



abc CONSULTANTS
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Consultant Design Brief

STRUCTURAL ENGINEERING CONSULTANCY

Project#: 25054
40-76 William Street
Leichhardt

for
Anprisa Pty Ltd
447-451 Parramatta Rd
Leichhardt, NSW

14.10.2025 | Ref:25054: Rev A | Approved for Issue by : Ryan Campbell, Director ABC Consultants

revision register

Rev A	DA Design	17/10/2025



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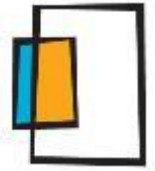
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1. PROJECT DESCRIPTION AND REFERENCES

1.1 INTRODUCTION

This DA structural report has been prepared to outline the structural design criteria and performance requirements for all elements within the project designed by ABC Consultants and all Design and Construct Sub-consultants.

1.2 CLIENT AND PROJECT DETAILS

Client: Anprisa

Site address: 40-76 William St, Leichhardt

The proposed development of a five-to-eleven storey residential flat building comprises:

- The demolition of existing structures including the partial retention and adaptive reuse of the existing warehouse façade.
- Construction of a residential flat building including 203 units inclusive of 5% affordable housing.
- Two basement levels.
- Public domain works.

The existing site is T-Shaped and is bounded by North St to the West and William St to the North with adjacent properties to the East and South of the site.

1.3 DEVELOPMENT CONSENT CONDITIONS

The design of the project will be in accordance with the Development Application conditions and relevant design practices, Australian Standards, and BCA provisions.

1.4 BASE ARCHITECTURAL DOCUMENTS

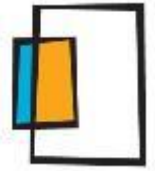
This report is based on architectural sketches and drawing produced by Woods Bagot Architects received Oct 2025.

1.5 STRUCTURE DESCRIPTION

The tower structure consists of a maximum 11 storey concrete frame building with post tensioned flat floor supported on blade concrete columns. The columns run through the building from the footing level to the roof with changes in size occurring over the height of the building. The columns are required to rotate or transition to a different column shape at the underside of the Ground typically and there is a transfer level present at Ground along with additional transfer zones in Level 1 and Basement 1 as result of the loading dock located beneath one of the tower as shown in the attached structural scheme.

A concrete pad foundation system is assumed for the building due to the depth of excavation should be reaching rock. The final design would be confirmed after further design development is carried out.

Lateral loads due to robustness requirements, wind, earthquake and shoring walls are resisted by a combination of concrete lift and stair cores and outrigger walls.



1.6 OTHER DOCUMENTS

Other related consultant documents will be used on “As Required” basis.

1.7 AUSTRALIAN STANDARDS AND BCA

The following Australian Standards in combination with BCA are used in the structural design of this project.

AS/NZS 1170.0/2002 – Part 0: Structural design actions
AS/NZS 1170.1/2002 – Part 1: Permanent, imposed and other actions
AS/NZS 1170.2/2021 – Part 2: Wind actions
AS/NZS 1170.4/2007– Part 4: Earthquake loads
AS3600 – 2018: Concrete structures
AS4100 – 2020: Steel structures
AS3700 – 2018: Masonry structures
AS2159 – 2009: Piling
AS 4678-2002: Retaining walls

2. SITE SPECIFIC INFORMATION AND GROUND CONDITIONS

ABC Consultants assumes that external reports are professionally prepared by appropriately qualified sub-consultants and reviewed by the client. ABC Consultants rely on the information provided in these external reports and will not be held responsible for any of the recommendations or advice contained within these reports.

2.1 GEOTECHNICAL REPORT

Geotechnical investigations and reports have been prepared.

2.2 FOUNDATION DESIGN CONCEPT

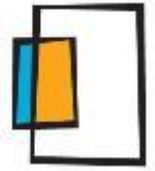
A concrete pad and strip footing foundation is to be the most suitable option for this project if strong rock present at BEL and with the project having 2 basement levels. The quantity and size of the pads would be determined during further design development.

2.3 SLAB ON GROUND

The ground slab will be constructed generally over engineered compacted fill. The slab would be constructed on vapour barrier over 50mm thick 20mm single sized granular fill over engineered compacted fill. The slab would be generally 120mm thick for light vehicle areas. It is understood no heavy vehicle traffic is expected on the slab including the garbage and removalist trucks.

2.4 TEMPORARY AND PERMANENT BATTERS

Refer to Geotechnical Report for any advice on temporary and permanent batters.



2.5 SITE RETENTION AND RETAINING WALLS

Excavation up to 9m is proposed for the site.

For the majority of the site, the shoring system is to be as documented in the structural drawings.

2.6 GROUND WATER TABLE

Geotechnical report will review as part of their works and further advice will be received during the DD phase of works.

2.7 EARTHQUAKE

The earthquake design will be carried out in accordance with the Earthquake Design Code AS1170.4-2007. Refer to Section 3.9 'Earthquake Loads' for design parameters.

2.8 WIND TUNNEL REPORTS

A pedestrian wind study will be undertaken as part of the DA works, please refer to this report for more details.

2.9 DILAPIDATION REPORTS

Dilapidation report of neighbouring sites might be required.

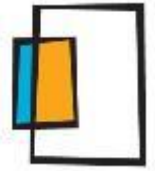
2.10 EXISTING FAÇADE RETENTION

The existing façade on the corner of North and William Streets is required to be retained and integrated into the proposed development. This will require a temporary steel frame which will ensure the stability of the existing facade while the existing building is demolished behind this façade and during the construction phase of the new development.

The proposed temporary steel frame will be constructed on the outside of the building footprint on the footpath adjoining these two facade walls along North St and William St for the full length of the existing façade wall. The steel frame will consist of a series of bracing frames and includes a platform to serve as a Class B hoarding to the site as well as a temporary retention of the facade frame.

By locating the braced frame on the outside of the existing structure and over the existing footpaths, this allows for a safe and careful demolition of the existing building while maintaining the stability of the existing façade as the installation of the support frame works can commence prior to any demolition works occurring and once the new building has been constructed and it now provides the permanent support to the existing face then this retention frame can be removed.

Please refer to the concept sketches in the attached Appendix for details on the proposed methodology for this facade retention frame.



3. DESIGN LOADING INFORMATION

Loads and their appropriate load combinations will be in accordance with AS1170.0, AS1170.1, AS1170.2 and AS1170.4. The applied loading is summarised in this section of this design brief.

3.1 SELF WEIGHT LOADS (SW)

Self Weight loads shall be calculated as provided for in the current version of AS1170. Part 1: Permanent, imposed and other actions.

Material densities are taken from AS1170.1.

3.2 SUPERIMPOSED PERMANENT AND IMPOSED LOADS

Imposed loads are taken from AS1170. Part 1: Permanent, imposed and other actions. The following table describes the more significant loading on the project, and further clarification of floor loads can be obtained by referring to the loading diagrams in the structural set of drawings.

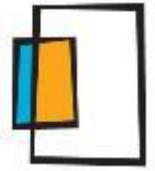
Pattern imposed loads shall be considered if applicable in accordance with Clause 2.4.4 of AS3600. Imposed load reduction shall be applied as per AS1170.1 if appropriate for vertical elements.

