

West Culburra Subdivision Development
Transport and Accessibility Impact Assessment
Addendum Report

transportation planning, design and delivery



# West Culburra Subdivision Development

# Transport and Accessibility Impact Assessment

# Addendum Report

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## 1. Introduction

## 1.1 Background

The West Culburra development involves a mixed use subdivision development over approximately 110 hectares (ha) on land bounded to the north by the Crookhaven River, Lake Woollumboola and the existing urban area of Culburra to the east, Jervis Bay National Park to the south and Coonamia Road to the west.

A major project application (no. 09\_0088) was lodged with the NSW Department of Planning and Infrastructure (DoPI) in April 2010 seeking approval for the Concept Plan under Part 3A of the Environmental Planning and Assessment Act 1979.

GTA Consultants was commissioned by Reality Realizations Pty Ltd in May 2012 to undertake a Transport and Accessibility Impact Assessment for the proposed West Culburra subdivision development and in particular to address Section 5 (Traffic and Access) of the Director-General's Environmental Assessment Requirements (DGR's) dated 27 May 2010.

The exhibition of the Environmental Assessment (EA) for the project ended on 07 June 2013. As outlined in correspondence from the Department of Planning and Infrastructure (DoPI) dated 02 July 2013, the application for the project is due to be transitioned to State Significant Development (SSD) and as such a Response to Submissions is required by 01 October 2013.

GTA Consultants was commissioned by Reality Realizations in August 2013 to address the submission comments related to transport and accessibility made by Roads and Maritime Services (RMS) and to prepare an addendum report documenting our findings and recommendations.

Table 1.1 lists the comments made by RMS and the corresponding sections of the report where these are addressed. A copy of the correspondence dated 07 June 2013 is contained in vii.

Table 1.1: RMS Submission Comments and Relevant Report Sections

Roads and Maritime Services (letter dated 07 June 2013 - Appendix A)	Addressed in:
RMS notes the traffic generation resulting from the development of the site is estimate to be 573 vehicle movements in the peak hour as per Section 7.1.1 of GTA Consultant's Traffic Impact Assessment.	Section 2
RMS has undertaken further analysis using SIDRA to assess the likely impacts of full development of the subject site on the intersection of the Princes Highway and Kalandar Street. This assessment, based on the Friday AM 120 <sup>th</sup> highest hour, indicated the Average Delay would increase from the current 86.4 seconds for all vehicles to 145.2 seconds for all vehicles. The analysis indicated that the Degree of Saturation would increase from the current 1.049 for all vehicles to 1.175. The long term impact of the development has not been assessed.	Section 3
The above analysis indicated that the subject development will likely have a very significant on the intersection of the Princes Highway and Kalandar Street. The proponent should identify suitable infrastructure required to ameliorate these impacts on the network as a result of the subject development. These should be submitted to the Department and forwarded to RMS for comment.	



Roads and Maritime Services (letter dated 07 June 2013 - Appendix A)	Addressed in:
The proponent should carry out further traffic analysis based on full development of the site. The analysis should include 10 year projected traffic volumes including background traffic growth and likely traffic volumes with and without the development. The impacts on the network of full development of the site should be assessed prior to determination of the project application for concept approval.	Section 3.2
RMS notes there are a number of signalised intersections on the Princes Highway north of Kalandar Street. For accurate representation of the likely impacts of subject development on the highway, the proponent should undertake corridor modelling with the use of a program such as LinSig to include the coordinated nature of intersections at this location in the traffic analysis.	Section 3.4
RMS is responsible for all permanent speed zoning in NSW regardless of the classification of the road. RMS objects to the suggested speed zone change on Culburra Road. Speed zones should be intuitive to the driver, with the road environment and adjacent land uses effectively informing the driver of the speed zone. On this basis, RMS considers that the existing 100km/hr speed zone at this location is appropriate and should be maintained. Without appropriate changes to the road environment along the relevant section Culburra Road, a lower speed zone would likely lead to compliance and enforcement issues.	Section 4
Council should seek to reduce the number of access points to Culburra Road where possible. All access to stage 5 should be via the roundabout and the new collector road if possible. Research indicates that increased access density correlates highly with increased crash rate, and on this basis, accesses should be consolidated wherever possible.	Section 5
The proponent should ensure that that Safe Intersection Sight Distance is available at all new intersections in accordance with Austroads Guide to Road Design Part 4a: Unsignalised and Signalised Intersections Table 3.2, in both directions.	Section 6

Following the letter from RMS dated 07 June 2013, GTA Consultants liaised with RMS to seek clarification on the issues raised. Subsequent comments made by the RMS and the corresponding sections of the report where these are addressed are listed in Table 1.2.

Table 1.2: RMS Submission Comments and Relevant Report Sections

Roads and Maritime Services (letter dated 18 September 2013 - Appendix A)	Addressed in:
RMS advises that it currently has no upgrades planned for the intersection of the Princes Highway and Kalandar Street or north of Kinghorne Street in this vicinity.	Section 3.3
RMS accepts the distribution assignment used to analyse the proportion of traffic generated by the subject development leaving the Culburra area and travelling west to the Princes Highway. However, this analysis should include the full development including the Stage 6 Industrial lots.	Section 3
RMS supports the proposed assessment of the site at full development and a 10	Section 3.2



Roads and Maritime Services (letter dated 18 September 2013 - Appendix A)	Addressed in:
year projected scenario with and without the development. The proponent should identify suitable infrastructure to ameliorate any impacts on the network as result of the total development.	
RMS notes the distance between the intersection of the Princes Highway and Kalandar Street and the nearest signalised intersection at Plunkett Street, Nowra. Given this, further corridor modelling using LinSig is not deemed to be necessary.	Section 3.4
RMS generally does not support the construction of roundabouts within speed zoned above 80km/hr. The proponent should determine an appropriate junction treatment at the intersection of the Princes Highway and the new collector road to allow safe access into and out of the development within the existing speed zone. In this regard, refer to Figure 4.9: Warrants for turn treatments on the major road at Unsignalised intersections, in Austroads Guide to Road Design – Part 4A: Unsignalised and signalised Intersections. RMS is unlikely to support a speed zone reduction at this location	Section 4

### 1.2 References

In preparing this report, reference has been made to the following:

- West Culburra Subdivision development Transport and Accessibility Impact Assessment, GTA Consultants, 2013
- Austroads Guide to Road Design Part 4A: Unsignalised and Signalised Intersections, 2010
- Correspondence from RMS dated 07 June 2013, 12 September 2013 and 18 September 2013 (vii)
- Nowra CBD Master Plan, Arup, 2011
- Nowra CBD Transport Strategy, Eppell Olsen and Partners, 2003
- Other documents referenced in the report.



## 2. Traffic Generation

#### 2.1 Traffic Generation

Traffic generation estimates for the proposed development would usually be sourced from the Guide to Traffic Generating Developments (RMS, 2002). Application of the traffic generation rates contained in the RMS Guide results in an estimated traffic generation of 573 vehicle movements in a weekday peak hour as detailed in Section 7.1.1 of GTA Consultants Transport and Accessibility Impact Assessment.

Based on empirical traffic generation rates for the established areas of Culburra as supplied by Shoalhaven City Council's Traffic and Transport Unit, the residential component of the development (stages 2-5) is expected to generate up to 158 vehicle movements on the regional road network (west of Culburra) in the peak hour following full site development as summarised in Table 2.1.

Table 2.1: Estimated Development Traffic Generation – Residential Stages (Stages 2-5)

Peak Hour Scenario	Traffic Generation Rate (Shoalhaven City Council)	Proposed Residential Dwellings (Stages 2-5)	Traffic Generation Estimates (vehicles)
Friday AM	0.22		151
Friday PM	0.21	685	144
Saturday	0.23		158

Given the existing traffic generating characteristics of Culburra and the proportion of dedicated housing for the over 55 aged group proposed as part of the development, the adoption of the Culburra specific empirical traffic generation estimates is considered appropriate for use for intersection assessments. In correspondence dated 12 September 2013 RMS provided confirmation that the use of the empirical traffic generation rates for the intersection assessments is appropriate.

Further correspondence from RMS dated 18 September 2013 noted that traffic generated by the 28 industrial lots (stage 6) should be included in the intersection assessments. Table 2.2 summarises the estimated traffic generation of all development stages.

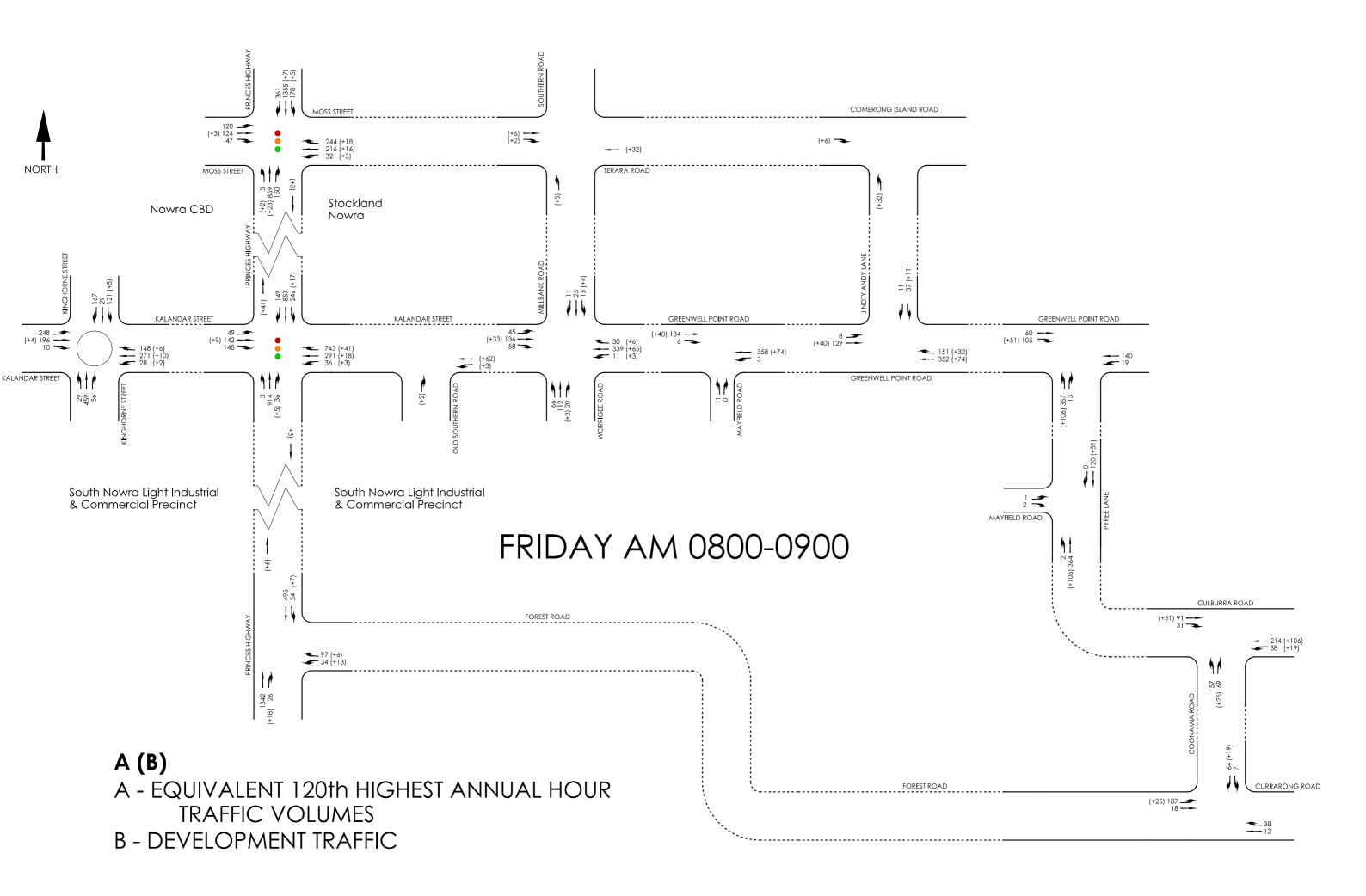
Table 2.2: Estimated Development Traffic Generation – All Development Stages

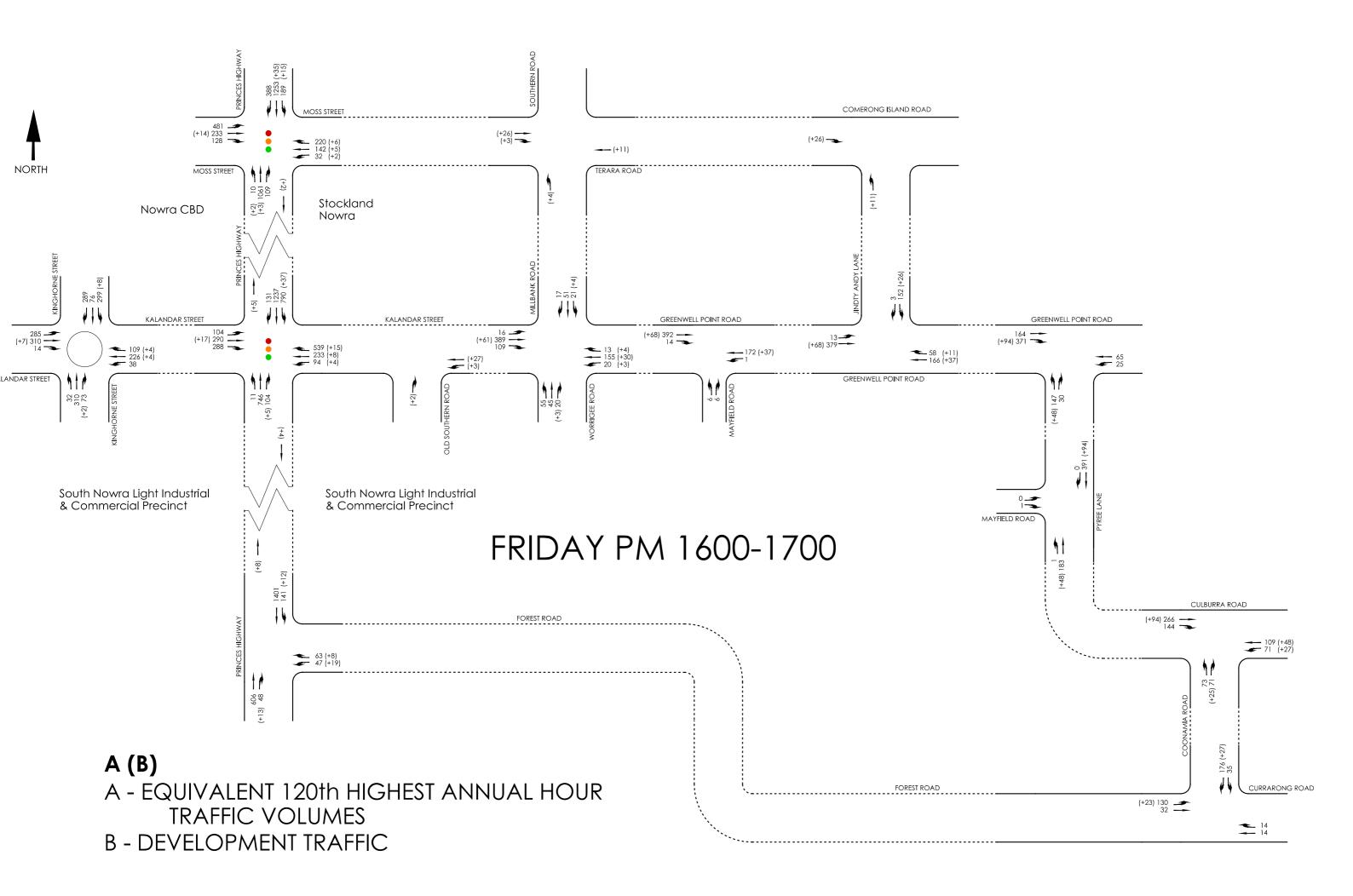
West Culburra Subdivision Development, Transport and Accessibility Impact Assessment,

Peak Hour Scenario	Traffic	Generation Estimates (vehicles)			
Peak Hour Scenario	Residential (Stages 2-5) Industrial (Stage 6)	Industrial (Stage 6)	Total		
Friday AM	151	48	199		
Friday PM	144	48	192		
Saturday	158	0	158		

As shown in Table 2.2, with the addition of the stage 6 industrial traffic, it is expected that the West Culburra subdivision development will generate 199, 192 and 158 vehicle movements on the regional road network (west of Culburra) during the respective Friday AM, Friday PM and Saturday peak hours on the regional road network (west of Culburra). It is anticipated that the stage 6 industrial lots will not generate any traffic during the Saturday peak hour (1200-1300).

Figure 2.1 and Figure 2.2 show the estimated increase in turning movements on the surrounding road network following full site development during the respective Friday AM and PM peak hours including traffic generated by the stage 6 industrial lots.







# 3. Traffic Impact

Assessment of the traffic impact of the proposed development on the relevant intersections was undertaken with SIDRA INTERSECTION<sup>1</sup> using 120<sup>th</sup> equivalent highest annual hour (HH) traffic volumes to account for the seasonal growth in traffic in the region. It is noted that the 120<sup>th</sup> HH represents peak hour volumes within the highest 1% of a year. Detailed results are contained in Appendix B.

## 3.1 Full Site Development (at completion)

Table 3.1 presents a summary of intersection operating conditions at the completion of full site development. Full results are contained in Appendix B. It is anticipated that the stage 6 industrial component of the development would not generate any traffic during the Saturday peak hour. As such, the results for the Saturday peak hour contained in Table 3.1 are unchanged from the future operating conditions presented in Table 7.11 of the Transport and Accessibility Impact Assessment prepared by GTA Consultants.

Table 3.1 shows that, under equivalent 120<sup>th</sup> HH traffic volumes with the addition of traffic generated by the development:

- the priority controlled intersections operate well with minimal delays and queues on all approaches during the three respective peak periods
- the Princes Highway intersections at Kalandar Street and Moss Street experience significant delays particularly during the Friday AM and Friday PM peak periods.
   However there is no significant change to the intersection Level of Service with the addition of development traffic.
- the addition of traffic generated by the Stage 6 industrial component has a negligible impact on the future operating conditions of the surveyed intersections.

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Table 3.1: Future Operating Conditions – Full Site Development (Equivalent 120th HH plus Development Traffic)

Intersection	Peak	Degree of Saturation (DOS)	Delay (sec)	95th Percentile Queue (m)	Level of Service (LOS)
	Friday AM	0.235	6.3	6	NA
Culburra Road/ Coonamia Road	Friday PM	0.196	6.0	6	NA
Coonamia Road	Saturday	0.177	6.4	5	NA
	Friday AM	0.255	0.9	6	NA
Culburra Road/ Mayfield Road	Friday PM	0.266	1.2	16	NA
ayo.aoaa	Saturday 0.173	1.0	9	NA	
	Friday AM	0.367	10.1	14	NA
Greenwell Point Road/ Pyree Lane	Friday PM	0.728	12.7	75	NA
	Saturday	0.307	8.4	10	NA
Greenwell Point	Friday AM	0.297	3.7	10	NA
Road/ Jindy Andy	Friday PM	0.278	4.5	8	NA
Lane	Saturday	0.218	3.8	6	NA
Greenwell Point	Friday AM	0.241	2.3	13	NA
Road/ Mayfield	Friday PM	0.274	2.8	27	NA
Road	Saturday	0.183	2.5	18	NA
Greenwell Point	Friday AM	0.464	7.7	18	NA
Road/ Millbank Road/ Worrigee	Friday PM	0.283	6.3	8	NA
Road	Saturday	0.163	5.6	4	NA
	Friday AM	1.082	100.7	465	F
Princes Highway/ Kalandar Street	Friday PM	1.143	130.2	502	F
	Saturday	0.983	67.6	415	E
Coonamia Road/	Friday AM	0.132	12.2	4	NA
Currarong Road/	Friday PM	0.293	12.4	10	NA
Forest Road	Saturday	0.238	12.4	8	NA
	Friday AM	0.741	15.8	73	В
Kalandar Street/ Kinghorne Street	Friday PM	0.789	16.3	83	В
3	Saturday	0.377	9.9	18	А
	Friday AM	0.739	2.3	6	NA
Princes Highway/ Forest Road	Friday PM	0.766	6.4	22	NA
<u> </u>	Saturday	0.598	3.8	15	NA
	Friday AM	1.069	110.1	477	F
Princes Highway/ Moss Street	Friday PM	1.243	224.2	798	F
	Saturday	0.873	48.3	199	D



## 3.2 10 Year Modelling Scenarios

## 3.2.1 Background Traffic Growth

The Review of Environmental Factors (REF) for the Princes Highway upgrade at South Nowra was completed in November 2009 and estimated a 2.5% linear growth rate in traffic volumes on this section of the Princes Highway up to 2028.

The application of a 2.5% linear growth rate for the Princes Highway for an equivalent 120<sup>th</sup> HH traffic scenario is however considered to be unrealistic. It is unlikely that there is sufficient capacity available on the Princes Highway to realistically accommodate such growth.

The West Culburra Subdivision development is anticipated to be the largest source of traffic growth in Culburra and most likely in the area between Culburra and the Princes Highway. As such, no growth rate has been applied to traffic east of the Princes Highway for the full site development + 10 years scenario. A 1% linear growth has been applied to traffic west of the Princes Highway.

In summary, the following growth rates have been applied to background traffic for the full site development + 10 years scenario:

- Princes Highway 1% linear growth
- West of Princes Highway 1% linear growth
- East of Princes Highway 2% linear growth.

This application of these background traffic growth rates affects the following four intersections included in GTA Consultants Transport and Accessibility Impact Assessment:

- Princes Highway/ Kalandar Street
- Kalandar Street/ Kinghorne Street
- Princes Highway/ Forest Road
- Princes Highway/Moss Street.

The remaining seven intersections are not subject to background traffic growth and as such 10 year modelling is unnecessary.

### 3.2.2 Without Development Traffic

West Culburra Subdivision Development, Transport and Accessibility Impact Assessment,

Table 3.2 presents a summary of intersection operating conditions under 120<sup>th</sup> HH traffic volumes including 10 year background traffic growth <u>without development traffic</u>. Detailed results are contained in Appendix B.



Table 3.2: 10 Year Operating Conditions (Equivalent 120th HH – No Development Traffic)

Intersection	Peak	Degree of Saturation (DOS)	Delay (sec)	95th Percentile Queue (m)	Level of Service (LOS)
	Friday AM	1.048	89.2	444	F
Princes Highway/ Kalandar Street	Friday PM	1.147	160.0	706	F
naianaa sii ssi	Saturday	0.982	73.5	496	F
	Friday AM	0.882	22.6	126	В
Kalandar Street/ Kinghorne Street	Friday PM	0.910	22.9	147	В
g	Saturday	0.421	10.1	21	А
	Friday AM	0.813	2.0	6	NA
Princes Highway/ Forest Road	Friday PM	0.873	6.8	27	NA
, erest nedd	Saturday	0.657	3.4	16	NA
	Friday AM	1.020	92.0	466	F
Princes Highway/ Moss Street	Friday PM	1.274	237.3	976	F
10000 011 001	Saturday	0.846	49.1	225	D

### 3.2.3 With Development Traffic

Table 3.3 presents a summary of intersection operating conditions under 120<sup>th</sup> HH traffic volumes including 10 year background traffic growth <u>with development traffic</u>. Detailed results are contained in Appendix B.

Table 3.3: 10 Year Operating Conditions (Equivalent 120th HH – plus Development Traffic)

Intersection	Peak	Degree of Saturation (DOS)	Delay (sec)	95th Percentile Queue (m)	Level of Service (LOS)
	Friday AM	1.085	102.8	512	F
Princes Highway/ Kalandar Street	Friday PM	1.179	168.4	706	F
naianaa sussi	Saturday	0.985	78.4	496	F
	Friday AM	0.896	23.9	135	В
Kalandar Street/ Kinghorne Street	Friday PM	0.926	24.6	161	В
·····g······a	Saturday	0.424	10.1	21	А
	Friday AM	0.813	2.3	7	NA
Princes Highway/ Forest Road	Friday PM	1.025	11.4	50	NA
	Saturday	0.657	4.2	19	NA
	Friday AM	1.133	134.2	635	F
Princes Highway/ Moss Street	Friday PM	1.400	312.1	1108	F
	Saturday	0.958	55.1	257	D

A comparison of intersection performance results shown in Table 3.2 and Table 3.3 indicates that the additional traffic generated by the development would result in a minor increase in vehicle delay in a 10 year scenario at the surveyed intersections. The intersection's Level of Service under a 10 year scenario are unchanged with or without the addition of development traffic.



