral, NSW	Source:	Drill logs and Qual	test reports		Plan By:	то
Figure:	xxxx					С	ouncil:	Great Lakes Shire	Council	Survey:	Not applicable			Project Manager:	GT
Version/ Date:	V4 4/04/20	23				Т	enure:	N/A		Projection:	Not applicable				
Our Ref:		DDH2 Log	g_P10			с	lient:	Tricon Mining Equi	pment Pty Ltd	Contour Interval:	Not applicable				
Name of I Project Num Client: Trico Project: Gec Location: Of Logged by: I	n Mining Equipn blogical Assessn ff Maytoms Lane MA/SK	ent Booral	Diametral Strength (Mpa)	Date Date Date Surfa Coor Drillin		: 08/12/2016 13/12/2016 5 AHD 8672.97 N:64 : D and E Dril	ing	Lithological Description	Environmental Compliance Solutions 10of 21 Additional Information						
144	46	95			3										
143	47		3.3	2.02			2					48		(
Diamond Drill Diamond Drill	48	95			3	* 						1		20	
141	49	100	8.85	5.75	137						Qualtest R	<u>esource Mate</u>	<u>rial Testin</u>	-	nent Extra Material Suit
140	_	100			2						Sample Number	Identification Hematized	Durable	Concrete Aggregate	Concrete Sand
Other infor	mation:						6				DDH2	Rhyolitic Tuff/ Ryhodacite (ii)	Yes	Yes (i)	Yes (i)
	ed within this log	have been	sourced f	rom field c	bservations	and geotechn	ical data collec	cted and provided by Qualte	est Laboratories.		DDH2 S-2	Rhyolitic Tuff Trachyte (ii)	Yes	Yes (i)	Yes (i)
>1: Rubble : Prepared B	zone with multip		y Data	Check	ed By: SK			Version: 3430_HQ_G	A_DDH2_V3		(ii) 1	Provided that the app deleterious alkali-silica he Rhyolitic Tuff/ Rhyo uitable boulders are	i reactivity. odacite are lik		



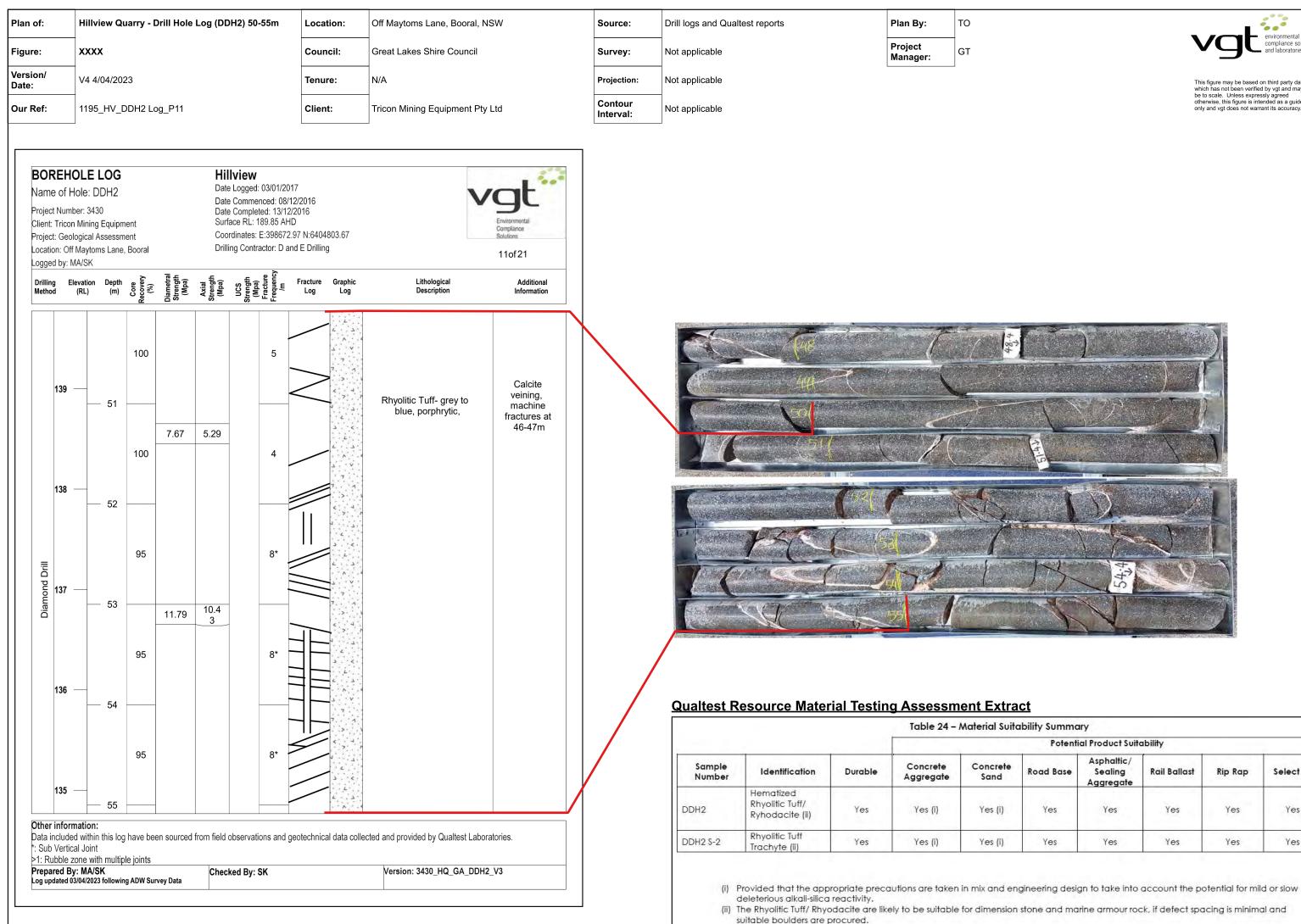


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Potential Product Suitability								
	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill			
	Yes	Yes	Yes	Yes	Yes			
1	Yes	Yes	Yes	Yes	Yes			

engineering design to take into account the potential for mild or slow

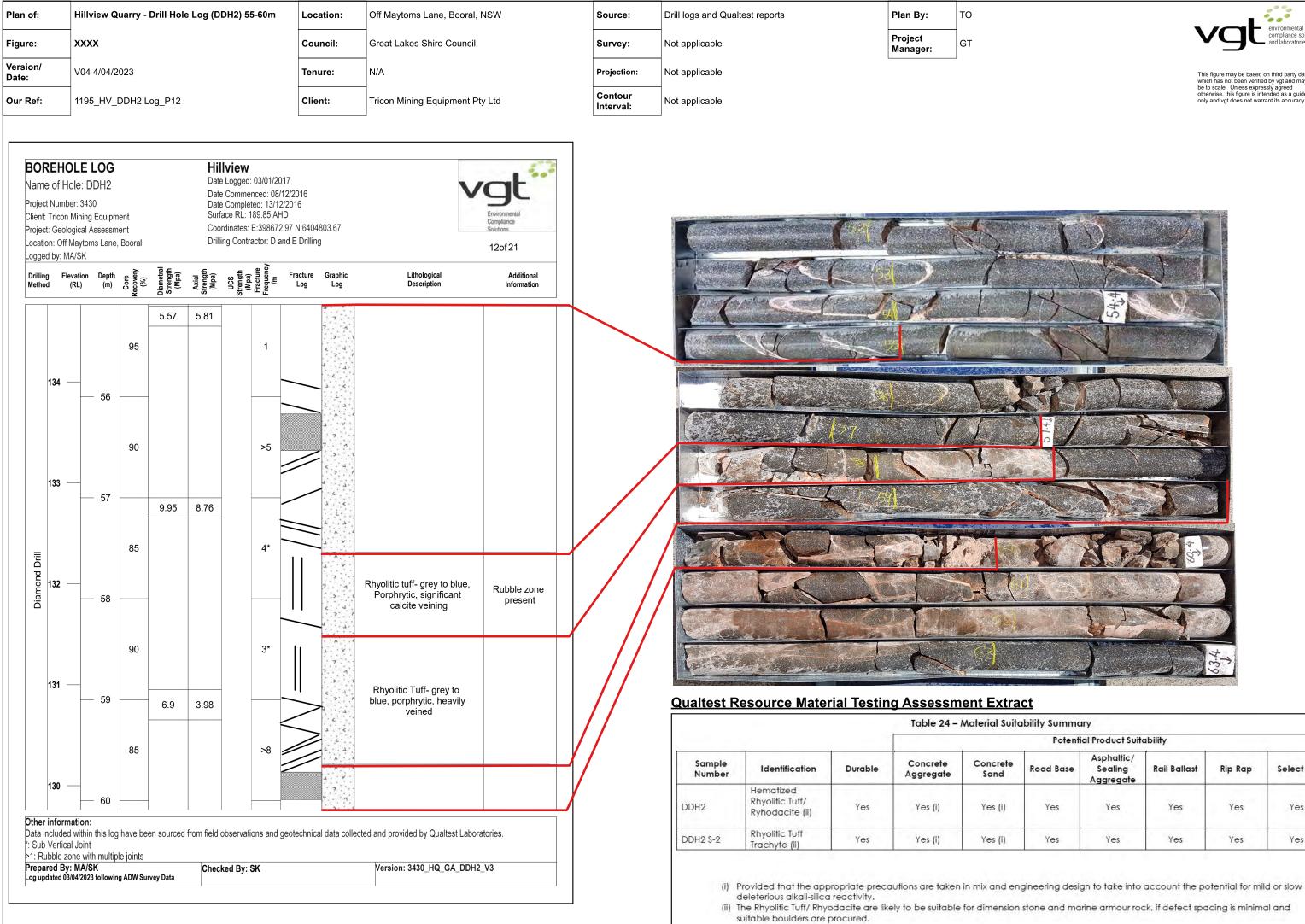
on stone and marine armour rock, if defect spacing is minimal and





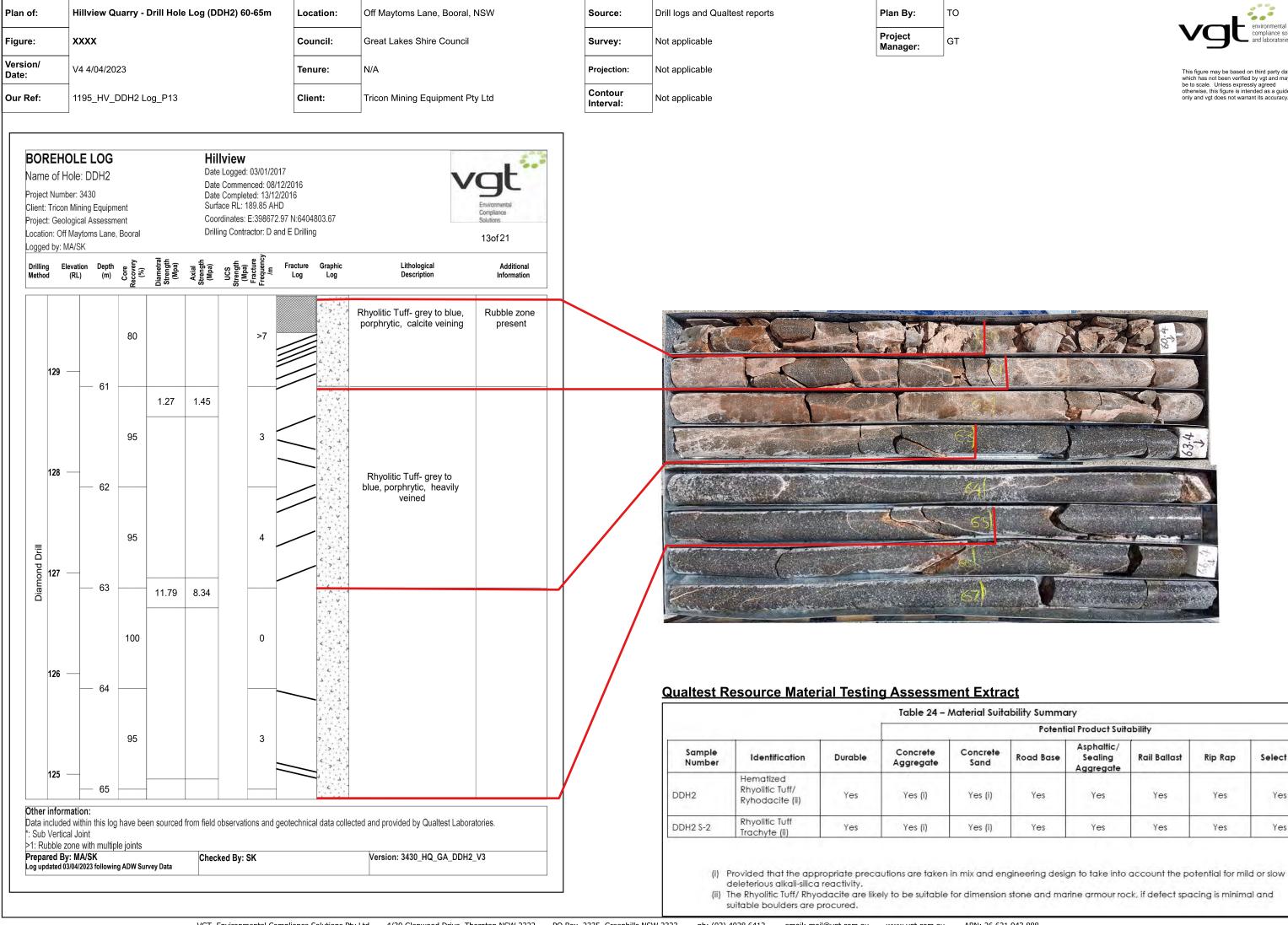


Potential Product Suitability								
	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill			
	Yes	Yes	Yes	Yes	Yes			
1	Yes	Yes	Yes	Yes	Yes			



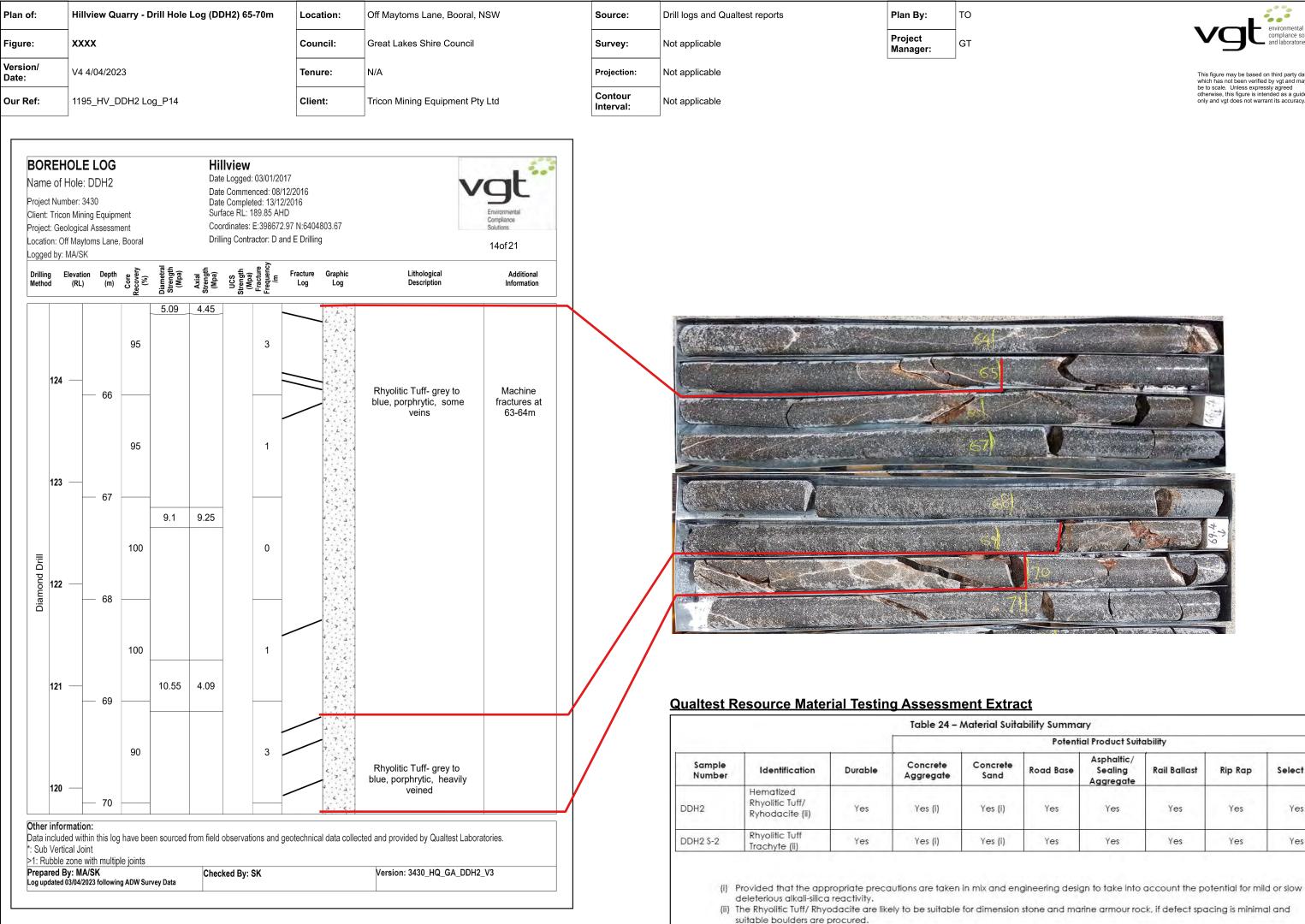


Potential Product Suitability							
1	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill		
	Yes	Yes	Yes	Yes	Yes		
1	Yes	Yes	Yes	Yes	Yes		





Potential Product Suitability								
	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill			
	Yes	Yes	Yes	Yes	Yes			
1	Yes	Yes	Yes	Yes	Yes			





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	Potential Product Suitability										
	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill						
	Yes	Yes	Yes	Yes	Yes						
1	Yes	Yes	Yes	Yes	Yes						

Plan	of:	н	lillview Q	uarry -	Drill Hole	e Log (Di	DH2) 70	0-75m	Loc	ation:	Off May	/toms Lane	e, Booral,	NSW		Source:	Dril	logs and C	ualtest rep	orts			Plan By:	то
Figu	re:	x	XXX						Со	uncil:	Great L	akes Shire.	e Council			Survey:	Not	applicable					Project Manager:	GT
Vers Date	ion/ :	v	4 4/04/20	23					Ten	ure:	N/A					Projection:	Not	applicable						
Our	Ref:	1	195_HV_[DDH2 L	.og_P15				Clie	ent:	Tricon I	Vining Equ	ipment P	ty Ltd		Contour Interval:	Not	applicable						
	Name Project I Client: T Project:	of Hol Number Tricon M Geolog n: Off Ma	lining Equipr ical Assessn aytoms Lane /SK	nent nent e, Booral		Dat Dat Dat Sur Coc Drill	te Comme te Comple face RL: ordinates: lling Contr	1: 03/01/20 enced: 08/ eted: 13/1: 189.85 AF : E:398672 ractor: D a	/12/2016 2/2016 HD 2.97 N:6404 and E Drillir				V	Environmental Compliance Solutions 15of 21										
	Drilling Method	Eleva (R	ation Depth RL) (m)	Core Recovery (%)	Diametral Strength (Mpa)	Axial Strength (Mpa)	UCS Strength (Mpa)	(mpa) Fracture Frequency /m	Fracture Log	Graphic Log		Lithological Description		Addition Information										
	Drill 1	19 — 18 —	— 71 — 72 — 73	95 90 90	9.69	1.67		3* 2* 6*																
		16 —	74	95	_			2*									G	lualtest	Resou	Irce M	aterial	Testii	ng Assess Table 24	<u>ment Ex</u> - Material

· L ·

Version: 3430_HQ_GA_DDH2_V3

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories.

Checked By: SK

115 -

Other information:

*: Sub Vertical Joint

>1: Rubble zone with multiple joints Prepared By: MA/SK Log updated 03/04/2023 following ADW Survey Data

- 75

			Table 24 – I	Material Suito	ibility Summa	iry			
					Potenti	al Product Suite	ability		
Sample Number	Identification	Durable	Concrete Aggregate	Concrete Sand	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill
DDH2	Hematized Rhyolitic Tuff/ Ryhodacite (ii)	Yes	Yes (i)	Yes (i)	Yes	Yes	Yes	Yes	Yes
DDH2 S-2	Rhyolitic Tuff Trachyte (ii)	Yes	Yes (i)	Yes (i)	Yes	Yes	Yes	Yes	Yes

(i) Provided that the appropriate precautions are taken in mix and engineering design to take into account the potential for mild or slow deleterious alkali-silica reactivity.
 (ii) The Rhyolitic Tuff/ Rhyodacite are likely to be suitable for dimension stone and marine armour rock, if defect spacing is minimal and suitable boulders are procured.



This figure may be based on third party data which has not been verified by vgt and may not be to scale. Unless expressly agreed otherwise, this figure is intended as a guide only and vgt does not warrant its accuracy.



<u>ktract</u>

Plan of:		Hillvie	w Quarry	/ - Drill Ho	ole Log (I	DH2) 75-80m	Loc	ation:	Off Maytoms Lane, Boora	I, NSW	\$	Source:	Drill logs and Qua	Itest reports		Plan By:	ТО
Figure:		xxxx					Co	uncil:	Great Lakes Shire Counci	I	:	Survey:	Not applicable			Project Manager:	GT
Version/ Date:		V4 4/0	4/2023				Ter	ure:	N/A		F	Projection:	Not applicable				
Our Ref:		1195_H	IV_DDH	2 Log_P16	6		Clie	ent:	Tricon Mining Equipment I	Pty Ltd		Contour Interval:	Not applicable				
Project N Client: Tr Project: (of Ho Number ricon M Geolog : Off M by: MA	le: DDH r: 3430 /ining Eq gical Asse laytoms L /SK	l2 uipment ssment ane, Boora		Da Da Da Sui Co Dri	Ilview te Logged: 03/01/2 te Commenced: 08 te Completed: 13/1 face RL: 189.85 Al ordinates: E:39867 ling Contractor: D	9/12/2016 2/2016 HD 2.97 N:6404			Environmental Compliance Solutions 16of 21							
Drilling Method	Elev (F	ation Do RL)	Core (m) Recovery	(%) Diametral Strength (Moa)	Axial Strength (Mpa)	UCS Strength (Mpa) Fracture Frequenc /m	Fracture Log	Graphic Log	Lithological Description	Additional Information			Late F		Lu	11	
				8.5	3.98		>	ч 			\wedge						<u>1</u>
			95			2		7 A 2 4					Antin				
1	14 —	_						J . L .	Rhyolitic Tuff- grey to blue, porphrytic, some	Machine fractures			The second se				
		- 7	6					L	veins	78-81.4m			And and a second second	the line			<mark>0</mark> 10
			100			4							Lik				
1 [,]	13 —							~ ~ ~ ~					Carl Con			- Jan	78
		- 7	7	9.89	5.25		-	2 2 2					+1·81				E.
			100			3*		د ۸									Para and Para
Diamond Drill								7. 1						K k			
Diam Diam	12 —	7	8	_				L . L									
								2 					81.4	57			
			95			1*		2					()	6	for the second s		1
1	11 —	7	.9	9.63	3.7			7.4					Qualtest R	esource Mater	rial Testin	a Assessm	ent Extra
						-						/				-	Material Suit
			100			2*		 					Sample			Concrete	Concrete
1	10 —		.0					· · · ·					Number	Identification Hematized	Durable	Aggregate	Sand
Other in		ation:						7. 1			/		DDH2	Rhyolitic Tuff/ Ryhodacite (ii)	Yes	Yes (i)	Yes (i)
Data incl *: Sub V	luded v ertical	within thi: Joint	s log have ultiple joint		ed from field	observations and	geotechnic	al data colle	cted and provided by Qualtest Lab	oratories.			DDH2 S-2	Rhyolitic Tuff Trachyte (ii)	Yes	Yes (i)	Yes (i)
Prepare	d By:	MA/SK		S Survey Data	Chee	cked By: SK			Version: 3430_HQ_GA_DDF	12_V3			(i)	Provided that the app deleterious alkali-silica	propriate preco	autions are taker	n in mix and er

VGT Environmental Compliance Solutions Pty Ltd 4/30 Glenwood Drive, Thornton NSW 2322 PO Box 2335, Greenhills NSW 2323 ph: (02) 4028 6412 email: mail@vgt.com.au www.vgt.com.au

suitable boulders are procured.



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Suitability Summary											
Potential Product Suitability											
rete d	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill						
(i)	Yes	Yes	Yes	Yes	Yes						
(i)	Yes	Yes	Yes	Yes	Yes						

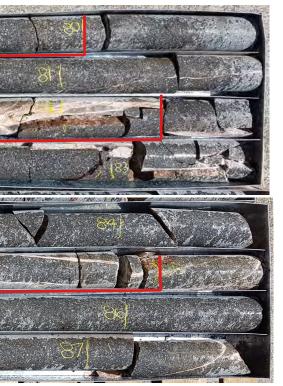
engineering design to take into account the potential for mild or slow

(ii) The Rhyolitic Tuff/ Rhyodacite are likely to be suitable for dimension stone and marine armour rock, if defect spacing is minimal and

an of:		Hillview	Quarry -	Drill Hole	e Log (DI	DH2) 80-85m	Loc	ation:	Off Maytoms Lane, Booral,	NSW	Source:	Drill logs and Qual	test reports		Plan By:	то
gure:		хххх					Cou	ıncil:	Great Lakes Shire Council		Survey:	Not applicable			Project Manager:	GT
rsion/ te:		V4 4/04/2	023				Ten	ure:			Projection:	Not applicable				
r Ref:		1195_HV	_DDH2 L	.og_P17			Clie	ent:	Tricon Mining Equipment P	ty Ltd	Contour Interval:	Not applicable				
									_							
Name Project Client: ⁻ Project:	of Ho Numbe Tricon I Geolog n: Off M by: MA		oment sment ne, Booral		Date Date Date Surfa Coor Drilli	Iview a Logged: 03/01/2 a Commenced: 04 b Completed: 13/ ace RL: 189.85 A rdinates: E:39863 ng Contractor: D	8/12/2016 12/2016 NHD 72.97 N:6404		V	Ervironmental Compliance Solutions 17of 21						
Drilling Method	Elev (vation Dep RL) (m	Core Recovery (%)	Diametral Strength (Mpa)	Axial Strength (Mpa)	UCS Strength (Mpa) Fracture Frequenc	Fracture Log	Graphic Log	Lithological Description	Additional Information						
			100			0		4 4 4								
	109 —	_		11.82	1.46			2 				(1
		- 81					-	3 V 7 7 7				418	-	- Andrews		
			95			1*		· · · · · · · · · · · · · · · · · · ·	Rhyolitic Tuff- grey to			197		pene	n l	
	108 —	82		-				7. 1 	blue, porphrytic, heavily veined							
													<u></u>	<u></u>		
nd Drill	107 —		95	7.51	2.52	7*		7. 1 				<u> Vilaniana</u>	<u> </u>			
Diamond Drill	107 —	- 83		-				4 						<u>en esterne</u>	T T	<u>i de la calacia</u>
			95			4*		2 			/		hter strange and	dia in the second		when from
	106 —	_						7. 1								
		- 84		-								Qualtest R	esource Mater	<u>rial Testin</u>		nent Extra Material Suito
			95	7.49	5.94	3*		L					1		-	1
	105 —							7			/	Sample Number	Identification Hematized	Durable	Concrete Aggregate	Concrete Sand
Other i	nform			-				4				DDH2	Rhyolitic Tuff/ Ryhodacite (ii)	Yes	Yes (i)	Yes (i)
Data in *: Sub \	cluded √ertical	within this I I Joint	-		from field o	observations and	d geotechnica	al data collec	ted and provided by Qualtest Labor	atories.		DDH2 \$-2	Rhyolitic Tuff Trachyte (ii)	Yes	Yes (i)	Yes (i)
>1: Rul Prepar Log upd	ed By: ated 03/	ne with mul MA/SK 04/2023 follov	upie joints ving ADW Su	rvey Data	Checl	ked By: SK			Version: 3430_HQ_GA_DDH2	_V3		(ii)	Provided that the app deleterious alkali-silica The Rhyolitic Tuff/ Rhyo suitable boulders are p	reactivity. odacite are lik		



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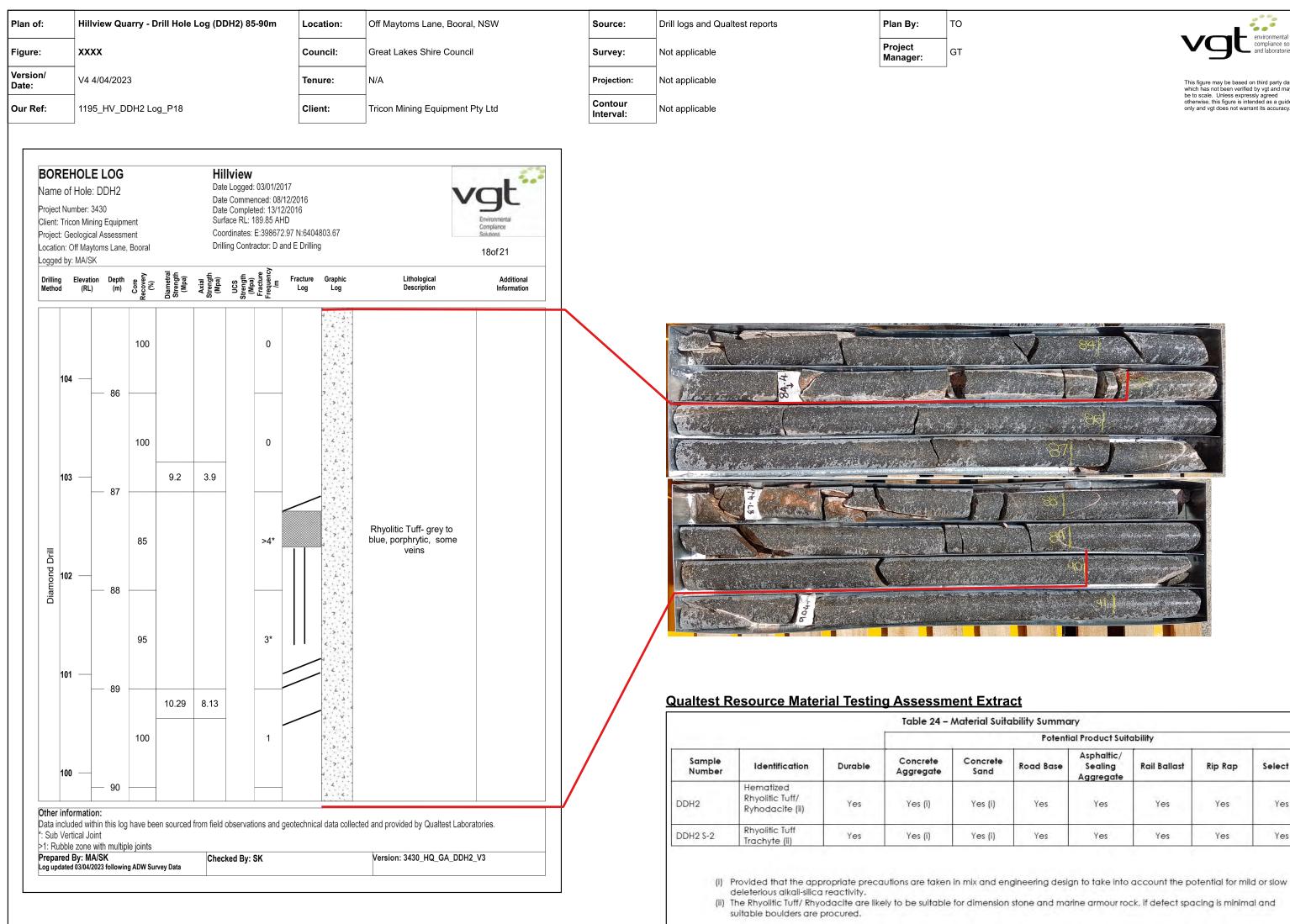


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Potential Product Suitability										
Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fil						
Yes	Yes	Yes	Yes	Yes						
Yes	Yes	Yes	Yes	Yes						

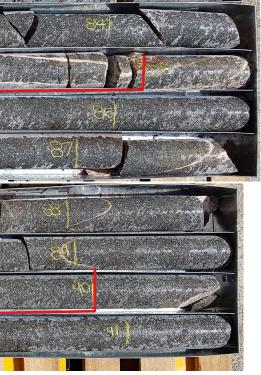
engineering design to take into account the potential for mild or slow

ion stone and marine armour rock, if defect spacing is minimal and

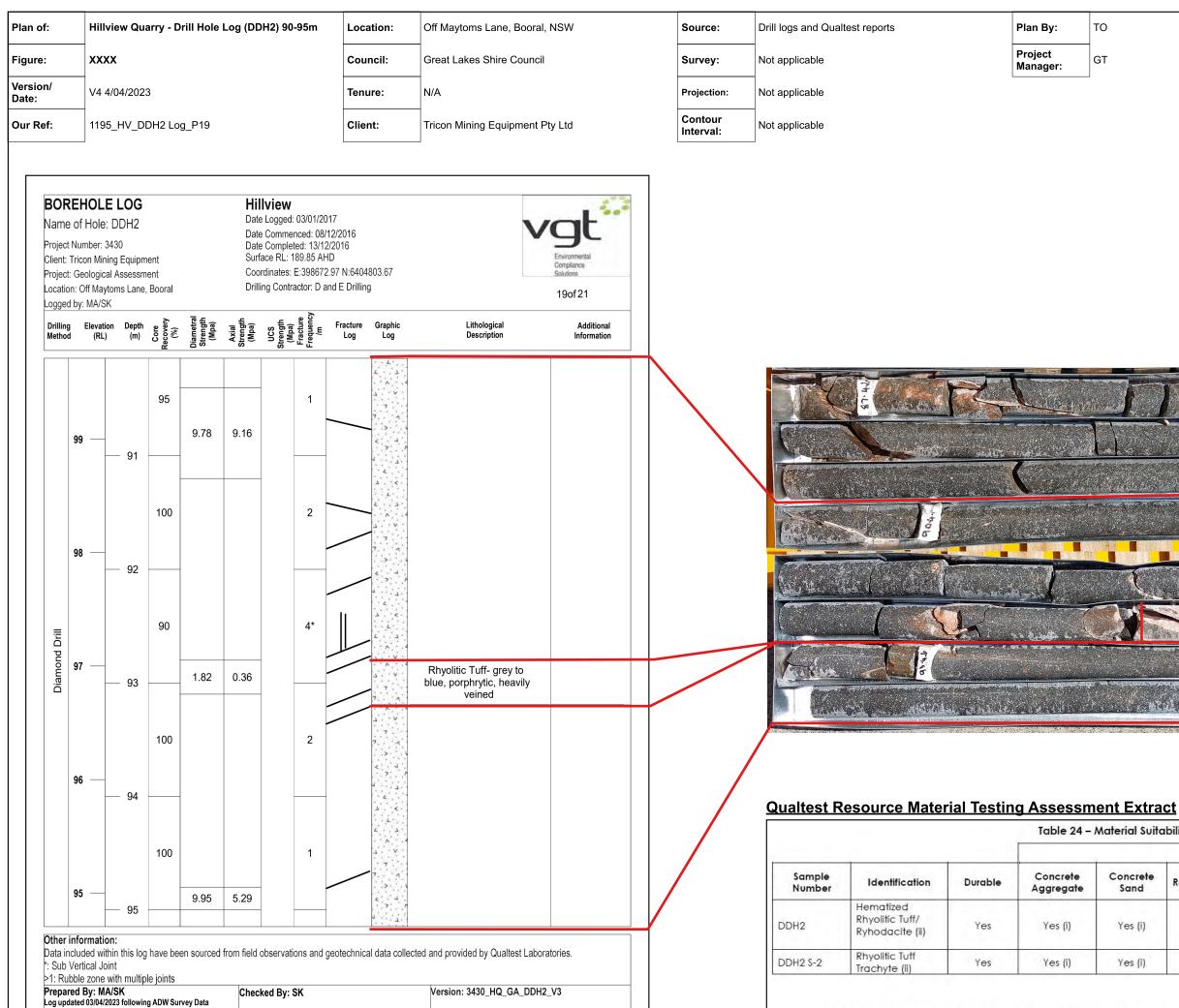




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	Potential Product Suitability											
	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill							
	Yes	Yes	Yes	Yes	Yes							
1	Yes	Yes	Yes	Yes	Yes							



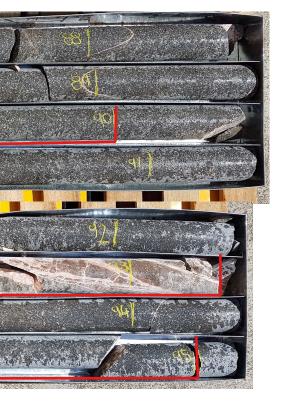
(i) Provided that the appropriate precautions are taken in mix and engineering design to take into account the potential for mild or slow deleterious alkali-silica reactivity

suitable boulders are procured.

ph: (02) 4028 6412 4/30 Glenwood Drive, Thornton NSW 2322 PO Box 2335, Greenhills NSW 2323 ABN: 26 621 943 888 VGT Environmental Compliance Solutions Pty Ltd email: mail@vgt.com.au www.vgt.com.au



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	Potential Product Suitability										
	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill						
	Yes	Yes	Yes	Yes	Yes						
1	Yes	Yes	Yes	Yes	Yes						

(ii) The Rhyolitic Tuff/ Rhyodacite are likely to be suitable for dimension stone and marine armour rock, if defect spacing is minimal and



deleterious alkali-silica reactivity

suitable boulders are procured.



