



Doc Ref: WI056-03F02(rev1)- WE Addendum Letter

Date: September 20, 2024

To: University of Sydney (Darlington), C-/ Campus Infrastructure & Services

Address: Services Building G12, 22 Codrington Street Darlington, NSW 2008

RE: WI056-03- A01 ROSS STREET TEACHING & LEARNING HUB,
UNIVERSITY OF SYDNEY
WIND ENVIRONMENT ADDENDUM LETTER

Windtech Consultants previously prepared a Pedestrian Wind Environment Study report to accompany a Development Application (DA) for the proposed A01 Ross Street Teaching & Learning Hub at the University of Sydney. The report presented the results of a wind tunnel study conducted to investigate the wind environment impact of the proposed development on the critical outdoor trafficable areas within and around the site (WI056-01F03(rev1) - WE Report, dated September 20, 2024). In the time since the report was issued, the development has undergone minor changes. Windtech have reviewed the design changes, which are based on the latest architectural drawings prepared by BVN (received June 21, 2024). The notable design changes to the development (with respect to potential wind impact) are outlined below:

- Minor changes to the building envelope, including:
 - An increase in the extent (depth) of the chamfer of the south-western and north-western corners of Level 02.
 - An increase in the extent of the building envelope between Levels 03 to 06.
- The removal of the external café seating along the south-eastern corner of Level 02.
- The inclusion of permeable screening along the western aspect of Level 02.
- Minor landscape design changes across Level 02.

Following a review of the latest drawing set, Windtech expects that the changes to the building form will have a negligible impact on the majority of findings presented within the previous Pedestrian Wind Environment Study report for Level 2.

The removal of the Café allows for the external seating area along the south-eastern corner of Level 2 be assessed under the Standing Criterion as defined in the City of Sydney DCP (2012). This results in a reduction in



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the severity of the comfort exceedances in this area. It is expected that the prevailing southerly winds will still result in a downwashing and funnelling effect near the south-eastern corner of Level 2. It is also expected that the prevailing westerly winds will continue to create downwashing and funnelling effects near both the north-western and south-western corners of the development.

While it is expected that the existing densely foliating trees along Science Road will aid in wind mitigation and enhance pedestrian comfort along the southern frontage of the development, a detailed treatment scheme will need to be developed in conjunction with Windtech to ensure that the wind conditions for all trafficable outdoor areas around the development will be suitable for their intended uses. The areas highlighted above have been discussed with the design team, and a treatment scheme will be developed during the Design Development stage. This scheme may result in the inclusion of features such as strategic landscaping, screens, and/or awnings. Throughout the Design Development stage, wind tunnel testing will be undertaken to determine the extent/effectiveness of the resulting treatment scheme.

Regards,



Windtech Consultants

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Project Engineer



PEDESTRIAN WIND ENVIRONMENT STUDY

WI056-01- A01 ROSS STREET TEACHING & LEARNING HUB, UNIVERSITY OF SYDNEY

WI056-01F03(REV1)- WE REPORT

SEPTEMBER 20, 2024

Prepared for:

UNIVERSITY OF SYDNEY (DARLINGTON)

(C-/ CAMPUS INFRASTRUCTURE & SERVICES), SERVICES BUILDING G12,
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DOCUMENT CONTROL

Date	Revision History	Issued Revision	Prepared By (initials)	Instructed By (initials)	Reviewed & Authorised by (initials)
February 25, 2024	Initial.	0	LG/AFM	SWR	AFM
September 20, 2024	Updated mitigation measures.	1	ALD	SWR	EV

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EXECUTIVE SUMMARY

This report presents the results of a detailed investigation into the wind environment impact of the 'Ross Street Teaching & Learning Hub' development, located at the University of Sydney (NSW). Testing was performed at Windtech's boundary layer wind tunnel facility. The wind tunnel has a 3.0m wide working section and a fetch length of 14m, and measurements were taken from 16 wind directions at 22.5 degree increments. Testing was carried out using a 1:300 detailed scale model of the development, which was fabricated based on the architectural drawings received on December 7, 2023. The effects of nearby buildings and land topography have been accounted for through the use of a proximity model which represents an area with a radius of 375m.

Peak gust and mean wind speeds were measured at selected critical outdoor trafficable locations within and around the subject development. Wind velocity coefficients representing the local wind speeds are derived from the wind tunnel and are combined with a statistical model of the regional wind climate (which accounts for the directional strength and frequency of occurrence of the prevailing regional winds) to provide the equivalent full-scale wind speeds at the site. The wind speed measurements are compared with criteria for pedestrian comfort and safety, based on Gust-Equivalent Mean (GEM) and annual maximum gust winds, respectively. The model was tested in the wind tunnel without the effect of any forms of wind ameliorating devices such as screens, balustrades, etc., which are not already shown in the architectural drawings. The effect of vegetation was also excluded from the testing. The existing site conditions were also tested, for comparison.

The results of the study indicate that wind conditions for the majority of trafficable outdoor locations within and around the development will be suitable for their intended uses. However, some areas will experience strong winds which will exceed the criteria for comfort and/or safety. These areas have been discussed with the design team and a detailed treatment scheme will be developed in conjunction with Windtech during the Design Development stage. This scheme may result in the inclusion of features such as strategic landscaping, screens, and/or awnings. Throughout the Design Development stage, wind tunnel testing will be undertaken to determine the extent/effectiveness of the resulting treatment scheme.

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INTRODUCTION

A wind tunnel study has been undertaken to assess wind speeds at selected critical outdoor trafficable areas within and around the subject development. The test procedures followed for this wind tunnel study were based on the guidelines set out in the Australasian Wind Engineering Society Quality Assurance Manual (AWES-QAM-1-2019), ASCE 7-16 (Chapter C31), and CTBUH (2013).

A scale model of the development was prepared, including the surrounding buildings and land topography. Testing was performed at Windtech's boundary layer wind tunnel facility. The wind tunnel has a 3.0m wide working section and a fetch length of 14m, and measurements were taken from 16 wind directions at 22.5 degree increments. The wind tunnel was configured to the appropriate boundary layer wind profile for each wind direction. Wind speeds were measured using either Dantec hot-wire probe anemometers or pressure-based wind speed sensors, positioned to monitor wind conditions at critical outdoor trafficable areas of the development.

The model was tested in the wind tunnel without the effect of any forms of wind ameliorating devices such as screens, balustrades, etc., which are not already shown in the architectural drawings. The effect of vegetation was also excluded from the testing. The wind speeds measured during testing were combined with a statistical model of the regional wind climate to provide the equivalent full-scale wind speeds at the site. The measured wind speeds were compared against appropriate criteria for pedestrian comfort and safety. Areas that were exposed to strong winds have been discussed with the design team, and a detailed treatment scheme will be developed in conjunction with Windtech during the Design Development stage. These treatments could be in the form of retaining vegetation that is already proposed for the site, or including additional vegetation, screens, awnings, etc. Note however that, in accordance with the AWES Guidelines (2014), only architectural elements or modifications are used to treat winds that represent an exceedance of the existing wind conditions and exceed the safety limit.

WIND TUNNEL MODEL

Wind tunnel testing was carried out using a 1:300 scale model of the development and surroundings. The study model incorporates all necessary architectural features on the façade of the development to ensure an accurate wind flow is achieved around the model, and was constructed using a Computer Aided Manufacturing (CAM) process to ensure that a high level of detail and accuracy is achieved. The study model was fabricated based on the architectural drawings received on December 7, 2023 (referenced in Appendix F). The effect of nearby buildings and land topography has been accounted for through the use of a proximity model, which represents a radius of 375m from the development site. Photographs of the wind tunnel model are presented in Figures 1. A plan of the proximity model is provided in Figures 2.

Approximately 375m to the south-west of the site are the currently under-construction RPA project (15 storeys) and the Sydney Biomedical Accelerator (SBA). To the west across Western Avenue is the A03 site, which has an approved envelope Sydney University proposes to develop in the future. Furthermore, the University will be exploring the installation of additional fencing and landscaping on the oval immediately to the south of the site.

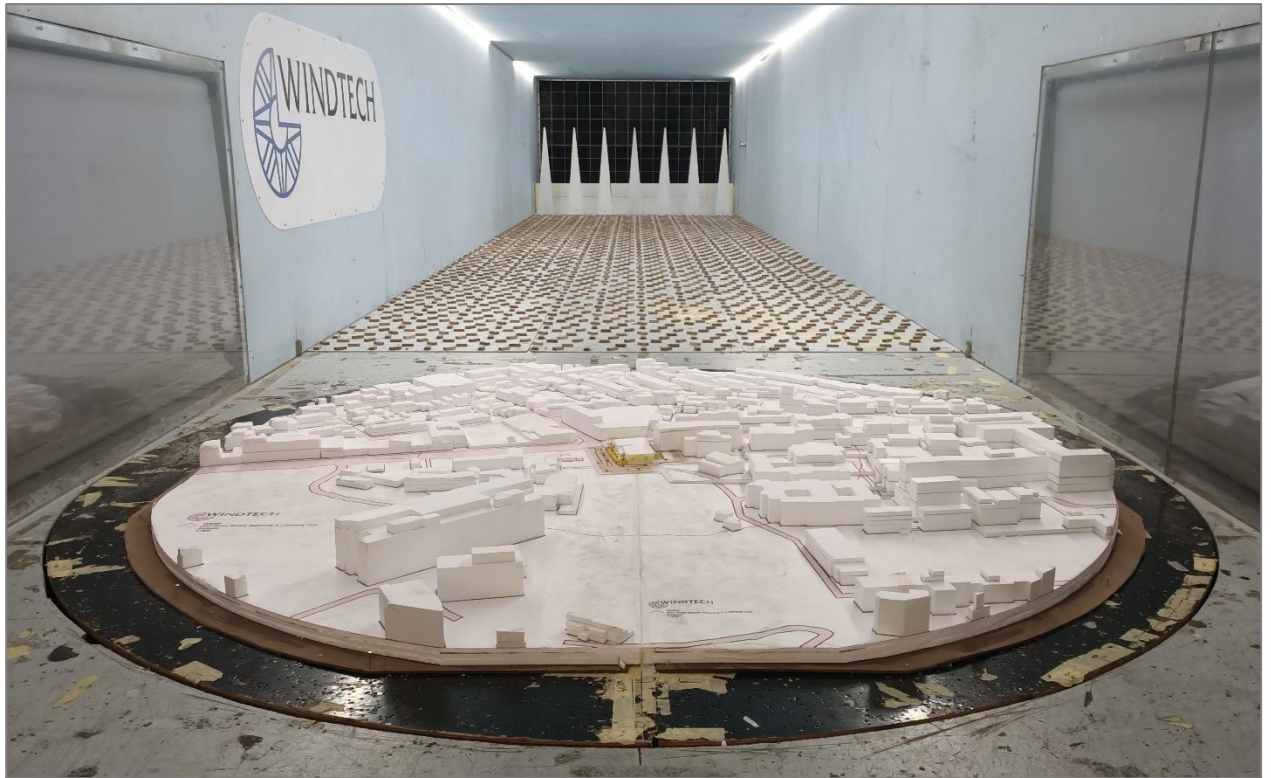


Figure 1a: Photograph of the Wind Tunnel Model – Existing (view from the south)



Figure 1b: Photograph of the Wind Tunnel Model – Existing (view from the east)



Figure 1c: Photograph of the Wind Tunnel Model – Existing (view from the north)



Figure 1d: Photograph of the Wind Tunnel Model – Existing (view from the west)

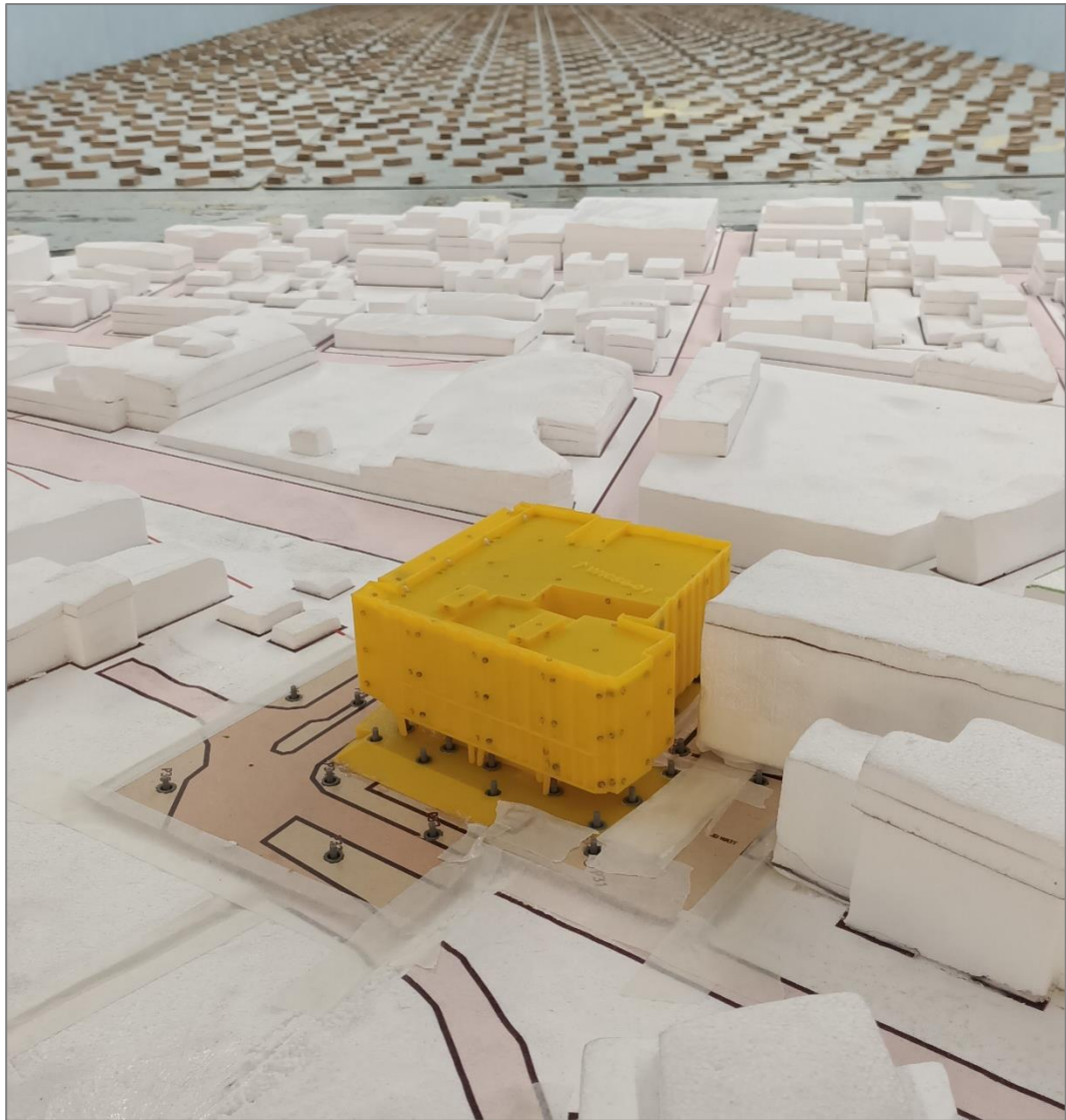


Figure 1e: Photograph of the Wind Tunnel Model – Proposed (view from the south-east)



Figure 1f: Photograph of the Wind Tunnel Model – Proposed (view from the south)



Figure 1g: Photograph of the Wind Tunnel Model – Proposed (view from the east)



Figure 1h: Photograph of the Wind Tunnel Model – Proposed (view from the north)



Figure 1i: Photograph of the Wind Tunnel Model – Proposed (view from the west)

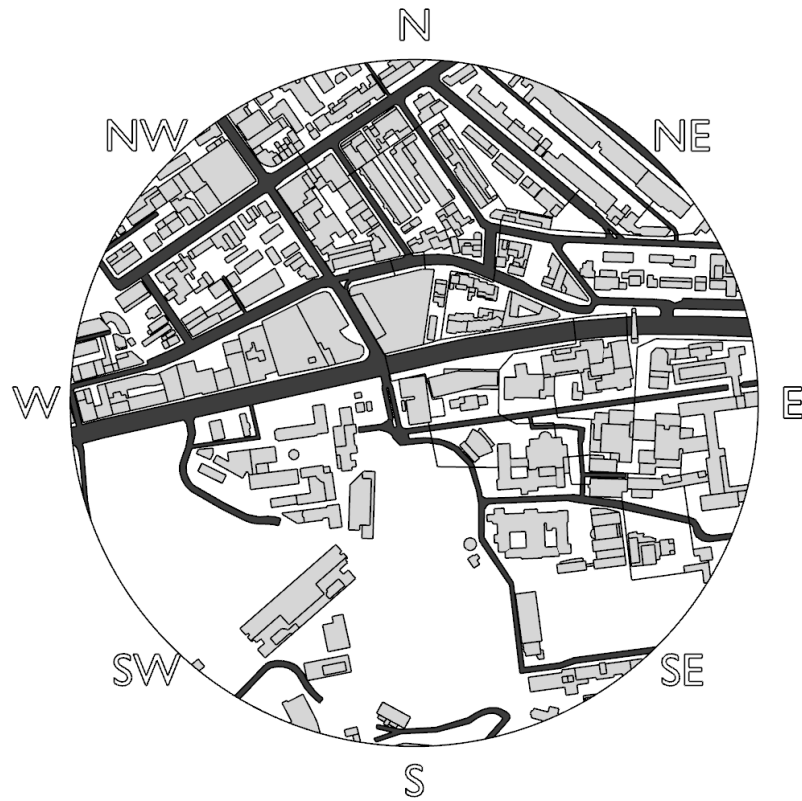


Figure 2a: Proximity Model Plan – Centred to the Existing Site



Figure 2b: Proximity Model Plan – With the Proposed Development (yellow)

BOUNDARY LAYER WIND PROFILES AT THE SITE

The roughness of the surface of the earth has the effect of slowing down the wind near the ground. This effect is observed up to the boundary layer height, which can range between 500m to 3km above the earth's surface depending on the roughness of the surface (ie: oceans, open farmland, etc). Within this range the prevailing wind forms a boundary layer wind profile.

Various wind codes and standards and other publications classify various types of boundary layer wind flows depending on the surface roughness z_0 . Descriptions of typical boundary layer wind profiles, based on D.M. Deaves and R.I. Harris (1978), are summarised as follows:

- Flat terrain ($0.002\text{m} < z_0 < 0.003\text{m}$). Examples include inland water bodies such as lakes, dams, rivers, etc, and the open ocean.
- Semi-open terrain ($0.006\text{m} < z_0 < 0.01\text{m}$). Examples include flat deserts and plains.
- Open terrain ($0.02\text{m} < z_0 < 0.03\text{m}$). Examples include grassy fields, semi-flat plains, and open farmland (without buildings or trees).
- Semi-suburban/semi-forest terrain ($0.06\text{m} < z_0 < 0.1\text{m}$). Examples include farmland with scattered trees and buildings and very low-density suburban areas.
- Suburban/forest terrain ($0.2\text{m} < z_0 < 0.3\text{m}$). Examples include suburban areas of towns and areas with dense vegetation such as forests, bushland, etc.
- Semi-urban terrain ($0.6\text{m} < z_0 < 1.0\text{m}$). Examples include centres of small cities, industrial parks, etc.
- Urban terrain ($2.0\text{m} < z_0 < 3.0\text{m}$). Examples include centres of large cities with many high-rise towers, and also areas with many closely-spaced mid-rise buildings.

The boundary layer wind profile does not change instantly due to changes in the terrain roughness. It can take many kilometres (at least 100km) of constant surface roughness for the boundary layer wind profile to achieve a state of equilibrium. Hence an analysis of the effect of changes in the upwind terrain roughness is necessary to determine an accurate boundary layer wind profile at the development site location.

The proximity model accounts for the effect of the near field topographic effects as well as the influence of the local built forms. To account for further afield effects, an assessment of the upwind terrain roughness has been undertaken based on the method given in AS/NZS1170.2:2011, using a fetch ranging from 20 to 60 times the study reference height (as per the recommendation by AS/NZS1170.2:2011). An aerial image showing the surrounding terrain is presented in Figure 3 for a range of 1.2km from the edge of the proximity model used for the wind tunnel study. The resulting mean and gust terrain and height multipliers at the site location are presented in Table 1, referenced to the study reference height (which is approximately half the height of the subject development since typically we are most interested in the wind effects at the ground plane). Details of the boundary layer wind profiles at the site are combined with the regional wind model (see Section 4) to determine the site wind speeds.

Table 1: Approaching Boundary Layer Wind Profile Analysis Summary (at the study reference height)

Wind Sector (degrees)	Terrain and Height Multiplier			Turbulence Intensity I_v	Equivalent Terrain Category (AS/NZS1170.2:2011 naming convention)
	$k_{tr,T=1hr}$ (hourly)	$k_{tr,T=10min}$ (10min)	$k_{tr,T=3s}$ (3sec)		
0	0.50	0.54	0.91	0.279	3.3
30	0.55	0.59	0.95	0.245	3.0
60	0.44	0.48	0.86	0.318	3.5
90	0.47	0.51	0.89	0.297	3.4
120	0.49	0.52	0.90	0.288	3.3
150	0.51	0.54	0.92	0.275	3.2
180	0.44	0.48	0.86	0.318	3.5
210	0.49	0.52	0.90	0.289	3.3
240	0.48	0.52	0.89	0.294	3.3
270	0.48	0.52	0.89	0.294	3.3
300	0.44	0.48	0.86	0.318	3.5
330	0.53	0.56	0.93	0.261	3.1

NOTE: These terrain and height multipliers are to be applied to a basic regional wind speed averaged over 3-seconds. Divide these values by 1.10 for a basic wind speed averaged over 0.2-seconds, 0.69 for a basic wind speed averaged over 10-minutes, or 0.66 for a basic wind speed averaged over 1-hour.

For each of the 16 wind directions tested in this study, the approaching boundary layer wind profiles modelled in the wind tunnel closely matched the profiles listed in Table 1. Plots of the boundary layer wind profiles used for the wind tunnel testing are presented in Appendix E of this report.

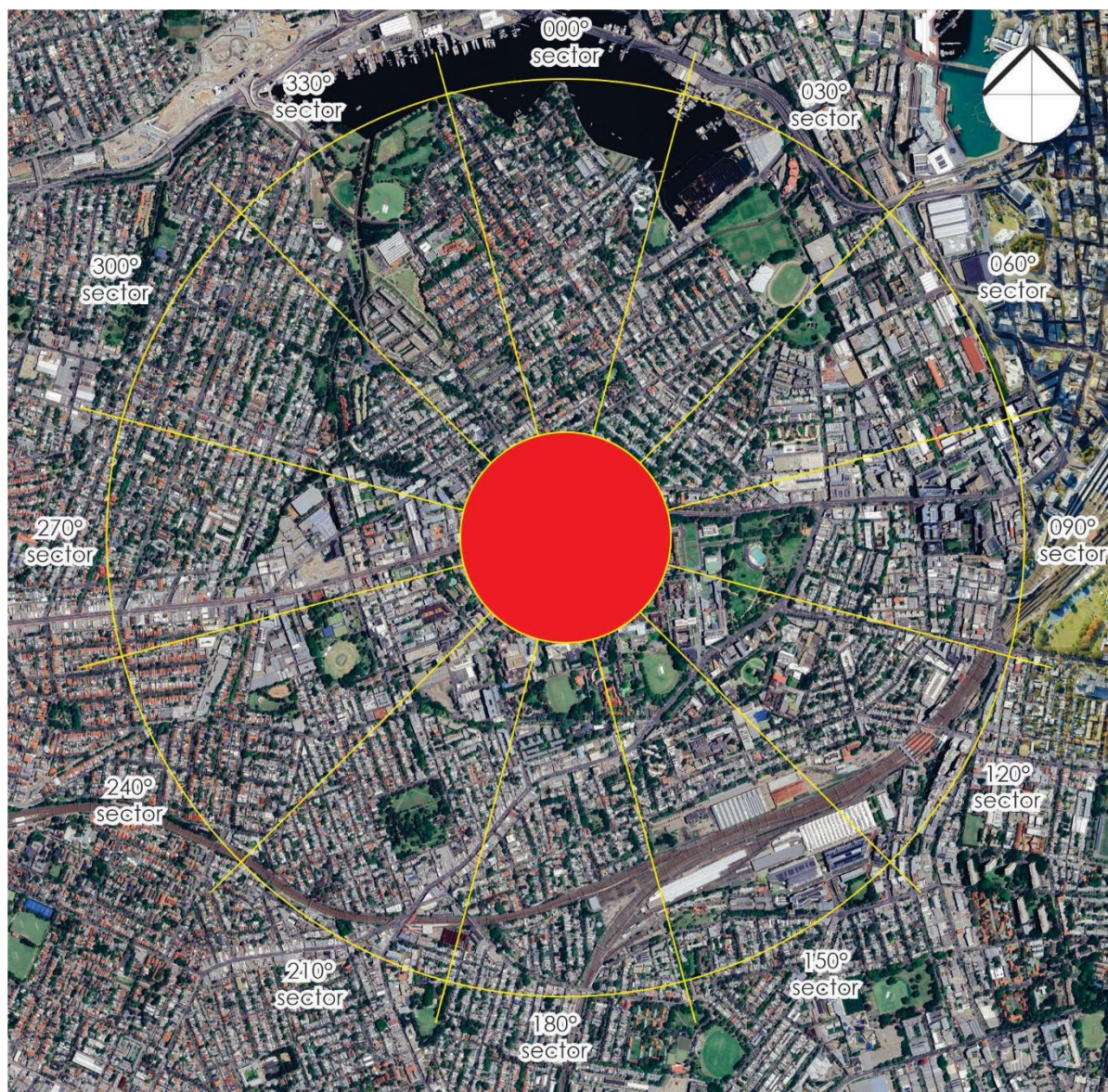


Figure 3: Aerial Image of the Surrounding Terrain (radius of 1.2km from the edge of the proximity model)

4 REGIONAL WIND MODEL

The regional wind model used in this study was determined from an analysis of measured directional mean wind speeds obtained at the meteorological recording station located at Kingsford Smith Airport (Sydney Airport). Data was collected from 1995 to 2016 and corrected so that it represents winds over standard open terrain at a height of 10m above ground for each wind direction. From this analysis, directional probabilities of exceedance and directional wind speeds for the region are determined. The directional wind speeds are summarised in Table 2. The directional wind speeds and corresponding directional frequencies of occurrence are presented in Figure 4.

The data indicates that the southerly winds are by far the most frequent winds for the Sydney region, and are also the strongest. The westerly winds occur most frequently during the winter season for the Sydney region, and although they are typically not as strong as the southerly winds, they are usually a cold wind and hence can be a cause for discomfort for outdoor areas. North-easterly winds occur most frequently occur during the warmer months of the year for the Sydney region, and hence are usually welcomed within outdoor areas since they are typically not as strong as the southerly or westerly winds.

