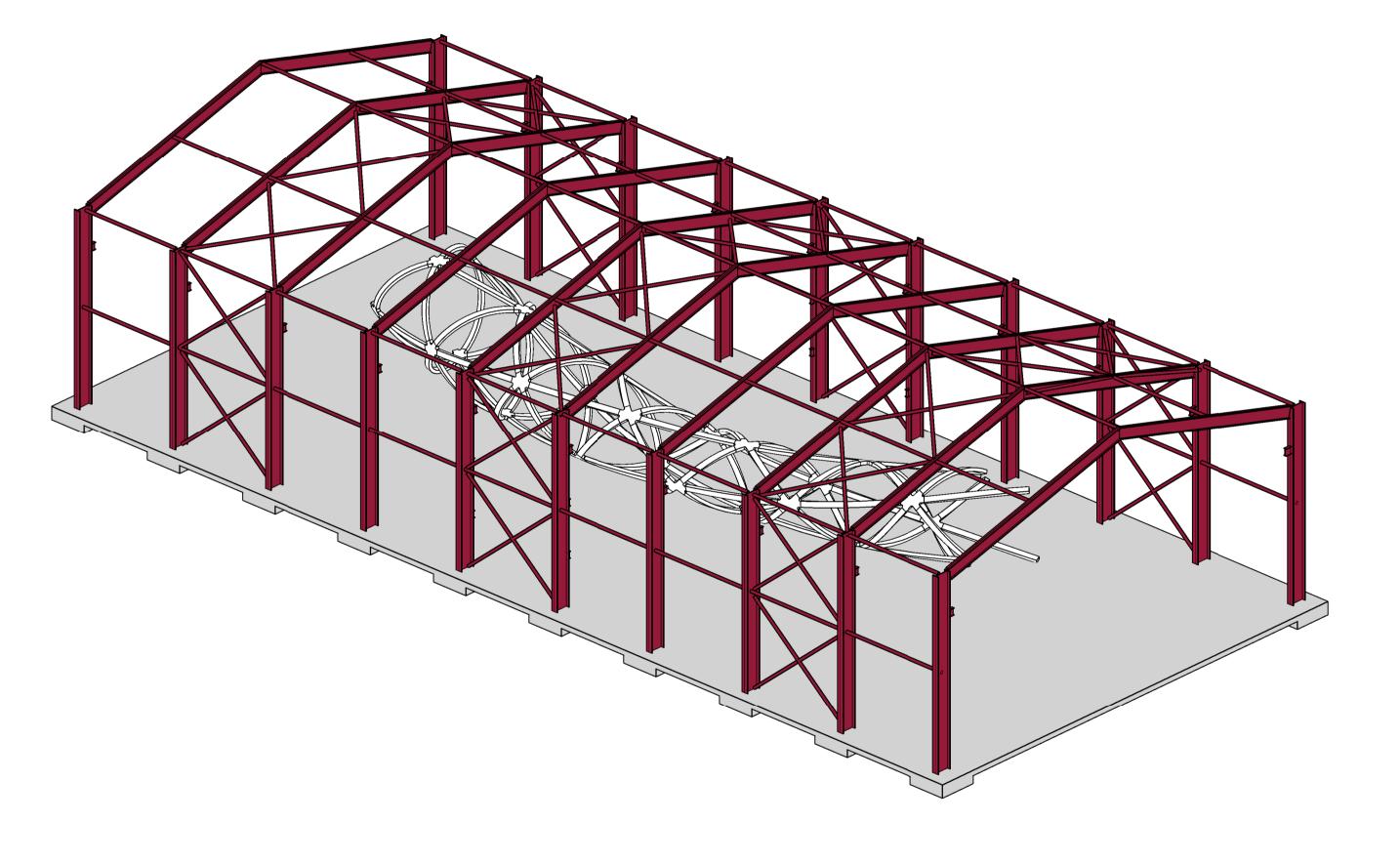
LACHLAN'S LINE TEMPORARY PORTAL SHED Structural Drawings

Structural Drawing List			
Sheet No.	Sheet Name		
S-0001	Cover Sheet		
S-0010	General Notes		
S-0020	Typical Details - Concrete		
S-0025	Typical Details - Steel - Sheet 1		
S-0100	Slab on Ground Plan & Details		
S-0200	Structural Framing Plans		
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S-0220	Steelwork Details - Sheet 1		

Do not scale



Isometric View

* with Stage 4 Helix Structure inside for information

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Project Title

LACHLAN'S LINE

TEMPORARY PORTAL SHED

Drawing Title

Cover Sheet

Scale at A1

N.T.S.