Potential Product Suitability										
	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill					
	Yes	Yes	Yes	Yes	Yes					
	Yes	Yes	Yes	Yes	Yes					

(i) Provided that the appropriate precautions are taken in mix and engineering design to take into account the potential for mild or slow

(ii) The Rhyolitic Tuff/ Rhyodacite are likely to be suitable for dimension stone and marine armour rock, if defect spacing is minimal and

хххх									I, NSW	Source:	Drill logs and Qual	•		-	то
						Cou	ncil:	Great Lakes Shire Counci	I	Survey:	Not applicable			Project Manager:	GT
V4 4/04	/2023					Tenu	ure:	N/A		Projection:	Not applicable				
1195_H	IV_DDH2	Log_P21				Clie	nt:	Tricon Mining Equipment F	⊃ty Ltd	Contour Interval:	Not applicable				
					3/01/2017	7									
er: 3430 Mining Ec ogical Ass	luipment essment	I	Dat Sur Coc	te Completed face RL: 189 prdinates: E:	d: 13/12/2 9.85 AHD 398672.9	2016 97 N:6404			Environmental Compliance Solutions 21of 21						
evation D (RL)	epth Core Core Recovery	(%) Diametral Strength (Mna)	Axial Strength (Mpa)	UCS Strength (Mpa) Fracture	Frequency /m	Fracture Log	Graphic Log	Lithological Description	Additional Information						
	85				0										
	101							Hole Terminated at Target Depth					T-I-I		
	102														
	103														
	104										Qualtest R	esource Mater	<u>rial Testin</u>	-	14
												1			
											Sample Number	Identification	Durable	Concrete Aggregate	Concrete Sand
	105										DDH2	Hematized Rhyolitic Tuff/ Ryhodacite (ii)	Yes	Yes (i)	Yes (i)
ation: within the	is log have	been source	ed from field	observation	ns and ge	otechnica	al data collec	ted and provided by Qualtest Labo	pratories.		DDH2 S-2	Rhyolitic Tuff Trachyte (ii)	Yes	Yes (i)	Yes (i)
one with n : MA/SK			Chec	cked By: Sk	(Version: 3430_HQ_GA_DDH	I2_V3		(ii)	deleterious alkali-silica The Rhyolitic Tuff/ Rhyo	reactivity. odacite are lik		
	LE LC le: DDI r: 3430 lining Ec gical Ass laytoms I /SK ration D RL) ation: within th Joint ne with n MA/SK	LE LOG le: DDH2 r: 3430 Aining Equipment gical Assessment laytoms Lane, Boora /SK ration Depth go go (m) go go go assessment assessment laytoms Lane, Boora /SK assessment	LELOG le: DDH2 r: 3430 /ining Equipment gical Assessment laytoms Lane, Booral /SK ration Depth g $g g g g g g g g g g g g g g g g g g$	le: DDH2 Data T: 3430 Data Jining Equipment Sur gical Assessment Coordination of the set of the	LE LOG Hillview le: DDH2 Date Logged: 0 r: 3430 Date Commend gical Assessment Coordinates: E: laytoms Lane, Booral Drilling Contract /SK registration registration attion mm gives registration 101 stion: stion: stion: within this log have been sourced from field observatior Joint not stion: MAISK MAISK Checked By: SP	LE LOG Hillview le: DDH2 Date Logged: 03/01/2011 r: 3430 Date Completed: 13/1/22 Jining Equipment Surface RL: 189.85 AHD gical Assessment Coordinates: E: 398672.5 laytoms Lane, Booral Drilling Contractor: D and /SK	LE LOG le: DDH2 Hillview Date Logget: 03/01/2017 Date Commenced: 03/12/2016 Surface RL: 18/38/AHD r: 3430 Surface RL: 18/38/AHD Jayloms Lane, Booral Drilling Contractor: D and E Drilling JSK Strace RL: 18/38/AHD ation Bage R ation Bage R Image Ruphment Strace RL: 18/38/AHD JSK Drilling Contractor: D and E Drilling JSK Image Ruphment ation Base Image Ruph Base Image Ruphment Base JSK Image Ruphment Image Ruphment Base Image Ruphm	Le LOG Hillview Le: DDH2 ::340 Date Logge::0301/2017 ining Equipment :22016 ipcal Assessment :2007 laytoms Lane, Booral :2007 SK :2007 ation provide the interview ining Countrates: :3308/72.97 N:6404803.67 Date Logge: :3102 SK :2007 ation provide the interview ining Countrates: :3308/72.97 N:6404803.67 Dilling Contractor: D and E Drilling SK :2007 ation :2008 ining Countractor: D and E Drilling ining Countractor: :2007 ining Countractor: :2007	LE LOG	LE LOC 67.300 Arring Equipment 1000 Arring Expansion Laptons Lane, Board 101 min S S E Hilly can be Commenced 001/2016 Suffice RL: 188.5 AHD 2016/188.5 AHD	Little Little Little Little Little Little Little If If Date Logget (001/01/17) Date Logget (001/01/17) Date Logget (001/02/16) Jack Commerced Strates Date Logget (001/02/16) Surface RL (186 Str Million) Date Logget (001/02/16) Jack Commerced Strates I Date Logget (001/02/16) Date Logget (001/02/16) Surface RL (186 Str Million) Date Logget (001/02/16) Jack Commerced Strates I Date Logget (001/02/16) Date Logget (001/02/16) Surface RL (186 Str Million) Date Logget (001/02/16) Jack Death Document Coordinates: E39972 Str Ms Hold 03 Str Date Logget (001/02/16) Surface RL (186 Str Million) Jack Death Document Date Logget (001/02/16) Surface RL (186 Str Million) Date Logget (001/02/16) Jack Death Document Date Logget (001/02/16) Surface RL (186 Str Million) Date Logget (001/02/16) Jack Death Document Date Logget (001/02/16) Date Logget (001/02/16) Surface RL (186 Str Million) Jack Death Document Date 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to public data LE Del to public data Del to public data Del to public data LE Del to public data D	Hindow Internet Internet Marketing EE DOG Um suppression Um suppression Um suppression Hindow Um suppression Um suppression Um suppression Um suppression Hindow Um suppression Um suppression Um suppression Um suppression Um suppression Hindow Um suppression Um suppression Um suppression Um suppression Um suppression Hindow Um suppression Um suppression Um suppression Um suppression Um suppression Hindow Hindow Um suppression Um suppression Um suppression Um suppression Hindow Hindow Um suppression



This figure may be based on third party data which has not been verified by vgt and may not be to scale. Unless expressly agreed otherwise, this figure is intended as a guide only and vgt does not warrant its accuracy.

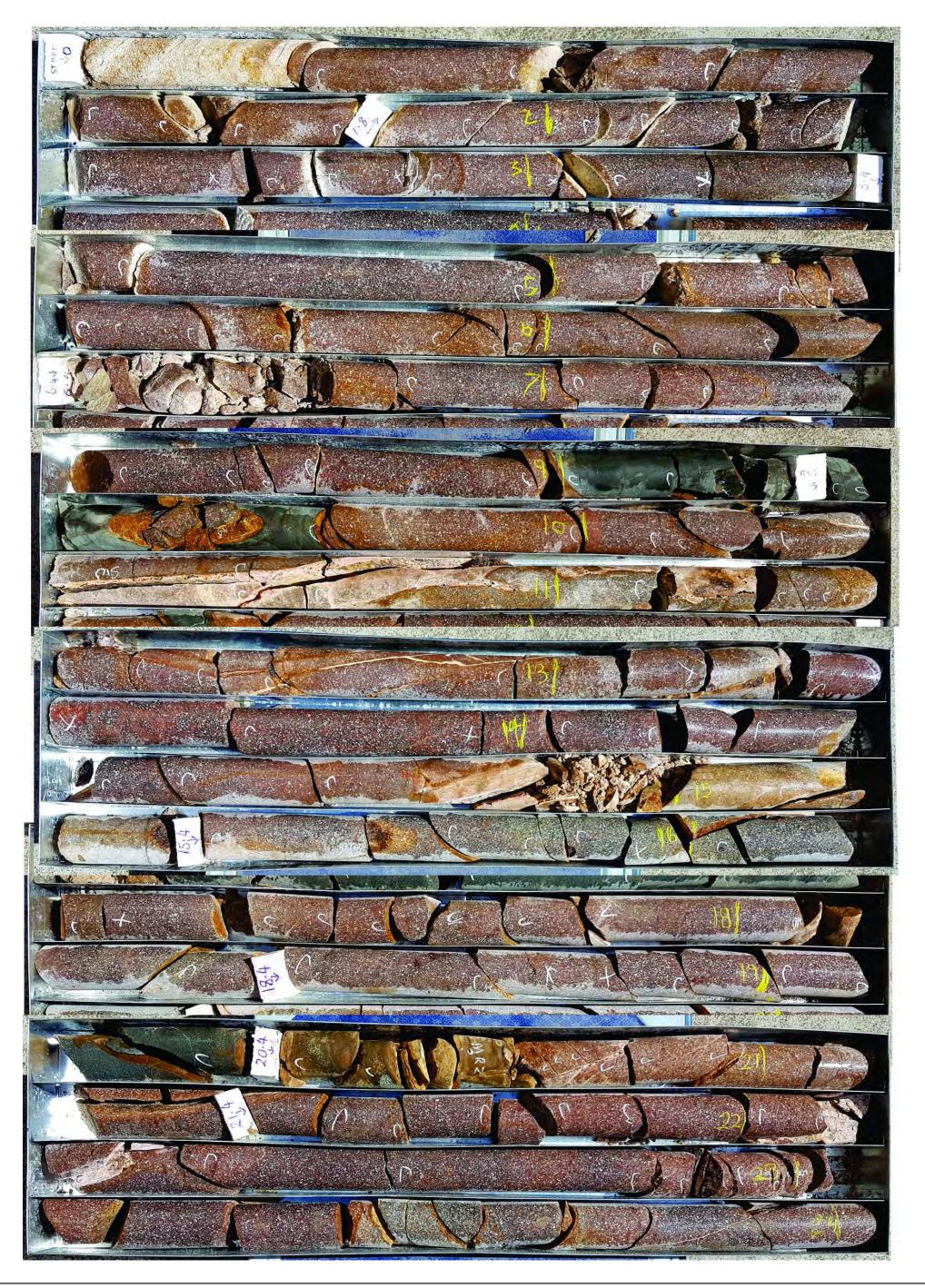


uitability Summary											
Potential Product Suitability											
te	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill						
	Yes	Yes	Yes	Yes	Yes						
	Yes	Yes	Yes	Yes	Yes						

engineering design to take into account the potential for mild or slow

on stone and marine armour rock, if defect spacing is minimal and

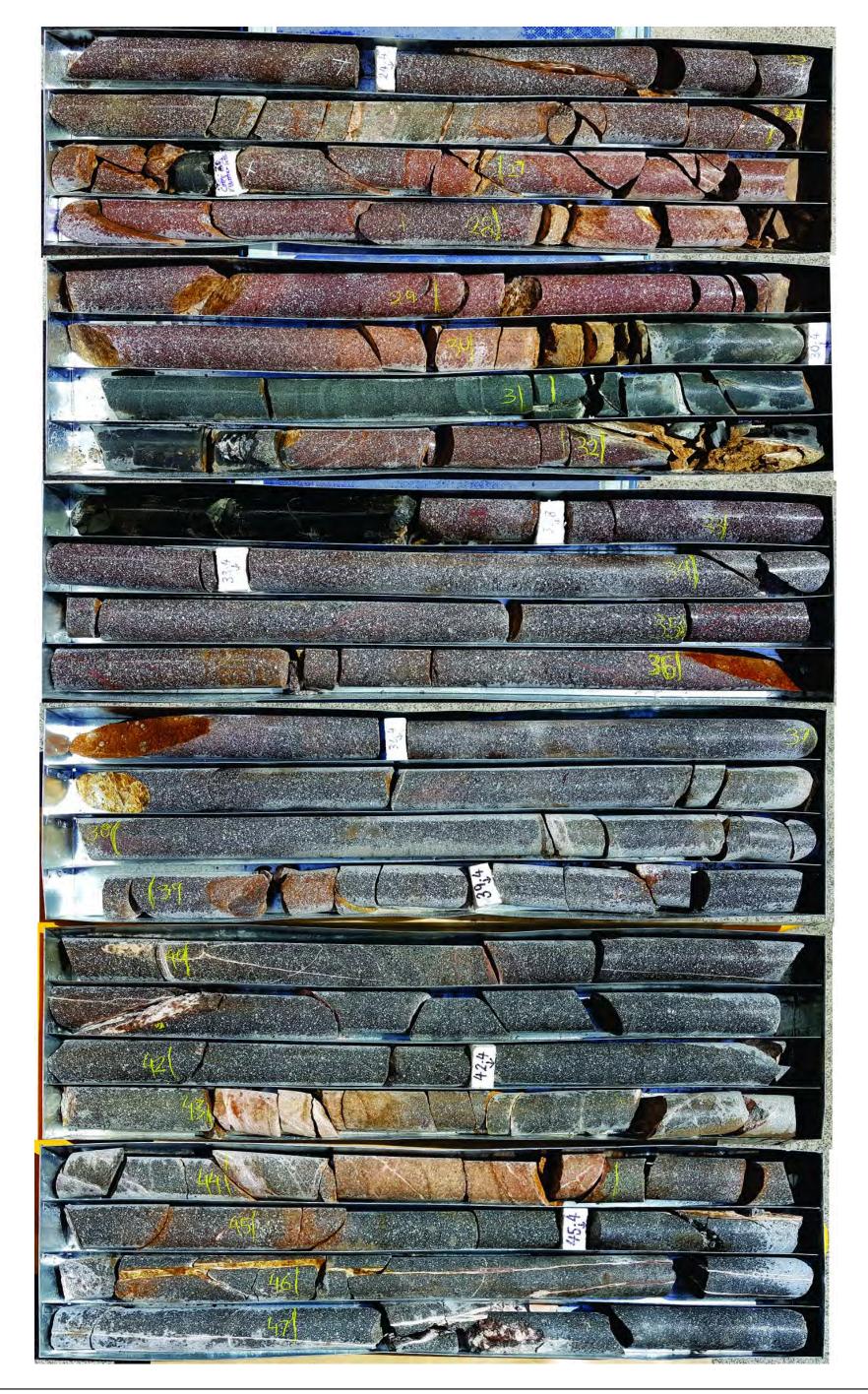
Plan of	Hillview Quarry - BBH2 Core Photos	Location:	Off Maytoms Lane, Booral, NSW	Source:	VGT Photography 2023	Plan By:	то	
Figure	Page 1	Council:	Great Lakes Shire Council	Survey:	Not applicable	Project Manager:	GVT	vgt environmental compliance solutions and laboratories
Versio Date:	/ V0 21/02/2023	Tenure:	Not Applicable	Projection:	Not applicable		_	This figure may be based on third party data which has not been verified by vgt and may not be to scale. Unless
Our Re	1195_HV_SEARS_Q008_V0_BHH2 Core Box 1 to 6	Client:	Tricon Mining Equipment Pty Ltd	Contour Interval:	Not applicable			expressly agreed otherwise, this figure is intended as a guide only and vgt does not warrant its accuracy.



Pla	an of:	Hillview Quarry - BBH2 Core Photos	Location:	Off Maytoms Lane, Booral, NSW	Source:	VGT Photography 2023	Plan By:	то	1. T
Fig	jure:	Page 2	Council:	Great Lakes Shire Council	Survey:	Not applicable	Project Manager:	GVT	Vgc environmental compliance solutions and laboratories
Ve Da	rsion/ te:	V0 21/02/2023	Tenure:	Not Applicable	Projection:	Not applicable			This figure may be based on third party data which has not been verified by vgt and may not be to scale. Unless
Ou	ır Ref:	1195_HV_SEARS_Q008_V0_BHH2 Core Box 7 to 12	Client:	Tricon Mining Equipment Pty Ltd	Contour Interval:	Not applicable			expressly agreed otherwise, this figure is intended as a guide only and vgt does not warrant its accuracy.



P	lan of:	Hillview Quarry - BBH2 Core Photos	Location:	Off Maytoms Lane, Booral, NSW	Source:	VGT Photography 2023	Plan By:	то	
Fi	igure:	Page 3	Council:	Great Lakes Shire Council	Survey:	Not applicable	Project Manager:	GVT	voit compliance solutions and laboratories
	ersion/ ate:	V0 21/02/2023	Tenure:	Not Applicable	Projection:	Not applicable		-	This figure may be based on third party data which has not been verified by vgt and may not be to scale. Unless
0	ur Ref:	1195_HV_SEARS_Q008_V0_BHH2 Core Box 13 to 18	Client:	Tricon Mining Equipment Pty Ltd	Contour Interval:	Not applicable			expressly agreed otherwise, this figure is intended as a guide only and vgt does not warrant its accuracy.



Р	lan of:	Hillview Quarry - BBH2 Core Photos	Location:	Off Maytoms Lane, Booral, NSW	Source:	VGT Photography 2023	Plan By:	то		
Fi	gure:	Page 4	Council:	Great Lakes Shire Council	Survey:	Not applicable	Project Manager:	GVT	vojte environmental compliance solutions and laboratories	
	ersion/ ate:	V0 21/02/2023	Tenure:	Not Applicable	Projection:	Not applicable		-	This figure may be based on third party data which has not been verified by vgt and may not be to scale. Unless	
0	ur Ref:	1195_HV_SEARS_Q008_V0_BHH2 Core Box 19 to 24	Client:	Tricon Mining Equipment Pty Ltd	Contour Interval:	Not applicable			expressly agreed otherwise, this figure is intended as a guide only and vgt does not warrant its accuracy.	



Pla	n of:	Hillview Quarry - BBH2 Core Photos	Location:	Off Maytoms Lane, Booral, NSW	Source:	VGT Photography 2023	Plan By:	то	
Fig	ure:	Page 5	Council:	Great Lakes Shire Council	Survey:	Not applicable	Project Manager:	GVT	Vgc environmental compliance solutions and laboratories
Ver Dat	sion/ e:	V0 21/02/2023	Tenure:	Not Applicable	Projection:	Not applicable			This figure may be based on third party data which has not been verified by vgt and may not be to scale. Unless
Ou	r Ref:	1195_HV_SEARS_Q008_V0_BHH2 Core Box 25 to 26	Client:	Tricon Mining Equipment Pty Ltd	Contour Interval:	Not applicable			expressly agreed otherwise, this figure is intended as a guide only and vgt does not warrant its accuracy.



VGT Environmental Compliance Solutions Pty Ltd 4/30 Glenwood Drive, Thornton NSW 2322 PO Box 2335, Greenhills NSW 2323 ph: (02) 4028 6412 email: mail@vgt.com.au www.vgt.com.au ABN: 26 621 943 888

Name of Hole: DDH2

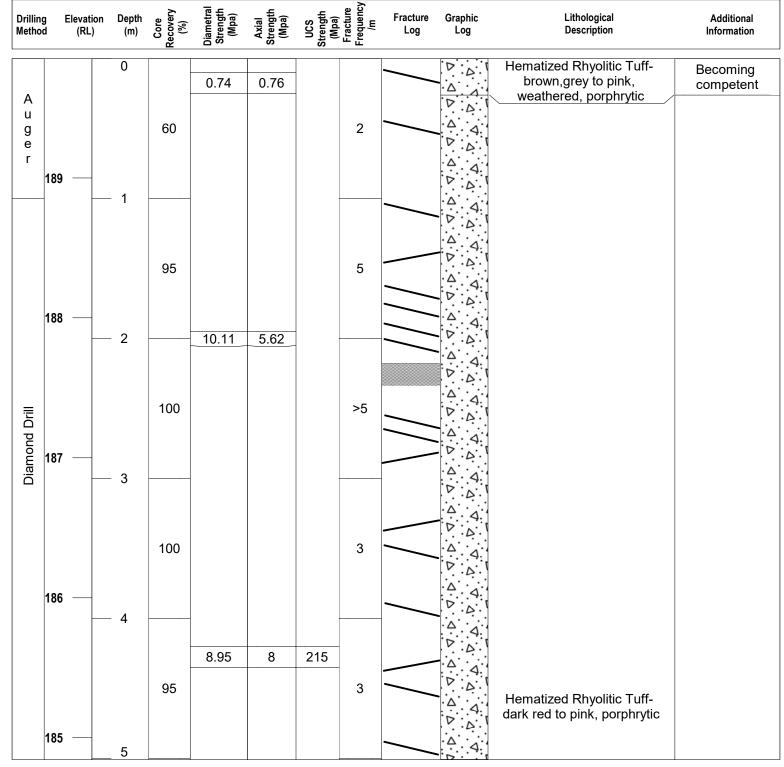
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK .og updated 03/04/2023 following ADW Survey Data	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3
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Name of Hole: DDH2

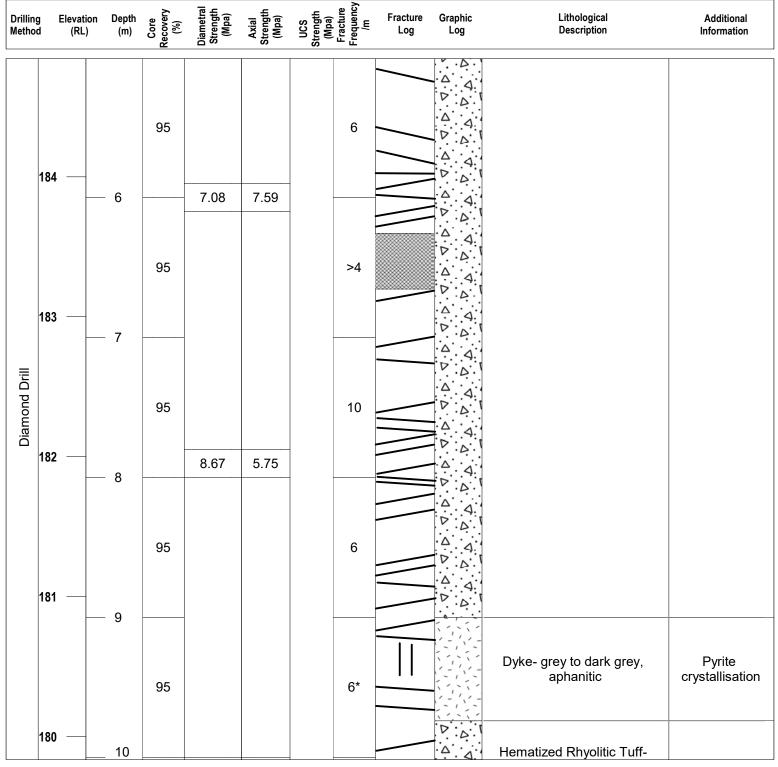
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

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Prepared By: MA/SK	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3
Log updated 03/04/2023 following ADW Survey Data	-	

Name of Hole: DDH2

Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Drilling Metho	g Elevatio d (RL)	n Depth (m)	Core Recovery (%)	Diametral Strength (Mpa)	Axial Strength (Mpa)	UCS Strength (Mpa) Fracture Frequency /m	Fracture Log	Graphic Log	Lithological Description	Additional Information
				9.54	5.28		>		dark red to pink, porphrytic	
	179 —	— 11	95			4*		0.0.0.0.0.0 0.0.0.0.0	Hematized Rhyolitic Tuff- dark red to pink, porphrytic	Feldspathic veining
	178 —	— 12	95	4.51	2.85	5			Hematized Rhyolitic Tuff- orange to pink, weathered, porphrytic Dyke- grey to dark grey, aphanitic Hematized Rhyolitic Tuff-	Plagioclase alteration Some pyrite crystallisation Some weathering
Diamond Drill	177 —		100			7			Hematized Rhyolitic Tuff- dark red to pink, porphrytic	Feldspathic veining
	470	— 13	95			1*				venning
	176 —	— 14	95	9.65	8.76	>6			Hematized Rhyolitic Tuff-dark red, porphrytic	
	175 —	15							Hematized Rhyolitic Tuff-	

Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3
Log updated 03/04/2023 following ADW Survey Data	-	

Name of Hole: DDH2

Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling

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Drillin Metho	g Elevati d (RL)	on Depth (m)	Core Recovery (%)	Diametral Strength (Mpa)	Axial Strength (Mpa)	UCS Strength (Mpa) Fracture Frequency	Fracture Log	Graphic Log	Lithological Description	Additional Information
			-	3.54	3.09				orange, weathered, friable, porphrytic	
	174 —	— 16	95			6		A . A . A . A . A . A . A	Hematized Rhyolitic Tuff- dark red to pink, porphrytic,	
			85			>6	\sim	4. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0.	Hematized Rhyolitic Tuff- orange, weathered, friable, porphrytic Dyke- grey to dark grey,	Some pyrite
	173 —	— 17		6.79	5.54				aphanitic	crystallisation
Diamond Drill	172 —	— 18	95			7		D. 0. D. 0. D. 0		
	171 —		85	10.39	7.26	6			Hematized Rhyolitic Tuff- dark red to pink, porphrytic	Some feldspathic veining
		— 19	95			7			Hematized Rhyolitic Tuff- grey to dark red, porphrytic	
	170 —	20						0.0.0	Hematized Rhyolitic Tuff- dark red to pink, porphrytic	

Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3
Log updated 03/04/2023 following ADW Survey Data	-	

Name of Hole: DDH2

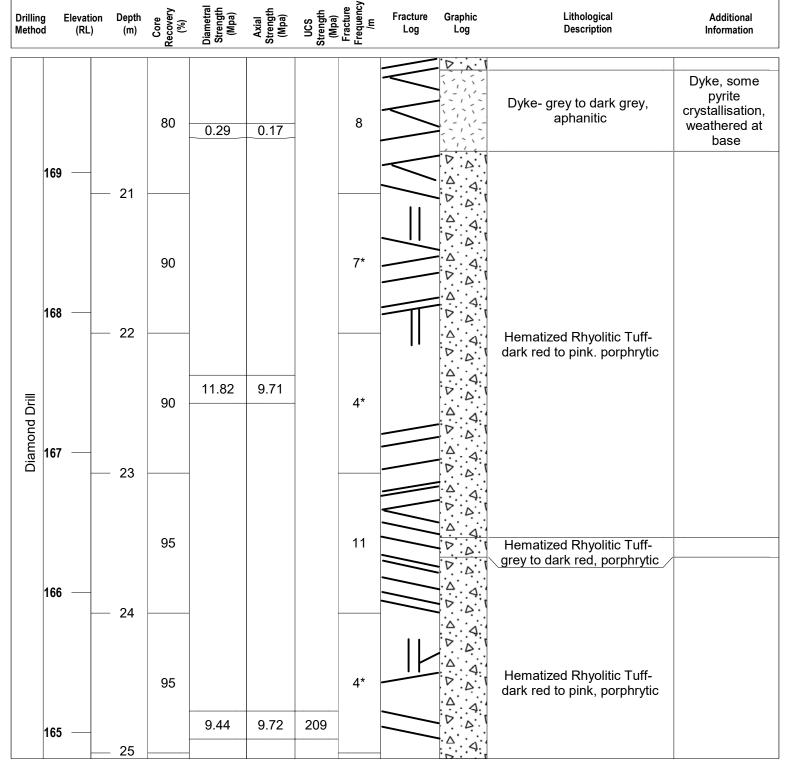
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK Log updated 03/04/2023 following ADW Survey Data	Version: 3430_HQ_GA_DDH2_V3
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Name of Hole: DDH2

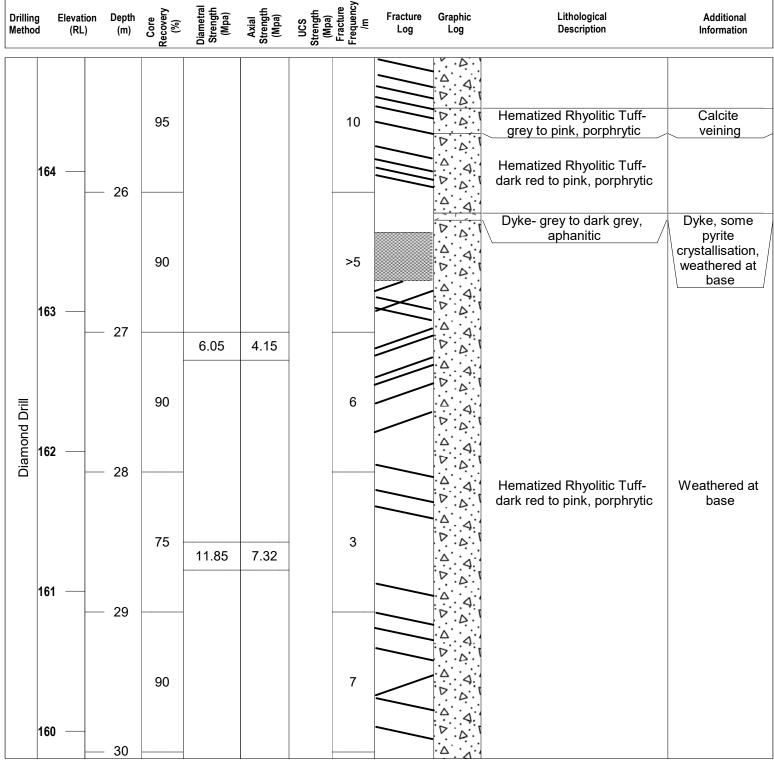
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Log updated 05/04/2025 following ADW Survey Data	Prepared By: MA/SK Log updated 03/04/2023 following ADW Survey Data	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3
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Name of Hole: DDH2

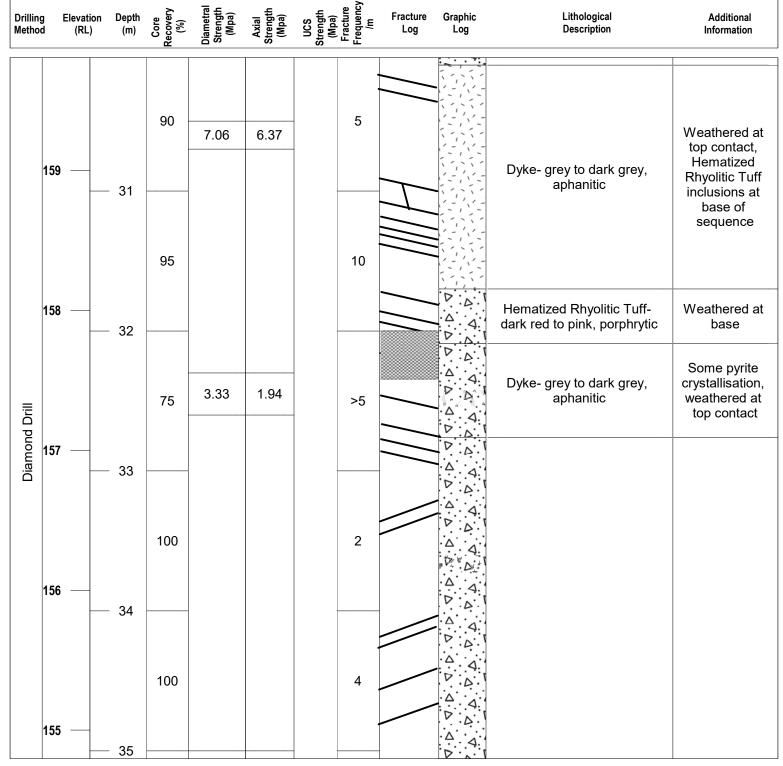
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

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Prepared By: MA/SK Log updated 03/04/2023 following ADW Survey Data	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3