The recurrence intervals examined in this study are for exceedances of 5% (per 90 degree sector) of the pedestrian comfort criteria using Gust-Equivalent Mean (GEM) wind speeds, and annual maximum wind speeds (per 22.5 degree sector) for the pedestrian safety criterion. Note that the 5% probability wind speeds presented in Table 2 are only used for the directional plot presented in Figure 4 and are not used for the integration of the probabilities.

Table 2: Regional Directional Wind Speeds (hourly means, at 10m height in standard open terrain) (m/s)

Wind Direction	5% Exceedance	Annual Maximum
N	5.9	9.9
NNE	9.9	12.9
NE	9.7	12.3
ENE	7.5	10.0
E	6.3	9.3
ESE	6.2	9.1
SE	7.0	10.1
SSE	8.5	12.2
S	10.3	13.9
SSW	10.0	14.1
SW	6.9	11.9
WSW	9.3	13.6
W	9.8	14.4
WNW	8.8	14.3
NW	6.7	12.6
NNW	5.5	10.7

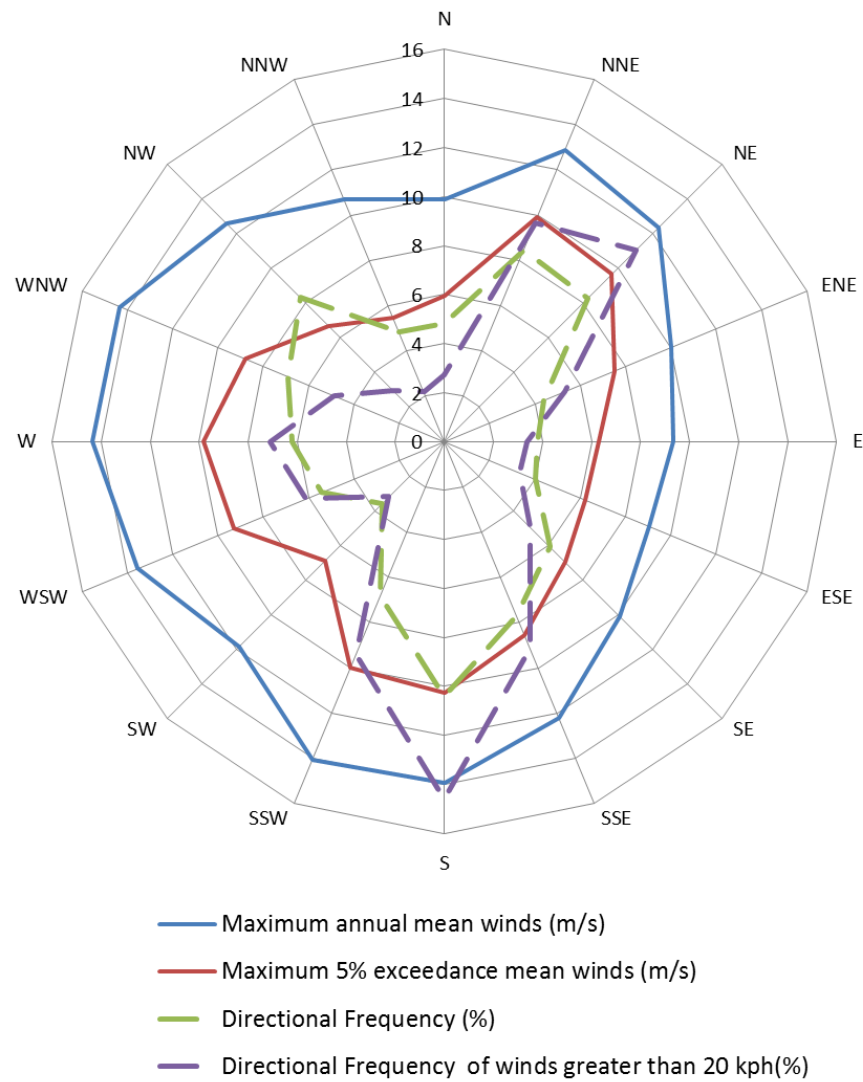


Figure 5: Annual and 5% Exceedance Hourly Mean Wind Speeds, and Frequencies of Occurrence, for the Sydney Region (at 10m height in standard open terrain)

PEDESTRIAN WIND COMFORT AND SAFETY

The acceptability of wind conditions for an area is determined by comparing the measured wind speeds against an appropriate criteria. This section outlines how the measured wind speeds were obtained, the criteria considered for the development, as well as the critical trafficable areas that were assessed and their corresponding criteria designation.

5.1 Measured Wind Speeds

Wind speeds were measured using either Dantec hot-wire probe anemometers or pressure-based wind speed sensors, positioned to monitor wind conditions at critical outdoor trafficable areas of the development. The reference mean free-stream wind speed measured in the wind tunnel, which is at a full-scale height of 200m and measured 3m upstream of the study model.

Measurements were acquired for 16 wind directions at 22.5 degree increments using a sample rate of 1,024Hz. The full methodology of determining the wind speed measurements at the site from either the Dantec Hot-wire probe anemometers or pressure-based wind speed sensors is provided in Appendix B. Based on the results of the analysis of the boundary layer wind profiles at the site (see Section 3), and incorporating the regional wind model (see Section 4), the data sampling length of the wind tunnel test for each wind direction corresponds to a full-scale sample length ranging between 30 minutes and 1 hour. Research by A.W. Rofail and K.C.S. Kwok (1991) has shown that, in addition to the mean and standard deviation of the wind being stable for sample lengths of 15 minutes or more (full-scale), the peak value determined using the upcrossing method is stable for sample lengths of 30 minutes or more.

5.2 Wind Speed Criteria Used for This Study

For this study, the measured wind conditions for the various critical outdoor trafficable areas around the subject development are compared against the criteria presented in the Draft Sydney Development Control Plan 2012 - Central Sydney Planning Review Amendment, which supersedes the criteria detailed in the City of Sydney Development Control Plan 2012 (SDCP2012).

For pedestrian comfort, the Draft Sydney DCP 2012 requires that the hourly mean wind speed, or Gust-Equivalent Mean (GEM) wind speed (whichever is greater for each wind direction), must not exceed 8m/s for walking, 6m/s for standing, and 4m/s for sitting. These are based on a 5% probability of exceedance.

For pedestrian safety, the Draft Sydney DCP 2012 defines a safety limit criterion of 24m/s, based on an annual maximum 0.5 second gust wind speed, which applies to all areas.

Furthermore, in accordance with the provisions of the Draft Sydney DCP 2012, the existing conditions for the pedestrian footpaths around the site are also analysed as part of this study to determine the impact of the subject development. If it is found that the existing conditions exceed the relevant criteria, then the target wind speed for that area with the inclusion of the proposed development is to at least match the existing site conditions.

In accordance with the provisions of the Draft Sydney DCP 2012, the wind speed assessment is undertaken for winds occurring between 6am and 10pm (AEST).

A more detailed comparison of published criteria for pedestrian wind comfort and safety is provided in Appendix A.

For this study the measured wind conditions of the selected critical outdoor trafficable areas are compared against two sets of criteria; one for pedestrian safety, and one for pedestrian comfort. The safety criterion is applied to the annual maximum gust winds, and the comfort criteria is applied to Gust Equivalent Mean (GEM) winds. In accordance with ASCE (2003), the GEM wind speed is defined as follows:

$$GEM = \max\left(\bar{V}, \frac{\hat{V}}{1.85}\right) \quad (5.1)$$

where:

$\bar{V}$ is the mean wind speed.

$\hat{V}$ is the gust wind speed.

The criteria considered in this study are summarised in Tables 3 and 4 for pedestrian comfort and safety, respectively. The results of the wind tunnel study are presented in the form of directional plots attached in Appendix C of this report. For each study point there is a plot of the annual probability of exceedance of the assigned comfort criteria, and a plot for the annual maximum gust wind speeds using the safety criterion. The velocity coefficients are also presented in the form of directional plots in Appendix D

Table 3: Pedestrian Comfort Criteria (Draft Sydney DCP 2012)

Classification	Description	Maximum 5% Exceedance GEM Wind Speed (m/s)
Sitting	Outdoor areas that involve seating such as parks, dining areas in restaurants, amphitheatres, etc.	4.0
Standing	Short duration stationary activities (generally less than 1 hour), including window shopping, waiting areas, etc.	6.0
Walking	For pedestrian thoroughfares, private swimming pools, most communal areas, private balconies and terraces, etc.	8.0

Table 4: Pedestrian Safety Criterion (Draft Sydney DCP 2012)

Classification	Description	Annual Maximum Gust Wind Speed (m/s)
Safety	Safety criterion applies to all trafficable areas.	24

5.3 Layout of Study Points

For this study a total of 38 study point locations were selected for analysis in the wind tunnel. This includes the following:

- 24 study points on Level 2 (ground plane).
- 14 study points within the surrounding areas.

The locations of the various study points tested for this study, as well as the target wind speed criteria for the various outdoor trafficable areas of the development, are presented in Figure 5 in the form of marked-up plans. It should be noted that only the most critical outdoor locations of the development have been selected for analysis.

Target Criteria

- City of Sydney DCP 2012:
 - Wind Comfort Standard for Sitting Criterion of 4m/s (5% exceedance)
 - Safety Criterion of 24m/s (gust - 0.1% exceedance) for safety.
- City of Sydney DCP 2012:
 - Wind Comfort Standard for Standing Criterion of 6m/s (5% exceedance)
 - Safety Criterion of 24m/s (gust - 0.1% exceedance) for safety.
- City of Sydney DCP 2012:
 - Wind Comfort Standard for Walking Criterion of 8m/s (5% exceedance)
 - Safety Criterion of 24m/s (gust - 0.1% exceedance) for safety.

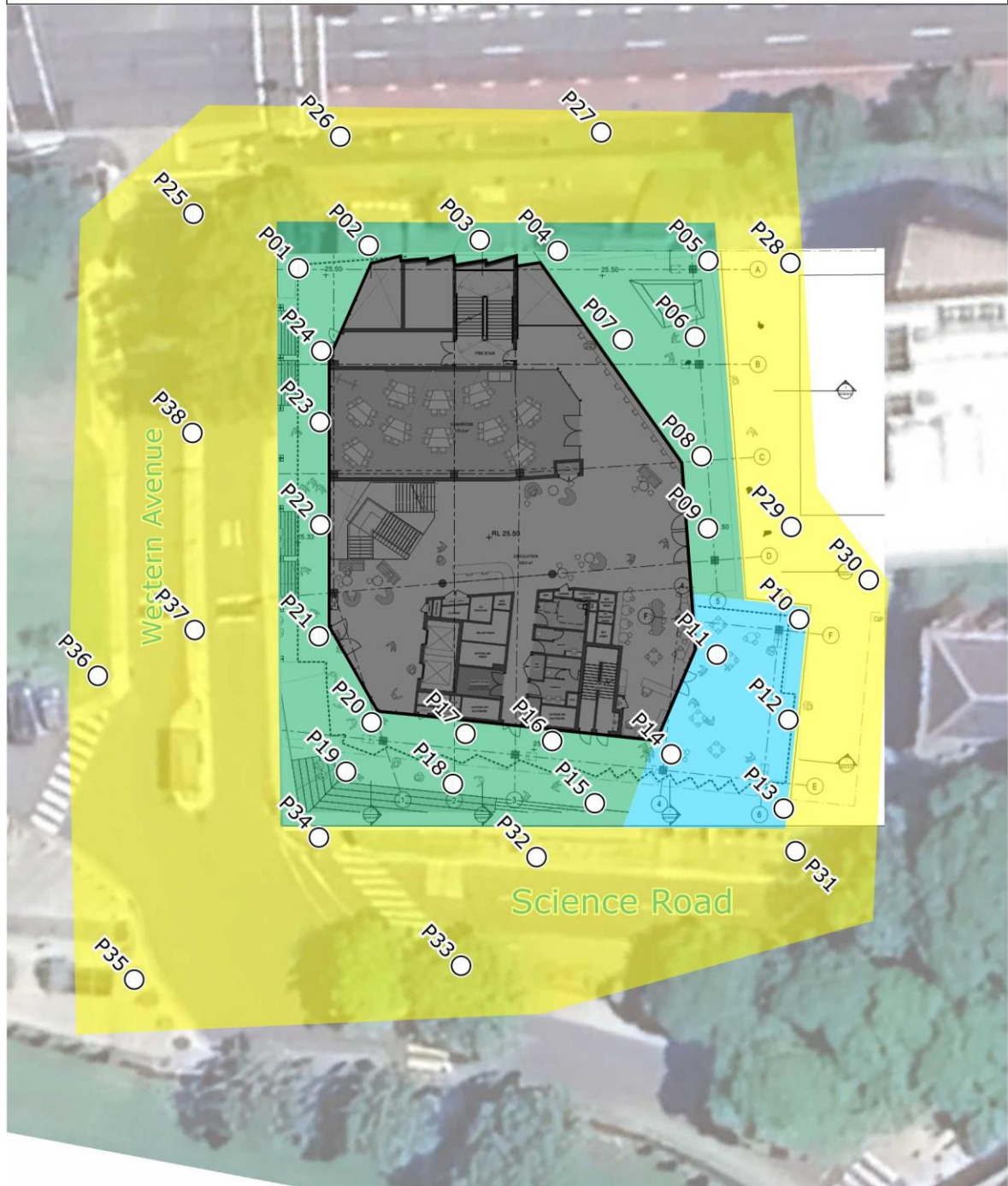


Figure 5: Study Point Locations and Target Wind Speed Criteria – Level 2 (Ground)

6 RESULTS AND DISCUSSION

The results of the wind tunnel study are presented in the form of directional plots in Appendix C for all study points locations, summarised in Table 5, and shown on marked-up plans in Figure 6. The velocity coefficients are also presented in the form of directional plots in Appendix D. The wind speed criteria that the wind conditions should achieve are also listed in Table 5 for each study point location, as well as in Figure 5.

The results of the study indicate that wind conditions for the majority of trafficable outdoor locations within and around the development will be suitable for their intended uses. However, some areas will experience strong winds which will exceed the relevant criteria for comfort and/or safety. These areas have been discussed with the design team and a detailed treatment scheme will be developed in conjunction with Windtech during the Design Development stage. This scheme may result in the inclusion of features such as strategic landscaping, screens, and/or awnings. Throughout the Design Development stage, wind tunnel testing will be undertaken to determine the extent/effectiveness of the resulting treatment scheme.

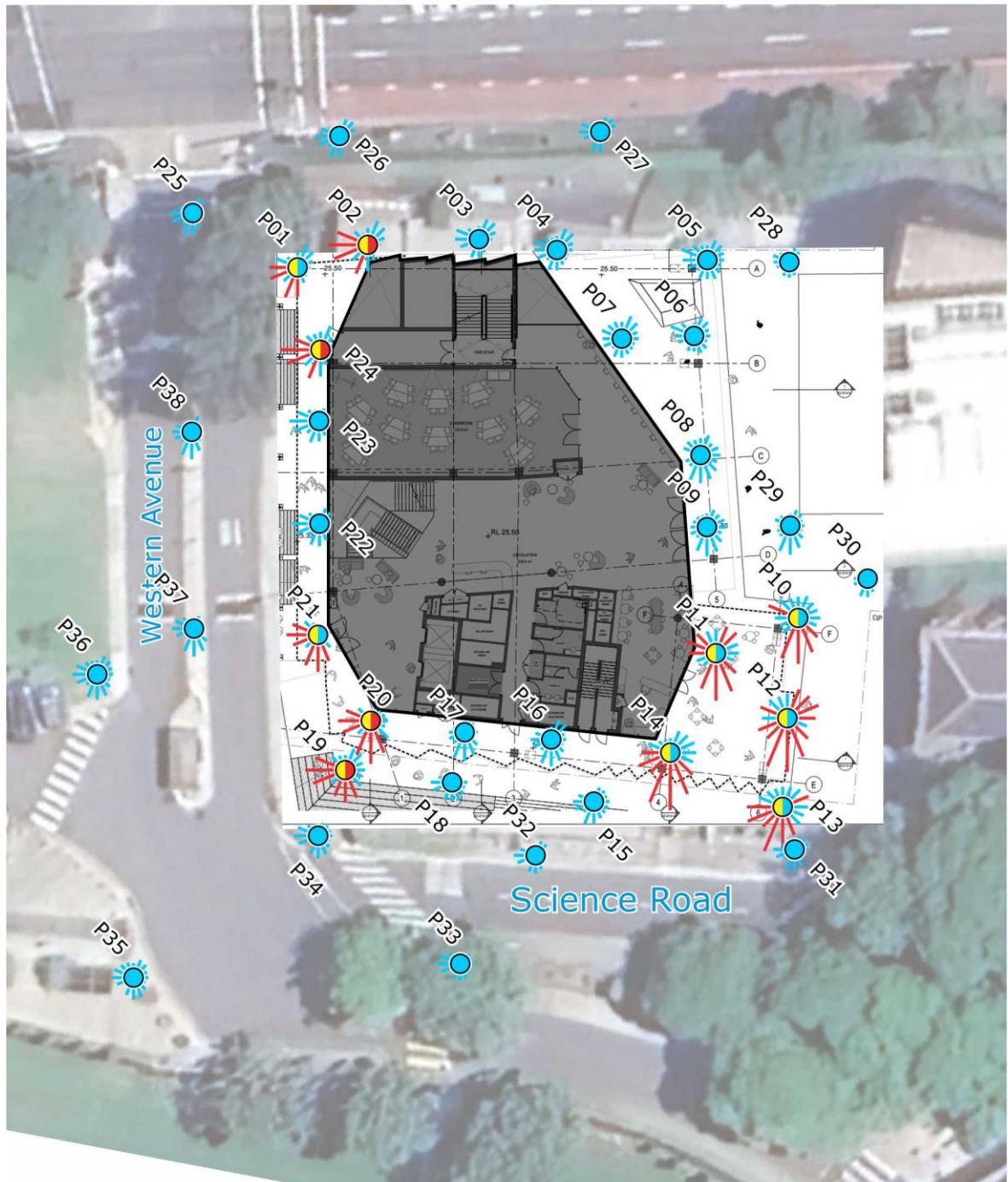
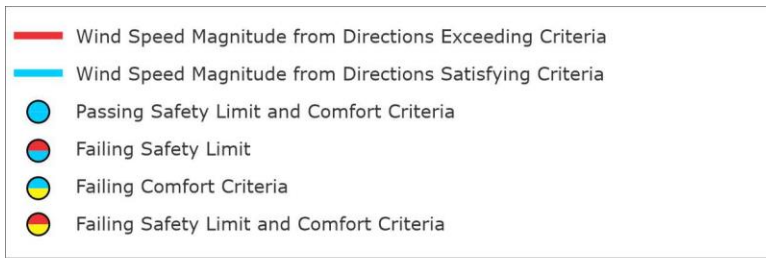


Figure 6: Wind Tunnel Results – Level 2 (Ground)
(results shown without treatments applied)

Table 5: Wind Tunnel Results Summary

Study Point	GEM (5% exceedance)			Annual Gust			Final Result	Description of Treatment
	Criterion (m/s)	Results (m/s)	Grade	Criterion (m/s)	Results (m/s)	Grade		
P01	6.0	6.7	Fail	24	20	Pass	Fail	To be determined in Design Development stage.
P02	6.0	8.0	Fail	24	27	Fail	Fail	To be determined in Design Development stage.
P03	6.0	5.1	Pass	24	18	Pass	Pass	-
P04	6.0	5.3	Pass	24	17	Pass	Pass	-
P05	6.0	5.5	Pass	24	20	Pass	Pass	-
P06	6.0	4.7	Pass	24	18	Pass	Pass	-
P07	6.0	5.2	Pass	24	15	Pass	Pass	-
P08	6.0	6.0	Pass	24	17	Pass	Pass	-
P09	6.0	5.9	Pass	24	18	Pass	Pass	-
P10	4.0	6.3	Fail	24	19	Pass	Fail	To be determined in Design Development stage.
P11	4.0	6.4	Fail	24	20	Pass	Fail	To be determined in Design Development stage.
P12	4.0	7.3	Fail	24	22	Pass	Fail	To be determined in Design Development stage.
P13	4.0	6.4	Fail	24	21	Pass	Fail	To be determined in Design Development stage.
P14	4.0	7.7	Fail	24	24	Pass	Fail	To be determined in Design Development stage.
P15	6.0	4.9	Pass	24	15	Pass	Pass	-
P16	6.0	5.6	Pass	24	16	Pass	Pass	-
P17	6.0	5.7	Pass	24	17	Pass	Pass	-
P18	6.0	4.5	Pass	24	14	Pass	Pass	-
P19	6.0	8.0	Fail	24	26	Fail	Fail	To be determined in Design Development stage.
P20	6.0	8.6	Fail	24	27	Fail	Fail	To be determined in Design Development stage.
P21	6.0	7.6	Fail	24	23	Pass	Fail	To be determined in Design Development stage.
P22	6.0	5.5	Pass	24	18	Pass	Pass	-
P23	6.0	5.8	Pass	24	20	Pass	Pass	-
P24	6.0	7.8	Fail	24	26	Fail	Fail	To be determined in Design Development stage.
P25	8.0	6.6	Pass	24	19	Pass	Pass	-
Existing		5.6	Pass		16	Pass	Pass	
P26	8.0	6.7	Pass	24	20	Pass	Pass	-
Existing		6.6	Pass		18	Pass	Pass	

Study Point	GEM (5% exceedance)			Annual Gust			Final Result	Description of Treatment
	Criterion (m/s)	Results (m/s)	Grade	Criterion (m/s)	Results (m/s)	Grade		
P27	8.0	6.1	Pass	24	19	Pass	Pass	-
Existing		6.8	Pass		21	Pass	Pass	
P28	8.0	4.8	Pass	24	16	Pass	Pass	-
Existing		7.0	Pass		20	Pass	Pass	
P29	8.0	7.6	Pass	24	23	Pass	Pass	-
Existing		6.9	Pass		21	Pass	Pass	
P30	8.0	4.9	Pass	24	15	Pass	Pass	-
Existing		5.0	Pass		15	Pass	Pass	
P31	8.0	6.4	Pass	24	19	Pass	Pass	-
Existing		5.8	Pass		16	Pass	Pass	
P32	8.0	5.7	Pass	24	19	Pass	Pass	-
Existing		4.9	Pass		15	Pass	Pass	
P33	8.0	6.1	Pass	24	20	Pass	Pass	-
Existing		5.9	Pass		18	Pass	Pass	
P34	8.0	6.8	Pass	24	21	Pass	Pass	-
Existing		6.3	Pass		18	Pass	Pass	
P35	8.0	6.8	Pass	24	20	Pass	Pass	-
Existing		6.7	Pass		19	Pass	Pass	
P36	8.0	7.6	Pass	24	19	Pass	Pass	-
Existing		7.2	Pass		19	Pass	Pass	
P37	8.0	7.5	Pass	24	23	Pass	Pass	-
Existing		6.6	Pass		20	Pass	Pass	
P38	8.0	7.1	Pass	24	22	Pass	Pass	-
Existing		6.0	Pass		18	Pass	Pass	

Note that, for any study points listed in Table 5 with two rows of results data, the second row is for the existing site conditions. The test results shown in Table 5 are without any treatments applied.

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Melbourne, W.H., 1978, "Criteria for Environmental Wind Conditions". Journal of Wind Engineering and Industrial Aerodynamics, vol. 3, pp241-249.

Rofail, A.W., and Kwok, K.C.S., 1991, "A Reliability Study of Wind Tunnel Results of Cladding Pressures". Proceedings of the 8th International Conference on Wind Engineering, Canada.

Rofail, A.W., 2007, "Comparison of Wind Environment Criteria against Field Observations". 12th International Conference of Wind Engineering, Cairns, Australia.

Standards Australia and Standards New Zealand, AS/NZS 1170.2, 2011, "SAA Wind Loading Standard, Part 2: Wind Actions".

APPENDIX A PUBLISHED ENVIRONMENTAL CRITERIA

A.1 Wind Effects on People

The acceptability of wind in an area is dependent upon the use of the area. For example, people walking or window-shopping will tolerate higher wind speeds than those seated at an outdoor restaurant. Quantifying wind comfort has been the subject of much research and many researchers, such as A.G. Davenport, T.V. Lawson, W.H. Melbourne, and A.D. Penwarden, have published criteria for pedestrian comfort for pedestrians in outdoor spaces for various types of activities. This section discusses and compares the various published criteria.

A.2 A.D. Penwarden (1973) Criteria for Mean Wind Speeds

A.D. Penwarden (1973) developed a modified version of the Beaufort scale which describes the effects of various wind intensities on people. Table A.1 presents the modified Beaufort scale. Note that the effects listed in this table refers to wind conditions occurring frequently over the averaging time (a probability of occurrence exceeding 5%). Higher ranges of wind speeds can be tolerated for rarer events.

Table A.1: Summary of Wind Effects on People (A.D. Penwarden, 1973)

Type of Winds	Beaufort Number	Hourly Mean Wind Speed (m/s)	Effects
Calm	0	0 - 0.3	
Calm, light air	1	0.3 - 1.6	No noticeable wind
Light breeze	2	1.6 - 3.4	Wind felt on face
Gentle breeze	3	3.4 - 5.5	Hair is disturbed, clothing flaps, newspapers difficult to read
Moderate breeze	4	5.5 - 8.0	Raises dust, dry soil and loose paper, hair disarranged
Fresh breeze	5	8.0 - 10.8	Force of wind felt on body, danger of stumbling
Strong breeze	6	10.8 - 13.9	Umbrellas used with difficulty, hair blown straight, difficult to walk steadily, wind noise on ears unpleasant
Near gale	7	13.9 - 17.2	Inconvenience felt when walking
Gale	8	17.2 - 20.8	Generally impedes progress, difficulty balancing in gusts
Strong gale	9	20.8 - 24.5	People blown over

A.3 A.G. Davenport (1972) Criteria for Mean Wind Speeds

A.G. Davenport (1972) also determined a set of criteria in terms of the Beaufort scale and for various return periods. Table A.2 presents a summary of the criteria based on a probability of exceedance of 5%.

Table A.2: Criteria by A.G. Davenport (1972)

Classification	Activities	5% exceedance Mean Wind Speed (m/s)
Walking Fast	Acceptable for walking, main public accessways.	7.5 - 10.0
Strolling, Skating	Slow walking, etc.	5.5 - 7.5
Short Exposure Activities	Generally acceptable for walking & short duration stationary activities such as window-shopping, standing or sitting in plazas.	3.5 - 5.5
Long Exposure Activities	Generally acceptable for long duration stationary activities such as in outdoor restaurants & theatres and in parks.	0 - 3.5

A.4 T.V. Lawson (1975) Criteria for Mean Wind Speeds

In 1973, T.V. Lawson, while referring to the Beaufort wind speeds of A.D. Penwarden (1973) (as listed in Table A.1), quoted that a Beaufort 4 wind speed would be acceptable if it is not exceeded for more than 4% of the time, and that a Beaufort 6 wind speed would be unacceptable if it is exceeded more than 2% of the time. Later, in 1975, T.V. Lawson presented a set of criteria very similar to those presented in A.G. Davenport (1972) (as listed in Table A.2). These criteria are presented in Table A.3 and Table A.4 for safety and comfort respectively.