Role
Structural

Suitability
For Information

Arup Job No
247451-89

Rev
P1

Name **S-0001**

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PRELIMINARY ONLY

GENERAL

- G1 Structural drawings to be read in conjunction with the structural specifications and all architectural and other consultants' drawings and specifications.
- G2 Any discrepancies shall be referred to the Architect for a decision before proceeding with the work.
- G3 For setting out dimensions refer to architectural drawings. No dimensions are to be obtained by scaling from drawings.
- G4 Unless otherwise noted, all levels are given in metres and all dimensions are in millimetres.
- G5 The Builder shall be responsible for maintaining the stability of the structure until its completion and shall ensure that no part of the structure is overstressed by excessive construction loading or contains "locked in" stresses from temporary conditions.
- G6 All materials and workmanship shall be in accordance with Australian standards and codes of practice except where varied by the specification and/or drawings. These Standards for this project shall be determined by reference to the document history on the Standards Australia website as of October 2016. This determination of applicable Standards shall be carried out during the Tender period and any queries relating to the appropriate Standard shall be raised with Arup during this period.
- G7 Each aspect of structural work is to be checked by the Engineer after the Builder has satisfied himself that it is correct, and before it is concealed. The Engineer is to be given 48 hours notice of inspection being required.
- G8 Design wind loads: To AS/NZS 1170.2

Importance Level (BCA Table B1.2a) = 2 Annual probability of exceedance = 1:500 Regional Wind speeds V25 = 37 m/s Basic wind speed V20 (serviceability) = 37 m/s Terrain category = 2.5

G9 Design earthquake loads: to AS/NZS 1170.4

Earthquake design category = II Site sub soil class = Ce Site Hazard Factor = 0.08 Annual probability of exceedance (BCA Table B1.2b) =

Structural ductility factor / Performance factor = 0.38

G10 Abbreviations

NTS - Not to scale UNO - Unless noted otherwise NSOP - Not shown on plan

NSOE - Not shown on elevation.

- G11 Prior to construction of all concrete floors and walls, the contractor shall prepare, for review by the architect and design consultant, fully co-ordinated floor plans and wall elevations showing all proposed penetrations, openings and cast-in fixings, which shall also require the planning and coordination of all services who may require such penetrations, openings and fixings.
- G12 When construction involves modification or addition to existing structure, the builder shall advise if boney and/or spalling concrete, exposed reinforcement, rust stains, cracked concrete and cracked masonry walls are observed These areas will need to be rectified as directed by the engineer. These drawings have been prepared based on available existing structural drawings and limited site inspections and should not be relied upon as being a true representation of the existing structure. It is the builder's responsibility to inspect the existing structure and advise of any discrepancies and obtain instructions from Arup prior to

SLAB ON GRADE

Do not scale

- SG1 Remove all top soil including roots and any other organic matter, store top soil as required.
- SG2 Excavate to the required formation level and proof roll formation with 6 passes of 10 tonne smooth wheeled roller soft areas are to be removed and replaced with suitable fill compacted to the density nominated below.
- SG3 Where shown on the drawings, base and sand blinding are to be placed and compacted as specified.
- SG4 Fill material shall meet the requirements of the specification. Place in maximum 150mm thick layers in accordance with as 1289.5.4.1 for cohesive soils and in accordance with as 1289.5.6.1 for granular soils. Maximum depth of fill is to be 750mm unless noted otherwise by the engineer. The following table shall be used for minimum relative compaction levels of fill material.
- SG5 Clay subgrade formation is to be maintained at optimum moisture content -0,+3% prior to covering.