# 3.3 Princes Highway/ Kalandar Street Intersection

The RMS raised specific concerns of the impact of development traffic on the Princes Highway/ Kalandar Street intersection. This Section discusses the impact of development traffic on this intersection in detail.

### 3.3.1 Existing Operation

Considering the 10 year modelling results presented above, Table 3.4 presents detailed results of the existing operation of the Princes Highway/Kalandar Street intersection using 120<sup>th</sup> HH traffic volumes.

Table 3.4: Princes Highway/ Kalandar Street Intersection – Existing Operating Conditions (Equivalent 120th Highest Annual Hour)

Peak	Leg	Degree of Saturation (DOS)	Delay (sec)	95th Percentile Queue (m)	Level of Service (LOS)
	Princes Highway (south)	1.049	110.6	363	F
	Kalandar Street (east)	1.039	122.6	403	F
Friday AM	Princes Highway (north)	1.042	43.1	181	D
	Kalandar Street (west)	0.700	63.7	93	E
	All Vehicles	1.049	86.4	403	F
	Princes Highway (south)	1.038	108.9	281	F
	Kalandar Street (east)	1.065	141.9	335	F
Friday PM	Princes Highway (north)	109.9	109.9	558	F
	Kalandar Street (west)	1.101	135.2	389	F
	All Vehicles	1.101	119.6	558	F
Saturday	Princes Highway (south)	0.966	64.8	230	E
	Kalandar Street (east)	0.968	90.3	248	F
	Princes Highway (north)	0.961	50.2	389	D
	Kalandar Street (west)	0.771	67.3	105	E
	All Vehicles	0.968	63.3	389	E

On the basis of this assessment, under equivalent 120th HH traffic volumes the Princes Highway/Kalandar Street intersection is already overcapacity during the existing Friday AM and PM peak periods. It is clear that the intersection experiences significant delays during the three peak periods assessed. Significant queues are experienced on the southern and eastern approaches during the Friday AM peak period and on all approaches during the Friday PM peak period.

### 3.3.2 Full Site Development

#### **RMS** Analysis

RMS undertook additional SIDRA analysis of the Princes Highway/ Kalandar Street intersection to assess the impact of full site development during the Friday AM 120<sup>th</sup> HH. This analysis indicated an increase in Average Delay from the current 86.4 seconds for all vehicles to 145.2 seconds for all vehicles. In correspondence dated 12 September 2013 (vii), RMS confirmed that this additional



analysis utilised the RMS Guide traffic generation estimate of 573 vehicle movements rather than the Culburra specific empirical traffic generation estimate of 158 vehicle movements.

As mentioned in Section 2, correspondence from RMS dated 12 September 2013 (vii), confirmed that use of the empirical traffic generation rates for the intersection assessments is appropriate.

#### GTA Consultants Analysis

A summary of the total traffic flows at the intersection and increase of development traffic at the intersection for the three peak periods is shown in Table 3.5.

Table 3.5: Princes Highway/Kalandar Street Intersection – Traffic Flow Summary

Peak	Equivalent 120th HH Intersection	Additional Development Traffic Flow at Intersection			
Period Flow (vehicles – all movements)		Vehicles (all movements)	Increase on 120th HH Flows		
Friday AM	3610	92	2.5%		
Friday PM	4567	86	1.9%		
Saturday	4089	41	1.0%		

As shown in Table 3.5, the addition of development traffic represents a negligible increase of traffic at the intersection comprising 2.5%, 1.9% and 1.0% of the 120<sup>th</sup> HH traffic flows for the Friday AM, PM and Saturday peak periods respectively.

Table 3.6 presents detailed results of intersection operating conditions under 120<sup>th</sup> HH traffic volumes following full site development. Detailed results of the SIDRA analysis are contained in Appendix B.

Table 3.6: Princes Highway/ Kalandar Street Intersection – Future Operating Conditions with Full Site Development (Equivalent 120th Highest Annual Hour)

Peak	Leg	Degree of Saturation (DOS)	Delay (sec)	95th Percentile Queue (m)	Level of Service (LOS)
	Princes Highway (south)	1.082	134.4	395	F
	Kalandar Street (east)	1.072	147.0	465	F
Friday AM	Princes Highway (north)	1.042	43.6	185	D
	Kalandar Street (west)	0.719	64.7	96	E
	All Vehicles	1.082	100.7	465	F
	Princes Highway (south)	1.114	143.2	300	F
	Kalandar Street (east)	1.124	185.2	378	F
Friday PM	Princes Highway (north)	1.081	95.7	502	F
	Kalandar Street (west)	1.143	151.8	412	F
	All Vehicles	1.143	130.2	502	F
	Princes Highway (south)	0.983	69.4	240	E
Saturday	Kalandar Street (east)	0.968	90.0	257	F
	Princes Highway (north)	0.981	56.7	415	E
	Kalandar Street (west)	0.781	67.7	107	E
	All Vehicles	0.983	67.6	415	E

As shown in Table 3.6, the addition of development traffic to the intersection results in negligible increase in delay and does not a have a significant impact on the intersection Level of Service (LOS).



### 3.3.3 Full Site Development + 10 Years

Future year SIDRA modelling was undertaken for a scenario 10 years after full site development.

Table 3.7summarises the presents a summary of intersection operating conditions under 120<sup>th</sup> HH traffic volumes including 10 year background traffic growth <u>without</u> development traffic while Table 3.8 presents a summary of intersection operating conditions <u>with</u> development traffic. Detailed results of the SIDRA analysis are contained in Appendix B.

Table 3.7: Princes Highway/ Kalandar Street Intersection – Future Operating Conditions (Equivalent 120<sup>th</sup> HH – without Development Traffic)

Peak	Leg	Degree of Saturation (DOS)	Delay (sec)	95th Percentile Queue (m)	Level of Service (LOS)
	Princes Highway (south)	1.032	102.9	412	F
	Kalandar Street (east)	1.048	135.8	444	F
Friday AM	Princes Highway (north)	1.019	44.3	220	D
	Kalandar Street (west)	0.826	79.4	117	F
	All Vehicles	1.048	89.2	444	F
	Princes Highway (south)	1.142	187.3	425	F
	Kalandar Street (east)	1.147	213.6	427	F
Friday PM	Princes Highway (north)	1.141	127.4	706	F
	Kalandar Street (west)	1.136	163.6	498	F
	All Vehicles	1.147	160.0	706	F
Saturday	Princes Highway (south)	0.966	83.4	319	F
	Kalandar Street (east)	0.955	91.0	262	F
	Princes Highway (north)	0.982	58.5	496	E
	Kalandar Street (west)	0.907	86.2	139	F
	All Vehicles	0.982	73.5	496	F



Table 3.8: Princes Highway/ Kalandar Street Intersection – Future Operating Conditions (Equivalent 120th HH – 10 years plus Development Traffic)

Peak	Leg	Degree of Saturation (DOS)	Delay (sec)	95th Percentile Queue (m)	Level of Service (LOS)
	Princes Highway (south)	1.058	120.7	439	F
	Kalandar Street (east)	1.085	163.1	512	F
Friday AM	Princes Highway (north)	1.019	44.9	225	D
	Kalandar Street (west)	0.846	81.3	122	F
	All Vehicles	1.085	102.8	512	F
	Princes Highway (south)	1.170	202.1	425	F
	Kalandar Street (east)	1.148	214.4	443	F
Friday PM	Princes Highway (north)	1.141	130.4	706	F
	Kalandar Street (west)	1.179	189.5	560	F
	All Vehicles	1.179	168.4	706	F
Saturday	Princes Highway (south)	0.983	92.3	339	F
	Kalandar Street (east)	0.985	104.6	293	F
	Princes Highway (north)	0.982	58.2	496	E
	Kalandar Street (west)	0.918	86.7	143	F
	All Vehicles	0.985	78.4	496	F

A comparison of intersection performance results shown in Table 3.7 and Table 3.8 indicates that the additional traffic generated by the full development would result in relatively minor increase in delays. The intersection Level of Service is unchanged with the addition of traffic generated by the development over 10 year scenarios.

### 3.3.4 Traffic Impact Summary

A summary of intersection operating conditions under equivalent 120<sup>th</sup> HH for existing, full site development and 10 years after development is presented in Table 3.9.



Table 3.9: Princes Highway/ Kalandar Street - Intersection Operating Conditions Summary - All Vehicles (Equivalent 120th Highest Annual Hour)

Peak	Peak Scenario		Delay (sec)	95th Percentile Queue (m)	Level of Service (LOS)
	Existing	1.049	86.4	403	F
Friday ANA	Full Site Development	1.082	100.8	465	F
Friday AM	Existing + 10 Years	1.048	89.2	444	F
	Full Site Development + 10 Years	1.085	102.8	512	F
	Existing	1.101	119.6	558	F
Friday DM	Full Site Development	1.143	130.2	502	F
Friday PM	Existing + 10 Years	1.147	160.0	706	F
	Full Site Development + 10 Years	1.179	168.4	706	F
Saturday	Existing	0.968	63.3	389	E
	Full Site Development	0.983	67.6	415	E
	Existing + 10 Years	0.982	73.5	496	F
	Full Site Development + 10 Years	0.985	78.4	496	F

#### 3.3.5 Intersection Improvements

The RMS submission to DoPI recommended that suitable infrastructure be identified to ameliorate the impacts of traffic generated by the development at the intersection. As demonstrated by the SIDRA results above, the intersection currently operates overcapacity in the Friday AM and PM peak periods under 120<sup>th</sup> HH traffic volumes without the traffic generated by the development. Improvements are needed at the intersection irrespective of what development occurs at Culburra.

In correspondence dated 18 September 2012 RMS confirmed that it currently has no upgrades planned for the Princes Highway/ Kalandar Street intersection or along the Princes Highway corridor in Nowra north of Kinghorne Street.

Given this, GTA Consultants undertook a desktop investigation to identify potential improvements to increase capacity at the intersection. Under existing 120<sup>th</sup> HH traffic volumes, the following improvements were identified and incorporated into the SIDRA model to provide a significant improvement in intersection Level of Service (LOS D or better):

- Kalandar Street (eastern approach) provision of an additional, dedicated left-turn slip lane
- **Kalandar Street (western approach)** provision of a dedicated right turn lane and an additional, dedicated through-lane
- **Princes Highway (northern and southern approaches)** removal of the concrete medians and provision of an additional through-lane in each direction

West Culburra Subdivision Development, Transport and Accessibility Impact Assessment,

The layout of the Princes Highway/ Kalandar Street Intersection incorporating these changes is shown in Figure 3.1.



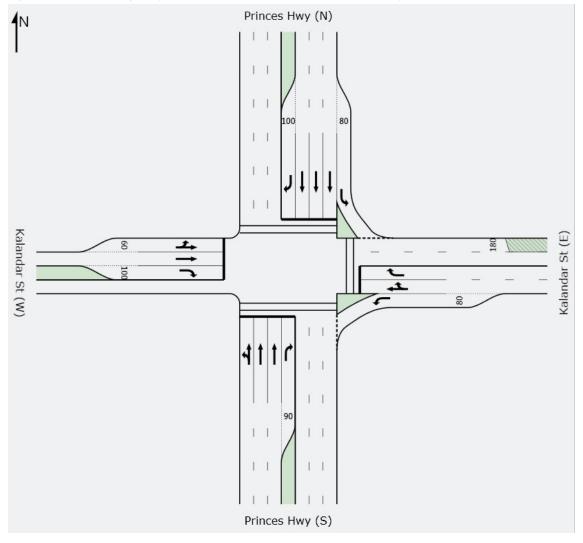


Figure 3.1: Princes Highway/ Kalandar Street Intersection - potential layout

Significant improvements are therefore required at the intersection to reduce delays and provide a LOS D or better under existing 120<sup>th</sup> HH equivalent traffic volumes. The most critical improvement required is increasing the capacity of the Princes Highway to three lanes in each direction. This increase in capacity would be required beyond the intersection approaches to ensure there is sufficient upstream and downstream capacity.

The capacity issues at the Princes Highway/ Kalandar Street intersection are part of a wider traffic congestion problem along the Princes Highway in Nowra and any improvements need to be considered in the broader context of traffic improvements in Nowra.

### 3.3.6 East Nowra Sub-Arterial (ENSA)

Whilst RMS has suggested that no road improvements are planned in the immediate future, an East Nowra Sub-Arterial (ENSA) road link has been proposed for over 10 years to provide a north-south road link east of the Princes Highway which connects Kalandar Street at Old Southern Road with North Street and Junction Street. The ENSA is intended to alleviate traffic congestion on Kalandar Street, the Princes Highway and on rural lanes east of Nowra. An indicative alignment of the ENSA is shown in Figure 3.2.



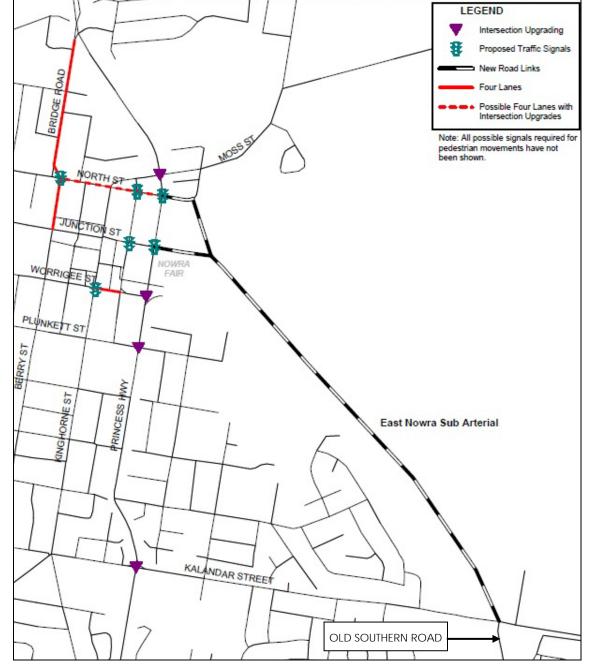


Figure 3.2: East Nowra Sub-Arterial (ENSA) Alignment

Source: Nowra CBD Transport Strategy, 2003 (Eppell Olsen and Partners)

It is anticipated that the ENSA would reduce traffic volumes on the Princes Highway through Nowra CBD and Kalandar Street and alleviate congestion at the Princes Highway/Kalandar Street intersection.

The Nowra CBD Master Plan (Arup, 2011) states that RMS and Shoalhaven City Council support the need for ENSA and the associated works required along the Princes Highway, however there is no funding commitment to build the road at this stage by either party.



Given the significant improvements required at the Princes Highway/ Kalandar Street intersection and along the Princes Highway in the vicinity of the intersection, the ENSA arguably represents an alternative which would have greater strategic benefit.

## 3.4 Princes Highway Corridor Modelling

The RMS' submission to DoPI noted the signalised intersections on the Princes Highway north of Kalandar Street and requested that corridor modelling be undertaken using LinSig software or similar.

The West Culburra Subdivision development is approximately 20km from Nowra. SIDRA modelling was undertaken for the two Princes Highway intersections in Nowra most likely to be utilised by development traffic; Moss Street and Kalandar Street. These intersections are approximately 1.7km apart and two signalised intersections (Worrigee Street and Plunkett Street) are located between them.

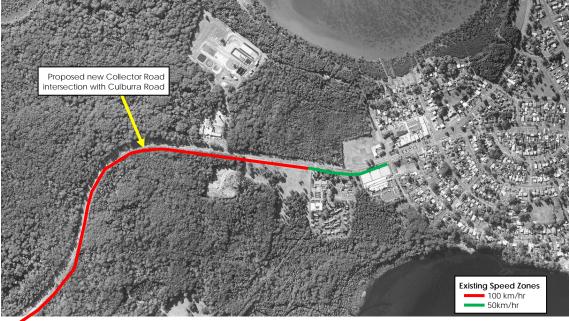
Following discussions with RMS it was confirmed in correspondence dated 18 September 2013 that corridor modelling was unnecessary. A copy of the correspondence is contained in Appendix A.



# 4. Culburra Road Speed Zoning

The main access to the development site was proposed via a roundabout on Culburra Road at the eastern end of new Collector Road. A 100km/hr speed zone is currently in place on Culburra Road in the vicinity of the proposed Collector Road intersection. A reduction in the speed zone to 50km/hr was proposed for all approaches to the proposed roundabout. Figure 4.1 shows the existing speed zoning in the vicinity of the development site and the indicative location of the proposed roundabout.





Background Image Source: NearMap

RMS is responsible for all permanent speed zoning in NSW regardless of the classification of the road. RMS does not support a reduction in the speed zone on Culburra Road and considers the existing 100km/hr speed zone on this section of Culburra Road to be appropriate.

On this basis, whilst GTA considers that the roundabout proposal is acceptable, an alternative junction treatment has been proposed at the intersection with further details contained in Section 5.



## Access to Culburra Road

A new Collector Road is proposed to provide access to Stages 3, 4 and 5 with two connections to Culburra Road. The eastern access will be the primary means of accessing Stages 3, 4 and 5. It is understood that the western access to the development will not be provided at this stage. The location of the western access is anticipated to be in the vicinity of the existing unsealed access track.

RMS generally does not support roundabouts within speed zones above 80km/hr and has stipulated that an appropriate junction treatment at the Princes Highway/ Collector Road should be proposed to allow safe access into and out of the development within the existing 100km/hr speed zone.

RMS has stipulated that Council seek to reduce the number of access points to Culburra Road and stipulated that all access to stage 6 (industrial) be via the Collector Road/Culburra Road intersection.

### 5.1 Warrants for Turn Treatments

Figure 5.1 shows the warrants for turn treatments for roads with a design speed of greater than or equal to 100km/hr taken from Austroads Guide to Road Design – Part 4A: Unsignalised and Signalised Intersections.

1 2 80 Furn Volume 'Q<sub>R</sub>' or 'Q<sub>L</sub>' (Veh/h) 60 40 CHR/(AUL or CHL) CHR(S)/ AUL(S) 20 BAR/BAL 0 0 200 600 800 400 1000 1200 Major Road Traffic Volume 'Q<sub>M</sub>' (Veh/h)

Figure 5.1: Warrants for turn treatments on the major road at unsignalised intersections (Design speed ≥ 100km/hr)

Source: Austroads Guide to Road Design - Part 4A: Unsignalised and Signalised Intersections, 2010 (Figure 4.9a, pg. 46)

West Culburra Subdivision Development, Transport and Accessibility Impact Assessment,

It is anticipated that until the western connection to Culburra Road is established, the eastern intersection of the new Collector Road and Culburra Road will be the sole vehicular access point to development land units 3, 4 and 5.

The traffic volume on Culburra Road 10 years after development is estimated to be in excess of 500 vehicles per hour (vph).



The y-axis 'Turn Volume' on Figure 5.1 refers to the major road turning volumes, i.e. vehicles turning from Culburra Road into the new Collector Road which are estimated to be in excess of 80 vph 10 years after full-site development.

#### 5.1.1 Geometric Requirements

Based on the estimated traffic volumes, Figure 5.1 indicates that for an unsignalised intersection a channelised T-junction with a full length right-turn treatment on the major road (CHR) is warranted with either an auxiliary left-turn lane (AUL) treatment or channelised left-turn lane (CHL) on the major road.

Based on Table 7.1 and Table 8.2 of Austroads Guide to Road Design Part 4A and the existing 100km/hr speed zone on Culburra Road, the major road left and right turn lanes are required to be a minimum of 40m long with a 30m taper. There are no numerical warrants for the provision of acceleration lanes.

#### 5.1.2 Schematic Layout

Figure 5.2 shows a schematic layout of a priority controlled intersection treatment at the Culburra Road/Collector Road (east) intersection with left and right turn lanes on Culburra Road.

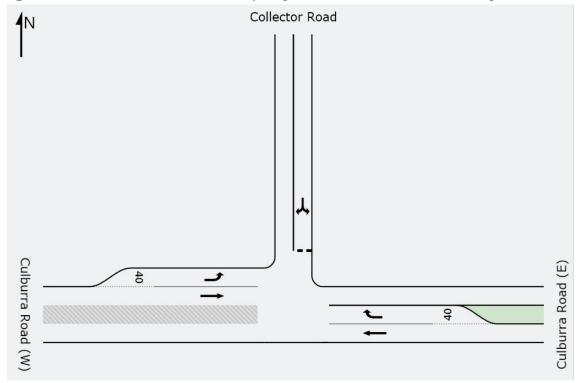


Figure 5.2: Culburra Road/Collector Road - priority controlled intersection schematic layout

## 5.2 Intersection Operation

West Culburra Subdivision Development, Transport and Accessibility Impact Assessment,

An assessment of the operation of the priority controlled intersection treatment was undertaken using SIDRA. This analysis indicates that the intersection would operate satisfactorily with minimal queues and delays on all approaches with the addition of development traffic.



The traffic flows used were as per the previous assessment undertaken of a roundabout treatment at the intersection, however additional traffic generated by the stage 6 industrial lots was added for the Friday AM and PM peak periods.

The analysis results are summarised in Table 5.1 with full results contained in Appendix B.

Table 5.1: Culburra Road/new Collector Road (east) - Intersection Operating Conditions Summary (Full Site Development)

Peak	Leg	Degree of Saturation (DOS)	Delay (sec)	95th Percentile Queue (m)	Level of Service (LOS)
	East	0.144	4.1	3	N/A
Friday AM	North	0.364	9.4	13	А
	West	0.111	2.1	0	N/A
Friday PM	East	0.316	9.7	11	N/A
	North	0.333	15.1	11	В
	West	0.190	0.4	0	N/A
Saturday	East	0.186	6.5	5	N/A
	North	0.219	8.8	6	А
	West	0.125	0.1	0	N/A

As shown in Table 5.1, this alternative priority controlled treatment at the intersection of Culburra Road and the new Collector Road intersection with left and right turn lanes on Culburra Road would operate will with minimal queues and delays on all approaches.



# 6. Sight Distance Assessment

The safe intersection sight distance (SISD) criteria are outlined in Table 3.2 of Austroads Guide to Road Design – Part 4A: Unsignalised and Signalised Intersections. Based on a 100km/hr design speed and a reaction time of 2.5 seconds, the Approach Sight Distance (ASD) for vehicles on Culburra Road is 262 metres. Based on a design speed of 50km/hr and a reaction time of 2.0 seconds, the ASD for vehicles on the new Collector Road is 55 metres.

An assessment of the location of the proposed Collector Road intersection with Culburra Road indicates that the required ASD as stipulated in Table 3.2 of Austroads Guide to Road Design – Part 4A can be achieved on all approaches.



## 7. Conclusion

Based on the analysis and discussions presented within this report, the following conclusions are made:

- i Based on empirical traffic generation rates for the established areas of Culburra as supplied by Shoalhaven City Council's Traffic and Transport Unit, the proposed development is expected to generate up to 199 vehicle movements on the regional road network (west of Culburra) in the peak hour following full site development (including traffic generated by the stage 6 industrial lots). The adoption of this estimate for use for intersection assessments is considered appropriate.
- ii SIDRA analysis of the Princes Highway/ Kalandar Street intersection indicates that significant delays are currently experienced during peak periods and that further capacity improvements are required, regardless of what development occurs at Culburra.
- iii The addition of development traffic at the Princes Highway/ Kalandar Street has a negligible impact on vehicle delay and does not affect the intersection Level of Service.
- iv RMS has confirmed that it currently has no upgrades planned for the Princes Highway/ Kalandar Street intersection or north of Kinghorne Street.
- v Significant improvements are required at the Princes Highway/ Kalandar Street intersection to reduce delays currently experienced under existing equivalent 120<sup>th</sup> HH traffic volumes. Increases in capacity upstream and downstream of this intersection would also be required to improve intersection performance.
- vi The East Nowra Sub-Arterial (ENSA) road link may be a viable alternative to upgrading the Princes Highway/Kalandar Street intersection and provide benefits at a regional scale by reducing traffic on the Princes Highway and Kalandar Street.
- vii RMS does not support the use of a roundabout within speed zones above 80km/hr. As such the proposed roundabout at the intersection of the new Collector Road with Culburra Road is not supported by RMS.
- viii Based on the estimated traffic volumes at the Culburra Road/ new Collector Rod intersection and the existing 100km/hr speed zone on Culburra Road remaining, an alternative channelised T-junction with a full length right-turn treatment on the major road (CHR) is warranted in line with Figure 4.9 Austroads Guide to Road Design Part 4A: Unsignalised and Signalised Intersections.
- ix An assessment of the location of the proposed Collector Road intersection with Culburra Road indicates that the required ASD as stipulated in Table 3.2 of Austroads Guide to Road Design Part 4A can be achieved on all approaches.