Table A.3: Safety Criteria by T.V. Lawson (1975)

Classification	Activities	Annual Mean Wind Speed (m/s)
Safety (all weather areas)	Accessible by the general public.	0 – 15
Safety (fair weather areas)	Private areas, balconies/terraces, etc.	0 – 20

Table A.4: Comfort Criteria by T.V. Lawson (1975)

Classification	Activities	5% exceedance Mean Wind Speed (m/s)
Business Walking	Objective Walking from A to B.	8 - 10
Pedestrian Walking	Slow walking, etc.	6 - 8
Short Exposure Activities	Pedestrian standing or sitting for short times.	4 – 6
Long Exposure Activities	Pedestrian sitting for a long duration.	0 - 4

A.5 W.H. Melbourne (1978) Criteria for Gust Wind Speeds

W.H. Melbourne (1978) introduced a set of criteria for the assessment of environmental wind conditions that were developed for a temperature range of 10°C to 30°C and for people suitably dressed for outdoor conditions. These criteria are presented in Table A.5, and are based on maximum gust wind speeds with a probability of exceedance of once per year.

Table A.5: Criteria by W.H. Melbourne (1978)

Classification	Activities	Annual Gust Wind Speed (m/s)
Limit for Safety	Completely unacceptable: people likely to get blown over.	23
Marginal	Unacceptable as main public accessways.	16 - 23
Comfortable Walking	Acceptable for walking, main public accessways	13 - 16
Short Exposure Activities	Generally acceptable for walking & short duration stationary activities such as window-shopping, standing or sitting in plazas.	10 - 13
Long Exposure Activities	Generally acceptable for long duration stationary activities such as in outdoor restaurants & theatres and in parks.	0 - 10

A.6 Comparison of the Published Wind Speed Criteria

W.H. Melbourne (1978) presented a comparison of the criteria of various researchers on a probabilistic basis. Figure A.1 presents the results of this comparison, and indicates that the criteria of W.H. Melbourne (1978) are comparatively quite conservative. This conclusion was also observed by A.W. Rofail (2007) when undertaking on-site remedial studies. The results of A.W. Rofail (2007) concluded that the criteria by W.H. Melbourne (1978) generally overstates the wind effects in a typical urban setting due to the assumption of a fixed 15% turbulence intensity for all areas. It was observed in A.W. Rofail (2007) that the 15% turbulence intensity assumption is not real and that the turbulence intensities at 1.5m above ground is at least 20% and in a suburban or urban setting is generally in the range of 30% to 60%.

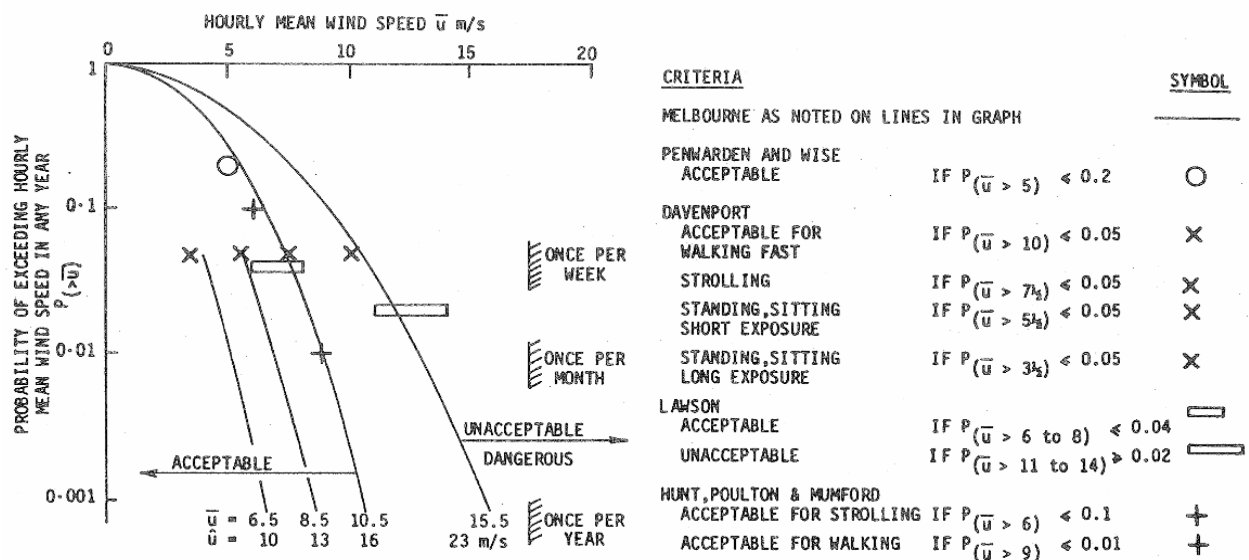


Figure A.1: Comparison of Various Mean and Gust Wind Environment Criteria, assuming 15% turbulence and a Gust Factor of 1.5 (W.H. Melbourne, 1978)

A.7 References relating to Pedestrian Comfort Criteria

Davenport, A.G., 1972, "An approach to human comfort criteria for environmental conditions". Colloquium on Building Climatology, Stockholm.

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Lawson, T.V., 1973, "The wind environment of buildings: a logical approach to the establishment of criteria". Bristol University, Department of Aeronautical Engineering.

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Melbourne, W.H., 1978, "Criteria for Environmental Wind Conditions". Journal of Wind Engineering and Industrial Aerodynamics, vol. 3, pp241-249.

Penwarden, A.D. (1973). "Acceptable Wind Speeds in Towns", Building Science, vol. 8: pp259–267.

Penwarden, A.D., Wise A.F.E., 1975, "Wind Environment Around Buildings". Building Research Establishment Report, London.

Rofail, A.W., 2007, "Comparison of Wind Environment Criteria against Field Observations". 12th International Conference of Wind Engineering, Cairns, Australia.

APPENDIX B DATA ACQUISITION

The wind tunnel testing procedures utilised for this study were based on the guidelines set out in the Australasian Wind Engineering Society Quality Assurance Manual (AWES-QAM-1-2019), ASCE 7-16 (Chapter C31), and CTBUH (2013). The wind speed measurements for the wind tunnel study were determined as coefficients using data acquired by either Dantec hot-wire probe anemometers or pressure-based wind speed sensors and converted to full-scale wind speeds using details of the regional wind climate obtained from an analysis of directional wind speed recordings from the local meteorological recording station(s).

B.1 Measurement of the Velocity Coefficients

The study model and proximity model were setup within the wind tunnel which was configured to the appropriate boundary layer profile, and the wind velocity measurements were monitored using either Dantec hot-wire probe anemometers or pressure-based wind speed sensors at selected critical outdoor locations. The wind velocity results presented in this study for each study point are representative of wind at a full-scale height of approximately 1.5m above ground/slab level. In the case of the Dantec hot-wire probe anemometers, the support of the probe is mounted such that the probe wire is vertical as much as possible to ensure that the measured wind speeds are independent of wind direction along the horizontal plane. In addition, care was taken in the alignment of the hot-wire probe wire and in avoiding wall-heating effects.

Wind speed measurements were made in the wind tunnel for the wind directions described within this report. Data was acquired for each wind direction using a sample rate of 1024Hz. The sample length was determined to produce a full-scale sample time that is sufficient for this type of study. In the case of the pressure-based wind speed sensors, the phase lag between the various channels where data is acquired simultaneously is within 10% of a typical pressure cycle, and the signal is low-pass filtered at 500Hz and then digital filtering is applied over this range to provide an unbiased response from the pressure measurement system (A.W. Rofail, 2004).

The mean, gust and standard deviation velocity coefficients were determined from the data acquired in the wind tunnel. The gust velocity coefficients were also derived for each wind direction from by the following relation:

$$\hat{C}_V = \bar{C}_V + g \cdot \sigma_{C_V} \quad \text{B.1}$$

where:

$\hat{C}_V$ is the gust velocity coefficient.

$\bar{C}_V$ is the mean velocity coefficient.

g is the peak factor, taken as 3.0 for a 3-sec gust and 3.4 for a 0.5-sec gust.

σ_{C_V} is the standard deviation of the velocity coefficient measurement.

In the case of a Dantec hot-wire probe anemometer, the velocity coefficient is obtained as follows:

$$C_V = \frac{C_{V,study}}{C_{V,200m}} \quad \text{B.2}$$

where:

$C_{V,study}$ is the velocity coefficient measurement obtained from the Dantec hot-wire probe anemometer at the study point location.

$C_{V,200m}$ is the velocity coefficient measurement obtained from the Dantec hot-wire probe anemometer at the free-stream reference location at 200m height upwind of the model in the wind tunnel.

However, in the case of the pressure-based wind speed sensors, these are determined from the measured differential mean, standard deviation and maximum pressure coefficients obtained from the wind speed sensor. For this analysis all calculations are performed on the square root of the differential pressure measurements. The velocity coefficient at the pressure-based wind speed sensor location is then calculated as follows:

$$C_V = \frac{\alpha + \beta\sqrt{\Delta p}}{V_{200m}} \quad \text{B.3}$$

where:

C_V is the velocity coefficient measurement at the study point location.

α is a calibration coefficient for the pressure-based wind speed sensor.

β is a calibration coefficient for the pressure-based wind speed sensor.

Δp is the differential pressure obtained from the pressure-based wind speed sensor at the study point location.

V_{200m} is the wind speed at the free-stream reference location of 200m height (full-scale) in the wind tunnel, which is determined directly in the wind tunnel using a pitot static probe.

B.2 Calculation of the Full-Scale Results

The full-scale results determine if the wind conditions at a study location satisfy the designated criteria of that location. More specifically, the full-scale results need to determine the probability of exceedance of a given wind speed at a study location. To determine the probability of exceedance, the measured velocity coefficients were combined with a statistical model of the local wind climate that relates wind speed to a probability of exceedance. Details of the wind climate model are outlined in Section 4 of the main report.

The statistical model of the wind climate includes the impact of wind directionality as any local variations in wind speed or frequency with wind direction. This is important as the wind directions that produce the highest wind speed events for a region may not coincide with the most wind exposed direction at the site.

The methodology adopted for the derivation of the full-scale results for the maximum gust and the GEM wind speeds are outlined in the following sub-sections.

B.3 Maximum Gust Wind Speeds

The full-scale maximum gust wind speed at each study point location is derived from the measured coefficient using the following relationship:

$$V_{study} = V_{ref,RH} \left(\frac{k_{200m,tr,T=1hr}}{k_{RH,tr,T=1hr}} \right) C_V \quad B.4$$

where:

V_{study} is the full-scale wind speed at the study point location.

$V_{ref,RH}$ is the full-scale reference wind speed at the study reference height. This value is determined by combining the directional wind speed data for the region (detailed in Section 4) and the upwind terrain and height multipliers for the site (detailed in Section 3).

$k_{200m,tr,T=1hr}$ is the hourly mean terrain and height multiplier at the free-stream reference location of 200m height.

$k_{RH,tr,T=1hr}$ is the hourly mean terrain and height multiplier at the study reference height (Section 3).

C_V is the velocity coefficient, obtained from either Equation B.2 (in the case of Dantec hot-wire probe anemometers) or Equation B.3 (in the case of pressure-based wind speed sensors).

The value of $V_{ref,RH}$ varies with each prevailing wind direction. Wind directions where there is a high probability that a strong wind will occur have a higher directional wind speed than other directions. To determine the directional wind speeds, a probability level must be assigned for each wind direction. These probability levels are set following the approach used in AS/NZS1170.2:2011, which assumes that the major contributions to the combined probability of exceedance of a typical load effect comes from only two 45 degree sectors.

B.4 Maximum Gust-Equivalent Mean Wind Speeds

The contribution to the probability of exceedance of a specified wind speed (ie: the desired wind speed for pedestrian comfort, as per the criteria) was calculated for each wind direction. These contributions are then combined over all wind directions to calculate the total probability of exceedance of the specified wind speed. To calculate the probability of exceedance for a specified wind speed a statistical wind climate model was used to describe the relationship between directional wind speeds and the probability of exceedance. A detailed description of the methodology is given by T.V. Lawson (1980).

The criteria used in this study is referenced to a probability of exceedance of 5% of a specified wind speed.

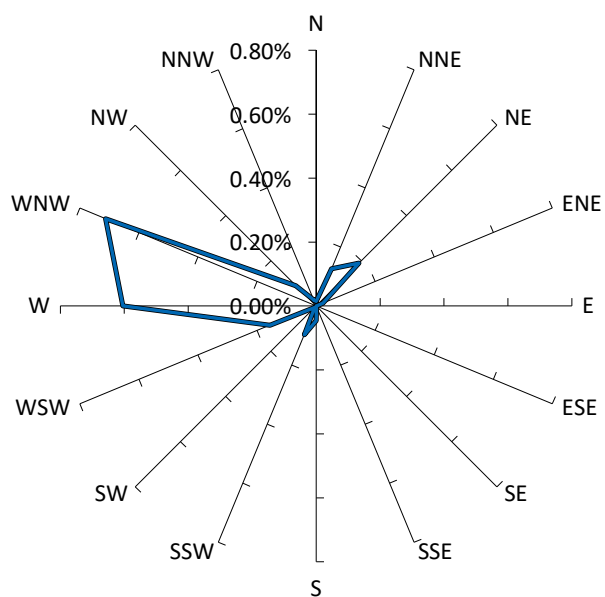
B.5 References relating to Data Acquisition

- American Society of Civil Engineers (ASCE), ASCE-7-16, 2016, "Minimum Design Loads for Buildings and Other Structures".
- Australasian Wind Engineering Society, QAM-1, 2019, "Quality Assurance Manual: Wind Engineering Studies of Buildings", edited by Rofail A.W., et al.
- Council on Tall Buildings and Urban Habitat (CTBUH), 2013, "Wind tunnel testing of high-rise buildings", CTBUH Technical Guides.
- Lawson, T.V., 1980, "Wind Effects on Buildings - Volume 1, Design Applications". Applied Science Publishers Ltd, Ripple Road, Barking, Essex, England.
- Rofail A.W., Tonin, R., and Hanafi, D., 2004, "Sensitivity of frequency response to type of tubing", Australasian Wind Engineering Workshop, Darwin.
- Standards Australia and Standards New Zealand, AS/NZS 1170.2, 2011, "SAA Wind Loading Standard, Part 2: Wind Actions".

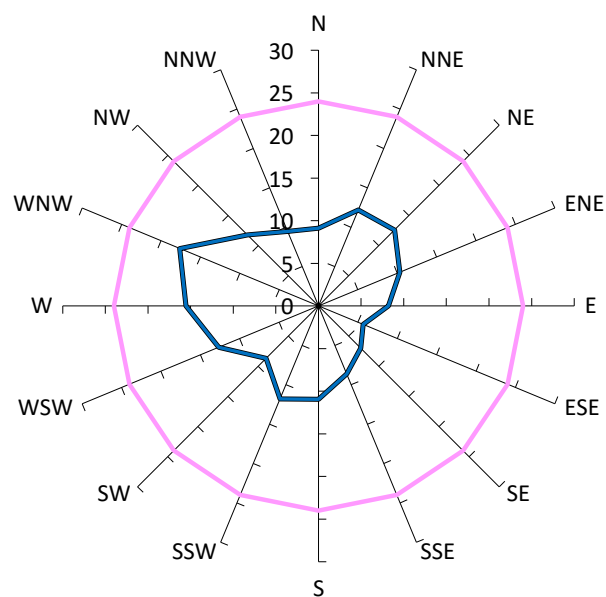
APPENDIX C DIRECTIONAL PLOTS OF WIND TUNNEL RESULTS



WI056-01- A01 Ross Street Teaching & Learning Hub, University of Sydney

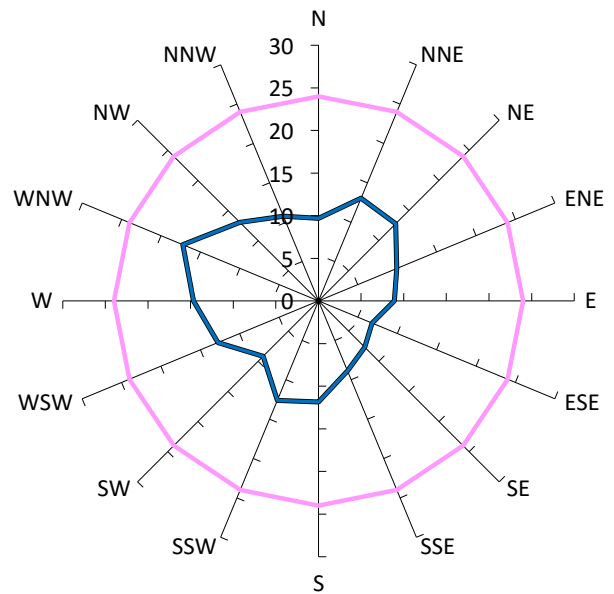
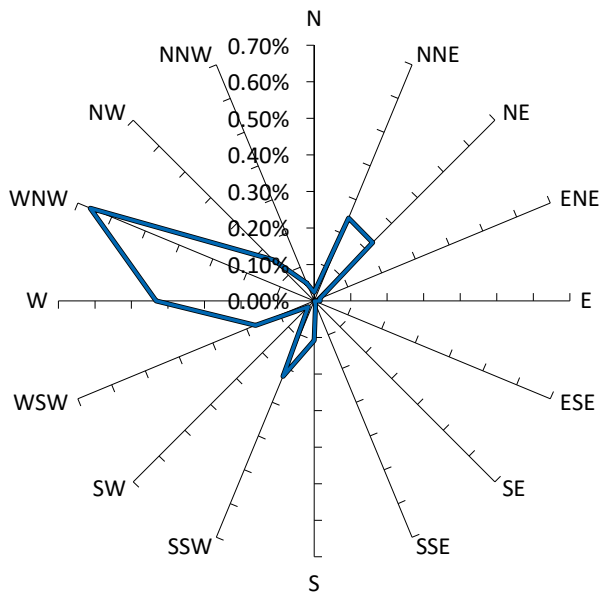


Annual probability of exceeding 6m/s (%)



Annual Maximum Gust (m/s)

Results for P04



Annual probability of exceeding 6m/s (%)

Annual Maximum Gust (m/s)

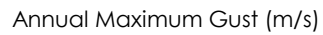
Description		GEM Wind Speed (m/s)	Peak Gust m/s
	Criterion: Wind Comfort Standard for Standing Criterion (6m/s). Safety Limit (23m/s).	6.0	24
	With development "as proposed", no vegetation or other treatments.	5.3	17
			
			
			
			
			
			
			



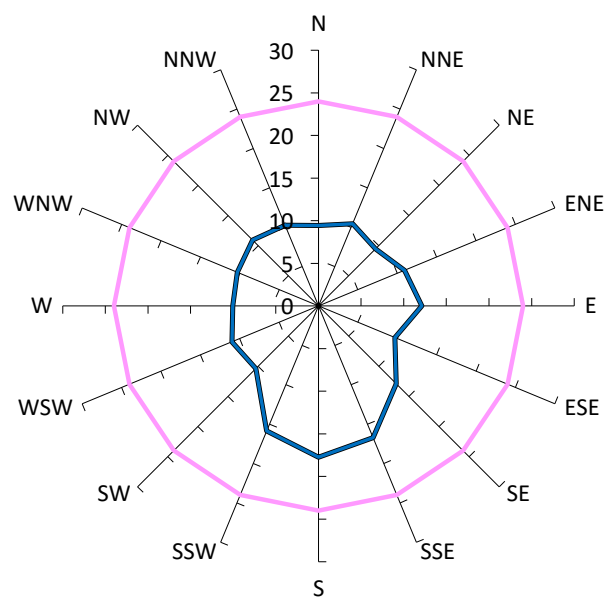
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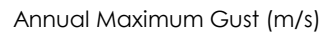


Annual Maximum Gust (m/s)

WI056-01- A01 Ross Street Teaching & Learning Hub, University of Sydney



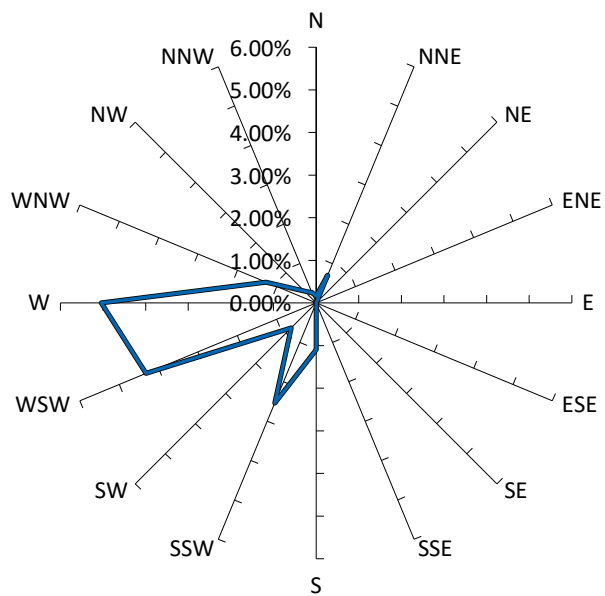
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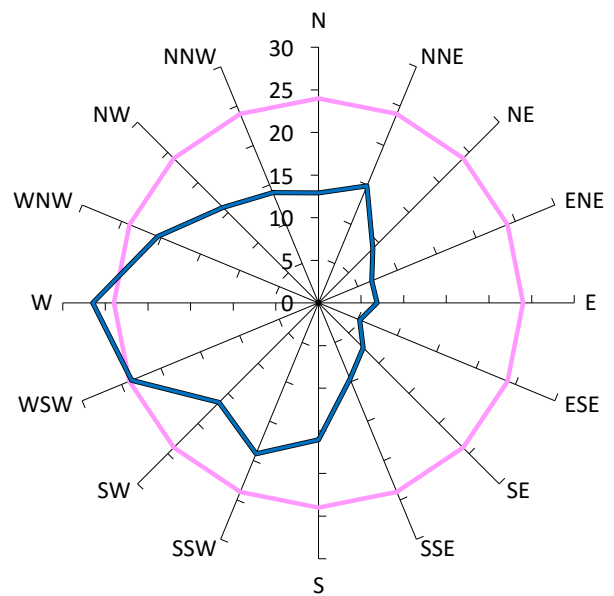
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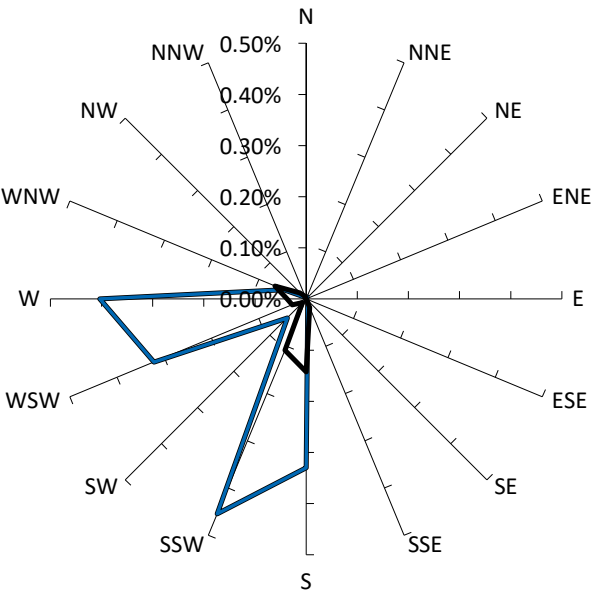


Annual probability of exceeding 6m/s (%)

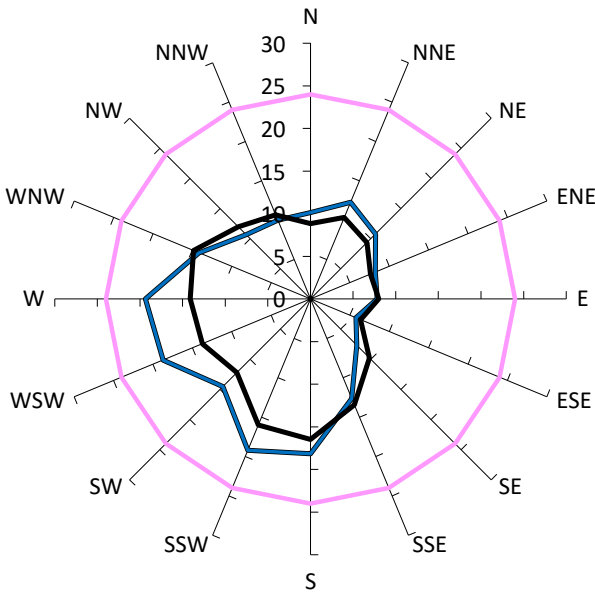


Annual Maximum Gust (m/s)

Results for P25



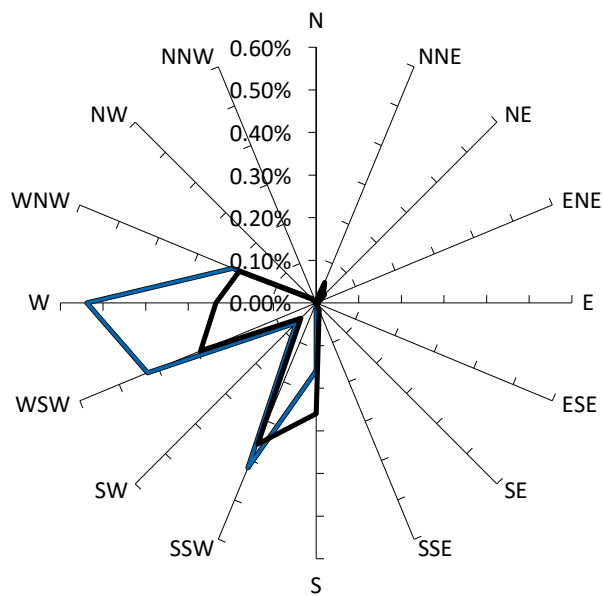
Annual probability of exceeding 8m/s (%)



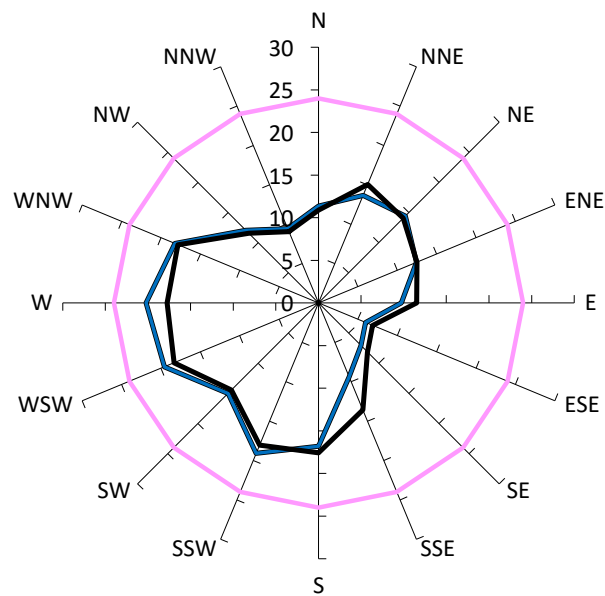
Annual Maximum Gust (m/s)