Location	Permanent (Dead) Load	Imposed (Live) Load
Car park - General	0.5 kPa	2.5 kPa
Car park – Storage	1.5 kPa	2.4 kPa/m height
Loading Dock	1.5 kPa	15 kPa
Residential floor-internal	1.8 kPa	1.5 kPa
Residential Balconies	1.8 kPa	2.0 kPa
Roof	2.5 kPa	5.0 kPa
Plant rooms	2.0 kPa	7.5 kPa or plant loads whichever is less
Retail/Commercial - internal	1.5 kPa	5.0 kPa
Retail/Commercial Balconies	2.5 kPa	5.0 kPa
Lobbies/Stairs	1.5 kPa	4.0 kPa
Other Gravity Loads:		
Facade Loading	1.0 kPa/m height	

Refer to architectural specifications for the façade type for various building types and locations of landscaping with loads to be determined from architectural drawings. Generally, no heavy external facades/walls have been allowed for residential building such as double brick.

3.3 BALCONY BALUSTRADE LOADS

For balustrade type refer to architectural specifications. For balustrade gravity and imposed barrier loads refer to AS1170.1 appendix A and table 3.3 respectively.



3.4 TRANSFER LOADS

Transfer loads from columns or walls have been allowed for in the design of the transfer structure. These loads will be based on the permanent (dead) and imposed (live) loads provisions above along with the relevant wind and earthquake loadings.

3.5 RETAINING WALL EARTH PRESSURES

Temporary and permanent battering and retaining walls are to be designed in accordance with the geotechnical advice and AS 4678 requirements. A minimum surcharge of 20.0 kPa should be allowed for the perimeter retaining wall design with additional possibly required by the construction team. For the shoring wall design loads, please refer to the structural drawings.

3.6 HYDROSTATIC PRESSURES ON WALLS AND SLABS

All slabs and retaining walls have been assumed to be drained with no hydrostatic applied pressure. This requires provisions for drainage cells/strip drains for shoring walls.

3.7 BUILDING WIND LOADS

Wind loads would be as per AS1170.2 for Region A2 wind, Terrain Category 3, Building Importance Level of 2 and design life of 50 years.

3.8 CONSTRUCTION LIVE LOADS

No allowance has been made in the design for the construction loading exceeding the loading values noted in the section 3.2 of this report. It is assumed that all the slabs would be back propped down to the lowest level if subject to the construction loadings exceeding their design loadings.

3.9 SNOW LOADS

Based on AS1170.3 snow loads are not applicable to this project.

3.10 EARTHQUAKE LOADS

The following parameters should be adopted for the earthquake design as per AS1170.4-2007 and the BCA.

Hazard Factor:	$Z = 0.08$
Life Span:	50 years
Site subsoil classification	TBC
Probability of exceedance	$k_p = 1.0$
Importance Level	2
Structural ductility factor	$\mu = 1.0$



3.11 ROBUSTNESS LOADS

In accordance with the draft requirements of AS1170.0/2002 Amendment 3 the robustness load is taken as 1.0% of each floor gravity load ($G + \psi cQ$) for structural taller than 15m and 1.5% of each floor gravity load ($G + \psi cQ$) for other buildings applied simultaneously at all floors.

3.12 BLAST AND IMPACT LOADING

It is noted that the design of the building and structure and boundary walls has not been designed for any vehicle impact loads (other than car-park barriers loads taken from AS1170.1), nor has the building or its structure been designed for any blast/explosion loadings or terrorist induced loading events.

4. LIMIT STATE DESIGN CRITERIA FOR STRUCTURAL ELEMENTS

4.1 GENERAL DESIGN APPROACH

The limit state design for strength, serviceability and stability of the relevant structural elements within the building will generally follow the established criteria in the relevant material design codes AS4100 and AS3600 unless noted otherwise below.

4.2 STRUCTURAL MOVEMENTS

Building Sways (Deflection) subject to service wind loads shall satisfy;

- Total lateral deflection to not exceed height/500.
- Inter-storey deflection to not exceed inter-storey/500 or 12mm, whichever is lesser

Floor Deflection criteria is to AS3600 table 2.3.2 requirements for concrete floors and AS 1170.0 table C1 requirements for Timber and steel structures. It is recommended that the total long term deflection of the concrete floors is limited to 20mm.

Building inter-story drift at the earthquake ultimate state loads, shall not exceed 1.5% of the storey height for each level.

Attachment of cladding and façade panels to the seismic force resisting system shall have sufficient deformation and rotational capacity to accommodate the design storey drift. In order to satisfy the Hebel fire rating detail, the maximum long term differential deflection between floors is limited to 10mm.

4.3 FIRE RESISTANCE

Generally the buildings are classified as Type A construction. The fire rating requirements for relevant Fire Construction Type/class of building are specified in the BCA.

Structure has been designed to satisfy FRL requirements complying with AS3600, AS3700 and AS4100.

4.4 CRACK CONTROL

Generally, all internal suspended slabs will be designed for a moderate degree of crack control, except for external roof slabs over living areas where a strong degree of crack control will be adopted. For Post tension slabs, this will result in a minimum post tension stress of 1.4MPa and 1.8MPa for moderate and strong degree of crack control, respectively. Refer to Section 4.8 for further minimum requirement to external slabs over living areas.

The exposed slabs such as roof should be designed for temperature effects such as "Hot Top" effect.



Pour strips or Temporary Movement Joints (TMJs) should be introduced where appropriate to minimise the long-term creep and shrinkage effect of the concrete and these should be coordinated with the builder construction programme. None have been shown on the structural drawings produced by ABC Consultants.

4.5 MINIMUM CONNECTION REQUIREMENTS AND TIES

All connections, including but not limited to beam/slabs to columns/wall, precast, etc, shall be designed to clause 6.2.3 AS1170.0 for the transfer of the lateral loads and robustness.

4.6 DURABILITY & MAINTENANCE REQUIREMENTS

Durability requirement are to be as per AS 3600, AS 4100, AS 2311 and AS 2312. Unless noted otherwise a min 50 year design life for concrete elements and min 25 year for steelwork must be adopted. This could be overridden to higher values by Operation and Maintenance requirements imposed by builder and relevant authorities.

4.7 COLUMN AND SLAB STIFFNESS

For the purposes of both post tensioned and reinforced slab designs;

- a. Column stiffness is to be limited to a maximum of 20% for the slab flexure and one-way shear design, but 100% for punching shear design.
- b. Slab stiffness of un-cracked sections are to be limited to a maximum of 60% of gross section capacity for reinforced slab, and 85% in post tensioned slab. The use of a higher slab stiffness for post tensioned slabs may be used if justified by the PT designer. Justification should consider restraints from walls, cores and columns and include calculations.

4.8 CONCRETE ROOF SLABS OVER LIVING SPACES

External suspended slabs that form part of a roof immediately over a residential apartment will have the following minimum design measures to improve the performance of the slab, and minimise the risk of water egress to living area directly under them. However, the slab will not be designed to retain water alone and waterproofing measures to both the slab and joints are to be in accordance with the waterproofing consultant details.

Roof slabs immediately over apartments will include a minimum of the following;

1. A concrete strength of 40 MPa
2. A minimum post tensioned stress of 1.8MPa, and
3. A minimum of SL82 top reinforcement throughout.

Waterproofing membranes and concrete additives are to be provided as per Architects and Waterproofing consultant details.

4.9 FLOOR VIBRATIONS

Floors shall be designed to ensure that there are only slight perceptible vibrations under footfall effects, or from other internal or external sources.