# Appendix A

Correspondence from RMS

Our Ref: STH10/00063/03

Contact: Andrea Boes 4221 2771 Your Ref: MP09 0088 File: 10/06041-2



The General Manager Department of Planning & Infrastructure GPO Box 39 Sydney NSW 2001

Attention: Sarah Waterworth

# SHOALHAVEN CITY COUNCIL - MAJOR PROJECT 09\_0088 - MIXED USE SUBDIVISION, WEST CULBURRA - EXHIBITION OF EA FOR CONCEPT PLAN

Dear Sir/Madam

Reference is made to your letter dated 17 April 2013 regarding the subject project application forwarded to Roads and Maritime Services (RMS) for consideration.

RMS has reviewed the submitted information and has concerns regarding the additional traffic generated by the subject development and the increased pressure on the classified road network. In this regard, RMS offers the following comments for the Department's consideration:

- RMS notes the traffic generation resulting from the full development of the site is estimated to be 573 vehicle movements in the peak hour as per Section 7.1.1 of GTA Consultant's *Traffic Impact Assessment*.
- RMS has undertaken further analysis using SIDRA to assess the likely impacts of full development of the subject site on the intersection of the Princes Highway and Kalandar Street. This assessment, based on the Friday AM 120<sup>th</sup> highest hour, indicated that the Average Delay would increase from the current 86.4 seconds for all vehicles to 145.2 seconds for all vehicles. The analysis indicated that the Degree of Saturation would increase from the current 1.049 for all vehicles to 1.175. The long term impact of the development on the road network has not been assessed.
- The above analysis indicated that the subject development will likely have a very significant impact on the intersection of the Princes Highway and Kalandar Street. The proponent should identify suitable infrastructure required to ameliorate these impacts on the network as a result of the subject development. These should be submitted to the Department and forwarded to RMS for comment.

- The proponent should carry out further traffic analysis based on full development of the site. The analysis should include 10 year projected traffic volumes including background traffic growth and likely traffic volumes with and without the development. The impacts on the network of full development of the site should be assessed prior to determination of the project application for concept approval.
- RMS notes there are a number of signalised intersections on the Princes Highway north of Kalandar Street. For accurate representation of the likely impacts of the subject development on the highway, the proponent should undertake corridor modelling with the use of a program such as LinSig to include the coordinated nature of intersections at this location in the traffic analysis.
- RMS is responsible for all permanent speed zoning in NSW regardless of the classification of the road. RMS objects to the suggested speed zone change on Culburra Road. Speed zones should be intuitive to the driver, with the road environment and adjacent land uses effectively informing the driver of the speed zone. On this basis, RMS considers that the existing 100km/hr speed zone at this location is appropriate and should be maintained. Without appropriate changes to the road environment along the relevant section of Culburra Road, a lower speed zone would likely lead to compliance and enforcement issues.
- Council should seek to reduce the number of access points to Culburra Road where
  possible. All access to stage 5 should be via the roundabout and the new collector
  road if possible. Research indicates that increased access density correlates highly
  with increased crash rates, and on this basis, accesses should be consolidated
  wherever possible.
- The proponent should ensure that Safe Intersection Sight Distance is available at all new intersections in accordance with Austroads *Guide to Road Design Part 4a: Unsignalised and Signalised Intersections* Table 3.2, in both directions.

0 7 JUN 2013

If you have any questions please contact Andrea Boes on 02 4221 2771.

Yours faithfully

Brian Lefoe

Road Safety and Traffic Manager

Network Management, Southern Region

Cc – The General Manager, Shoalhaven City Council (via email)

**Roads & Maritime Services** 

### **Justin Murphy**

From: BOES Andrea R < Andrea.BOES@rms.nsw.gov.au>

Sent: Thursday, 12 September 2013 3:38 PM

**To:** Justin Murphy

**Cc:** mark.schofield@planning.nsw.gov.au; council@shoalhaven.nsw.gov.au

Subject: RE: West Culburra Subdivision Development (Major Project 09\_0088, RMS Ref:

STH10/00063)

#### Hi Justin

As per our conversation today, please find following RMS' comments in response to your email of 3 September 2013.

• RMS notes the traffic generation resulting from the development of the site is estimate to be 573 vehicle movements in the peak hour as per Section 7.1.1 of GTA Consultant's Traffic Impact Assessment.

Based on empirical traffic generation rates for the established areas of Culburra as supplied by Shoalhaven City Council's Traffic and Transport Unit, the proposed development is expected to generate up to 158 vehicle movements on the regional road network (west of Culburra) in the peak hour following full site development. Given the existing traffic generating characteristics of Culburra and the proportion of dedicated housing for the over 55 aged group proposed as part of the development, we consider that the adoption of the Culburra specific empirical traffic generation should be used in the intersection assessments not the 573 vehicles suggested by RMS.

#### RMS agrees with this approach.

• RMS has undertaken further analysis using SIDRA to access the likely impacts of full development of the subject site on the intersection of the Princes Highway and Kalandar Street. This assessment, based on the Friday AM 120<sup>th</sup> highest hour, indicated the Average Delay would increase from the current 86.4 seconds for all vehicles to 145.2 seconds for all vehicles. The analysis indicated that the Degree of Saturation would increase from the current 1.049 for all vehicles to 1.175. The long term impact of the development has not been assessed.

It is assumed that the additional SIDRA analysis undertaken by RMS for the Friday AM 120<sup>th</sup> highest hour utilised RTA's development traffic generation estimate of 573 vehicle movements in the peak hour. Can you please confirm whether this is the case.

#### Yes.

• The above analysis indicated that the subject development will likely have a very significant impact on the intersection of the Princes Highway and Kalandar Street. The proponent should identify suitable infrastructure required to ameliorate these impacts on the network as a result of the subject development. These should be submitted to the Department and forwarded to RMS for comment.

SIDRA analysis of the Princes Highway/ Kalandar Street intersection indicates that significant delays are currently experienced during peak periods and that further capacity improvements are required, regardless of what development occurs at Culburra. Any infrastructure improvements identified by GTA at this intersection needs to be commensurate with RMS proposals along the Princes Highway corridor. Can you please confirm whether RMS are proposing any improvements along the Princes Highway in Nowra north of Kinghorne Street and in particular at the Kalandar Street intersection.

RMS advises that it currently has no upgrades planned for the intersection of the Princes Highway and Kalandar Street or north of Kinghorne Street in this vicinity.

 The proponent should carry out further traffic analysis based on full development of the site. The analysis should include 10 year projected traffic volumes including background traffic growth and likely traffic volumes with and without the development. The impacts on the network of full development of the site should be assessed prior to determination of the project application for concept approval. The future year traffic modelling undertaken by GTA was for full development of the site (estimated to be approx. 8 years after commencement). It is understood that the West Culburra subdivision development will be the major source of growth in the Culburra area over the next 10 years and also the major source of growth in the area between Culburra and the Princes Highway over the same period. Any modelling of the Kalandar Street intersection 10 years after opening would simply show the intersection continuing to operate overcapacity.

Once potential improvements at the Kalandar Street intersection are known, it would then be appropriate to assess a full site development + 10 year scenario.

#### RMS supports this approach.

• RMS notes there are a number of signalised intersections on the Princes Highway north of Kalandar Street. For accurate representation of the likely impacts of subject development on the highway, the proponent should undertake corridor modelling with the use of a program such as LinSig to include the coordinated nature of intersections at this location in the traffic analysis.

The West Culburra Subdivision development is approximately 20km from Nowra. GTA undertook SIDRA modelling of the two Princes Highway intersections in Nowra most likely to be utilised by development traffic; Moss Street and Kalandar Street. These intersections are approximately 1.7km apart and two signalised intersections (Worrigee Street and Plunkett Street) are located between them. Given the significant distance between these intersections, can RMS please confirm whether corridor modelling using LinSig software or similar is required?

RMS notes the distance between the intersection of the Princes Highway and Kalandar Street and the nearest signalised intersection at Plunkett Street. Given this, further corridor modelling using LinSig is not deemed to benecessary.

RMS is responsible for all permanent speed zoning in NSW regardless of the classification of the road. RMS objects to the suggested speed zone change on Culburra Road. Speed zones should be intuitive to the driver, with the road environment and adjacent land uses effectively informing the driver of the speed zone. On this bases, RMS considers that the existing 100km/hr speed zone at this location is appropriate and should be maintained. Without appropriate changes to the road environment along the relevant section Culburra Road, a lower speed zone would likely lead to compliance and enforcement issues

The main access to the development is proposed via a new roundabout at the eastern intersection of the new Collector Road and Culburra Road. The reduced speed limit on approach to the roundabout was deemed appropriate and then this was to be maintained from the roundabout to Culburra to prevent a short 100km/h speed zone between the 50km/h speed zone at the roundabout and the entrance to Culburra Road (a length of approx. 700m). On this basis, can you please clarify whether RMS considers the existing 100km/h speed zone on this section of Culburra Road appropriate.

RMS does not support roundabouts within speed zones above 80km/hr. The proponent should determine an appropriate junction treatment at the intersection of the Princes Highway and the new collector road to allow safe access into and out of the development within the existing speed zone. In this regard, refer to Figure 4.9: Warrants for turn treatments on the major road at unsignalised intersections, in Austroads *Guide to Road Design - Part 4A: Unsignalised and Signalised Intersections*. RMS is unlikely to support a speed zone reduction at this location.

If you have any questions please do not hesitate to contact me on 4221 2771.

#### Regards

#### **Andrea Boes**

**Development Assessment Officer** 

Road Safety & Traffic Management Network Management | Southern Region T 02 4221 2771 F 02 4221 2777 Roads and Maritime Services
90 Crown Street Wollongong NSW 2500 | **PO Box 477 Wollongong** NSW 2520

**From:** Justin Murphy - GTA Consultants [mailto:Justin.Murphy@gta.com.au]

Sent: Tuesday, 3 September 2013 6:08 PM

To: MILLET Chris P

Cc: Ken Hollyoak - GTA Consultants

**Subject:** West Culburra Subdivision Development (major project 09\_0088)

Hi Chris,

Following our conversation this morning and in preparation for our meeting on Monday 16 September, we would like clarification on some of the comments made in the RMS letter dated 07 June 2013 (attached):

• RMS notes the traffic generation resulting from the development of the site is estimate to be 573 vehicle movements in the peak hour as per Section 7.1.1 of GTA Consultant's Traffic Impact Assessment.

Based on empirical traffic generation rates for the established areas of Culburra as supplied by Shoalhaven City Council's Traffic and Transport Unit, the proposed development is expected to generate up to 158 vehicle movements on the regional road network (west of Culburra) in the peak hour following full site development. Given the existing traffic generating characteristics of Culburra and the proportion of dedicated housing for the over 55 aged group proposed as part of the development, we consider that the adoption of the Culburra specific empirical traffic generation should be used in the intersection assessments not the 573 vehciles suggested by RMS.

RMS has undertaken further analysis using SIDRA to access the likely impacts of full development of the
subject site on the intersection of the Princes Highway and Kalandar Street. This assessment, based on the
Friday AM 120th highest hour, indicated the Average Delay would increase from the current 86.4 seconds
for all vehicles to 145.2 seconds for all vehicles. The analysis indicated that the Degree of Saturation would
increase from the current 1.049 for all vehicles to 1.175. The long term impact of the development has not
been assessed.

It is assumed that the additional SIDRA analysis undertaken by RMS for the Friday AM 120<sup>th</sup> highest hour utilised RTA's development traffic generation estimate of 573 vehicle movements in the peak hour. Can you please confirm whether this is the case.

The above analysis indicated that the subject development will likely have a very significant impact on the
intersection of the Princes Highway and Kalandar Street. The proponent should identify suitable
infrastructure required to ameliorate these impacts on the network as a result of the subject development.
These should be submitted to the Department and forwarded to RMS for comment.

SIDRA analysis of the Princes Highway/ Kalandar Street intersection indicates that significant delays are currently experienced during peak periods and that further capacity improvements are required, regardless of what development occurs at Culburra. Any infrastructure improvements identified by GTA at this intersection needs to be commensurate with RMS proposals along the Princes Highway corridor. Can you please confirm whether RMS are proposing any improvements along the Princes Highway in Nowra north of Kinghorne Street and in particular at the Kalandar Street intersection.

 The proponent should carry out further traffic analysis based on full development of the site. The analysis should include 10 year projected traffic volumes including background traffic growth and likely traffic volumes with and without the development. The impacts on the network of full development of the site should be assessed prior to determination of the project application for concept approval.

The future year traffic modelling undertaken by GTA was for full development of the site (estimated to be approx. 8 years after commencement). It is understood that the West Culburra subdivision development will be the major source of growth in the Culburra area over the next 10 years and also the major source of growth in the area between Culburra and the Princes Highway over the same period. Any modelling of the Kalandar Street intersection 10 years after opening would simply show the intersection continuing to operate overcapacity.

Once potential improvements at the Kalandar Street intersection are known, it would then be appropriate to assess a full site development + 10 year scenario.

RMS notes there are a number of signalised intersections on the Princes Highway north of Kalandar Street.
 For accurate representation of the likely impacts of subject development on the highway, the proponent should undertake corridor modelling with the use of a program such as LinSig to include the coordinated nature of intersections at this location in the traffic analysis.

The West Culburra Subdivision development is approximately 20km from Nowra. GTA undertook SIDRA modelling of the two Princes Highway intersections in Nowra most likely to be utilised by development traffic; Moss Street and Kalandar Street. These intersections are approximately 1.7km apart and two signalised intersections (Worrigee Street and Plunkett Street) are located between them. Given the significant distance between these intersections, can RMS please confirm whether corridor modelling using LinSig software or similar is required?

RMS is responsible for all permanent speed zoning in NSW regardless of the classification of the road. RMS objects to the suggested speed zone change on Culburra Road. Speed zones should be intuitive to the driver, with the road environment and adjacent land uses effectively informing the driver of the speed zone. On this bases, RMS considers that the existing 100km/hr speed zone at this location is appropriate and should be maintained. Without appropriate changes to the road environment along the relevant section Culburra Road, a lower speed zone would likely lead to compliance and enforcement issues

The main access to the development is proposed via a new roundabout at the eastern intersection of the new Collector Road and Culburra Road. The reduced speed limit on approach to the roundabout was deemed appropriate and then this was to be maintained from the roundabout to Culburra to prevent a short 100km/h speed zone between the 50km/h speed zone at the roundabout and the entrance to Culburra Road (a length of approx. 700m). On this basis, can you please clarify whether RMS considers the existing 100km/h speed zone on this section of Culburra Road appropriate.

Let me know if you have any queries with the above or would like to discuss.

Regards.

Justin Murphy
Project Manager - Traffic & Transport
GTA Consultants
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0434 676 461
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From: Justin Murphy

Sent: Thursday, 29 August 2013 3:32 PM

To: Chris Millet (christopher millet@rta.nsw.gov.au)

Cc: Ken Hollyoak

**Subject:** West Culburra Subdivision Development (major project 09\_0088)

Hi Chris,

We recently received RMS' comments (attached) on the transport impact assessment undertaken by GTA on the above project.

Before we proceed further, I wonder if it would be possible if my colleague Ken Hollyoak and I could come and discuss these issues with you in Wollongong.

I spoke with Brian Lefoe this afternoon in relation to the project and he suggested I contact you.

Both Ken and I are available every day next week except Monday. Do you have any time available to meet?

If you have any further queries, or would like to discuss, please don't hesitate to contact me.

Kind Regards.

Justin Murphy
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Our Ref: STH10/00063/04 Contact: Andrea Boes 4221 2771

Your Ref: 12S1231000



GTA Consultants Level 6, 15 Help Street Chatswood NSW 2067

Attention: Justin Murphy

# SHOALHAVEN CITY COUNCIL - MAJOR PROJECT 09\_0088 - WEST CULBURRA MIXED USE SUBDIVISION - FURTHER COMMENTS - EXHIBITION OF EA

Dear Sir/Madam

Reference is made to your email of 3 September 2013 regarding the project application forwarded to Roads and Maritime Services (RMS) for consideration.

RMS has reviewed the additional information provided and offers the following comments for your consideration:

- RMS advises that it currently has no upgrades planned for the intersection of the Princes Highway and Kalandar Street or north of Kinghorne Street in this vicinity.
- RMS accepts the distribution assignment used to analyse the proportion of traffic generated by the subject development leaving the Culburra area and travelling west to the Princes Highway. However, this analysis should include the full development including the Stage 6 Industrial lots.
- RMS supports the proposed assessment of the site at full development and a 10 year projected scenario with and without the development. The proponent should identify suitable infrastructure to ameliorate any impacts on the network as a result of the total development.
- RMS notes the distance between the intersection of the Princes Highway and Kalandar Street and the nearest signalised intersection at Plunkett Street, Nowra. Given this, further corridor modelling using LinSig is not deemed to be necessary.
- RMS generally does not support the construction of roundabouts within speed zones above 80km/hr. The proponent should determine an appropriate junction treatment at the intersection of the Princes Highway and the new collector road to allow safe access into and out of the development within the existing speed zone. In this regard, refer to Figure 4.9: Warrants for turn treatments on the major road at Unsignalised intersections, in Austroads *Guide to Road Design Part 4A: Unsignalised and Signalised Intersections*. RMS is unlikely to support a speed zone reduction at this location.

#### Roads & Maritime Services

If you have any questions please contact Andrea Boes on 4221 2771.

Yours faithfully

Brian Lefoe

Road Safety and Traffic Manager

Network Management, Southern Region

1-8 SEP 2013

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# Appendix B

# SIDRA INTERSECTION Results

13S1231000 - West Culburra Subdivision Princes Highway-Kalandar Street Friday AM (0800-0900) - Equivalent 120th HH

Existing

Signals - Fixed Time Cycle Time = 135 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Per	formance - \	Vehicles								
	_	Demand	107	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Courtle	Deimona	veh/h	%	v/c	sec		veh	m		per veh	km/h
	Princes F	• , ,	0.0	4.040	400.7	1005	00.0	000.0	4.00	4.47	40.0
1	L	3	0.0	1.049	103.7	LOS F	39.3	288.0	1.00	1.17	16.8
2	Т	962	5.6	1.049	111.7	LOS F	49.4	362.6	1.00	1.25	15.6
3	R	38	0.0	0.459	81.6	LOS F	2.6	18.3	1.00	0.73	19.2
Approa	ich	1003	5.3	1.049	110.6	LOS F	49.4	362.6	1.00	1.23	15.7
East: K	alandar S	St (E)									
4	L	38	9.4	1.039	115.9	LOS F	51.4	370.2	1.00	1.23	10.7
5	T	306	3.1	1.039	106.6	LOS F	51.4	370.2	1.00	1.23	10.1
6	R	782	2.9	1.039	129.2	LOS F	56.2	403.3	1.00	1.21	9.7
Approa	ich	1126	3.1	1.039	122.6	LOS F	56.2	403.3	1.00	1.22	9.8
North: F	Princes H	wy (N)									
7	L	259	7.3	0.261	9.2	LOS A	8.0	5.7	0.07	0.64	53.4
8	Т	898	7.3	0.734	40.8	LOS C	24.3	180.8	0.88	0.78	29.9
9	R	157	11.3	1.042	112.3	LOS F	12.0	91.7	1.00	1.09	15.2
Approa	ich	1314	7.8	1.042	43.1	LOS D	24.3	180.8	0.74	0.79	29.1
West: K	Kalandar (	St (W)									
10	L	52	13.6	0.700	65.8	LOS E	9.0	66.1	0.95	0.90	19.0
11	Т	149	1.6	0.700	57.2	LOS E	12.8	92.9	0.97	0.86	18.0
12	R	156	5.3	0.700	69.3	LOS E	12.8	92.9	1.00	0.85	17.9
Approa	ich	357	4.9	0.700	63.7	LOS E	12.8	92.9	0.98	0.86	18.1
All Veh	icles	3800	5.5	1.049	86.4	LOSF	56.2	403.3	0.91	1.04	16.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestrians	S					
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P1	Across S approach	53	45.6	LOS E	0.2	0.2	0.82	0.82
P3	Across E approach	53	37.0	LOS D	0.2	0.2	0.74	0.74
P5	Across N approach	53	61.6	LOS F	0.2	0.2	0.96	0.96
All Pede	estrians	159	48.1	LOS E			0.84	0.84

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Culburra Subdivision.sip



13S1231000 - West Culburra Subdivision Princes Highway-Kalandar Street Friday PM (1600-1700) - Equivalent 120th HH

Existing

Signals - Fixed Time Cycle Time = 135 seconds (Optimum Cycle Time - Minimum Delay)

Mover	nent Per	formance - \	/ehicles								
	_	Demand	1.0.7	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0 "	D :	veh/h	%	v/c	sec		veh	m		per veh	km/h
	Princes F	• , ,									
1	L	12	0.0	1.038	100.6	LOS F	29.4	210.2	1.00	1.12	17.2
2	Т	785	2.6	1.038	107.4	LOS F	39.3	281.0	1.00	1.20	16.1
3	R	109	1.4	1.004	120.5	LOS F	9.8	69.4	1.00	1.08	14.3
Approa	ch	906	2.4	1.038	108.9	LOS F	39.3	281.0	1.00	1.18	15.8
East: K	alandar S	St (E)									
4	L	99	1.5	1.065	132.7	LOS F	42.9	306.0	1.00	1.27	9.5
5	Т	245	2.4	1.065	123.7	LOS F	42.9	306.0	1.00	1.27	8.9
6	R	567	1.8	1.065	151.3	LOS F	47.2	335.4	1.00	1.26	8.5
Approa	ch	912	2.0	1.065	141.9	LOS F	47.2	335.4	1.00	1.26	8.7
North: I	Princes H	wy (N)									
7	L	<mark>815</mark>	1.1	1.000 <sup>3</sup>	35.7	LOS C	18.5	130.6	0.21	0.77	32.5
8	Т	1318	3.1	1.100	162.7	LOS F	77.6	557.9	1.00	1.54	11.6
9	R	138	5.4	0.587	44.5	LOS D	6.1	44.8	0.99	0.79	28.6
Approa	ch	2272	2.5	1.100	109.9	LOS F	77.6	557.9	0.71	1.22	15.7
West: k	Kalandar S	St (W)									
<mark>10</mark>	L	<mark>109</mark>	9.5	1.000 <sup>3</sup>	56.8	LOS E	13.7	99.1	0.93	0.90	21.0
11	Т	305	0.0	1.101	113.6	LOS F	55.2	388.8	0.97	1.13	11.0
12	R	303	1.0	1.101	185.3	LOS F	55.2	388.8	1.00	1.41	8.1
Approa	ch	718	1.9	1.101	135.2	LOS F	55.2	388.8	0.97	1.22	10.2
All Veh	icles	4807	2.3	1.101	119.6	LOSF	77.6	557.9	0.86	1.22	13.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Moven	nent Performance -	Pedestrians	\$					
		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	53	53.3	LOS E	0.2	0.2	0.89	0.89
P3	Across E approach	53	38.5	LOS D	0.2	0.2	0.76	0.76
P5	Across N approach	53	53.3	LOS E	0.2	0.2	0.89	0.89
All Pede	estrians	159	48.4	LOS E			0.84	0.84

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



13S1231000 - West Culburra Subdivision Princes Highway-Kalandar Street Saturday - Equivalent 120th HH Existing

Signals - Fixed Time Cycle Time = 135 seconds (Optimum Cycle Time - Minimum Delay)

Movem	nent Pe	rformance - \	Vehicles								
		Demand	107	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South: I	Princes H	veh/h	%	v/c	sec		veh	m		per veh	km/h
1		1wy (3)	25.0	0.872	74.6	LOS F	32.0	229.4	0.99	0.98	21.9
	L	_									
2	T	960	2.6	0.872	60.0	LOS E	32.0	229.4	0.99	0.96	23.9
3	R	116	4.5	0.966	104.3	LOS F	9.6	69.5	1.00	1.03	16.0
Approac	ch	1081	2.9	0.966	64.8	LOS E	32.0	229.4	0.99	0.97	22.8
East: Ka	alandar S	St (E)									
4	L	89	0.0	0.968	87.7	LOS F	35.3	248.4	1.00	1.15	13.4
5	Т	211	0.6	0.968	78.7	LOS F	35.3	248.4	1.00	1.15	12.6
6	R	543	1.0	0.968	95.1	LOS F	35.3	248.4	1.00	1.09	12.5
Approac	ch	843	0.8	0.968	90.3	LOS F	35.3	248.4	1.00	1.12	12.6
North: F	Princes H	lwy (N)									
7	L	496	0.3	0.507	9.3	LOS A	2.0	14.2	0.08	0.65	53.1
8	Т	1386	1.0	0.961	65.3	LOS E	55.2	389.4	1.00	1.11	22.6
9	R	91	11.6	0.482	42.7	LOS D	3.4	26.4	0.98	0.77	29.4
Approac	ch	1973	1.3	0.961	50.2	LOS D	55.2	389.4	0.77	0.98	26.6
West: K	Calandar	St (W)									
10	L	66	2.0	0.771	70.3	LOS E	10.8	76.1	0.96	0.96	18.0
11	Т	168	0.0	0.771	61.7	LOS E	15.0	105.2	0.98	0.92	17.1
12	R	173	0.8	0.771	71.6	LOS F	15.0	105.2	1.00	0.88	17.5
Approac	ch	407	0.6	0.771	67.3	LOS E	15.0	105.2	0.98	0.91	17.4
All Vehi	cles	4304	1.5	0.968	63.3	LOS E	55.2	389.4	0.89	1.00	21.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestrian	s					
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P1	Across S approach	53	53.3	LOS E	0.2	0.2	0.89	0.89
P3	Across E approach	53	32.7	LOS D	0.1	0.1	0.70	0.70
P5	Across N approach	53	61.6	LOS F	0.2	0.2	0.96	0.96
All Ped	estrians	159	49.2	LOS E			0.85	0.85