CONCRETE

S50

S65

- C1 All concrete work to be in accordance with AS 3600.
- *C2 Concrete shall be as follows unless noted otherwise on the drawings: 80mm ±15mm Slump:

Max. aggregate: 20mm Cement: type gp * Strength Min. cement Max w/c (kg/m3) grade ratio S25 250 0.55 S32 280 0.55 S40

*The fines ratio which is defined as the proportion by weight

330

400

400

0.50

0.45

0.45

passing the 1.18mm sieve size to the total aggregate in the mix, must not exceed 0.4.

or where control required is less critical.

80mm ±15mm Slump: Max. aggregate: 20mm N20, N25, N32, N40 or N50.

Maximum drying shrinkage strain measured in accordance with AS 1012 Part 13 shall not exceed 650 x 10-6 at eight weeks for all critical elements.

Unless noted otherwise, the characteristic concrete strength and clear cover to the reinforcement (including fitments) shall be as follows:

Element	Exposed to weather		Not Exposed to weather	
	Grade (Mpa)	Cover (mm)	Grade (Mpa)	Cover (mm)
Blinding and Mass Concrete	N15	-	N15	-
Slab on Grade	S32	40	S25	20

- Sizes of concrete elements do not include thickness of applied finishes.
- Beam depths are noted first and include thickness of slab if
- Cure concrete by keeping continuously wet for 7 days or by covering with clear plastic sheeting's. Alternative equivalent methods of curing may be submitted to the Engineer for
- C7 All penetrations required in concrete are shown on the structural drawings and/or an approved coordinated penetrations drawing, if any. The Builder is to seek the approval of the Engineer if any additional penetrations are
- C8 No plumbing or vacuum pipes to be cast into slabs unless shown otherwise on the structural drawings or the Engineer has given written approval.
- C9 If plastic shrinkage of the concrete is observed due to rapid drying or other conditions, apply a single spray coat of aliphatic alcohol evaporation retardant ("MasterKure 111CF-Confilm" by Master Builders or equal approved) immediately after screeding. Note: "Confilm" is not a curing Slurry to lubricate concrete pump lines shall not be used in

any structural member.

CONSTRUCTION TOLERANCES

CT1 Unless noted otherwise, standard construction tolerances

Shall be based on the requirements of AS 3600 and AS 4100.

- CT2 Any construction tolerances noted in the drawings or Specification are to be considered in conjunction with the Standard tolerances as indicated above unless specifically noted as an exception to, or in lieu of a standard tolerance
- CT3 Attention is drawn to the fact that tolerances of the existing building are unknown and a site investigation to prove the setting out of members is required. Unless noted otherwise. new steel members are to be set out to align with the centroids and centrelines of existing structural elements.

STRUCTURAL STEEL

- S1 All workmanship & materials to be in accordance with AS 4100 and the steel specification. All other materials (such as bolts, washers, straps and ties) which are not specifically indicated on the drawings but required in construction shall be provided at the builders expense. All section sizes have been based on the OneSteel catalogue.
- Steel shall have the following minimum values of yield stress (fy) UNO. Hot rolled sections to AS/NZS 3679.1: Grade 300 plus Hot rolled plates to AS/NZS 3678; Grade 350 Welded I sections to AS/NZS 3679.2: Grade 300 Hollow sections to AS 1163: Grade C350 Circular Hollow sections to AS 1163: Grade C350
- At least three weeks prior to the commencement of fabrication submit two copies of shop detail drawings for
- S4 No steelwork shall be fabricated until all review comments on the workshop drawings have been resolved to the Engineers satisfaction.

examination by the Engineer.

- S5 All dimensions and levels (including H.D. Bolts as constructed) affecting new steelwork shall be checked on site and incorporated in workshop drawings.
- S6 The Builder shall be responsible for providing temporary bracing until the structure is complete.
- S7 Bolts types and designations shall be as follows:

4.6/S Commercial bolts to as AS/NZS 1111, snug tightened. 8.8/S High strength structural bolts, nuts and hardened 8.8/TB.-High strength structural bolts as above, fully tensioned to AS 4100 in a bearing type joint. 8.8/TF. High strength structural bolts as above, fully tensioned to AS 4100 in a friction type joint with contact surfaces left uncoated.

