

# North Sydney Public School

## Geotechnical Desktop Study

NSW Department of Education



**Reference: SYDG290593AB**

13 August 2021

# NORTH SYDNEY PUBLIC SCHOOL

---

## Geotechnical Desktop Study

**Report reference number: SYDG290593AB**

13 August 2021

## PREPARED FOR

**NSW Department of Education**  
Level 8, 259 George Street  
Sydney NSW 2000

## PREPARED BY

**Tetra Tech Coffey**  
Level 19, Tower B, Citadel Tower, 799 Pacific Highway  
Chatswood  
NSW 2067 Australia  
p: +61 2 9406 1000  
f: +61 2 9415 1678  
ABN 55 139 460 521

## QUALITY INFORMATION

### Revision history

Revision	Description	Date	Author	Reviewer	Approver
V01	Final	30/07/2021	AM	RMT	STP
V02	Final – Incorporate comments from town planner	13/08/2021	AM	STP	STP

### Distribution

Report Status	No. of copies	Format	Distributed to	Date
V01	1	PDF	Alfred Jury / Tarini Pathak	30/07/2021
V02	1	PDF	Alfred Jury / Tarini Pathak	13/08/2021

## CONTENTS

<b>1.</b>	<b>INTRODUCTION .....</b>	<b>1</b>
<b>2.</b>	<b>DESKTOP REVIEW .....</b>	<b>2</b>
2.1	Site Description .....	2
2.2	Regional Geology .....	2
2.3	Soils Landscape .....	2
2.4	Groundwater .....	2
2.5	Acid Sulphate Soils .....	3
2.6	Previous Investigations .....	3
<b>3.</b>	<b>PRELIMINARY DISCUSSIONS AND RECOMMENDATIONS FOR PROPOSED DEVELOPMENT .....</b>	<b>3</b>
3.1	Geotechnical Ground Model .....	3
3.2	Site Classification .....	4
3.3	Foundations .....	4
3.3.1	Shallow Foundations .....	4
3.3.2	Deep Foundations .....	4
3.4	Excavations .....	5
3.4.1	Bench / Batter Slopes .....	5
3.4.2	Excavation Support .....	6
3.5	Groundwater Impacts .....	6
3.6	Soil Salinity .....	6
<b>4.</b>	<b>RECOMMENDATIONS FOR FURTHER INVESTIGATION .....</b>	<b>6</b>
<b>5.</b>	<b>PROJECT FEASIBILITY .....</b>	<b>7</b>
<b>6.</b>	<b>CLOSURE .....</b>	<b>7</b>

## LIST OF TABLES

Table 1	Approximate Rock Levels from 2019 Coffey Investigation .....	3
Table 2	Preliminary Geotechnical Model .....	4
Table 3	Preliminary Pile Foundation Design Parameters .....	5
Table 4	Recommended Batter Slopes .....	6
Table 5	Earth Pressure Coefficients for Retaining Wall Design .....	6

## APPENDICES

---

<b>APPENDIX A: SSDA DESIGN.....</b>	<b>8</b>
<b>APPENDIX B: SITE PLAN.....</b>	<b>9</b>
<b>APPENDIX C: PREVIOUS INVESTIGATION (2019) BOREHOLE LOGS .....</b>	<b>10</b>



# 1. INTRODUCTION

---

Tetra Tech Coffey Pty Ltd (Coffey) was engaged by NSW Department of Education (DoE) in response to RFT SINSW01975/21 for Geotechnical & Contamination investigations at North Sydney Public School. The purpose of this report is to present a desktop review of available geotechnical information and provide preliminary geotechnical discussions and recommendations to support DoE in their State Significant Development Application (SSDA) and respond to the Secretary's Environmental Assessment Requirements (SEARs) for SSD-11869481 dated 24 December 2020. This have been completed in general accordance with our fee proposal, reference SYDGE29053AA\_Rev 3, dated 23 June 2021.