Name of Hole: DDH2

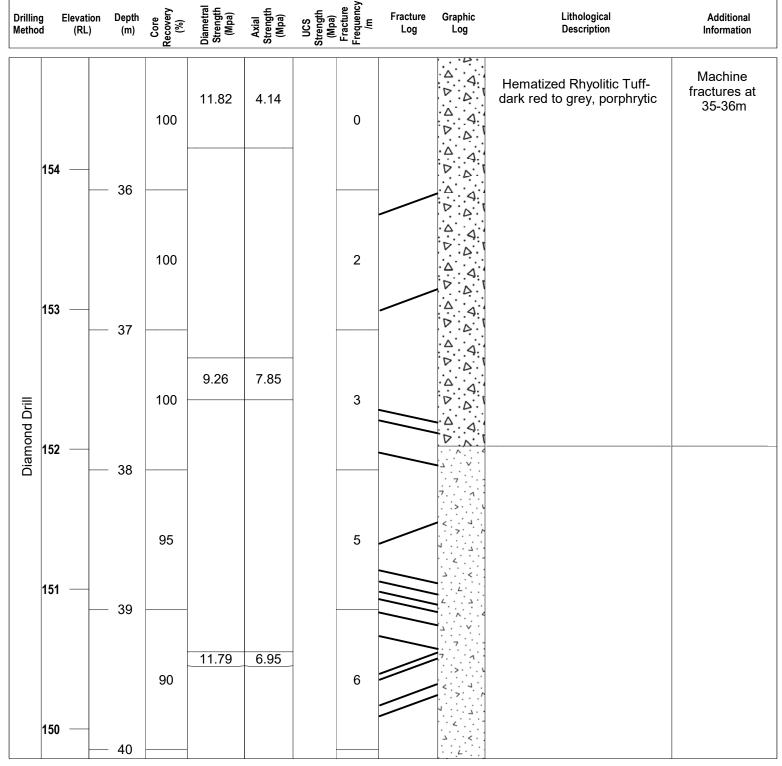
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK Log updated 03/04/2023 following ADW Survey Data	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3
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Name of Hole: DDH2

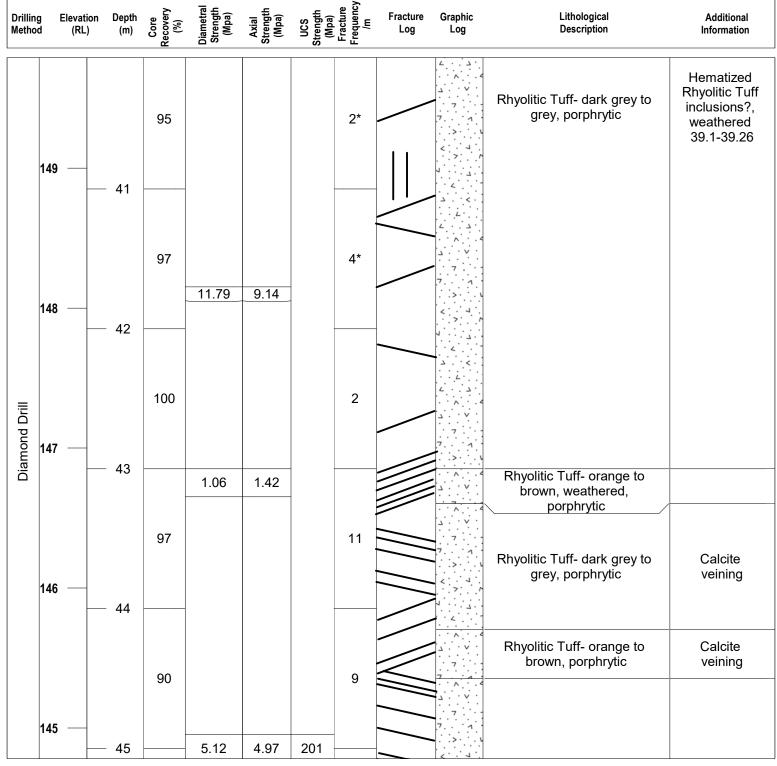
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3
Log updated 03/04/2023 following ADW Survey Data		

Name of Hole: DDH2

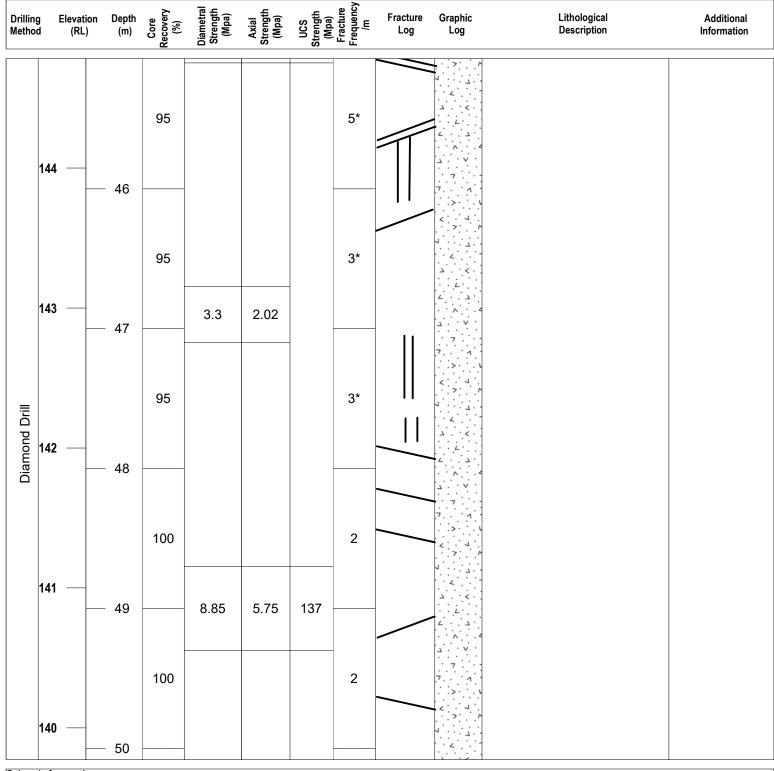
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

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Prepared By: MA/SK Log updated 03/04/2023 following ADW Survey Data	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3
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Name of Hole: DDH2

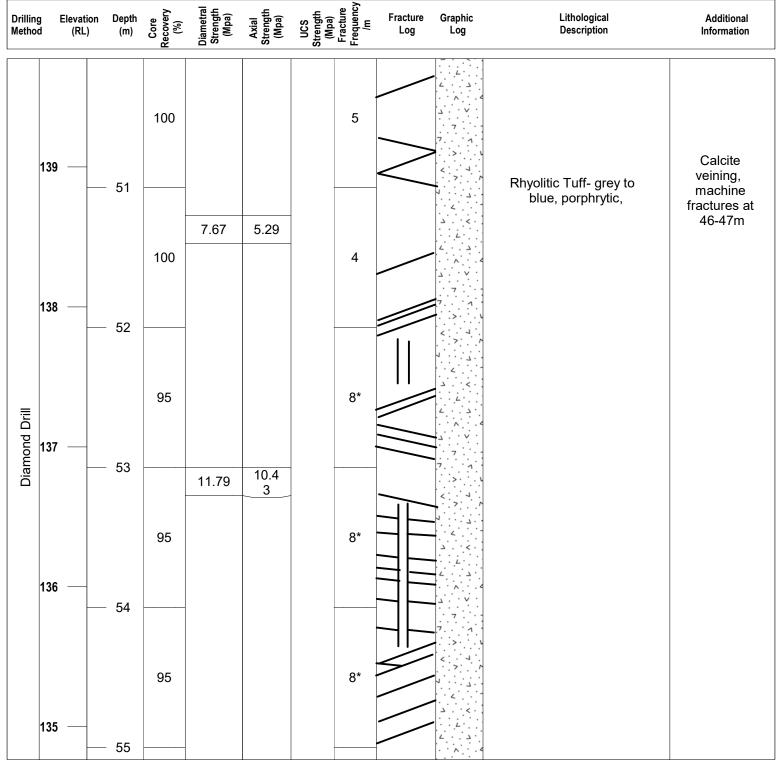
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK Log updated 03/04/2023 following ADW Survey Data	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3
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Name of Hole: DDH2

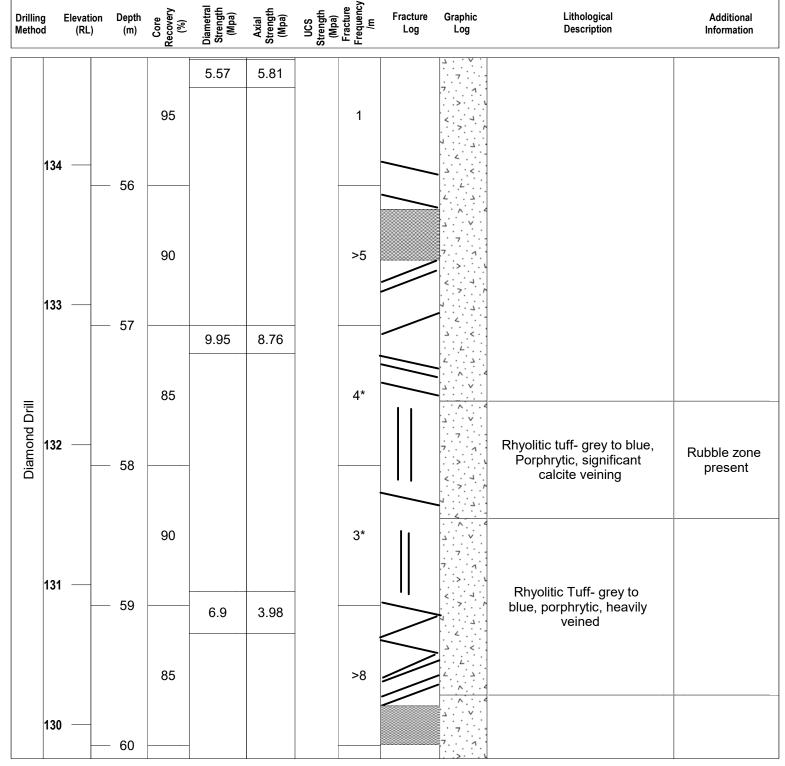
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK Log updated 03/04/2023 following ADW Survey Data	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3
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Name of Hole: DDH2

Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Drilling Method	j Ele	vation RL)	Depth (m)	Core Recovery (%)	Diametral Strength (Mpa)	Axial Strength (Mpa)	UCS Strength (Mpa) Fracture Frequency /m	Fracture Log	Graphic Log	Lithological Description	Additional Information
	129 –			80			>7		۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲	Rhyolitic Tuff- grey to blue, porphrytic, calcite veining	Rubble zone present
			- 61	0.5	1.27	1.45			· · · · · · · · · · · · · · · · · · ·		
	128 –		- 62	95			3			Rhyolitic Tuff- grey to blue, porphrytic, heavily veined	
Diamond Drill	407			95			4			Venied	
Diamo	127 –		- 63		11.79	8.34			· · · · · · · · · · · · · · · · · · ·		
	126 –		0.4	100			0				
			- 64	95			3		· · · · · · · · · · · · · · · · · · ·		
	125 –		- 65								

Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK Log updated 03/04/2023 following ADW Survey Data	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3
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Name of Hole: DDH2

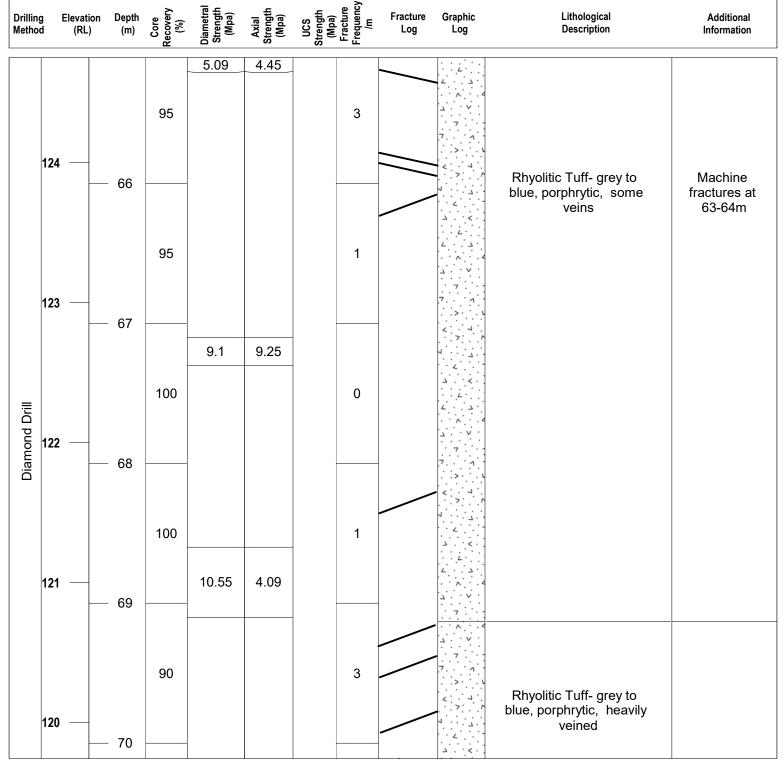
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK .og updated 03/04/2023 following ADW Survey Data	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3
--	----------------	-----------------------------

Name of Hole: DDH2

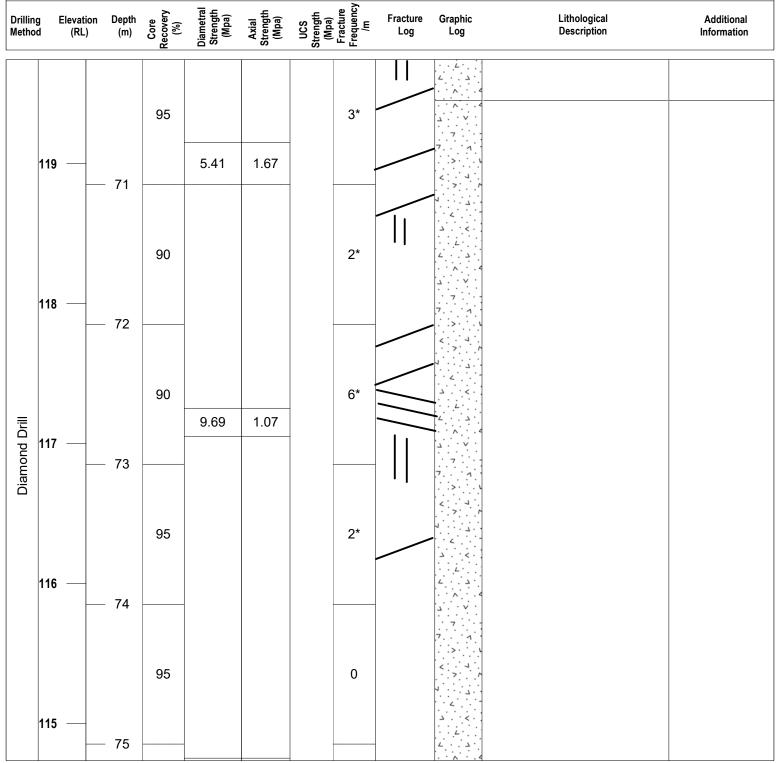
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK Checked By: SK Version: 3430_H Log updated 03/04/2023 following ADW Survey Data Checked By: SK Version: 3430_H	HQ_GA_DDH2_V3
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Name of Hole: DDH2

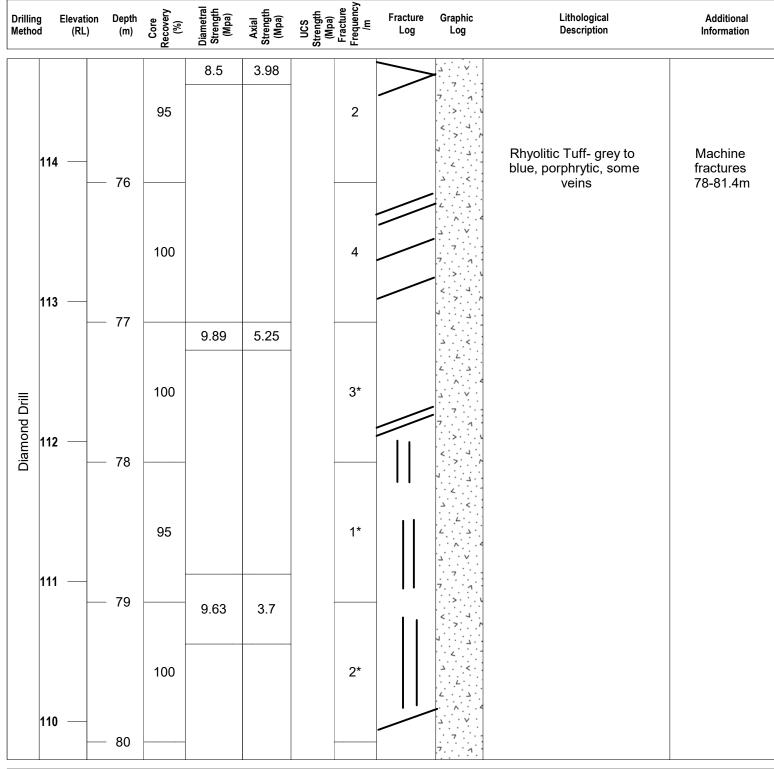
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK Log updated 03/04/2023 following ADW Survey Data	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3
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Name of Hole: DDH2

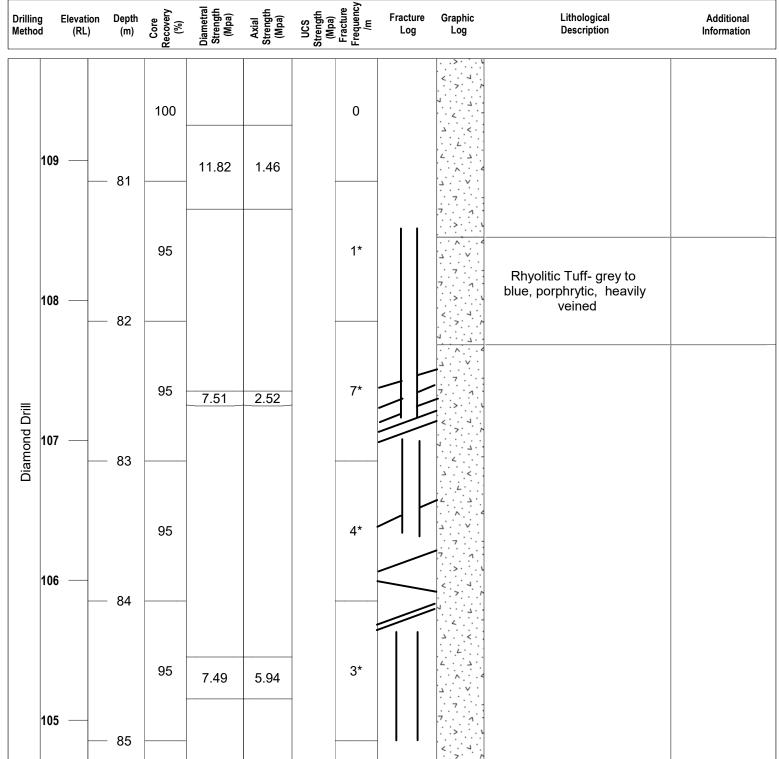
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK Log updated 03/04/2023 following ADW Survey Data	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3
-3-F		

Name of Hole: DDH2

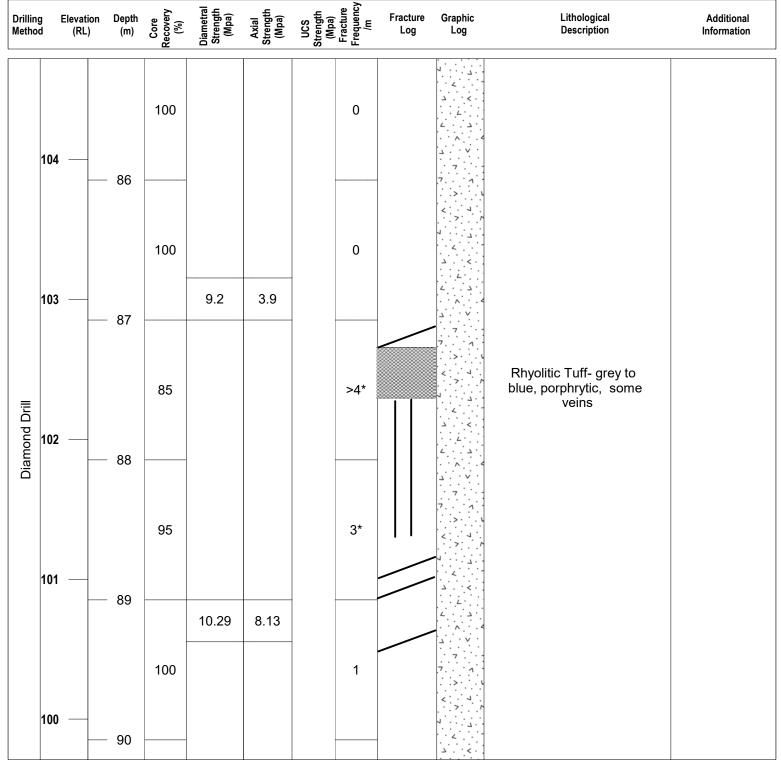
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK Log updated 03/04/2023 following ADW Survey Data	ecked By: SK	Version: 3430_HQ_GA_DDH2_V3
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Name of Hole: DDH2

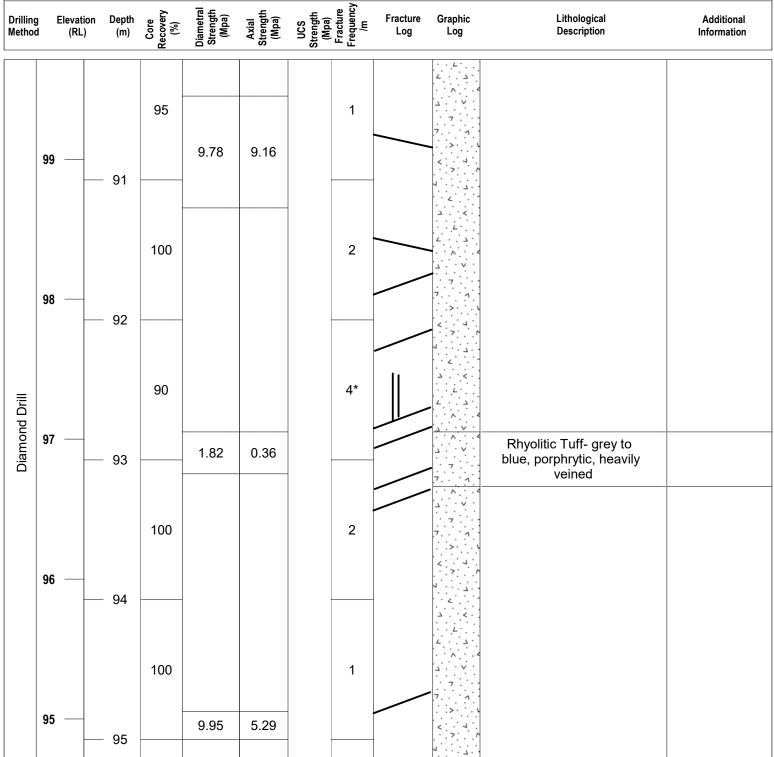
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK Log updated 03/04/2023 following ADW Survey Data	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3

Name of Hole: DDH2

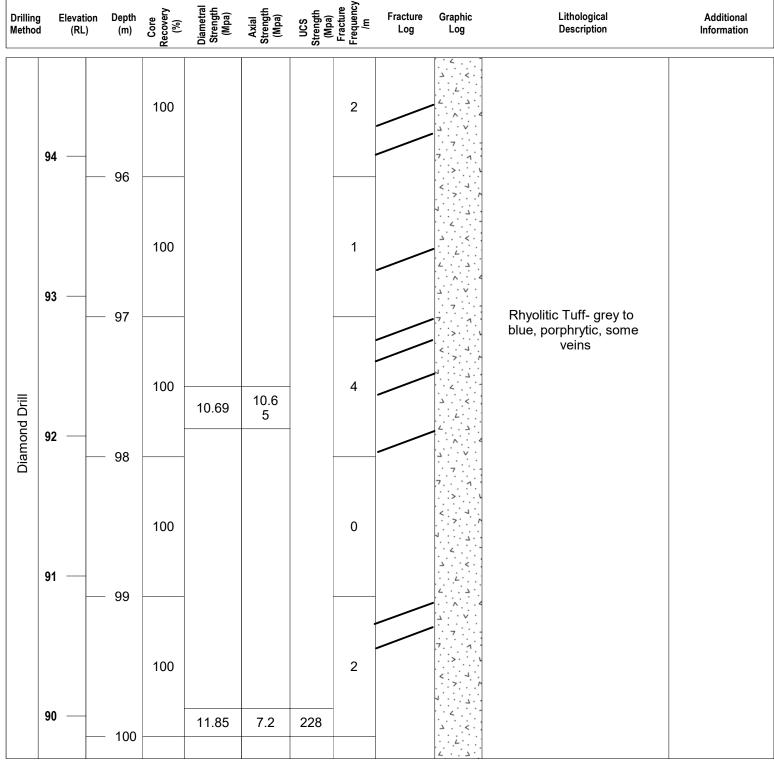
Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK .og updated 03/04/2023 following ADW Survey Data	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3
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Name of Hole: DDH2

Project Number: 3430 Client: Tricon Mining Equipment Project: Geological Assessment Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Logged: 03/01/2017 Date Commenced: 08/12/2016 Date Completed: 13/12/2016 Surface RL: 189.85 AHD Coordinates: E:398672.97 N:6404803.67 Drilling Contractor: D and E Drilling



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Drilling Nethod	Elevation (RL)	n Depth (m)	Core Recovery (%)	Diametral Strength (Mpa)	Axial Strength (Mpa)	UCS Strength (Mpa) Fracture Frequency /m	Fracture Log	Graphic Log	Lithological Description	Additional Information
Diamond Drill	89 —	— 101	85			0				
	88 —	— 102							Hole Terminated at Target Depth	
	87 —	— 103								
	86 —	— 104								
	85 —	— 105								

Other information:

Data included within this log have been sourced from field observations and geotechnical data collected and provided by Qualtest Laboratories. *: Sub Vertical Joint

Prepared By: MA/SK Log updated 03/04/2023 following ADW Survey Data	Checked By: SK	Version: 3430_HQ_GA_DDH2_V3

Name of Hole: PH 3

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced:27/09/2016 Date Completed: 29/09/2016 Surface RL: 194.38m AHD Coordinates: E: 398601 N:6404686 Drilling Contractor: Total Drilling



1 of 6

Drill Type		Water Photo Level Log	Graphic Log	Description	Additional Information
	194 — 0 193 — 1			Weathered Hematized Rhyolitic Tuff - Dark Red/Red, porphyritic.	
	192 — 2 191 — 3 191 — 4 190 — 5 189 — 5			Weathered Hematized Rhyolitic Tuff- Dark Red/Red, porphyritic	Carbonate clay
Hammer	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$			Weathered Hematized Rhyolitic Tuff- Dark Red/red, porphrytic	
	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$			Hematized Rhyolitic Tuff- dark purple/grey,	Some weathering
	175 — 19 20	CENTS			

Other Information: No Piezometer installed Log updated 03/04/2023 following ADW Survey Data

Logged By: MA/SK	Checked By: SK	3430_TH_GA_LOG_PH3_V2	

Name of Hole: PH 3

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced:27/09/2016 Date Completed: 29/09/2016 Surface RL: 194.38m AHD Coordinates: E: 398601 N:6404686 Drilling Contractor: Total Drilling



2 of 6

Drill Type	Elevation (RL)	Depth (m)	Water Level	Photo Log	Graphic Log	Description	Additional Information
Hammer	174 173 172 171 171 170 169 168 167 166 165 164 163 164 163 161 160	 21 22 23 24 25 26 27 28 29 30 31 32 33 34 				Rhyolitic Tuff, dark purple/grey, porphyritic	
	159 —	— 35 — 36	110			Hematized Rhyolitic Tuff/ Dyke-light grey to dark grey, porphrytic to aphanitic	Calcite precipitation
	158 — 157 —	37				Dyke- dark grey to blue, aphanitic	Pyrite crystalisation, magnetic,calcite precipitation
	156 —	- 38				Weathered Rhyolitic Tuff- dark blue/grey, porphyritic	
	155 —	39 40	ALMA A		· · · · · · · · · · · · · · · · · · ·		

Other Information: No Piezometer installed Log updated 03/04/2023 following ADW Survey Data

Logged By: MA/SK	Checked By: SK	3430_TH_GA_LOG_PH3_V2

Name of Hole: PH 3

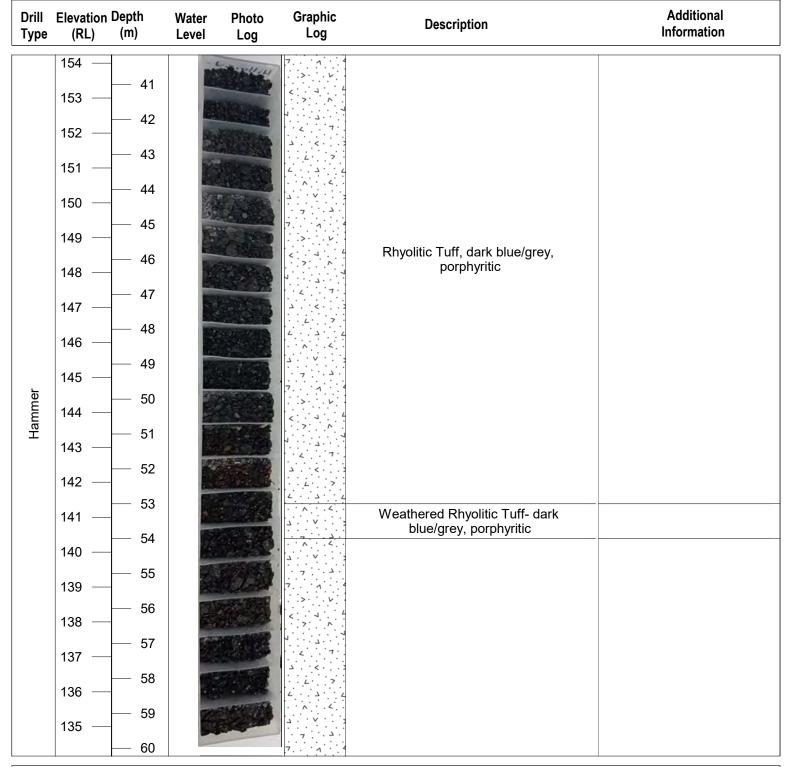
Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced:27/09/2016 Date Completed: 29/09/2016 Surface RL: 194.38m AHD Coordinates: E: 398601 N:6404686 Drilling Contractor: Total Drilling



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Logged By: MA/SK	Checked By: SK	3430_TH_GA_LOG_PH3_V2

Name of Hole: PH 3

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced:27/09/2016 Date Completed: 29/09/2016 Surface RL: 194.38m AHD Coordinates: E: 398601 N:6404686 Drilling Contractor: Total Drilling



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Logged By: MA/SK	Checked By: SK	3430_TH_GA_LOG_PH3_V2

Name of Hole: PH 3

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced:27/09/2016 Date Completed: 29/09/2016 Surface RL: 194.38m AHD Coordinates: E: 398601 N:6404686 Drilling Contractor: Total Drilling



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Drill Type	Elevation (RL)	Depth (m)	Water Level	Photo Log	Graphic Log	Description	Additional Information
	114 — 113 —	81	2	Calify a	· · · · · · · · · · · · · · · · · · ·		
	112 —	- 82					
	112 -	- 83		131234			
	110 —	- 84			· · · · · · · · · · · · · · · · · · ·		
	109 —	- 85			, , , , , , , , , , , , , , , , , , ,		
	108 —	86			· · · · · · · · · · · · · · · · · · ·		
	107 —	87			· · · · · · · · · · · · · · · · · · ·	Rhyolitic Tuff- Grey/blue to red,	Some weathering,
	106 —	88		12.49	· · · · · · · · · · · · · · · · · · ·	porphyritic.	magnetic, calcite veining
	105 —	89		No. Con	· · · · · · · · · · · · · · · · · · ·		
Hammer	104 —	90		N 160	<		
	103 —	91		and a	× · · · · · · · · · · · · · · · · · · ·	Rhyolitic Tuff, dark blue/grey,	
	102 —	92			۲۰۰۰ ۲۰۰۰ ۲	porphyritic	
	101 —	93			L		
	100 —	94					
	99 —	95		a sub	4 · · · · · · · · · · · · · · · · · · ·	Rhyolitic Tuff Grey/blue to red,	Some weathering,
	98 —	96			· · · · · · · · · · · · · · · · · · ·	porphyritic.	magnetic, calcite veining
	97 —	97		-	· · · · · · · · · · · · · · · · · · ·		
	96 —	98		100	4	Rhyolitic Tuff, dark blue/grey, porphyritic	
	95 —	99			N		
	94	100			· · · · · · · · · · · · · · · · · · ·		

Logged By: MA/SK	Checked By: SK	3430_TH_GA_LOG_PH3_V2

Name of Hole: PH 3

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced:27/09/2016 Date Completed: 29/09/2016 Surface RL: 194.38m AHD Coordinates: E: 398601 N:6404686 Drilling Contractor: Total Drilling



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$\begin{bmatrix} 87 \\ - \\ 86 \\ - \\ 108 \\ 85 \\ - \\ 109 \\ 84 \\ - \\ 110 \\ 83 \\ - \\ 111 \\ 82 \\ - \\ 112 \\ 82 \\ - \\ 112 \\ 82 \\ - \\ 112 \\ 81 \\ - \\ 113 \\ 80 \\ - \\ 114 \\ 80 \\ - \\ 114 \\ 80 \\ - \\ 114 \\ 80 \\ - \\ 114 \\ 80 \\ - \\ 116 \\ 79 \\ - \\ 115 \\ - \\ 79 \\ - \\ 115 \\ - \\ 79 \\ - \\ 116 \\ 78 \\ - \\ 116 \\ 78 \\ - \\ 116 \\ 77 \\ - \\ 117 \\ 76 \\ - \\ 118 \\ - \\ 118 \\ - \\ 118 \\ - \\ 118 \\ - \\ 118 \\ - \\ 110 \\ - \\ 100 \\ - \\ 100 \\ - \\ 100 \\ - \\ 100 \\ - \\ 100 \\ - \\ 100 \\ - \\ 100 \\ - \\ 100 \\ - \\ 1$	Drill Type	Elevation (RL)	Depth (m)	Water Level	Photo Log	Graphic Log	Description	Additional Information
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$		94 93 92 91 90 88 87 86 85 84 83	 101 102 103 104 105 106 107 108 109 110 111 112 				Rhyolitic Tuff, dark blue/grey, porphyritic	contamination, weathered
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$			-			L	Hole terminated due to difficult drilling conditions	
$\begin{array}{ c c c c c }\hline 77 & - & 117 \\ \hline 77 & - & 118 \\ \hline 76 & - & 118 \end{array}$		79 — 78 —	116					
		77 —	- 117					
		76 — 75 —						