Where 8.8 Bolts are shown as torqued (TB or TF) they shall be assembled in accordance with AS 4100 using "coronet" load indicating washers.

- All bolts shall be M20 8.8/S U.N.O. No steel to steel connection shall have less than 2M16 8.8/S bolts U.N.O.
- S9 M12 and smaller bolts to be grade 4.6/S. M16 and larger bolts to be high strength snug tightened grade 8.8/S unless shown otherwise.
- S10 All holding down bolts shall be commercial bolts or be made from bars with minimum yield stress 250 MPa with threads to AS 1275 unless shown otherwise.
- S11 All plates shall be 10mm thick U.N.O.
- S12 All reactions, where shown, are in kN Ultimate limit state U.N.O.
- S13 The ends of all hollow sections shall be sealed with 3mm end plates U.N.O.
- S14 Unless specified otherwise corrosion protection shall be as

(a) Internal steelwork generally: Primed and Painted (b) External steelwork: Hot dip Galvanized

(c) Members embedded in either leaf or the cavity of external masonry walls to be hot dip galvanised in accordance with AS/NZS 4680. Minimum coating to be 500 g/m2.

(d) Bolts, nuts and washers to be supplied as hot dip galvanised by the bolt manufacturer. After tightening, exposed faces of bolts, nuts and washers shall be prepared and coated as specified, or as for adjacent steelwork.

- (e) Holding down bolts to be hot dipped galvanised.
- S15 In addition to the finish specified, steelwork in contact with the ground is to be coated with a Interzone 954 HS or equivalent approved, to a minimum thickness of 0.4mm.
- S16 For galvanised coatings, repair field damage and site welds with two coats of a two pack epoxy polyamide zinc-rich paint containing a minimum of 92% zinc dust in the dry film which shall be built up to 75 micrometres minimum.
- S17 For paint coatings, repair field damage and site welds using the same parent coating system. Successive paint layers shall be abraded to a feathered edge of approximately 25mm width per layer. Mechanically grind surfaces to achieve smooth and bright metal comparable to class 2.5. Paint reinstatement shall fully cover the feathered part of the same layer.
- S18 All sealed hollow sections to be galvanised shall have vent holes as per the galvaniser's requirement, to the Engineers approval. These are to be shown on the workshop drawings.
- S19 Grout under baseplates to be high strength cementitious nonshrink (Masterflow 870 by Master Builders or equal approved) unless shown otherwise.

- S20 Cold formed steel structures shall conform to AS/NZS 4600.
- S21 Cold formed sections such as purlins and girts are to be roll formed from zinc coated high strength "zinc hi-ten" steel strip, conforming to AS 1397 - G450 - Z450 (450 MPa minimum yield stress, 450g/m2 minimum coating mass). Sections are to have dimensions equivalent to those shown in the BHP catalogue.
- S22 Purlin cleats are to be in accordance with the manufacturer's standard details except where the top flange of the purlin is between 250mm and 750mm above supporting steelwork 75 x 75 x 8 angle cleats shall be used. Purlins shall be fixed using approved flanged bolts and washers.
- S23 Bridging to be in accordance with the manufacturer's standard
- S24 Ceiling systems, ductwork etc. to be suspended from purlins should be fixed with hook bolts through the purlin web. The flanges of the purlins or girts shall not be holed.
- S25 Provide 50 x 50 x 3 galvanised angle trimmers to carry sheeting as required at purlin and girt ends. Fix with one No. 14 Teks screw per purlin/girt. This will generally require two angles on hips and valleys, one angle on gables and one vertical angle at corners.
- S26 Contractor shall provide minimum 100mm concrete cover around all steel members or steel components adjacent to and exposed to soil. Minimum reinforcement in cover concrete shall be N12 @ 300 EW.
- S27 Abbreviations. C.F.W. - Continuous fillet weld F.S.B.W. - Full strength butt weld P.P.B.W. - Partial penetration butt weld
- S28 Majority of existing structure is encased in concrete. Any breaking out or scabbling of concrete for testing or new connections to be made good using Renderoc HB70 or similar approved product.
- S29 Match fitting of all steelwork in shop is required to ensure correct alignment and fit up.
- S30 Match fitting of all steelwork in shop is required to ensure correct alignment and fit up.
- Welds on external steelwork to be finished to architects
- S32 8.8 grade bolt, nut and washer assemblies are to comply with AS 1252. Prior to bolts being installed on site the contractor shall provide a manufacturers test certificate and a certificate of compliance with AS 1252 from a NATA accredited testing
- S33 In all cases where the wall thickness of SHS or RHS is less than 6mm and to which a fin plate is welded, provide a 6mm plate stiffener, width to be 15mm less than face of section, welded all around.

laboratory to the engineer for review.