Coffey has previously conducted geotechnical and contamination investigations at the North Sydney Public School as documented in our site investigation report ref SYDGE232786AD. However, at the time of this investigation, it was proposed that the new building would be located within the playground area to the north of the existing School Hall. As a result, no borehole investigations were carried out within the current development footprint.

An SSDA Design has now been prepared (presented in Appendix A). This SSDA seeks consent for alterations and additions to the existing North Sydney Public School. The proposal entails:

- Demolition of the existing hall (building B), haven building (building C) and six temporary buildings;
- Construction of a three storey building comprising:
  - staff administration rooms;
  - 16 homebases;
  - a new library;
  - hall;
  - out of school hours care facilities;
  - covered outdoor learning area;
  - bicycle parking and end of trip facilities for staff; and
  - services, amenities and access.
- New entry gate and forecourt from Bay Road;
- Internal refurbishment of building G ground floor from the existing library to 3 homebases;
- Capacity for an increase in student numbers from 869 to 1,012; and
- Associated tree removal, landscaping and excavation.

The proposal maintains:

- The gates and fence of former Crows Nest House including the entrance from Pacific Highway and Bay Road;
- Existing gate along McHatton Street;
- The outdoor play area to the east of Building A;
- Existing covered outdoor learning area adjacent to Building A;
- The basketball courts and staff carpark in the western portion of the site;
- The significant tree planting on all school boundaries;
- Buildings A, D and F noting minor internal refurbishments are being undertaken outside of the SSDA scope of work (exempt development) to improve student amenities and canteen; and
- Building G noting ground floor internal refurbishment is proposed in the SSDA.

This report addresses identifies and discusses perceived geotechnical issues and constraints for this proposed development.

## 2. DESKTOP REVIEW

---

### 2.1 SITE DESCRIPTION

The site is located within the existing North Sydney Public School, 182 Pacific Highway, North Sydney. The site includes an at-grade parking area, basketball courts, school buildings and demountables. The school is bounded by Pacific Highway to the east, McHatton Street to the north, a pedestrian and cycle path to the west, and Bay Road to the south. A site plan illustrating the project boundary is presented in Appendix B.

The site slopes to the south with a series of benches, reducing from an RL of approximately 89 m AHD on the McHatton Street boundary to 83 m AHD at the Bay Road boundary. A sandstone block retaining wall separates the site from Bay Road with an elevation difference of approximately 1 m to Bay Road.

A site visit by Coffey on 21 June 2021 noted within the footprint of the proposed development footprint a stepped synthetic area, a grassed play area with play equipment, several existing school buildings including demountables, concrete paths and steps, planters and several established trees. No rock outcropping was observed nor any major cracking of existing structures. Concrete paths appeared in good condition. The asphalt shade cloth area immediately west of the School Hall showed signs of potential settlement, with cracking up to 10 mm wide, up to 2 m long and depressions up to 20 mm.

Surface water was noted as flowing south into drains or a gutter above the Bay Road retaining wall.

### 2.2 REGIONAL GEOLOGY

Reference to the NSW Seamless Geology (March, 2020) database indicates the site is underlain by Ashfield Shale of the Wianamatta Group, characterised by dark-grey to black claystone-siltstone and fine sandstone-siltstone laminite. Hawkesbury Sandstone (which underlies Ashfield Shale), is a medium to very coarse-grained quartz sandstone with very minor shale and laminite lenses outcropping at lower elevation approximately 160 m south-west of the site.

### 2.3 SOILS LANDSCAPE

Reference to the Soil Landscapes of Sydney 1:100,00 Sheet 9030 Map and report indicates the soil landscape of the site locality is on the boundary of the 'Blacktown Residual Soil' and 'GyMEA Erosional Soil' units.