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 1. Culburra -Coonamia (Future Fri AM-120th HH)

13S1231000 - West Culburra Subdivision Culburra Road-Coonamia Road Friday AM (0800-0900) - Equivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Mover	nent Perf	formance - \	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay	Level of Service	95% Back of Vehicles	Distance	Prop. Queued	Effective Stop Rate per veh	Average Speed
South:	Coonamia		70	V/C	sec		veh	m		per veri	km/h
1	L	165	0.0	0.235	14.5	LOS B	0.8	5.6	0.46	0.80	61.0
3	R	99	6.5	0.206	17.9	LOS B	0.8	6.1	0.56	0.87	56.9
Approa	ch	264	2.4	0.235	15.8	LOS B	8.0	6.1	0.50	0.82	59.4
East: C	ulburra Ro	d (E)									
4	L	60	2.9	0.033	11.5	LOSA	0.0	0.0	0.00	0.74	63.3
5	T	337	3.7	0.177	0.0	LOSA	0.0	0.0	0.00	0.00	80.0
Approa	ch	397	3.6	0.177	1.7	NA	0.0	0.0	0.00	0.11	76.6
West: 0	Culburra R	d (W)									
11	Т	149	12.3	0.083	0.0	Χ	X	X	X	0.00	80.0
12	R	33	3.6	0.034	13.2	LOSA	0.1	0.9	0.43	0.72	61.1
Approa	ch	182	10.8	0.083	2.4	NA	0.1	0.9	0.08	0.13	75.4
All Veh	icles	843	4.8	0.235	6.3	NA	0.8	6.1	0.17	0.34	70.2

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: 1. Culburra -Coonamia (Future Fri PM-120th HH)

13S1231000 - West Culburra Subdivision Culburra Road-Coonamia Road Friday PM (1600-1700) - Equivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Mover	nent Perf	formance - V	ehicles								
Mov ID	Turn	Demand	HV	Deg.	Average	Level of	95% Back o		Prop.	Effective	Average
IVIOV ID	Tulli	Flow veh/h	ПV %	Satn v/c	Delay sec	Service	Vehicles veh	Distance m	Queued	Stop Rate per veh	Speed km/h
South:	Coonamia		/0	V/C	366		VEII	- '''		per veri	KIII/II
1	L	77	7.7	0.109	13.9	LOSA	0.3	2.3	0.33	0.72	62.6
3	R	101	2.0	0.195	16.8	LOS B	0.8	5.6	0.54	0.85	57.9
Approa	ch	178	4.5	0.195	15.6	LOS B	0.8	5.6	0.45	0.79	59.8
East: C	ulburra Ro	d (E)									
4	L	103	2.0	0.056	11.5	LOSA	0.0	0.0	0.00	0.74	63.3
5	T	165	2.6	0.086	0.0	LOSA	0.0	0.0	0.00	0.00	80.0
Approa	ch	268	2.4	0.086	4.4	NA	0.0	0.0	0.00	0.28	72.1
West: 0	Culburra R	d (W)									
11	T	379	1.1	0.196	0.0	Χ	X	Χ	Χ	0.00	79.9
12	R	152	3.9	0.137	12.7	LOSA	0.6	4.1	0.37	0.72	61.4
Approa	ch	531	1.9	0.196	3.7	NA	0.6	4.1	0.11	0.21	73.1
All Veh	icles	977	2.5	0.196	6.0	NA	0.8	5.6	0.14	0.33	70.1

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Culburra Subdivision.sip



Site: 2. Culburra-Mayfield (Future Fri AM-120th HH)

Culburra Road-Mayfield Road Friday AM (0800-0900) - Equivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Moven	nent Per	formance - V	ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South:	Culburra F		/0	V/ O	360		٧٥١١			per veri	KIII/II
1	L	2	0.0	0.255	10.1	LOSA	0.0	0.0	0.00	1.73	57.1
2	T	495	0.0	0.255	0.0	LOSA	0.0	0.0	0.00	0.00	80.0
Approa	ch	497	0.0	0.255	0.0	NA	0.0	0.0	0.00	0.01	79.9
North: 0	Culburra F	Road (N)									
8	Т	180	0.0	0.094	2.8	LOSA	0.9	6.0	0.59	0.00	62.7
9	R	1	0.0	0.094	13.0	LOSA	0.9	6.0	0.59	1.31	60.3
Approa	ch	181	0.0	0.094	2.8	NA	0.9	6.0	0.59	0.01	62.7
West: N	/layfield R	oad									
10	L	1	0.0	0.009	17.0	LOS B	0.0	0.2	0.63	0.70	43.8
12	R	2	0.0	0.009	17.0	LOS B	0.0	0.2	0.63	0.80	44.0
Approa	ch	3	0.0	0.009	17.0	LOS B	0.0	0.2	0.63	0.77	43.9
All Vehi	icles	681	0.0	0.255	0.9	NA	0.9	6.0	0.16	0.01	74.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: 2. Culburra-Mayfield (Future Fri PM-120th HH)

Culburra Road-Mayfield Road Friday PM (1600-1700) - Equivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Moven	nent Per	formance - '	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South:	Culburra F		70	• • • • • • • • • • • • • • • • • • • •			7011			por vori	1311/11
1	L	1	0.0	0.129	10.1	LOSA	0.0	0.0	0.00	1.73	57.1
2	Т	243	4.6	0.129	0.0	LOSA	0.0	0.0	0.00	0.00	80.0
Approa	ch	244	4.6	0.129	0.0	NA	0.0	0.0	0.00	0.01	79.9
North: 0	Culburra F	Road (N)									
8	Т	511	1.8	0.266	1.4	LOSA	2.2	15.6	0.47	0.00	65.6
9	R	1	0.0	0.266	11.6	LOSA	2.2	15.6	0.47	1.39	60.7
Approa	ch	512	1.8	0.266	1.4	NA	2.2	15.6	0.47	0.00	65.6
West: N	//ayfield R	oad									
10	L	1	0.0	0.035	64.1	LOS E	0.1	1.0	0.84	0.63	23.7
12	R	1	100.0	0.035	69.5	LOS E	0.1	1.0	0.84	0.96	26.3
Approa	ch	2	50.0	0.035	66.8	LOS E	0.1	1.0	0.84	0.80	25.1
All Vehi	icles	758	2.8	0.266	1.2	NA	2.2	15.6	0.32	0.01	69.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: 3. Greenwell Pt-Pyree (Future Fri AM-120th HH)

13S1231000 - West Culburra Subdivision Greenwell Point Road-Pyree Lane Friday AM (0800-0900) - Equivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Movem	nent Perf	ormance - \	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: F	yreen Ln										
1	L	487	1.9	0.266	11.2	Χ	X	X	X	0.69	58.8
3	R	14	8.3	0.020	13.0	LOSA	0.1	0.6	0.34	0.69	56.9
Approac	ch	501	2.1	0.266	11.2	LOSA	0.1	0.6	0.01	0.69	58.8
East: G	reenwell F	Pt Rd (E)									
4	L	20	66.7	0.094	14.9	LOS B	0.0	0.0	0.00	1.42	58.9
5	Т	147	5.6	0.094	0.0	LOSA	0.0	0.0	0.00	0.00	80.0
Approac	ch	167	12.9	0.094	1.8	NA	0.0	0.0	0.00	0.17	76.8
West: G	reenwell l	Pt Rd (W)									
11	Т	63	5.6	0.034	0.0	Χ	X	X	Χ	0.00	80.0
12	R	164	7.4	0.367	19.1	LOS B	1.9	14.0	0.63	0.95	49.6
Approac	ch	227	6.9	0.367	13.8	LOS A	1.9	14.0	0.46	0.68	55.5
All Vehic	cles	896	5.3	0.367	10.1	NA	1.9	14.0	0.12	0.59	60.6

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: 3. Greenwell Pt-Pyree (Future Fri PM-120th HH)

13S1231000 - West Culburra Subdivision Greenwell Point Road-Pyree Lane Friday PM (1600-1700) - Equivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Moven	nent Perf	ormance - V	ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: I	Pyreen Ln		/0	V/C	366		VEII	- '''		per veri	KIII/II
1	L	205	5.8	0.115	11.3	Х	Х	Х	Х	0.69	58.9
3	R	32	0.0	0.039	11.5	LOSA	0.2	1.1	0.23	0.68	58.0
Approa	ch	237	5.0	0.115	11.4	LOSA	0.2	1.1	0.03	0.69	58.7
East: G	reenwell F	Pt Rd (E)									
4	L	26	0.0	0.050	10.9	LOSA	0.0	0.0	0.00	1.07	58.9
5	T	68	4.3	0.050	0.0	LOSA	0.0	0.0	0.00	0.00	80.0
Approa	ch	95	3.1	0.050	3.0	NA	0.0	0.0	0.00	0.30	72.9
West: G	Greenwell I	Pt Rd (W)									
11	Т	173	1.7	0.090	0.0	Χ	X	Х	Х	0.00	80.0
12	R	489	1.5	0.728	19.7	LOS B	10.6	75.2	0.74	0.98	48.7
Approa	ch	662	1.6	0.728	14.6	LOS B	10.6	75.2	0.54	0.73	54.3
All Vehi	icles	994	2.5	0.728	12.7	NA	10.6	75.2	0.37	0.68	56.7

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1231000 - West Culburra Subdivision Greenwell Point Road-Jindy Andy Lane Friday AM (0800-0900) - Equivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Movem	nent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
North E	ast: Gree	nwell Point Ro	ad (NE)								
25	Т	448	4.8	0.237	0.0	LOS A	0.0	0.0	0.00	0.00	80.0
26	R	193	0.7	0.297	11.2	LOS A	1.4	9.6	0.47	0.74	45.9
Approac	ch	641	3.6	0.297	3.4	NA	1.4	9.6	0.14	0.22	66.5
North W	lest: Jind	y Andy Lane									
27	L	51	3.0	0.051	12.0	LOS A	0.2	1.3	0.30	0.69	57.2
29	R	12	30.0	0.090	39.8	LOS C	0.3	2.5	0.84	0.96	35.7
Approac	ch	62	8.1	0.090	17.2	LOS B	0.3	2.5	0.40	0.74	51.5
South V	Vest: Gre	enwell Point R	oad (SW)								
30	L	8	42.9	0.102	13.5	LOS A	0.0	0.0	0.00	1.48	58.9
31	Т	178	8.7	0.102	0.0	LOSA	0.0	0.0	0.00	0.00	80.0
Approac	ch	186	10.2	0.102	0.6	NA	0.0	0.0	0.00	0.07	78.8
All Vehi	cles	889	5.3	0.297	3.7	NA	1.4	9.6	0.13	0.23	67.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1231000 - West Culburra Subdivision Greenwell Point Road-Jindy Andy Lane Friday AM (1600-1700) - Equivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Moven	nent Perf	formance - V	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
North E	ast: Greei	nwell Point Ro								po. 10	
25	Т	214	5.9	0.114	0.0	LOSA	0.0	0.0	0.00	0.00	80.0
26	R	73	2.4	0.210	17.5	LOS B	0.8	5.7	0.67	0.90	40.5
Approa	ch	286	5.0	0.210	4.4	NA	0.8	5.7	0.17	0.23	65.4
North V	Vest: Jindy	/ Andy Lane									
27	L	187	1.9	0.278	14.9	LOS B	1.2	8.2	0.55	0.89	53.9
29	R	3	0.0	0.012	22.4	LOS B	0.0	0.3	0.71	0.85	46.1
Approa	ch	191	1.8	0.278	15.0	LOS B	1.2	8.2	0.56	0.89	53.7
South V	Vest: Gree	enwell Point R	oad (SW)								
30	L	14	11.1	0.252	11.6	LOS A	0.0	0.0	0.00	1.37	58.9
31	T	471	1.9	0.252	0.0	LOSA	0.0	0.0	0.00	0.00	80.0
Approa	ch	484	2.1	0.252	0.3	NA	0.0	0.0	0.00	0.04	79.2
All Vehi	icles	961	2.9	0.278	4.5	NA	1.2	8.2	0.16	0.26	68.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: 5. Greenwell Pt-Mayfield (Future Fri AM-120th HH)

13S1231000 - West Culburra Subdivision Greenwell Point Road-Mayfield Road Friday AM (0800-0900) - Équivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Movem	nent Peri	formance - V	/ehicles								
M 15	<b>-</b>	Demand	1.15.7	Deg.	Average	Level of	95% Back o		Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0 " 5		veh/h	%	v/c	sec		veh	m		per veh	km/h
South E	East: Mayf	ield Road									
21	L	12	0.0	0.020	12.5	LOS A	0.1	0.5	0.49	0.72	47.8
23	R	1	0.0	0.020	12.5	LOSA	0.1	0.5	0.49	0.82	47.9
Approac	ch	13	0.0	0.020	12.5	LOS A	0.1	0.5	0.49	0.73	47.8
North E	ast: Gree	nwell Point Ro	ad (NE)								
24	L	3	0.0	0.241	10.1	LOSA	0.0	0.0	0.00	1.73	57.1
25	Т	455	3.8	0.241	0.0	LOSA	0.0	0.0	0.00	0.00	80.0
Approac	ch	458	3.7	0.241	0.1	NA	0.0	0.0	0.00	0.01	79.8
South V	Vest: Gree	enwell Point Ro	oad (SW)								
31	T	183	9.2	0.114	6.7	LOSA	1.7	13.0	0.76	0.00	58.7
32	R	6	0.0	0.114	16.9	LOS B	1.7	13.0	0.76	1.17	56.0
Approac	ch	189	8.9	0.114	7.0	NA	1.7	13.0	0.76	0.04	58.6
All Vehi	cles	660	5.1	0.241	2.3	NA	1.7	13.0	0.23	0.03	71.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: 5. Greenwell Pt-Mayfield (Future Fri PM-120th HH)

13S1231000 - West Culburra Subdivision Greenwell Point Road-Mayfield Road Friday PM (1600-1700) - Équivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Movem	nent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South E	ast: May	field Road									
21	L	6	0.0	0.045	19.7	LOS B	0.1	1.1	0.57	0.64	41.8
23	R	6	25.0	0.045	21.0	LOS B	0.1	1.1	0.57	0.89	41.9
Approac	ch	13	12.5	0.045	20.3	LOS B	0.1	1.1	0.57	0.77	41.9
North E	ast: Gree	nwell Point Ro	ad (NE)								
24	L	1	0.0	0.118	10.1	LOSA	0.0	0.0	0.00	1.73	57.1
25	Т	220	5.7	0.118	0.0	LOSA	0.0	0.0	0.00	0.00	80.0
Approac	ch	221	5.7	0.118	0.0	NA	0.0	0.0	0.00	0.01	79.9
South V	Vest: Gre	enwell Point R	oad (SW)								
31	T	484	1.4	0.274	3.2	LOSA	3.8	26.6	0.66	0.00	61.0
32	R	15	0.0	0.274	13.4	LOSA	3.8	26.6	0.66	1.17	60.2
Approac	ch	499	1.4	0.274	3.5	NA	3.8	26.6	0.66	0.03	61.0
All Vehic	cles	733	2.9	0.274	2.8	NA	3.8	26.6	0.46	0.04	65.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: 6. Greenwell Pt-Millbank-Worrigee (Future Fri AM-120th HH)

13S1231000 - West Culburra Subdivision Greenwell Point Road-Millbank Road-Worrigee Road Friday AM (0800-0900) - Equivalent 120th HH Future - Full Site Development Stop (Two-Way)

Mover	nent Per	formance - \	/ehicles								
Mov ID		Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South:	Worrigee		70	V/C	300		VOII	- '''		per veri	KIII/II
1	L	69	5.1	0.464	23.8	LOS B	2.5	18.3	0.73	1.13	40.3
2	T	118	3.0	0.464	22.4	LOS B	2.5	18.3	0.73	1.11	38.2
3	R	24	0.0	0.093	23.8	LOS B	0.3	2.2	0.73	1.00	40.0
Approa	ich	212	3.3	0.464	23.0	LOS B	2.5	18.3	0.73	1.10	39.1
East: G	reenwell l	Point Road (E)	)								
4	L	15	0.0	0.230	10.1	LOSA	0.0	0.0	0.00	1.64	57.1
5	Т	425	3.0	0.230	0.0	LOSA	0.0	0.0	0.00	0.00	80.0
6	R	38	7.4	0.034	11.4	LOSA	0.1	1.0	0.33	0.68	55.1
Approa	ich	478	3.2	0.230	1.2	NA	0.1	1.0	0.03	0.10	76.7
North: I	Millbank R	Road									
7	L	18	16.7	0.036	14.1	LOS A	0.1	0.6	0.33	0.86	48.0
8	T	26	13.6	0.155	25.4	LOS B	0.6	4.2	0.77	1.00	36.6
9	R	12	0.0	0.155	25.6	LOS B	0.6	4.2	0.77	1.00	38.9
Approa	ich	56	11.8	0.155	21.8	LOS B	0.6	4.2	0.62	0.96	40.4
West: 0	Greenwell	Point Road (V	V)								
10	L	47	7.5	0.123	10.4	LOSA	0.0	0.0	0.00	1.30	57.1
11	Т	178	9.1	0.123	0.0	LOSA	0.0	0.0	0.00	0.00	80.0
12	R	61	5.8	0.069	12.6	LOSA	0.3	1.9	0.47	0.76	53.9
Approa	ich	286	8.1	0.123	4.4	NA	0.3	1.9	0.10	0.38	69.2
All Veh	icles	1032	5.1	0.464	7.7	NA	2.5	18.3	0.22	0.43	60.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: 6. Greenwell Pt-Millbank-Worrigee (Future Fri PM-120th HH)

13S1231000 - West Culburra Subdivision Greenwell Point Road-Millbank Road-Worrigee Road Friday PM (1600-1700) - Equivalent 120th HH Future - Full Site Development Stop (Two-Way)

Movem	nent Pe	rformance - V	/ehicles								
	_	Demand	1.0.7	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0 4 1	A /	veh/h	%	v/c	sec		veh	m		per veh	km/h
	Norrigee				400				0.50		40.0
1	L	58	0.0	0.227	18.2	LOS B	0.9	6.0	0.52	0.85	43.8
2	Т	47	0.0	0.227	17.0	LOS B	0.9	6.0	0.52	1.00	41.7
3	R	24	5.3	0.121	29.0	LOS C	0.4	3.0	0.80	1.00	37.1
Approac	ch	129	1.0	0.227	19.8	LOS B	0.9	6.0	0.57	0.93	41.6
East: G	reenwell	Point Road (E)									
4	L	24	7.1	0.117	10.4	LOSA	0.0	0.0	0.00	1.49	57.1
5	Т	195	5.5	0.117	0.0	LOS A	0.0	0.0	0.00	0.00	80.0
6	R	18	0.0	0.021	12.5	LOSA	0.1	0.5	0.49	0.73	53.7
Approac	ch	237	5.2	0.117	2.0	NA	0.1	0.5	0.04	0.21	74.8
North: N	/lillbank F	Road									
7	L	26	0.0	0.052	14.7	LOS B	0.1	0.9	0.49	0.92	46.7
8	Т	54	2.8	0.283	26.5	LOS B	1.1	8.0	0.80	1.04	35.6
9	R	18	0.0	0.283	27.4	LOS B	1.1	8.0	0.80	1.04	37.9
Approac	ch	98	1.5	0.283	23.5	LOS B	1.1	8.0	0.72	1.00	38.7
West: G	reenwell	Point Road (W	/)								
10	L	17	0.0	0.253	10.1	LOSA	0.0	0.0	0.00	1.64	57.1
11	Т	474	0.7	0.253	0.0	LOSA	0.0	0.0	0.00	0.00	80.0
12	R	115	1.3	0.099	11.1	LOSA	0.4	2.8	0.33	0.69	55.1
Approac	ch	605	0.8	0.253	2.4	NA	0.4	2.8	0.06	0.18	73.6
All Vehic	cles	1069	1.9	0.283	6.3	NA	1.1	8.0	0.18	0.35	63.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1231000 - West Culburra Subdivision Princes Highway-Kalandar Street

Friday AM (0800-0900) - Equivalent 120th HH

Future - Full Site Development

Signals - Fixed Time Cycle Time = 135 seconds (Optimum Cycle Time - Minimum Delay)

Movem	ent Per	formance - \	/ehicles								
Marrido	T	Demand	1.07	Deg.	Average	Level of	95% Back		Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South: E	Princes H	veh/h	%	v/c	sec		veh	m		per veh	km/h
		wy (3) 3	0.0	1.082	128.4	LOS F	44.0	322.3	1.00	1.22	14.1
1	L	_									
2	T	962	5.6	1.082	136.7	LOSF	53.8	394.6	1.00	1.32	13.4
3	R	42	0.0	0.510	81.9	LOS F	2.9	20.4	1.00	0.74	19.2
Approac	ch	1007	5.3	1.082	134.4	LOS F	53.8	394.6	1.00	1.30	13.5
East: Ka	alandar S	t (E)									
4	L	41	9.4	1.072	140.4	LOS F	60.2	433.4	1.00	1.31	9.1
5	Т	325	3.1	1.072	131.1	LOS F	60.2	433.4	1.00	1.31	8.5
6	R	825	2.9	1.072	153.6	LOS F	64.8	465.1	1.00	1.28	8.4
Approac	ch	1192	3.1	1.072	147.0	LOS F	64.8	465.1	1.00	1.29	8.4
North: F	rinces H	wy (N)									
7	L	277	7.3	0.279	9.2	LOSA	0.8	6.2	0.07	0.64	53.4
8	Т	898	7.3	0.751	42.2	LOS C	24.8	184.6	0.90	0.79	29.4
9	R	157	11.3	1.042	112.3	LOS F	12.0	92.4	1.00	1.09	15.2
Approac	ch	1332	7.8	1.042	43.6	LOS D	24.8	184.6	0.74	0.80	29.0
West: K	alandar S	St (W)									
10	L	52	13.6	0.719	67.4	LOS E	9.4	68.8	0.96	0.91	18.7
11	Т	159	1.6	0.719	58.6	LOS E	13.3	96.3	0.97	0.87	17.7
12	R	156	5.3	0.719	69.9	LOS E	13.3	96.3	1.00	0.86	17.8
Approac	ch	366	4.9	0.719	64.7	LOS E	13.3	96.3	0.98	0.87	17.9
All Vehic	cles	3897	5.4	1.082	100.7	LOSF	64.8	465.1	0.91	1.08	14.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestrians	;					
May ID	Description	Demand	Average		Average Back		Prop.	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	53	44.8	LOS E	0.2	0.2	0.81	0.81
P3	Across E approach	53	37.8	LOS D	0.2	0.2	0.75	0.75
P5	Across N approach	53	61.6	LOS F	0.2	0.2	0.96	0.96
All Ped	estrians	159	48.1	LOS E			0.84	0.84

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Project: P:\12S1200-1299\12S1231000 - West Cullburra MWT\Modelling\SIDRA\130925sid-12S1231000 West

Culburra Subdivision.sip



13S1231000 - West Culburra Subdivision Princes Highway-Kalandar Street

Friday PM (1600-1700) - Equivalent 120th HH

Future - Full Site Development

Signals - Fixed Time Cycle Time = 125 seconds (Optimum Cycle Time - Minimum Delay)