- S34 The ends of all hollow sections shall be sealed with 6mm end plates U.N.O.
- S35 TF interfaces between mating plates (and steel surface directly under washers) to have minimum slip factor of 0.45 to be verified through testing.
- S36 All mating surfaces to achieve full contact bearing in accordance with AS 4100
- S37 Open sections on their sides with flanges vertically up shall have 10mm diameter drain holes at quarter points in length.
- S38 Where noted slotted holes should have bolts with lock nuts.
- S39 Contractor and detailer to allow for all shims, plates, washers etc to facilitate adjustment to suit the tolerances as a result of the contractors method of construction.
- The contractor shall be responsible for the provision of sufficient temporary supports, props, ties or bracing as required to efficiently deal with wind or other eccentric loading experienced by the structure during their chosen erection sequence.
- Not all structural steel is necessarily shown on the structural drawings. Refer also to the architect's drawings.
- We acknowledge that not every detail has been drawn in either the architectural or structural documents. Tenderers are to allow for the cost of the structural steel shop detailer to establish or resolve details in close liaison with the architect and structural engineer. A response from the structural steel sub-contractor that they we unaware of the complication of detailing / fabrication required being unexpectably high, even though they viewed the tender documents will not be accepted. Refer to the specification regarding the obligations of detailing non-typical connections, secondary attachments

STRUCTURAL STEEL WELDING

SW1 All welding to be Structural Purpose (SP) in accordance with AS/NZS 1554. The weld consumable strength shall be as

Nominal yield strength of steel to be welded	Norminal tensile strength of weld metal, <i>f</i> uw (MPa)	
All steel with grade ≤ 300 MPa	430	
All steel with 300 < Grade ≤ 450 MPa	490	
Quench & tempered steel to Grade 690 MPa	760	

- SW2 Welds to be 6mm continuous fillet welds unless otherwise noted. Butt welds are to be complete penetration butt welds as defined in AS/NZS 1554 U.N.O.
- SW3 Non destructive testing of welds shall be carried out in accordance with the specification.
- SW4 Contractor is responsible for welding joint preparations and welding procedures, including (but not limited to): Welding procedures, required root openings, root face dimensions, groove angles, backing bars, copes, surface roughness values, and welding tapers of unequal parts.
- SW5 Sequences and procedures of welding shall minimize the effect of shrinkage and residual stresses, and maintain erection
- SW6 All groove and butt welds shall be complete joint penetration Welds, unless noted otherwise. All partial penetration groove Weld sizes shown on the drawings refer to effective throat
- SW7 All complete penetration welds shall be ultrasonically tested, except where plate thickness is less than 6mm the welds shall be magnetic particle tested.
- SW8 Fillet weld sizes shown on the structural drawings are minimum sizes. Increase weld size to aws minimum sizes, based on plate thicknesses. Minimum weld size (excluding seal welds) to be 6mm.
- SW9 Fillet weld lengths shown on the drawings are the net effective length required. Where length of weld is not shown it shall be full length of joint, member or plate.
- SW10 All steel permanently exposed to the weather that is not hotdipped galvanized inside and out shall have additional seal welding to protect members and connections from moisture infiltration. Show seal welding on shop drawings for review by the architect/engineer.
- SW11 Protective coating systems shall be applied in accordance with the schedule in the specification unless noted otherwise
- on the contract drawings. SW12 Site welding shall only be carried out with prior written
- consent of Arup structural engineer. SW13 All steel plate subject to through plate tension provide Z15

CRANE LOADS

CL1 Weights of crane components are based on a 30m span crane from Munck.