The Blacktown soils are generally brown-black clay and loam residual soils derived from the underlying Ashfield Shale. They typically range from slightly acid (pH 6.5) to strongly acid (pH 4.0), increasing acidity with depth. Blacktown residual soils are typically moderately reactive and moderately to highly plastic. The potential erosion hazard varies from low to high, often dependant on topography.

GyMEA soils are generally yellow-brown clayey sand and sandy clay loams. Derived from the erosion of the Hawkesbury Sandstone, GyMEA soil landscapes display undulating to rolling rises and low hills, with localised rock outcropping and benches. The soils typically range from slightly acid (pH 6.5) to strongly acid (4.5 pH), are low to moderately reactive, with high to extreme potential erosion hazard.

### 2.4 GROUNDWATER

Reference to the NSW Water All Groundwater Map (2021) indicates there are no registered groundwater bores within 500 m of site.

## 2.5 ACID SULPHATE SOILS

Reference to NSW Department of Planning, Industry and Environment eSPADE resource indicates the site has “no known occurrences of acid sulfate soils”. This is consistent with the site geology and therefore no impact is expected by the proposed development. Hence, an Acid Sulfate Soils Management Plan is not required.

## 2.6 PREVIOUS INVESTIGATIONS

Coffey previously completed five boreholes at the site in 2019 (Ref: SYDGE232786AD). The borehole locations are shown on the Site Plan in Appendix B. These boreholes encountered a thin layer of Fill, typically less than 300 mm, underlain by low to medium residual clay 0.75 to 1.8 m thick. The residual soil was underlain by very low to medium strength Ashfield Shale. Groundwater was not encountered in any borehole location.

Coffey has been provided a detailed survey plan of the site as part of the current scope of works. Using this plan and referencing the 2019 boreholes, Table 1 has been produced to approximate the top elevation of Class V Shale. The previous investigation logs are provided in Appendix C.

**Table 1 Approximate Rock Levels from 2019 Coffey Investigation**

Borehole	Approximate Surface RL(m AHD)	Approximate Top of Class V Shale (m AHD) <sup>1</sup>
BH01	85.0	83
BH02	86.5	84.1
BH03	88.3	85.8
BH04	86.6	84.1
BH05	86.8	84.3

1. Classification based on Pells et al. (2019) Classification of Sandstone and Shales in the Sydney Region: A Forty Year Review.

The Site Plan in Appendix B also shows the locations of other nearby Coffey projects. Coffey conducted a geotechnical investigation at 225-235 Pacific Highway in 2014 (Ref: GEOTLCOV25162AA). This investigation observed Fill, typically 0.5 to 2.0 m thick, overlying residual clay typically to a depth of 2.4 to 3.0 m below ground level (m BGL). Boreholes encountered sandstone at an upper elevation of approximately 78.5 m AHD with minor siltstone interbeds, very low strength grading to high strength with depth.

Groundwater was not encountered at 225-235 Pacific Highway investigations during borehole drilling. Two groundwater monitoring wells were installed, with subsequent recording of groundwater levels between 68.5 to 73.2 m AHD.

## 3. PRELIMINARY DISCUSSIONS AND RECOMMENDATIONS FOR PROPOSED DEVELOPMENT

### 3.1 GEOTECHNICAL GROUND MODEL

Based on the outcomes of this desktop study, the preliminary geotechnical model for the development site is presented in Table 2.

**Table 2 Preliminary Geotechnical Model**

Unit	Origin	Description	Approximate Top of Unit (m AHD)	Range of Unit Thickness (m)	Rock Classification <sup>1</sup>
1	Fill	Concrete, asphalt, and sandy gravel	Surface	0.05 – 0.25	N/A
2	Residual Soil	CLAY, low to medium plasticity, trace fine to coarse gravel, stiff to very stiff	84.7 – 87.8	0.75 – 2.0	N/A
3A	Shale	Grey-brown, highly to moderately weathered, very low to low strength	83.0 – 85.8	1.9 – >3.3	Class V/IV
3B		Pale brown and grey, slightly weathered to fresh, medium strength	81.0 – 82.5	-	Class III
4	Sandstone	Pale grey with red bands, fine to medium grained, highly weathered to moderately weathered, very low to low strength	Unproven for this site, potentially near 78.5	--	Unproven though potentially Class V/IV based on nearby site

1. Classification based on Pells et al. (2019) Classification of Sandstone and Shales in the Sydney Region: A Forty Year Review.