Logged By: MA/SK	Checked By: SK	3430_TH_GA_LOG_PH3_V2

Name of Hole: PH 4

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 04/10/2016 Date Completed: 11/10/2016 Surface RL: 189.83 AHD Coordinates: E: 398671.72 N: 6404803.76 Drilling Contractor: Total Drilling



1 of 7

Drill Type	Elevation D (RL))epth (m)		eter Photo Log	Graphic Log	Description	Additional Information
	189 — 188 —		0	HARRING .		Weathered Hematized Rhyolitic Tuff- Cream to red in colour, porphrytic.	Clayey
	187 —		2			Hematized Rhyolitic Tuff/Dyke- red to grey/black aphanitic to porphrytic	
	186 —		4	all and a			
	185 —	!	5			Hematized Rhyolitic Tuff- Red to purple, porphrytic	
	184 — 183 —	(6			pulple, polphrytic	
	182 —		7				
	181 —		8			Hematized Rhyolitic Tuff/Dyke- red to grey/black aphanitic to porphrytic	
Hammer	180 —		10	200			Dykes, magnetic
Ham	179 —		11		D		
	178 —		12				
	177 —		13			Hematized Rhyolitic Tuff- red/purple,	
	176 —		14				
	175 — 174 —		15	ARA		porphrytic	
	173 —		16 17				
	172 —		18	No.			
	171 —		19			Fines- brown to cream in colour, isolated fragments of Rhyolitic Tuff	Magnetic, possible fault/shear zone?
	170 —		20	anotana			

Other Information: Hole Drilled to 140m, Piezo Installed to RL 123m due to blockage. Log updated 03/04/2023 following ADW Survey Data

Logged By: MA	Checked By: SK	Version: 3430_TH_LOG_PH4_V2

Name of Hole: PH 4

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 04/10/2016 Date Completed: 11/10/2016 Surface RL: 189.83 AHD Coordinates: E: 398671.72 N: 6404803.76 Drilling Contractor: Total Drilling



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Drill Type	Elevation (RL)	Depth (m)				Additional Information
	169 — 168 — 167 — 166 — 165 —		21 22 23 24 25		Hematized Rhyolitic Tuff- red/purple, porphrytic	Minute traces of calcite
	163 — 162 —		26 27		Hematized Rhyolitic Tuff/Dyke- red to grey/black aphanitic to porphrytic	Dyke, magnetic
ler	161 — 160 —		28 29		Hematized Rhyolitic Tuff- red to purple porphrytic	
Hammer	159 —		30		Hematized Rhyolitic Tuff/Dyke- red to grey/black aphanitic to porphrytic	
	158 —		31		Dyke- black to dark grey, fine crystal size, aphanitic texture.	Pyrite crystallisation
	157 — 156 — 155 — 154 —		 32 33 34 35 36 		Hematized Rhyolitic Tuff/Dyke- red to grey/black aphanitic to porphrytic	Calcite precipitation at 35-36m, magnetic
	153 — 152 — 151 —		37		Hematized Rhyolitic Tuff - red in Colour, porphrytic	Calcite precipitation, magnetic
	150 —		39 40		Rhyolitic Tuff- grey/blue to brown,	

Other Information: Hole Drilled to 140m, Piezo Installed to RL 123m due to blockage. Log updated 03/04/2023 following ADW Survey Data

Logged By: MA Checked By: SK Version: 3430_TH_LOG_PH4_V2

Name of Hole: PH 4

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 04/10/2016 Date Completed: 11/10/2016 Surface RL: 189.83 AHD Coordinates: E: 398671.72 N: 6404803.76 Drilling Contractor: Total Drilling



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Drill Type	Elevation I (RL)	Depth (m)			Graphic Log	Description	Additional Information
	149 —		44	- Consellar	7. 1 4 	Altered, porphrytic,	
	148 —		41 42		7 · · · · · · · · · · · · · · · · · · ·	Rhyolitic Tuff- grey to blue in colour, porphrytic	Magnetic
	147 —		42 43			Rhyolitic Tuff- grey to blue in colour, porphrytic	Magnetic, calcite precipitation
	146 —		44		· · · · · · · · · · · · · · · · · · ·		Magnetic, calcite
	145 —		45		7. 1 4 	Rhyolitic Tuff- grey blue to red, slightly weathered, porphrytic	precipitation, some quartz veining
	144 —		46		· · · · · · · · · · · · · · · · · · ·		
	143 —		47		· · · · · · · · · · · · · · · · · · ·	Rhyolitic Tuff- grey to blue in colour,	Calcite precipitation, magnetic, some quartz
	142 —	 	48		<	porphrytic	veining
۲.	141 —		49				
Hammer	140 —		50		· · · · · · · · · · · · · · · · · · ·		
T	139 —		50 51		<		
	138 —		52	AND SECURING	ч ч ч ч ч ч ч ч ч		
	137 —		53				
	136 —		54		2 · · · · · · · · · · · · · · · · · · ·		
	135 —		55		7.31 7.31 		
	134 —		-				
	133 —		57				
	132 —		56 57 58 59 60				
	131 —		59		· · · · · · · · · · · · · · · · · · ·		
	130 —		60		7		

Other Information: Hole Drilled to 140m, Piezo Installed to RL 123m due to blockage. Log updated 03/04/2023 following ADW Survey Data

Logged By: MA	Checked By: SK	Version: 3430_TH_LOG_PH4_V2

Name of Hole: PH 4

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 04/10/2016 Date Completed: 11/10/2016 Surface RL: 189.83 AHD Coordinates: E: 398671.72 N: 6404803.76 Drilling Contractor: Total Drilling



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Drill Type	Elevation (RL)	Depth (m)	Piezom Design	neter Pho Lo		Graphic Log	Description	Additional Information
	129 —		61			L	Rhyolitic Tuff- grey blue to red, slightly weathered, porphrytic	Quartz Veining, magnetic, calcite precipitation
	128 —		62		··· ···	< · · · · ·		
	127 —		62 63	13	۲			
	126 —		64					
	125 —		65	FREE				
	124 —		66		· · · · · · · · · · · · · · · · · · ·			
	123 —	 	67	To and the second se	· · · · · · · · · · · · · · · · · · ·	2		
	122 —		68			· · · · · ·		
ŗ	121 —		68 69	-	· · · · · · · · · · · · · · · · · · ·	1.4		
Hammer	120 —		70		< · · · · · · · · · · · · · · · · · · ·			
T	119 —		71 72		· · · · · · · · · · · · · · · · · · ·	л. 7. <.		
	118 —	-	72			· ۲ ـ ۲		
	117 —		73	A STREET			Rhyolitic Tuff- grey to blue in colour,	Possible fault
	116 —		73 74			<u></u>	porphrytic	
	115 —		75		· · · · · · · · · · · · · · · · · · ·			
	114 —		76			ч ч ч ч		
	113 —	-	77					
	114 — 113 — 112 — 111 — 110 —		78		. د د	< · · · · · · · · · · · · · · · · · · ·		
	111 —		79			× · · · · ·		
	110 —		80		· 7 			

Other Information: Hole Drilled to 140m, Piezo Installed to RL 123m due to blockage. Log updated 03/04/2023 following ADW Survey Data

Logged By: MA	Checked By: SK	Version: 3430_TH_LOG_PH4_V2

Name of Hole: PH 4

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 04/10/2016 Date Completed: 11/10/2016 Surface RL: 189.83 AHD Coordinates: E: 398671.72 N: 6404803.76 Drilling Contractor: Total Drilling



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Drill Type	Elevation (RL)	Depth Pie (m) Des	zometer sign	Photo Log	Graphic Log	Description	Additional Information
	109 —	- 81			× × × ×		
	108 —	- 82		123			
	107 —	- 83			· · · · · · · · · · · · · · · · · · ·		
	106 —	- 84			· · · · · · · · · · · · · · · · · · ·		
	105 —	- 85			7. 7 . C		
	104 —	- 86			4 		
	103 —	- 87			· · · · · · · · · · · · · · · · · · ·		
	102 —	- 88			L. 7 L		
	101 —	- 89			· · · · · · · · · · · · · · · · · · ·		
Hammer	100 —	90			· · · · · · · · · · · · · · · · · · ·		
На	99 —	91					
	98 —	92		and a	<		
	97 —	93			۷ · ۰ · ۰ · ۰		
	96 —	- 94			۰ د ₇		
	95 —	- 95			· · · · · · · · · · · · · · · · · · ·		
	94 —	06		263	N		
	93 —	- 97		2.6	۲۰۰۲ کر ۲۰۰۶ ۲۰۰۲ ک	Rhyolitic Tuff- grey to blue/ red, porphrytic	Slight weathering, quartz veined, calcite precipitation, magnetic
	92 —	98		the s	7	Po. P. 1 740	magnetic
	91 —	— 99			7. 1.4 		
	90 —	— 100			4		

Other Information: Hole Drilled to 140m, Piezo Installed to RL 123m due to blockage. Log updated 03/04/2023 following ADW Survey Data

Logged By: MA Checked By: SK Version: 3430_TH_LOG_PH4_V2

Name of Hole: PH 4

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 04/10/2016 Date Completed: 11/10/2016 Surface RL: 189.83 AHD Coordinates: E: 398671.72 N: 6404803.76 Drilling Contractor: Total Drilling



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Drill Type	Elevation (RL)	Depth Piezor (m) Desig	neter Photo n Log	Graphic Log	Description	Additional Information
	89 —	101	i Cililia	۲۰۰ ، ۲۰۰ ، ۲۰۰ ، ۲۰۰ ، ۲۰۰ ، ۲۰۰ ، ۲۰۰ ، ۲۰۰		
	88 —	102		· · L · 7 · L		
	87 —	103				
	86 —	104		· · · · · · · · · · · · · · · · · · ·		
	85 —	105				
	84 —	106		· · · · · · · · · · · · · · · · · · ·		
	83 —	107		· · · · · · · · · · · · · · · · · · ·		
	82 —	108		7.7.4		
<u>ب</u>	81 —	109		< J		
Hammer	80 —	- 110		· · · · · · · · · · · · · · · · · · ·		
T	79 —	111		L		
	78 —	112		· · · · · د · · · د		
	77 —	113				
	76 —	114		7. ^ /		
	75 —	115		۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲ ۲		
	74 —	1105		ے		
	73 —			· · · · · · · · · · · · · · · · · · ·		
	72 —	118		<		
	71 —	- 119 - 120		2 7.		
	70 —		c			

Other Information: Hole Drilled to 140m, Piezo Installed to RL 123m due to blockage. Log updated 03/04/2023 following ADW Survey Data

Logged By: MA	Checked By: SK	Version: 3430_TH_LOG_PH4_V2

Name of Hole: PH 4

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 04/10/2016 Date Completed: 11/10/2016 Surface RL: 189.83 AHD Coordinates: E: 398671.72 N: 6404803.76 Drilling Contractor: Total Drilling



7 of 7

Drill Type		Depth (m)	Piezometer Design	Photo Log	Graphic Log	Description	Additional Information
	69 — 68 —					Dyke- black to dark grey, aphanitic	Calcite veining, magnetic
	67 — 66 —		123 ⁹		· · · · · · · · · · · · · · · · · · ·		
	65 —		125				
	64 —		126		ل ۲۰.		
	63 — 62 —		127 128		۲ <u>۲</u> ۲ ۲۰۰۲ ۲		
	61 —		129			Rhyolitic Tuff- grey to blue slightly Slight We red, porphrytic veine	
Hammer	60 —		130		× × × × × ×		
Т	59 —		131				Slight Weathering, quartz veined, magnetic
	58 — 57 —		132 133				
	56 —		134		· · · · · · · · · · · · · · · · · · ·		
	55 —		135		<		
	54 —				 <!--</td--><td></td>		
	53 — 52 —		137				
	51 —		138 139				
	50 —		1000				

Other Information: Hole Drilled to 140m, Piezo Installed to RL 123m due to blockage. Log updated 03/04/2023 following ADW Survey Data

Logged By: MA	Checked By: SK	Version: 3430_TH_LOG_PH4_V2		

Name of Hole: PH 5

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 12/10/2016 Date Completed: 27/10/2016 Surface RL:180.93 m AHD Coordinates: E: 398462.10 N: 6404674.32 Drilling Contractor: Total Drilling



1 of 5

Drill Type	Elevation Depth Piez (RL) (m) Desi	cometer Photo ign Log	Graphic Log	Description	Additional Information
	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$			Weathered Hematized Rhyolitic Tuff- orange to red, porphrytic,	Clayey
Hammer	177 - 4 $176 - 5$ $175 - 6$ $174 - 7$ $173 - 8$ $172 - 9$ $171 - 10$ $170 - 11$ $169 - 12$ $168 - 13$ $167 - 14$ $166 - 15$			Hematized Rhyolitic Tuff-Red to orange, porphrytic	Magnetic, some weathering
	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$			Hematized Rhyolitic Tuff- orange, red to blue, dark grey, porphrytic	Magnetic, some weathering

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA	Checked By:SK	Version: 3430_TH_GA_LOG_PH5_V3
	· · · · , ·	

Name of Hole: PH 5

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 12/10/2016 Date Completed: 27/10/2016 Surface RL:180.93 m AHD Coordinates: E: 398462.10 N: 6404674.32 Drilling Contractor: Total Drilling



2 of 5

Drill	Elevation Depth Piezometer	Photo	Graphic	Description	Additional
Type	(RL) (m) Design	Log	Log		Information
Hammer	160 21 159 22 158 23 157 24 156 25 155 26 154 27 153 28 152 29 151 30 150 31 149 32 146 35 144 37 143 38 142 39 141 40			Hematized Rhyolitic Tuff- orange red to purple, porphrytic	Magnetic, some weathering

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA Checked By:SK Version: 3430_TH_GA_LOG_PH5_V3

Name of Hole: PH 5

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 12/10/2016 Date Completed: 27/10/2016 Surface RL:180.93 m AHD Coordinates: E: 398462.10 N: 6404674.32 Drilling Contractor: Total Drilling



3 of 5

Drill	Elevation Depth Piezom	phic Description	Additional
Type	(RL) (m) Design	og	Information
	(RL) (m) Design $140 - 41$ $139 - 42$ $138 - 43$ $137 - 44$ $136 - 45$ $135 - 46$ $134 - 47$ $133 - 48$ $132 - 49$ $131 - 50$ $130 - 51$ $129 - 52$ $128 - 53$ $127 - 54$ $126 - 55$ $125 - 56$ $124 - 57$		Information

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA Checked By:SK Version: 3430_TH_GA_LOG_PH5_V3

Name of Hole: PH 5

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 12/10/2016 Date Completed: 27/10/2016 Surface RL:180.93 m AHD Coordinates: E: 398462.10 N: 6404674.32 Drilling Contractor: Total Drilling



4 of 5

Drill Type	Elevation (RL)	Depth P (m) D	iezometer esign	Photo Log	Graphic Log	Description	Additional Information
	120 —	61			· · · · · · · · · · · · · · · · · · ·		
	119 —	- 62					
	118 —	- 63			<		
	117 —	- 64			ے		
	116 —	- 65			· · · · · · · · · · · · · · · · · · ·		
	115 —	- 66			· · · · · · · · · · · · · · · · · · ·		
	114 —	- 67					
	113 —	68			· · · · · · · · · · · · · · · · · · ·		
Ļ	112 —	- 69			7		
Hammer	111 —	- 70		-	· · · · · · · · · · · · · · · · · · ·		
Ĩ	110 —	- 71			<		
	109 —	- 72					
	108 —	- 73			· . L		
	107 —	- 74		-	· · · · · · · · · · · · · · · · · · ·		
	106 —	- 75			7. 7 K		
	105 —	- 76			4.7.7		
	104 —	- 77			2 7		
	103 —	- 78			L. 7 L		
	102 —	- 79			· · · · · · · · · · · · · · · · · · ·		
	101 —	- 80			· · · · · · · · · · · · · · · · · · ·		

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA Checked By:SK Version: 3430_TH_GA_LOG_PH5_V3

Name of Hole: PH 5

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 12/10/2016 Date Completed: 27/10/2016 Surface RL:180.93 m AHD Coordinates: E: 398462.10 N: 6404674.32 Drilling Contractor: Total Drilling



5 of 5

99 - 82 98 - 83 97 - 84 96 - 85 95 - 86 94 - 87 93 - 88 92 - 89 91 - 90	7 Λ ζ 2 7 Λ 3 ζ 7 4 ζ 7 4 ζ 7 4 ζ 7 4 ζ 7 4 ζ 7 5 ζ ζ 7 Λ ζ <	ematized Rhyolitic Tuff/Dyke - Red to dark grey/dark blue, porphrytic to anphinitic	Possible fault/shear zone, some weathering

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA

Name of Hole: PH7

Project Number: 3430 Client: Tricon Mining Equipment Project: Hillview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 07/12/2016 Date Completed: 07/12/2016 Surface RL: 190.25m AHD Coordinates: E:398673.19 N:6404802.76 Drilling Contractor: D and E Drilling



1 of 3

Drill Type	Elevation (RL)	Depth (m)	Piezometer Design	Photo Log	Graphic Log	Description	Additional Information
Auger	189 —	0				Weathered Rhyodacite- pink to brown, Porphrytic	20cm topsoil
	188 — 187 — 186 — 185 — 184 —	2 3 4 5 6				Hematized Rhyolitic Tuff- Pink to orange, porphyritic	Some weathering
Hammer	183 — 182 — 181 — 180 —	— 7 — 8 — 9				Hematized Rhyolitic Tuff, pink, purple to orange, porphrytic	Slight weathering,
Han	179 — 178 — 177 —	1: 1:	2			Hematized Rhyolitic Tuff/ Dyke- pink, purple to dark grey/blue, porphrytic to aphanitic	Multiple small dykes?,magnetic
	176 — 175 — 174 —	14 19 10	6			Hematized Rhyolitic Tuff- pink, purple to orange, porphrytic	Slight weathering
	173 — 172 —	— 1		1.1.5		Hematized Rhyolitic Tuff/ Dyke- pink, purple to dark grey/blue, porphrytic to aphanitic	Hematite nodule?, dykes, magnetic
	171 —		8 9 1			Hematized Rhyolitic Tuff- pink to purple, porphrytic	
	170 —	2				Hematized Rhyolitic Tuff/ Dyke- pink, purple	

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA Checked By: SK Version: 3430_TH_GA_LOG_PH7_V3

Name of Hole: PH7

Project Number: 3430 Client: Tricon Mining Equipment Project: Hillview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 07/12/2016 Date Completed: 07/12/2016 Surface RL: 190.25m AHD Coordinates: E:398673.19 N:6404802.76 Drilling Contractor: D and E Drilling



2 of 3

Drill Type	Elevation Depth Piezomete (RL) (m) Design	r Photo Log	Graphic Log	Description	Additional Information
		Part and they		to dark grey/blue, porphrytic to aphanitic	Dyke
	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	A CARA		Weathered Hematized Rhyolitic Tuff- pink to brown, porphrytic	
	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$			Hematized Rhyolitic Tuff- pink to purple,	
		aká d		porphrytic	
	165 - 25		P P	Hematized Rhyolitic Tuff- pink, purple to orange, porphrytic	Some weathering
	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$			Hematized Rhyolitic Tuff/ Dyke- pink, purple to dark grey/blue, porphrytic to aphanitic	Magnetic
		AE 3		Hematized Rhyolitic Tuff, pink, purple to	
<u> </u>	161 — 29			orange, porphrytic	Some weathering
Hammer	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$			Hematized Rhyolitic Tuff/ Dyke- pink, purple to dark grey/blue, porphrytic to aphanitic	Dyke
	159 — 31 — 158 — 32 — 31	4			
				Dyke- dark grey to dark blue, aphanitic	Magnetic
	154 - 36			Hematized Rhyolitic Tuff/ Dyke- pink, purple	
	153 — 37 152 — 38			Hematized Rhyolitic Tuff/ Dyke- pink, purple to dark grey/blue, porphrytic to aphanitic	Dyke
	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$				
		ATTAIN T			

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA	Checked By: SK	Version: 3430_TH_GA_LOG_PH7_V3
33 3	j i i i i j i	

Name of Hole: PH7

Project Number: 3430 Client: Tricon Mining Equipment Project: Hillview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 07/12/2016 Date Completed: 07/12/2016 Surface RL: 190.25m AHD Coordinates: E:398673.19 N:6404802.76 Drilling Contractor: D and E Drilling



3 of 3

Drill Type		Depth (m)	Piezometer Design	Photo Log	Graphic Log	Description	Additional Information
	149 — 148 —	— 4 ² — 42				Rhyolitic tuff/Dyke- Dark grey to dark blue, porphrytic to aphanitic	Dyke
Hammer	147 — 146 — 145 —	— 42 — 43 — 44 — 48			· · · · · · · · · · · · · · · · · · ·	Rhyolitic tuff - dark grey/blue to orange, Porphrytic	Quartz veining, some weathering, some water intercepted @ 44m
	144 — 143 —	— 46 — 47				Hole Terminated at Target Depth	
	142 —	- 48	8				

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA Checked By: SK Version: 3430_TH_GA_LOG_PH7_V3

Name of Hole: PH 8

Project Number: 3430 Client: Tricon Mining Equipment Project: Hillview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 13/12/2016 Date Completed: 13/12/2016 Surface RL: 181.15m AHD Coordinates: E:398463.58 N: 6404673.63 Drilling Contractor: D and E Drilling



1 of 3

Drill Type	Elevation (RL)		Piezometer Design	Photo Log	Graphi Log		Additional Information
	181 — 180 — 179 — 178 —	0 1 3				Weathered Hematized Rhyolitic Tuff- orange to brown, porphrytic	Dyke, Some weathering
	177 — 176 —	4 5				Hematized Rhyolitic Tuff- brown to orange, porphrytic	Some weathering
	175 — 174 — 173 —	6 7 8				Weathered Dyke/Hematized Rhyolitic Tuff- prange, brown to grey, porphrytic to aphanitic	Magnetic
Hammer	172 171 170 169 168 167 166 165 164 163	1 1 1 1	0 1 2 3 4 5 6 7 8 9 0			łematized Rhyolitic Tuff- dark purple, grey to orange, porphrytic	Slight weathering
	162 —	1	9	Hilling .		Rhyolitic Tuff- dark grey to blue, porphrytic	

Other Information: Log updated 03/04/2023 following ADW Survey Data						
Logged By: MA	Checked By: SK	Version: 3430_TH_GA_LOG_PH8_V3				

Name of Hole: PH 8

Project Number: 3430 Client: Tricon Mining Equipment Project: Hillview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 13/12/2016 Date Completed: 13/12/2016 Surface RL: 181.15m AHD Coordinates: E:398463.58 N: 6404673.63 Drilling Contractor: D and E Drilling



2 of 3

Drill Type	Elevation (RL)		Piezometer Design	Photo Log	Graphic Log	Description	Additional Information
	161 — 160 — 159 — 158 —	2 2	1 2 3			Rhyolitic Tuff- dark blue, grey to orange, porphrytic	Some weathering
	157 — 156 — 155 —	2			· · · · · · · · · · · · · · · · · · ·	eathered Rhyolitic Tuff- brown to orange, porphrytic	
	154 — 153 —		7 8 9		· · · · · · · · · · · · · · · · · · ·	Rhyolitic Tuff- dark blue, grey to orange, porphrytic	Some weathering
Hammer	152 — 151 —	3(9		· · · · · · · · · · · · · · · · · · ·		
	150 — 149 —	3:			<	Rhyolitic Tuff- dark grey to blue, porphrytic	Some weathering, calcite veining
	148 — 147 —	3	4		· · · · · · · · · · · · · · · · · · ·		
	146 — 145 —				< · · · · · · · · · · · · · · · · · · ·		
	144 — 143 —	3		7	د. ۲ ۷. ۰۰		
	142 — 141	3		- William	· · · · · · · · · · · · · · · · · · ·		

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA Checked By: SK Version: 3430_TH_GA_LOG_PH8_V3

Name of Hole: PH 8

Project Number: 3430 Client: Tricon Mining Equipment Project: Hillview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 13/12/2016 Date Completed: 13/12/2016 Surface RL: 181.15m AHD Coordinates: E:398463.58 N: 6404673.63 Drilling Contractor: D and E Drilling



3 of 3

Drill Type	Elevation (RL)	Depth (m)	Piezometer Design	Photo Log	Graphic Log	Description	Additional Information
Hammer	141 140 139 138 137 136	4 4 4				Rhyolitic Tuff- dark blue, grey to orange, Porphrytic	Some calcite veining
	135 —		io 17			Hole Terminated at Taget Depth	
	134 —		18				

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA

Name of Hole: PH 9

Project Number: 3430 Client: Tricon Mining Equipment Project: Hillview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 15/12/2016 Date Completed: 15/12/2016 Surface RL:158.04m AHD Coordinates: E: 398794.30 N: 6404421.15 Drilling Contractor: D and E Drilling



1 of 2

Drill Type	Elevation (RL)	Depth Pie (m) De	zometer Photo sign Log	Graphic Log	Description	Additional Information
Auger	158 — 157 — 156 — 155 —	0			Weathered Hematized Rhyolitic Tuff- pink to orange, porphrytic	
	154 —	4			Fines- orange to dark orange, silty, some coarse quartz fragments	
	153 — 152 —	- 6			Weathered Hematized Rhyolitic Tuff- orange to brown, porphrytic, some fines	
	151 — 150 —	- 7		· D · D	Fines- orange to dark orange, silty, some coarse quartz fragments and black aphanitic textured fragments	
mer	149 — 148 —	9			Weathered Hematized Rhyolitic Tuff pink to orange, porphrytic	
Hammer	147 —	11		D A D	Fines- orange to dark orange, silty, some coarse quartz fragments	
	146 — 145 —	12 13			Weathered- pink to orange, porphrytic	
	144 — 143 —	14 15		F	Fines- orange to dark orange, silty, aphanitic textured fragments	
	142 —	16			Weathered Hematized Rhyolitic Tuff- orange to brown, porphrytic	
	141 — 140 —	17 18			Weathered Hematized Rhyolitic Tuff- orange to brown, porphrytic	
	139 —	- 19				
	138	20 =		$\cdot \Delta \cdot \cdot \Delta$		

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA Checked By: SK Version: 3430_TH_GA_LOG_PH9_V3

Name of Hole: PH 9

Project Number: 3430 Client: Tricon Mining Equipment Project: Hillview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 15/12/2016 Date Completed: 15/12/2016 Surface RL:158.04m AHD Coordinates: E: 398794.30 N: 6404421.15 Drilling Contractor: D and E Drilling



2 of 2

Drill Type	Elevation (RL)	Depth Piezometer (m) Design	Photo G Log	raphic Log	Description		Additional Information
	137 — 136 — 135 — 134 — 133 — 132 — 131 —			4. D. A. D. D. D. D. A. D.	Hematized Rhyolitic Tuff- purple, porph		Some weathering
Hammer	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Weathered Hematized Rhyolitic Tuff- orange, brown to dark blue, porphrytic to aphanitic	ie, porphrytic to	Calcite precipitation Dyke Calcite precipitation,			
			Dyke-dark blue to black, aphanitic	ck, aphanitic	Pyrite crystallisation		
	127 — 126 — 125 — 124 —			0 D D D D 0 D	Hematized Rhyolitic Tuff- pink, Porphr		Calcite precipitation
	123 —	35°			Dyke-dark blue to bla	ck, aphanitic	Calcite precipitation, Pyrite crystallisation
	122 —				Hole Terminated at T	arget Depth	
	121 —	37 38					
her In	formation	Log updated 03/04/20	23 following ADW	Survey D	Data		
ogged	By: MA		Checked By: SK		Ver	sion: 3430_TH_GA_LO	G_PH9_V3

Name of Hole: PH 10

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 14/12/2016 Date Completed: 14/12/2016 Surface RL: 157.98m AHD Coordinates: E: 398794.41 N:6404422.35 Drilling Contractor: D andE Drilling



1 of 4

Drill Type	Elevation Depth (RL) (m)	Piezometer Design	Photo Log	Graphic Log	Description	Additional Information
	0 157 — 1 156 — 2				athered Hematized Rhyolitic Tuff- wn, red to light orange, porphrytic	
	$\begin{array}{cccccccccccccccccccccccccccccccccccc$			A Wea	athered Hematized Rhyolitic Tuff- wn, red to light orange, porphrytic	
	153 <u>5</u> 152 <u>6</u>				athered Hematized Rhyolitic Tuff/ Dyke- red, orange to dark grey, porphrytic to aphanitic	
	151 — 7	,		Wea E	athered Hematized Rhyolitic Tuff/ Dyke- red, orange to dark grey, porphrytic to aphanitic	Calcite precipitation
Hammer	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	0 1 2 3		D A D A D A Weather brow	athered Hematized Rhyolitic Tuff- wn, red to light orange, porphrytic	Hardness increasing @12m
	142 — 1 141 — 1 140 — 1	5 6 7 8 9		A A A Wea A A A brow	athered Hematized Rhyolitic Tuff- wn, red to light orange, porphrytic	
			D :			

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA Checked By: SK Version: 3430_TH_GA_LOG_PH10_V3

Name of Hole: PH 10

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 14/12/2016 Date Completed: 14/12/2016 Surface RL: 157.98m AHD Coordinates: E: 398794.41 N:6404422.35 Drilling Contractor: D andE Drilling



2 of 4

Drill Type	Elevation Depth (RL) (m)	n Piezometer Design	Photo Log	Graphic Log	Description	Additional Information
	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	21 22 22 23 23 24 25 26 26 27		He He	ematized Rhyolitic Tuff- orange to dark red, porphrytic	Some weathering
	130 — 2 129 — 2				eathered Hematized Rhyolitic Tuff/ Dyke-orange, red to grey, blue, porphrytic to aphanitic	Calcite precipitation
mer					Dyke- dark grey to blue, aphanitic	Pyrite crystalation, calcite precipitation
Hammer	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$			D A D A D A Her	matized Rhyolitic Tuff- dark red to light purple, porphrytic	Some weathering
	122 — 3 121 — 3	35 36 37 38			eathered Hematized Rhyolitic Tuff/ /ke-dark red, purple to dark grey, porphrytic to aphanitic	Calcite precipitation
	119 — 3			D. D. D. D. D		

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA Checked By: SK Version: 3430_TH_GA_LOG_PH10_V3

Name of Hole: PH 10

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 14/12/2016 Date Completed: 14/12/2016 Surface RL: 157.98m AHD Coordinates: E: 398794.41 N:6404422.35 Drilling Contractor: D andE Drilling



3 of 4

Drill Type	Elevation (RL)		Piezomet Design	er Photo Log	Graphi Log		Additional Information
	117 — 116 —	— 4 ² — 42				Hematized Rhyolitic Tuff- dark purple to dark grey, porphrytic	Calcite precipitation, some weathering
	115 — 114 —	— 43 — 44	3 4 4				
	113 — 112 — 111 —	— 48 — 46				Rhyolitic Tuff- light grey to blue, porphrytic	Calcite precipitation
	110 —	— 47 — 48	в		· · · · · · · · · · · · · · · · · · ·		
Hammer	109 — 108 —	— 49 — 50		14.1204			
Ï	107 —	— 5 ⁻	EE			Dyke- dark grey to blue, aphanitic	Pyrite crystalisation, magnetic, calcite precipitation
	106 —	- 52			7	Rhyolitic Tuff- light grey, porphrytic	
	105 — 104 —	— 53 — 54	4	TE B		Dyke- dark grey to blue, aphanitic	Dyke, pyrite crystalisation, magnetic, calcite precipitation
	103 —	58			······································	Hematized Rhyolitic Tuff/ Dyke- light grey to dark grey, porphrytic to aphanitic	Magnetic, calcite precipitation
	102 —	— 56			۲۰۰۰ ۲۰ ۲۰۰۰ ۲۰۰۰ ۲۰	Rhyolitic Tuff- light grey, porphrytic	Water intersected
	101 — 100 —	— 57 — 58				Rhyolitic Tuff/ Dyke- light grey to dark grey, porphrytic to aphanitic	Magnetic, calcite precipitation
	99 —	59	9			Dyke- dark grey to blue, aphanitic	Dyke, pyrite crystallisation, magnetic, calcite
	98 —	60		4/2023 following	<		precipitation, some

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA

Version: 3430_TH_GA_LOG_PH10_V3

Name of Hole: PH 10

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 14/12/2016 Date Completed: 14/12/2016 Surface RL: 157.98m AHD Coordinates: E: 398794.41 N:6404422.35 Drilling Contractor: D andE Drilling



4 of 4

Drill Type	Elevatior (RL)	ם Depth (m)	Piezometer Design	Photo Log	Graphic Log	Description		Additional Information
				a missellist				weathering
	97 — 96 —	6 ²	2			Rhyolitic Tuff/ Basalt- lig dark grey, porphrytic to		Magnetic, calcite precipitation
	95 —	- 6	3	A Parts	7			
	94 —	64			د ₇			
	93 —	- 6			<			
	92 —	- 6	6		2 7			
Hammer	91 —	- 6		AND A	· · › · · · · · · · · · · · · · · · · ·			
Haı	90 —	- 68	8	AND A	7	Rhyolitic Tuff- dark purp porphrytic	ple to grey,	
	89 —	- 69			· · · · · · · · · · · · · · · · · · ·			
	88 —	- 70	∘┆ <u>╡</u> ┥┣		<			
	87 —	- 7 [.]			×			
	86 —	- 72			× ۲ ۲			
	85 —	- 7:	3					
	84 —	- 74				Rhyolitic Tuff/ Basalt- lig dark grey, Porphrytic to		Magnetic, calcite precipitation
	83 —	- 7				Hole Terminated at Ta	rget Depth	
	82 —	⊥ 7(6 ———					
ther Inf	formation	: Log up	odated 03/04/20	23 following	g ADW Survey	Data		
ogged	By: MA			Checked	By: SK		Version: 3430_TH_G	A_LOG_PH10_V3

Name of Hole: PH 11

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 19/12/2016 (Collected) Date Completed: 19/12/2016 Surface RL: 113.57m AHD Coordinates: E:399018.56 N:6404504.41 Drilling Contractor: D and E Drilling