Floors shall be designed to ensure they comply with the recommended acceleration and velocity limits in the relevant standards. The R value method outlined in Annex A of AS2670.2 and Appendix A of BS6472 will be used. A dynamic assessment shall be undertaken in accordance with SCI P354 Design Guidelines or approved equivalent. The floor structures shall be designed to achieve a maximum "Multiplying Factor" R value as follows:

- a. Residential Areas R factor = 2.0 (maximum) daytime and 1.4 night time



The following parameters shall be used for the analysis:

- a. Weight of 1 person 746N (76kg x 9.81)
- b. Critical damping ratio – maximum of:
 - 2.0% for open areas such as open public spaces, restaurants and retail
 - 4.0% for residential areas with regularly spaced full height walls
- c. Dynamic Concrete Modulus and Concrete Mass in accordance with Section 4.1.3 of SCI P354.

Slabs are not designed for vibration emanating from plant equipment, ducting, fans etc. All vibrations from plant are to be isolated at the source with dampers and vibration isolation devices.

5. CONSTRUCTION MATERIALS: CODES, PROPERTIES AND CONSTRUCTION PRACTICES

5.1 CONCRETE

The design, material properties and construction of all reinforced and pre-stressed concrete elements shall comply with the provisions of AS3600 and any other relevant reference noted in this brief.

For detailed information on the specification of concrete elements refer to the Concrete Specification. In particular, note of the curing requirements within the specification to prevent shrinkage and drying shrinkage cracking.

5.2 STRUCTURAL STEEL

The design, material properties and construction of all structural steel elements shall comply with the provisions of AS4100 and have ACRS compliance.

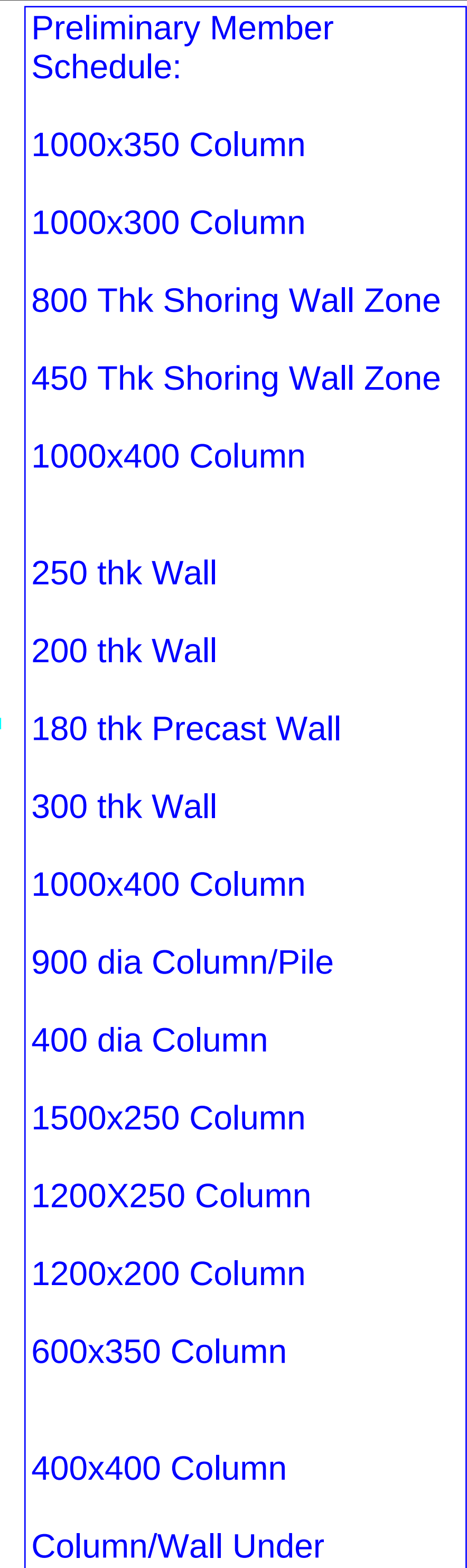
For detailed information on the specification of structural steel elements refer to the Structural Steel Specification.

5.3 MASONRY

The design, material properties and construction of all masonry elements shall comply with the provisions of AS3700 and any other relevant reference noted in this brief.



Appendix A – Structural Drawings by ABC Consultants



NOTE: DO NOT SCALE OFF DRAWINGS. REFER TO ARCHITECTURAL PLANS. VERIFY DIMENSIONS ON SITE.

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Street Address
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Postal Address
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NORTH RYDE BC NSW 16
ABN 15 126 019 190

PROJECT: 40-76 William St Leichhardt, NSW	JOB NUMBER: 23054	DRG NUMBER: SK - B2		
	DESIGNED BY: RC	DATE: Sept 25		
TITLE: Concrete Profile - Basement 2	DRAWN BY: RC	SCALE: 1:200 @ A1	SIZE: A1	REV: P2



Preliminary Member Schedule:

- 1000x350 Column
- 1000x300 Column
- 800 Thk Shoring Wall Zone
- 450 Thk Shoring Wall Zone
- 1000x400 Column
- 250 thk Wall
- 200 thk Wall
- 180 thk Precast Wall
- 300 thk Wall
- 1000x400 Column
- 900 dia Column/Pile
- 400 dia Column
- 1500x250 Column
- 1200x250 Column
- 1200x200 Column
- 600x350 Column
- 400x400 Column
- Column/Wall Under

Slab profiles TBC after column coordination works complete

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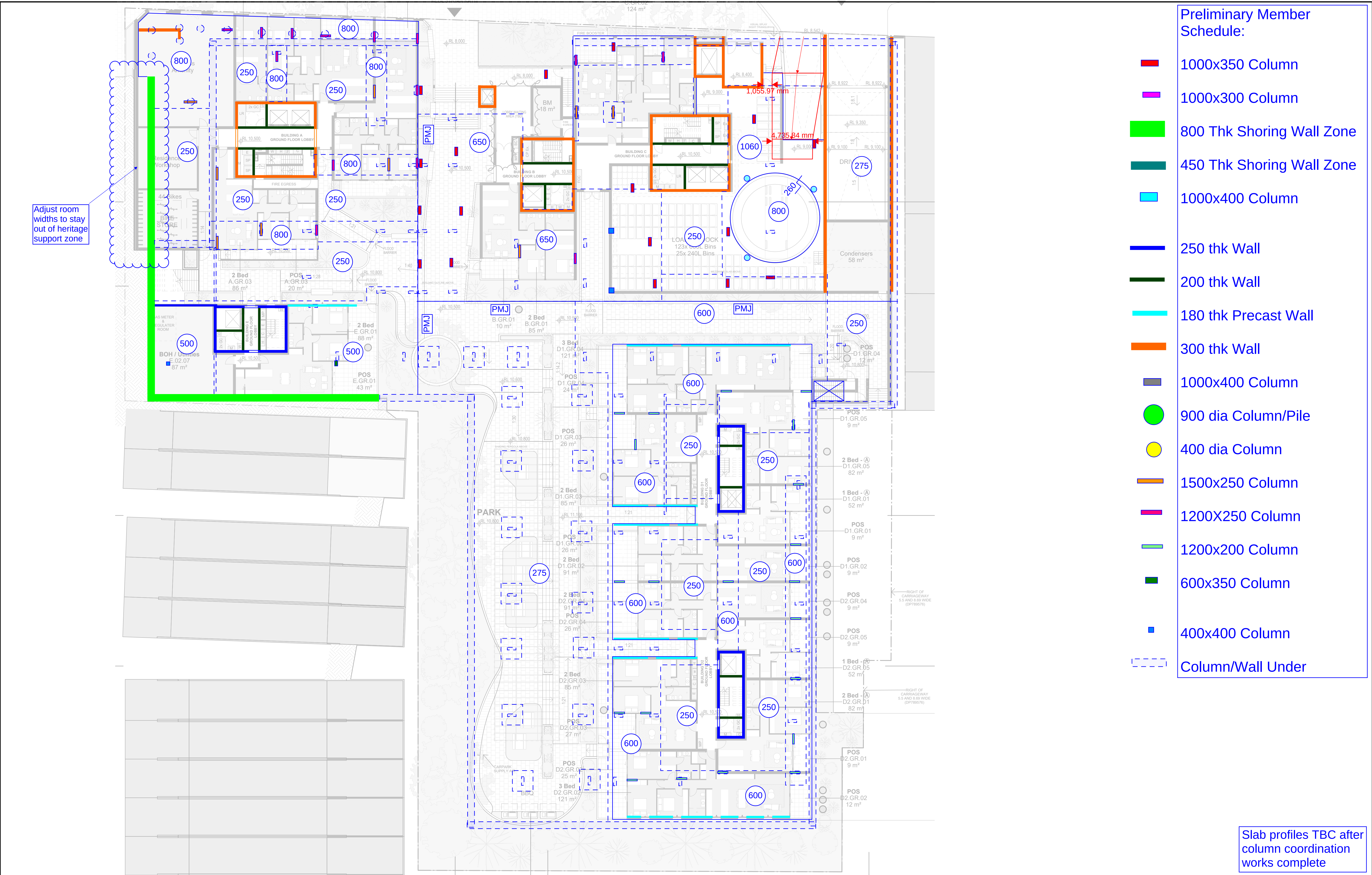
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ABN 15 126 619 190