## 3.2 SITE CLASSIFICATION

Where there is less than 0.4 m depth of fill, the site is expected to be classed as Class M – moderately reactive with respect to AS 2870 – 2011 Residential Slabs and Footings site classification.

## 3.3 FOUNDATIONS

For the design of the proposed new structures it is expected that shallow pad or pile footings on weathered shale bedrock would be practicable.

### 3.3.1 Shallow Foundations

Considering the proposed development and ground conditions, Coffey consider shallow footings on class V shale may be feasible for the main building structure but expect that a deeper bored pile solution would be more suitable. Where ancillary structures be required, they should be founded on competent natural material and may be designed using a maximum allowable bearing pressure of 200 kPa for stiff to very stiff Residual Soil. To reduce the risk of excessive differential settlement, we recommended that all footing should be founded on similar material.

### 3.3.2 Deep Foundations

Bored pile footings are considered suitable the proposed development at this site. For preliminary assessment of piles, the parameters in Table 4 could be adopted.

**Table 3 Preliminary Pile Foundation Design Parameters**

Geotechnical Unit <sup>(1)</sup>	Ultimate End Bearing (MPa) <sup>(2)</sup>	Serviceability (allowable) End Bearing (MPa)	Ultimate Shaft Adhesion (kPa)	Elastic Modulus (MPa) <sup>(3)</sup>
Class V Shale	2	0.7	50	50
Class IV Shale	3	1	150	250
Class III Shale	6 <sup>(4)</sup>	3 <sup>(4)</sup>	500	700

Notes:

1. Rock classified using Pells et al. (2019) Classification of Sandstone and Shales in the Sydney Region: A Forty Year Review.
2. Assumes a minimum embedment depth of at least 0.5 m into the relevant bearing stratum.
3. For limit state design, serviceability should be assessed using the Young's modulus value to check that settlements are within tolerable limits.
4. There is a risk that Class III Shale could be underlain by lower strength sandstone at about 78.5 m AHD. This could affect pile performance (depending on pile depth and diameter). Further investigation may be required.

If a design of bored piles is adopted, particular attention needs to be given to ensuring the socket is cleaned and roughened using a suitable scraper such as a tooth, oriented perpendicular to the auger shaft prior to pouring of concrete.

For limit state design a geotechnical reduction factor ( $\phi_g$ ) is to be applied to the ultimate geotechnical pile capacity assessed using the ultimate shaft resistance and end bearing values to derive the design ultimate geotechnical pile capacity. In accordance with AS2159-2009,  $\phi_g$  is dependent on assignment of an Average Risk Rating (ARR) which takes into account various geotechnical uncertainties, redundancy of the foundation system, construction supervision, and the quantity and type of pile testing. The assessment of  $\phi_g$  therefore depends on the structural design of the foundation system as well as the design and construction method, and testing (if any) to be employed by the designer and piling contractor. Where no pile load testing is undertaken, a  $\phi_g$  value of 0.6 should be adopted.

For all footing design, where a Serviceability End Bearing Pressure of greater than 1,000kPa is adopted, the rock quality across the building footprint must be assessed by a cored borehole investigation.

All footings should be inspected by a geotechnical engineer to confirm that a suitable founding stratum has been reached.

## 3.4 EXCAVATIONS

While no basement levels are proposed, some excavations into sloping ground at the proposed new building site may be required for the construction of a sub-floors.