1 of 2

	$\begin{array}{cccccccccccccccccccccccccccccccccccc$				
Hammer	- 6 107 - 7 106 - 8 105 - 9 104 - 1 103 - 1		Hematized F bro	Rhyolitic Tuff- orange to own, porphrytic	
	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	2 3 4 5 6	Dyke-orang	ered Rhyolitic Tuff/ le, brown to dark grey, nrytic to aphanitic	Calcite precipitation
	97 — 1 96 — 1 95 — 1				

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA Checked By: SK Version: 3430_TH_GA_LOG_PH11_V3

Name of Hole: PH 11

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 19/12/2016 (Collected) Date Completed: 19/12/2016 Surface RL: 113.57m AHD Coordinates: E:399018.56 N:6404504.41 Drilling Contractor: D and E Drilling



2 of 2

Drill Type	Elevation (RL)		Piezometer Design	Photo Log	Graphic Log	Description	Additional Information
	93 —	— 2 [.]				brown, porphrytic	
	92 —	— 22					
	91 —	— 23					
	90 —	— 24	<u>ک</u> 4			Hole Terminated	
	89 —	— 2	5				
	88 —	— 20	6				

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA

Version: 3430_TH_GA_LOG_PH11_V3

Name of Hole: PH 12

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 19/12/2016 (collected) Date Completed: 19/12/2016 Surface RL: 88.15m AHD Coordinates: E: 399114.48 N: 6404843.48 Drilling Contractor: D and E Drilling



1 of 1

Drill Type	Elevation Depth (RL) (m)		Photo Log	Graphic Log	Description	Additional Information
	88 0		CINER CINE		Weathered Rhyolitic Tuff - brown to purple	
	87 — 1 86 — 2 85 — 3				Weathered Hematized Rhyolitic Tuff- brown, dark purple to orange, porphrytic	
	$ \begin{array}{cccccccccccccccccccccccccccccccccccc$			Hematized Rhyolitic Tuff-dark purple to orange, porphrytic	Some weathering	
				Fi fragm	Fines- brown, some isolated fragments of weathered Hematized Rhyolitic Tuff	Possible fault or shear zone?
Hammer	$\begin{array}{cccccccccccccccccccccccccccccccccccc$				Hematized Rhyolitic Tuff-dark purple to orange, porphrytic	Some weathering, some calcite veining

Other Information: Log updated 03/04/2023 following ADW Survey Data

Logged By: MA Checked By: SK Version: 3430_TH_GA_LOG_PH12_V3

Name of Hole: PH 12

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 19/12/2016 (collected) Date Completed: 19/12/2016 Surface RL: 88.15m AHD Coordinates: E: 399114.48 N: 6404843.48 Drilling Contractor: D and E Drilling



1 of 1

Drill Type	Elevation (RL)	Depth (m)	Piezometer Design	Photo Log	Graphic Log	Description		<i>l</i> In	Additional formation
		20)						
Other In	formation:	Log up	dated 03/04/202	3 following A	ADW Survey Data				
Logged	By: MA			Checked By	r: SK				
Logged	⊔y. I¥IA			CHECKEU D	,		Version: 3430_T	H_GA_LOG_PH	12_V3

Name of Hole: PH 13

Project Number: 3430 Client: Tricon Mining Equipment Project: Hilview Geological Investigation Location: Off Maytoms Lane, Booral Logged by: MA/SK

Hillview

Date Commenced: 19/12/2016 (Collected) Date Completed: 19/12/2016 Surface RL: 100.24m AHD Coordinates: E: 398919.44 N: 6405189.27 Drilling Contractor: D and E Drilling



1 of 1

Drill Type		epth (m)	Piezometer Design	Photo Log	Graphic Log	Description	Additional Information
	100 — 99 — 98 —	:	0 1 2			Weathered Hematized Rhyolitic Tuff- pink, orange to brown, porphrytic	Clay, possible joints?
Hammer	97 — 96 — 95 — 94 — 93 — 92 — 91 —		3 4 5 6 7 8 9			Weathered Hematized Rhyolitic Tuff- pink, orange to brown, porphrytic	Some Clay, possible joints?
На	90 — 89 — 88 —		11 12 13			Hematized Rhyolitic Tuff/ Dyke- dark red, purple to grey, dark grey, porphrytic to aphanitic	Calcite precipitation
	87 — 86 — 85 —					Hematized Rhyolitic Tuff- dark red, purple to orange, porphrytic	Some Weathering
	84 —					Hematized Rhyolitic Tuff/ zdyke- dark red, purple to grey, dark	Calcite precipitation
	83 —			STOCK -	D . A . D	grey, porphrytic to aphanitic Hematized Rhyolitic Tuff- dark red, purple to orange, porphrytic	Some Weathering
	82 —			Share and		Dyke- dark grey, aphanitic, some calcite veining	Calcite precipitation, some pyrite mineralisation

 Other Information: Log updated 03/04/2023 following ADW Survey Data

 Logged By: MA
 Checked By: SK

 Version: 3430_TH_GA_LOG_PH13_V3



Appendix D: Qualtest Resource Material Testing

Resource Material Testing Assessment

Hillview Quarry, Karuah

NEW15P-0045-AC 13 April 2017





13 April 2017

Tricon Mining Equipment Pty Ltd RMB1085 Pacific Highway Somersby NSW 2250

Attention: Greg Thompson

Dear Greg,

RE: PROPOSED RESOURCE DEVELOPMENT – HILLVIEW QUARRY, KARUAH MATERIAL TESTING ASSESSMENT

Please find enclosed our Material Testing Assessment report for the proposed Quarry Resource Development located at Karuah and referred to as Hillview Quarry.

The report includes test results, descriptions and discussion of the engineering properties of the range of rock types encountered during investigation drilling performed within the resource. A broad range of tests were performed to demonstrate the engineering properties of the resource, primarily for use as construction aggregate in the concrete, asphalt, sealing and road building industry.

If you have any questions regarding this report, please do not hesitate to contact Alan Cullen or the undersigned.

For and on behalf of Qualtest Laboratory (NSW) Pty Ltd

2rc

Jason Lee Principal Geotechnical Engineer

Alan Cullen Principal Geotechnician

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Appendix A:	Results of Laboratory Testing – Qualtest (2017)
Appendix B:	Results of Laboratory Testing – External (2017)

1.0 Introduction

Qualtest Laboratory NSW Pty Ltd (Qualtest) is pleased to present this report on behalf of Tricon Mining Equipment Pty Ltd (Tricon) for the proposed resource development located at Karuah and referred to as Hillview Quarry.

This report presents the results of laboratory testing conducted by Qualtest and other specialist laboratories on samples obtained from the recent drilling investigations performed by VGT Pty Ltd - Environmental Compliance Solutions (VGT), together with discussion and recommendations on the engineering properties of the resource, primarily for use as construction aggregate in the concrete, asphalt, sealing and road building industry.

2.0 Scope of Testing

The following scope of testing was undertaken on samples retrieved from cored boreholes. The scope performed was aimed at providing a broad suite of testing to cover as many potential markets within the construction materials industry. Tests were chosen to give an overall performance comparison with current Australian Standards (AS2758) and NSW Roads and Maritime Services (RMS) standards. The tests covered physical characteristics, durability, geological properties, and chemical performance of the recovered samples. The scope of testing performed on samples of potential quarry materials provided included:

- AS1141.4 Bulk Density
- AS1141.5 Fine Particle Density & Absorption
- AS1141.6 Coarse Particle Density & Absorption
- AS1141.6.1 Coarse & Fine Weighted Bulk Density & Absorption
- AS1141.11 Particle Size Distribution
- AS1141.12 Material finer than 75 micron
- AS1141.14 Particle Shape
- A\$1141.22 Wet/Dry Strength Variation
- AS1141.23
 Los Angeles Abrasion
- AS1141.24
 Sodium Sulphate Soundness
- AS1141.35
 Sugar in Aggregates
- AS1141.41/42
 Polished Aggregate Friction Test
- A\$1012.20
 Chlorides and Sulphates
- AS4133.4.1 Point Load Strength
- AS4133.4.2.1 Uniaxial Compressive Strength of Rock
- A\$1289.3.1.1,3.2.1,3.3.1,3.4.1 Atterberg Limits including Plasticity Index
- RMS T219
 Acid soluble Sulphates
- RMS T363
 Alkali Reactivity
- RMST659
 Methylene Blue Value
- ASTM C 295
 Petrographic analysis

The purpose of the study was to assess the available rock types, and potential uses of each material within the construction industry. The scope of tests listed above was carried out on a selection of composite rock core samples considered to be representative of the potential resource, sampled from between 0.0 and 100.0m from recovered core from Borehole DDH 2 across the varying depths.

The resource identified two representative rock types based upon the Petrographic Analysis results as summarised in Table 1. A composite of representative samples were selected and tested based on their potential material qualities and potential suitability for use as construction industry material.

Sample ID	Depth (m)	Rock Type
DDH2 P-1	Composite Sample	Hematized Rhyolitic Tuff
DDH2 P-2	Composite Sample	Rhyolitic Tuff

Table 1 – Description of Representative Rock Types and Samples Tested

3.0 Results

3.1 Bulk Density (AS1141.4)

Bulk Density is defined as the mass of particles divided by the total volume they occupy. The total volume includes particle volume, inter-particle void volume and internal pore volume.

Sample ID	Rock Type	Uncompacted Bulk Density (t/m ³)	Compacted Bulk Density (t/m ³)	AS2758 Concrete Aggregate Spec.
DDH2 (0.0 to 100.0m)	Rhyolitic Tuff	1.33	1.50	> 1.2 t/m³

Table 2 – Bulk Density Results (AS1141.4)

3.2 Particle Density and Water Absorption (AS1141.5, 6 & 6.1)

The Particle Density and Water Absorption test is universally accepted within the Australian Construction Industry as the definitive measure of fine & coarse aggregate density and water absorption. It is used to determine these properties for both coarse grained aggregate and natural and manufactured sands.

The Particle Density test produces results similar to Specific Gravity (Apparent Particle Density), but also takes into account the voids that may be present in the material being tested. At the same time, the amount of water that is held within those voids is calculated and reported as the Water Absorption of the material. The definition of the four reportable parameters which are calculated is set out below: -

<u>Apparent Density:</u> The dry mass of particles divided by their volume, with the volume including only the impermeable voids.

<u>Particle Density – Dry:</u> The dry mass of particles divided by their volume, with the volume including both permeable and impermeable voids.

<u>Particle Density - Saturated Surface Dry (SSD):</u> The SSD mass of particles divided by their volume, with the volume including both permeable and impermeable voids.

<u>Water Absorption</u>: The ratio expressed as a percentage, of the mass of water held in the permeable voids of the particles brought to SSD condition following soaking under water for 24 hours, to the oven dried mass of the material.

The test properties listed above provide key design parameters for concrete and asphalt mixes.

For Concrete mixes, the SSD density which accounts for water contained within permeable voids allows for calculation of the mix yield and concrete voids in the design calculation process. Attempts have been made in some specifications to limit the water absorption of aggregates, to minimise the practical difficulties that arise when dealing with highly absorptive aggregates. Because the density determination has accounted for the water in voids, they do not impede the cement hydration process, thus a more accurate determination of water demand and water-cement ratio design is possible. In other words, whilst low water absorptions are preferable, higher absorptions do not present insurmountable hurdles to concrete and asphalt mix designs.

Sample ID	Rock Type	Particle Density Dry Basis (t/m ³)	Particle Density SSD Basis (t/m ³)	Apparent Particle Density (t/m ³)	Water Absorption (%)	AS2758 Concrete Aggregate Spec. Dry Basis
DDH2 - composite (0.0m to 100.0m)	Rhyolitic Tuff	2.47	2.52	2.60	2.1	> 2.1 t/m³

Table 3 – Particle Density Fine Results (AS1141.5 - <4.75mm)

The AS 1141.5 Particle Density test can be used to determine particle density and water absorption properties in natural and manufactured fine materials.

For fine aggregates, the particle density test is carried out on material of size less than 4.75mm and a test portion of about 500g. This portion is immersed in water at room temperature for at least 24 hours and agitated in a manner that removes all the entrapped air. This is to ensure the pores are filled with water. The sample is then dried back to a point where the SSD condition can be determined.

This point is normally determined by using a cone apparatus and tamping rod.

When the material is Surface Saturated Dry (SSD), it should collapse on removal of the supporting cone. If it fails to collapse, it is deemed to still be too wet and further drying is required. This procedure of determining SSD works well for rounded natural sands with low quantities of passing 75 micron fines.

The results indicate that the fines produced are below the preferable limit of 3% in RMS 3152 for Asphalt.

The composite rhyolitic tuff sample tested returned a low water absorption value of 2.1%. The Concrete and Asphalt industry generally prefers materials to have water absorptions less than 2.5%.

Sample ID	Rock Type	Particle Density Dry Basis (t/m ³)	Particle Density SSD Basis (t/m ³)	Apparent Particle Density (t/m ³)	Water Absorption (%)	AS2758 Concrete Aggregate Spec. Dry Basis
DDH2 - composite (0.0m to 100.0m)	Rhyolitic Tuff	2.49	2.53	2.58	1.3	> 2.1 t/m³

Table 4 - Particle Density Coarse Results (AS1141.6 - >4.75 mm to <19.0mm)

Whilst the fines portion of Rhyolitic Tuff from DDH2 had an absorption of 2.1%, the coarse fraction produced a lower water absorption of 1.3%. Both results are within acceptable and normal working limits.

When assessing aggregates for mix design purposes that have both fine and coarse portions, it is normal to refer to their weighted ratio density and absorption to get an overall picture of the Particle Density and Water Absorption characteristics as outlined in Table 5.

Sample ID	Rock Type	Particle Density Dry Basis (t/m³)	Particle Density SSD Basis (t/m ³)	Apparent Particle Density (t/m ³)	Water Absorption (%)	AS2758 Concrete Aggregate Spec. Dry Basis
DDH2 - composite (0.0m to 100.0m)	Rhyolitic Tuff	2.48	2.52	2.59	1.7	> 2.1 t/m³

 Table 5 – Particle Density Results (AS1141.6 - Weighted)

The particle density and water absorption results obtained from the composite sample of Rhyolitic Tuff from Borehole DDH2 (in either it's fine, coarse or combined weighted state), all meet and are below the normal absorption maximum requirement of 2.5% sought by the concrete and asphalt Industry. The dry density and apparent particle density of material from this source also exceed the minimum density requirements of 2.1 t/m3. This would indicate that the resource density and absorption characteristics are reasonably uniform, and blending or mixing of the resource products during production process (either by design or inadvertently), should not create undue problems.

3.3 Particle Size Distribution (AS1141.11.1)

The Particle Size Distribution or grading of the aggregates tested is primarily a function of the crushing process. The rock core samples provided for testing were crushed in a laboratory "Jaw Crusher" to produce a targeted 20mm minus run of crush product. This crushed product was then graded to produce the particle size distribution results presented in Table 6.

Once graded, certain portions of the graded aggregate and fines were extracted from the total sample to perform the respective testing nominated in the testing schedule.

In the particle size distribution test, the aggregate is dried to a constant mass and then separated through a series of sieves, made with punched plate and woven wire in progressively smaller openings. For samples tested to Australian Standards, the sieves conform to AS1152 and form part of the metric "half Series", where each successive sieve opening is half the size of the next largest sieve in the series.

Once separated, the mass of particles retained on each sieve is measured and compared with the mass of the total sample. Particle size distribution is then expressed as the cumulative mass percentage passing each sieve. Results are presented in a tabulated form or as graphs in a logarithmic format.

The test method for grading used in this assessment is described in AS1141.11.1, and was performed in the material's dried, unwashed state. The percent passing the 75 micron fraction was performed to AS1141.12 method and is a washed method where the dried sample is soaked for 12 hours and then washed over a 75 micron wash sieve.

Particle size distribution, or grading, is one of the most influential and commonly reported characteristics of an aggregate. Grading influences concrete durability, road base compatibility, porosity, workability, cement and water requirements, strength and shrinkage. However, it is the total aggregate grading in the mix that is critical to the mix performance. The grading of an individual component is not critical to the mix performance and an unsuitable grading can be improved by blending with other components. In this case the individual grading is not critical but once the blend is established, the consistency of individual components is critical to the production of a consistent product.

The individual grading percentages presented in Table 6, whilst not comparable to any individual specification, are indicative of a material that when crushed with normal crushing and screening equipment will produce similar material with the same physical characteristics and shape. The shape of individual particles can be changed and improved depending upon the screening and crushing equipment utilised in the production process. Improvement in the shape to a more cubical dimension will ultimately improve some durability aspects associated with the rock crushed from this resource, such as wet /dry variation.

Sample ID	Rock Type	Sieve Size (mm)	%Passing
		19.0	100
		13.2	92
		9.5	73
	Rhyolitic Tuff	6.7	58
		4.75	43
DDH2 - composite		2.36	26
(0.0m to 100.0m)		1.18	17
		0.60	11
		0.425	8
		0.300	6
		0.150	2
		0.075	0

Table 6 – Particle Size Distribution Results (AS1141.11)

3.4 Passing 75 Micron in Aggregates – By Washing (AS1141.12)

The percentage passing 75 microns is calculated as the loss on washing expressed as a percentage of the original sample mass.

The 75-micron fraction size is used in Australia as a near approximation to the 60-micron size limit that in geological terms marks the boundary between fine Sand and Silt/Clay.

In natural aggregates, the total passing 75 microns will include the silt and clay fractions and will be composed of silt and clay minerals. In many specifications including AS 2758.1, the percentage passing 75 microns has been specified as a control over clay and silt fines that may cause water and cement demand, shrinkage, cracking, and control the permeability of some materials.

Percent passing results provided in Table 6 give indicative values that may be expected from material crushed in the normal manner.

The percent passing the 75 micron is an indication of the silt and clay fines within a particular crushed product. The percentage returned from crushed rock composite sample DDH2 was 2%, this is expected to consist of predominately silt size fraction as the fines produced were non-plastic when Atterberg limit testing was performed on the recovered fines.

Sample ID	Rock Type	Material Finer Than 75µ (%)	AS2758 Concrete Aggregate Spec	RMS 3151 Sealing Aggregate Spec
DDH2 - composite (0.0m to 100.0m)	Rhyolitic Tuff	2.0	0 – 2	0 - 1

Table 7 – Material Finer Than 75µ Results (AS1141.12)

The results indicate that after the primary run of crush process in the laboratory, material passing the 75 micron sieve was equal to the specification limits. In the quarry production environment, this can be improved by either by appropriate screening, dust extraction systems or by washing of the end product. Through implementation of such measures, it is anticipated that specification limits outlined in Table 7 may be met. Manufactured sand for concrete production is generally washed to control the minus 75 micron percentage.

3.5 Particle Shape by Proportional Calliper (AS1141.14)

The shape of the aggregate and crushed fines is primarily influenced by the crushing and screening process. The material processed and tested has been crushed with a primary "jaw crusher" which produces more flaky and elongated shapes than a cone crusher or "Barmac" system.

Aggregate is generally shaped through a series of primary and secondary crushers and recirculated to obtain more cubical, better shaped aggregate for concrete, sealing aggregate and asphalt production.

The shape of individual particles influences not only the durability test results performance, but also the physical flow ability of concrete and the ultimate compatibility and stability of the end product. Improving the shape characteristics of a material to make it more cubical can have a positive influence on these physical and durability test parameters.

The particle shape test determines the percent of Misshapen or poorly shaped less desirable particles within a sample at 2:1 and 3:1 size ratios. The test determines the shape of individual particles and grades them according to their length, breadth and thickness.

The samples tested demonstrated total misshapen particles percentages of 7% to 3%. These well shaped results are not typical of a single run of crush product through a primary jaw crusher. The percentage misshapen should decrease when recirculated over a secondary crushing process such as a "cone crusher", which will generally halve these current percentage misshapen values.

It is assessed that aggregate crushed in the above manner (single run of crush, through a jaw crusher) would be suitable for road base pavement products and certain asphalt applications, but would generally require further processing to be suitable for high strength concrete and superior performance aggregate.

Sample ID	Rock Type	Total Misshapen Particles 2:1 (%)	Total Misshapen Particles 3:1 (%)	AS2758 Concrete Aggregate Spec	RMS 3151 Sealing Aggregate Spec	RMS 3152 Asphalt Aggregate Spec
DDH2 - composite (0.0m to 100.0m)	Rhyolitic Tuff	7	3	<35%	<35%	<35%

Table 8 – Particle Shape Results (AS1141.14)

3.6 Wet/Dry Strength Variation (AS1141.22)

The Wet/Dry Strength Variation test determines the load (in KN) required to produce 10% fines in an aggregate sample when tested in its dry (oven), and wet (SSD) conditions. The difference between these two strengths is expressed as a % variation. Wet strength is typically lower than the dry strength due to the presence of moisture within the aggregate particles during crushing, while a large difference between wet and dry conditions indicates potential water sensitivity of the rock when placed in saturated site conditions.

Sample ID	Rock Type	Dry Strength (kN)	Wet Strength (kN)	Wet/Dry Strength Variation (%)	AS2758 Concrete Aggregate Spec
DDH2 - composite (0.0m to 100.0m)	Rhyolitic Tuff	246	215	13	<25%

Table 9 – Wet/Dry Strength Variation Results (AS1141.22)

The wet/dry variation test is an excellent method of assessing the durability and wet strength of a product. The wet/dry test performed on a composite sample of the cored rock from DDH2 indicate that despite the crushed rocks shape characteristics, the Rhyolitic Tuff material performed well, further demonstrating the rock deposits high strength and durability characteristics.

It is assessed that in general the Rhyolitic Tuff would be suitable for concrete and asphalt production as it returned results well less than 25%.

DDH2 composite returned a low wet/dry variation result of 13%, however this result is expected to improve further with better shaping which would occurs during actual production.

Based on these wet/dry results the Rhyolitic Tuff rock types are assessed to be hard and durable and suitable for concrete, asphalt and road base applications. Blending of the different rock types in the correct ratios would also improve the overall performance of the end products. Tested DDH2 composite material also complied with the wet strength criteria of greater than 100kN.

3.7 Los Angles Abrasion Value (AS1141.23)

This test involves placing a portion of aggregate in a steel drum, fitted with an internal baffle, with a specified number of steel balls. The drum is rotated a fixed number of times to distress the aggregate by impact loading, with some contribution from self-abrasion.

The test was originally developed in the context of wear resistance in cold areas (where the surfacing is trafficked by studded tyres) and for relatively low strength rocks such as limestone. The test result is determined by sizing the post-test material at a specified sieve size (1.8 mm) with the ratio of the -1.8mm fraction to the original charge mass giving the LA value. The higher the LA value, the less durable the rock. It is generally accepted that results of lower than 25% indicate that the rock is suitable for most construction industry applications.

The results of the testing on the composite sampled material from DDH2 demonstrated that the Rhyolitic Tuff performed well under the Los Angles abrasion test, with values of 17%. There is a general correlation of the Los Angeles Value test results with wet/dry strength variation testing.

Sample ID	Rock Type	Los Angeles Value	AS2758 Concrete Aggregate Spec
DDH2 - composite (0.0m to 100.0m)	Rhyolitic Tuff	17	<25%

Table 10 – Los Angeles Abrasion Value Results (AS1141.23)

3.8 Sodium Sulphate Soundness (AS1141.24)

This test is commonly used to assess an aggregate's suitability for use in concrete. Whilst the test is designed for aggregates in concrete, the exposure classifications can be used as a guide when assessing the suitability of source rock for similar applications.

The test is designed to assess the resistance of rock fabric to salt crystallization pressure, which is a measure of the susceptibility of the rock to physical breakdown. The extent of this breakdown is assessed by repeated immersion of the aggregate in a saturated solution of Sodium Sulphate. This is followed by oven drying to dehydrate the salt precipitated in permeable pores and rehydration of the salt during subsequent immersion, which generates crystallization pressure. Internal expansive forces derived from rehydration of the salt on re-immersion, mimic freezing (crystallization of water) or salt action. This provides a measure of the integrity of the rock fabric when subjected to physical weathering associated with the expansion caused by the freezing of water in aggregate pores, and the expansion of salts on rehydration in marine conditions following repeated wetting and drying.

Sample ID	Individual Fractions Tested (mm)	Individual Sample Size Loss (%)	
	Loss 19.0 to 13.2mm	0.1	
	Loss 13.2 to 9.5mm	0.3	
	Loss 9.5 to 4.75 mm	0.1	
DDH2 - composite (0.0 to 100.0m) Rhyolitic Tuff	Loss 4.75 to 2.36 mm	0.3	
	Loss 2.36 to 1.18 mm	0.4	
	Loss 1.18 to 0.600mm	0.5	
	Total Weighted Loss	0.3	

Table 11 - Sodium Sulphate Soundness Results (AS1141.24)

The results of Sodium Sulphate Soundness testing are highly dependent on particle size, a consequence of variation to the surface area/volume ratio and its effect on the degree of saturation achieved during the wetting cycle.

High loss results are a fairly reliable indication of poor durability consequently low losses are an indication of good durability. The Rhyolitic Tuff from this resource has extremely low losses, which demonstrates the sources excellent durability characteristics making it suitable for all concrete and asphalt applications.

AS2758 specification breaks the requirements into 3 sub-classes based on the material's maximum weighted average loss of 6%, 9% and 12%. The weighted loss results for the composite sampled material within DDH2 of the resource are all below 6% and are therefore assessed to be suitable for any exposure conditions, including armour rock and dimension stone, providing suitable quarrying methods are used to minimise fractures.

3.9 Stripping and Initial Adhesion (RMS T230 & RMS T238)

Adhesion between bitumen and aggregate is one of the functional properties that guarantee durability of asphalt mixes and seals. Adhesion can be reduced and the cohesion within asphalt lost through the presence of water at the bitumen/stone interface. This is referred to as "stripping".

Adhesion is greatly influenced predominantly by mineral characteristics within parent rock as well as within the properties of the bitumen. Reduced resistance against stripping of certain aggregates can be improved by the addition of hydrated lime (or filler containing hydrated lime) to an asphalt mix. The use of additives in bitumen can improve the adhesion but in general work more selectively and depending on the aggregate/bitumen combination.

The resistance to stripping and initial adhesion test is an assessment of the extent of stripping and initial adhesion between cover aggregates and bituminous binder under wet and dry conditions. The assessment can be performed with or without treatment with pre-coating materials and addition of bitumen agent.

The following results have been performed using standard pre-coat and bitumen additives and show that the crushed aggregate from DDH2 performed well in both the stripping and adhesion tests performed using additive combinations stated.

Bitumen class 170, polyseal \$35E bitumen binder and \$AMI precoat with 1% redicote422/60 were used in the stripping and adhesion tests performed, with both binder combinations tested returning adhesions of less than 5%, and stripping values of less than 2%.

Method	Property	Result	Specification RMS 3258
RMS T238	Initial Adhesion using SAMI C170 binder with 7% kerosene, on NEW17W0307-S01 aggregates, precoated with SAMI standard precoat plus 1% redicote 422/60, soaked curing condition and unsoaked curing condition	<5 <5	Maximum 10% Stripping
RMS T230	Resistance to stripping using SAMI C170 binder on same precoated aggregates, plus 1% redicote422/60.0ven temperature for conditioning=68.5Cand tendency for aggregates to crumble when pulled is <2%	<2	Maximum 10% Stripping
RMS T238	Initial Adhesion using Polyseal S35E binder with 8% kerosene, on NEW17W0307-S01 aggregates ,precoated with SAMI standard precoat plus 1% redicote 422/60, soaked curing condition and unsoaked curing condition	<5 <5	Maximum 10% Stripping
RMS T230	Resistance to stripping using polyseal S35E binder on same precoated aggregates, plus 1% redicote422/60.0ven temperature for conditioning=68.5Cand tendency for aggregates to crumble when pulled is <2%	<2	Maximum 10% Stripping

Table 12 – Stripping and Initial Adhesion	n (RMS T230 & RMS T238)
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The samples where this testing was performed were relatively flaky and demonstrated more breakdown than would normally be expected from crushed rock of this type in the stripping test. It is considered that if better shaped aggregate was produced, the end test result would have been improved.

3.10 Cerchar Abrasion

The Cerchar Scratch test (1986) was developed in France in the mid-1980s to help predict the wear of cutters on the tunnel boring machines. The test is a useful low cost method of predicting the rate of wear of cutter heads, excavator bucket teeth, and crushing and screening plant. The test is performed on a small freshly broken rock sample, requiring less than 25mm in size. The sample is scratched by a hardened sharp heat treated alloy steel needle of defined geometry over a length of 10mm in 1 second, under a static load of 70N. This provides a wear classification referred to as the Cerchar Abrasivity Index (CAI). The CAI is calculated as the average measure of the worn-flat diameters (in tenths of mm) on the testing needle.



Figure 1 above shows the cerchar abrasivity machine and the 10mm long scratches on rock samples.

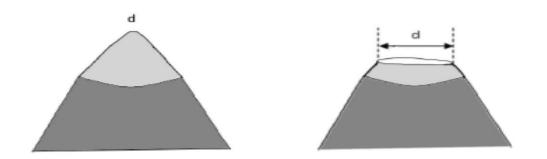


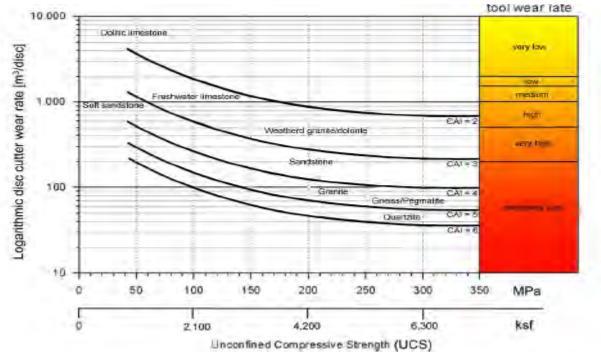
Figure 2 above shows how the CAI is obtained from a steel needle's "sharp point" (left); the test produces a conical blunt surface where its new diameter is measured in integers of 0.1mm, with a scale going from 1 to 6.

The cerchar abrasivity test performed on pieces of aggregate from DDH2 and tested in the manner described above returned a CAI rating of 2.5%. This abrasivity rating places the Rhyolitic Tuff rock from DDH2 in the 'Very High' tool wear bracket of around 500m3 of excavation per cutter disc.

Sample ID	Classification	Average CAI _s (HRC55)
DDH2 - composite sample	Rhyolitic Tuff High Abrasiveness	2.71

Table 13 - Cerchar	Abrasivity	Index
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The table shown below modified from Maidl (2001) shows the comparison of field unconfined compressive strength, tool wear rate and predicted excavated cubic metre wear rate for cutter discs.



3.11 Sugar in Aggregates (AS1141.35)

Sugar retards the set-in concrete; therefore, aggregates are tested to ensure there is no presence of sugar in materials crushed and proposed for use as concrete aggregate. No sugar was detected within any samples of crushed aggregate tested.

Table 14 – Sugar in Aggregates Results (AS1141	.35)
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Sample ID	Rock Type	Content
DDH2 - composite (0.0m to 100.0m)	Rhyolitic Tuff	Not Detected

3.12 Soluble Salts (AS1012.20)

Excessive quantities of some soluble salts may cause efflorescence on concrete, corrosion of reinforcing steel, or disintegration of the mass of concrete. Permissible levels of soluble salts are generally expressed as a proportion of the relevant ion present in the concrete by mass of concrete or by mass of Portland cement.

AS1141.20 determines the chloride ion and sulphate ion content within aggregates proposed for use as concrete aggregate. The total individual percentage of chloride and sulphate ion reportable in aggregate is for amounts in excess of 0.01%. The total combined aggregate chloride salt content should not exceed 0.04% for use in reinforced concrete. The total sulphate ion content allowable within a concrete mix and expressed as SO₃ should not exceed 5% by mass of Portland cement.

Table 15 - Chlorides Results (1012.20)

Sample ID	Rock Type	Chlorides (Cl) (%)	Sulphates (SO4) (%)
DDH2 - composite (0.0m to 100.0m)	Rhyolitic Tuff	0.004	0.001

Table 16 – Sulphates Results (RMS T219)

Sample ID	Rock Type	Sulphates (SO₃) (%)
DDH2 - composite (0.0m to 100.0m)	Rhyolitic Tuff	0.010

Samples from the deposit (depending upon the rock source) demonstrated different content levels of soluble salt ions, but despite slightly elevated sulphates (SO₄) the total sulphates expressed as (SO₃) are below AS2758.1 guideline requirements.

3.13 Alkali Reactivity (RMS T363)

In most concrete, aggregates are more or less chemically inert. However, some aggregates react with the alkali hydroxides in concrete, causing expansion and cracking over a period of many years. This alkali-aggregate reaction has two forms: Alkali-Silica Reaction (ASR) and Alkali-Carbonate Reaction (ACR).

Alkali-Silica Reaction (ASR) is of more concern as aggregates containing reactive silica materials are more common. In ASR, aggregates containing certain forms of silica will react with alkali hydroxide in concrete to form a gel that swells as it adsorbs water from the

surrounding cement paste or the environment. These gels can induce enough expansive pressure to damage concrete.

Alkali-silica reaction can be controlled using certain supplementary cementitious materials. In proper proportions, silica fume, fly ash, and ground granulated blast-furnace slag have significantly reduced or eliminated expansion due to alkali-silica reactivity. In addition, lithium compounds have been used to reduce ASR.

Although potentially reactive aggregates exist throughout Australia, alkali-silica reaction distress in concrete is not that common because of the measures taken to control it. It is also important to note that not all ASR gel reactions produce destructive swelling.

Alkali-Carbonate Reaction (ACR) is observed within certain dolomitic rocks. Dedolomitisation (the breaking down of dolomite) is normally associated with expansion. This reaction and subsequent crystallization of brucite may cause considerable expansion. The deterioration caused by alkali-carbonate reactions is similar to that caused by ASR, however, ACR is relatively rare because aggregates susceptible to this phenomenon are less common and are usually unsuitable for use in concrete for other reasons. Sampled materials are not considered dolomitic rocks so ACR is considered unlikely in regards to material from this source.

The samples obtained from the respective boreholes were crushed and combined with SL-Berrima GP cement and tested for alkali reactivity in accordance with RMS T363 procedures. The mortar bar expansion percentages set out in Table 17 range from a maximum of 0.098% at 10 days, to a maximum of 0.206% at 21 days.

Based on the assessment criteria, the samples are classified as slowly reactive as the sample has less than 0.10% expansion at 10 days (Slowly-Reactive) and have greater than 0.10% expansion (Slowly Reactive) after 21 days.

Sample ID	Rock Type	10 Day Expansion (%)	21 Day Expansion (%)	Classification
DDH2 - composite (0.0 to 100.0m)	Rhyolitic Tuff	0.098	0.206	Slowly Reactive*

Table 17- Alkali Reactivity Results (RMS T363)

Where samples are reactive further testing with fly ash based cements are recommended.