PROJECT: 40-76 William St Leichhardt, NSW	JOB NUMBER: 23054	DRG NUMBER: SK - B1		
	DESIGNED BY: RC	DATE: Sept 25		
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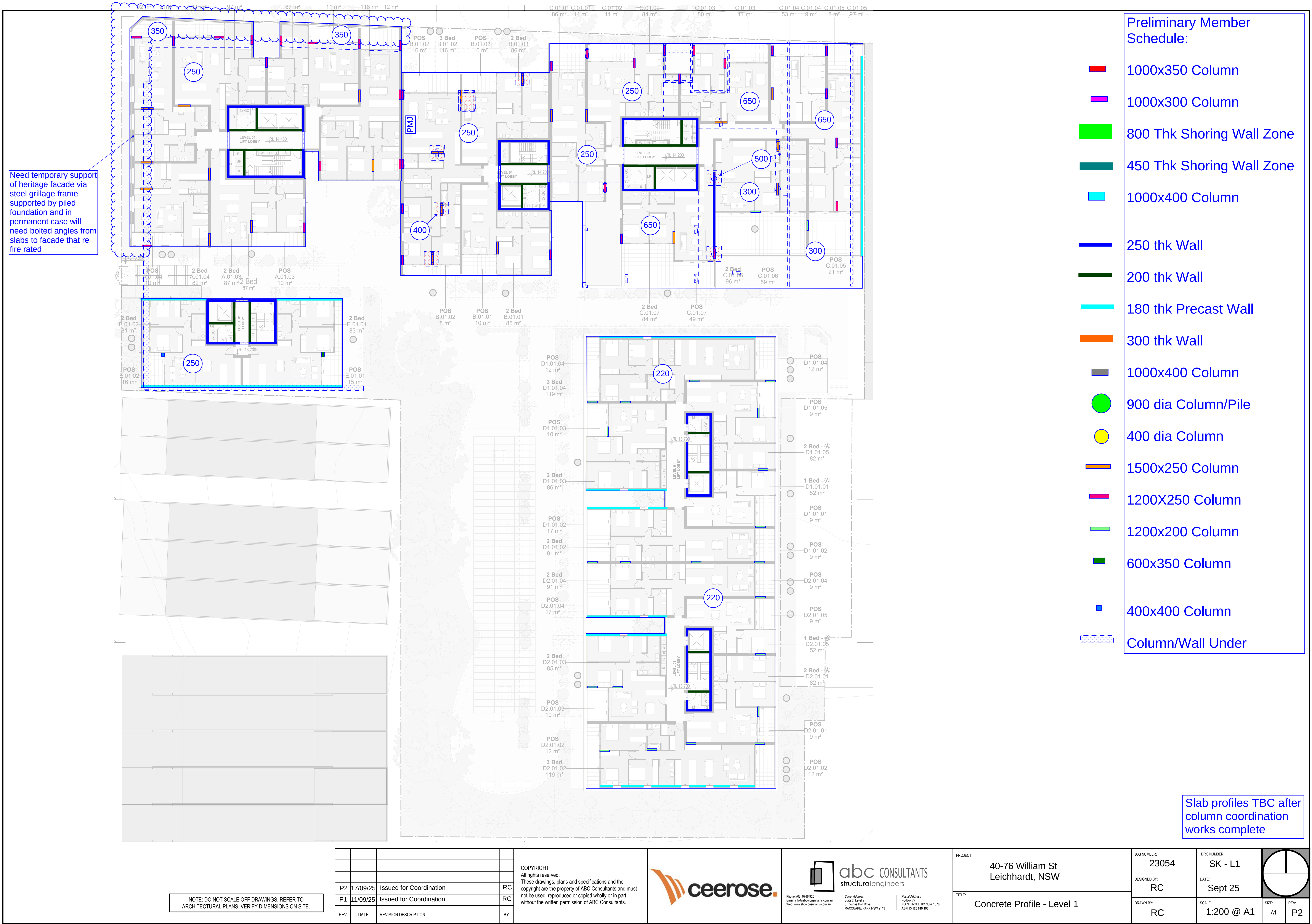
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TITLE: Concrete Profile - Level Ground		DRAWN BY: RC	SCALE: 1:200 @ A1		





Preliminary Member Schedule:

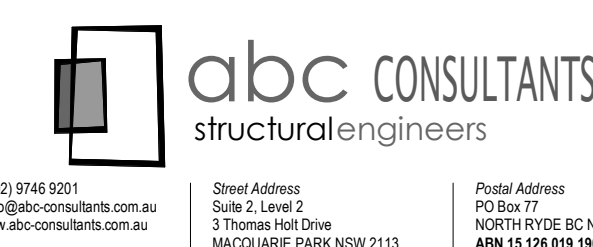
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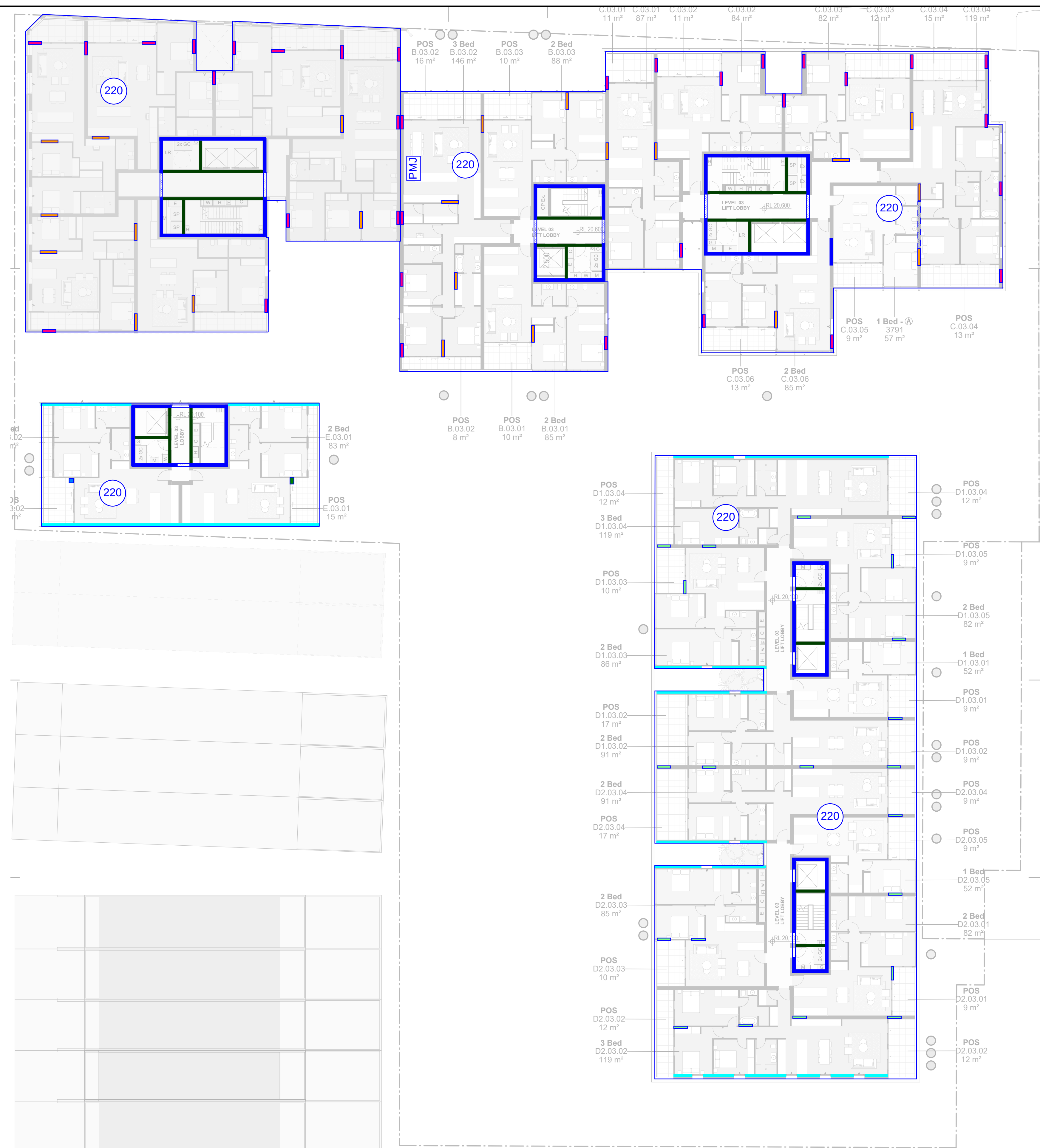
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TITLE:	Concrete Profile - Level 2

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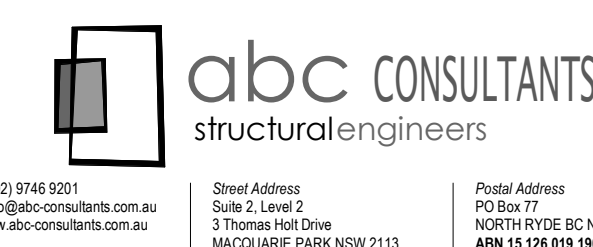
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- 1200x200 Column
- 600x350 Column
- 400x400 Column
- Column/Wall Under

Slab profiles TBC after column coordination works complete

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Preliminary Member Schedule:

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PROJECT:
40-76 William St
Leichhardt, NSW

TITLE:
Concrete Profile - Level 4

JOB NUMBER:
23054

DESIGNED BY:
RC

DRAWN BY:
RC

DRG NUMBER:
SK - L4

DATE:
Sept 25

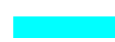
SCALE:
1:200 @ A1

SIZE:
A1

REV:
P2



Preliminary Member Schedule:

- | | |
|---|---------------------------|
|  | 1000x350 Column |
|  | 1000x300 Column |
|  | 800 Thk Shoring Wall Zone |
|  | 450 Thk Shoring Wall Zone |
|  | 1000x400 Column |
|  | 250 thk Wall |
|  | 200 thk Wall |
|  | 180 thk Precast Wall |
|  | 300 thk Wall |
|  | 1000x400 Column |
|  | 900 dia Column/Pile |
|  | 400 dia Column |
|  | 1500x250 Column |
|  | 1200X250 Column |
|  | 1200x200 Column |
|  | 600x350 Column |
|  | 400x400 Column |
|  | Column/Wall Under |

Slab profiles TBC after column coordination works complete

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PROJECT:	40-76 William St Leichhardt, NSW	JOB NUMBER:	23054	DRG NUMBER:	SK - L5	
		DESIGNED BY:	RC	DATE:	Sept 25	
TITLE:	Concrete Profile - Level 5	DRAWN BY:	RC	SCALE:	1:200 @ A1	SIZE: A1 REV: P2



- Preliminary Member Schedule:
- 1000x350 Column
 - 1000x300 Column
 - 800 Thk Shoring Wall Zone
 - 450 Thk Shoring Wall Zone
 - 1000x400 Column
 - 250 thk Wall
 - 200 thk Wall
 - 180 thk Precast Wall
 - 300 thk Wall
 - 1000x400 Column
 - 900 dia Column/Pile
 - 400 dia Column
 - 1500x250 Column
 - 1200x250 Column
 - 1200x200 Column
 - 600x350 Column
 - 400x400 Column
 - Column/Wall Under

NOTE: DO NOT SCALE OFF DRAWINGS. REFER TO ARCHITECTURAL PLANS. VERIFY DIMENSIONS ON SITE.

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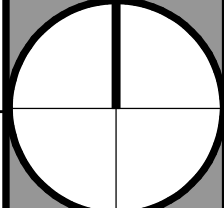
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PROJECT:	40-76 William St Leichhardt, NSW
TITLE:	Concrete Profile - Level 6

JOB NUMBER:	23054	DRG NUMBER:	SK - L6
DESIGNED BY:	RC	DATE:	Sept 25
DRAWN BY:	RC	SCALE:	1:200 @ A1
SIZE:	A1	REV:	P2





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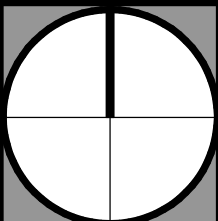
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PROJECT:	40-76 William St Leichhardt, NSW
TITLE:	Concrete Profile - Level 7

JOB NUMBER:	23054	DRG NUMBER:	SK - L7
DESIGNED BY:	RC	DATE:	Sept 25
DRAWN BY:	RC	SCALE:	1:200 @ A1
SIZE:	A1	REV:	P2





- Preliminary Member Schedule:
- 1000x350 Column
 - 1000x300 Column
 - 800 Thk Shoring Wall Zone
 - 450 Thk Shoring Wall Zone
 - 1000x400 Column
 - 250 thk Wall
 - 200 thk Wall
 - 180 thk Precast Wall
 - 300 thk Wall
 - 1000x400 Column
 - 900 dia Column/Pile
 - 400 dia Column
 - 1500x250 Column
 - 1200X250 Column
 - 1200x200 Column
 - 600x350 Column
 - 400x400 Column
 - Column/Wall Under