Description		GEM Wind Speed (m/s)	Peak Gust m/s
Criterion: Wind Comfort Standard for Walking Criterion (8m/s). Safety Limit (23m/s).		8.0	24
With development "as proposed", no vegetation or other treatments.		6.6	19
Existing Site Conditions.		5.6	16

Results for P26



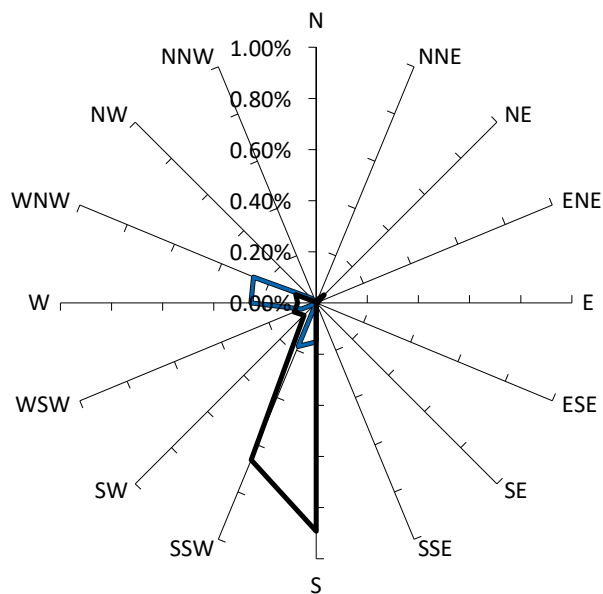
Annual probability of exceeding 8m/s (%)



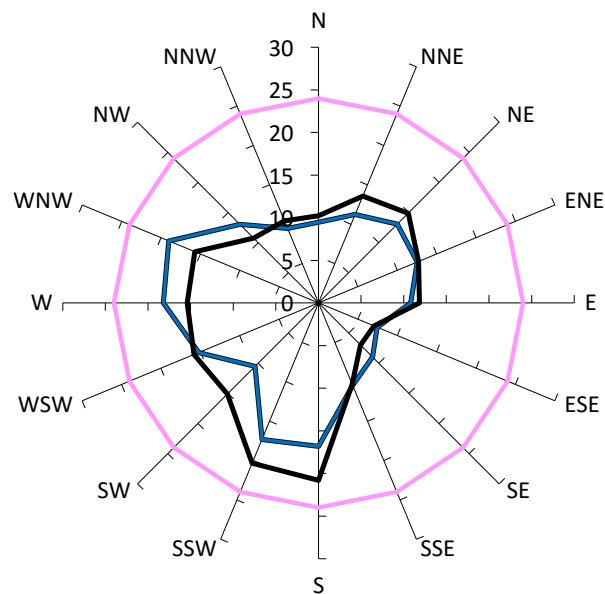
Annual Maximum Gust (m/s)

Description		GEM Wind Speed (m/s)	Peak Gust m/s
 Criterion: Wind Comfort Standard for Walking Criterion (8m/s). Safety Limit (23m/s).		8.0	24
 With development "as proposed", no vegetation or other treatments.		6.7	20
 Existing Site Conditions.		6.6	18
			
			
			
			
			
			

Results for P27



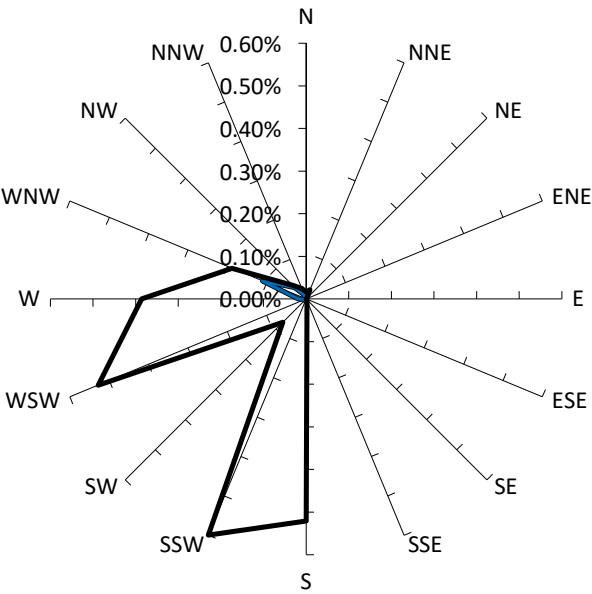
Annual probability of exceeding 8m/s (%)



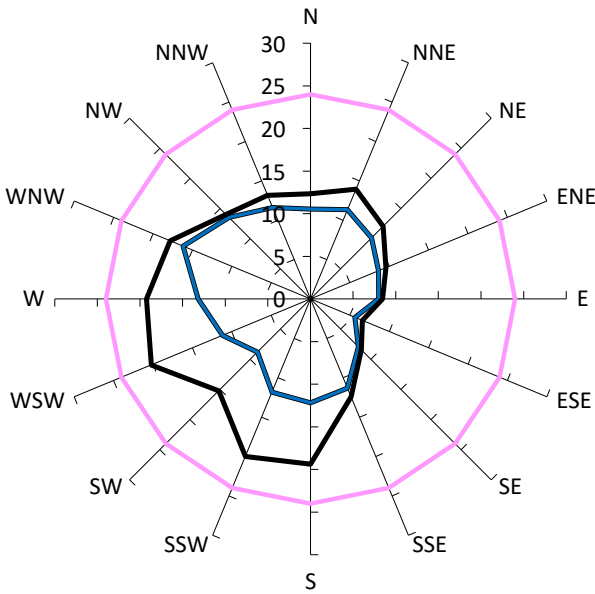
Annual Maximum Gust (m/s)

Description		GEM Wind Speed (m/s)	Peak Gust m/s
	Criterion: Wind Comfort Standard for Walking Criterion (8m/s). Safety Limit (23m/s).	8.0	24
	With development "as proposed", no vegetation or other treatments.	6.1	19
	Existing Site Conditions.	6.8	21
			
			
			
			
			
			

Results for P28



Annual probability of exceeding 8m/s (%)

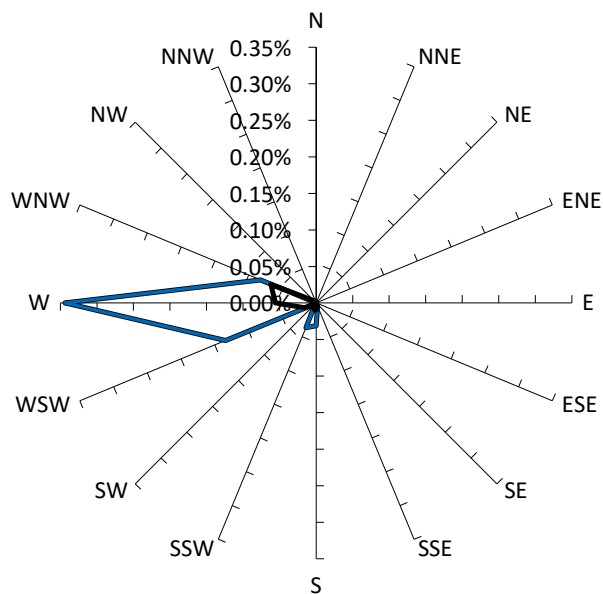


Annual Maximum Gust (m/s)

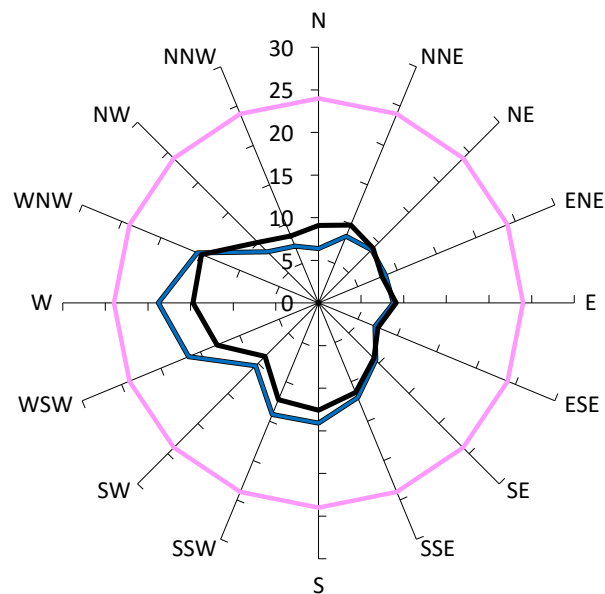
Description		GEM Wind Speed (m/s)	Peak Gust m/s
	Criterion: Wind Comfort Standard for Walking Criterion (8m/s). Safety Limit (23m/s).	8.0	24
	With development "as proposed", no vegetation or other treatments.	4.8	16
	Existing Site Conditions.	7.0	20



Results for P32



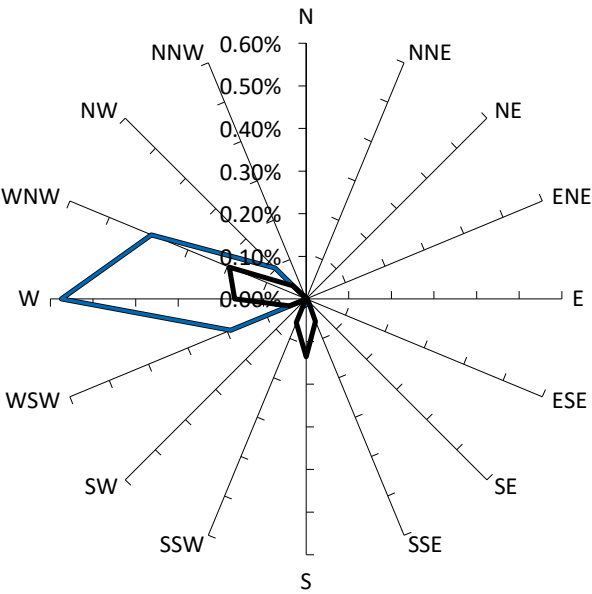
Annual probability of exceeding 8m/s (%)



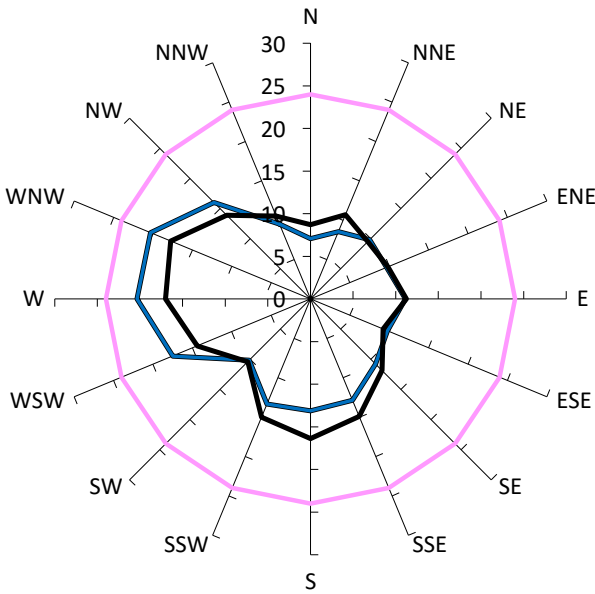
Annual Maximum Gust (m/s)

Description	GEM Wind Speed (m/s)	Peak Gust m/s
Criterion: Wind Comfort Standard for Walking Criterion (8m/s). Safety Limit (23m/s).	8.0	24
With development "as proposed", no vegetation or other treatments.	5.7	19
Existing Site Conditions.	4.9	15

Results for P33



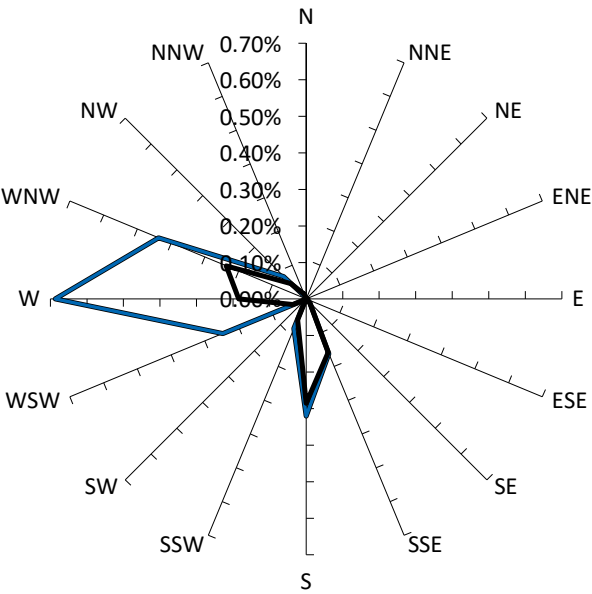
Annual probability of exceeding 8m/s (%)



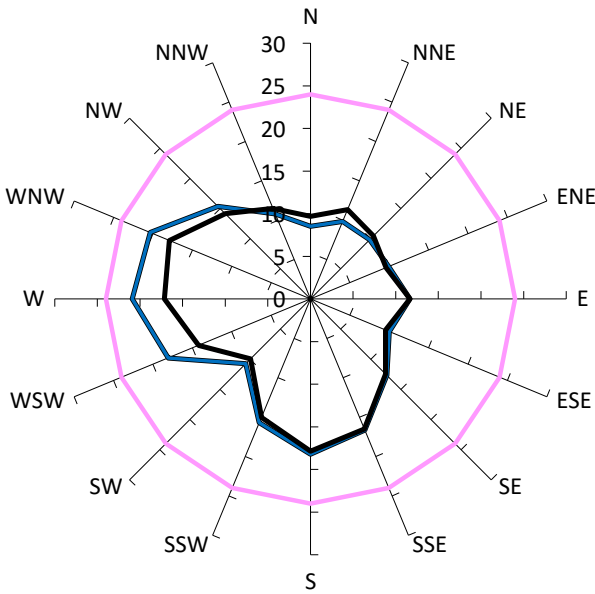
Annual Maximum Gust (m/s)

Description		GEM Wind Speed (m/s)	Peak Gust m/s
Criterion: Wind Comfort Standard for Walking Criterion (8m/s). Safety Limit (23m/s).		8.0	24
With development "as proposed", no vegetation or other treatments.		6.1	20
Existing Site Conditions.		5.9	18

Results for P34

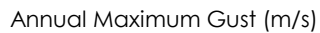


Annual probability of exceeding 8m/s (%)



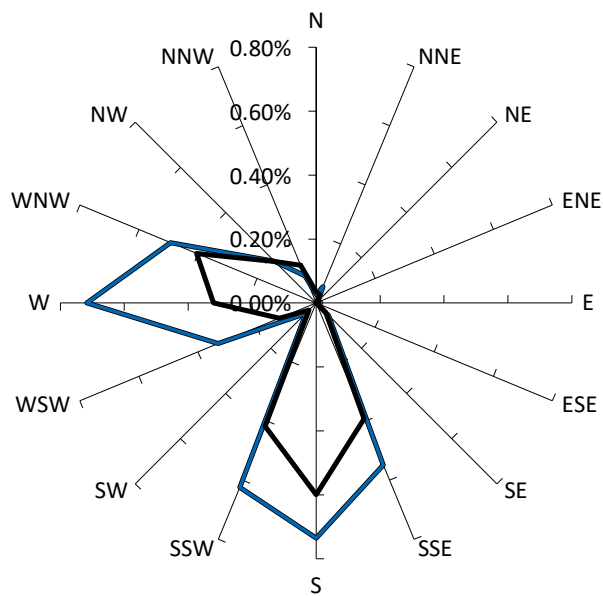
Annual Maximum Gust (m/s)

Description		GEM Wind Speed (m/s)	Peak Gust m/s
	Criterion: Wind Comfort Standard for Walking Criterion (8m/s). Safety Limit (23m/s).	8.0	24
	With development "as proposed", no vegetation or other treatments.	6.8	21
	Existing Site Conditions.	6.3	18

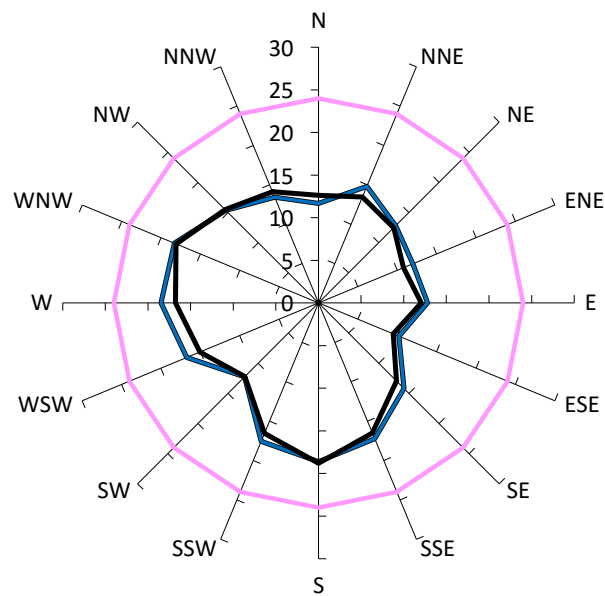


WI056-01- A01 Ross Street Teaching & Learning Hub, University of Sydney

Results for P36



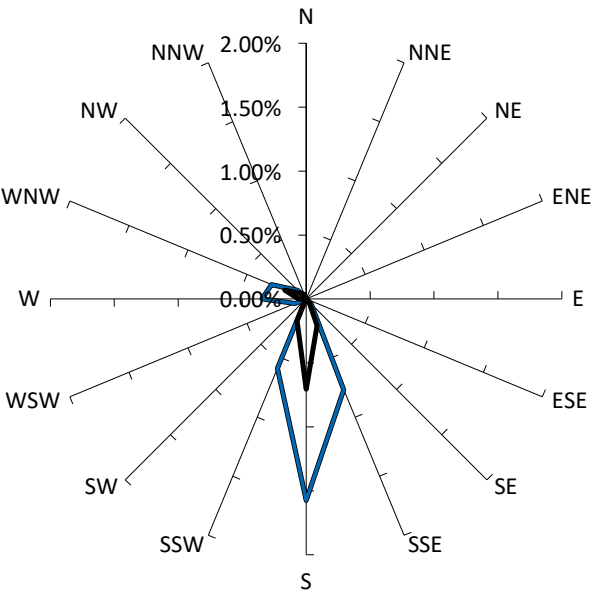
Annual probability of exceeding 8m/s (%)



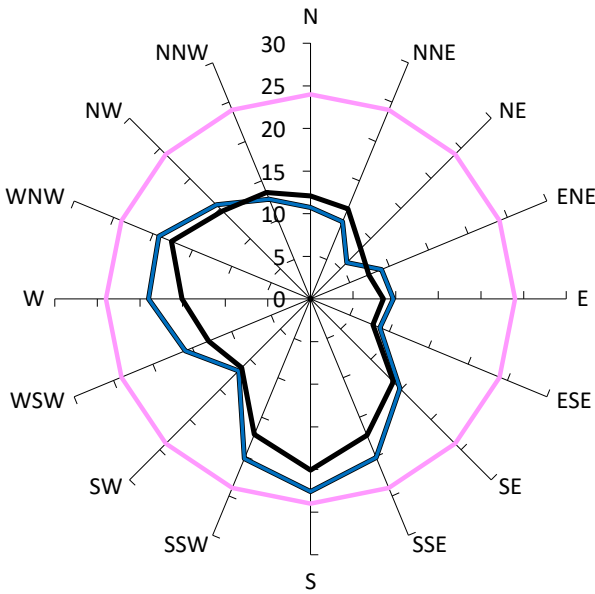
Annual Maximum Gust (m/s)

Description		GEM Wind Speed (m/s)	Peak Gust m/s
	Criterion: Wind Comfort Standard for Walking Criterion (8m/s). Safety Limit (23m/s).	8.0	24
	With development "as proposed", no vegetation or other treatments.	7.6	19
	Existing Site Conditions.	7.2	19
			
			
			
			
			
			

Results for P37



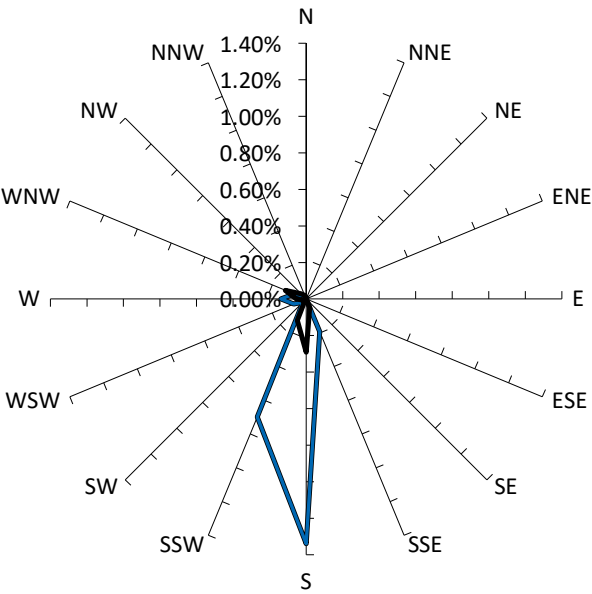
Annual probability of exceeding 8m/s (%)



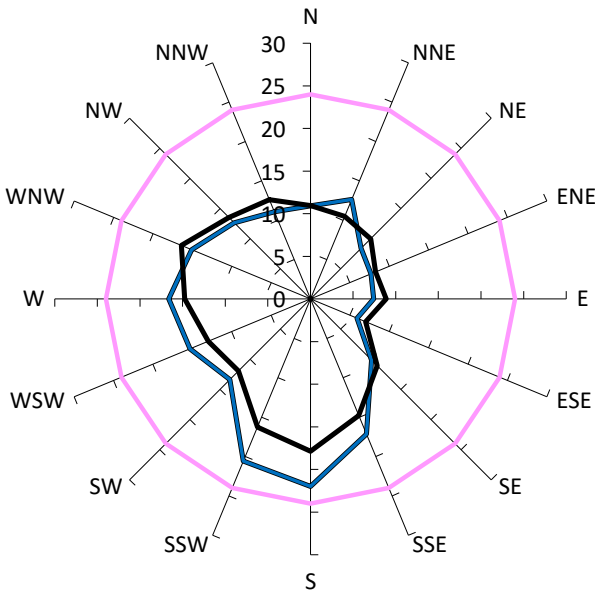
Annual Maximum Gust (m/s)

Description		GEM Wind Speed (m/s)	Peak Gust m/s
	Criterion: Wind Comfort Standard for Walking Criterion (8m/s). Safety Limit (23m/s).	8.0	24
	With development "as proposed", no vegetation or other treatments.	7.5	23
	Existing Site Conditions.	6.6	20

Results for P38



Annual probability of exceeding 8m/s (%)

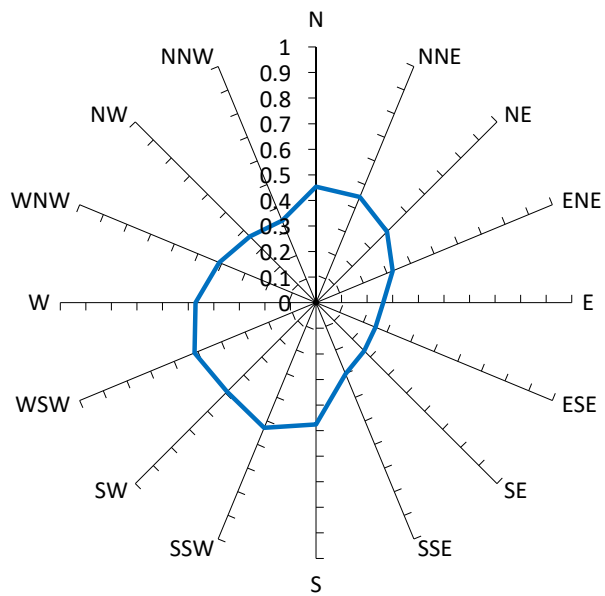


Annual Maximum Gust (m/s)

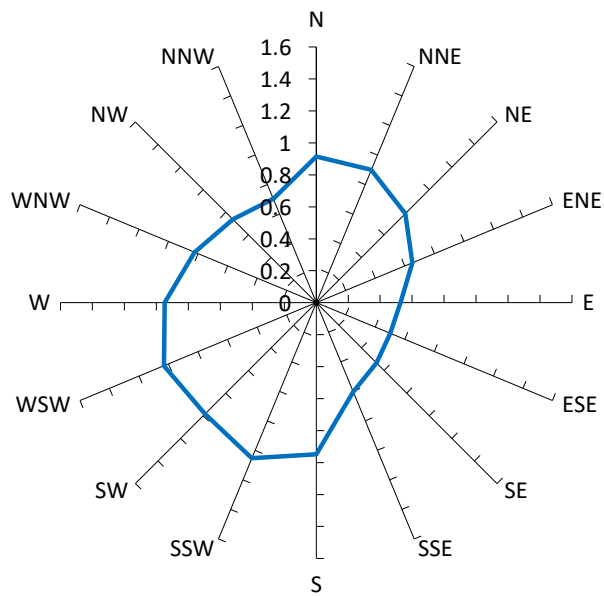
Description		GEM Wind Speed (m/s)	Peak Gust m/s
	Criterion: Wind Comfort Standard for Walking Criterion (8m/s). Safety Limit (23m/s).	8.0	24
	With development "as proposed", no vegetation or other treatments.	7.1	22
	Existing Site Conditions.	6.0	18

APPENDIX D DIRECTIONAL VELOCITY COEFFICIENT PLOTS

Results for P01



GEM Velocity Coefficients



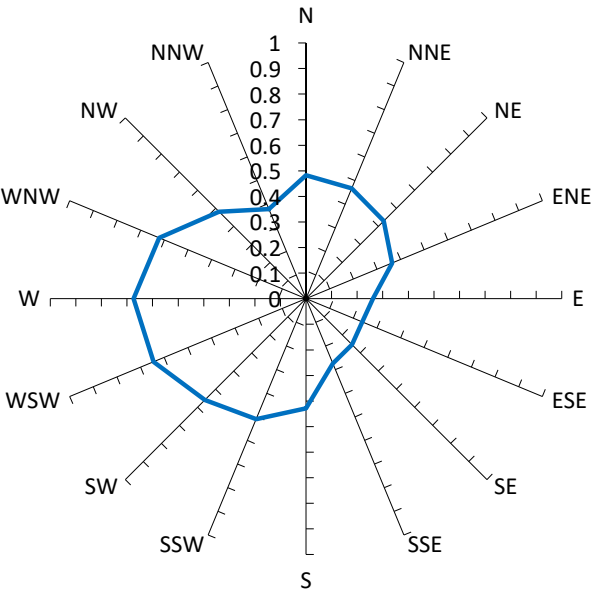
Gust Velocity Coefficients



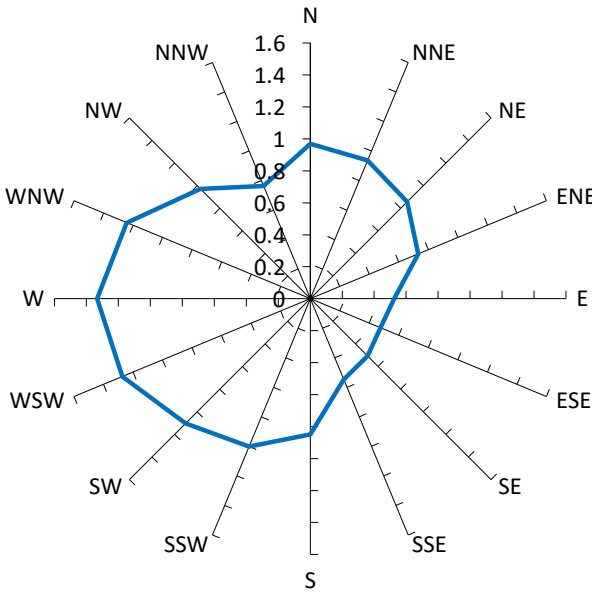
With development "as proposed", no vegetation or other treatments.