### 3.4.1 Bench / Batter Slopes

Batter slopes or bench excavations may be possible where excavations are set back sufficiently from adjacent structures and the site boundary. The batter slopes or benches should be scaled following excavation to remove loose materials which could slide or topple from the face during construction and pose a risk to construction personnel. A summary of the recommended maximum batter slopes for the natural geotechnical units is presented in Table 4. Flatter batter slopes and/or erosion controls and surface drainage may be required depending on local conditions.

**Table 4 Recommended Batter Slopes**

Unit	Short-term Batter Slope (up to 2-month)	Long-term Batter Slope
Unit 2 – CLAY	1H:1V	2H:1V
Unit 3A – Shale	1H:1V	2H:1V
Unit 3B - Shale	0.5H:1V	1.5H:1V

### 3.4.2 Excavation Support

Where insufficient space is available for unsupported, open excavations, excavation support such as shoring or other temporary retaining structures can be employed in excavations in soils or highly weathered rock. However, given the expected site conditions, excavations up to 3m deep are not expected to encounter Unit 3B or better bedrock.

Table 5 presents recommended design parameters for the design of the temporary retaining structures where there is a level retained ground surface. The recommended  $K_0$  values assume that some wall movement and relaxation of horizontal stress will occur due to excavation. Retaining wall analyses will need to consider surcharges, footing loads from adjacent structures and roads and hydrostatic pressure.

**Table 5 Earth Pressure Coefficients for Retaining Wall Design**

Unit	Bulk Density $\gamma$ (kN/m <sup>3</sup> )	Effective Cohesion $c'$ (kPa)	Effective Friction Angle $\phi'$ (degrees)	Coefficient of Active Earth pressure, $K_a$	Coefficient of Earth pressure at rest, $K_0$	Coefficient of Passive Earth pressure, $K_p$	Elastic Modulus (MPa) $E_h$
2	17	0	25	0.4	0.5	2.5	2
3A	20	5	25	0.4	0.5	2.5	7.5
3B	22	10	25	0.4	0.5	2.5	100

## 3.5 GROUNDWATER IMPACTS

Based on our understanding of the site development and given an approximate reduced level for groundwater of 68.5 to 73.2 m AHD, Coffey do not expect the proposed development would encounter or adversely impact the groundwater environment or groundwater quality.

## 3.6 SOIL SALINITY

There is no information at present to suggest that soil salinity would pose a significant risk to the development or surrounds. Accordingly, a Salinity Management Plan is not required.

## 4. RECOMMENDATIONS FOR FURTHER INVESTIGATION

Though existing borehole locations are not within the development footprint, the site conditions appear horizontally consistent. Depending on the final design details beyond the SSDA, there may not be a need for significant further investigation. However, this should be assessed when the final design is more advanced and footing/column loads are known.



We expect that it would also be prudent to drill some boreholes within the building footprint to ground truth pile locations and confirm expected bedrock conditions. This would also allow for more accurate pile-cage construction and increased confidence in the anticipated ground conditions.

We also recommend additional chemical testing of soils for aggressivity to concrete and steel structures to inform the concrete selection for piles.

We note that additional geotechnical investigations addressing these recommendations are currently planned by DoE to be carried out in the near future.

## 5. PROJECT FEASIBILITY

---

Based on our site observations, the preliminary geotechnical model, and experience on similar projects, the proposed development is considered feasible from a geotechnical perspective. In our opinion, the proposed development would present a low risk to surrounding structures and the groundwater environment, provided that appropriate site-specific investigation, design assessments, and construction monitoring normally associated with this type of development are carried out.

## 6. CLOSURE

---

The preliminary geotechnical assessment and recommendations of this report are based on a desk study limited to regional information and existing subsurface investigation data that is not located within the development footprint.

Subsurface conditions can be complex and vary over relatively short distances – and over time. Site specific investigations will be required to support detailed design. Detailed design and construction should not proceed based on this desk study report without further advice from us.