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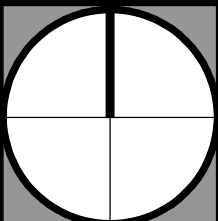
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PROJECT:	40-76 William St Leichhardt, NSW
TITLE:	Concrete Profile - Level 8

JOB NUMBER:	23054	DRG NUMBER:	SK - L8
DESIGNED BY:	RC	DATE:	Sept 25
DRAWN BY:	RC	SCALE:	1:200 @ A1
SIZE:	A1	REV:	P2





- Preliminary Member Schedule:
- 1000x350 Column
 - 1000x300 Column
 - 800 Thk Shoring Wall Zone
 - 450 Thk Shoring Wall Zone
 - 1000x400 Column
 - 250 thk Wall
 - 200 thk Wall
 - 180 thk Precast Wall
 - 300 thk Wall
 - 1000x400 Column
 - 900 dia Column/Pile
 - 400 dia Column
 - 1500x250 Column
 - 1200X250 Column
 - 1200x200 Column
 - 600x350 Column
 - 400x400 Column
 - Column/Wall Under

Slab profiles TBC after column coordination works complete

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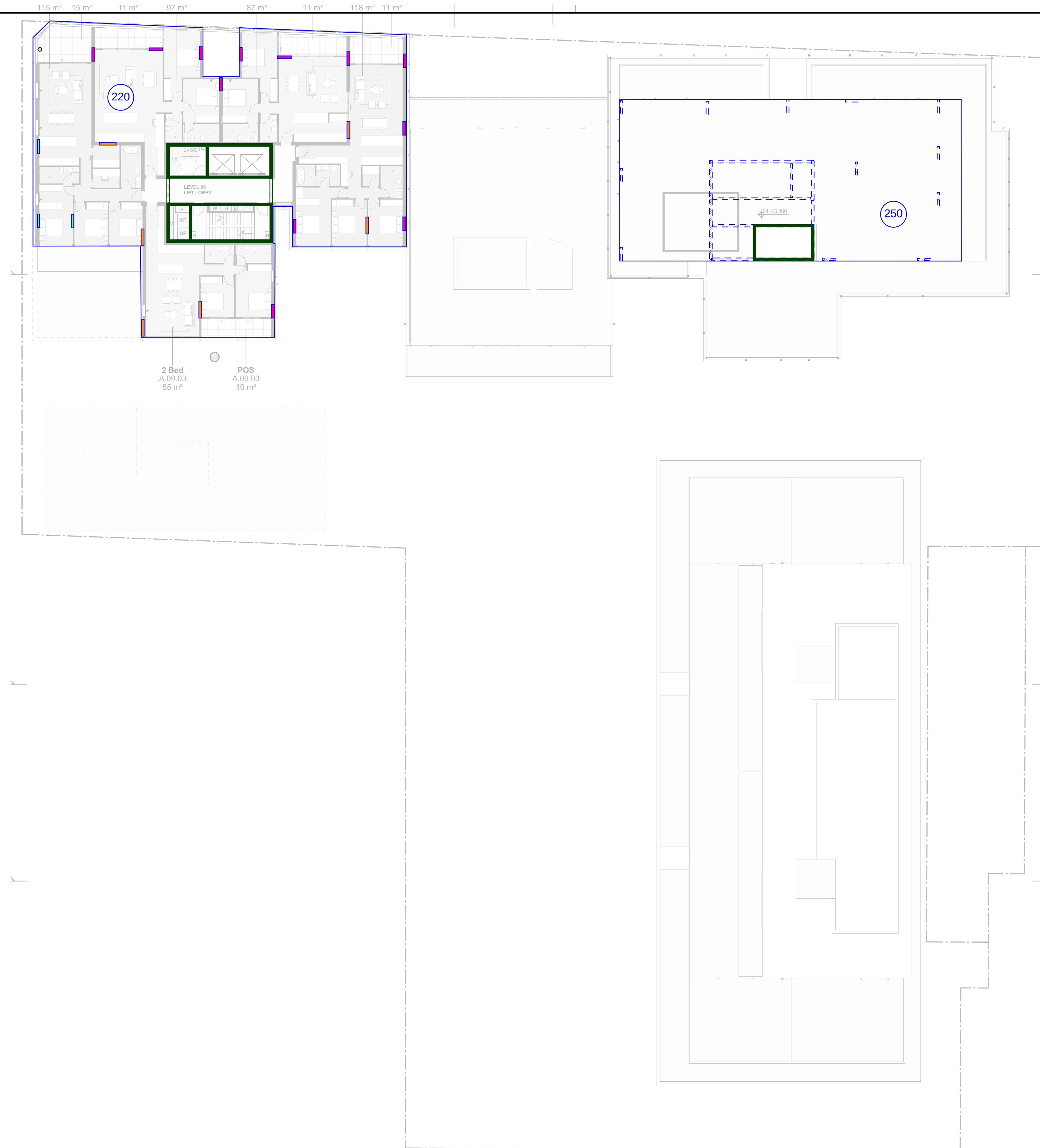


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ABN 15 126 619 199

PROJECT:	40-76 William St Leichhardt, NSW	JOB NUMBER:	23054	DRG NUMBER:	SK - L9		
TITLE:	Concrete Profile - Level 9	DESIGNED BY:	RC	DATE:	Sept 25		
		DRAWN BY:	RC	SCALE:	1:200 @ A1	SIZE:	A1
						REV:	P2



- Preliminary Member Schedule:
- 1000x350 Column
 - 1000x300 Column
 - 800 Thk Shoring Wall Zone
 - 450 Thk Shoring Wall Zone
 - 1000x400 Column
 - 250 thk Wall
 - 200 thk Wall
 - 180 thk Precast Wall
 - 300 thk Wall
 - 1000x400 Column
 - 900 dia Column/Pile
 - 400 dia Column
 - 1500x250 Column
 - 1200X250 Column
 - 1200x200 Column
 - 600x350 Column
 - 400x400 Column
 - Column/Wall Under

Slab profiles TBC after column coordination works complete

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ABN 15 126 619 199

PROJECT: 40-76 William St Leichhardt, NSW		JOB NUMBER: 23054	DRG NUMBER: SK - L10		
		DESIGNED BY: RC	DATE: Sept 25		
TITLE: Concrete Profile - Level 10		DRAWN BY: RC	SCALE: 1:200 @ A1	SIZE: A1	REV: P2



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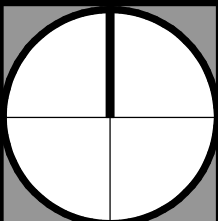
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A/N 15 126 619 190

PROJECT:	40-76 William St Leichhardt, NSW
TITLE:	Concrete Profile - Level 11

JOB NUMBER:	23054	DRG NUMBER:	SK - L11
DESIGNED BY:	RC	DATE:	Sept 25
DRAWN BY:	RC	SCALE:	1:200 @ A1
SIZE:	A1	REV:	P2





Preliminary Member Schedule:	
	1000x350 Column
	1000x300 Column
	800 Thk Shoring Wall Zone
	450 Thk Shoring Wall Zone
	1000x400 Column
	250 thk Wall
	200 thk Wall
	180 thk Precast Wall
	300 thk Wall
	1000x400 Column
	900 dia Column/Pile
	400 dia Column
	1500x250 Column
	1200X250 Column
	1200x200 Column
	600x350 Column
	400x400 Column
	Column/Wall Under