Results for P02



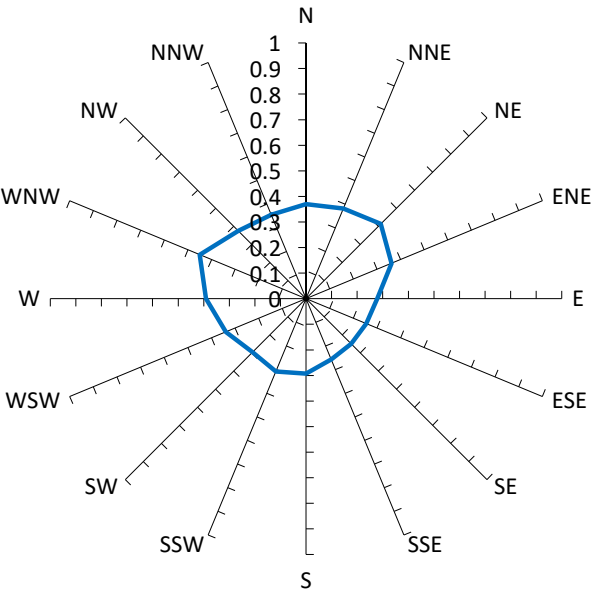
GEM Velocity Coefficients



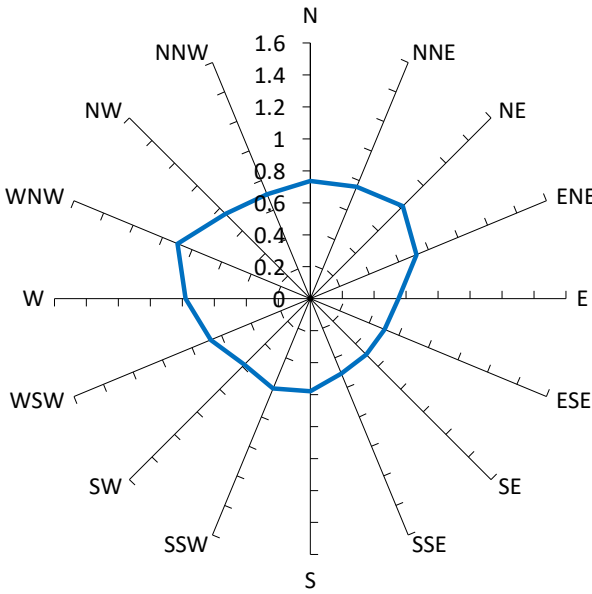
Gust Velocity Coefficients

With development "as proposed", no vegetation or other treatments.

Results for P03



GEM Velocity Coefficients



Gust Velocity Coefficients



With development "as proposed", no vegetation or other treatments.

Black line

Red line

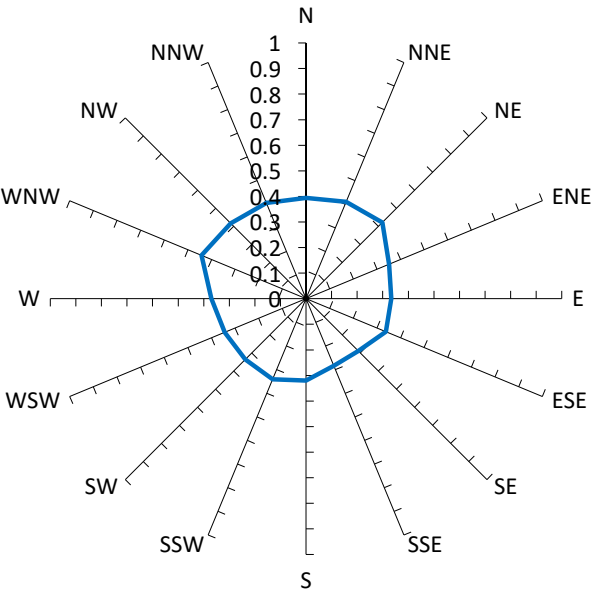
Yellow line

Green line

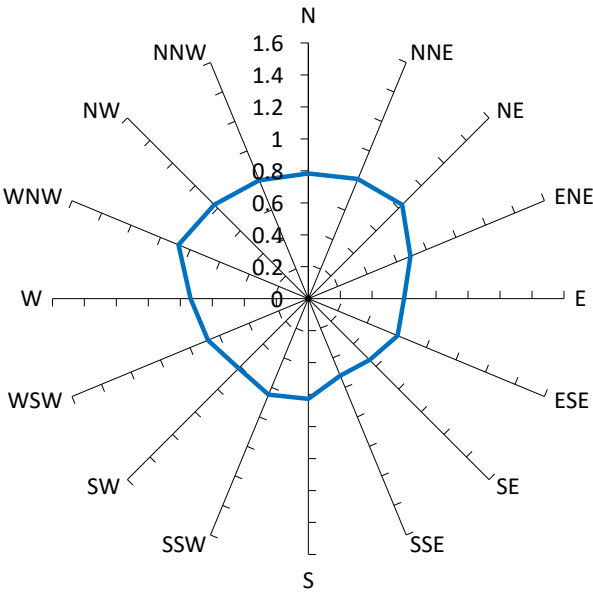
Blue line

Purple line

Results for P04



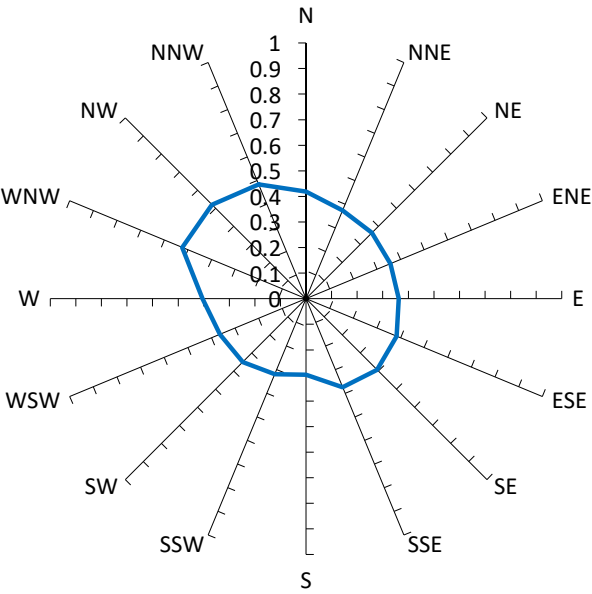
GEM Velocity Coefficients



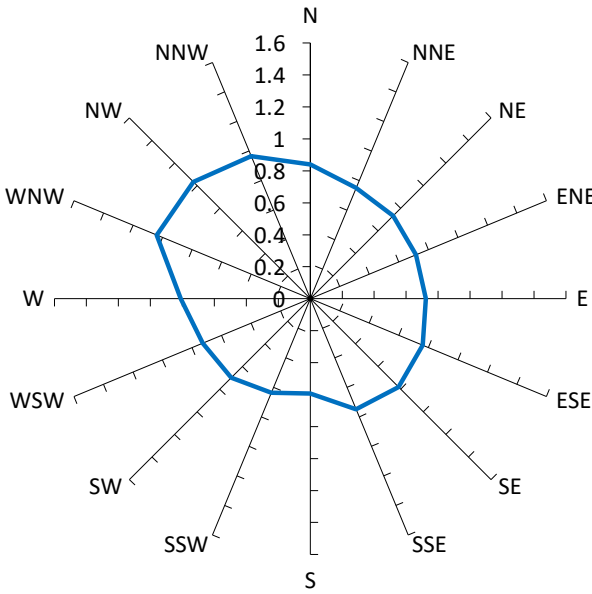
Gust Velocity Coefficients

With development "as proposed", no vegetation or other treatments.

Results for P05



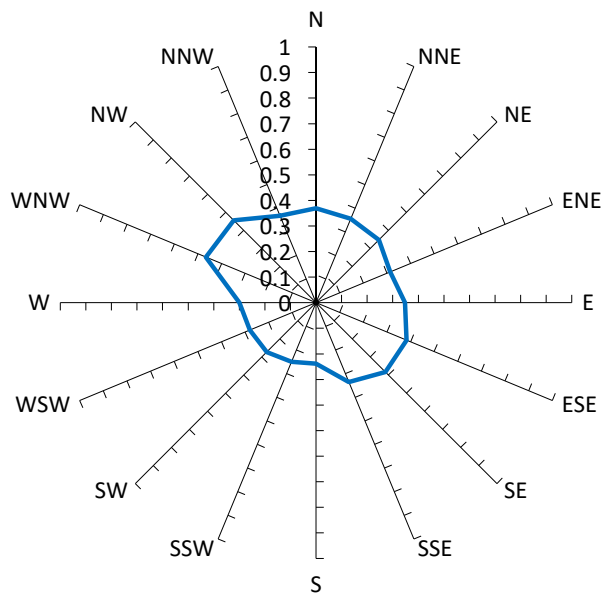
GEM Velocity Coefficients



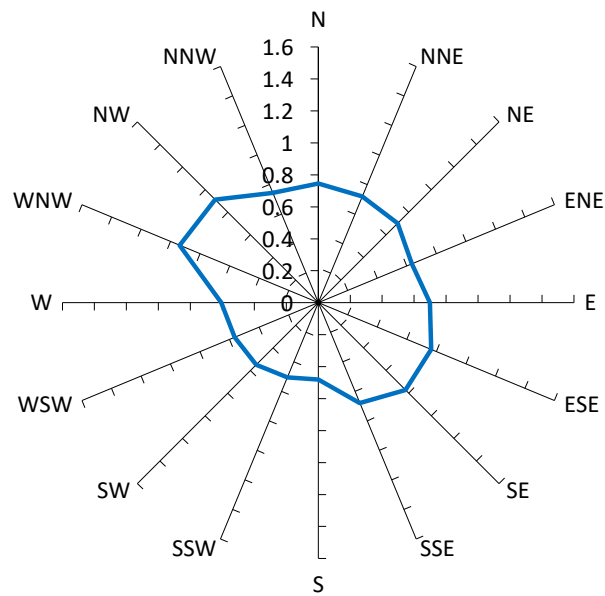
Gust Velocity Coefficients

With development "as proposed", no vegetation or other treatments.

Results for P06



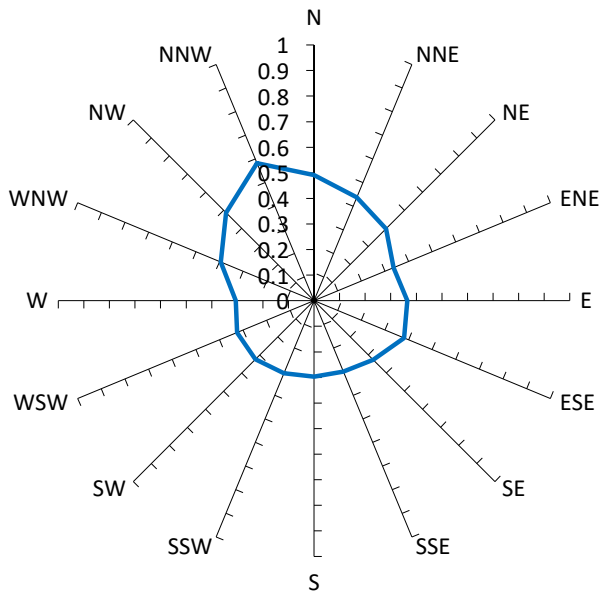
GEM Velocity Coefficients



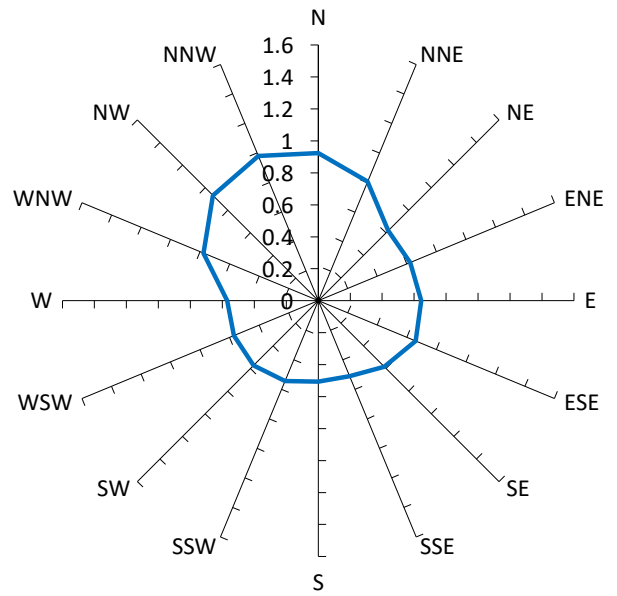
Gust Velocity Coefficients

With development "as proposed", no vegetation or other treatments.

Results for P07



GEM Velocity Coefficients



Gust Velocity Coefficients

— With development "as proposed", no vegetation or other treatments.

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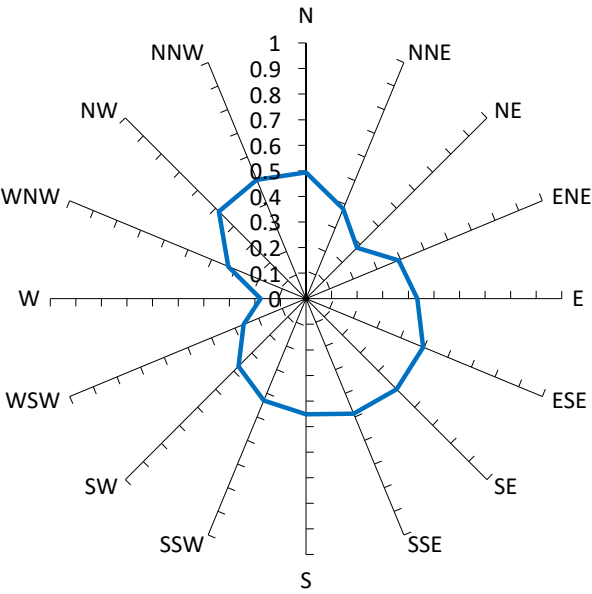
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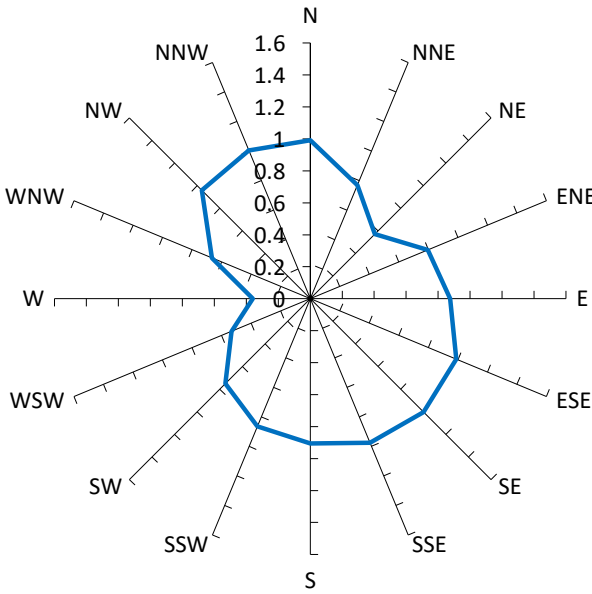
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Results for P08



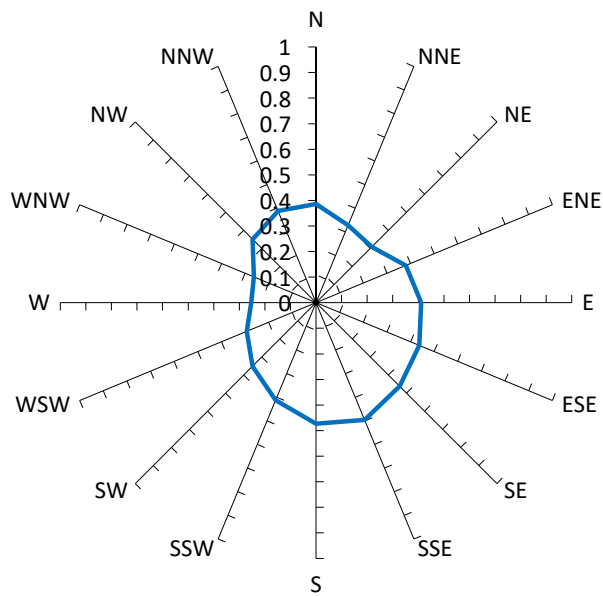
GEM Velocity Coefficients



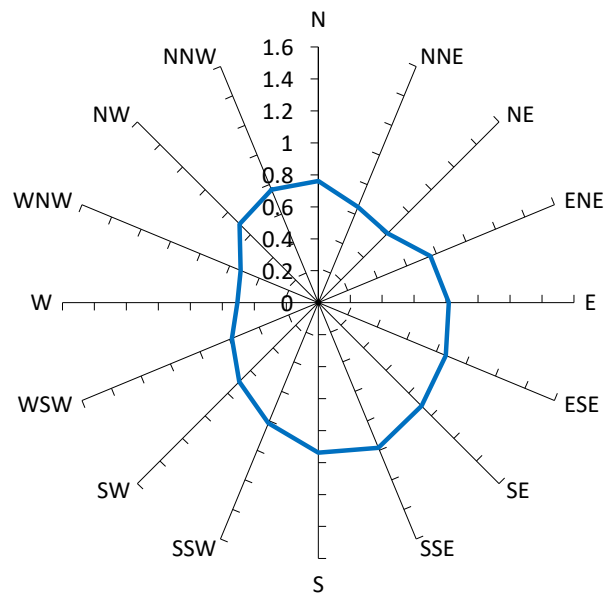
Gust Velocity Coefficients

With development "as proposed", no vegetation or other treatments.

Results for P09



GEM Velocity Coefficients



Gust Velocity Coefficients



— With development "as proposed", no vegetation or other treatments.

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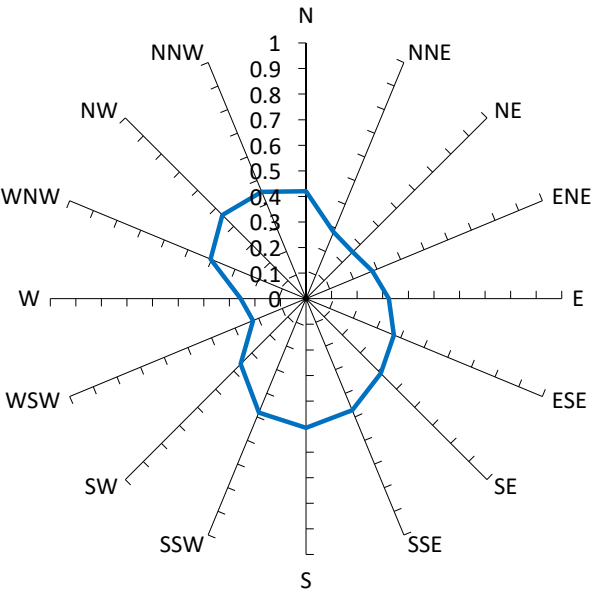
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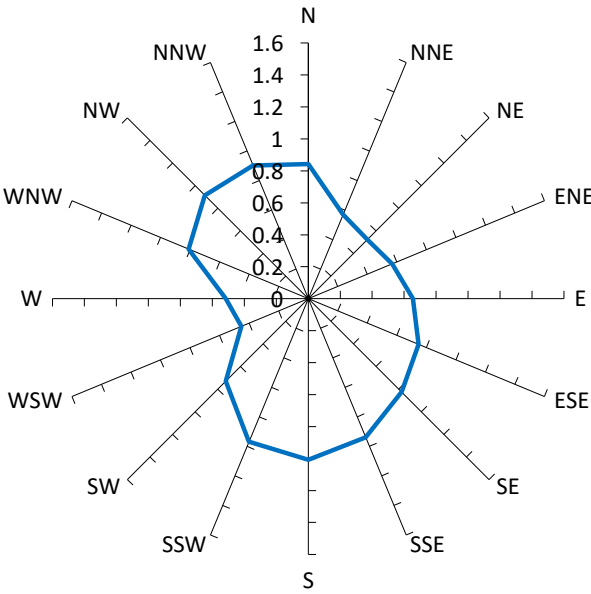
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Results for P10



GEM Velocity Coefficients

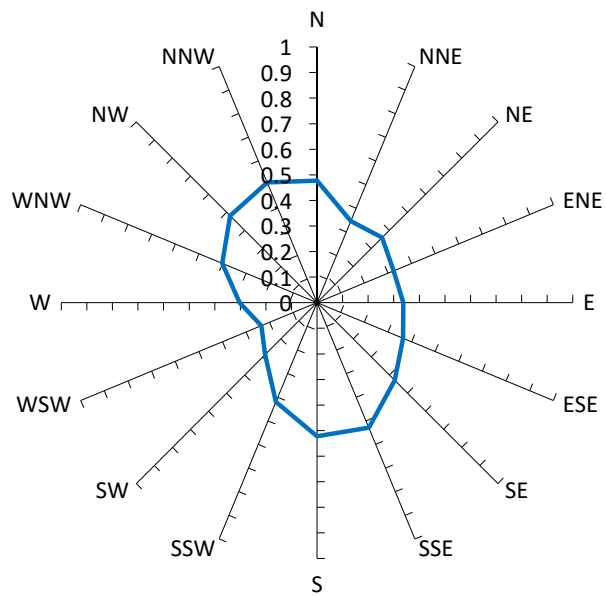


Gust Velocity Coefficients

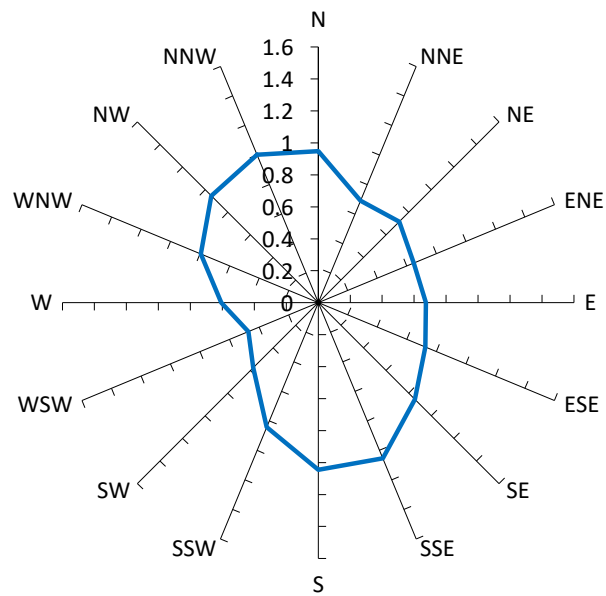
With development "as proposed", no vegetation or other treatments.



Results for P11



GEM Velocity Coefficients

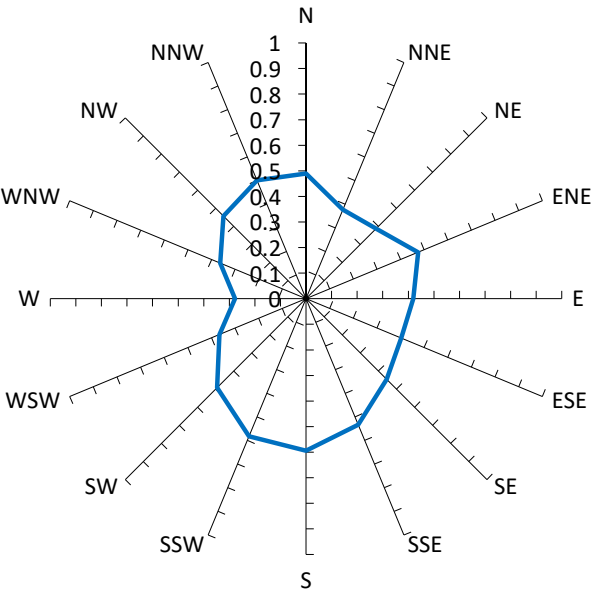


Gust Velocity Coefficients

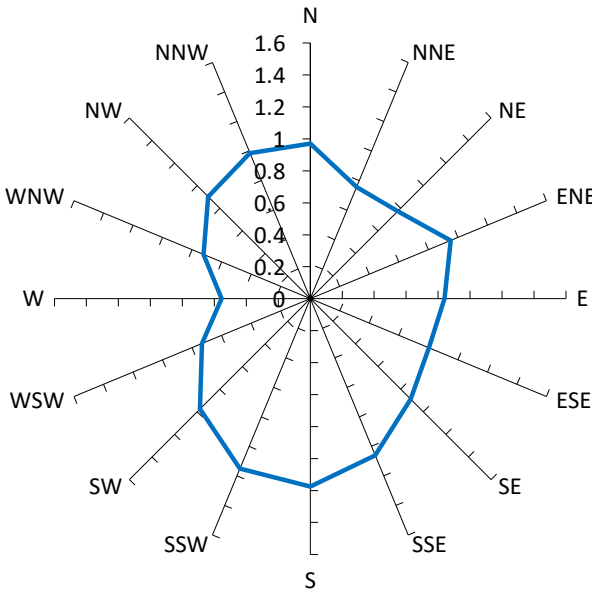


With development "as proposed", no vegetation or other treatments.