Mov ID Turn         Flow veh/h         HV         Satin veh/h         Delay sec         Service veh land         Overhicles between the per veh land         Distance per veh land         Stop Rate per veh land         Special section           South: Princes Hwy (S)         1         L         12         0.0         1.081         129.8         LOS F         33.1         236.8         1.00         1.20           2         T         785         2.6         1.081         135.9         LOS F         41.9         300.0         1.00         1.30           3         R         115         1.4         1.114         194.3         LOS F         13.1         92.8         1.00         1.24           Approach         912         2.4         1.114         143.2         LOS F         50.7         361.5         1.00         1.24           4         L         103         1.5         1.124         176.9         LOS F         50.7         361.5         1.00         1.44           5         T         254         2.4         1.124         196.9         LOS F         50.7         361.5         1.00         1.44           4         L         103         1.5         1.124         1	Marria		ر د	/-  - ! -								
Mov ID   Turn   Flow veh/h   W   Satin veh/h   Service   Vehicles   Distance   Queued   Stop Rate   Spread veh/h   Welvh   W	Moven	nent Per		enicles		A	1 1	050/ D			T##:	A
Veh/h   %   V/c   Sec   Veh   m   Per Veh   South: Princes Hwy (S)	Mov ID	Turn		HV								Average Speed
South: Princes Hwy (S)  1 L 12 0.0 1.081 129.8 LOS F 33.1 236.8 1.00 1.20 2 T 785 2.6 1.081 135.9 LOS F 41.9 300.0 1.00 1.30 3 R 115 1.4 1.114 194.3 LOS F 13.1 92.8 1.00 1.24  Approach 912 2.4 1.114 143.2 LOS F 41.9 300.0 1.00 1.29  East: Kalandar St (E)  4 L 103 1.5 1.124 176.9 LOS F 50.7 361.5 1.00 1.44 5 T 254 2.4 1.124 167.9 LOS F 50.7 361.5 1.00 1.44 6 R 583 1.8 1.124 194.1 LOS F 53.2 378.4 1.00 1.41  Approach 940 2.0 1.124 185.2 LOS F 53.2 378.4 1.00 1.42  North: Princes Hwy (N)  7 L 871 1.1 0.982 33.1 LOS C 18.5 130.6 0.22 0.78 8 T 1302 3.1 1.081 143.3 LOS F 69.8 501.5 1.00 1.50 9 R 138 5.4 0.576 41.9 LOS C 5.7 41.7 0.98 0.79  Approach 2311 2.5 1.081 95.7 LOS F 69.8 501.5 0.70 1.19  West: Kalandar St (W)  10 L 109 9.5 1.000 54.9 LOS D 13.7 99.2 0.95 0.90 11 T 323 0.0 1.143 125.4 LOS F 58.6 412.2 1.00 1.56  Approach 736 1.8 1.143 151.8 LOS F 58.6 412.2 1.00 1.56	11101 12	1 3111					Service			Queueu		speed km/h
2       T       785       2.6       1.081       135.9       LOS F       41.9       300.0       1.00       1.30         3       R       115       1.4       1.114       194.3       LOS F       13.1       92.8       1.00       1.24         Approach       912       2.4       1.114       143.2       LOS F       41.9       300.0       1.00       1.24         East: Kalandar St (E)         4       L       103       1.5       1.124       176.9       LOS F       50.7       361.5       1.00       1.44         5       T       254       2.4       1.124       167.9       LOS F       50.7       361.5       1.00       1.44         6       R       583       1.8       1.124       194.1       LOS F       53.2       378.4       1.00       1.41         Approach       940       2.0       1.124       185.2       LOS F       53.2       378.4       1.00       1.42         North: Princes Hwy (N)         7       L       871       1.1       0.982       33.1       LOS C       18.5       130.6       0.22       0.78         8	South:	Princes H			V/O			VCII			per veri	KITI/II
3 R 115 1.4 1.114 194.3 LOS F 13.1 92.8 1.00 1.24 Approach 912 2.4 1.114 143.2 LOS F 41.9 300.0 1.00 1.29  East: Kalandar St (E)  4 L 103 1.5 1.124 176.9 LOS F 50.7 361.5 1.00 1.44 5 T 254 2.4 1.124 167.9 LOS F 50.7 361.5 1.00 1.44 6 R 583 1.8 1.124 194.1 LOS F 53.2 378.4 1.00 1.41 Approach 940 2.0 1.124 185.2 LOS F 53.2 378.4 1.00 1.42  North: Princes Hwy (N)  7 L 871 1.1 0.982 33.1 LOS C 18.5 130.6 0.22 0.78 8 T 1302 3.1 1.081 143.3 LOS F 69.8 501.5 1.00 1.50 9 R 138 5.4 0.576 41.9 LOS C 5.7 41.7 0.98 0.79  Approach 2311 2.5 1.081 95.7 LOS F 69.8 501.5 0.70 1.19  West: Kalandar St (W)  10 L 109 9.5 1.000 54.9 LOS D 13.7 99.2 0.95 0.90 11 T 323 0.0 1.143 125.4 LOS F 58.6 412.2 0.97 1.21 12 R 303 1.0 1.143 215.0 LOS F 58.6 412.2 0.97 1.21 12 R 303 1.0 1.143 215.0 LOS F 58.6 412.2 0.98 1.31	1	L	12	0.0	1.081	129.8	LOS F	33.1	236.8	1.00	1.20	14.0
Approach         912         2.4         1.114         143.2         LOS F         41.9         300.0         1.00         1.29           East: Kalandar St (E)           4         L         103         1.5         1.124         176.9         LOS F         50.7         361.5         1.00         1.44           5         T         254         2.4         1.124         167.9         LOS F         50.7         361.5         1.00         1.44           6         R         583         1.8         1.124         194.1         LOS F         53.2         378.4         1.00         1.41           Approach         940         2.0         1.124         185.2         LOS F         53.2         378.4         1.00         1.42           North: Princes Hwy (N)         T         L         871         1.1         0.982         33.1         LOS C         18.5         130.6         0.22         0.78           8         T         1302         3.1         1.081         143.3         LOS F         69.8         501.5         1.00         1.50           9         R         138         5.4         0.576         41.9         LOS F	2	Т	785	2.6	1.081	135.9	LOS F	41.9	300.0	1.00	1.30	13.4
East: Kalandar St (E)  4 L 103 1.5 1.124 176.9 LOS F 50.7 361.5 1.00 1.44 5 T 254 2.4 1.124 167.9 LOS F 50.7 361.5 1.00 1.44 6 R 583 1.8 1.124 194.1 LOS F 53.2 378.4 1.00 1.41  Approach 940 2.0 1.124 185.2 LOS F 53.2 378.4 1.00 1.42  North: Princes Hwy (N)  7 L 871 1.1 0.982 33.1 LOS C 18.5 130.6 0.22 0.78 8 T 1302 3.1 1.081 143.3 LOS F 69.8 501.5 1.00 1.50 9 R 138 5.4 0.576 41.9 LOS C 5.7 41.7 0.98 0.79  Approach 2311 2.5 1.081 95.7 LOS F 69.8 501.5 0.70 1.19  West: Kalandar St (W)  10 L 109 9.5 1.000 54.9 LOS D 13.7 99.2 0.95 0.90 11 T 323 0.0 1.143 125.4 LOS F 58.6 412.2 0.97 1.21 12 R 303 1.0 1.143 215.0 LOS F 58.6 412.2 0.98 1.31	3	R	115	1.4	1.114	194.3	LOS F	13.1	92.8	1.00	1.24	9.6
4       L       103       1.5       1.124       176.9       LOS F       50.7       361.5       1.00       1.44         5       T       254       2.4       1.124       167.9       LOS F       50.7       361.5       1.00       1.44         6       R       583       1.8       1.124       194.1       LOS F       53.2       378.4       1.00       1.41         Approach       940       2.0       1.124       185.2       LOS F       53.2       378.4       1.00       1.41         Approach       940       2.0       1.124       185.2       LOS F       53.2       378.4       1.00       1.42         North: Princes Hwy (N)         7       L       871       1.1       0.982       33.1       LOS C       18.5       130.6       0.22       0.78         8       T       1302       3.1       1.081       143.3       LOS F       69.8       501.5       1.00       1.50         9       R       138       5.4       0.576       41.9       LOS F       69.8       501.5       0.70       1.19         West: Kalandar St (W)	Approa	ch	912	2.4	1.114	143.2	LOS F	41.9	300.0	1.00	1.29	12.8
5         T         254         2.4         1.124         167.9         LOS F         50.7         361.5         1.00         1.44         6 R         583         1.8         1.124         194.1         LOS F         53.2         378.4         1.00         1.41           Approach         940         2.0         1.124         185.2         LOS F         53.2         378.4         1.00         1.42           North: Princes Hwy (N)         Volume Transport of the princes Hwy (N)           7         L         871         1.1         0.982         33.1         LOS C         18.5         130.6         0.22         0.78         0.78         0.78         0.79         0.78         0.79         0.79         0.79         0.79         0.79         0.79         0.79         0.79         0.79         0.79         0.79         0.70         1.19         0.79         0.70         1.19         0.79         0.70         1.19         0.79         0.70         1.19         0.70         1.19         0.70         1.19         0.70         1.19         0.70         1.19         0.70         1.19         0.70         0.70         1.19         0.70         0.70         1.19         0.70	East: K	alandar S	t (E)									
6       R       583       1.8       1.124       194.1       LOS F       53.2       378.4       1.00       1.41         Approach       940       2.0       1.124       185.2       LOS F       53.2       378.4       1.00       1.42         North: Princes Hwy (N)         7       L       871       1.1       0.982       33.1       LOS C       18.5       130.6       0.22       0.78         8       T       1302       3.1       1.081       143.3       LOS F       69.8       501.5       1.00       1.50         9       R       138       5.4       0.576       41.9       LOS C       5.7       41.7       0.98       0.79         Approach       2311       2.5       1.081       95.7       LOS F       69.8       501.5       0.70       1.19         West: Kalandar St (W)         10       L       109       9.5       1.000 <sup>3</sup> 54.9       LOS D       13.7       99.2       0.95       0.90         11       T       323       0.0       1.143       125.4       LOS F       58.6       412.2       0.97       1.21         12	4	L	103	1.5	1.124	176.9	LOS F	50.7	361.5	1.00	1.44	7.4
Approach       940       2.0       1.124       185.2       LOS F       53.2       378.4       1.00       1.42         North: Princes Hwy (N)       7       L       871       1.1       0.982       33.1       LOS C       18.5       130.6       0.22       0.78         8       T       1302       3.1       1.081       143.3       LOS F       69.8       501.5       1.00       1.50         9       R       138       5.4       0.576       41.9       LOS C       5.7       41.7       0.98       0.79         Approach       2311       2.5       1.081       95.7       LOS F       69.8       501.5       0.70       1.19         West: Kalandar St (W)         10       L       109       9.5       1.000³       54.9       LOS D       13.7       99.2       0.95       0.90         11       T       323       0.0       1.143       125.4       LOS F       58.6       412.2       0.97       1.21         12       R       303       1.0       1.143       215.0       LOS F       58.6       412.2       0.98       1.31           Approach	5	T	254	2.4	1.124	167.9	LOS F	50.7	361.5	1.00	1.44	6.9
North: Princes Hwy (N)  7 L 871 1.1 0.982 33.1 LOS C 18.5 130.6 0.22 0.78 8 T 1302 3.1 1.081 143.3 LOS F 69.8 501.5 1.00 1.50 9 R 138 5.4 0.576 41.9 LOS C 5.7 41.7 0.98 0.79  Approach 2311 2.5 1.081 95.7 LOS F 69.8 501.5 0.70 1.19  West: Kalandar St (W)  10 L 109 9.5 1.000 54.9 LOS D 13.7 99.2 0.95 0.90 11 T 323 0.0 1.143 125.4 LOS F 58.6 412.2 0.97 1.21 12 R 303 1.0 1.143 215.0 LOS F 58.6 412.2 1.00 1.56  Approach 736 1.8 1.143 151.8 LOS F 58.6 412.2 0.98 1.31	6	R	583	1.8	1.124	194.1	LOS F	53.2	378.4	1.00	1.41	6.8
7       L       871       1.1       0.982       33.1       LOS C       18.5       130.6       0.22       0.78         8       T       1302       3.1       1.081       143.3       LOS F       69.8       501.5       1.00       1.50         9       R       138       5.4       0.576       41.9       LOS C       5.7       41.7       0.98       0.79         Approach       2311       2.5       1.081       95.7       LOS F       69.8       501.5       0.70       1.19         West: Kalandar St (W)         10       L       109       9.5       1.000 <sup>3</sup> 54.9       LOS D       13.7       99.2       0.95       0.90         11       T       323       0.0       1.143       125.4       LOS F       58.6       412.2       0.97       1.21         12       R       303       1.0       1.143       215.0       LOS F       58.6       412.2       1.00       1.56         Approach       736       1.8       1.143       151.8       LOS F       58.6       412.2       0.98       1.31	Approa	ch	940	2.0	1.124	185.2	LOS F	53.2	378.4	1.00	1.42	6.9
8       T       1302       3.1       1.081       143.3       LOS F       69.8       501.5       1.00       1.50         9       R       138       5.4       0.576       41.9       LOS C       5.7       41.7       0.98       0.79         Approach       2311       2.5       1.081       95.7       LOS F       69.8       501.5       0.70       1.19         West: Kalandar St (W)         10       L       109       9.5       1.000 <sup>3</sup> 54.9       LOS D       13.7       99.2       0.95       0.90         11       T       323       0.0       1.143       125.4       LOS F       58.6       412.2       0.97       1.21         12       R       303       1.0       1.143       215.0       LOS F       58.6       412.2       1.00       1.56         Approach       736       1.8       1.143       151.8       LOS F       58.6       412.2       0.98       1.31	North: F	Princes H	wy (N)									
9         R         138         5.4         0.576         41.9         LOS C         5.7         41.7         0.98         0.79           Approach         2311         2.5         1.081         95.7         LOS F         69.8         501.5         0.70         1.19           West: Kalandar St (W)           10         L         109         9.5         1.000 <sup>3</sup> 54.9         LOS D         13.7         99.2         0.95         0.90           11         T         323         0.0         1.143         125.4         LOS F         58.6         412.2         0.97         1.21           12         R         303         1.0         1.143         215.0         LOS F         58.6         412.2         1.00         1.56           Approach         736         1.8         1.143         151.8         LOS F         58.6         412.2         0.98         1.31	7	L	871	1.1	0.982	33.1	LOS C	18.5	130.6	0.22	0.78	33.8
Approach         2311         2.5         1.081         95.7         LOS F         69.8         501.5         0.70         1.19           West: Kalandar St (W)           10         L         109         9.5         1.000³         54.9         LOS D         13.7         99.2         0.95         0.90           11         T         323         0.0         1.143         125.4         LOS F         58.6         412.2         0.97         1.21           12         R         303         1.0         1.143         215.0         LOS F         58.6         412.2         1.00         1.56           Approach         736         1.8         1.143         151.8         LOS F         58.6         412.2         0.98         1.31	8	Т	1302	3.1	1.081	143.3	LOS F	69.8	501.5	1.00	1.50	12.9
West: Kalandar St (W)  10 L 109 9.5 1.000 <sup>3</sup> 54.9 LOS D 13.7 99.2 0.95 0.90  11 T 323 0.0 1.143 125.4 LOS F 58.6 412.2 0.97 1.21  12 R 303 1.0 1.143 215.0 LOS F 58.6 412.2 1.00 1.56  Approach 736 1.8 1.143 151.8 LOS F 58.6 412.2 0.98 1.31	9	R	138	5.4	0.576	41.9	LOS C	5.7	41.7	0.98	0.79	29.6
10       L       109       9.5       1.000³       54.9       LOS D       13.7       99.2       0.95       0.90         11       T       323       0.0       1.143       125.4       LOS F       58.6       412.2       0.97       1.21         12       R       303       1.0       1.143       215.0       LOS F       58.6       412.2       1.00       1.56         Approach       736       1.8       1.143       151.8       LOS F       58.6       412.2       0.98       1.31	Approa	ch	2311	2.5	1.081	95.7	LOS F	69.8	501.5	0.70	1.19	17.4
11     T     323     0.0     1.143     125.4     LOS F     58.6     412.2     0.97     1.21       12     R     303     1.0     1.143     215.0     LOS F     58.6     412.2     1.00     1.56       Approach     736     1.8     1.143     151.8     LOS F     58.6     412.2     0.98     1.31	West: K	Kalandar S	St (W)									
12     R     303     1.0     1.143     215.0     LOS F     58.6     412.2     1.00     1.56       Approach     736     1.8     1.143     151.8     LOS F     58.6     412.2     0.98     1.31	<mark>10</mark>	L	<mark>109</mark>	9.5	<mark>1.000</mark> <sup>3</sup>	54.9	LOS D	13.7	99.2	0.95	0.90	21.5
Approach 736 1.8 1.143 151.8 LOS F 58.6 412.2 0.98 1.31	11	T	323	0.0	1.143	125.4	LOS F	58.6	412.2	0.97	1.21	10.2
	12	R	303	1.0	1.143	215.0	LOS F	58.6	412.2	1.00	1.56	7.1
All Vehicles 4898 2.3 1.143 130.2 LOS F 69.8 501.5 0.86 1.27	Approa	ch	736	1.8	1.143	151.8	LOSF	58.6	412.2	0.98	1.31	9.2
	All Vehi	icles	4898	2.3	1.143	130.2	LOSF	69.8	501.5	0.86	1.27	12.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Moven	nent Performance -	Pedestrians	S					
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P1	Across S approach	53	51.1	LOS E	0.2	0.2	0.90	0.90
P3	Across E approach	53	36.1	LOS D	0.1	0.1	0.76	0.76
P5	Across N approach	53	51.1	LOS E	0.2	0.2	0.90	0.90
All Ped	estrians	159	46.1	LOS E			0.86	0.86

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Culburra Subdivision sip



Site: 8. Coonamia-Currarong-Forest (Future Fri AM-120th HH)

13S1231000 - West Culburra Subdivision Coonamia Road-Currarong Road-Forest Road Friday AM (0800-0900) - Equivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Movem	nent Per	formance - V	ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
East: Cu	urrarong l		/0	V/C	300		VCII			per veri	KIII/II
5	T	13	0.0	0.042	1.0	LOSA	0.2	1.3	0.33	0.00	78.5
6	R	40	0.0	0.042	13.4	LOSA	0.2	1.3	0.33	0.77	68.2
Approac	ch	53	0.0	0.042	10.4	NA	0.2	1.3	0.33	0.59	70.5
North: C	Coonamia	Road									
7	L	7	0.0	0.012	13.1	LOSA	0.0	0.2	0.23	0.68	67.5
9	R	87	1.8	0.127	14.3	LOSA	0.5	3.6	0.38	0.74	66.5
Approac	ch	95	1.6	0.127	14.3	LOS A	0.5	3.6	0.36	0.74	66.6
West: F	orest Roa	ad									
10	L	223	1.8	0.132	12.7	LOSA	0.0	0.0	0.00	0.79	69.1
11	Т	19	6.3	0.132	0.0	LOSA	0.0	0.0	0.00	0.00	100.0
Approac	ch	242	2.1	0.132	11.7	NA	0.0	0.0	0.00	0.72	70.9
All Vehic	cles	389	1.7	0.132	12.2	NA	0.5	3.6	0.13	0.71	69.7

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: 8. Coonamia-Currarong-Forest (Future Fri PM-120th HH)

13S1231000 - West Culburra Subdivision Coonamia Road-Currarong Road-Forest Road Friday PM (1600-1700) - Equivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Movem	nent Per	formance - V	ehicles								
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back o Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
East: C	urrarong l	Road									
5	Т	15	0.0	0.020	0.7	LOS A	0.1	0.7	0.30	0.00	81.7
6	R	15	0.0	0.020	13.2	LOSA	0.1	0.7	0.30	0.86	69.4
Approac	ch	29	0.0	0.020	7.0	NA	0.1	0.7	0.30	0.43	75.2
North: C	Coonamia	Road									
7	L	37	0.0	0.059	13.1	LOSA	0.1	0.9	0.22	0.69	67.6
9	R	214	4.0	0.293	14.4	LOSA	1.4	10.1	0.39	0.74	66.7
Approac	ch	251	3.4	0.293	14.2	LOS A	1.4	10.1	0.36	0.73	66.8
West: F	orest Roa	ad									
10	L	163	5.4	0.109	13.0	LOS A	0.0	0.0	0.00	0.83	69.1
11	Т	34	4.3	0.109	0.0	LOSA	0.0	0.0	0.00	0.00	100.0
Approac	ch	197	5.2	0.109	10.8	NA	0.0	0.0	0.00	0.69	73.0
All Vehi	cles	477	4.0	0.293	12.4	NA	1.4	10.1	0.21	0.70	69.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: 9. Kalandar St-Kinghorne St (Future Fri AM-120th HH)

13S1231000 - West Culburra Subdivision Kalandar Street-Kinghorne Street-Albatross Road Friday AM (0800-0900) - Equivalent 120th HH Future - Full Site Development Roundabout

Movem	ent Per	formance - V	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: K	(inghorne	Sttreet									
1	L	31	7.7	0.722	20.0	LOS B	10.1	72.6	0.98	1.13	39.5
2	Т	483	3.4	0.722	18.5	LOS B	10.1	72.6	0.98	1.12	39.7
3	R	59	0.0	0.722	23.3	LOS B	10.1	72.6	0.98	1.12	37.9
Approac	h	573	3.3	0.722	19.1	LOS B	10.1	72.6	0.98	1.12	39.5
East: Ka	landar S	treet									
4	L	327	7.5	0.437	7.9	LOSA	2.8	20.6	0.46	0.59	45.1
6	R	161	2.3	0.437	12.6	LOSA	2.8	20.6	0.46	0.78	42.0
Approac	h	488	5.8	0.437	9.4	LOS A	2.8	20.6	0.46	0.65	44.0
North: K	inghorne	Street									
7	L	133	3.7	0.333	8.8	LOS A	2.3	16.3	0.58	0.66	47.1
8	Т	31	0.0	0.333	7.9	LOSA	2.3	16.3	0.58	0.63	47.1
9	R	176	3.4	0.333	12.0	LOSA	2.3	16.3	0.58	0.74	45.5
Approac	h	339	3.2	0.333	10.4	LOSA	2.3	16.3	0.58	0.70	46.2
South W	est: Alba	tross Road									
30	L	261	4.5	0.741	20.3	LOS B	9.7	71.5	1.00	1.19	38.1
32	R	221	8.1	0.741	24.5	LOS B	9.7	71.5	1.00	1.20	36.9
Approac	h	482	6.2	0.741	22.2	LOS B	9.7	71.5	1.00	1.20	37.5
All Vehic	eles	1882	4.7	0.741	15.8	LOS B	10.1	72.6	0.78	0.94	40.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Site: 9. Kalandar St-Kinghorne St (Future Fri PM-120th HH)

13S1231000 - West Culburra Subdivision Kalandar Street-Kinghorne Street-Albatross Road Friday AM (1600-1700) - Equivalent 120th HH Future - Full Site Development Roundabout

		Demand		Deg.	Average	Level of	95% Back o	f Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/r
South: K	inghorne	Sttreet									
1	L	34	0.0	0.584	15.5	LOS B	5.8	40.6	0.90	1.00	42.6
2	Т	326	0.0	0.584	14.2	LOS A	5.8	40.6	0.90	0.99	42.8
3	R	79	0.0	0.584	19.2	LOS B	5.8	40.6	0.90	1.02	40.6
Approac	h	439	0.0	0.584	15.2	LOS B	5.8	40.6	0.90	0.99	42.4
East: Ka	landar St	reet									
4	L	282	0.0	0.446	8.9	LOS A	2.9	20.5	0.62	0.71	43.9
6	R	119	0.0	0.446	13.8	LOS A	2.9	20.5	0.62	0.84	40.9
Approac	h	401	0.0	0.446	10.4	LOSA	2.9	20.5	0.62	0.75	42.9
North: K	inghorne	Street									
7	L	323	0.0	0.783	15.8	LOS B	11.6	81.1	0.98	1.05	41.6
8	Т	80	0.0	0.783	15.0	LOS B	11.6	81.1	0.98	1.04	41.7
9	R	304	0.0	0.783	18.9	LOS B	11.6	81.1	0.98	1.05	40.2
Approac	h	707	0.0	0.783	17.0	LOS B	11.6	81.1	0.98	1.05	41.0
South W	est: Alba	tross Road									
30	L	300	0.0	0.789	17.5	LOS B	11.9	83.0	1.00	1.14	39.8
32	R	348	0.0	0.789	21.6	LOS B	11.9	83.0	1.00	1.14	38.4
Approac	h	648	0.0	0.789	19.7	LOS B	11.9	83.0	1.00	1.14	39.1
All Vehic	eles	2196	0.0	0.789	16.3	LOS B	11.9	83.0	0.91	1.01	40.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Site: 10. Princes Hwy-Forest (Future Fri AM-120th HH)