Slab profiles TBC after column coordination works complete

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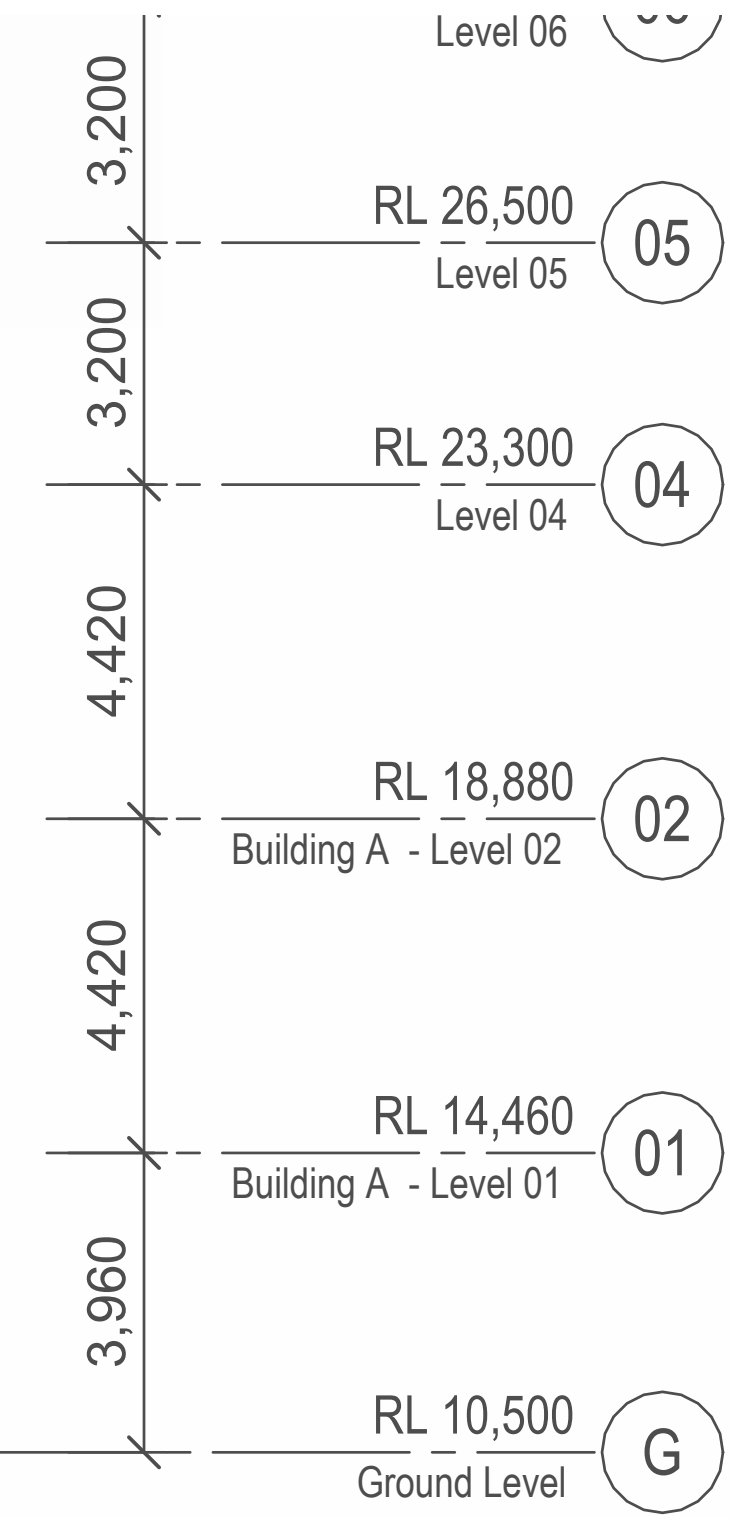


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A/N 15 126 619 190

PROJECT: 40-76 William St Leichhardt, NSW	JOB NUMBER: 23054	DRG NUMBER: SK - Rf		
	DESIGNED BY: RC	DATE: Sept 25		
	TITLE: Concrete Profile - Roof	DRAWN BY: RC	SCALE: 1:200 @ A1	SIZE: A1
		REV: P2		

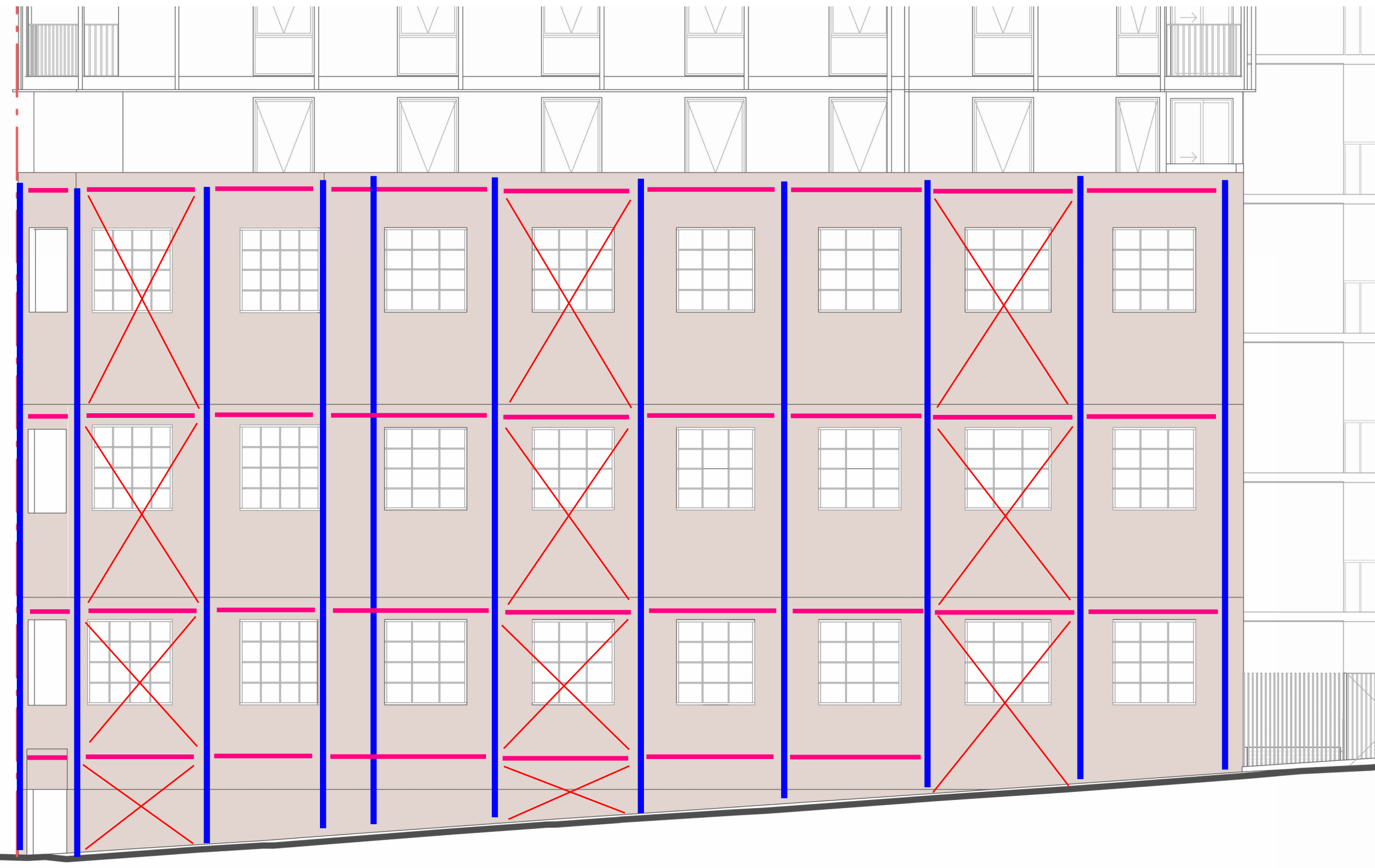
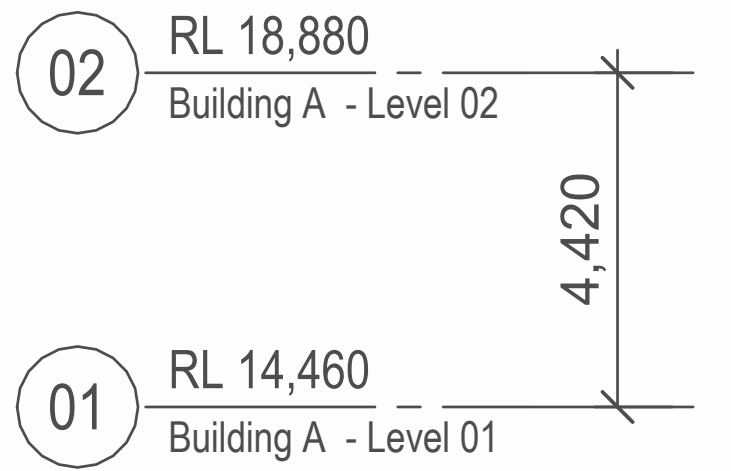