Results for P12



GEM Velocity Coefficients

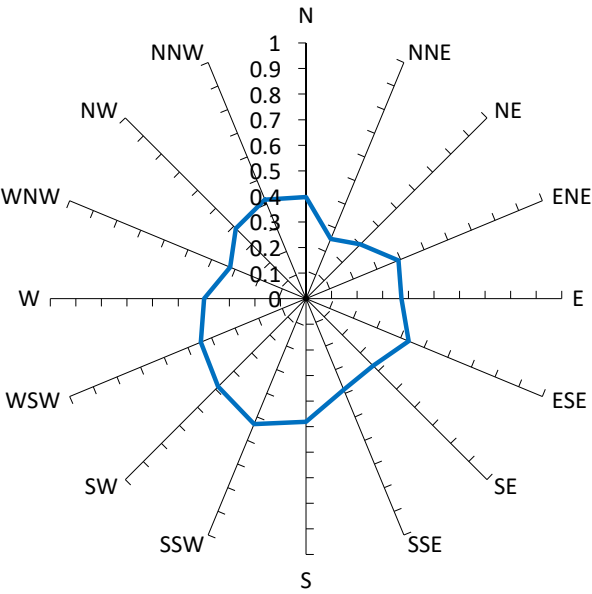


Gust Velocity Coefficients

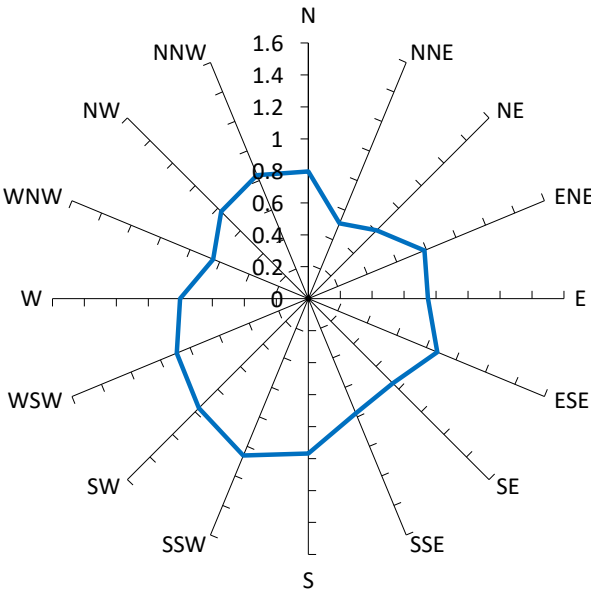


With development "as proposed", no vegetation or other treatments.

Results for P13



GEM Velocity Coefficients

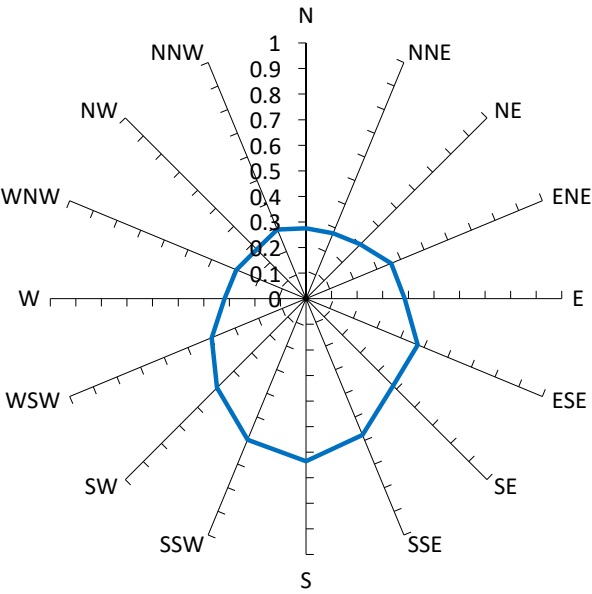


Gust Velocity Coefficients

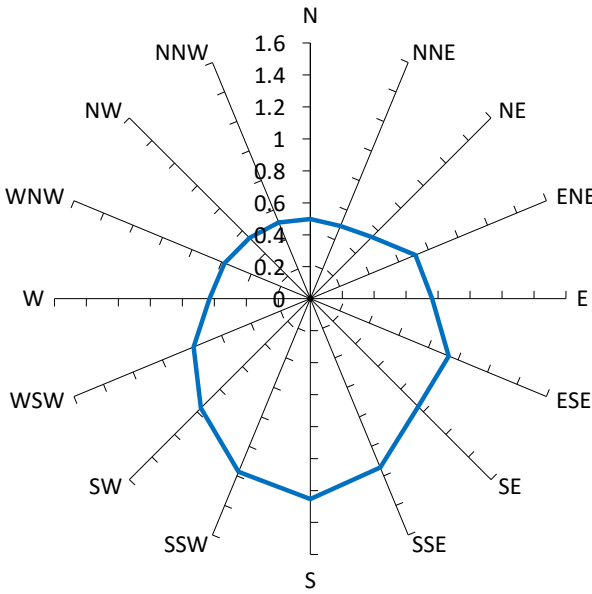


With development "as proposed", no vegetation or other treatments.

Results for P14



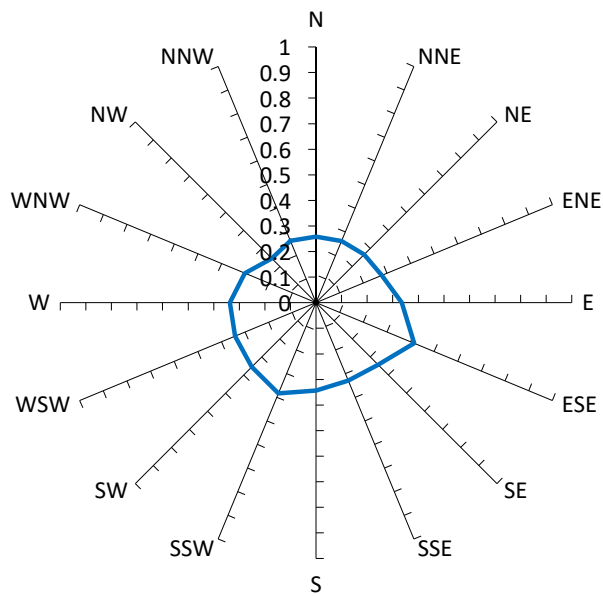
GEM Velocity Coefficients



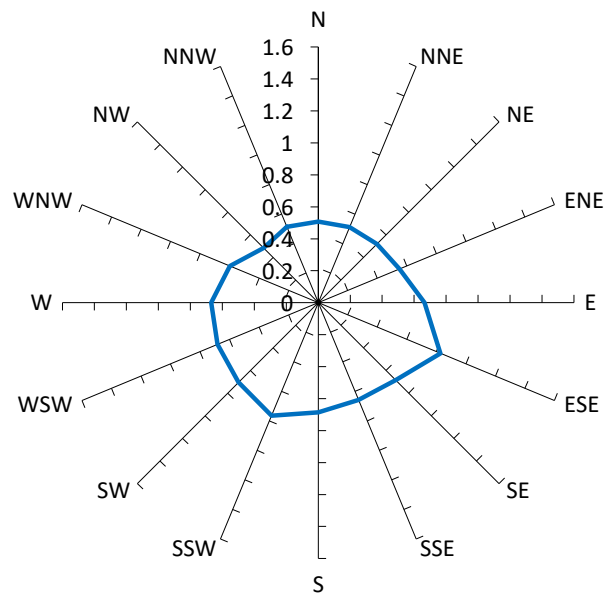
Gust Velocity Coefficients

With development "as proposed", no vegetation or other treatments.

Results for P15



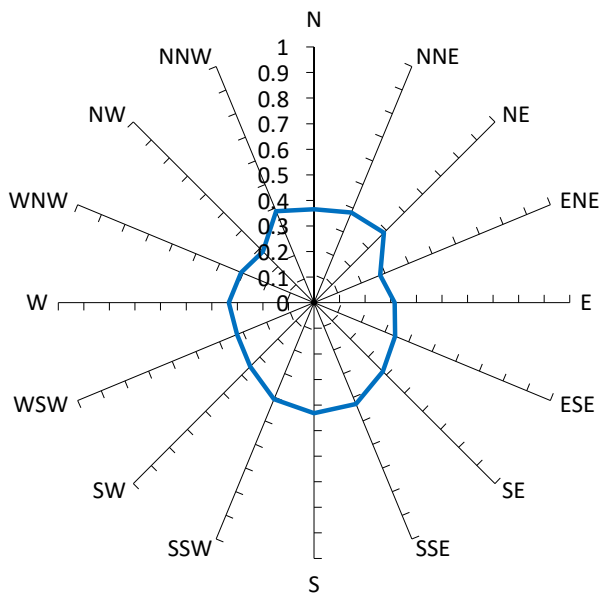
GEM Velocity Coefficients



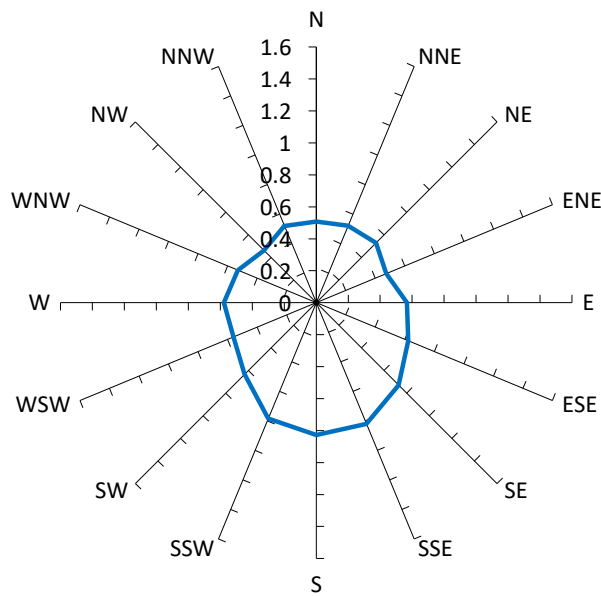
Gust Velocity Coefficients

With development "as proposed", no vegetation or other treatments.

Results for P16



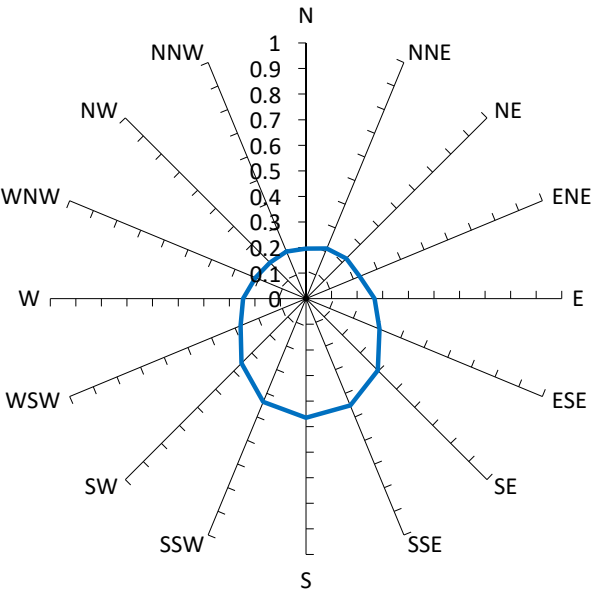
GEM Velocity Coefficients



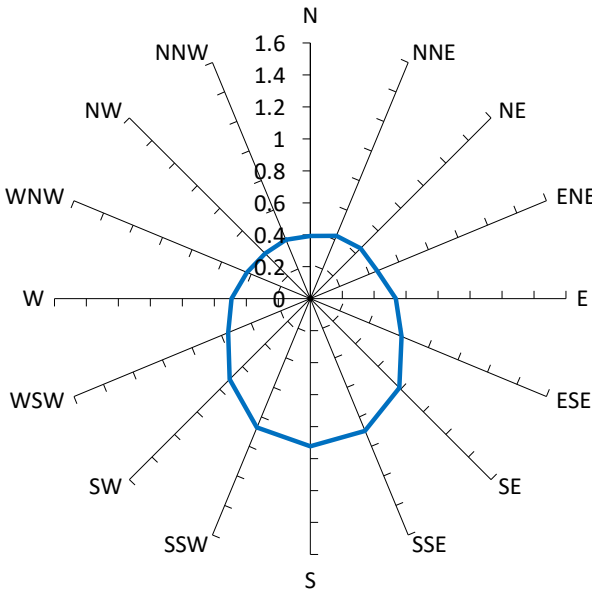
Gust Velocity Coefficients

With development "as proposed", no vegetation or other treatments.

Results for P17



GEM Velocity Coefficients



Gust Velocity Coefficients

— With development "as proposed", no vegetation or other treatments.

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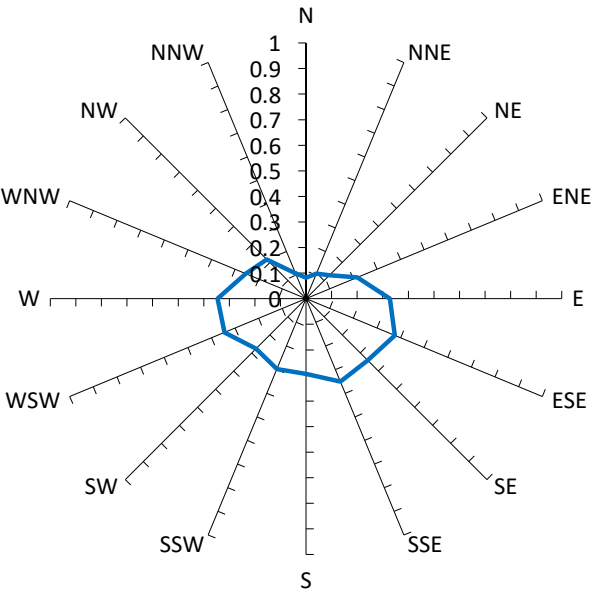
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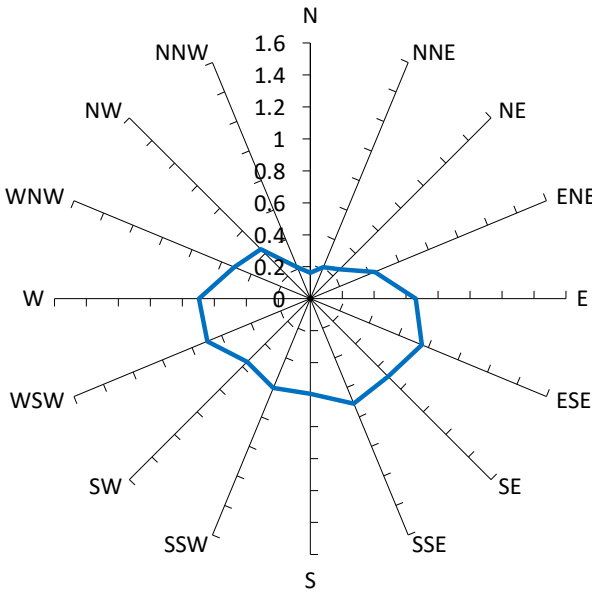
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Results for P18



GEM Velocity Coefficients



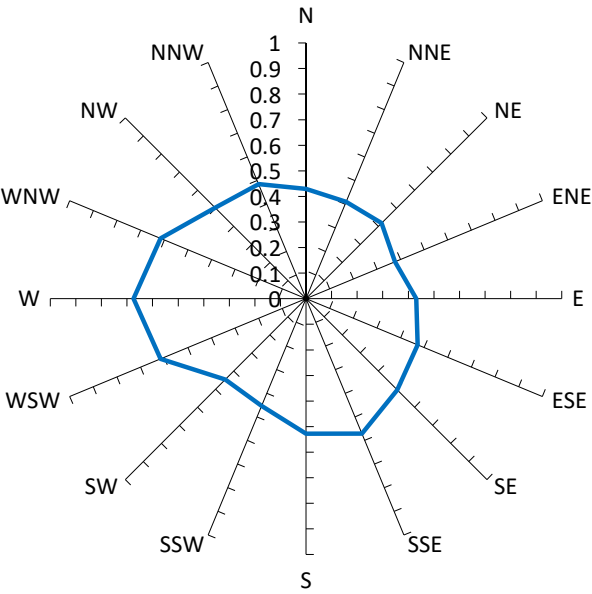
Gust Velocity Coefficients



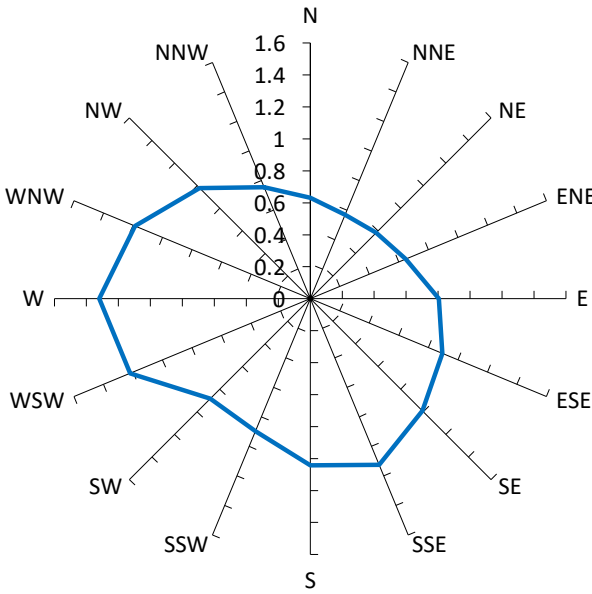
With development "as proposed", no vegetation or other treatments.



Results for P19



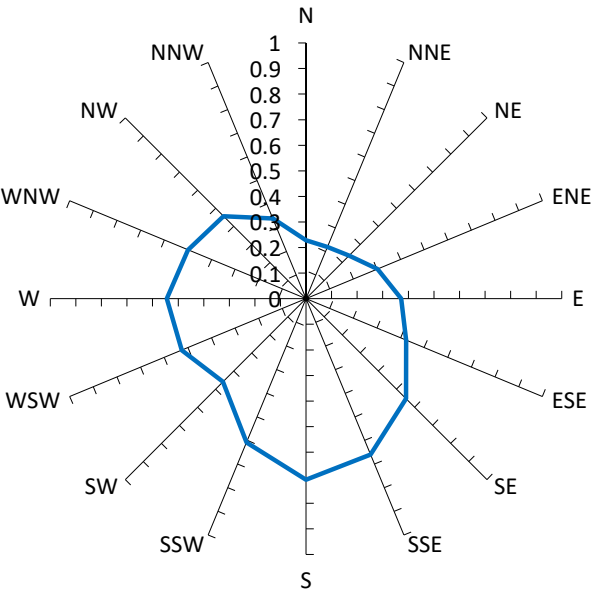
GEM Velocity Coefficients



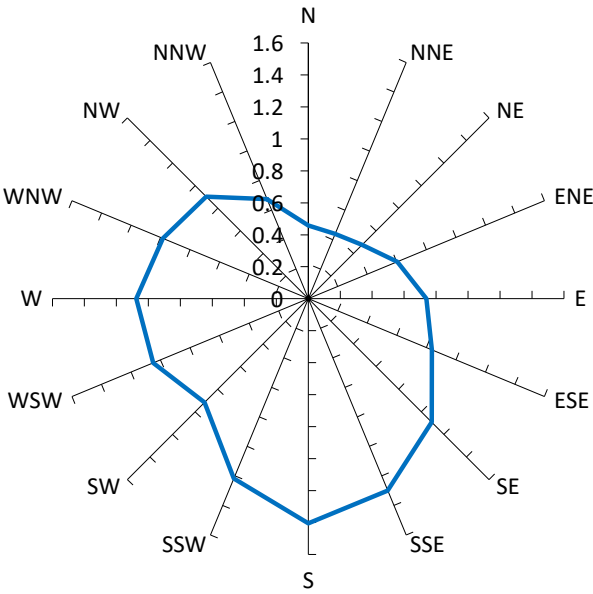
Gust Velocity Coefficients

With development "as proposed", no vegetation or other treatments.

Results for P20



GEM Velocity Coefficients

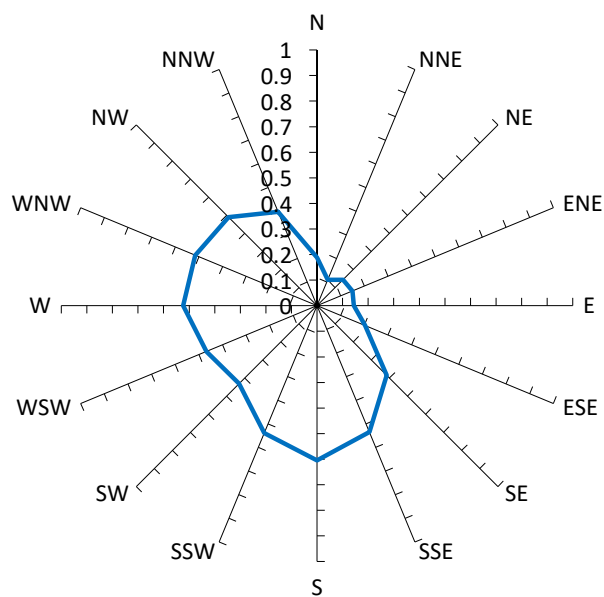


Gust Velocity Coefficients

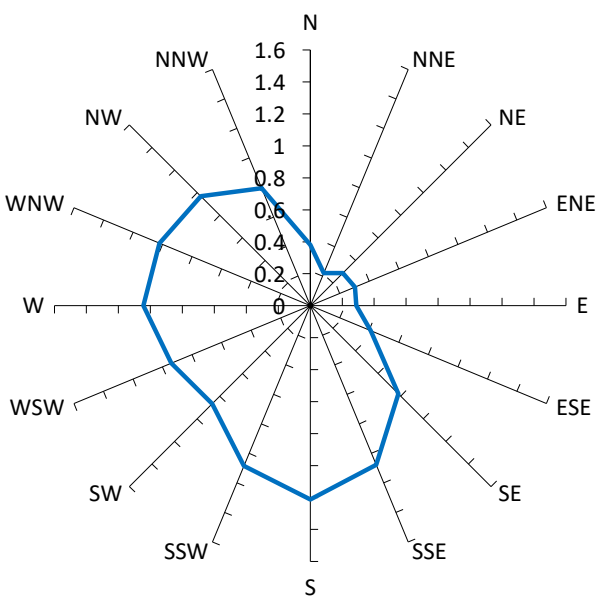
With development "as proposed", no vegetation or other treatments.



Results for P21



GEM Velocity Coefficients

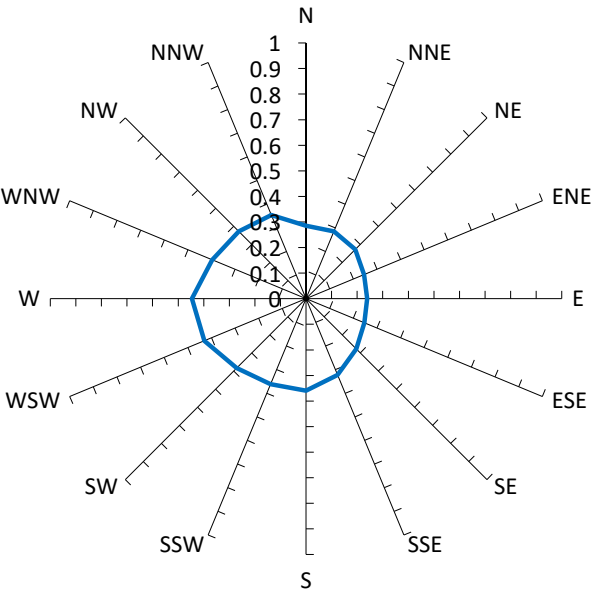


Gust Velocity Coefficients

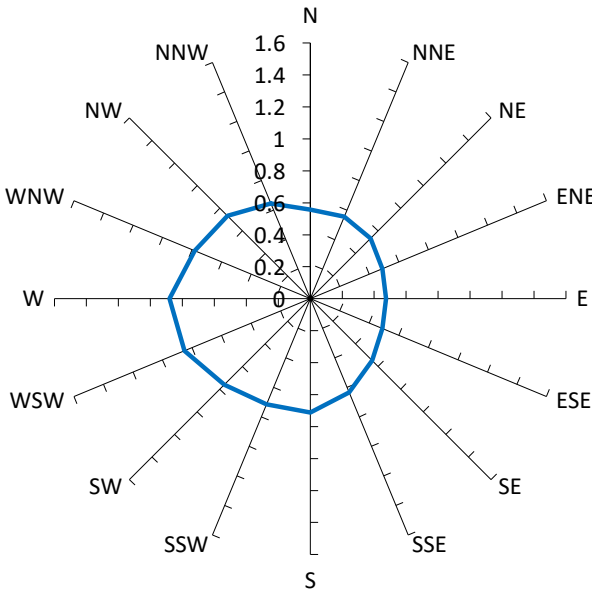


With development "as proposed", no vegetation or other treatments.

Results for P22



GEM Velocity Coefficients



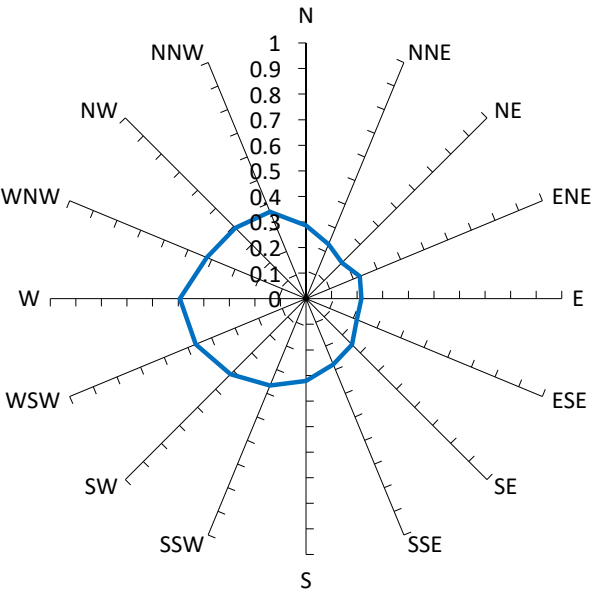
Gust Velocity Coefficients



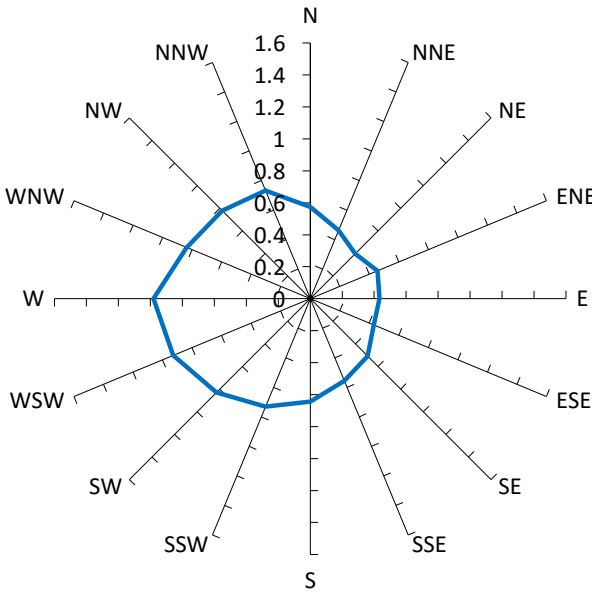
With development "as proposed", no vegetation or other treatments.