13S1231000 - West Culburra Subdivision Princes Highway-Forest Road Friday AM (0800-0900) - Equivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Moven	nent Per	formance - \	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: I	Princes H	lwy (S)									
2	Т	1413	3.1	0.739	0.4	Χ	X	X	X	0.00	98.7
3	R	46	13.0	0.073	17.4	LOS B	0.3	2.1	0.57	0.84	56.6
Approac	ch	1459	3.4	0.739	0.9	NA	0.3	2.1	0.02	0.03	97.0
South E	ast: Fore	st Road (Med	ian RT)								
23	R	108	2.3	0.059	8.1	LOS A	0.0	0.0	0.00	0.61	53.3
Approac	ch	108	2.3	0.059	8.1	LOSA	0.0	0.0	0.00	0.61	53.3
East: Fo	orest Roa	ıd									
4	L	49	3.3	0.109	13.2	LOSA	0.3	2.1	0.54	0.82	51.3
6	R	108	2.3	0.233	15.2	LOS B	0.8	6.1	0.61	0.89	49.4
Approac	ch	158	2.6	0.233	14.5	LOS B	8.0	6.1	0.59	0.87	50.0
North: F	Princes H	wy (N)									
7	L	64	8.3	0.037	13.0	LOSA	0.0	0.0	0.00	0.76	63.3
8	Т	521	16.0	0.295	0.0	LOS A	0.0	0.0	0.00	0.00	100.0
Approac	ch	585	15.1	0.295	1.4	NA	0.0	0.0	0.00	0.08	95.3
All Vehi	cles	2311	6.3	0.739	2.3	NA	0.8	6.1	0.05	0.13	88.9

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: 10. Princes Hwy-Forest (Future Fri PM-120th HH)

13S1231000 - West Culburra Subdivision Princes Highway-Forest Road Friday PM (1600-1700) - Equivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Mover	nent Per	formance - V	/ehicles								
Mov ID		Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
	Princes H	, ,									
2	Т	638	6.0	0.340	0.1	Х	Х	X	Х	0.00	99.8
3	R	64	2.9	0.466	49.1	LOS D	1.6	11.6	0.95	1.03	30.8
Approa	ich	702	5.7	0.466	4.6	NA	1.6	11.6	0.09	0.09	86.1
South E	East: Fore	st Road (Media	an RT)								
23	R	75	2.2	0.041	8.1	LOSA	0.0	0.0	0.00	0.61	53.3
Approa	ich	75	2.2	0.041	8.1	LOSA	0.0	0.0	0.00	0.61	53.3
East: F	orest Roa	d									
4	L	69	0.0	0.639	63.0	LOS E	2.3	16.2	0.97	1.09	26.8
6	R	75	2.2	0.766	81.4	LOS F	3.1	21.9	0.98	1.17	22.8
Approa	ch	144	1.2	0.766	72.5	LOS F	3.1	21.9	0.97	1.13	24.6
North: I	Princes H	wy (N)									
7	L	161	2.0	0.088	12.7	LOSA	0.0	0.0	0.00	0.75	63.3
8	Т	1475	2.0	0.766	0.0	LOSA	0.0	0.0	0.00	0.00	100.0
Approa	ich	1636	2.0	0.766	1.2	NA	0.0	0.0	0.00	0.07	95.8
All Veh	icles	2557	3.0	0.766	6.4	NA	3.1	21.9	0.08	0.16	79.2

X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1231000 - West Culburra Subdivision

Princes Highway-Moss Street

Friday AM (0800-0900) - Equivalent 120th HH

Future - Full Site Development

Signals - Fixed Time Cycle Time = 135 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Per	formance - \	Vehicles								
		Demand		Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South:	Princes H	veh/h	%	v/c	sec		veh	m		per veh	km/h
1	L	5 (S)	0.0	0.974	100.6	LOSF	24.6	183.4	1.00	1.11	16.6
2	T	928	7.6	1.054	122.4	LOST	32.7	243.9	1.00	1.22	14.6
3	R	158	3.7	0.982	109.7	LOSF	13.6	98.2	1.00	1.06	14.9
Approa		1092	7.0	1.054	120.4	LOS F	32.7	243.9	1.00	1.20	14.6
Дрргоа	CII	1092	7.0	1.034	120.4	LOST	32.1	243.9	1.00	1.20	14.0
East: M	loss St (E	Ξ)									
4	L	37	3.4	0.384	48.3	LOS D	4.0	29.0	0.81	0.77	25.3
5	T	244	4.1	1.029	107.3	LOS F	48.2	350.4	0.97	1.18	13.5
6	R	276	4.6	1.029	129.2	LOS F	48.2	350.4	1.00	1.29	13.4
Approa	ch	557	4.3	1.029	114.3	LOS F	48.2	350.4	0.97	1.21	13.9
North: F	Princes H	wy (N)									
7	L	197	5.0	1.000 <sup>3</sup>	55.7	LOS D	29.0	213.4	0.99	0.92	25.2
8	T	1479	6.8	1.069	122.0	LOS F	64.4	477.1	1.00	1.33	14.5
9	R	335	3.7	1.040	111.7	LOS F	27.1	195.8	1.00	1.13	14.7
Approa	ch	2011	6.1	1.069	113.8	LOS F	64.4	477.1	1.00	1.25	15.1
West: N	∕loss St (\	N)									
10	L	126	10.3	0.268	22.5	LOS B	3.2	24.4	0.67	0.75	24.5
11	Т	135	3.6	0.538	54.2	LOS D	11.1	81.6	0.96	0.79	10.1
12	R	49	11.9	0.538	62.0	LOS E	11.1	81.6	0.96	0.82	12.6
Approa	ch	311	7.6	0.538	42.5	LOS D	11.1	81.6	0.84	0.78	14.5
All Vehi	icles	3969	6.2	1.069	110.1	LOSF	64.4	477.1	0.98	1.20	14.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Moven	nent Performance -	Pedestrian	s					
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P1	Across S approach	53	53.3	LOS E	0.2	0.2	0.89	0.89
P3	Across E approach	53	40.8	LOS E	0.2	0.2	0.78	0.78
P5	Across N approach	53	61.6	LOS F	0.2	0.2	0.96	0.96
P7	Across W approach	53	56.0	LOS E	0.2	0.2	0.91	0.91
All Pede	estrians	212	53.0	LOS E			0.88	0.88

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



13S1231000 - West Culburra Subdivision

Princes Highway-Moss Street

Friday PM (1600-1700) - Equivalent 120th HH

Future - Full Site Development

Signals - Fixed Time Cycle Time = 135 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Per	formance - \	Vehicles								
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back ( Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Princes H	lwy (S)									
1	L	13	0.0	1.149	210.3	LOS F	42.7	306.5	1.00	1.43	8.8
2	Т	1120	3.1	1.243	265.7	LOS F	59.6	428.5	1.00	1.64	7.7
3	R	115	0.0	1.191	266.0	LOS F	16.2	113.5	1.00	1.31	7.0
Approa	ch	1247	2.8	1.243	265.2	LOSF	59.6	428.5	1.00	1.61	7.6
East: M	loss St (E	<u>:</u> )									
4	L	37	4.3	0.382	56.3	LOS D	4.1	29.1	0.87	0.77	23.2
5	Т	155	0.0	1.026	104.3	LOS F	35.1	248.3	0.97	1.13	13.7
6	R	238	1.9	1.026	129.6	LOS F	35.1	248.3	1.00	1.28	13.4
Approa	ch	429	1.4	1.026	114.2	LOS F	35.1	248.3	0.98	1.18	14.0
North: F	Princes H	wy (N)									
7	L	<mark>215</mark>	3.7	1.000 <sup>3</sup>	62.2	LOS E	29.6	213.6	1.00	0.95	23.3
8	T	1525	3.5	1.238	245.0	LOS F	97.2	700.6	1.00	1.75	8.2
9	R	240	0.7	1.167	209.9	LOS F	27.8	195.8	1.00	1.29	8.7
Approa	ch	1979	3.0	1.238	220.9	LOS F	97.2	700.6	1.00	1.60	8.8
West: N	∕loss St (\	N)									
10	L	160	1.2	0.330	19.1	LOS B	3.4	24.2	0.61	0.75	26.5
11	Т	260	0.0	1.217	270.3	LOS F	113.3	798.1	1.00	1.75	2.5
12	R	135	1.1	1.217	277.7	LOS F	113.3	798.1	1.00	1.75	3.3
Approa	ch	901	0.8	1.217	229.1	LOS F	113.3	798.1	0.89	1.46	3.9
All Vehi	icles	4557	2.3	1.243	224.2	LOSF	113.3	798.1	0.98	1.43	8.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Moven	nent Performance -	Pedestrian	s					
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P1	Across S approach	53	61.6	LOS F	0.2	0.2	0.96	0.96
P3	Across E approach	53	44.0	LOS E	0.2	0.2	0.81	0.81
P5	Across N approach	53	45.6	LOS E	0.2	0.2	0.82	0.82
P7	Across W approach	53	56.0	LOS E	0.2	0.2	0.91	0.91
All Pede	estrians	212	51.8	LOS E			0.87	0.87

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



13S1231000 - West Culburra Subdivision Princes Highway-Kalandar Street Friday AM (0800-0900) - Equivalent 120th HH

10 Years - No Development

Signals - Fixed Time Cycle Time = 150 seconds (Practical Cycle Time)
Design Life Analysis (Final Year): Results for 10 years

Mover	nent Pe	rformance - \	Vehicles								
May ID	Turre	Demand	111/	Deg.	Average	Level of	95% Back		Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South:	Princes F	veh/h	%	v/c	sec		veh	m		per veh	km/h
1	L	3	0.0	1.032	94.5	LOSF	42.7	313.4	1.00	1.15	18.1
2	T	1058	5.6	1.032	103.5	LOSF	56.2	412.0	1.00	1.21	16.5
3	R	38	0.0	0.340	85.2	LOSF	2.8	19.5	1.00	0.73	18.6
Approa		1100	5.4	1.032	102.9	LOS F	56.2	412.0	1.00	1.19	16.6
Арріоа	CII	1100	3.4	1.032	102.9	LOST	30.2	412.0	1.00	1.19	10.0
East: K	alandar S	St (E)									
4	L	38	9.4	1.048	128.8	LOS F	57.4	413.3	1.00	1.23	9.8
5	T	306	3.1	1.048	119.6	LOS F	57.4	413.3	1.00	1.23	9.2
6	R	782	2.9	1.048	142.5	LOS F	61.9	443.8	1.00	1.20	9.0
Approa	ch	1126	3.1	1.048	135.8	LOS F	61.9	443.8	1.00	1.21	9.0
North: I	Princes H	lwy (N)									
7	L	259	7.3	0.235	9.2	LOS A	0.9	6.4	0.07	0.64	53.4
8	Т	988	7.3	0.762	43.1	LOS D	29.6	220.0	0.89	0.79	29.1
9	R	173	11.3	1.019	104.0	LOS F	12.8	98.5	1.00	1.06	16.1
Approa	ch	1419	7.8	1.019	44.3	LOS D	29.6	220.0	0.75	0.79	28.7
West: k	Kalandar	St (W)									
10	L	57	13.6	0.826	82.9	LOS F	11.7	85.9	0.98	1.00	16.0
11	Т	149	1.6	0.826	73.5	LOS F	16.1	117.2	0.98	0.96	15.2
12	R	171	5.3	0.826	83.5	LOS F	16.1	117.2	1.00	0.91	15.6
Approa	ch	378	5.1	0.826	79.4	LOS F	16.1	117.2	0.99	0.95	15.5
All Veh	icles	4023	5.6	1.048	89.2	LOS F	61.9	443.8	0.91	1.03	16.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestrian	S					
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P1	Across S approach	63	49.6	LOS E	0.2	0.2	0.81	0.81
P3	Across E approach	63	38.2	LOS D	0.2	0.2	0.71	0.71
P5	Across N approach	63	69.1	LOS F	0.3	0.3	0.96	0.96
All Pede	estrians	189	52.3	LOS E			0.83	0.83

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Processed: Thursday, 26 September 2013 8:31:18 AM SIDRA INTERSECTION 5.1.13.2093 Project: P:\12S1200-1299\12S1231000 - West Cullburra MWT\Modelling\SIDRA\130925sid-12S1231000 West

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13S1231000 - West Culburra Subdivision Princes Highway-Kalandar Street

Friday PM (1600-1700) - Equivalent 120th HH

10 Years - No Development

Signals - Fixed Time Cycle Time = 150 seconds (Practical Cycle Time)

Design Life Analysis (Final Year): Results for 10 years

Movem	ent Per	formance - V	/ehicles								_
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: F	Princes H	wy (S)									
1	L	13	0.0	1.142	187.2	LOS F	46.8	334.7	1.00	1.28	10.2
2	Т	864	2.6	1.142	195.4	LOS F	59.4	424.8	1.00	1.40	10.0
3	R	109	1.4	0.992	122.8	LOS F	10.4	73.5	1.00	1.04	14.1
Approac	ch	986	2.4	1.142	187.3	LOS F	59.4	424.8	1.00	1.36	10.3
East: Ka	alandar S	t (E)									
4	L	99	1.5	1.147	203.0	LOS F	56.0	399.0	1.00	1.40	6.5
5	Т	245	2.4	1.147	194.0	LOS F	56.0	399.0	1.00	1.40	6.1
6	R	567	1.8	1.147	223.9	LOS F	60.0	426.9	1.00	1.37	6.0
Approac	h	912	2.0	1.147	213.6	LOS F	60.0	426.9	1.00	1.38	6.1
North: P	rinces H	wy (N)									
7	L	832	1.1	0.864	14.4	LOSA	9.7	68.4	0.24	0.72	47.3
8	Т	1432	3.1	1.141	201.6	LOS F	98.3	706.2	1.00	1.65	9.7
9	R	152	5.4	0.562	46.7	LOS D	7.3	53.8	0.97	0.80	27.8
Approac	h	2416	2.6	1.141	127.4	LOS F	98.3	706.2	0.74	1.28	14.0
West: K	alandar S	St (W)									
<mark>10</mark>	L	<mark>120</mark>	9.5	1.000 <sup>3</sup>	57.0	LOS E	13.6	99.1	0.90	0.90	20.8
11	Т	305	0.0	1.136	143.7	LOS F	70.7	497.7	0.96	1.18	9.1
12	R	333	1.0	1.136	220.2	LOS F	70.7	497.7	1.00	1.45	7.0
Approac	h	759	1.9	1.136	163.6	LOS F	70.7	497.7	0.97	1.25	8.7
All Vehic	cles	5072	2.3	1.147	160.0	LOS F	98.3	706.2	0.87	1.31	10.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Moven	nent Performance -	Pedestrians	\$					
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Back Pedestrian	of Queue Distance	Prop. Queued	Effective Stop Rate
	Boompaon	ped/h	sec	Service	ped	m	Queueu	per ped
P1	Across S approach	63	59.9	LOS E	0.2	0.2	0.89	0.89
P3	Across E approach	63	40.3	LOS E	0.2	0.2	0.73	0.73
P5	Across N approach	63	55.5	LOS E	0.2	0.2	0.86	0.86
All Pede	estrians	189	51.9	LOS E			0.83	0.83

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



13S1231000 - West Culburra Subdivision

Princes Highway-Kalandar Street

Saturday - Equivalent 120th HH

10 Years - No Development

Signals - Fixed Time Cycle Time = 150 seconds (Practical Cycle Time)

Design Life Analysis (Final Year): Results for 10 years

Movem	ent Per	formance - \	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: F	Princes H		,,	****			7011			poi voii	1011
1	L	6	25.0	0.945	95.9	LOS F	44.6	319.5	1.00	1.07	18.0
2	Т	1056	2.6	0.945	80.1	LOS F	44.6	319.5	1.00	1.06	19.8
3	R	116	4.5	0.966	112.6	LOS F	10.5	76.1	1.00	1.01	15.1
Approac	h	1178	2.9	0.966	83.4	LOS F	44.6	319.5	1.00	1.06	19.2
East: Ka	ılandar S	t (E)									
4	L	89	0.0	0.955	88.0	LOS F	37.2	261.6	1.00	1.11	13.4
5	Т	211	0.6	0.955	79.1	LOS F	37.2	261.6	1.00	1.11	12.6
6	R	543	1.0	0.955	96.1	LOS F	37.2	261.6	1.00	1.06	12.4
Approac	h	843	8.0	0.955	91.0	LOS F	37.2	261.6	1.00	1.07	12.6
North: P	rinces H	wy (N)									
7	L	496	0.3	0.519	9.4	LOS A	2.3	16.1	80.0	0.65	52.9
8	Т	1525	1.0	0.982	75.4	LOS F	70.2	495.7	1.00	1.13	20.6
9	R	100	11.6	0.421	43.5	LOS D	4.0	30.7	0.96	0.78	29.1
Approac	:h	2120	1.3	0.982	58.5	LOS E	70.2	495.7	0.78	1.00	24.3
West: Ka	alandar S	St (W)									
10	L	73	2.0	0.907	86.1	LOS F	13.9	97.9	0.99	1.01	15.4
11	Т	168	0.0	0.907	78.9	LOS F	19.7	138.6	0.99	1.00	14.4
12	R	190	0.8	0.907	92.6	LOS F	19.7	138.6	1.00	0.99	14.4
Approac	h	431	0.7	0.907	86.2	LOS F	19.7	138.6	1.00	1.00	14.6
All Vehic	cles	4572	1.6	0.982	73.5	LOSF	70.2	495.7	0.90	1.03	19.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped				
P1	Across S approach	63	57.2	LOS E	0.2	0.2	0.87	0.87				
P3	Across E approach	63	32.7	LOS D	0.2	0.2	0.66	0.66				
P5	Across N approach	63	69.1	LOS F	0.3	0.3	0.96	0.96				
All Pede	estrians	189	53.0	LOS E			0.83	0.83				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 9. Kalandar St-Kinghorne St (10 year NO-DEV Fri AM-120th HH)

13S1231000 - West Culburra Subdivision Kalandar Street-Kinghorne Street-Albatross Road Friday AM (0800-0900) - Equivalent 120th HH 10 Years - No Development Roundabout

Design Life Analysis (Final Year): Results for 10 years

Movem	ent Perf	ormance - V	ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: Kinghorne Sttreet											
1	L	34	7.7	0.834	28.6	LOS C	16.1	116.0	1.00	1.35	34.2
2	Т	531	3.4	0.834	27.1	LOS B	16.1	116.0	1.00	1.34	34.3
3	R	65	0.0	0.834	31.9	LOS C	16.1	116.0	1.00	1.34	33.1
Approac	h	630	3.3	0.834	27.7	LOS B	16.1	116.0	1.00	1.34	34.2
East: Ka	landar St	reet									
4	L	346	7.5	0.474	8.1	LOS A	3.2	23.3	0.50	0.61	44.8
6	R	171	2.3	0.474	12.8	LOSA	3.2	23.3	0.50	0.78	41.8
Approac	h	518	5.8	0.474	9.7	LOSA	3.2	23.3	0.50	0.67	43.7
North: K	inghorne	Street									
7	L	140	3.7	0.369	9.0	LOSA	2.6	18.5	0.62	0.69	46.9
8	Т	34	0.0	0.369	8.1	LOSA	2.6	18.5	0.62	0.65	46.9
9	R	193	3.4	0.369	12.2	LOSA	2.6	18.5	0.62	0.76	45.3
Approac	h	367	3.2	0.369	10.6	LOSA	2.6	18.5	0.62	0.72	46.1
South W	est: Alba	tross Road									
30	L	287	4.5	0.882	35.7	LOS C	17.0	125.5	1.00	1.49	30.0
32	R	239	8.1	0.882	39.9	LOS C	17.0	125.5	1.00	1.49	29.4
Approac	h	526	6.2	0.882	37.6	LOS C	17.0	125.5	1.00	1.49	29.7
All Vehic	eles	2040	4.6	0.882	22.6	LOS B	17.0	125.5	0.80	1.10	36.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Site: 9. Kalandar St-Kinghorne St (10 Year NO-DEV Fri PM-120th HH)

13S1231000 - West Culburra Subdivision Kalandar Street-Kinghorne Street-Albatross Road Friday PM (1600-1700) - Equivalent 120th HH 10 Years - No Development Roundabout

Design Life Analysis (Final Year): Results for 10 years

Movem	ent Per	formance - V	ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: Kinghorne Sttreet											
1	L	37	0.0	0.680	19.4	LOS B	8.1	57.0	0.98	1.13	39.6
2	T	359	0.0	0.680	18.1	LOS B	8.1	57.0	0.98	1.13	39.8
3	R	85	0.0	0.680	23.1	LOS B	8.1	57.0	0.98	1.14	38.0
Approac	h	481	0.0	0.680	19.1	LOS B	8.1	57.0	0.98	1.13	39.4
East: Ka	landar S	treet									
4	L	306	0.0	0.502	9.7	LOSA	3.6	25.1	0.67	0.76	43.6
6	R	126	0.0	0.502	14.5	LOS B	3.6	25.1	0.67	0.87	40.2
Approac	h	432	0.0	0.502	11.1	LOSA	3.6	25.1	0.67	0.79	42.5
North: K	inghorne	Street									
7	L	346	0.0	0.879	22.2	LOS B	17.9	125.5	1.00	1.24	37.0
8	T	88	0.0	0.879	21.4	LOS B	17.9	125.5	1.00	1.24	37.1
9	R	335	0.0	0.879	25.4	LOS B	17.9	125.5	1.00	1.24	36.0
Approac	h	769	0.0	0.879	23.5	LOS B	17.9	125.5	1.00	1.24	36.5
South W	est: Alba	tross Road									
30	L	330	0.0	0.910	29.7	LOS C	21.0	146.8	1.00	1.45	32.6
32	R	375	0.0	0.910	33.9	LOS C	21.0	146.8	1.00	1.45	31.8
Approac	h	705	0.0	0.910	31.9	LOS C	21.0	146.8	1.00	1.45	32.1
All Vehic	eles	2386	0.0	0.910	22.9	LOS B	21.0	146.8	0.94	1.20	36.3

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Site: 9. Kalandar St-Kinghorne St (10 Year NO-DEV Sat-120th HH)

13S1231000 - West Culburra Subdivision Kalandar Street-Kinghorne Street-Albatross Road Saturday - Equivalent 120th HH 10 Years - No Development Roundabout

Design Life Analysis (Final Year): Results for 10 years

Movem	ent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: K	(inghorne	Sttreet									
1	L	23	12.5	0.256	10.9	LOSA	1.6	11.6	0.63	0.76	47.2
2	T	178	1.6	0.256	9.1	LOSA	1.6	11.6	0.63	0.69	47.3
3	R	32	0.0	0.256	14.0	LOSA	1.6	11.6	0.63	0.82	44.4
Approac	h	234	2.5	0.256	10.0	LOSA	1.6	11.6	0.63	0.71	46.8
East: Ka	landar S	treet									
4	L	219	6.0	0.294	7.6	LOS A	1.6	11.7	0.40	0.57	45.5
6	R	105	1.4	0.294	12.4	LOSA	1.6	11.7	0.40	0.78	42.1
Approac	h	324	4.5	0.294	9.2	LOSA	1.6	11.7	0.40	0.64	44.3
North: K	inghorne	Street									
7	L	167	0.8	0.350	8.8	LOSA	2.3	16.4	0.57	0.67	47.2
8	Т	23	0.0	0.350	7.9	LOS A	2.3	16.4	0.57	0.63	47.2
9	R	173	2.6	0.350	12.0	LOSA	2.3	16.4	0.57	0.75	45.5
Approac	h	362	1.6	0.350	10.3	LOS A	2.3	16.4	0.57	0.70	46.4
South W	est: Alba	tross Road									
30	L	177	1.4	0.421	8.3	LOS A	3.0	21.2	0.61	0.65	47.1
32	R	257	1.7	0.421	12.5	LOSA	3.0	21.2	0.61	0.77	45.4
Approac	h	434	1.6	0.421	10.8	LOSA	3.0	21.2	0.61	0.72	46.0
All Vehic	eles	1355	2.4	0.421	10.1	LOSA	3.0	21.2	0.55	0.69	45.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Site: 10. Princes Hwy-Forest (10 Year NO-DEV Fri AM-120th HH)

13S1231000 - West Culburra Subdivision Princes Highway-Forest Road Friday AM (0800-0900) - Equivalent 120th HH 10 Years - No Development Giveway / Yield (Two-Way) Design Life Analysis (Final Year): Results for 10 years