3.14 Polished Aggregate Friction Value (PAFV – AS1141.41/42)

In NSW, the measurement of aggregate polishing values can be obtained from using a horizontal testing wheel/flat mould system and applied to Australian Polished Aggregate Friction Value (PAFV).

An aggregate's resistance to polishing is measured by performing pendulum friction tests on laboratory polished pieces of aggregate and comparing to a known reference sample of 'Panmure Basalt'. The Australian standards relevant to polishing of aggregates are:

AS1141.40-1999: Methods for sampling and testing aggregates – Polished aggregate friction value - Vertical road-wheel machine.

AS1141.41-1999: Methods for sampling and testing aggregates – Polished aggregate friction value – Horizontal bed machine.

AS1141.42-1999: Methods for sampling and testing aggregates – Pendulum friction test.

The first two standards describe alternative methods for accelerated polishing of aggregates in the laboratory, using coarse and then fine abrasive materials to wear away (polish) the aggregate's micro texture. Following polishing by either method, the third standard is used to

determine the friction angle value of the resulting polished aggregate using a pendulum friction test. Depending on the test method used, this value is then reported as the PAFV (AS1141-41 & 42, and also NSW RMS 3151 & 3152) or PSV (other states) of the aggregate.

The two polishing regimes in AS1141.40 and AS1141.41 were designed to give results that were comparable between the two methods. However, there are differences between the PAFV and the PSV test, including the coarse abrasive, water and abrasive feed rates, polishing time, reference aggregate and rubber for friction testing. Therefore, PSV and PAFV results may not be directly comparable.

The RMS 3151 and 3152 specification standards for use as sealing aggregates and asphalt aggregate vary slightly with regards to their PAFV requirements. Aggregates with PAFV values of greater than 44 are required for sealing aggregates and greater than 48 for asphalt. The general accepted rule is that the higher the PAFV value, the better the performance of the aggregate in terms of skid resistance. This should also be judged in conjunction with suitable aggregate strength, abrasion and durability results.

Sample ID	Rock Type	PAFV Value	RMS 3151 Sealing Aggregate	RMS 3152 Asphalt Aggregate
DDH2 - composite (0.0 to 100.0m)	Rhyolitic Tuff	50	>44	≥48

Table 18 – PAFV Results (AS1141.41/42)

Samples that were crushed and tested in accordance AS1141.41 and AS1141.42 displayed PAFV values in excess of the specification requirements. It is therefore assessed that all resource material tested may be suitable for use as both sealing and asphalt aggregate.

Whilst all samples tested performed well relating to their strength and abrasion characteristics, durability and mineralogical make up should also be considered when assessing their performance.

The rhyolite materials from DDH2, performed well and are likely to be suitable materials for sealing and asphalt aggregate.

3.15 Methylene Blue Value

Methylene Blue (MB) dye absorption has been used for a considerable period as a means of determining and specifying the presence of clay minerals in aggregates. The test determines the quantity of MB dye required to coat the active agents in the soil in a mono molecular layer.

The test is completed on the passing 75-micron fraction recovered from a sample of fine aggregate of known mass. A MB of 1 mg/ml is titrated against slurry of the passing 75-micron material. As each aliquot of MB is added, the sample is tested for end point by removing a small drop of the slurry on a stirring rod and placing the dyed dust and liquid drop onto a filter paper. The filter paper draws off a 'halo' of water from around the dust particles. At the end point, when the dust cannot absorb any further MB, this 'halo' is permanently stained a light blue colour. The Methylene Blue Value (MBV) of the aggregate is reported as the number of milligrams of dye absorbed per gram of material passing 75 microns.

The MBV expresses the quantity of MB required to cover the total surface of the clay fraction with a mono-molecular layer of the MB. It is therefore proportional to the product of the clay content times the specific surface of the clay. However, the result can be affected by the presence of organics, zeolites and iron hydroxides. Some literature also suggests minor absorption by carbonates and unbalanced charged particles on freshly crushed surfaces, but these effects are considered minor. The MB test is used to evaluate the amount and nature of deleterious fines in a fine aggregate which may cause stripping. The higher the MBI the more susceptible the aggregate or mix will be to stripping.

The ISSA procedure (Bulletin145) recommends that mineral aggregate fillers and fines be rejected if the MBV exceeds 10mg/g for basalt rocks or 7 mg/g for grit stones (meta-Greywackes). RMS specifications for fines used in asphalt (RMS 3152) require further investigation of an aggregate if the MBV exceeds 8mg/g.

Sample ID	Rock Type	Methylene Blue Value (mg/g)	Recommended Limit RMS 3152
DDH2 - composite (0.0 to 100.0m)	Rhyolitic tuff	2.5	8mg/g

Table 19 - Methylene Blue Value Results

The MBV obtained on crushed fines from borehole DDH2 indicate that the clay activity in the samples tested is substantially below recommended limits.

This would indicate that rock quarried from the resource meets the recommended specification requirements and would be unlikely to liberate further fines during the production processes.

3.16 Atterberg Limits (AS1289.3.1.1, 3.2.1, 3.3.1, 3.4.1)

The knowledge of the crushed rock soil consistency is important in defining or classifying a rock type and aids in predicting crushed rock performance when used as a construction material. The soil or crushed rock fines have been assessed for their soil consistency by means of the Atterberg Limit test.

For cohesive soils, there is a range of moisture contents within which the soil (silt or clay) is of a plastic consistency. The Atterberg Limits test provides a means of measuring the plastic range of a cohesive soil in numerical terms.

Water can be added and mixed into cohesive soil until the soil becomes slurry, and behaves as a viscous fluid. This is defined as the liquid state. As the cohesive soil is dried back slowly and evenly it begins to gain strength and offers resistance to deformation. The condition is known as the plastic state. Further reduction of moisture in the soil will cause it to shrink and become stiffer until it shows little plastic condition and the cohesive soil becomes brittle. This is defined as the semi-solid state. With further drying, the soil will continue to shrink until no further moisture can be removed. At this point further drying has no more effect on the volume change and the soil is defined as being in a solid state.

The change from one phase to the next does not occur at a precise time but takes place as a transition over a period of time. The plastic range is then reported in the following terms:

- Liquid Limit (W_L) Liquid state;
- Plastic Limit (W_P) Plastic state;
- Plasticity Index (IP) Plasticity Index;
- Linear Shrinkage (Ls) Liquid linear shrinkage.

The plasticity index is not determined by a test, it is the measure of plasticity of a soil as the difference between the Liquid Limit (W_L) and the Plastic Limit (W_P). The liquid Linear Shrinkage (Ls) is defined as the decrease in length, expressed as a percentage of the original length.

The plastic limit test is used throughout engineering specifications and the construction industry as a defining test of materials plasticity.

The test is performed on the material component that passes the 425-micron test sieve. The materials are then moistened and cured and its respective plastic and liquid state determined.

Rock samples obtained from the resource were crushed and the produced fines were collected and tested to determine their plasticity. Atterberg Limit test results are presented in Table 20.

Sample ID	Rock Type	Liquid Limit (%)	Plastic limit (%)	Plasticity Index	Linear shrinkage (%)
DDH2 - composite (0.0 to 100.0m)	Rhyolitic Tuff	Not obtainable	Not obtainable	Non-Plastic	0.0

Table 20 – Atterberg Limits Results (AS1289.3.1.1, 3.2.1, 3.3.1, 3.4.1)

The crushed rock fines produced from the composite sample from DDH2 sources did not display any reaction to the Atterberg Limits test. They were Non-Plastic, meaning that the liquid limit and or plastic limit could not be obtained due to the non- reactive nature of the fines produced.

The production of Non-plastic fines from these source rocks is ideal for the production of good quality aggregate for use as concrete and road sealing purposes, but is not ideal for the production of top quality road base and select which require some plasticity in order to produce a cohesive uniform compactable blend.

Further testing of overburden materials should be performed in order to establish likely blending ratios required going forward for the introduction of plastic fines during the production process whilst producing road base materials.

3.17 Point Load Test (RMS T223)

The point load strength index test is a method developed for determining the strength of rock specimens in the field with portable equipment.

Specimens in the form of either rock core (the diametric and axial test) or irregular lumps (the irregular lump test) are broken by application of a concentrated load using a pair of conical platens. A point load strength index (Is (50)) is obtained and may be used to classify rocks by strength. It can also be used to estimate uniaxial compressive strength where index-to-strength conversation factors are used.

The point load test is an accepted rock mechanics testing procedure used for the calculation of a rock strength index. This index can be used to estimate other rock strength parameters. The rock strength determined by the point load test, like the load frame strengths that they estimate, is an indication of the intact rock strength, and not necessarily the strength of the rock mass.

Specimens should be tested at close to their in-situ moisture content and strengths may vary due to sample geometry and rock properties. Rock that is bedded, schistose or otherwise shows observable anisotropy should be tested in both weakest and strongest directions. The uncorrected point load strength is corrected using equivalent core diameter calculation and expressed in MPa as Point Load Index (Is (50)).

Sample ID	Rock Type	Strength Diametral Is50 (MPa)	Strength Axial Is50 (MPa)	Anisotropy Index
DDH2 (4.20 to 4.35m)		8.95	8.00	1.12
DDH2 (10.00 to 10.20m)		9.54	5.28	1.80
DDH2(22.30 to 22.50m)		11.82	9.71	1.22
DDH2(30.50 to 30.70m)		7.06	6.37	1.11
DDH2 (41.70 to 41.80m)		11.79	9.14	1.29
DDH2 (51.20 to 51.40m)	Rhyolitic Tuff	7.67	5.29	1.45
DDH2 (62.90 to 63.20m)		11.79	8.34	1.41
DDH2 (70.70 to 71.00m)		5.41	1.67	3.25
DDH2 (80.60 to 81.20m)		11.82	1.46	8.10
DDH2 (90.40 to 91.20m)		9.78	9.16	1.07
DDH2 (99.80 to 100.00m)		11.85	7.20	1.65

The point load test results performed on rocks from this resource indicate that the material has I_{550} strength values range between 1.5 MPa to 12 MPa, placing it in the high strength to extremely high strength classification category. Rock of this strength rating is generally suitable for most construction purposes.

3.18 Uniaxial Compressive Strength (AS4133.4.2.1)

Compressive strength is probably the most widely used and quoted rock engineering parameter. Under uniaxial load conditions, the maximum stress that a rock sample can sustain is referred to as Uniaxial Compressive Strength (UCS). It is one of the most important mechanical properties of rock material, used in design, analysis and modelling of quarry benches and infrastructure design.

The method is based on a rock sample of 54 mm in diameter and with a height from 2.5 to 3 times greater than its diameter. The specimen is placed in the compression test machine and compressive force is applied to the sample. The force should have a constant increase over time until the sample breaks and is disintegrated, while simultaneously axial and lateral deformations are measured. The result is Uniaxial Compressive Strength. If axial deformations are measured, Young's modulus of elasticity can be obtained. Where lateral deformations are measured, the sample's Poisson's coefficient is also obtained.

Sample ID	Rock Type	UCS (MPa)
DDH2(4.20 to 4.35m)		215
DDH2 (24.70 to 24.90m)		209
DDH2 (44.90 to 45.10m)	Rhyolitic Tuff	201
DDH2 (48.70 to 49.30m)		137
DDH2(99.80 to 100.00m)		228

Table 22– Uniaxial Compressive Strength Results (AS4133.4.2.1)

The uniaxial compressive strengths obtained indicate the intact rock strength of the sample to be strong to very strong.

Point load testing indicated that the material is between 1.5 Mpa to 12 MPa (very strong), while Uniaxial Compressive Strength testing performed on a sample of rhyolitic tuff as outlined in Table 22 returned values ranging from 137MPa to 215MPa on samples taken over a range of depths from the surface to a depth of 100m. These results indicate a good correlation with the numerous point load strength tests performed on DDH2. These uniaxial compressive strengths

mirror the Point load testing performed and indicate the rock from DDH2 is in the "very strong" range over the majority of the cored profile.

3.19 Petrographic Examination (ASTM C295)

Petrographic examination of rock and source material is normally conducted in accordance with ASTM C295 by preparation of a petrological thin section of the source rock in question (microns thick), mounted on a glass slide suitable for use in a stereoscopic polarising microscope of high resolution at 500 x magnification.

A point count of approximately 100 individual points on the slide is made by viewing the slide and identifying and classifying each point. Due to the nature of this test, the results can vary significantly from one petrographic to another for the same sample. For this reason, petrographic examination reports are often used as a screening tool only for specifics, to identify potentially deleterious minerals and any classification is normally confirmed or denied based upon subsequent physical, mechanical or chemical tests undertaken on rock samples taken from the source in question.

Selected samples of core of each rock type encountered were dispatched for analysis. Geochempet Services performed Petrographic analysis and geological descriptions for potential uses on the material based on the mineralogical assessment. The results of the petrographic analysis are summarised in the table below.

Sample Number	Identification	Durable	Suitable for Concrete Aggregate	Suitable for Concrete Sand	Suitable for Road base	Suitable for Asphaltic/ Sealing Aggregate	Suitable for Rail Ballast	Suitable for Rip Rap	Free Silica Content (%)
DDH2 P-1	Hematized Rhyolitic Tuff	Yes	Yes (i)(ii)	Yes (i)	Yes	Yes	Yes	Yes	29 - 30
DDH2 P-2	Rhyolitic Tuff	Yes	Yes (i)(ii)	Yes (i)	Yes	Yes	Yes	Yes	29

 Table 23 – Petrographic Analysis Results (ASTM C295)

(i) Provided that the appropriate precautions are taken in mix and engineering design to take into account the potential for mild or slow deleterious alkali-silica reactivity.

(ii) Quality may be diminished by the presence of about 6% weak minerals, which could increase water demand in concrete products.

4.0 Summary

Based on the laboratory testing and drilling investigation program undertaken the resource primarily consists of two representative rock types of similar quality, varying quantities and depths across the site. Both these materials from an engineering, quarrying and construction perspective are very similar in nature whilst being technically different from a geological viewpoint. The laboratory testing undertaken has been performed on a composite sample of both rock types. The conclusions regarding the geological differences between both rock types are set out below along with summary of the combined properties of both materials from an engineering and construction perspective.

Both rock types presented for petrographic analysis from DDH2 are described as crystal acid tuffs with a composition equivalent broadly to rhyolite or more specifically to rhyodacite (i.e. equivalent to acid volcanic rock) which is now devitrified, only slightly altered, finely crystalline,

unweathered, non - porous and carrying between 2% and 6% weak mineral inclusions. Both rock types characterised are classified as hard, strong and predicted to be durable.

The Hematized Rhyolitic Tuff/ Rhyodacite samples tested had 17% to 19% of finely microcrystalline quartz and it is predicted to have substantial potential for deleterious alkali-silica reactivity in concrete.

However, if appropriate precautions are taken such as the use of flyash and shrinkage limited cement blends in the production and design phase this can be controlled. RMS T363 accelerated Mortar Bar testing performed by Network Geotechnics on crushed aggregate from DDH2 in combination with Port Kembla GP cement are classified as slowly reactive as the sample has less than 0.10% expansion at 10 days (Slowly-Reactive) and have greater than 0.10% expansion (Slowly Reactive) after 21 days.

Due to the non-porous and unweathered nature of the Hematized Rhyolitic Tuff /Rhyodacite it is also likely to be suitable for use as Dimension Stone and Armour Rock subject to consideration of the impact of jointing or veining from visual inspection of the quarry face.

The rock is considered to have adequate strength and durability for use in Asphaltic / Sealing Aggregate and is also suitable for use as a source of Roadbase, Rail Ballast, and Rip Rap.

The RMS 3151 and 3152 specification standards for use as sealing aggregates and asphalt aggregate vary slightly with regards to their PAFV requirements. Aggregates with PAFV values of greater than 44 are required for sealing aggregates and greater than 48 for asphalt. The general accepted rule is that the higher the PAFV value, the better the performance of the aggregate in terms of skid resistance. This should also be judged in conjunction with suitable aggregate strength, abrasion and durability results.

The sample that was crushed and tested in accordance AS1141.41 and AS1141.42 returned a PAFV value of 50. This is in excess of the specification requirements of either 44 or 48. It is therefore assessed that the resource material tested may be suitable for use as both sealing and asphalt aggregate.

The uniaxial compressive strengths mirror the Point load testing performed and indicate the rock from DDH2 is in the "very strong" range over the majority of the cored profile, with Uniaxial compressive strength testing performed on core samples indicate the material has a compressive strength of 137 to 215 MPa.

The produced fines were non-plastic and water absorption test results indicate that the weighted average performed on coarse and fine material obtained was 1.7%. This is within the specified requirements for concrete & asphalt but is also expected to improve once suitable crushing and processing techniques are implemented.

Chemical testing performed demonstrate the low soluble salts and relatively low Methylene Blue Adsorption rates are consistent with material suitable for use as Concrete Sand and Asphalt filler.

The free silica content within both samples is about 29% to 30% with 10% and 12% as common Quartz and 17% to 19% as finely micro-crystalline quartz all locked within crystalline rock. Cerchar abrasivity index testing also indicates that material from this source will be highly abrasive on plant and equipment.

The products may be won either separately or in conjunction with each other and blended to meet individual specification requirements. Table 25 below outlines the potential suitability for each material source.

				Potential Product Suitability										
Sample Number	Identification	Durable	Concrete Aggregate	Concrete Sand	Road Base	Asphaltic/ Sealing Aggregate	Rail Ballast	Rip Rap	Select Fill					
DDH2	Hematized Rhyolitic Tuff/ Ryhodacite (ii)	Yes	Yes (i)	Yes (i)	Yes	Yes	Yes	Yes	Yes					
DDH2 S-2	Rhyolitic Tuff Trachyte (ii)	Yes	Yes (i)	Yes (i)	Yes	Yes	Yes	Yes	Yes					

Table 24 – Material Suitability Summary

(i) Provided that the appropriate precautions are taken in mix and engineering design to take into account the potential for mild or slow deleterious alkali-silica reactivity.

(ii) The Rhyolitic Tuff/ Rhyodacite are likely to be suitable for dimension stone and marine armour rock, if defect spacing is minimal and suitable boulders are procured.

5.0 Limitations

The findings presented in the report and used as the basis for recommendations presented herein were obtained using normal, industry accepted geotechnical design practices and standards. To our knowledge, they represent a reasonable interpretation of the general conditions of the site.

The extent of testing associated with this assessment is limited to discrete borehole locations. It should be noted that subsurface conditions between and away from the borehole locations may be different to those observed during the field work and used as the basis of the recommendations contained in this report.

If subsurface conditions encountered during construction differ from those given in this report, further advice should be sought without delay.

Data and opinions contained within the report may not be used in other contexts or for any other purposes without prior review and agreement by Qualtest. If this report is reproduced, it must be in full.

If you have any further questions regarding this report, please do not hesitate to contact Alan Cullen or the undersigned.

For and on behalf of Qualtest Laboratory (NSW) Pty Ltd

Jason Lee Principal Geotechnical Engineer

Alan Cullen Principal Geotechnician

Attachments:

Appendix A: Appendix B: Results of Laboratory Testing – Qualtest (2017) Results of Laboratory Testing – External (2017) **APPENDIX A:**

Results of Laboratory Testing - Qualtest (2017)



QUALTEST Laboratory (NSW) Pty Ltd (20708) 8 Ironbark Close Warabrook NSW 2304 T: 02 4968 4468 F: 02 4960 9775 E: admin@qualtest.com.au W: www.qualtest.com.au ABN: 98 153 268 896

				Repo	rt No: MAT:NEW17	
Material Test Rep	oort					Issue No: 1
Client: VGT Pty Ltd Unit 4/30 Glenwo Thornton NSW					Accredited for compliance wit The results of the tests, calibr measurements included in thi to Australian/national standar	ations and/or s document are tracea
Principal: Project No.: NEW15P-0045 Project Name: Material Testing						
				ACCREDITATION	NATA Accredited Laboratory Date of Issue: 8/03/2017	Number: 18686
Sample Details					ze Distribution	
Sample ID: NEW17 Client Sample ID: -	W-0307S01			Method: Drying by:	AS 1141.11.1 Oven	
Sampling Method:SampleDate Sampled:23/01/2Source:On-Site				Note:	Sample Not Washed	
Material: Rock Co Specification: No Spe Project Location: Thornto Sample Location: DDH2	cification			Sieve Size 19.0mm 13.2mm 9.5mm 6.7mm 4.75mm	% Passing 100 92 73 58 43	Limits
Other Test Results				2.36mm 1.18mm	26 17	
Description Finer 75µm (%) Drying Method Misshapen Particles (%) Flat Particles (%) Elongated Particles (%) Flat & Elongated Particles (%) Calliper Ratio	Method AS 1141.12 AS 1141.14	Result 2 Oven 7 5.8 1.6 0.0 2:1	Limits	600µm 425µm 300µm 150µm 75µm	11 8 6 2 0	
Unrounded P Misshapen Particles (%)	SD values used for fract AS 1141.14	ion selection 3				
Flat Particles (%) Elongated Particles (%) Flat & Elongated Particles (%)	A3 1141.14	1.9 1.2 0.0				
Calliper Ratio Unrounded P	SD values used for fract	3:1 ion selection		Chart		
Nominal Sample Size (mm) Nature of Sample Agg Size and Crush Details Fraction Size Wet Strength (kN) Dry Strength (kN) Wet/Dry Strength Variation (%) Breakdown Occurred	Con	20 shed Rock stant Rate) + 9.5 mm 215 246 13 No		% Passing		
Cylinder Size (diameter in mm)		150		40		/
Los Angeles Value Test Grading	AS 1141.23	17 B		30		
Loss 19.0 to 13.2 mm (%) Loss 13.2 to 9.5 mm (%) Loss 9.5 to 4.75 mm (%)	AS 1141.24	0.1 0.3 0.1		mugal	300µm	4.75mm 8.7mm 9.5mm 13.2mm

Comments

NP = Non Plastic



QUALTEST Laboratory (NSW) Pty Ltd (20708) 8 Ironbark Close Warabrook NSW 2304 T: 02 4968 4468 F: 02 4960 9775 E: admin@qualtest.com.au W: www.qualtest.com.au ABN: 98 153 268 896

				Керо	rt No: MAT:NEW17	
Material Test Rep	ort					Issue No:
Client: VGT Pty Ltd Unit 4/30 Glenwoo Thornton NSW 23	od Drive				Accredited for compliance with The results of the tests, calibra measurements included in this to Australian/national standard	ations and/or document are tracea
Dringingly				NATA		
Principal:					XX	
Project No.: NEW15P-0045 Project Name: Material Testing				WORLD RECOGNISED	Approved Signatory: Adam Dv (Senior Geotechnician) NATA Accredited Laboratory N Date of Issue: 8/03/2017	
Sample Details				Particle Siz	ze Distribution	
Sample ID: NEW17W	/-0307S01			Method:	AS 1141.11.1	
Client Sample ID: -				Drying by:	Oven	
Sampling Method:SampledDate Sampled:23/01/201Source:On-Site	17			Note:	Sample Not Washed	
Material: Rock Cor						
Specification: No Specification:				Sieve Size 19.0mm	% Passing	Limits
Project Location: Thornton, Sample Location: DDH2	, NSVV			13.2mm	100 92	
				9.5mm	73	
				6.7mm	58	
				4.75mm	43	
Other Test Results				2.36mm	26	
				1.18mm	17	
Description	Method	Result	Limits	600µm	11	
Loss 1.18 to 0.600 mm (%)		0.5		425µm 300µm	8	
Total Weighted Loss (%) Uncompacted Bulk Density (t/m ³)	AS 1141.4	0.3		150µm	6 2	
Compacted Bulk Density (t/m ³)	A0 1141.4	1.50		75µm	0	
Aggregate Moisture Condition	As	Received			· ·	
Nominal Size Of Sample (mm)		20				
Apparent Particle Density (t/m³)	AS 1141.5	2.60				
Particle Density Dry (t/m ³)		2.47				
Particle Density SSD (t/m³)		2.52				
		∩ 4				
		2.1		_		
Apparent Particle Density - Weighted (t/m ³)	AS 1141.6.1	2.59		_		
Apparent Particle Density - Weighted (t/m³) Particle Density Dry (t/m³)	AS 1141.6.1	2.59 2.48		_		
Apparent Particle Density - Weighted (t/m³) Particle Density Dry (t/m³) Particle Density SSD (t/m³)	AS 1141.6.1	2.59 2.48 2.52		Chart		
Apparent Particle Density - Weighted (t/m³) Particle Density Dry (t/m³) Particle Density SSD (t/m³) Water Absorption (%)		2.59 2.48 2.52 1.7		Chart		
Apparent Particle Density - Weighted (t/m³) Particle Density Dry (t/m³) Particle Density SSD (t/m³) Water Absorption (%) Apparent Particle Density - Coarse (t/m³)	AS 1141.6.1 AS 1141.6.1	2.59 2.48 2.52		Chart		
Apparent Particle Density - Weighted (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Apparent Particle Density - Coarse (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³)		2.59 2.48 2.52 1.7 2.58		1.000		
Apparent Particle Density - Weighted (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Apparent Particle Density - Coarse (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%)	AS 1141.6.1	2.59 2.48 2.52 1.7 2.58 2.49 2.53 1.3		1.000		/
Apparent Particle Density - Weighted (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Apparent Particle Density - Coarse (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Sample History	AS 1141.6.1 AS 1289.1.1	2.59 2.48 2.52 1.7 2.58 2.49 2.53 1.3 Air-dried		1.000		
Apparent Particle Density - Weighted (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Apparent Particle Density - Coarse (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Sample History Preparation	AS 1141.6.1 AS 1289.1.1 AS 1289.1.1	2.59 2.48 2.52 1.7 2.58 2.49 2.53 1.3 Air-dried Dry Sieved				
Apparent Particle Density - Weighted (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Apparent Particle Density - Coarse (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Sample History Preparation Linear Shrinkage (%)	AS 1141.6.1 AS 1289.1.1	2.59 2.48 2.52 1.7 2.58 2.49 2.53 1.3 Air-dried Dry Sieved 0.0				
Apparent Particle Density - Weighted (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Apparent Particle Density - Coarse (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Sample History Preparation Linear Shrinkage (%) Mould Length (mm)	AS 1141.6.1 AS 1289.1.1 AS 1289.1.1	2.59 2.48 2.52 1.7 2.58 2.49 2.53 1.3 Air-dried Dry Sieved 0.0 250				
Apparent Particle Density - Weighted (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Apparent Particle Density - Coarse (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Sample History Preparation Linear Shrinkage (%) Mould Length (mm) Crumbling	AS 1141.6.1 AS 1289.1.1 AS 1289.1.1	2.59 2.48 2.52 1.7 2.58 2.49 2.53 1.3 Air-dried Dry Sieved 0.0 250 No		100 100 100 100 100 100 100 100 100 100		
Apparent Particle Density - Weighted (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Apparent Particle Density - Coarse (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Sample History Preparation Linear Shrinkage (%) Mould Length (mm) Crumbling Curling	AS 1141.6.1 AS 1289.1.1 AS 1289.1.1	2.59 2.48 2.52 1.7 2.58 2.49 2.53 1.3 Air-dried Dry Sieved 0.0 250		100 100 100 100 100 100 100 100 100 100		
Water Absorption (%) Apparent Particle Density - Weighted (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Apparent Particle Density - Coarse (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Sample History Preparation Linear Shrinkage (%) Mould Length (mm) Crumbling Curling Cracking Liquid Limit (%)	AS 1141.6.1 AS 1289.1.1 AS 1289.1.1	2.59 2.48 2.52 1.7 2.58 2.49 2.53 1.3 Air-dried Dry Sieved 0.0 250 No No		100 100 100 100 100 100 100 100 100 100		
Apparent Particle Density - Weighted (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Apparent Particle Density - Coarse (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Sample History Preparation Linear Shrinkage (%) Mould Length (mm) Crumbling Curling Cracking	AS 1141.6.1 AS 1289.1.1 AS 1289.1.1 AS 1289.3.4.1 AS 1289.3.1.1	2.59 2.48 2.52 1.7 2.58 2.49 2.53 1.3 Air-dried Dry Sieved 0.0 250 No No No		100 100 100 100 100 100 100 100 100 100		
Apparent Particle Density - Weighted (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Apparent Particle Density - Coarse (t/m ³) Particle Density Dry (t/m ³) Particle Density SSD (t/m ³) Water Absorption (%) Sample History Preparation Linear Shrinkage (%) Mould Length (mm) Crumbling Curling Cracking Liquid Limit (%)	AS 1141.6.1 AS 1289.1.1 AS 1289.1.1 AS 1289.3.4.1 AS 1289.3.1.1	2.59 2.48 2.52 1.7 2.58 2.49 2.53 1.3 Air-dried Dry Sieved 0.0 250 No No No No No		100 100 100 100 100 100 100 100 100 100		4 Term 6 Trum 8 Sam 13 Jan 19 Jan

Comments

NP = Non Plastic



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Issue Number:

This report replaces all previous issues

Point Load Strength Report - Diametral and Axial Testing of Rock Core

															A 1		This report replace	all previous issue
Client:	VGT Pty	/ Ltd						R	eport No.:			PL:NEW	/17W-0073		Acc	redited for complic	nce with ISO/IEC 1702	5.
Project:	Materia	al Testing						Р	roject No.:			NEW15P-0045 {This document may not be reproduced except in full.}					pt in full.}	
ocation:	: Thornto	on, NSW						V	Vork Order	No:		NEW	/17W-0073	NA	TA		NX	7
								S	ample No.:				S01		-	\bigcirc	(I)	/
Date Sampled: 8/12 - 13/12/2016														Approved Si	gnatory:	Adam Dwy		
Date Tested: 10/01/2017													WORLD REC			Title: S	enior Geotechnicia	
		•												ACCREDI	TATION	Date	of Issue:	13/01/201
Test Met	hod: AS	5 4133.4.1 - 2007													NATA Accredi	ted Laboratory N	lumber:	1868
							Diametral Te	st				Axial Test						
Borehole	Test Depth (m)	Rock Type	Moisture Condition	Length L (mm)	Diameter D (mm)	Load P (kN)	l _s (Mpa)	Size Correction	І _{s(50)} (Мра)	Strength Classification	Width W (diameter) (mm)	Platen Separation D (mm)	Load P (kN)	l _s (Mpa)	Size Correction	I _{s(50)} (Mpa)	Strength Classification	Anisotropy Index I _{a(50)}
DDH2	0.10 - 0.25	Light Brown	N	99.5	60.9	2.52	0.68	1.09	0.74	м	60.9	40	2.25	0.73	1.050	0.76	м	0.98
DDH2	1.95 - 2.05	Blue / Pink	N	99.3	60.9	34.31	9.25	1.09	10.11	EH	60.9	41.22	17.01	5.32	1.057	5.62	VH	1.80
DDH2	4.20 - 4.35	Blue / Pink	N	111.3	60.9	30.37	8.19	1.09	8.95	VH	60.9	36.78	22.14	7.76	1.030	8.00	νн	1.12
DDH2	5.90 - 6.10	Blue / Pink	N	133.7	60.9	24.03	6.48	1.09	7.08	VH	60.9	36.84	21.04	7.37	1.030	7.59	νн	0.93
DDH2	7.80 - 8.00	Blue / Pink	N	127.6	60.9	29.42	7.93	1.09	8.67	VH	60.9	36.81	15.93	5.58	1.030	5.75	VH	1.51
DDH2	10.00 - 10.20	Blue / Pink	Ν	108.6	57.0	29.20	8.99	1.06	9.54	VH	60.9	45.52	17.26	4.89	1.081	5.28	νн	1.80
DDH2	11.40 - 11.60	Light Grey / Pink	Ν	141.7	60.9	15.32	4.13	1.09	4.51	VH	60.9	44.13	9.10	2.66	1.073	2.85	н	1.58
DDH2	13.90 - 14.10	Blue / Pink	N	88.5	60.9	32.75	8.83	1.09	9.65	VH	60.9	40.29	26.04	8.34	1.051	8.76	VH	1.10

3.54

6.79

10.39

0.29

11.82

9.44

6.05

11.85

7.06

3.33

VH

VH

EH

L

EH

VН

VH

EH

VH

VH

Abbreviation

EH

VH

н М

L

VL

60.9

60.7

60.9

60.3

60.7

60.7

60.7

60.7

60.6

60.6

41.37

49.42

38.28

41.4

61.32

50.7

58.31

35.54

43.2

38.9

89.0	60.7	11.24	3.05	1.09
Strength Cl	assification:	I _{s(50)} M	pa Ter	m
		> 10	Ext	remely High

12.01

22.92

35.25

0.97

40.00

31.87

20.42

40.00

23.79

3.24

6.22

9.50

0.27

10.83

8.65

5.54

10.86

6.48

1.09

1.09

1.09

1.09

1.09

1.09

1.09

1.09

1.09

N = Natural S = Saturated

D = Drv

Light Grey

Blue

Blue

Light Brown

Blue

Blue

Blue / Brown

Blue

Dark Blue

Dark Blue

DDH2

DDH2

DDH2 DDH2

DDH2

DDH2

DDH2

DDH2

DDH2

DDH2

Moisture Condition:

15.00 - 15.30

16.75 - 17.00

18.40 - 18.60

20.50 - 20.60

22.30 - 22.50

24.70 - 24.90

27.00 - 27.20

28.50 - 28.70

30.50 - 30.70

32.30 - 32.60

Ν

Ν

Ν

Ν

Ν

Ν

Ν

Ν

Ν

Ν

146.7

92.4

115.6

68.7

117.6

104.8

118.9

130.8

163.0

60.9

60.7

60.9

60.3

60.8

60.7

60.7

60.7

60.6

ly High Strength 3 to 10 Very High Strength 1 to 3 **High Strength**

0.3 to 1 Medium Strength 0.1 to 0.3 Low Strength Very Low Strength < 0.1

Comments: * Specimens approached the loading apparatus' maximum capabilities, so testing was halted short of failure. The true result is > 40 kN. The true corresponding Mpa results will also be greater than reported.