The attached document entitled “Important information about your Coffey report” forms an integral part of this report and presents additional information about it uses and limitations.

## APPENDIX A: SSDA DESIGN

---

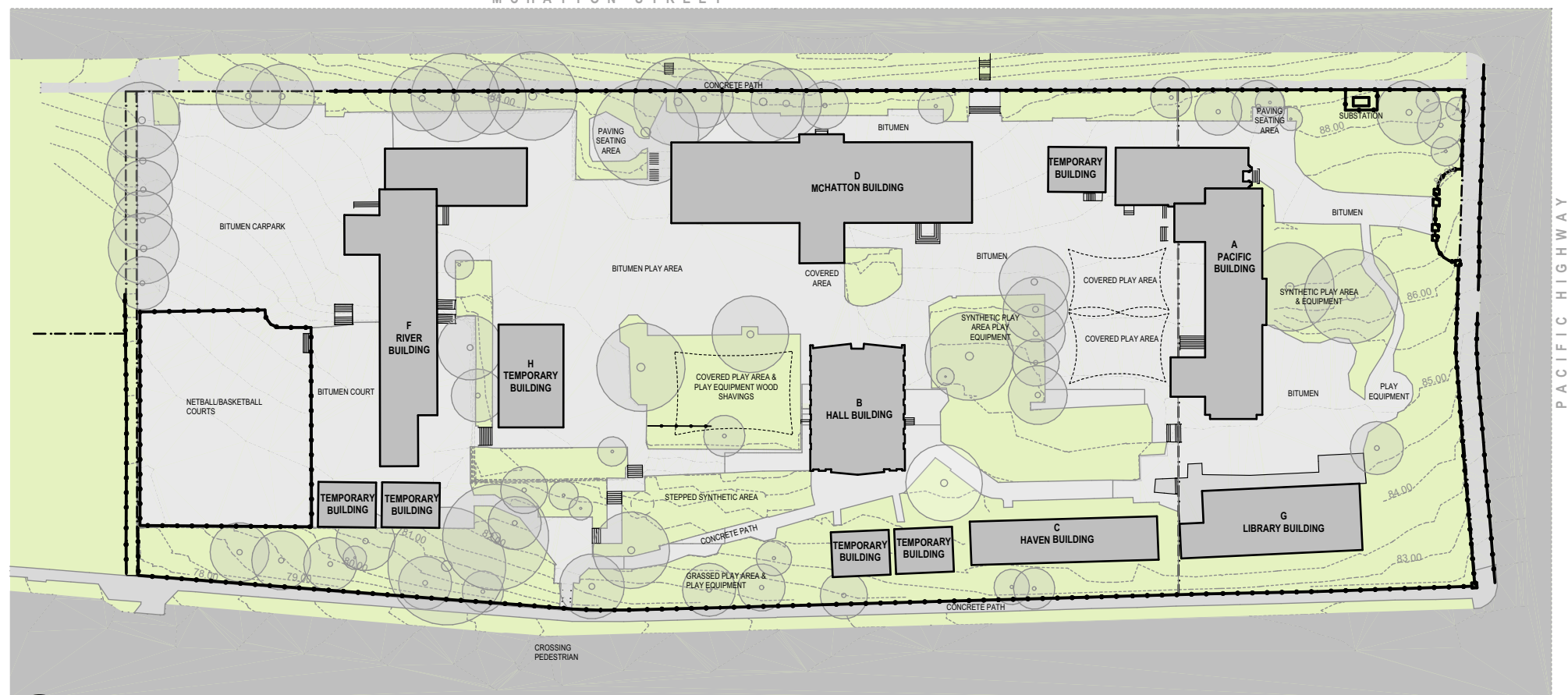
# NORTH SYDNEY PUBLIC SCHOOL

## FOR NSW DEPARTMENT OF EDUCATION

7068WA01