Moven	nent Per	formance - \	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South:	South: Princes Hwy (S										
2	Т	1554	3.1	0.813	0.6	Χ	X	X	X	0.00	98.1
3	R	27	13.0	0.046	17.8	LOS B	0.2	1.3	0.58	0.84	56.0
Approa	ch	1581	3.3	0.813	0.9	NA	0.2	1.3	0.01	0.01	97.1
South E	East: Fore	st Road (Medi	ian RT)								
23	R	102	2.3	0.056	8.1	LOS A	0.0	0.0	0.00	0.61	53.3
Approa	ch	102	2.3	0.056	8.1	LOSA	0.0	0.0	0.00	0.61	53.3
East: F	orest Roa	d									
4	L	36	3.3	0.082	13.7	LOSA	0.2	1.6	0.56	0.83	50.8
6	R	102	2.3	0.230	15.6	LOS B	0.8	5.9	0.63	0.90	49.1
Approa	ch	138	2.6	0.230	15.1	LOS B	8.0	5.9	0.61	0.88	49.5
North: F	Princes H	wy (N)									
7	L	57	8.3	0.032	13.0	LOSA	0.0	0.0	0.00	0.76	63.3
8	Т	573	16.0	0.324	0.0	LOSA	0.0	0.0	0.00	0.00	100.0
Approa	ch	630	15.3	0.324	1.2	NA	0.0	0.0	0.00	0.07	96.1
All Vehi	icles	2451	6.3	0.813	2.0	NA	0.8	5.9	0.04	0.10	90.2

#### X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Culburra Subdivision - 10 Year Scenarios.sip 8000056, GTA CONSULTANTS, ENTERPRISE

Project: P:\12S1200-1299\12S1231000 - West Cullburra MWT\Modelling\SIDRA\130925sid-12S1231000 West



Site: 10. Princes Hwy-Forest (10 Year NO-DEV Fri PM-120th HH)

13S1231000 - West Culburra Subdivision Princes Highway-Forest Road Friday PM (1600-1700) - Equivalent 120th HH 10 Years - No Development Giveway / Yield (Two-Way) Design Life Analysis (Final Year): Results for 10 years

Movem	nent Per	formance - V	ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: F	Princes H	wy (S)									
2	Т	702	6.0	0.374	0.1	Χ	X	X	Χ	0.00	99.7
3	R	51	2.9	0.501	64.1	LOS E	1.7	11.9	0.96	1.03	25.4
Approac	ch	752	5.8	0.501	4.4	NA	1.7	11.9	0.06	0.07	86.6
South E	ast: Fore	st Road (Media	an RT)								
23	R	66	2.2	0.036	8.1	LOSA	0.0	0.0	0.00	0.61	53.3
Approac	ch	66	2.2	0.036	8.1	LOSA	0.0	0.0	0.00	0.61	53.3
East: Fo	orest Roa	d									
4	L	49	0.0	0.644	85.6	LOS F	2.1	15.0	0.98	1.08	22.0
6	R	66	2.2	0.873	125.9	LOS F	3.8	27.4	0.99	1.24	16.8
Approac	ch	116	1.3	0.873	108.7	LOSF	3.8	27.4	0.98	1.17	18.7
North: P	rinces H	wy (N)									
7	L	148	2.0	0.081	12.7	LOSA	0.0	0.0	0.00	0.75	63.3
8	Т	1622	2.0	0.843	0.0	LOSA	0.0	0.0	0.00	0.00	100.0
Approac	ch	1771	2.0	0.843	1.1	NA	0.0	0.0	0.00	0.06	96.4
All Vehic	cles	2705	3.0	0.873	6.8	NA	3.8	27.4	0.06	0.13	78.9

#### X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: 10. Princes Hwy-Forest (10 Year NO-DEV Sat-120th HH)

13S1231000 - West Culburra Subdivision Princes Highway-Forest Road Saturday - Equivalent 120th HH 10 Years - No Development Giveway / Yield (Two-Way)

Design Life Analysis (Final Year): Results for 10 years

Movem	ent Per	formance - V	ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: F	Princes H	wy (S)									
2	Т	893	1.7	0.463	0.1	Χ	X	X	Χ	0.00	99.6
3	R	40	0.0	0.176	28.7	LOS C	0.6	4.0	0.87	0.97	43.1
Approac	:h	933	1.6	0.463	1.3	NA	0.6	4.0	0.04	0.04	95.5
South E	ast: Fore	st Road (Media	an RT)								
23	R	88	0.0	0.048	8.0	LOSA	0.0	0.0	0.00	0.61	53.3
Approac	:h	88	0.0	0.048	8.0	LOSA	0.0	0.0	0.00	0.61	53.3
East: Fo	rest Roa	d									
4	L	45	0.0	0.254	30.6	LOS C	0.8	5.7	0.88	0.99	38.8
6	R	88	3.0	0.573	42.9	LOS D	2.2	15.7	0.93	1.09	33.2
Approac	:h	134	2.0	0.573	38.7	LOS C	2.2	15.7	0.91	1.05	34.9
North: P	rinces H	wy (N)									
7	L	125	0.0	0.067	12.5	LOSA	0.0	0.0	0.00	0.75	63.3
8	Т	1268	1.7	0.657	0.0	LOSA	0.0	0.0	0.00	0.00	100.0
Approac	:h	1393	1.6	0.657	1.1	NA	0.0	0.0	0.00	0.07	96.1
All Vehic	cles	2548	1.6	0.657	3.4	NA	2.2	15.7	0.06	0.13	86.6

## X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

Processed: Thursday, 26 September 2013 9:00:20 AM SIDRA INTERSECTION 5.1.13.2093

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13S1231000 - West Culburra Subdivision Princes Highway-Moss Street Friday AM (0800-0900) - Equivalent 120th HH

10 Years - No Development

Signals - Fixed Time Cycle Time = 150 seconds (Practical Cycle Time)
Design Life Analysis (Final Year): Results for 10 years

Movem	nent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back ( Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: F	Princes H		/0	V/C	366		VCII	'''		pei veii	KIII/II
1	L	3	0.0	0.943	96.7	LOS F	26.9	200.4	1.00	1.05	17.2
2	Т	995	7.6	1.020	109.8	LOS F	34.7	258.6	1.00	1.15	15.8
3	R	158	3.7	1.007	130.5	LOS F	15.7	113.3	1.00	1.07	12.9
Approac	ch	1156	7.1	1.020	112.6	LOS F	34.7	258.6	1.00	1.14	15.4
East: Mo	oss St (E	)									
4	L	34	3.4	0.378	54.1	LOS D	4.0	28.8	0.81	0.77	23.7
5	Т	227	4.1	1.013	106.7	LOS F	46.7	339.3	0.97	1.12	13.5
6	R	257	4.6	1.013	126.0	LOS F	46.7	339.3	1.00	1.22	13.7
Approac	ch	518	4.3	1.013	112.8	LOS F	46.7	339.3	0.97	1.15	14.0
North: P	rinces H	wy (N)									
7	L	187	5.0	0.997	51.9	LOS D	28.9	212.6	0.89	0.94	26.4
8	T	1601	6.8	0.997	82.6	LOS F	62.9	465.6	0.98	1.11	19.3
9	R	<mark>386</mark>	3.7	1.000 <sup>3</sup>	98.4	LOS F	27.1	195.8	1.00	1.10	16.2
Approac	ch	2174	6.1	1.000	82.7	LOSF	62.9	465.6	0.97	1.09	19.1
West: M	loss St (V	V)									
10	L	139	10.3	0.306	23.3	LOS B	3.7	28.2	0.66	0.76	24.0
11	Т	131	3.6	0.602	63.0	LOS E	12.7	93.4	0.98	0.81	9.0
12	R	54	11.9	0.602	70.8	LOS F	12.7	93.4	0.98	0.82	11.3
Approac	ch	324	7.9	0.602	47.3	LOS D	12.7	93.4	0.84	0.79	13.5
All Vehic	cles	4172	6.3	1.020	92.0	LOS F	62.9	465.6	0.97	1.09	16.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Moven	nent Performance -	Pedestrians						
Mov ID	Description	Demand	Average		Average Back		Prop.	Effective
טו ייטוייו	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	63	59.0	LOS E	0.2	0.2	0.89	0.89
P3	Across E approach	63	38.9	LOS D	0.2	0.2	0.72	0.72
P5	Across N approach	63	69.1	LOS F	0.3	0.3	0.96	0.96
P7	Across W approach	63	59.0	LOS E	0.2	0.2	0.89	0.89
All Ped	estrians	252	56.5	LOS E			0.86	0.86



13S1231000 - West Culburra Subdivision Princes Highway-Moss Street Friday PM (1600-1700) - Equivalent 120th HH 10 Years - No Development

Signals - Fixed Time Cycle Time = 150 seconds (Practical Cycle Time)
Design Life Analysis (Final Year): Results for 10 years

Mover	nent Per	formance - \	Vehicles	_							
		Demand	107	Deg.	Average	Level of	95% Back		Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South	Princes H	veh/h	%	v/c	sec		veh	m		per veh	km/h
30uiii.	L	12	0.0	1.139	207.0	LOSF	48.2	346.1	1.00	1.40	9.0
	_										
2	T	1229	3.1	1.232	262.6	LOSF	67.5	485.1	1.00	1.61	7.8
3	R	115	0.0	1.158	245.6	LOSF	16.1	112.4	1.00	1.25	7.5
Approa	ıch	1355	2.8	1.232	260.7	LOS F	67.5	485.1	1.00	1.57	7.7
East: N	loss St (E	)									
4	L	34	4.3	0.411	64.3	LOS E	4.4	31.6	0.89	0.77	21.5
5	Т	149	0.0	1.103	154.7	LOS F	43.7	309.3	0.97	1.24	10.3
6	R	232	1.9	1.103	194.9	LOS F	43.7	309.3	1.00	1.42	9.7
Approa	ıch	415	1.4	1.103	169.8	LOS F	43.7	309.3	0.98	1.30	10.4
		(A.1)									
	Princes H	, , <u> </u>		3							
7	L	<mark>199</mark>	3.7	1.000 <sup>3</sup>	63.0	LOS E	29.6	213.6	0.96	0.95	23.1
8	Т	1639	3.5	1.228	244.1	LOS F	109.6	790.6	0.99	1.68	8.2
9	R	261	0.7	1.116	170.2	LOS F	27.8	195.8	1.00	1.20	10.4
Approa	ıch	2099	2.9	1.228	217.7	LOS F	109.6	790.6	0.99	1.55	8.9
West: I	Moss St (V	V)									
10	L	157	1.2	0.330	19.3	LOS B	3.3	23.4	0.58	0.74	26.4
11	Т	245	0.0	1.274	325.8	LOS F	138.5	976.3	1.00	1.77	2.1
12	R	148	1.1	1.274	333.2	LOS F	138.5	976.3	1.00	1.77	2.8
Approa	ıch	950	0.9	1.274	278.8	LOS F	138.5	976.3	0.88	1.48	3.3
All Veh	icles	4819	2.4	1.274	237.3	LOSF	138.5	976.3	0.98	1.40	8.0

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Moven	nent Performance -	Pedestrians						
Mov ID	Description	Demand	Average		Average Back		Prop.	Effective
טו ייטוייו	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	63	69.1	LOS F	0.3	0.3	0.96	0.96
P3	Across E approach	63	44.9	LOS E	0.2	0.2	0.77	0.77
P5	Across N approach	63	48.0	LOS E	0.2	0.2	0.80	0.80
P7	Across W approach	63	59.0	LOS E	0.2	0.2	0.89	0.89
All Ped	estrians	252	55.2	LOS E			0.86	0.86



Princes Highway-Moss Street Saturday - Equivalent 120th HH 10 Years - No Development

Signals - Fixed Time Cycle Time = 150 seconds (Practical Cycle Time)

Design Life Analysis (Final Year): Results for 10 years

Movem	nent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: F	Princes H	-	/0	V/C	360		VCII	'''		pei veii	KIII/II
1	L	13	0.0	0.782	65.5	LOS E	26.9	192.8	0.95	0.92	23.1
2	Т	1284	2.7	0.846	58.8	LOS E	31.4	224.7	0.97	0.90	24.2
3	R	104	0.0	0.526	78.6	LOS F	7.4	51.8	0.99	0.79	19.1
Approac	ch	1401	2.4	0.846	60.3	LOS E	31.4	224.7	0.97	0.89	23.8
East: Mo	oss St (E	)									
4	L	32	0.0	0.254	60.4	LOS E	2.7	19.2	0.86	0.74	22.2
5	Т	80	0.0	0.681	62.2	LOS E	14.8	104.8	0.97	0.80	19.1
6	R	146	1.8	0.681	71.7	LOS F	14.8	104.8	1.00	0.84	20.1
Approac	ch	258	1.0	0.681	67.3	LOS E	14.8	104.8	0.97	0.81	20.0
North: P	rinces H	wy (N)									
7	L	115	2.3	0.741	43.3	LOS D	19.8	140.7	0.71	0.97	29.5
8	T	1433	1.5	0.741	36.7	LOS C	31.6	224.2	0.81	0.73	31.6
9	R	284	1.0	0.824	47.3	LOS D	12.9	91.0	1.00	0.89	26.8
Approac	ch	1832	1.5	0.824	38.8	LOS C	31.6	224.2	0.83	0.77	30.7
West: M	loss St (V	V)									
10	L	342	0.0	0.722	26.2	LOS B	10.8	75.4	0.78	0.81	22.2
11	Т	154	0.0	0.810	69.1	LOS E	19.9	140.2	1.00	0.92	8.3
12	R	113	1.3	0.810	76.5	LOS F	19.9	140.2	1.00	0.92	10.6
Approac	ch	609	0.2	0.810	46.4	LOS D	19.9	140.2	0.88	0.86	14.2
All Vehic	cles	4100	1.6	0.846	49.1	LOS D	31.6	224.7	0.90	0.83	25.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestrians	;					
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P1	Across S approach	63	69.1	LOS F	0.3	0.3	0.96	0.96
P3	Across E approach	63	34.0	LOS D	0.2	0.2	0.67	0.67
P5	Across N approach	63	68.2	LOS F	0.3	0.3	0.95	0.95
P7	Across W approach	63	47.2	LOS E	0.2	0.2	0.79	0.79
All Ped	estrians	252	54.6	LOS E			0.85	0.85



13S1231000 - West Culburra Subdivision Princes Highway-Kalandar Street

Friday AM (0800-0900) - Equivalent 120th HH

10 Years - With Development

Signals - Fixed Time Cycle Time = 150 seconds (Practical Cycle Time)

Design Life Analysis (Final Year): Results for 10 years

Mover	nent Per	rformance - \	Vehicles								
Mov IC	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back ( Vehicles	Distance	Prop. Queued	Effective Stop Rate	Average Speed
South:	Princes H	veh/h łwv (S)	%	v/c	sec		veh	m		per veh	km/h
1	L	3	0.0	1.058	112.9	LOS F	46.8	343.2	1.00	1.19	15.7
2	Т	1058	5.6	1.058	122.2	LOS F	59.9	439.3	1.00	1.27	14.6
3	R	42	0.0	0.378	85.5	LOS F	3.1	21.7	1.00	0.74	18.6
Approa	ich	1104	5.3	1.058	120.7	LOSF	59.9	439.3	1.00	1.25	14.7
East: K	alandar S	St (E)									
4	L	41	9.4	1.085	156.2	LOS F	67.0	482.9	1.00	1.31	8.3
5	Т	325	3.1	1.085	146.9	LOS F	67.0	482.9	1.00	1.31	7.7
6	R	825	2.9	1.085	169.7	LOS F	71.3	511.5	1.00	1.27	7.7
Approa	ich	1192	3.1	1.085	163.1	LOSF	71.3	511.5	1.00	1.28	7.7
North:	Princes H	wy (N)									
7	L	277	7.3	0.300	9.3	LOSA	1.0	7.1	0.07	0.64	53.3
8	Т	988	7.3	0.777	44.5	LOS D	30.2	224.6	0.90	0.80	28.5
9	R	173	11.3	1.019	104.1	LOS F	12.9	99.2	1.00	1.06	16.1
Approa	ıch	1437	7.8	1.019	44.9	LOS D	30.2	224.6	0.75	0.80	28.5
West: I	Kalandar S	St (W)									
10	L	57	13.6	0.846	85.2	LOS F	12.3	90.0	0.98	1.01	15.7
11	T	159	1.6	0.846	75.6	LOS F	16.8	122.1	0.99	0.98	14.9
12	R	171	5.3	0.846	85.1	LOS F	16.8	122.1	1.00	0.93	15.4
Approa	ıch	387	5.0	0.846	81.3	LOSF	16.8	122.1	0.99	0.96	15.2
All Veh	icles	4120	5.5	1.085	102.8	LOS F	71.3	511.5	0.91	1.08	14.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestrians	;					
Marrido	Description	Demand	Average	Level of			Prop.	Effective
Mov ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	63	48.8	LOS E	0.2	0.2	0.81	0.81
P3	Across E approach	63	38.9	LOS D	0.2	0.2	0.72	0.72
P5	Across N approach	63	69.1	LOS F	0.3	0.3	0.96	0.96
All Ped	estrians	189	52.3	LOS E			0.83	0.83

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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13S1231000 - West Culburra Subdivision

Princes Highway-Kalandar Street

Friday PM (1600-1700) - Equivalent 120th HH

10 Years - With Development

Signals - Fixed Time Cycle Time = 150 seconds (Practical Cycle Time)

Design Life Analysis (Final Year): Results for 10 years

Mover	nent Pe	rformance - \	/ehicles								
Mov ID	Turn	Demand	HV	Deg.	Average	Level of	95% Back		Prop.	Effective	Average
טו ייטוייו	Turn	Flow veh/h		Satn v/c	Delay	Service	Vehicles veh	Distance	Queued	Stop Rate	Speed /m/h
South:	Princes H		%	V/G	sec		ven	m		per veh	km/h
1	L	13	0.0	1.142	186.7	LOS F	47.1	336.9	1.00	1.28	10.2
2	T	864	2.6	1.142	195.2	LOSF	59.4	424.8	1.00	1.40	10.0
3	R	115	1.4	1.170	255.8	LOS F	16.4	116.3	1.00	1.26	7.6
Approa	ıch	991	2.4	1.170	202.1	LOS F	59.4	424.8	1.00	1.38	9.7
East: K	Calandar S	St (E)									
4	L	103	1.5	1.148	203.8	LOS F	57.9	412.4	1.00	1.40	6.5
5	Т	254	2.4	1.148	194.8	LOS F	57.9	412.4	1.00	1.40	6.1
6	R	583	1.8	1.148	224.8	LOS F	62.3	442.6	1.00	1.37	6.0
Approa	ıch	940	2.0	1.148	214.4	LOS F	62.3	442.6	1.00	1.38	6.1
North: I	Princes H	wy (N)									
7	L	871	1.1	0.924	27.6	LOS B	18.5	130.6	0.27	0.78	36.9
8	Т	1432	3.1	1.141	201.6	LOS F	98.3	706.2	1.00	1.65	9.7
9	R	152	5.4	0.587	48.0	LOS D	7.5	54.8	0.98	0.80	27.3
Approa	ıch	2455	2.5	1.141	130.4	LOS F	98.3	706.2	0.74	1.29	13.7
West: k	Kalandar	St (W)									
<mark>10</mark>	L	<mark>120</mark>	9.5	1.000 <sup>3</sup>	56.8	LOS E	13.6	99.1	0.90	0.89	20.9
11	Т	325	0.0	1.179	170.0	LOS F	79.6	560.0	0.96	1.26	7.9
12	R	333	1.0	1.179	256.5	LOS F	79.6	560.0	1.00	1.55	6.1
Approa	ich	779	1.9	1.179	189.5	LOS F	79.6	560.0	0.97	1.33	7.7
All Veh	icles	5165	2.3	1.179	168.4	LOSF	98.3	706.2	0.87	1.33	10.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Moven	nent Performance -	Pedestrians	5					
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Back Pedestrian	of Queue Distance	Prop. Queued	Effective Stop Rate
		ped/h	sec		ped	m		per ped
P1	Across S approach	63	59.0	LOS E	0.2	0.2	0.89	0.89
P3	Across E approach	63	40.3	LOS E	0.2	0.2	0.73	0.73
P5	Across N approach	63	55.5	LOS E	0.2	0.2	0.86	0.86
All Pede	estrians	189	51.6	LOS E			0.83	0.83



Princes Highway-Kalandar Street

Saturday - Equivalent 120th HH

10 Years - With Development

Signals - Fixed Time Cycle Time = 150 seconds (Practical Cycle Time)

Design Life Analysis (Final Year): Results for 10 years

Movem	ent Pe	rformance - '	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: F	Princes H		70	V/C	560		ven	111		per veri	KIII/II
1	L	6	25.0	0.967	105.2	LOS F	47.2	338.5	1.00	1.12	16.7
2	Т	1056	2.6	0.967	89.2	LOS F	47.2	338.5	1.00	1.11	18.4
3	R	118	4.5	0.983	119.3	LOS F	11.0	80.1	1.00	1.03	14.4
Approac	ch	1180	2.9	0.983	92.3	LOSF	47.2	338.5	1.00	1.10	17.9
East: Ka	alandar S	St (E)									
4	L	93	0.0	0.985	101.9	LOS F	41.7	293.1	1.00	1.16	11.9
5	Т	218	0.6	0.985	93.0	LOS F	41.7	293.1	1.00	1.16	11.2
6	R	559	1.0	0.985	109.6	LOS F	41.7	293.1	1.00	1.10	11.2
Approac	ch	869	8.0	0.985	104.6	LOSF	41.7	293.1	1.00	1.12	11.2
North: P	rinces F	łwy (N)									
7	L	505	0.3	0.529	9.4	LOSA	2.4	16.7	0.08	0.65	52.9
8	Т	1525	1.0	0.982	75.4	LOS F	70.2	495.7	1.00	1.13	20.6
9	R	100	11.6	0.402	43.0	LOS D	4.0	30.7	0.95	0.78	29.3
Approac	ch	2130	1.3	0.982	58.2	LOS E	70.2	495.7	0.78	1.00	24.3
West: Ka	alandar	St (W)									
10	L	73	2.0	0.918	84.8	LOS F	13.9	97.9	0.99	0.99	15.6
11	Т	174	0.0	0.918	78.7	LOS F	20.3	142.6	0.99	0.99	14.4
12	R	190	0.8	0.918	94.8	LOS F	20.3	142.6	1.00	1.01	14.1
Approac	ch	437	0.7	0.918	86.7	LOS F	20.3	142.6	1.00	1.00	14.5
All Vehic	cles	4615	1.6	0.985	78.4	LOSF	70.2	495.7	0.90	1.05	18.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Moven	nent Performance -	Pedestrian	S					
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P1	Across S approach	63	57.2	LOS E	0.2	0.2	0.87	0.87
P3	Across E approach	63	32.7	LOS D	0.2	0.2	0.66	0.66
P5	Across N approach	63	69.1	LOS F	0.3	0.3	0.96	0.96
All Pede	estrians	189	53.0	LOS E			0.83	0.83

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Site: 9. Kalandar St-Kinghorne St (10 year + DEV Fri AM-120th HH)

13S1231000 - West Culburra Subdivision Kalandar Street-Kinghorne Street-Albatross Road Friday AM (0800-0900) - Equivalent 120th HH 10 Years - With Development Roundabout

Moven	nent Per	formance - \	Vehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: I	Kinghorne	e Sttreet									
1	L	34	7.7	0.850	31.0	LOS C	17.4	125.3	1.00	1.40	32.9
2	Т	532	3.4	0.850	29.5	LOS C	17.4	125.3	1.00	1.40	33.1
3	R	65	0.0	0.850	34.3	LOS C	17.4	125.3	1.00	1.40	32.0
Approac	ch	631	3.3	0.850	30.1	LOS C	17.4	125.3	1.00	1.40	32.9
East: Ka	alandar S	Street									
4	L	359	7.5	0.490	8.2	LOS A	3.3	24.6	0.51	0.61	44.7
6	R	177	2.3	0.490	12.9	LOSA	3.3	24.6	0.51	0.78	41.7
Approac	ch	536	5.8	0.490	9.7	LOSA	3.3	24.6	0.51	0.67	43.7
North: k	Kinghorne	Street									
7	L	145	3.7	0.376	9.1	LOS A	2.6	19.0	0.62	0.69	46.9
8	Т	34	0.0	0.376	8.2	LOS A	2.6	19.0	0.62	0.66	46.8
9	R	194	3.4	0.376	12.2	LOSA	2.6	19.0	0.62	0.76	45.3
Approac	ch	373	3.2	0.376	10.6	LOSA	2.6	19.0	0.62	0.72	46.0
South V	Vest: Alba	atross Road									
30	L	287	4.5	0.896	38.4	LOS C	18.3	134.5	1.00	1.54	28.9
32	R	243	8.1	0.896	42.6	LOS D	18.3	134.5	1.00	1.53	28.4
Approac	ch	531	6.2	0.896	40.3	LOS C	18.3	134.5	1.00	1.54	28.7
All Vehi	icles	2069	4.6	0.896	23.9	LOS B	18.3	134.5	0.80	1.12	35.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Project: P:\12S1200-1299\12S1231000 - West Cullburra MWT\Modelling\SIDRA\130925sid-12S1231000 West Culburra Subdivision - 10 Year Scenarios.sip 8000056, GTA CONSULTANTS, ENTERPRISE