NORTH

ENTRANCE

Building A - North Elevation Existing Facade

WILLIAM ST



Building A - West Elevation Existing Facade

Preliminary Member Schedule:

- 1000x350 Column
- 1000x300 Column
- 800 Thk Shoring Wall Zone
- 450 Thk Shoring Wall Zone
- 1000x400 Column
- 250 thk Wall
- 200 thk Wall
- 180 thk Precast Wall
- 300 thk Wall
- 1000x400 Column
- 900 dia Column/Pile
- 400 dia Column
- 1500x250 Column
- 1200X250 Column
- 1200x200 Column
- 600x350 Column
- 400x400 Column
- Column/Wall Under

Slab profiles TBC after column coordination works complete

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A/N 15 126 619 190

PROJECT: 40-76 William St
Leichhardt, NSW

TITLE: Facade Retention Elevation

JOB NUMBER: 23054

DESIGNED BY: RC

DRAWN BY: RC

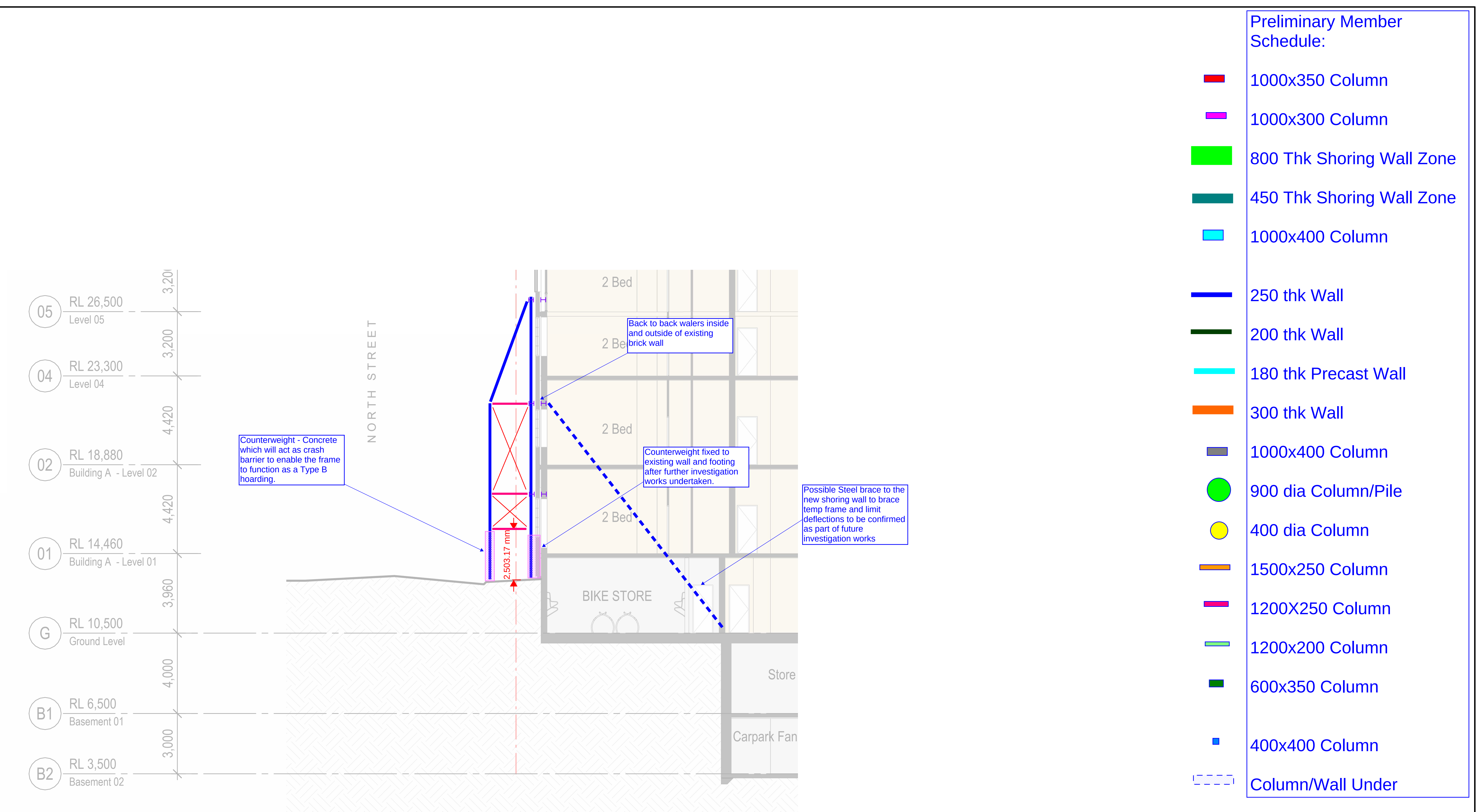
DRG NUMBER: SK - FacEle

DATE: Sept 25

SCALE: 1:200 @ A1

SIZE: A1

REV: P2



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A/N 15 126 619 199

PROJECT:	40-76 William St Leichhardt, NSW
TITLE:	Facade Retention Section

JOB NUMBER:	23054	DRG NUMBER:	SK - FacSec
DESIGNED BY:	RC	DATE:	Sept 25
DRAWN BY:	RC	SCALE:	1:200 @ A1
SIZE:	A1	REV:	P2