** Specimens fractured through weak vein.

9.37

19.25

20.72

0.51

39.83

34.43

16.37

19.69

19.90

5.58

2.92

5.04

6.98

0.16

8.40

8.79

3.63

7.17

5.97

1.86

1.058

1.100

1.039

1.056

1.155

1.106

1.142

1.021

1.067

1.042

3.09

5.54

7.26

0.17

9.71

9.72

4.15

7.32

6.37

1.94

VH

VH

VH

L

VH

VH

VH

VH

VH

н

1.15

1.22

1.43

1.71

1.22

0.97

1.46

1.62

1.11

1.72



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Point Load Strength Report - Diametral and Axial Testing of Rock Core

		Diametral Test		Axial Test		
Test Metho	d: AS 4133.4.1 - 2007			NATA Acc	redited Laboratory Number:	18686
				ACCREDITATION	Date of Issue:	13/01/2017
Date Tested	d: 10/01/2017			WORLD RECOGNISED	Title:	Senior Geotechnician
Date Sampl					Approved Signatory:	Adam Dwyer
		Sample No.:	S01		CAL	\sim
Location:	Thornton, NSW	Work Order No:	NEW17W-0073	NATA	N	
Project:	Material Testing	Project No.:	NEW15P-0045		{This document may not be reprodu	ced except in full.}
Client:	VGT Pty Ltd	Report No.:	PL:NEW17W-0073		Accredited for compliance with ISO/	EC 17025.
					This report	replaces all previous issues

				Diametral Test										Ах	ial Test			
Borehole	Test Depth (m)	Rock Type	Moisture Condition	Length L (mm)	Diameter D (mm)	Load P (kN)	l _s (Mpa)	Size Correction	l _{s(50)} (Mpa)	Strength Classification	Width W (diameter) (mm)	Platen Separation D (mm)	Load P (kN)	l _s (Mpa)	Size Correction	I _{s(50)} (Mpa)	Strength Classification	Anisotropy Index I _{a(50)}
DDH2	35.00 - 30.70	Blue / Pink	N	165.1	60.8	* 40.00	10.82	1.09	11.82	EH	60.8	54.31	15.49	3.68	1.124	4.14	VH	2.85
DDH2	37.20 - 37.50	Blue / Pink	Ν	190.4	60.8	31.35	8.48	1.09	9.26	VH	60.8	66.01	34.15	6.68	1.175	7.85	VH	1.18
DDH2	39.30 - 39.40	Blue / Pink	Ν	120.6	60.9	* 40.00	10.79	1.09	11.79	EH	60.9	42.11	21.37	6.54	1.062	6.95	VH	1.70
DDH2	41.70 - 41.80	Blue / Pink	Ν	111.0	60.9	* 40.00	10.79	1.09	11.79	EH	60.9	57.33	35.68	8.03	1.138	9.14	VH	1.29
DDH2	43.00 - 43.20	Grey / Light Brown	Ν	87.5	60.6	3.58	0.97	1.09	1.06	н	60.6	46.89	4.72	1.30	1.087	1.42	н	0.75
DDH2	44.90 - 45.10	Blue / Light Grey	Ν	163.5	60.7	17.29	4.69	1.09	5.12	VH	60.7	37.20	13.84	4.81	1.032	4.97	VH	1.03
DDH2	46.70 - 47.10	Blue / Pink	Ν	85.5	60.9	11.19	3.02	1.09	3.30	VH	60.9	41.32	6.11	1.91	1.057	2.02	н	1.64
DDH2	48.70 - 49.30	Blue / Pink	Ν	78.2	60.8	29.95	8.10	1.09	8.85	VH	60.8	36.80	15.92	5.59	1.030	5.75	VH	1.54
DDH2	51.20 - 51.40	Light Grey / Pink	Ν	83.3	60.9	26.02	7.02	1.09	7.67	VH	60.9	37.31	14.81	5.12	1.033	5.29	VH	1.45
DDH2	53.00 - 53.20	Light Grey / Pink	Ν	115.5	60.9	* 40.00	10.79	1.09	11.79	EH	60.9	44.63	33.56	9.70	1.076	10.43	EH	1.13
DDH2	55.10 - 55.30	Light Grey / Pink	Ν	110.4	60.8	18.85	5.10	1.09	5.57	VH	60.8	56.65	22.47	5.12	1.135	5.81	VH	0.96
DDH2	57.00 - 57.20	Blue / Pink	Ν	113.7	60.9	33.76	9.10	1.09	9.95	VH	60.9	51.16	31.33	7.90	1.109	8.76	VH	1.14
DDH2	58.90 - 59.20	Light Grey / Pink	Ν	142.5	60.8	23.36	6.32	1.09	6.90	VH	60.8	37.26	11.13	3.86	1.033	3.98	VH	1.73
DDH2	61.00 - 61.30	Light Grey / Pink	Ν	100.0	61.3	4.37	1.16	1.10	1.27	н	61.3	52.52	5.33	1.30	1.118	1.45	н	0.88
DDH2	62.90 - 63.20	Light Grey / Pink	N	124.6	60.9	* 40.00	10.79	1.09	11.79	EH	60.9	48.78	28.73	7.60	1.098	8.34	VH	1.41
DDH2	64.90 - 65.20	Blue / Pink	N	159.5	60.8	17.23	4.66	1.09	5.09	VH	60.8	36.73	12.29	4.32	1.029	4.45	VH	1.14
DDH2	67.10 - 67.30	Blue / Pink	Ν	95.1	60.8	30.80	8.33	1.09	9.10	VH	60.8	47.77	31.31	8.47	1.092	9.25	VH	0.98
DDH2	68.60 - 69.10	Light Grey / Pink	N	140.2	60.9	35.82	9.66	1.09	10.55	EH	60.9	56.83	15.85	3.60	1.136	4.09	VH	2.58
Moisture Cor	ndition:	D = Dry		Strength Cl	assification:	I _{s(50)} M	pa Te	rm		Abbreviation	C	omments	* Spec	imens appro	bached the lo	ading appa	ratus' maximu	m

EH

VH

н

Μ

L

VL

N = Natural S = Saturated

> 10 Extremely High Strength 3 to 10 Very High Strength 1 to 3 High Strength 0.3 to 1 Medium Strength 0.1 to 0.3 Low Strength Very Low Strength < 0.1

Comments: * Specimens approached the loading apparatus' maximum capabilities, so testing was halted short of failure. The true result is > 40 kN. The true corresponding Mpa results will also be greater than reported.

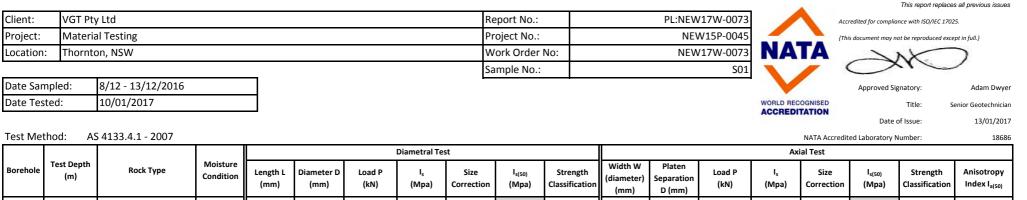
** Specimens fractured through weak vein.



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Issue Number: 1

Point Load Strength Report - Diametral and Axial Testing of Rock Core



Borenole	(m)	коск туре	Condition	Length L (mm)	Diameter D (mm)	Load P (kN)	ا _s (Mpa)	Size Correction	I _{s(50)} (Мра)	Strength Classification	(diameter) (mm)	Separation D (mm)	Load P (kN)	۱ _s (Mpa)	Size Correction	I _{s(50)} (Мра)	Strength Classification	Anisotropy Index I _{a(50)}
DDH2	70.70 - 71.00	Light Grey / Pink	Ν	105.2	60.9	18.37	4.95	1.09	5.41	νн	60.9	36.77	** 4.61	1.62	1.030	1.67	н	3.25
DDH2	72.60 - 72.80	Blue / Pink	Ν	116.9	60.9	32.87	8.86	1.09	9.69	VH	60.9	55.53	** 4.08	0.95	1.130	1.07	н	9.04
DDH2	75.10 - 75.30	Blue / Pink	Ν	121.1	60.8	28.78	7.79	1.09	8.50	VH	60.8	52.56	14.53	3.57	1.116	3.98	νн	2.13
DDH2	77.00 - 77.20	Blue	N	91.6	60.9	33.55	9.05	1.09	9.89	VH	60.9	47.61	17.74	4.81	1.092	5.25	νн	1.88
DDH2	78.80 - 79.30	Blue / White	N	92.4	61.0	32.77	8.81	1.09	9.63	VH	61.00	45.52	12.09	3.42	1.081	3.70	νн	2.61
DDH2	80.60 - 81.20	Blue / Pink	Ν	94.2	60.8	* 40.00	10.82	1.09	11.82	EH	60.80	50.03	** 5.12	1.32	1.104	1.46	н	8.10
DDH2	82.50 - 82.60	Blue / Pink	Ν	127.4	60.9	25.49	6.87	1.09	7.51	VH	60.9	52.31	** 9.17	2.26	1.115	2.52	н	2.98
DDH2	84.40 - 84.70	Blue / Pink	N	112.9	60.9	25.42	6.85	1.09	7.49	VH	60.9	43.91	18.88	5.55	1.072	5.94	νн	1.26
DDH2	86.70 - 87.00	Blue / Pink	N	134.6	60.9	31.23	8.42	1.09	9.20	VH	60.9	57.01	15.16	3.43	1.137	3.90	νн	2.36
DDH2	89.00 - 89.30	Blue / Pink	Ν	102.5	60.9	34.91	9.41	1.09	10.29	EH	60.9	38.19	23.17	7.82	1.039	8.13	VH	1.27
DDH2	90.40 - 91.20	Blue / Pink	Ν	152.1	60.9	33.19	8.95	1.09	9.78	VH	60.9	38.10	26.07	8.82	1.038	9.16	VH	1.07
DDH2	92.80 - 93.10	Light Grey / Pink	N	115.7	61.0	6.18	1.66	1.09	1.82	н	61.0	56.91	** 1.40	0.32	1.137	0.36	м	5.04
DDH2	94.80 - 95.00	Blue / Pink	N	134.8	60.8	33.69	9.11	1.09	9.95	VH	60.8	37.79	14.94	5.11	1.036	5.29	νн	1.88
DDH2	97.50 - 97.80	Blue	N	130.9	60.7	36.09	9.80	1.09	10.69	EH	60.7	50.63	37.69	9.63	1.106	10.65	EH	1.00
DDH2	99.80 - 100.00	Blue	N	138.1	60.7	* 40.00	10.86	1.09	11.85	EH	60.7	52.96	26.37	6.44	1.117	7.20	νн	1.65
Moisture Co	ndition:	D = Dry		Strength Cl	assification:	I _{s(50)} M	pa Te	rm		Abbreviation	C	omments	S: * Spec	imens appro	bached the lo	ading appa	ratus' maximu	m
		N = Natural				> 10	Ext	tremely High Stre	ength	EH	са	pabilities, so	testing was	nalted short	of failure. T	he true resu	ult is > 40 kN.	

VH

н М

L VL

N = Natural S = Saturated > 10 Extremely High Strength

< 0.1

Very High Strength 3 to 10 1 to 3 High Strength 0.3 to 1 Medium Strength

0.1 to 0.3 Low Strength Very Low Strength ** Specimens fractured through weak vein.

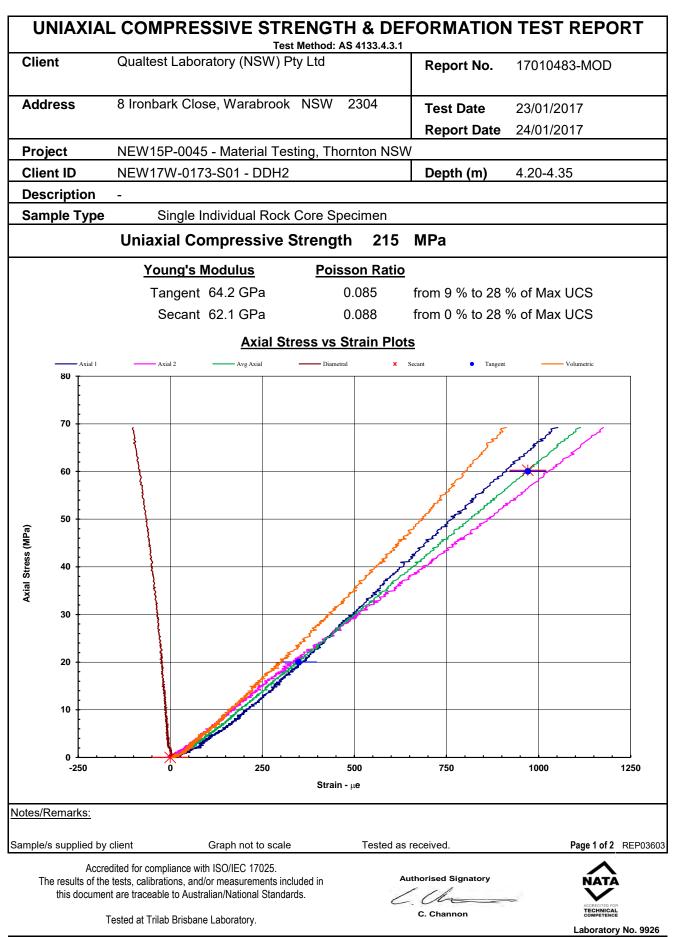
The true corresponding Mpa results will also be greater than reported.

APPENDIX B:

Results of Laboratory Testing – External (2017)



Perth 2 Kimmer Place, Queens Park WA 6107 Ph: +61 8 9258 8323



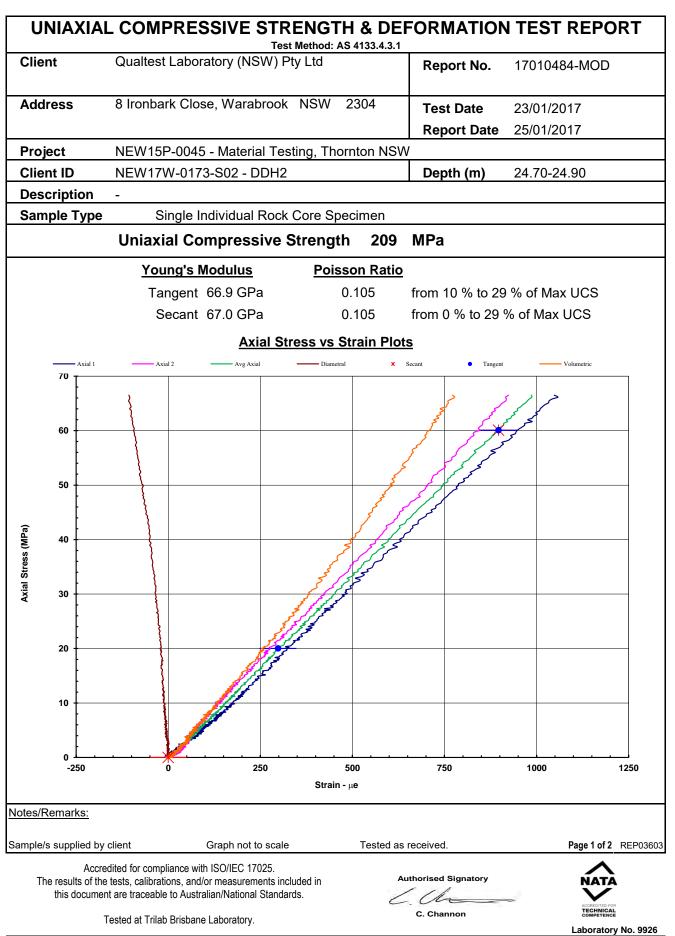


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		VE STRENGTH & Test Method: AS 4133			
Client	Qualtest Laborator	y (NSW) Pty Ltd	R	eport No.	17010483-MOD
Average Sample	e Diameter (mm)	60.8	Moisture (Content (%)	0.5
Sample Height (mm)	158.7	Wet Dens	ity (t/m³)	2.61
Duration of Test	(min)	12.95	Dry Densi	ty (t/m ³)	2.59
Rate of Loading		16.63	Bedding (?)	Nil
Mode of Failure		Disintegration .	Test Appa	ratus	Kelba 1000kN Load Cel
	CLIENT:	Qualtest Laboratory (NS	W) Pty L to	1	1
	PROJECT:	NEW15P-0045 - Materia	d		or
	LAB SAMPLE No.	Testing, Thornton NSW		BEFORE TE	51
	BOREHOLE:	17010483 NEW17W-0173-S01 - DI		E: 23/1/17 TH: 4.20-4.3	5
	Dominolit.	112 117 11-0175-501 - D1		111. 4.20-4.5	2
	CLIENT: PROJECT:	Qualtest Laboratory (NSV NEW15P-0045 - Material Testing, Thornton NSW		AFTER TEST	
				AFTER TEST	r
	PROJECT:	NEW15P-0045 - Material Testing, Thornton NSW	DATE		
	PROJECT: LAB SAMPLE No.	NEW15P-0045 - Material Testing, Thornton NSW 17010483	DATE	: 13/1/17	
	PROJECT: LAB SAMPLE No. BOREHOLE:	NEW15P-0045 - Material Testing, Thornton NSW 17010483 NEW17W-0173-S01 - DD	H2 DEPT	:: <u>13////</u> H: 4.20-4.35	
otes/Remarks: ample/s supplied by cli	PROJECT: LAB SAMPLE No. BOREHOLE:	NEW15P-0045 - Material Testing, Thornton NSW 17010483 NEW17W-0173-S01 - DD	DATE	:: <u>13////</u> H: 4.20-4.35	
ample/s supplied by cli Accredite The results of the te	PROJECT: LAB SAMPLE No. BOREHOLE:	NEW15P-0045 - Material Testing, Thornton NSW 17010483 NEW17W-0173-S01 - DD Internet State	H2 DEPT	:: <u>13////</u> H: 4.20-4.35	
ample/s supplied by cli Accredite The results of the te	PROJECT: LAB SAMPLE No. BOREHOLE:	NEW15P-0045 - Material Testing, Thornton NSW 17010483 NEW17W-0173-S01 - DD Internet State	Author	red.	



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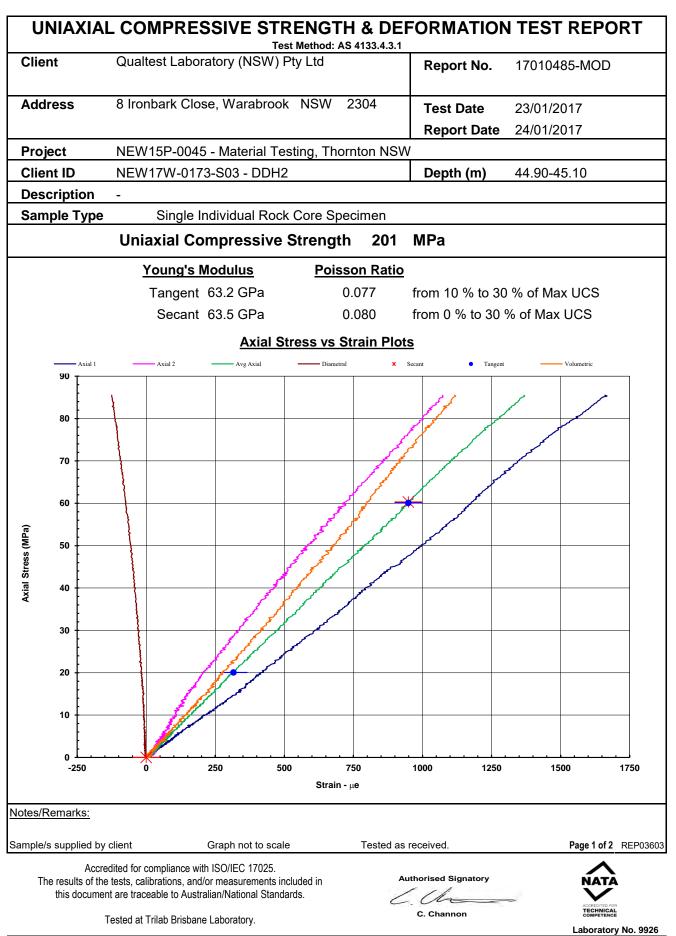


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UNIAXIAL	COMPRESSIV	/E STRENGTH Test Method: AS 4		FORMATION	I TEST REPORT
Client C	Qualtest Laboratory		133.4.3.1	Report No.	17010484-MOD
Average Sample	Diameter (mm)	60.7	Moist	ure Content (%)	0.4
Sample Height (n	nm)	158.1	Wet D	Density (t/m ³)	2.61
Duration of Test		8.73		ensity (t/m ³)	2.60
Rate of Loading (. ,	23.94		ng (°)	Nil
Mode of Failure		Disintegration		Apparatus	Kelba 1000kN Load Cell
	CLIENT:	Qualtest Laboratory (NSW) Pr	ty 1 td	
	PROJECT:	NEW15P-0045 - Mate		BEFORE TES	2T
		Testing, Thornton NS	W		51
	LAB SAMPLE No. BOREHOLE:	17010484 NEW17W-0173-S02 -	DDH2	DATE: 23/1/17 DEPTH: 24.70-24	90
	BOREHOLE:	NEW17W-0175-502 -	DDH2	DEFTH: 24.70-24	
	CLIENT:	Qualtest Laboratory ((NSW) P	ty Ltd	
	PROJECT:	NEW15P-0045 - Mate		AFTER TES	т
	LAB SAMPLE No.	Testing, Thornton NS 17010484		DATE: 23/1/17	
	BOREHOLE:	NEW17W-0173-S02 -		DEPTH: 24.70-24	.90
Notes/Remarks:					
Sample/s supplied by clie	ent Graph	not to scale	Tested as	received.	Page 2 of 2 REP03603
The results of the tes this document ar	d for compliance with ISO/IE ts, calibrations, and/or meas e traceable to Australian/Na ed at Trilab Brisbane Labora	surements included in tional Standards.	6	Authorised Signatory	ACCENTION FOR TECHNICAL Laboratory No. 9926



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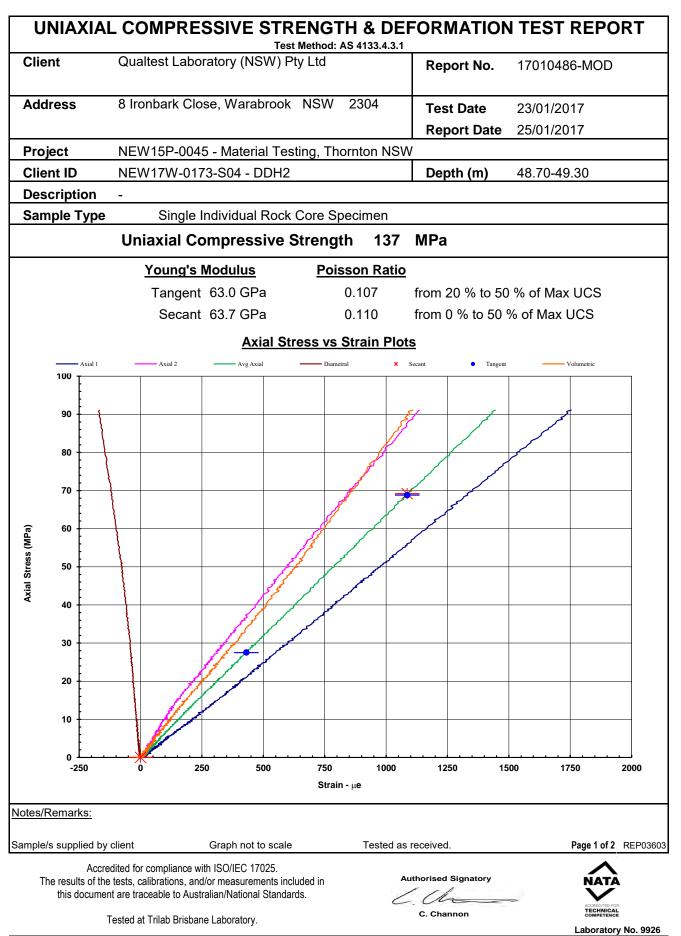


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Average Sample Diameter Sample Height (mm) Duration of Test (min) Rate of Loading (MPa/min Mode of Failure) F: CT: MPLE No. IOLE: T: CT:	60.6 160.2 10.20 19.71 Disintegration Qualtest Laboratory (NEW15P-0045 - Mate Testing, Thornton NS 17010485 NEW17W-0173-S03 -	Wet D Dry De Beddir Test A NSW) Pty rial W 1 DDH2 1	Apparatus Ke y Ltd BEFORE TEST DATE: 2.1/11/17 DEPTH: 44.90-45.10 WILLIA AFTER TEST	0.6 2.60 2.59 Nil elba 1000kN Load Ce
Sample Height (mm) Duration of Test (min) Rate of Loading (MPa/min Mode of Failure) F: CT: MPLE No. IOLE: T: CT:	10.20 19.71 Disintegration Qualtest Laboratory (NEW15P-0045 - Mate Testing, Thornton NS 17010485 NEW17W-0173-S03 -	Wet D Dry De Beddir Test A NSW) Pty rial W 1 DDH2 1	Pensity (t/m ³) ensity (t/m ³) ng (°) Apparatus Ke y Ltd BEFORE TEST DATE: 23/11/17 DEPTH: 44.90-45.10	2.59 Nil
Duration of Test (min) Rate of Loading (MPa/min Mode of Failure	F: CT: MPLE No. IOLE: F: CT:	19.71 Disintegration Qualtest Laboratory (NEW15P-0045 - Mate Testing, Thornton NS 17010485 NEW17W-0173-S03 - Qualtest Laboratory (NEW15P-0045 - Mate Testing, Thornton NS	Dry De Beddir Test A NSW) Pty rial W 1 DDH2 1	ensity (t/m ³) ng (°) Apparatus Ke y Ltd BEFORE TEST DATE: 2.1/11/4 DEPTH: 44.90-45.10	Nil
Rate of Loading (MPa/min Mode of Failure	F: CT: MPLE No. IOLE: F: CT:	19.71 Disintegration Qualtest Laboratory (NEW15P-0045 - Mate Testing, Thornton NS 17010485 NEW17W-0173-S03 - Qualtest Laboratory (NEW15P-0045 - Mate Testing, Thornton NS	Beddir Test A NSW) Pty erial W 1 DDH2 1	ng (°) Apparatus Ke y Ltd BEFORE TEST DATE: 23/1/17 DEPTH: 44.90-45.10	Nil
Mode of Failure	F: CT: MPLE No. IOLE: F: CT:	Disintegration Qualtest Laboratory (NEW15P-0045 - Mate Testing, Thornton NS 17010485 NEW17W-0173-S03 - Qualtest Laboratory (NEW15P-0045 - Mate Testing, Thornton NS	Test A	Apparatus Ke y Ltd BEFORE TEST DATE: 2.1/11/17 DEPTH: 44.90-45.10 WILLIA AFTER TEST	
PROJEC LAB SAN BOREH CLIENT PROJEC LAB SAN	CT: MPLE No. IOLE: F: CT:	NEW15P-0045 - Mate Testing, Thornton NS 17010485 NEW17W-0173-S03 - Qualtest Laboratory (NEW15P-0045 - Mate Testing, Thornton NS	rial W IDDH2 I	BEFORE TEST DATE: 2J/1).7 DEPTH: 44.90-45.10	
PROJEC LAB SAN BOREH CLIENT PROJEC LAB SAN	CT: MPLE No. IOLE: F: CT:	NEW15P-0045 - Mate Testing, Thornton NS 17010485 NEW17W-0173-S03 - Qualtest Laboratory (NEW15P-0045 - Mate Testing, Thornton NS	rial W IDDH2 I	BEFORE TEST DATE: 2J/1).7 DEPTH: 44.90-45.10	
BOREH CLIENT PROJEC LAB SAT	IOLE:	17010485 NEW17W-0173-S03 - Qualtest Laboratory (NEW15P-0045 - Mate Testing, Thornton NS	(NSW) Pty erial	DATE: 23/11/17 DEPTH: 44.90-45.10	
BOREH CLIENT PROJEC LAB SAT	IOLE:	NEW17W-0173-S03 - Qualtest Laboratory (NEW15P-0045 - Mate Testing, Thornton NS	DDH2 I (NSW) Pty erial (W	DEPTH: 44.90-45.10	
CLIENT PROJEC LAB SAT	Г: CT:	Qualtest Laboratory (NEW15P-0045 - Mate Testing, Thornton NS	(NSW) Pty erial W	y Ltd AFTER TEST	
LAB SA		Testing, Thornton NS	W		
DOREM		17010485 NEW17W-0173-S03 -		DATE: 23/1117 DEPTH: 44.90-45.10	
tes/Remarks:			Turi		D A (2) -
mple/s supplied by client		n not to scale	Tested as I	received.	Page 2 of 2 REP03
Accredited for complian The results of the tests, calibration this document are traceable to Tested at Trilab B				Authorised Signatory	NATA



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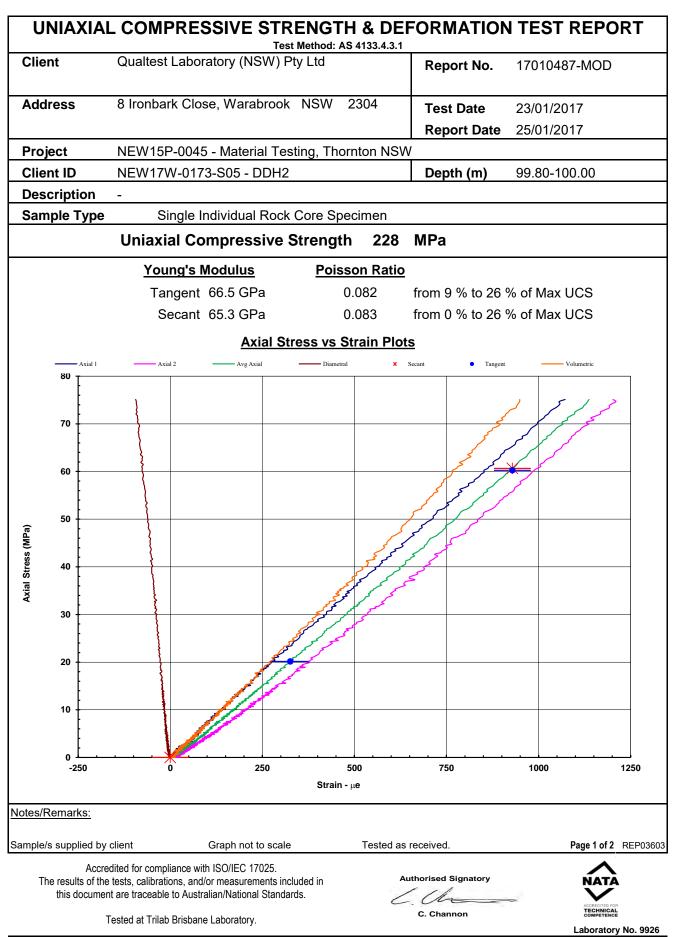


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Client Qual	Itest Laboratory	(NSW) Pty Ltd		Report No.	17010486-MOD
Average Sample Dia	meter (mm)	60.7	Moist	ure Content (%)	0.5
Sample Height (mm))	161.2	Wet	Density (t/m ³)	2.60
Duration of Test (min	ו)	9.35	Dry D	Density (t/m ³)	2.59
Rate of Loading (MPa	a/min)	14.68	Bedd	ing (°)	Nil
Mode of Failure		Disintegration	Test	Apparatus	Kelba 1000kN Load Ce
C	LIENT:	Qualtest Laboratory (NSW) P	'ty Ltd	
	ROJECT:	NEW15P-0045 - Mater		BEFORE TES	ST
1.4	AB SAMPLE No.	Testing, Thornton NSV 17010486	N		51
	OREHOLE:	NEW17W-0173-S04 - 1	DDH2	DATE: 23/1/17 DEPTH: 48.70-49	30
			Ser Se		
PF	LIENT: ROJECT:	Qualtest Laboratory (? NEW15P-0045 - Mater Testing, Thornton NSV	ial	AFTER TES	T
PF LA	ROJECT:	NEW15P-0045 - Mater	rial N		
PF LA	ROJECT: AB SAMPLE No.	NEW15P-0045 - Mater Testing, Thornton NSV 17010486	rial N	AFTER TES DATE: ZJ/ili7	
PF LA	ROJECT: AB SAMPLE No.	NEW15P-0045 - Mater Testing, Thornton NSV 17010486	rial N	AFTER TES DATE: ZJ/ili7	
PF	ROJECT: AB SAMPLE No. OREHOLE:	NEW15P-0045 - Mater Testing, Thornton NSV 17010486 NEW17W-0173-S04 - 1	rial N DDH2	AFTER TES DATE: ZJ/ili7	



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The results of calibrations and tests performed apply only to the specific instrument or sample at the time of test unless otherwise clearly stated. Reference should be made to Trilab's "Standard Terms and Conditions of Business" for further details. Trilab Pty Ltd ABN 25 065 630 506



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UNIAXIAL C	OMPRESSI	/E STRENGTH & Test Method: AS 41		FORMATION	I TEST REPORT
Client Q	ualtest Laboratory		JJ.4.3.1	Report No.	17010487-MOD
Average Sample [Diameter (mm)	60.4	Moist	ure Content (%)	0.4
Sample Height (m	m)	160.0	Wet D	Density (t/m ³)	2.64
Duration of Test (min)		10.73	Dry D	ensity (t/m ³)	2.63
Rate of Loading (MPa/min)		21.22	Beddi	ing (°)	Nil
Mode of Failure	,	Disintegration	Test /	Apparatus	Kelba 1000kN Load Cell
	CLIENT:	Qualtest Laboratory (N	SW) Pt	ty Ltd	
	PROJECT:	NEW15P-0045 - Mater		BEFORE TES	ST
	LAB SAMPLE No.	Testing, Thornton NSV 17010487		An and the set of the	
	BOREHOLE:	NEW17W-0173-S05 - E		DATE: 23/114 DEPTH: 99.80-10	0.00
	CLIENT: PROJECT:	Qualtest Laboratory (NEW15P-0045 - Mater Testing Thornton NSV	rial	ty Ltd AFTER TES	T
	LAB SAMPLE No.	Testing, Thornton NSV 17010487		DATE: 23/1/17	1
	BOREHOLE:	NEW17W-0173-S05 - 1	DDH2	DEPTH: 99.80-10	0.00
Notes/Remarks:	t Crock			received	
Sample/s supplied by clien	•		ested as	received.	Page 2 of 2 REP03603
The results of the tests this document are	for compliance with ISO/II s, calibrations, and/or mea- traceable to Australian/Na	surements included in ational Standards.		Authorised Signatory	
Teste	d at Trilab Brisbane Labora	atory.			Laboratory No. 9926