> Trolley/crab weight = 4565lbs = 20kN Crane weight = 53845lbs = 238kN Block and hook weight = 1000kg = 10kN

Crane type assumed "Class 3" = Heavy engineering shop cranes, heavy duty warehouse cranes.

> 22/12/17 DP ZL JD Issued for Information Chkd

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LACHLAN'S LINE TEMPORARY PORTAL SHED

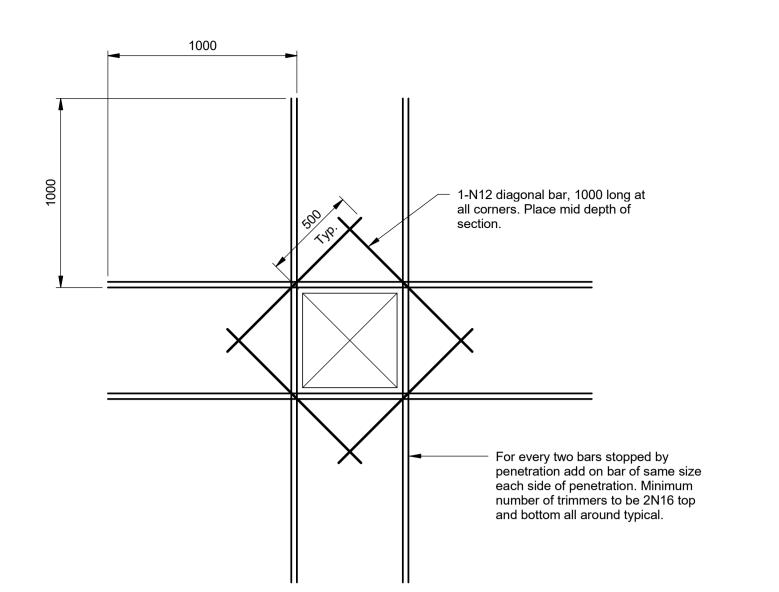
Drawing Title **General Notes**

Scale at A1 N.T.S. Structural Suitability For Information Arup Job No 247451-89 P1

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S-0010



	Lap Length (mm)		
Bar Dia.	Bottom Reinft. Less than 300mm concrete below bar	Top Reinft. More than 300mm concrete below bar	
N10	400	500	
N12	500	600	
N16	600	800	
N20	850	1100	
N24	1100	1400	
N28	1350	1750	
N32	1600	2100	
N36	1900	2400	
N40	2200	2800	

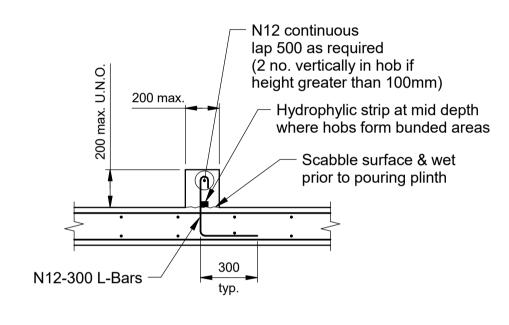
Beam & Slab Lap Length Schedule

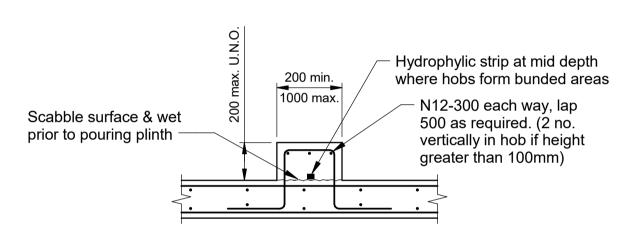
Lap Length Dia. (mm) N10, 12, 16 70 80 N24 96 N28 112 128 N32 N36 144 160