Results for P23



GEM Velocity Coefficients

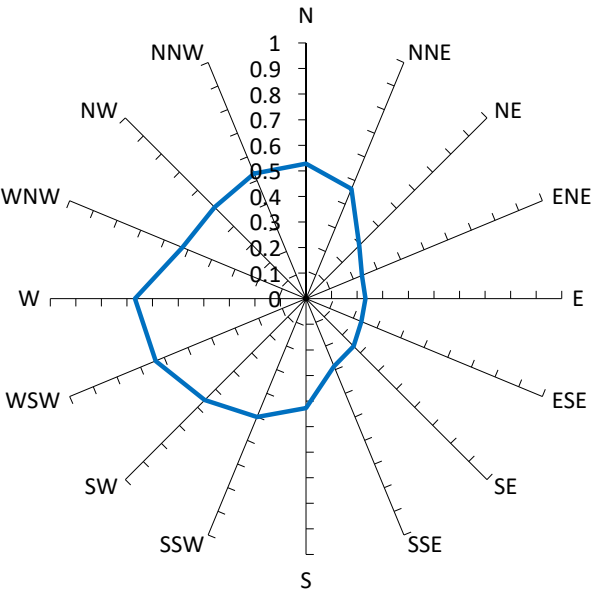


Gust Velocity Coefficients

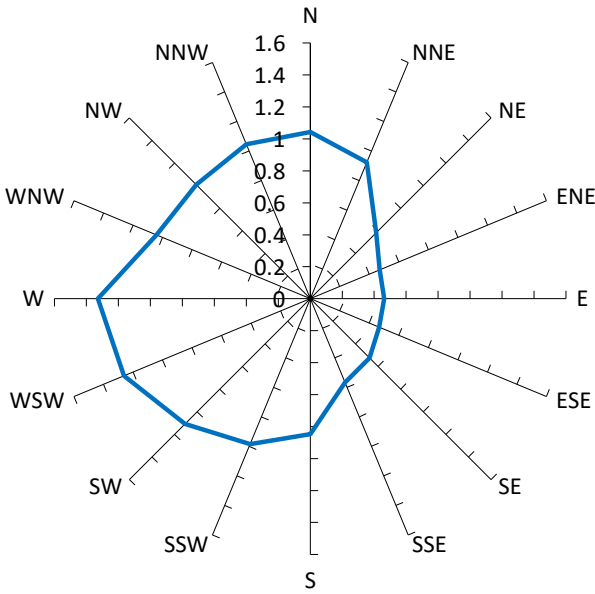


With development "as proposed", no vegetation or other treatments.

Results for P24



GEM Velocity Coefficients

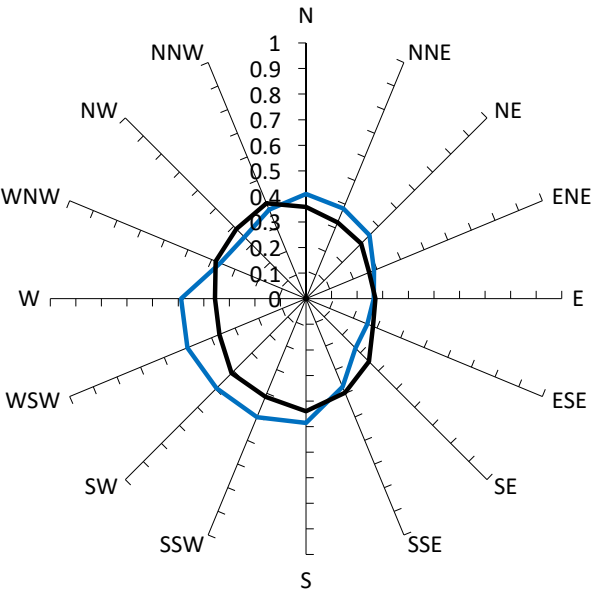


Gust Velocity Coefficients

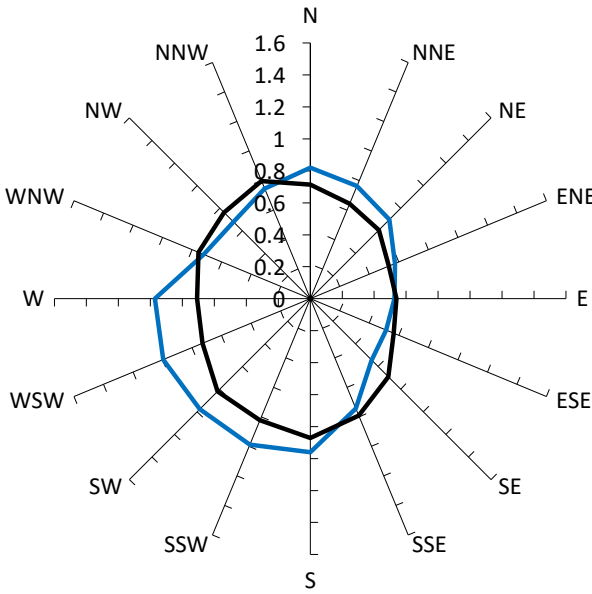
With development "as proposed", no vegetation or other treatments.



Results for P25



GEM Velocity Coefficients

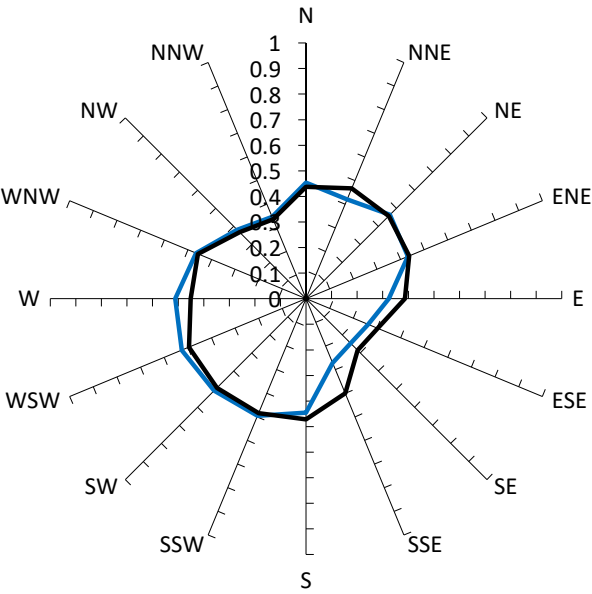


Gust Velocity Coefficients

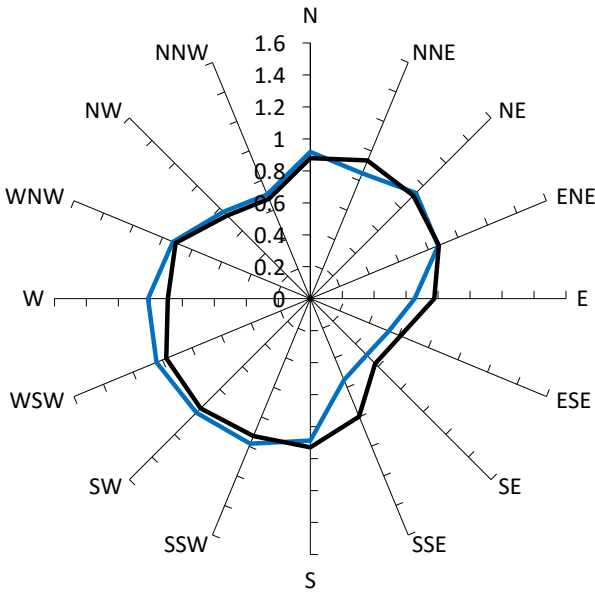
With development "as proposed", no vegetation or other treatments.

Existing Site Conditions.

Results for P26



GEM Velocity Coefficients

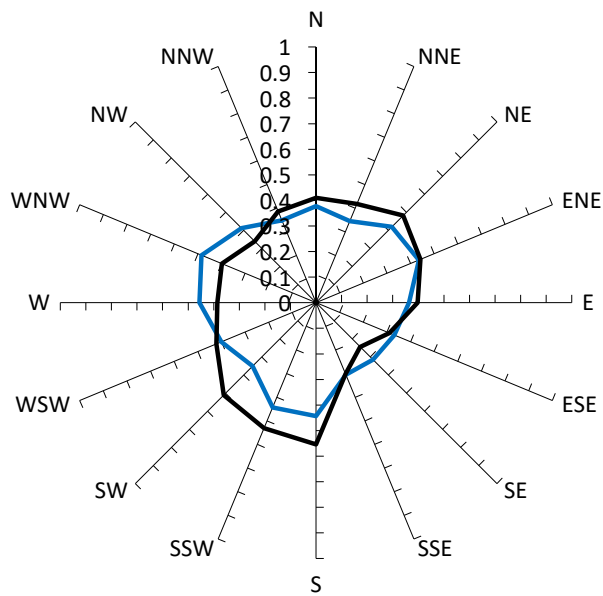


Gust Velocity Coefficients

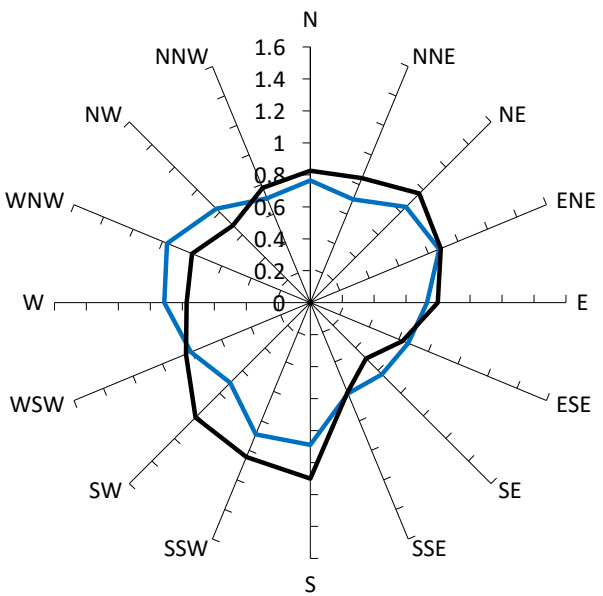
With development "as proposed", no vegetation or other treatments.

Existing Site Conditions.

Results for P27



GEM Velocity Coefficients

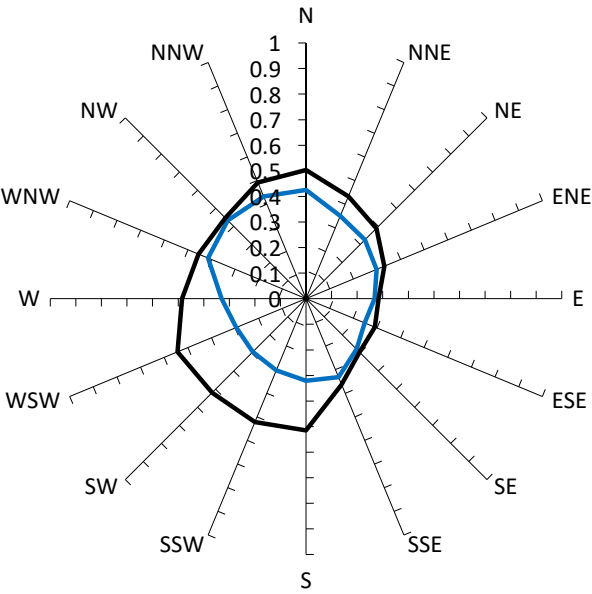


Gust Velocity Coefficients

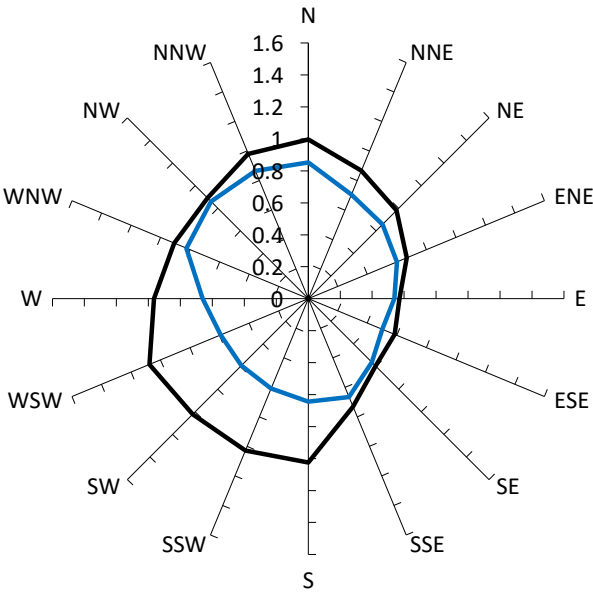
With development "as proposed", no vegetation or other treatments.

Existing Site Conditions.

Results for P28



GEM Velocity Coefficients

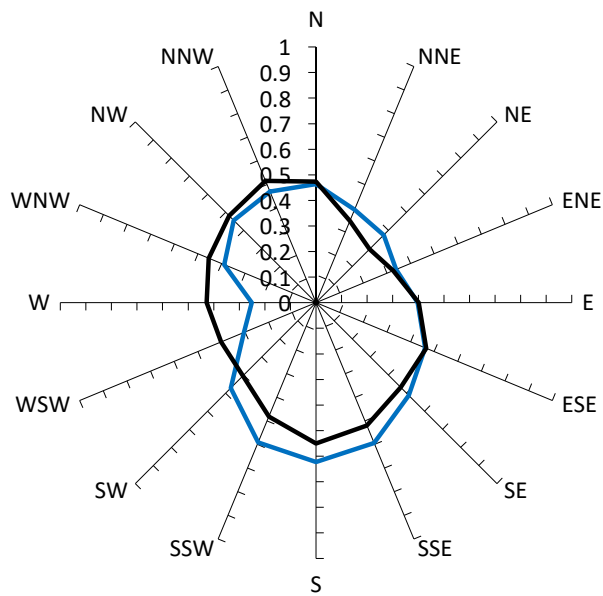


Gust Velocity Coefficients

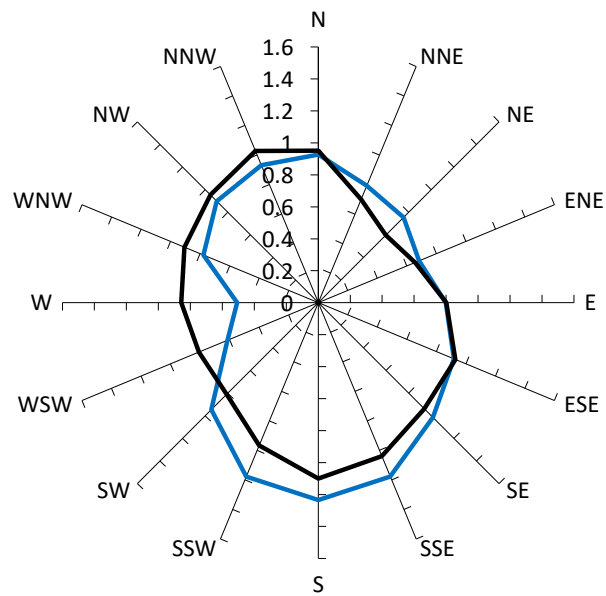
With development "as proposed", no vegetation or other treatments.

Existing Site Conditions.

Results for P29



GEM Velocity Coefficients

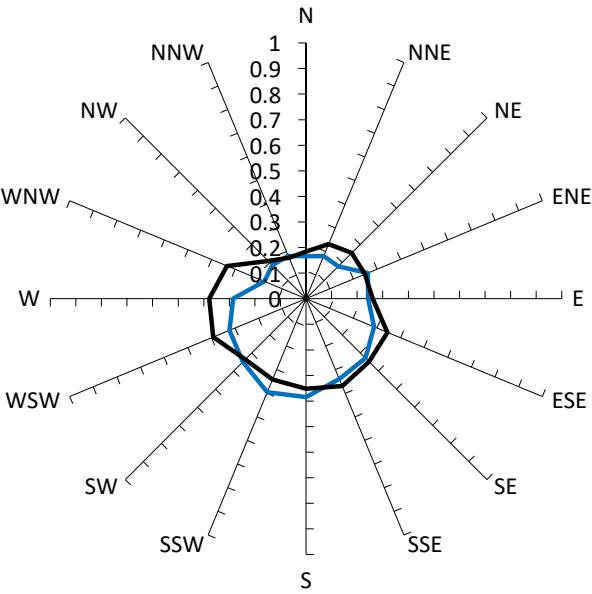


Gust Velocity Coefficients

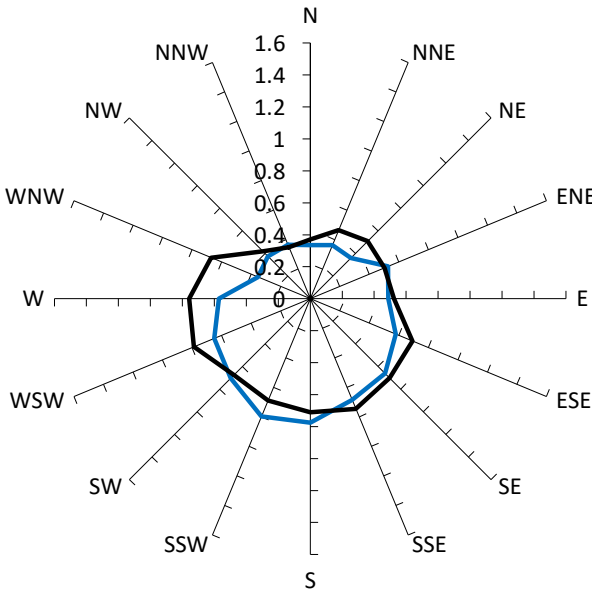
With development "as proposed", no vegetation or other treatments.

Existing Site Conditions.

Results for P30



GEM Velocity Coefficients

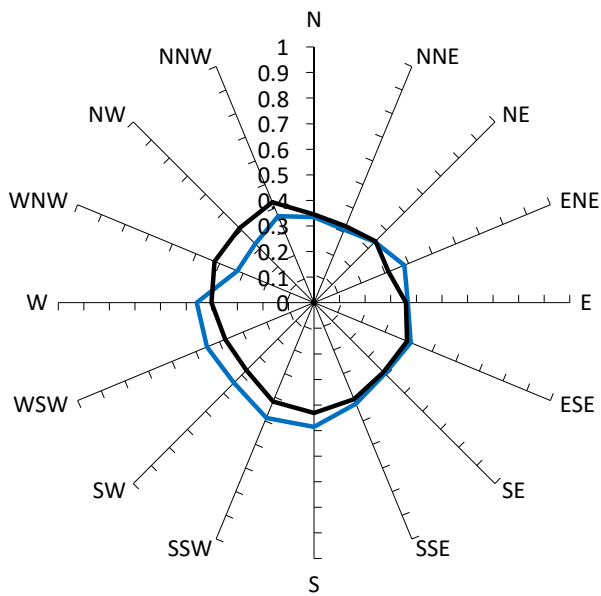


Gust Velocity Coefficients

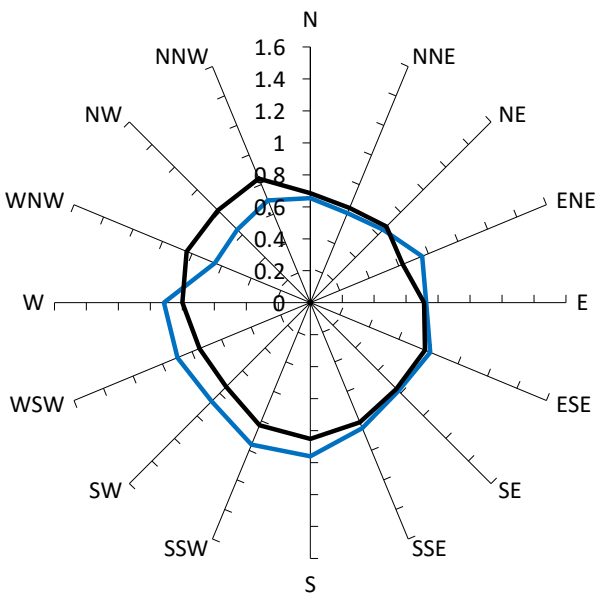
With development "as proposed", no vegetation or other treatments.

Existing Site Conditions.

Results for P31



GEM Velocity Coefficients



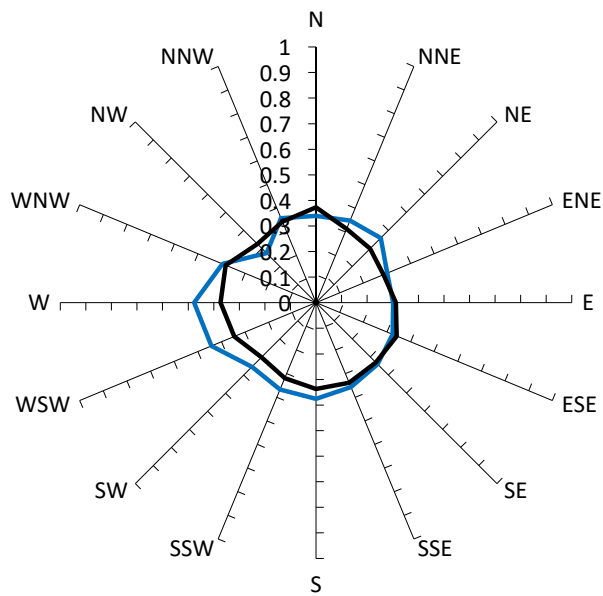
Gust Velocity Coefficients



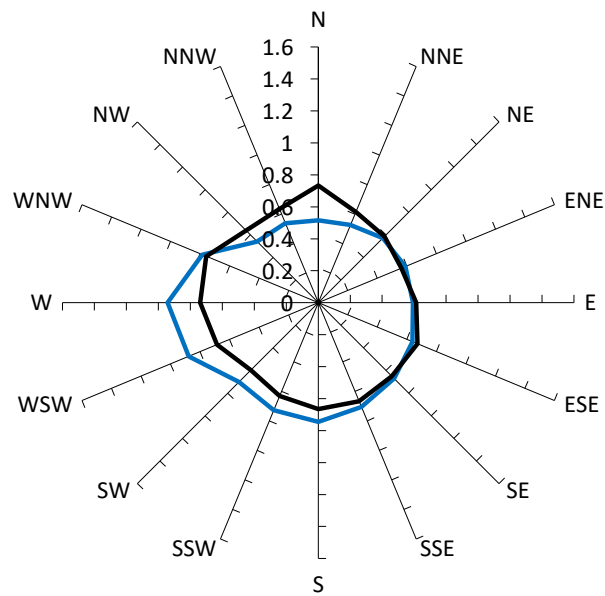
With development "as proposed", no vegetation or other treatments.

Existing Site Conditions.

Results for P32



GEM Velocity Coefficients

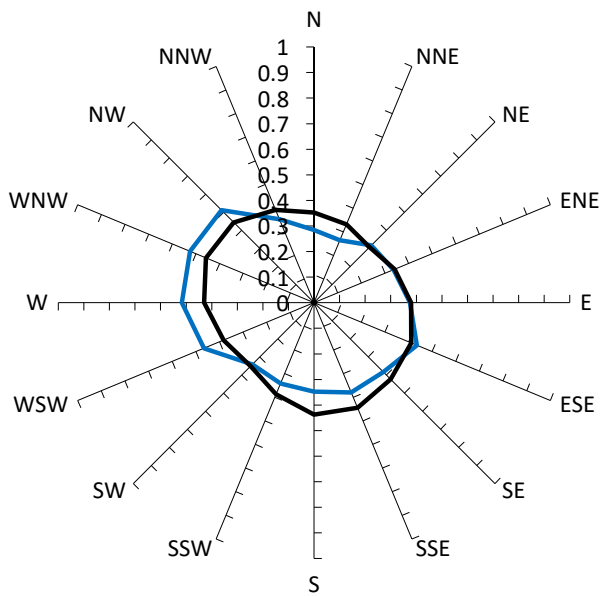


Gust Velocity Coefficients

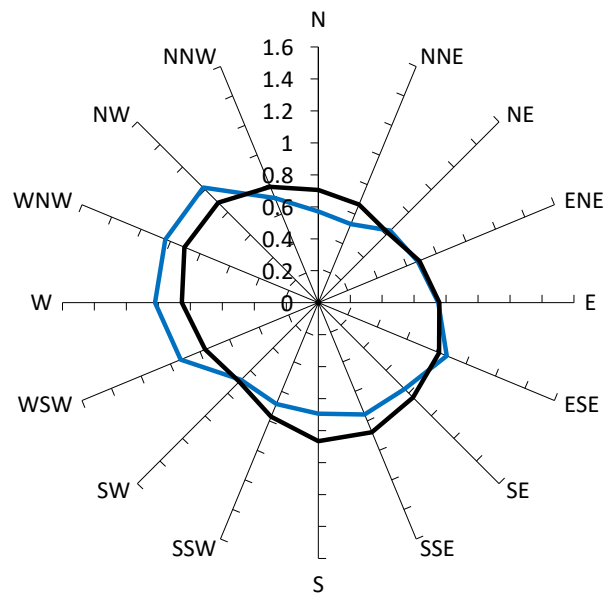
With development "as proposed", no vegetation or other treatments.

Existing Site Conditions.

Results for P33



GEM Velocity Coefficients

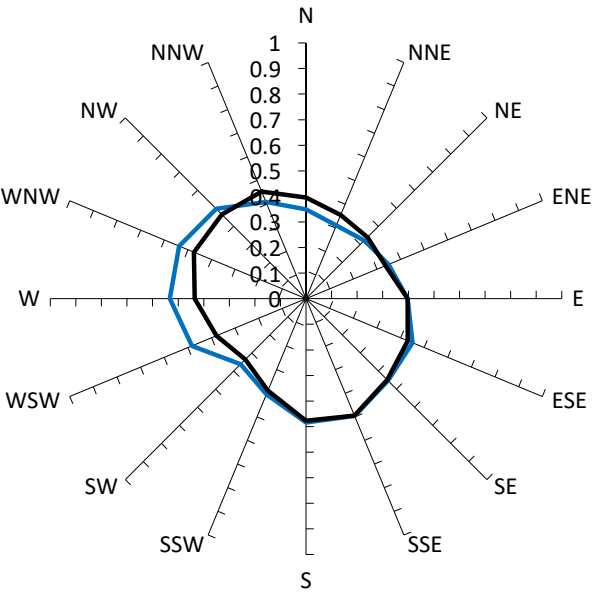


Gust Velocity Coefficients

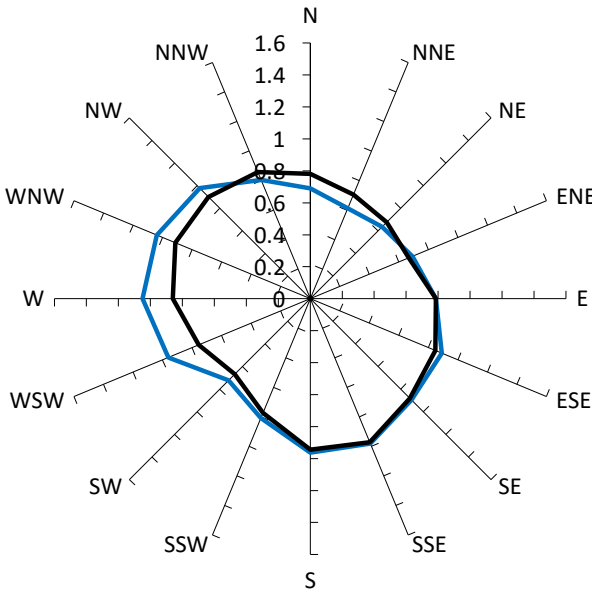
With development "as proposed", no vegetation or other treatments.