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		CERC	HAR ABRASIVI	TY INDEX TES	T REPO	ORT		
			est Method for Laboratory D	etermination of Abrasive		-		
Client	t Qualtest Laboratory (NSW) Pty Ltd Report No.		rt No.	17020012-CERC				
Address	8 Ironbar	k Close, Wara	abrook NSW 2304		Test	Date	1/02/2017	
						rt Date	2/02/2017	
Project	NEW15F	2-0045 - Mater	rial Testing - Various, I	NSW	Depth	n (m)	Not Supplied	ł
Client ID		-0307-S01 - DDH2 Sample Type		Single Indivi Core Specin				
Descriptio	UN -		SAMP	LE DETAILS				
Sample Dia	meter (mm):		60.6	Moisture Conter	at (9/).		0.2	
Sample Dia			69.9	Dry Density (t/m			2.64	
Surface Typ		Smoot	h (Saw Cut) Surface	Wet Density (t/m	-		2.65	
ounder y		Giilou		S OF TESTING	. /	<u>I</u>	2.00	
Hardness o	of Tip Used	17 HRC	Hardness of Tip Use	d 39 HRC	Hard	ness of Ti	p Used	53 HRC
Average Dia	-	*CAI	Average Diameter (m	_		ge Diamet		*CAI
0.5		5.02	0.34	3.39		0.24		2.40
			CAI v's	n : High abrasiven Hardness Plot				
6.00			Test Data	Line of Best	Fit			
5.00								
4.00								
					-			
B 3.00								
2.00								
1.00								
0.00		10	20 Ha	30 rdness (HRC)	40		50	60
Remarks:								
ample/s supplie	ed by client		* CAI values corrected for s	mooth surface.			Page: 1 of 2	REP068
The results of	the tests, calibrati nent are traceable	liance with ISO/IEC ons, and/or measur to Australian/Nation Brisbane Laborator	ements included in nal Standards.	Authorised S C. Chan C. Chan		5		ACCHEDITED FOR TECHNICAL COMPETENCE
							Lal	ooratory No. 992

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		AR ABRASIVITY INDEX		e Cerchar Method	
Client	Qualtest Laboratory (NS	SW) Pty Ltd	Report No.	17020012-CER	C
		BEFORE & AFTER PHOT	<u></u>		
			-		
	CLIENT:	Qualtest Laboratory (NSW) I	Pty Ltd		
	PROJECT:	NEW15P-0045 - Material Testing - Various, NSW	BEFORE TEST		
	LAB SAMPLE No.	17020012	DATE:1/02/17		
	BOREHOLE:	NEW17W-0307-S01 - DDH2	DEPTH: Not Supplie	ed	
	CLIENT:	Qualtest Laboratory (NSW) I	Pty Ltd		
	PROJECT:	NEW15P-0045 - Material Testing - Various, NSW	AFTER TEST		
	LAB SAMPLE No.	17020012	DATE: 1/02/17		
	BOREHOLE:	NEW17W-0307-S01 - DDH2	DEPTH: Not Supplied	d	
Remarks:					
The results	Accredited for compliance with ISO/IEC of the tests, calibrations, and/or measure cument are traceable to Australian/Natior Tested at Trilab Brisbane Laborator	ements included in nal Standards.	ace. orised Signatory Contractions C. Channon		REP06801

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ACCURATE QUALITY RESULTS FOR TOMORROW'S ENGINEERING

	ORGANIC MA		REPORT			
Client:	Qualtest Laboratory Pty Ltd	Source:	NEW 17W-0307-S01			
Address:	8 Ironbark Close, Warabrook NSW 2304	Sample Description:	Sandy GRAVEL			
Project:	Material Analysis	Report No:	B35669-OC			
Job No:	B17041	Lab No:	B35669			
Test Proce		f the organic matter cont				
Sampling: Preparatio	Sampled by Client n: Prepared in accordance with the test method		Date Sampled:	23/01/2017		
	Organic Matter (%)		0.1			
			Authorised Signatory:			
NAT	The results of the tests, calibrations and/or measurements included in this document are traceable to Australian/national standards. Accredited for compliance with ISO/IEC 17025. This document shall not be reproduced, except in full.		Brok	7/02/2017		
	NATA Accredited Laboratory Number: 14874		Brad Morris	Date:		
MAC GEO				Macquarie Geotechnical 3 Watt Drive Bathurst NSW 2795		



PRODUCT TEST REPORT

Test completed on: 17-03-2017

36389

ME70306

01-03-2017

RMS 3258

QTPO17-0071

NEW Aggregates

NEW17W-0307-S01

SAMI Bitumen Class 170

Test Report No.:

Client Sample No.:

SAMI Sample No.:

Date on P. Order:

Sample Details:

Binder Type:

Specification:

Product:



SAMI Bitumen Technologies

12 Grand Ave, Camellia NSW 2142 Or PO Box 164 Winston Hills, NSW 2153

Laboratory @samibitumen.com.au

Ph: 02 9638 0150 Fax: 02 8209 4873

Method	Property	Result	Specification
RMS T238	Initial Adhesion using SAMI C170 binder with 7% kerosene*, on NEW17W0307-S01 aggregates, precoated with SAMI standard precoat plus 1% Redicote 422/60, soaked		Maximum 10% stripping
	curing condition and	<5%	
	unsoaked curing condition	<5%	
RMS T230	Resistance to stripping using SAMI C170 binder on same precoated aggregates, plus 1% Redicote 422/60. Oven temperature for conditioning = 68.5C and tendency for aggregates to crumble when pulled is <2%	<2%	Maximum 10% stripping

Certificate Issued Date: 17-03-2017 Sampling Method: Test as received Testing Operator Name: M. T. Softening point of SAMI C170 (ME70258) = 48.5C; *Viscosity of cutback binder at test temperature =15000 stokes

p. Ch.

Authorised Officer of the Company B. Chik, Quality Manager

Doc: SAMI-IT09M29MC170 Issue A Revision 0 09/05/2007 Page 1 of 1



NEW Aggregates

NEW17W-0307-S01

PRODUCT TEST REPORT

Test completed on: 17-03-2017

36390

ME70306

01-03-2017

RMS 3268

QTPO17-0071

Polyseal S35E

Test Report No.:

Client Sample No.:

SAMI Sample No.:

Date on P. Order:

Sample Details:

Binder Type:

Specification:

Product:



SAMI Bitumen Technologies

12 Grand Ave, Camellia NSW 2142 Or PO Box 164 Winston Hills, NSW 2153

Laboratory @samibitumen.com.au

Ph: 02 9638 0150 Fax: 02 8209 4873

Method	Property	Result	Specification
RMS T238	Initial Adhesion using Polyseal S35E binder with 8% kerosene, on NEW17W-0307-S01 aggregates, precoated with SAMI standard precoat plus 1% Redicote 422/60, soaked curing condition and unsoaked curing conditions	<5 <5	Maximum 10% stripping
RMS T230	Resistance to stripping using Polyseal S35E binder on same precoated aggregates plus 1% Redicote 422/60. Oven temperature for conditioning = 74.0C and tendency for aggregates to crumble when pulled is <2%	<2	Maximum 10% stripping

Certificate Issued Date: 28-10-2016 Sampling Method: Test as received Testing Operator Name: G. Y. Softening point of Polyseal S35E (C17176) = 54.0C For RMS T238, viscosity of cutback binder at test temperature = 15000 stokes

p. Ch.

Authorised Officer of the Company B. Chik, Quality Manager

Doc: SAMI-IT09M29MCS35E Issue A Revision 0 25/10/2006 Page 1 of 1

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Page 1 of 3

SYDNEY ANALYTICAL LABORATORIES

Office: PO BOX 48 ERMINGTON NSW 2115

Laboratory: 1/4 ABBOTT ROAD SEVEN HILLS NSW 2147 Telephone: (02) 9838 8903 Fax: (02) 9838 8919 A.C.N. 003 614 695 A.B.N. 81 829 182 852 NATA No: 1884

ANALYTICAL REPORT for:

QUALTEST LABORATORY PTY LTD

8 IRONBARK CLOSE WARABROOK, NSW 2304

ATTN: ADAM DWYER

JOB NO:	SAL26195H
CLIENT ORDER:	17W-0307
DATE RECEIVED:	02/02/17
DATE COMPLETED:	10/02/17
TYPE OF SAMPLES:	AGGREGATE
NO OF SAMPLES:	1



. Issued on 10/02/17

Lance Smith (Chief Chemist)

SYDNEY ANALYTICAL LABORATORIES

ANALYTICAL REPORT

JOB NO: SAL26195H CLIENT ORDER: 17W-0307

	SAMPLES	Cl %	S04 % as S03	
1	NEW17W-0307-S01	0.004	0.001	
	od Code Daration	0.001 C32 P5	0.001 C33 P5	
			C	C33 P5

RESULTS ON DRY BASIS SAMPLE DESCRIPTION: DDH2

Page 3 of 3

SYDNEY ANALYTICAL LABORATORIES

ANALYTICAL REPORT

JOB NO: SAL26195H CLIENT ORDER: 17W-0307

METHODS OF PREPARATION AND ANALYSIS

The tests contained in this report have been carried out on the samples as received by the laboratory.

- P5 Sample dried, split and crushed to -150um
- C32 Acid Soluble Chloride AS1012.20
- C33 Acid Soluble Sulphate AS1012.20

Page 1 of 3

SYDNEY ANALYTICAL LABORATORIES

Office: PO BOX 48 ERMINGTON NSW 2115

Laboratory: 1/4 ABBOTT ROAD SEVEN HILLS NSW 2147 Telephone: (02) 9838 8903 Fax: (02) 9838 8919 A.C.N. 003 614 695 A.B.N. 81 829 182 852 NATA No: 1884

ANALYTICAL REPORT for:

QUALTEST LABORATORY PTY LTD

8 IRONBARK CLOSE WARABROOK, NSW 2304

ATTN: DANE CULLEN

JOB NO:	SAL26262C
CLIENT ORDER:	17W-0307
DATE RECEIVED:	29/03/17
DATE COMPLETED:	31/03/17
TYPE OF SAMPLES:	AGGREGATE
NO OF SAMPLES:	1



Issued on 31/03/17 Lance Smith (Chief Chemist)

SYDNEY ANALYTICAL LABORATORIES

ANALYTICAL REPORT

JOB NO: SAL26262C CLIENT ORDER: 17W-0307

	SAMPLES			S04	
		olo	as	SO3	
1	NEW17W-0307-S01		0	.010	

MDL	0.001
Method Code	C34
Preparation	P5

RESULTS ON DRY BASIS SAMPLE DESCRIPTION: DDH2

Page 3 of 3

SYDNEY ANALYTICAL LABORATORIES

ANALYTICAL REPORT

JOB NO: SAL26262C CLIENT ORDER: 17W-0307

METHODS OF PREPARATION AND ANALYSIS

The tests contained in this report have been carried out on the samples as received by the laboratory.

P5 Sample dried, split and crushed to -150um

C34 Acid Soluble Sulphate - RMS T219



TEST REPORT

Client: Principal: Project: Location:	Qualtest - Material Testing Various Locations, N	SW	Job No: Tested By: Checked By:	W07/3 TM HU	3100	Sheet: Date: Date:	1 of 1 3.02.17 6.02.17
	nple Description: Sample Number: Date Sampled: Client Number:	Rock Core (DDH2) 38 23/01/2017 NEW17W-0307-S01	Sample Proced Laboratory Nu Client Job Numl	mber:	Sampled by W59855 NEW17W-03		
	TEST PROCEDU		TEST RESUL				
Sı	ıgar	Not Detected/Detected	Not Detected	1			

REMARKS:



This document is issued in accordance with NATA's accreditation requirements. Accreditied for compliance with ISO/IEC 17025

APPROVED SIGNATORY Harry Ubungen

DATE 6/02/2017

Document No. RP125 Version 2 22-6-10

Wollongong Laboratory 1318



ACN 069 211 561 Unit 1/140 Industrial Road Oak Flats NSW 2529 Telephone 61 2 4257 4458 Facsimilie 61 2 4257 4463 Email southcoast@netgeo.com.au

TEST REPORT

Client: Project: Location: GTR Number :		ocations, NSW	R R	ob Number: eport Number: eport Date: ested By:	W07/3100 - 6/02/2017 Tim Mathie	
		Sample	Identification			
Sam	ple Description :	Rock Core (DDH2)	Sampling Proce	edure: Sampled I	By Client	
S	Sample Number:	38				
Labo	oratory Number:	W59855	Date Sa	ampled: 23/01/	/2017	
	Client Number:	NEW17W-0307-S01				
DE	TERMINATION	N OF LIGHT PARTICLES	6 - AS 1141.31			
	TEST PRO	DCEDURE	TEST RESUL	TS		
	Light Particles	%	0			

REMARKS:



This document is issued in accordance with NATA's accreditation requirements. Accreditied for compliance with ISO/IEC 17025

Wollongong Laboratory 1318

APPROVED SIGNATORY Harry Ubungen DATE 6/02/2017



TEST REPORT

Client:	Qualtest Laboratory		Job No:	W07/3100	Sheet:	1 of 1
Principal: Project:	- Material Testing		Tested By:	ТМ	Date:	3.02.17
Location:	Various Locations, I	NSW	Checked By:	HU	Date:	6.02.17
S	ample Description:	Rock Core (DDH2)	Sample Pro	ocedure:	Sampled By	Client
	Sample Number:	38				
	Date Sampled:	23/01/2017	Client	Number:	NEW17W-030	7-S01
L	aboratory Number:	W59855				

Determination of Methylene Blue Adsorption Value of Road Construction Materials

TEST PROCEDURE AS 1141.66		TEST RESULT
Methylene Blue Value (MBV)	mg/g	2.5

REMARKS:



Accreditied for compliance with ISO/IEC 17025

APPROVED SIGNATORY Harry Ubungen

DATE 6/02/2017

Document No RP145.2

Version 1 30-10-12

Wollongong Laboratory 1318



ACN 069 211 561 Unit 1/140 Industrial Road Oak Flats NSW 2529 AUSTRALIA Telephone: 61 2 4257 4458 Facsimile: 61 2 4257 4463 Email: southcoast@netgeo.com.au

TEST REPORT

Accelerated mortar bar test for the assessment of alkali-reactivity of aggregate

Client: Qualtest Project: Material Testing Location: Warabrook NSW
 Job No:
 W07/3100
 Page:
 1 of 1

 Report Number:
 1
 Ed. No:
 1

 Test Date
 10/03/2017
 1

This report replaces all previous issues of the above report number.

Date Sampled: 8/12/2016

Sample Location: Not Known

Sampled by: Client

Sampling Procedure: Sampled by Client

Sample Number: W60067 (#39)

Type of aggregate: Crushed Cores (DDH2)

Source of aggregate: Not Known

Type of cement used: Port Kembla GP

Source of cement : Cement Australia Port Kembla

Test Method: RMST363

Client sample number: NEW17W-0307-S01

Flow (%)	7
Water to cement ratio	0.42

Age (days)	Change in length (%)
1	0.004
3	0.032
7	0.069
10	0.098
14	0.138
21	0.206

Table T363/A				
Aggregate	Reactivity	Classification		

ortar Bar Expansion (%) in 1M NaOH (80°C)		Classification	
10 days	ys 21 days		
< 0.10*	< 0.10*	Non-reactive	
< 0.10*	≥ 0.10*	Slowly reactive	
≥ 0.10*	>> 0.10*	Reactive	

* 0.15% for naturally occurring fine aggregates

REMARKS:



Accredited for compliance with ISO/IEC 17025

APPROVED SIGNATORY
Tim Mathie

DATE 3/04/2017



ACN 069 211 561 Unit 1/140 Industrial Road Oak Flats NSW 2529 AUSTRALIA Telephone: +61 2 4257 4458 Facsimile: +61 2 4257 4463 Email: southcoast@netgeo.com.au

TEST REPORT

Test Date

DETERMINATION OF POLISHED AGGREGATE FRICTION VALUE

Client: Qualtest Project: Material Testing Location: Warrabrook NSW Job No: W07/3100 Report Number: 1 Page: 1 of 1 Ed. No: 1

This report replaces all previous issues of the above report number.

Date Sampled: 8/12/2016

Sample Location: Not known

Sampled by: Client

Sampling Procedure: Sampled by Client

Sample Number: W60067

Sample Description: Crushed Core (DDH2)

Client Sample Number: NEW17W-0307-S01

Size of aggregate tested: -9.5+6.7

Reference material: Panmure Basalt

15/03/2017

Ambient air temperature: 25°C

Size of slider: 75mm

Polishing Test Method: AS1141.41

Friction Testing Method: AS1141.42

	Test sample	Reference sample Panmure Basalt
The unpolished test sample mean friction value corrected to a temperature of 23° C	77	75
The polished test sample mean friction value corrected to a temperature of 23° C	48	49
The polished aggregate friction value (PAFV)	50	51

REMARKS:



Accredited for compliance with ISO/IEC 17025

Wollongong Laboratory: 1318

APPROVED SIGNATORY Tim Mathie DATE 16/03/2017



Geochempet Services

ABN 980 6945 3445 PETROLOGICAL and GEOCHEMICAL CONSULTANTS Principals: K.E. Spring BSc (Hons), MAppSc and H.M. Spring B.Sc



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PETROGRAPHIC REPORT ON A DRILL CORE SAMPLE (NEW17W-0117-S01)

prepared for

QUALTEST LABORATORY (NSW) PTY LTD WARABROOK, NSW

Order Number: QTPO17-0015

Invoice Number: 00007463

Client Ref: Adam Dwyer

Kent oping

K. E. Spring B.Sc (Hons), MAppSc 7 February 2017 Page 1 of 6

FEBRUARY, 2017

Ql170201

The material contained within this report may not be quoted other than in full. Extracts may be used only with expressed prior written approval of Geochempet Services

Issued by

Sample Number:	NEW17W-0117-S01	Date Sampled :	13/12/16
Borehole Descript.:	DDH2-P1	Date Received:	13/01/17
Project Name:	Material Testing		
Work Order No.:	NEW17W-0117	Project No. : NEW	15P-0045
Location:	Warrabrook, NSW		
<u>Work Requested</u>	Petrographic analysis in relation to s base, concrete sand/aggregate, aspha rap, marine armour rock and dimensi	ltic/sealing aggregate, r	
<u>Methods</u>	Account taken of ASTM C295 Assessment of Aggregates for Concre and rock for engineering purpo. (Appendix B), and of the content of Cement and Concrete Association of entitled Alkali Aggregate Reaction - Damage to Concrete Structures in Au ASTM C 294 Standard Guide for P Ballast and to the content of the Australia (AS 2758.7 – Appendix E Engineering Purposes- Part 7: Raily ASTM C1721-09 Standard Guide Dimension Stone, and in accordance Guide for Evaluation of Rock to be a	ete, the AS2758.1 – 201 ses part 1; Concrete of the 2015 joint publi of Australia and Standa <i>Guidelines on Minimisi</i> <i>ustralia</i> and in acco <i>Petrographic Assessmer</i> ne 1996 publication 6 B), entitled Aggregates way Ballast, and in acco <i>for Petrographic A</i> ce with ASTM D4992	4 Aggregates e aggregates ication of the rds Australia, ng the Risk of ordance with nt of Railway of Standards and Rock for cordance with ssessment of -07 Standard

Identification Hematized rhyolitic tuff

Description

The supplied sample of drill core consists of moderate red, apparently unweathered, quite robust acid tuff, displaying numerous phenoclasts of transparent quartz, pinkish feldspar, and sparse biotite and opaque oxide grains set in a very finely crystalline, obviously vitroclastic matrix. The ends of the core are a parallel set of limonite and manganese-coated joint surfaces. The rock can only be very lightly scratched by a steel tool.

Ql170201

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Plate 1. Photograph of the supplied drill core sample.

A thin section was prepared to permit detailed microscopic examination in transmitted polarised light of the drill core. An approximate mineralogical composition of the rock expressed in volume percent and based on a count of 100 widely spaced points falling within the thin section, is:

Durable Minerals

- 52% finely microcrystalline feldspars and quartz (17-18%) as devitrification products after former vitric shards
- 41% coarser feldspars and quartz (12%) as devitrification products and phenoclasts
- <1% opaque oxide phenoclasts
- <1% hornblende phenoclasts
 - 5% hematite
- <1% leucoxene
- <1% epidote

Weak &/or Deleterious Minerals

2% sericite/illite trace fine clay

In thin section, the rock is seen to represent acid tuffaceous rock in which numerous subhedral, corroded and broken phenoclasts (mainly about 0.2 to 5 mm) are dispersed through a finely devitrified matrix with obvious ghosts of former welded vitric shards (about 0.2 to 0.5 mm long) and minor compressed pumice.

The phenoclasts comprise finely clouded by clays and are very slightly sericitized with rare chloritization of plagioclase, finely clay-clouded but otherwise unaltered K-feldspar and beta-form quartz along with minor biotite (now converted to hematite, epidote, leucoxene and sericite), almost completely but similarly-altered hornblende and opaque oxide. The formerly vitroclastic matrix is now devitrified to a finely microcrystalline mosaic (mainly finer than 0.01 mm) with micro-spherulites and related fibrous sheafs of mainly feldspars and quartz. Minor,

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small patches or clasts in the matrix show coarser devitrification (up to about 0.2 mm grainsize). Fine sericite (possibly illitic) and chlorite is present in minor amounts in the devitrified matrix, which is pervasively, pigmented between former vitric shards by very fine hematite along with small patches of hematitized former mafic minerals.

Comments and Interpretations

The supplied drill core sample (labelled NEW17W-0117-S01) is interpreted to be devitrified tuff which is of broadly rhyolitic composition (using the classification criteria of the International Union of Geological Sciences) or of more specific rhyo-dacitic composition (using narrower, older established British/Australian criteria). The rock is thought to have originated as acid ashflow tuff (or "ignimbrite") composed of phenoclasts of quartz and feldspars dispersed through a welded matrix of vitric shards and minor compressed pumice. At some stage after initial solidification, the tuff was finely devitrified, hematitized and slightly sericitized.

For engineering purposes the rock represented in the supplied drill core sample may be summarised as:

- former vitric crystal **acid tuff** with a composition equivalent broadly to **rhyolite** or more specifically to rhyo-dacite (i.e. equivalent to acid volcanic rock)
- now devitrified, but otherwise only slightly altered
- now finely crystalline
- unweathered
- non-porous
- carrying about 2% of weak mineral (sericite/illite and a trace of fine clays)
- quite hard
- strong

The rock is predicted to be **durable**.

Because the rock carries an estimated 17-18% of finely micro-crystalline quartz (as a devitrification product), it is predicted to have **substantial potential for deleterious alkali-***silica reactivity in concrete*.

Thus, devitrified tuff of the type represented in the supplied sample is predicted to be **suitable for use as a source of manufactured concrete sand and concrete aggregate**: provided that appropriate precautions are taken in mix and engineering design to take account of its perceived potential for substantial deleterious alkali-silica reactivity.

Guidance on how to deal with the perceived potential for deleterious alkali-silica reactivity may be found in the 1996 joint publication of the *Cement and Concrete Association of Australia* and *Standards Australia*, entitled *Alkali Aggregate Reaction - Guidelines on Minimising the Risk of Damage to Concrete Structures in Australia*.

The rock is considered to have **more than adequate strength and durability for use in asphaltic/sealing aggregate**. Some rhyolitic rock types can present problems in relation to bonding to bitumen and polishing in service.

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Page 4 of 6

Thus, devitrified tuff of the type represented in the supplied sample is predicted to be **suitable for use as a source of road base, rail ballast** (subject to compliance with the Durability Criteria of CT147 / AS2758.7) **and rip rap**.

Non-porous, unweathered rock equivalent to the supplied sample may be suitable for use as a source of durable marine armour rock. However, supplementary observations at the quarry or using product rock of large size would be required to check whether rock fragments of suitable size, free of weak or permeable joints, veins or other physical defects can be obtained: the more weathered and fractured rock should be avoided. It is common for acid volcanic and sub-volcanic rock to be fairly closely jointed, a characteristic which facilitates quarrying and crushing, but which may limit the availability of armour rock of suitably large and stable size.

It seems from petrographic examination that the rhyolitic tuff may have potential for use as a source of 'granite' dimension stone ('granite' being a very broad, ill-defined term used by stonemasons for almost any crystalline rock which is not 'marble'), subject to appropriate consideration of the impact of jointing or veining from visual inspection at the quarry and the presence of illitic clay.

Free Silica Content

The free silica content is about 29-30%, with 12% as common quartz and 17-18% as finely micro-crystalline quartz, all locked within crystalline rock.

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Ql170201

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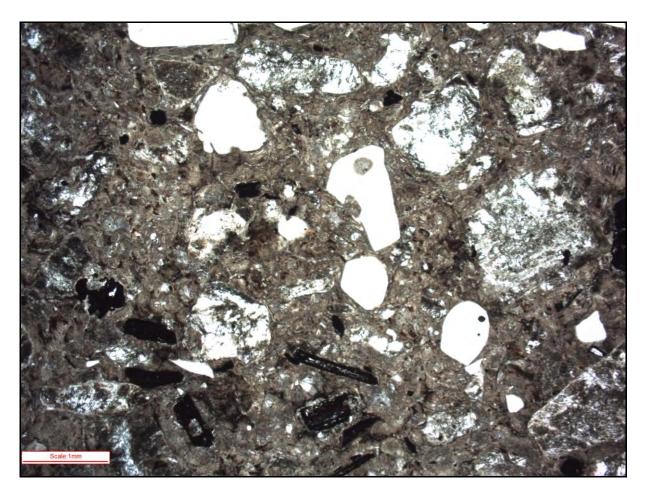
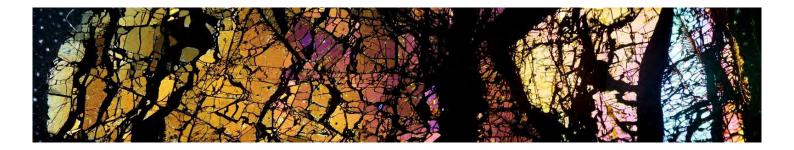


Plate 2. Low magnification, plane transmitted light image of part of hematitized acid tuff showing phenoclasts of quartz, feldspar, opaque oxide and altered biotite flakes in a finely devitrified matrix with obvious ghosts of former welded vitric shards showing fine pigmentation of hematite

FEBRUARY, 2017

Ql170201

Page 6 of 6



Geochempet Services

ABN 980 6945 3445 PETROLOGICAL and GEOCHEMICAL CONSULTANTS Principals: K.E. Spring BSc (Hons), MAppSc and H.M. Spring B.Sc



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PETROGRAPHIC REPORT ON A DRILL CORE SAMPLE (NEW17W-0117-S02)

prepared for

QUALTEST LABORATORY (NSW) PTY LTD WARABROOK, NSW

Order Number: QTPO17-0015

Invoice Number: 00007463

Client Ref: Adam Dwyer

Kent oping

K. E. Spring B.Sc (Hons), MAppSc 7 February 2017 Page 1 of 6

FEBRUARY, 2017

Ql170202

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Issued by

Sample Number:	NEW17W-0117-S02	Date Sampled:	13/12/16
Borehole Descript.:	DDH2-P2	Date Received:	13/01/17
Project Name:	Material Testing		
Work Order No.:	NEW17W-0117	Project No.: NEW1	5P-0045
Location:	Warrabrook, NSW		
<u>Work Requested</u>	Petrographic analysis in relation to s base, concrete sand/aggregate, asphal rap, marine armour rock and dimensio	ltic/sealing aggregate, ra	
<u>Methods</u>	Account taken of ASTM C295 Assessment of Aggregates for Concre and rock for engineering purpose (Appendix B), and of the content of Cement and Concrete Association of entitled Alkali Aggregate Reaction - C Damage to Concrete Structures in Au ASTM C 294 Standard Guide for P Ballast and to the content of the Australia (AS 2758.7 – Appendix B Engineering Purposes- Part 7: Raily ASTM C1721-09 Standard Guide Dimension Stone, and in accordance Guide for Evaluation of Rock to be u	ete, the AS2758.1 – 201 ses part 1; Concrete of the 2015 joint public of Australia and Standar <i>Guidelines on Minimisin</i> <i>ustralia</i> and in acco <i>Petrographic Assessmen</i> the 1996 publication of B), entitled Aggregates a vay Ballast, and in acco <i>for Petrographic As</i> with ASTM D4992-	4 Aggregates e aggregates cation of the rds Australia, ng the Risk of ordance with nt of Railway of Standards and Rock for ordance with ssessment of -07 Standard

Identification Rhyolitic tuff

Description

The supplied sample of drill core consists of light to medium grey, unweathered, quite robust acid tuff, displaying numerous phenoclasts of transparent quartz, white and pink feldspar, and sparse biotite and opaque oxide grains set in a very finely crystalline, obviously vitroclastic, matrix. The rock can only be very lightly scratched by a steel tool.

Ql170202



Plate 1. Photograph of the supplied drill core sample.

A thin section was prepared to permit detailed microscopic examination in transmitted polarised light of the drill core. An approximate mineralogical composition of the rock expressed in volume percent and based on a count of 100 widely spaced points falling within the thin section, is:

Durable Minerals

- 57% finely microcrystalline feldspars and quartz (19%) as devitrification products after former vitric shards
- 34% coarser feldspars and quartz (10%) as devitrification products and phenoclasts
- <1% opaque oxide phenoclasts
- <1% hornblende phenoclasts
- <1% hematite
 - 3% leucoxene
- <1% epidote

Weak &/or Deleterious Minerals

6% sericite/illite trace calcite trace fine clay

In thin section, the rock is seen to represent acid tuffaceous rock in which numerous subhedral, corroded and broken phenoclasts (mainly about 0.2 to 3 mm) are dispersed through a finely devitrified matrix with obvious ghosts of former welded vitric shards (about 0.2 to 0.5 mm long) and minor compressed pumice.

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The phenoclasts comprise finely clouded by clays and are slightly sericitized with rare carbonation of plagioclase, finely clay-clouded but otherwise unaltered K-feldspar and beta-form quartz along with minor biotite (now converted to hematite, epidote, leucoxene and sericite), almost completely but similarly-altered hornblende and opaque oxide. The formerly vitroclastic matrix is now devitrified to a finely microcrystalline mosaic (mainly finer than 0.01 mm) with spherulites and related fibrous sheafs of mainly feldspars and quartz. Minor, small patches or clasts in the matrix show coarser devitrification (up to about 0.2 mm grainsize). Fine sericite (possibly illitic) is present in minor amounts in the devitrified matrix, which is pervasively, faintly pigmented between former vitric shards by very fine hematite.

Comments and Interpretations

The supplied drill core sample (labelled NEW17W-0117-S02) is interpreted to be devitrified tuff which is of broadly rhyolitic composition (using the classification criteria of the International Union of Geological Sciences) or of more specific rhyo-dacitic composition (using narrower, older established British/Australian criteria). The rock is thought to have originated as acid ashflow tuff (or "ignimbrite") composed of phenoclasts of quartz and feldspars dispersed through a welded matrix of vitric shards and minor compressed pumice. At some stage after initial solidification, the tuff was finely devitrified and slightly sericitized, hematized and carbonated.

For engineering purposes the rock represented in the supplied drill core sample may be summarised as:

- former vitric crystal **acid tuff** with a composition equivalent broadly to **rhyolite** or more specifically to rhyo-dacite (i.e. equivalent to acid volcanic rock)
- now devitrified, but otherwise only slightly altered
- now finely crystalline
- unweathered
- non-porous
- carrying about 6% of weak mineral (sericite/illite and a trace of calcite and fine clays)
- quite hard
- strong

The rock is predicted to be **durable**.

Because the rock carries an estimated 19% of finely micro-crystalline quartz (as a devitrification product), it is predicted to have **substantial potential for deleterious alkali-silica reactivity in concrete**.

Thus, devitrified tuff of the type represented in the supplied sample is predicted to be **suitable for use as a source of manufactured concrete sand and concrete aggregate**: provided that appropriate precautions are taken in mix and engineering design to take account of its perceived potential for substantial deleterious alkali-silica reactivity.

Guidance on how to deal with the perceived potential for deleterious alkali-silica reactivity may be found in the 1996 joint publication of the *Cement and Concrete Association of Australia*

FEBRUARY, 2017

Ql170202

Page 4 of 6

and Standards Australia, entitled Alkali Aggregate Reaction - Guidelines on Minimising the Risk of Damage to Concrete Structures in Australia.

The rock is considered to have **more than adequate strength and durability for use in asphaltic/sealing aggregate**. Some rhyolitic rock types can present problems in relation to bonding to bitumen and polishing in service.

Thus, devitrified tuff of the type represented in the supplied sample is predicted to be **suitable for use as a source of road base, rail ballast** (subject to compliance with the Durability Criteria of CT147 / AS2758.7) **and rip rap**.

Non-porous, unweathered rock equivalent to the supplied sample may be suitable for use as a source of durable marine armour rock. However, supplementary observations at the quarry or using product rock of large size would be required to check whether rock fragments of suitable size, free of weak or permeable joints, veins or other physical defects can be obtained: the more weathered and fractured rock should be avoided. It is common for acid volcanic and sub-volcanic rock to be fairly closely jointed, a characteristic which facilitates quarrying and crushing, but which may limit the availability of armour rock of suitably large and stable size.

It seems from petrographic examination that the rhyolitic tuff may have potential for use as a source of 'granite' dimension stone ('granite' being a very broad, ill-defined term used by stonemasons for almost any crystalline rock which is not 'marble'), subject to appropriate consideration of the impact of jointing or veining from visual inspection at the quarry and the presence of illitic clay.

Free Silica Content

The free silica content is about 29%, with 10% as common quartz and 19% as finely microcrystalline quartz, all locked within crystalline rock.

Ql170202

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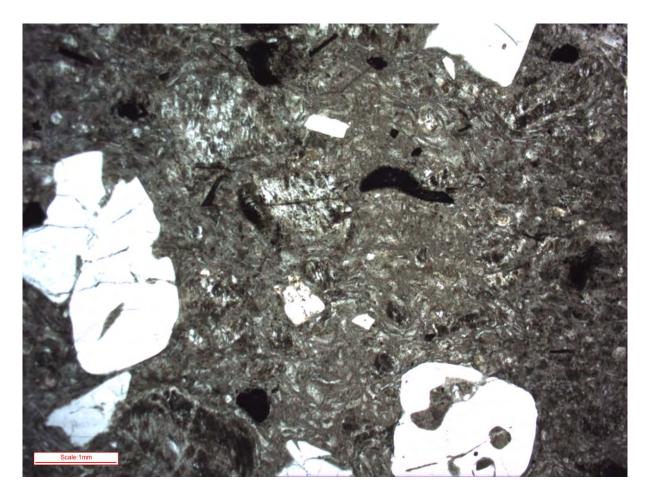


Plate 2. Low magnification, plane polarised, transmitted light image of part of acid tuff showing phenoclasts of white quartz, finely clouded feldspar, hematized and leucoxenized biotite and opaque oxide in a finely devitrified matrix with obvious ghosts of former welded vitric shards

FEBRUARY, 2017

Ql170202

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Beyond Compliance

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