Cog Length Schedule

Note: Minimum extension lengths beyond bend noted

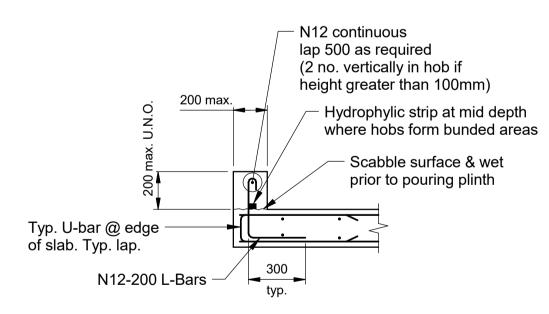
Standard Penetration

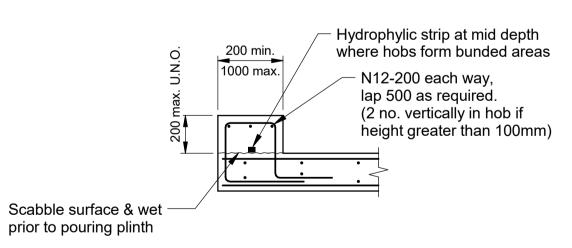




Typical Hob Detail 200mm Wide and Less

Typical Plinth Detail 200mm to 1000mm Wide

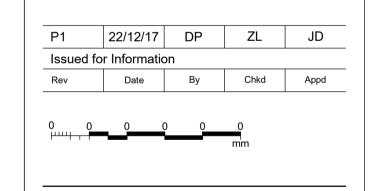




Typical Edge Hob Detail Under 200mm Wide

Do not scale

Typical Edge Plinth Detail 200mm to 1000mm Wide



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LACHLAN'S LINE TEMPORARY PORTAL SHED

Typical Details - Concrete

Scale at A1 N.T.S. Structural Suitability For Information Arup Job No 247451-89 P1 Name **S-0020**

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Typical Detail

100 < h ≤ 300 max

300 < h ≤ 600 max

 $600 < h \le 1000 \text{max}$

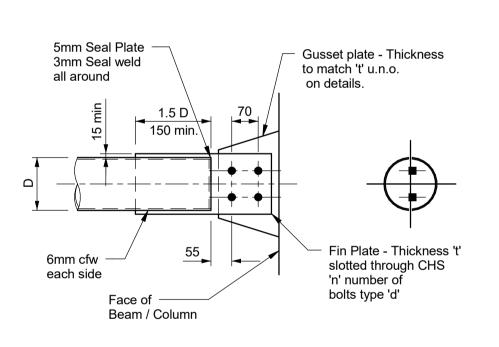
Roof Pitch < 20 degrees

Typical Purlin/Girt Connection Details

Nominal Purlin Section size (mm)	Fin Plate 'A'	Bolts 'D' (Grade 4.6/s u.n.o.)
100	8mm thick	2-M12
150	8mm thick	2-M12
200	8mm thick	2-M12
250	8mm thick	2-M12
300	12mm thick	2-M16
350	12mm thick	2-M16

Note:- 1. Details of bolt holes and laps to purlins and girts to be in accordance with manufactuers recommendations.

- 2. Details of bridging to purlins and girts to be in accordance with manufactuers recommendations.
- 3. At concrete connection provide 16mm base plate with 2M16 HIT-V anchors. Drill and epoxy to 125mm embedment with HIT-HY 200



Type SP-B / t / n-d

(2 Rows of Bolts)

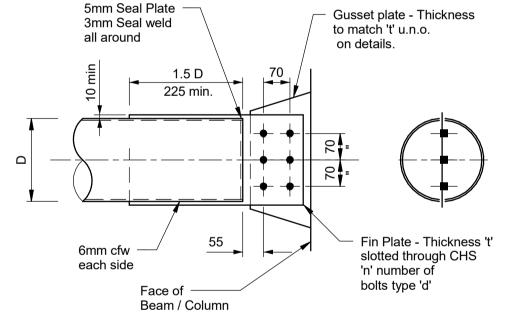
(Typical for CHS, RHS and SHS Sections)

where n = number of bolts

Do not scale

d = diameter of bolts

t = thickness of cleat pl

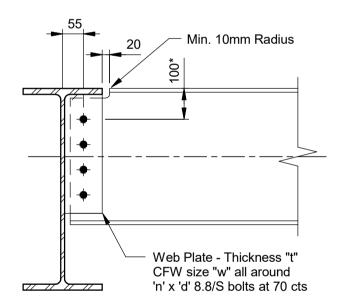


Type SP-C / t / n-d

(Typical for CHS, RHS and SHS Sections) where n = number of bolts d = diameter of bolts t = thickness of cleat pl