Site: 9. Kalandar St-Kinghorne St (10 Year + DEV Fri PM-120th HH)

13S1231000 - West Culburra Subdivision Kalandar Street-Kinghorne Street-Albatross Road Friday PM (1600-1700) - Equivalent 120th HH 10 Years - With Development Roundabout

Movem	ent Per	formance - V	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: k	Kinghorne									, , , , , , ,	
1	L	37	0.0	0.688	19.9	LOS B	8.4	58.6	0.98	1.15	39.2
2	Т	359	0.0	0.688	18.7	LOS B	8.4	58.6	0.98	1.14	39.4
3	R	86	0.0	0.688	23.6	LOS B	8.4	58.6	0.98	1.15	37.7
Approac	ch	482	0.0	0.688	19.6	LOS B	8.4	58.6	0.98	1.15	39.0
East: Ka	alandar S	treet									
4	L	311	0.0	0.512	9.8	LOSA	3.7	26.1	0.67	0.76	43.5
6	R	131	0.0	0.512	14.6	LOS B	3.7	26.1	0.67	0.88	40.1
Approac	ch	441	0.0	0.512	11.2	LOSA	3.7	26.1	0.67	0.80	42.4
North: K	inghorne	Street									
7	L	355	0.0	0.898	24.5	LOS B	19.8	138.9	1.00	1.31	35.6
8	Т	88	0.0	0.898	23.7	LOS B	19.8	138.9	1.00	1.31	35.7
9	R	335	0.0	0.898	27.7	LOS B	19.8	138.9	1.00	1.31	34.7
Approac	ch	778	0.0	0.898	25.8	LOS B	19.8	138.9	1.00	1.31	35.2
South W	/est: Alba	tross Road									
30	L	331	0.0	0.926	32.9	LOS C	23.1	161.4	1.00	1.53	31.1
32	R	382	0.0	0.926	37.0	LOS C	23.1	161.4	1.00	1.53	30.4
Approac	h	713	0.0	0.926	35.1	LOS C	23.1	161.4	1.00	1.53	30.7
All Vehic	cles	2414	0.0	0.926	24.6	LOS B	23.1	161.4	0.94	1.25	35.2

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Site: 9. Kalandar St-Kinghorne St (10 Year + DEV Sat-120th HH)

13S1231000 - West Culburra Subdivision Kalandar Street-Kinghorne Street-Albatross Road Saturday - Equivalent 120th HH 10 Years - With Development Roundabout

Movem	ent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: K	inghorne	Sttreet									
1	L	23	12.5	0.258	11.0	LOS A	1.6	11.7	0.64	0.77	47.1
2	Т	178	1.6	0.258	9.2	LOS A	1.6	11.7	0.64	0.69	47.3
3	R	33	0.0	0.258	14.1	LOSA	1.6	11.7	0.64	0.82	44.4
Approac	h	234	2.5	0.258	10.1	LOSA	1.6	11.7	0.64	0.72	46.8
East: Ka	landar S	treet									
4	L	224	6.0	0.300	7.6	LOSA	1.7	12.1	0.40	0.57	45.5
6	R	107	1.4	0.300	12.4	LOSA	1.7	12.1	0.40	0.78	42.1
Approac	h	332	4.5	0.300	9.2	LOSA	1.7	12.1	0.40	0.64	44.3
North: K	inghorne	Street									
7	L	168	8.0	0.353	8.8	LOSA	2.3	16.6	0.57	0.67	47.2
8	T	23	0.0	0.353	8.0	LOSA	2.3	16.6	0.57	0.63	47.2
9	R	173	2.6	0.353	12.0	LOSA	2.3	16.6	0.57	0.75	45.5
Approac	h	364	1.6	0.353	10.3	LOS A	2.3	16.6	0.57	0.71	46.4
South W	est: Alba	tross Road									
30	L	177	1.4	0.424	8.3	LOS A	3.0	21.4	0.62	0.65	47.0
32	R	260	1.7	0.424	12.5	LOSA	3.0	21.4	0.62	0.77	45.4
Approac	h	437	1.6	0.424	10.8	LOSA	3.0	21.4	0.62	0.72	46.0
All Vehic	les	1366	2.5	0.424	10.1	LOSA	3.0	21.4	0.56	0.70	45.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model used.

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Culburra Subdivision - 10 Year Scenarios.sip 8000056, GTA CONSULTANTS, ENTERPRISE

Project: P:\12S1200-1299\12S1231000 - West Cullburra MWT\Modelling\SIDRA\130925sid-12S1231000 West



Site: 10. Princes Hwy-Forest (10 Year + DEV Fri AM-120th HH)

13S1231000 - West Culburra Subdivision Princes Highway-Forest Road Friday AM (0800-0900) - Equivalent 120th HH 10 Years - With Development Giveway / Yield (Two-Way) Design Life Analysis (Final Year): Results for 10 years

Movem	nent Per	formance - \	<b>Vehicles</b>								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: F	Princes H	lwy (S)									
2	T	1554	3.1	0.813	0.6	Χ	X	X	Χ	0.00	98.1
3	R	46	13.0	0.079	18.0	LOS B	0.3	2.3	0.59	0.87	55.7
Approac	ch	1600	3.4	0.813	1.1	NA	0.3	2.3	0.02	0.03	96.4
South E	ast: Fore	st Road (Medi	ian RT)								
23	R	108	2.3	0.059	8.1	LOSA	0.0	0.0	0.00	0.61	53.3
Approac	ch	108	2.3	0.059	8.1	LOSA	0.0	0.0	0.00	0.61	53.3
East: Fo	orest Roa	ıd									
4	L	49	3.3	0.114	13.8	LOS A	0.3	2.2	0.57	0.85	50.7
6	R	108	2.3	0.253	16.2	LOS B	0.9	6.7	0.65	0.92	48.6
Approac	ch	158	2.6	0.253	15.4	LOS B	0.9	6.7	0.62	0.89	49.2
North: P	rinces H	wy (N)									
7	L	64	8.3	0.037	13.0	LOSA	0.0	0.0	0.00	0.76	63.3
8	Т	573	16.0	0.324	0.0	LOSA	0.0	0.0	0.00	0.00	100.0
Approac	ch	637	15.2	0.324	1.3	NA	0.0	0.0	0.00	0.08	95.7
All Vehic	cles	2504	6.3	0.813	2.3	NA	0.9	6.7	0.05	0.12	89.0

## X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: 10. Princes Hwy-Forest (10 Year + DEV Fri PM-120th HH)

13S1231000 - West Culburra Subdivision Princes Highway-Forest Road Friday PM (1600-1700) - Equivalent 120th HH 10 Years - With Development Giveway / Yield (Two-Way) Design Life Analysis (Final Year): Results for 10 years

Movem	nent Per	formance - V	ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: F	Princes H	wy (S)									
2	Т	702	6.0	0.374	0.1	Χ	Х	X	Χ	0.00	99.7
3	R	64	2.9	0.657	77.9	LOS F	2.4	16.9	0.98	1.06	21.8
Approac	ch	766	5.8	0.657	6.6	NA	2.4	16.9	0.08	0.09	81.0
South E	ast: Fore	st Road (Media	an RT)								
23	R	75	2.2	0.041	8.1	LOSA	0.0	0.0	0.00	0.61	53.3
Approac	ch	75	2.2	0.041	8.1	LOSA	0.0	0.0	0.00	0.61	53.3
East: Fo	orest Roa	d									
4	L	69	0.0	0.920	137.1	LOS F	4.3	29.8	0.99	1.26	15.7
6	R	75	2.2	1.025	193.2	LOS F	7.0	49.9	1.00	1.50	12.0
Approac	ch	144	1.2	1.025	166.1	LOSF	7.0	49.9	1.00	1.38	13.5
North: P	rinces H	wy (N)									
7	L	161	2.0	0.088	12.7	LOSA	0.0	0.0	0.00	0.75	63.3
8	Т	1622	2.0	0.843	0.0	LOSA	0.0	0.0	0.00	0.00	100.0
Approac	ch	1783	2.0	0.843	1.1	NA	0.0	0.0	0.00	0.07	96.1
All Vehic	cles	2768	3.0	1.025	11.4	NA	7.0	49.9	0.07	0.16	69.3

## X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Site: 10. Princes Hwy-Forest (10 Year + DEV Sat-120th HH)

13S1231000 - West Culburra Subdivision Princes Highway-Forest Road Saturday - Equivalent 120th HH 10 Years - With Development Giveway / Yield (Two-Way)

Design Life Analysis (Final Year): Results for 10 years

Movem	ent Per	formance - V	/ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back o Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
South: F	Princes H										
2	Т	893	1.7	0.463	0.1	Χ	X	Χ	Х	0.00	99.6
3	R	52	0.0	0.232	30.2	LOS C	0.8	5.5	0.88	0.98	41.8
Approac	h	944	1.6	0.463	1.8	NA	0.8	5.5	0.05	0.05	94.2
South E	ast: Fore	st Road (Media	an RT)								
23	R	98	0.0	0.053	8.0	LOSA	0.0	0.0	0.00	0.61	53.3
Approac	h	98	0.0	0.053	8.0	LOSA	0.0	0.0	0.00	0.61	53.3
East: Fo	rest Roa	d									
4	L	62	0.0	0.353	33.1	LOS C	1.2	8.4	0.90	1.01	37.5
6	R	98	3.0	0.654	48.1	LOS D	2.7	19.0	0.94	1.13	31.3
Approac	h	160	1.8	0.654	42.3	LOS C	2.7	19.0	0.93	1.08	33.5
North: P	rinces H	wy (N)									
7	L	137	0.0	0.074	12.5	LOS A	0.0	0.0	0.00	0.75	63.3
8	Т	1268	1.7	0.657	0.0	LOS A	0.0	0.0	0.00	0.00	100.0
Approac	h	1405	1.6	0.657	1.2	NA	0.0	0.0	0.00	0.07	95.8
All Vehic	cles	2607	1.5	0.657	4.2	NA	2.7	19.0	0.07	0.15	84.1

## X: Not applicable for Continuous movement.

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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13S1231000 - West Culburra Subdivision Princes Highway-Moss Street

Friday AM (0800-0900) - Equivalent 120th HH

10 Years - With Development

Signals - Fixed Time Cycle Time = 135 seconds (Optimum Cycle Time - Minimum Delay)

Move	ment Per	formance - \	Vehicles								
		Demand		Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	) Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Onvitle	Deimona III	veh/h	%	v/c	sec		veh	m		per veh	km/h
	Princes H	• , ,									
1	L	5	0.0	0.980	102.3	LOSF	27.4	204.4	1.00	1.13	16.4
2	Т	1019	7.6	1.060	125.5	LOS F	36.6	272.7	1.00	1.25	14.3
3	R	158	3.7	1.071	165.4	LOS F	17.1	123.2	1.00	1.19	10.6
Approa	ich	1182	7.1	1.071	130.7	LOS F	36.6	272.7	1.00	1.24	13.7
East: N	loss St (E	<u>.</u> )									
4	L	37	3.4	0.405	50.1	LOS D	4.3	30.8	0.82	0.78	24.8
5	Т	244	4.1	1.087	141.8	LOS F	55.3	401.6	0.97	1.30	11.0
6	R	276	4.6	1.087	172.4	LOS F	55.3	401.6	1.00	1.45	10.7
Approa	ich	557	4.3	1.087	150.9	LOS F	55.3	401.6	0.97	1.34	11.3
North:	Princes H	wy (N)									
7	L	<mark>197</mark>	5.0	1.000 <sup>3</sup>	52.8	LOS D	29.0	213.4	0.96	0.93	26.1
8	T	1659	6.8	1.133	163.2	LOS F	85.7	635.1	0.99	1.51	11.6
9	R	336	3.7	1.044	113.6	LOS F	27.1	195.8	1.00	1.14	14.5
Approa	ach	2192	6.1	1.133	145.7	LOS F	85.7	635.1	0.99	1.40	12.5
West: I	Moss St (V	N)									
10	L	139	10.3	0.303	22.4	LOS B	3.4	26.1	0.68	0.76	24.5
11	Т	135	3.6	0.555	54.4	LOS D	11.5	84.4	0.96	0.79	10.1
12	R	55	11.9	0.555	62.2	LOS E	11.5	84.4	0.96	0.82	12.6
Approa	ich	328	7.8	0.555	42.2	LOS C	11.5	84.4	0.84	0.78	14.7
All Veh	icles	4259	6.3	1.133	134.2	LOSF	85.7	635.1	0.98	1.30	12.7

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Moven	nent Performance -	Pedestrians	S					
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P1	Across S approach	53	55.1	LOS E	0.2	0.2	0.90	0.90
P3	Across E approach	53	38.5	LOS D	0.2	0.2	0.76	0.76
P5	Across N approach	53	61.6	LOS F	0.2	0.2	0.96	0.96
P7	Across W approach	53	54.2	LOS E	0.2	0.2	0.90	0.90
All Pede	estrians	212	52.4	LOS E			0.88	0.88

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Project: P:\12S1200-1299\12S1231000 - West Cullburra MWT\Modelling\SIDRA\130925sid-12S1231000 West Culburra Subdivision - 10 Year Scenarios.sip 8000056, GTA CONSULTANTS, ENTERPRISE



Princes Highway-Moss Street

Friday PM (1600-1700) - Equivalent 120th HH

10 Years - With Development

Signals - Fixed Time Cycle Time = 135 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Per	formance - \	/ehicles								
Mov ID	Turn	Demand Flow	HV	Deg. Satn	Average Delay	Level of Service	95% Back ( Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Average Speed
		veh/h	%	v/c	sec		veh	m		per veh	km/h
South:	Princes H	• , ,									
1	L	14	0.0	1.208	260.3	LOS F	52.9	379.7	1.00	1.55	7.3
2	Т	1232	3.1	1.307	320.3	LOS F	72.1	518.4	1.00	1.79	6.5
3	R	115	0.0	1.390	442.6	LOS F	21.7	151.7	1.00	1.49	4.4
Approa	ch	1360	2.8	1.390	330.0	LOSF	72.1	518.4	1.00	1.76	6.3
East: M	loss St (E	E)									
4	L	37	4.3	0.382	56.3	LOS D	4.1	29.1	0.87	0.77	23.2
5	Т	155	0.0	1.026	104.3	LOS F	35.1	248.3	0.97	1.13	13.7
6	R	238	1.9	1.026	129.6	LOS F	35.1	248.3	1.00	1.28	13.4
Approa	ch	429	1.4	1.026	114.2	LOSF	35.1	248.3	0.98	1.18	14.0
North: F	Princes H	wy (N)									
7	L	<mark>215</mark>	3.7	1.000 <sup>3</sup>	58.8	LOS E	29.6	213.6	1.00	0.95	24.2
8	Т	1812	3.5	1.391	362.2	LOS F	144.8	1043.9	1.00	2.12	5.8
9	R	252	0.7	1.149	192.5	LOS F	27.8	195.6	1.00	1.25	9.3
Approa	ch	2278	3.0	1.391	314.9	LOS F	144.8	1043.9	1.00	1.91	6.5
West: N	∕loss St (\	N)									
10	L `	153	1.2	0.330	19.4	LOS B	3.3	23.4	0.61	0.75	26.3
11	Т	260	0.0	1.400	433.2	LOS F	157.3	1108.4	1.00	2.13	1.6
12	R	148	1.1	1.400	440.7	LOS F	157.3	1108.4	1.00	2.13	2.1
Approa	ch	965	8.0	1.400	370.9	LOSF	157.3	1108.4	0.89	1.75	2.5
All Vehi	icles	5033	2.4	1.400	312.1	LOSF	157.3	1108.4	0.99	1.64	6.3

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

3 x = 1.00 due to short lane. Refer to the Lane Summary report for information about excess flow and related conditions.

Moven	nent Performance -	Pedestrian	s					
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate per ped
P1	Across S approach	53	61.6	LOS F	0.2	0.2	0.96	0.96
P3	Across E approach	53	41.6	LOS E	0.2	0.2	0.79	0.79
P5	Across N approach	53	47.3	LOS E	0.2	0.2	0.84	0.84
P7	Across W approach	53	55.1	LOS E	0.2	0.2	0.90	0.90
All Pede	estrians	212	51.4	LOS E			0.87	0.87

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

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Princes Highway-Moss Street Saturday - Equivalent 120th HH 10 Years - With Development

Signals - Fixed Time Cycle Time = 135 seconds (Optimum Cycle Time - Minimum Delay)

Moven	nent Pe	rformance -	Vehicles								
		Demand	107	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Courthy	Deimona	veh/h	%	v/c	sec		veh	m		per veh	km/h
	Princes F	• , ,	2.2	0.000	=10		20 =	20= 4	1.00	2.22	0.4.0
1	L	14	0.0	0.886	71.8	LOSF	28.7	205.4	1.00	0.98	21.6
2	Т	1300	2.7	0.958	73.3	LOS F	35.9	257.1	1.00	1.06	21.0
3	R	104	0.0	0.583	73.8	LOS F	6.8	47.7	1.00	0.79	20.0
Approa	ch	1418	2.4	0.958	73.4	LOS F	35.9	257.1	1.00	1.04	20.9
East: M	loss St (E	≣)									
4	L	35	0.0	0.243	52.6	LOS D	2.6	18.1	0.83	0.75	24.0
5	Т	85	0.0	0.651	53.7	LOS D	13.9	98.5	0.96	0.79	20.7
6	R	155	1.8	0.651	63.0	LOS E	13.9	98.5	0.98	0.83	21.7
Approa	ch	275	1.0	0.651	58.8	LOS E	13.9	98.5	0.96	0.81	21.7
North: F	Princes H	lwy (N)									
7	L	121	2.3	0.806	49.3	LOS D	23.3	165.6	0.82	0.99	27.4
8	T	1443	1.5	0.806	40.8	LOS C	31.6	224.1	0.89	0.82	29.8
9	R	284	1.0	0.946	64.9	LOS E	15.8	111.7	1.00	1.00	21.8
Approa	ch	1848	1.5	0.946	45.1	LOS D	31.6	224.1	0.90	0.86	28.2
West: N	loss St (	W)									
10	L	342	0.0	0.671	24.6	LOS B	9.2	64.3	0.79	0.81	23.1
11	Т	163	0.0	0.785	60.2	LOS E	18.3	128.8	1.00	0.91	9.3
12	R	114	1.3	0.785	67.6	LOS E	18.3	128.8	1.00	0.91	11.7
Approa	ch	619	0.2	0.785	41.9	LOS C	18.3	128.8	0.89	0.85	15.3
All Vehi	cles	4160	1.6	0.958	55.1	LOS D	35.9	257.1	0.94	0.92	23.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model used.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow	Average Delay	Level of Service	Average Back Pedestrian	Distance	Prop. Queued	Effective Stop Rate				
		ped/h	sec		ped	m		per ped				
P1	Across S approach	53	61.6	LOS F	0.2	0.2	0.96	0.96				
P3	Across E approach	53	34.8	LOS D	0.1	0.1	0.72	0.72				
P5	Across N approach	53	61.6	LOS F	0.2	0.2	0.96	0.96				
P7	Across W approach	53	46.5	LOS E	0.2	0.2	0.83	0.83				
All Ped	estrians	212	51.1	LOS E			0.86	0.86				

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.





Site: Culburra-Collector East -**Post-Dev AM** 

12S1231000 West Culburra Subdivision Culburra Road - Collector Road-East Friday AM (0800-0900) - Equivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Moven	nent Per	formance - V	ehicles								
Mov ID	Turn	Demand Flow veh/h	HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back of Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate per veh	Average Speed km/h
East: C	ulburra R	oad (E)									
5	Т	277	2.5	0.144	0.0	LOSA	0.0	0.0	0.00	0.00	100.0
6	R	108	1.0	0.101	14.6	LOS B	0.4	2.8	0.35	0.73	61.0
Approac	ch	385	2.1	0.144	4.1	NA	0.4	2.8	0.10	0.21	87.7
North: 0	Collector I	Road									
7	L	314	1.0	0.364	9.4	LOSA	1.8	12.5	0.43	0.70	48.4
9	R	11	1.0	0.364	9.4	LOSA	1.8	12.5	0.43	0.80	48.4
Approac	Approach		1.0	0.364	9.4	LOS A	1.8	12.5	0.43	0.70	48.4
West: C	ulburra F	Road (W)									
10	L	40	1.0	0.022	13.5	LOSA	0.0	0.0	0.00	0.77	63.6
11	Т	213	2.5	0.111	0.0	LOSA	0.0	0.0	0.00	0.00	100.0
Approac	ch	253	2.3	0.111	2.1	NA	0.0	0.0	0.00	0.12	93.6
All Vehi	cles	962	1.8	0.364	5.4	NA	1.8	12.5	0.19	0.35	69.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Collector Rd East.sip 8000056, GTA CONSULTANTS, ENTERPRISE



Site: Culburra-Collector East -**Post-Dev PM** 

12S1231000 West Culburra Subdivision Culburra Road - Collector Road-East Friday PM (1600-1700) - Equivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Movement Performance - Vehicles											
Mayde	T. 1995	Demand	1107	Deg.	Average	Level of	95% Back		Prop.	Effective	Average
Mov ID	Turn	Flow	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Footi C	V. Jb D.	veh/h	%	v/c	sec		veh	m		per veh	km/h
	Culburra Ro	( )									
5	Т	193	2.5	0.100	0.0	LOS A	0.0	0.0	0.00	0.00	100.0
6	R	314	1.0	0.316	15.7	LOS B	1.5	10.8	0.50	0.81	59.6
Approa	ich	506	1.6	0.316	9.7	NA	1.5	10.8	0.31	0.50	73.2
North:	North: Collector Road										
7	L	114	1.0	0.333	15.1	LOS B	1.5	10.6	0.60	0.86	44.1
9	R	40	1.0	0.333	15.1	LOS B	1.5	10.6	0.60	0.90	44.1
Approa	ich	154	1.0	0.333	15.1	LOS B	1.5	10.6	0.60	0.87	44.1
West: 0	Culburra R	toad (W)									
10	L	11	1.0	0.006	13.5	LOS A	0.0	0.0	0.00	0.77	63.6
11	Т	365	2.5	0.190	0.0	LOSA	0.0	0.0	0.00	0.00	100.0
Approa	ich	376	2.5	0.190	0.4	NA	0.0	0.0	0.00	0.02	98.8
All Veh	icles	1036	1.8	0.333	7.1	NA	1.5	10.8	0.24	0.38	73.1

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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Collector Rd East.sip 8000056, GTA CONSULTANTS, ENTERPRISE



Site: Culburra-Collector East -**Post-Dev SAT** 

12S1231000 West Culburra Subdivision Culburra Road - Collector Road-East Saturday - Equivalent 120th HH Future - Full Site Development Giveway / Yield (Two-Way)

Movement Performance - Vehicles											
Mov ID	Turn	Demand	HV	Deg.	Average	Level of	95% Back		Prop.	Effective	Average
IVIOV IL	Tuffi	Flow veh/h	пv %	Satn v/c	Delay	Service	Vehicles	Distance	Queued	Stop Rate per veh	Speed km/h
East: C	Culburra Ro		70	V/C	sec		veh	m		per veri	KIII/II
5	Т	251	2.5	0.131	0.0	LOSA	0.0	0.0	0.00	0.00	100.0
6	R	201	1.0	0.186	14.6	LOS B	0.8	5.4	0.36	0.74	61.0
Approa	ıch	452	1.8	0.186	6.5	NA	8.0	5.4	0.16	0.33	81.0
North:	North: Collector Road										
7	L	201	1.0	0.219	8.8	LOSA	0.9	6.3	0.38	0.68	48.9
9	R	1	1.0	0.219	8.8	LOSA	0.9	6.3	0.38	0.77	48.8
Approa	ich	202	1.0	0.219	8.8	LOSA	0.9	6.3	0.38	0.68	48.9
West: 0	Culburra R	Road (W)									
10	L	1	1.0	0.001	13.4	LOSA	0.0	0.0	0.00	0.77	63.6
11	Т	240	2.5	0.125	0.0	LOSA	0.0	0.0	0.00	0.00	100.0
Approa	ıch	241	2.5	0.125	0.1	NA	0.0	0.0	0.00	0.00	99.8
All Veh	icles	895	1.8	0.219	5.3	NA	0.9	6.3	0.17	0.32	73.6

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model used.

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