Existing Site Conditions.

Results for P34



GEM Velocity Coefficients

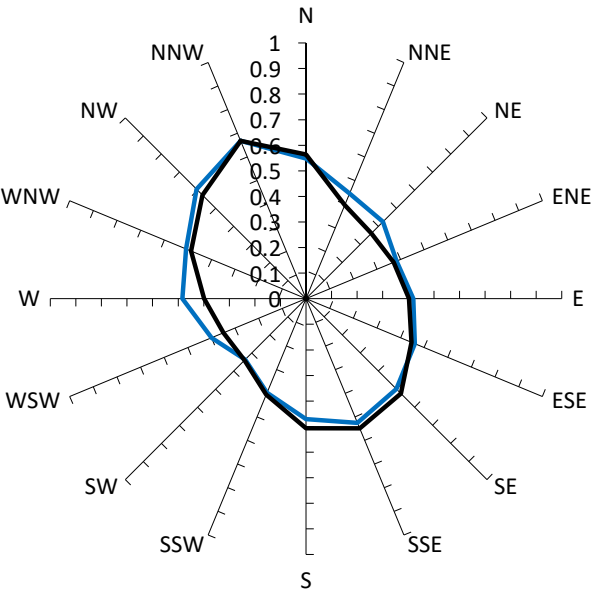


Gust Velocity Coefficients

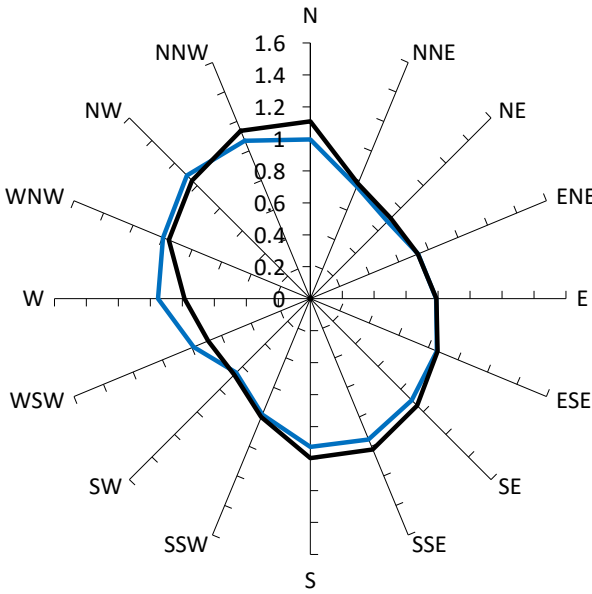
With development "as proposed", no vegetation or other treatments.

Existing Site Conditions.

Results for P35



GEM Velocity Coefficients

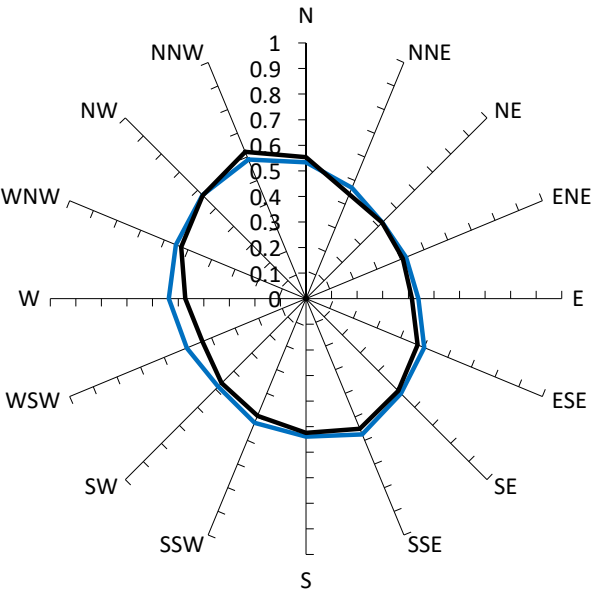


Gust Velocity Coefficients

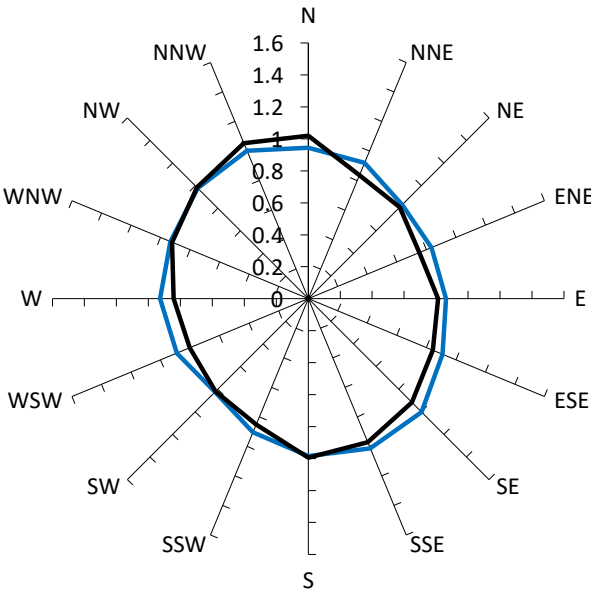
With development "as proposed", no vegetation or other treatments.

Existing Site Conditions.

Results for P36



GEM Velocity Coefficients

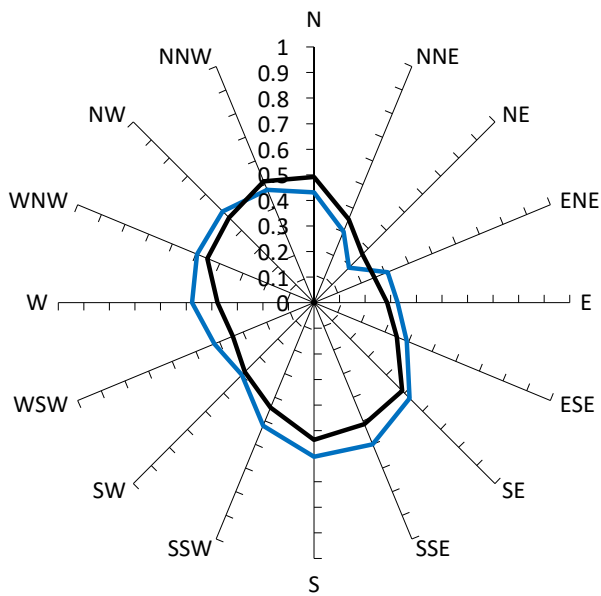


Gust Velocity Coefficients

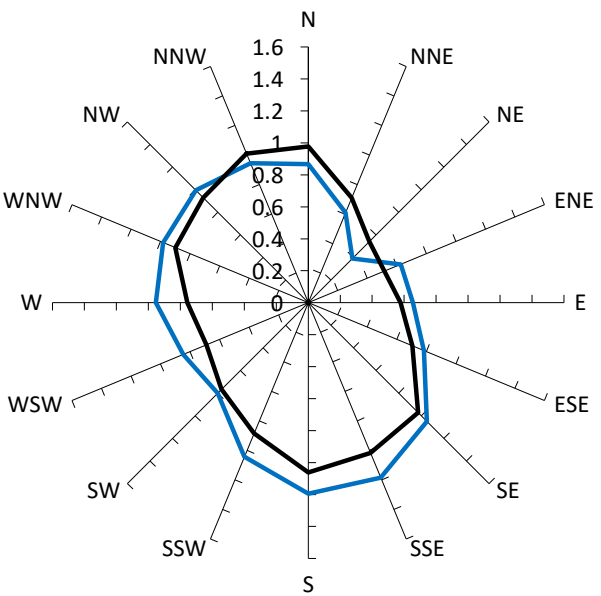
With development "as proposed", no vegetation or other treatments.

Existing Site Conditions.

Results for P37



GEM Velocity Coefficients

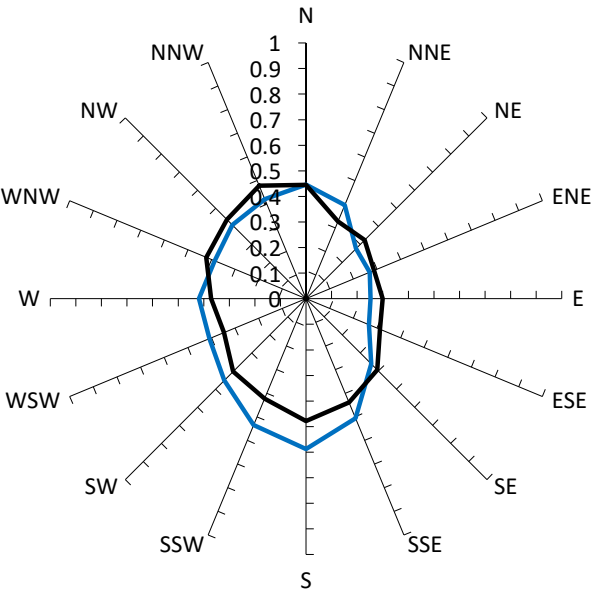


Gust Velocity Coefficients

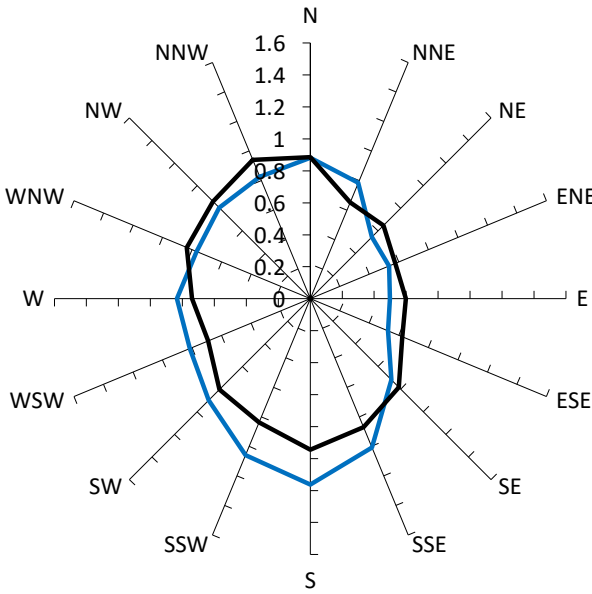
With development "as proposed", no vegetation or other treatments.

Existing Site Conditions.

Results for P38



GEM Velocity Coefficients



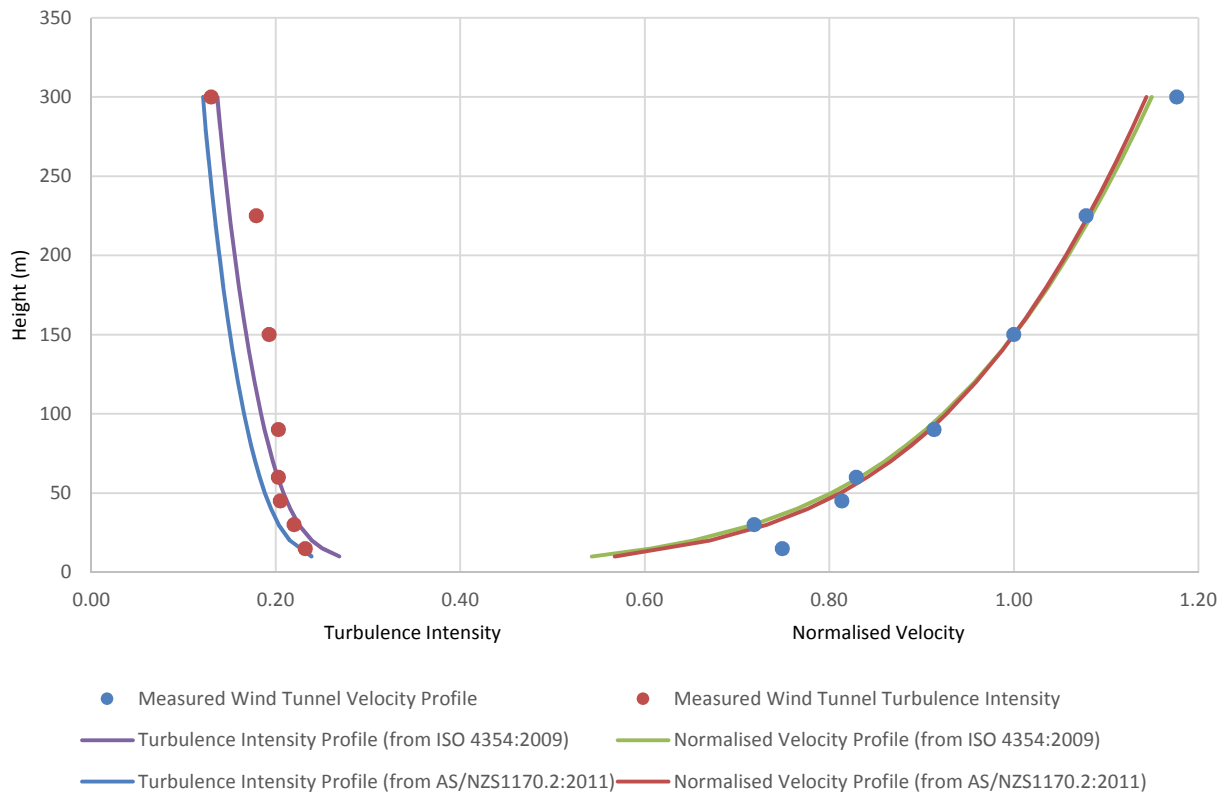
Gust Velocity Coefficients

With development "as proposed", no vegetation or other treatments.

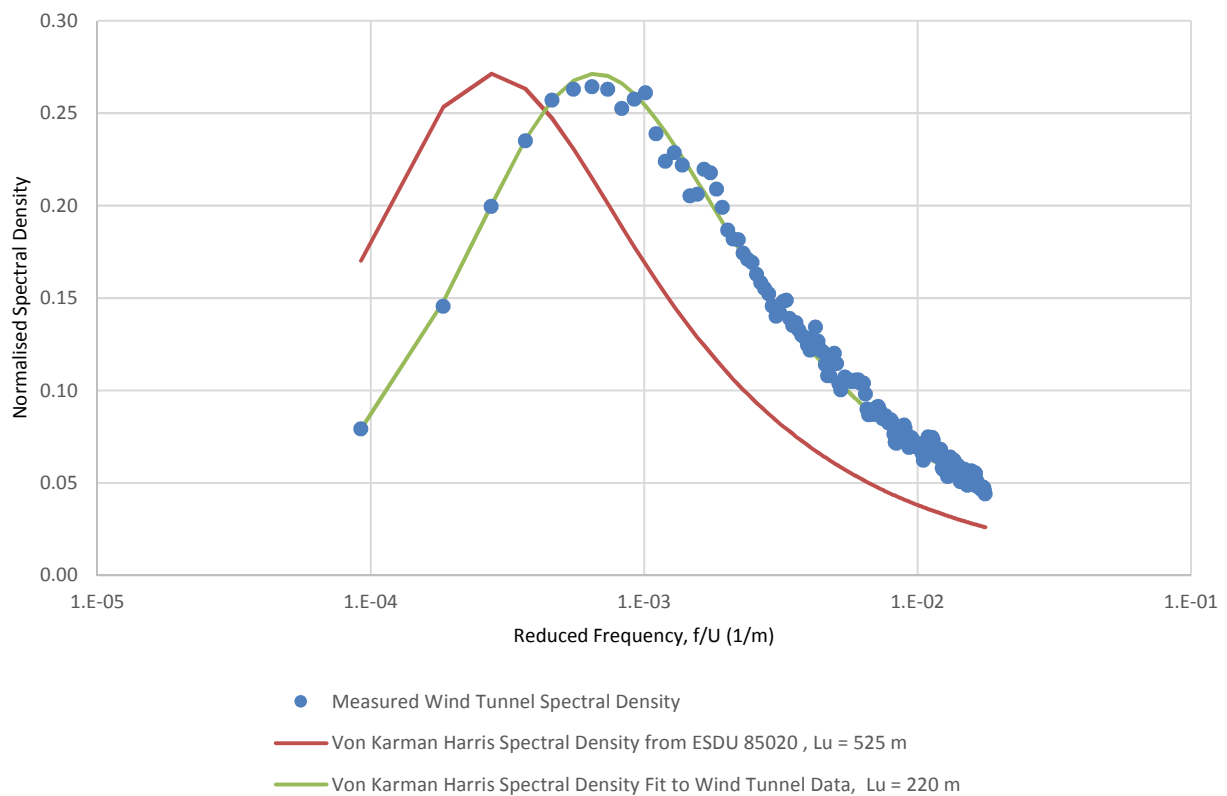
Existing Site Conditions.

APPENDIX E VELOCITY AND TURBULENCE INTENSITY PROFILES

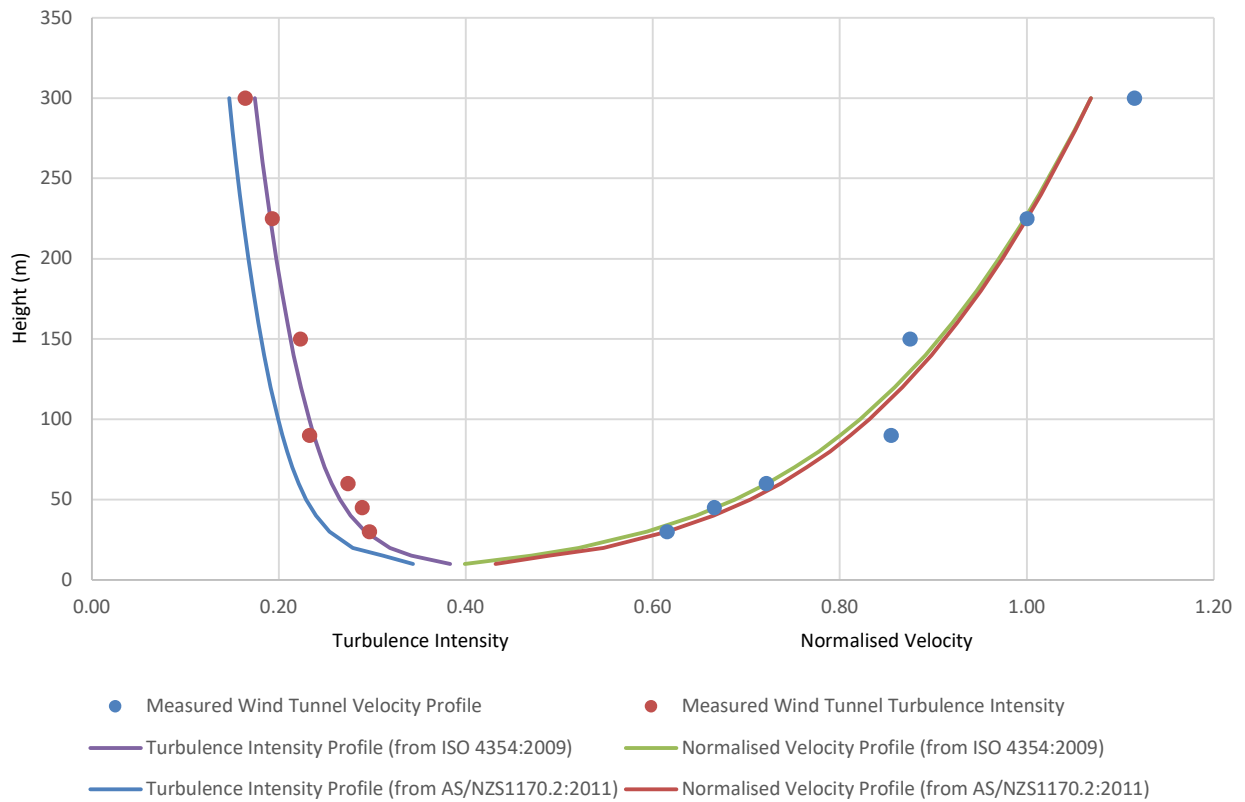
Mean Velocity and Turbulence Intensity for Suburban/Forest Terrain ($0.2\text{m} < z_0 < 0.3\text{m}$) (TC3) at a 1:300 Scale



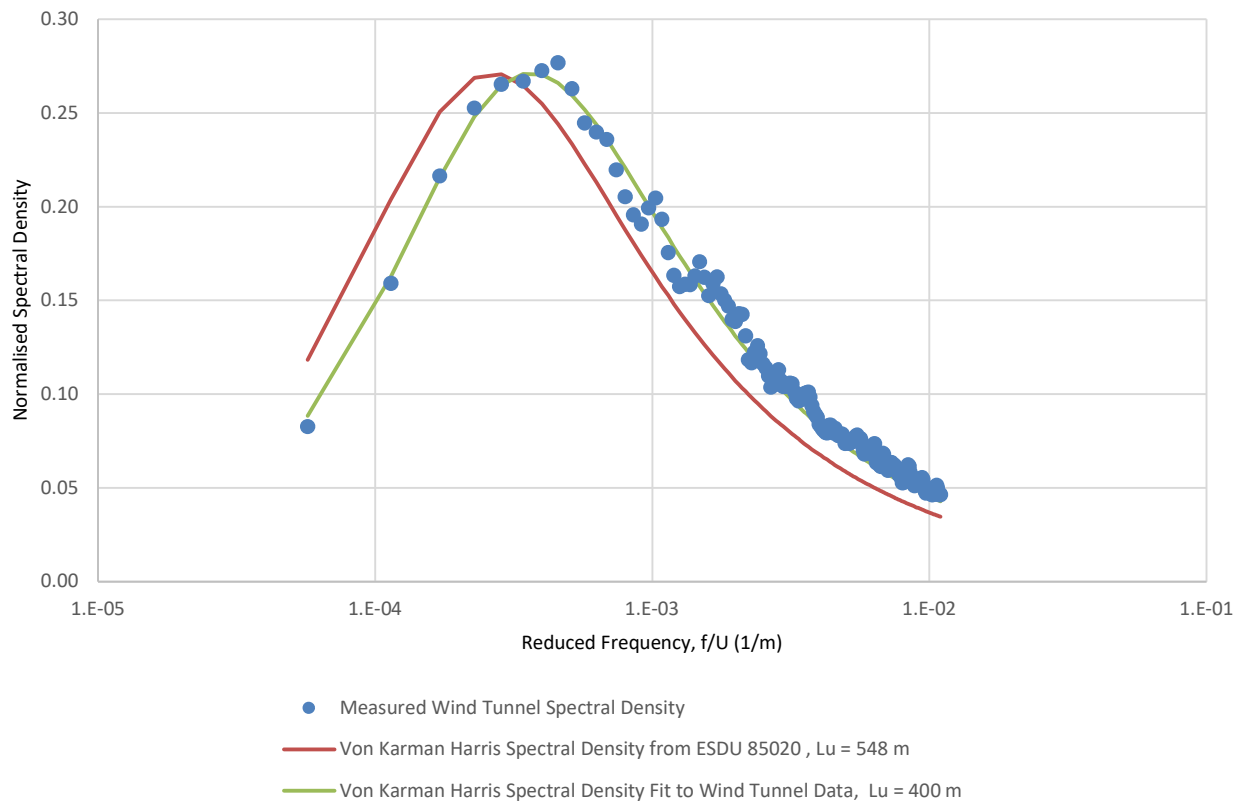
Longitudinal Spectra Density for Suburban/Forest Terrain ($0.2\text{m} < z_0 < 0.3\text{m}$) (TC3) at a 1:300 Scale



Mean Velocity and Turbulence Intensity for Semi-Urban Terrain ($0.6\text{m} < z_0 < 1.0\text{m}$) (TC3.5) at a 1:300 Scale



Longitudinal Spectra Density for Semi-Urban Terrain ($0.6\text{m} < z_0 < 1.0\text{m}$) (TC3.5) at a 1:300 Scale



APPENDIX F LIST OF ARCHITECTURAL DRAWINGS REFERENCED

Type of Files	File Name	Description	Revision
Revit	AR_2306010_RSL_BUILDING[3]	3D Model	Rev 0
PDF	AR-SD-B10-01-GA PLAN LEVEL 01	LVL 1	Rev 0
PDF	AR-SD-B10-01-GA PLAN LEVEL 02	LVL 2	Rev 0
PDF	AR-SD-B10-01-GA PLAN LEVEL 03	LVL 3	Rev 0
PDF	AR-SD-B10-01-GA PLAN LEVEL 04	LVL 4	Rev 0
PDF	AR-SD-B10-01-GA PLAN LEVEL 05	LVL 5	Rev 0
PDF	AR-SD-B10-06-GA PLAN ROOF	Roof	Rev 0
PDF	AR-SD-C10-01-NORTH AND EAST ELEVATION	Elevation	Rev 0
PDF	AR-SD-C10-02-WEST AND SOUTH ELEVATION	Elevation	Rev 0
PDF	AR-SD-D10-01-SECTIONS	Sections	Rev 0
PDF	AR-SD-D10-02-SECTIONS	Sections	Rev 0
PDF	AR-SD-D10-03-SECTIONS	Sections	Rev 0
PDF	AR-SD-D10-04-SECTIONS	Sections	Rev 0
PDF	Combined PDF	Combined	Rev 0
Dwg	AR_2306010_RSL_BUILDING[3]	3D Model	Rev 0
Dwg	AR-SD-B10-01-GA PLAN LEVEL 01	LVL 1	Rev 0
Dwg	AR-SD-B10-01-GA PLAN LEVEL 02	LVL 2	Rev 0
Dwg	AR-SD-B10-01-GA PLAN LEVEL 03	LVL 3	Rev 0
Dwg	AR-SD-B10-01-GA PLAN LEVEL 04	LVL 4	Rev 0
Dwg	AR-SD-B10-01-GA PLAN LEVEL 05	LVL 5	Rev 0
Dwg	AR-SD-B10-06-GA PLAN ROOF	Roof	Rev 0
Dwg	AR-SD-C10-01-NORTH AND EAST ELEVATION	Elevation	Rev 0
Dwg	AR-SD-C10-02-WEST AND SOUTH ELEVATION	Elevation	Rev 0
Dwg	AR-SD-D10-01-SECTIONS	Sections	Rev 0
Dwg	AR-SD-D10-02-SECTIONS	Sections	Rev 0
Dwg	AR-SD-D10-03-SECTIONS	Sections	Rev 0
Dwg	AR-SD-D10-04-SECTIONS	Sections	Rev 0