Slotted Plate

Typical Bracing End Connection Details (indicative only) Scale: 1: 10

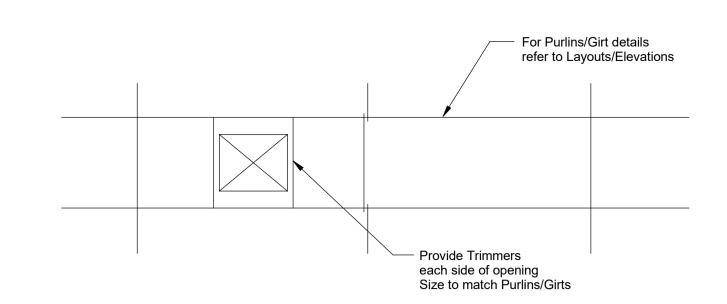


Beam to Beam

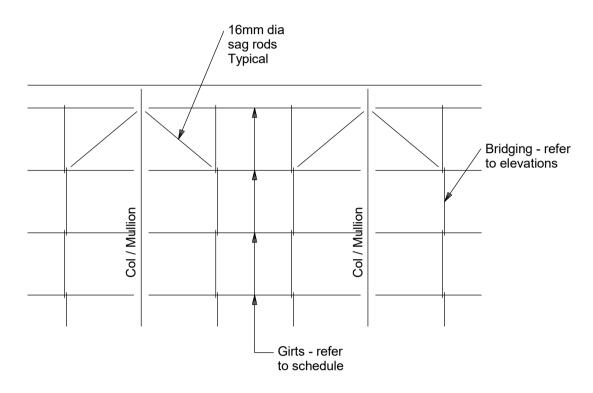
Web Side Plate - Type WP-A / t / n-d / w**

- * 70mm for members <250mm deep ** Welds 6 CFW UNO

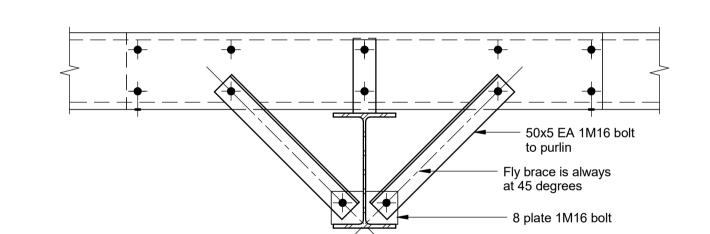
Typical Beam Connection Details (indicative only)



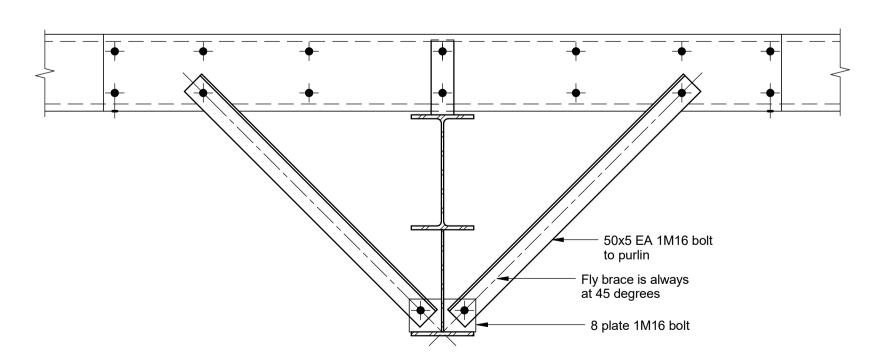
Typical Purlin/Girt Trimmer Detail At Roof/Wall Penetrations Typical Girt Details



Typical Girt Sag Rod Detail 1:100



Typical 'C' Purlin Fly Brace Detail



Typical 'C' Purlin Fly Brace Detail at Rafter End

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LACHLAN'S LINE TEMPORARY PORTAL SHED

Typical Details - Steel Sheet 1

Scale at A1 1: 10 Role Structural Suitability For Information Arup Job No 247451-89 P1

Name **S-0025**