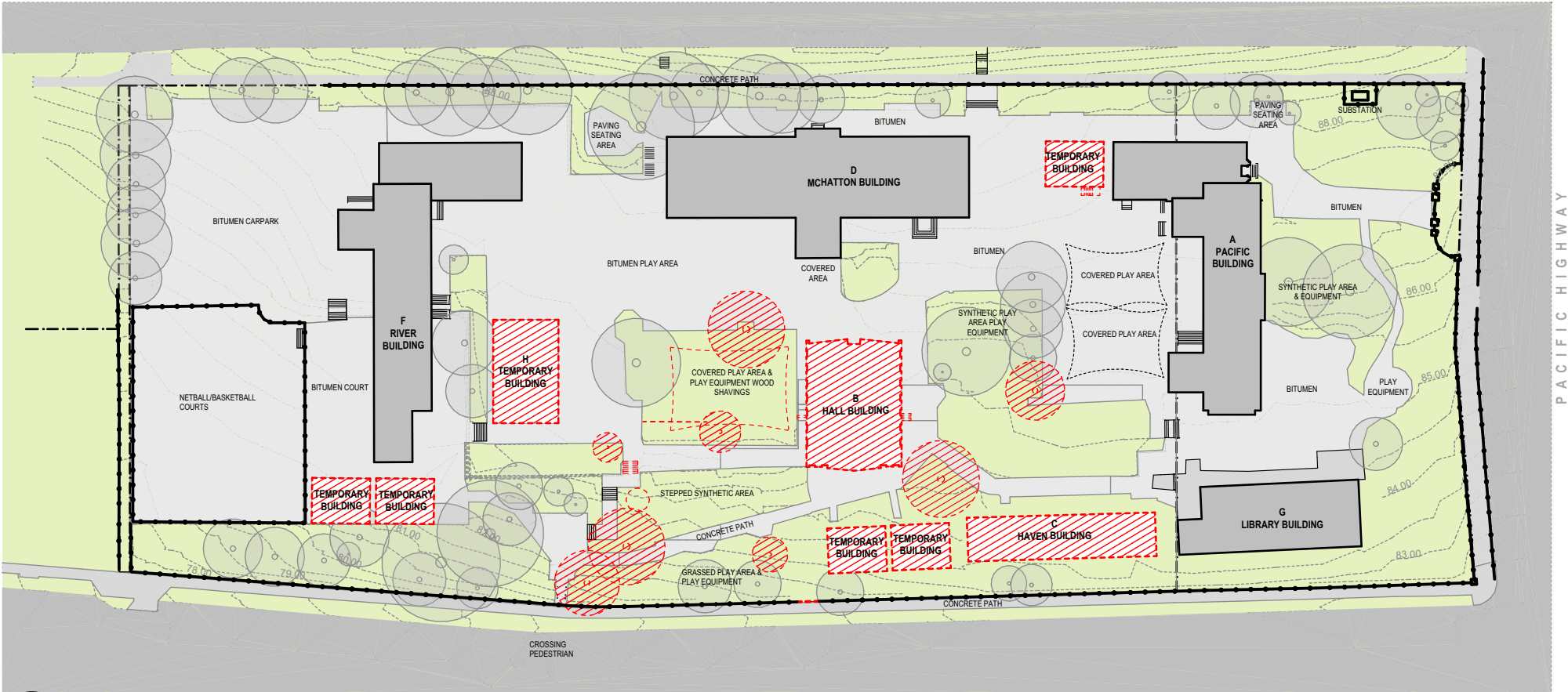
CD-000	Title Page	B
CD-001	Site Analysis 01	B
CD-002	Site Analysis 02	B
CD-003	Site Analysis 03	B
CD-004	Site Analysis 04	B
CD-005	Site Analysis 05	B
CD-006	Site Analysis 06	B
CD-101	Existing Site Plan	B
CD-102	Demolition Plan	C
CD-103	Proposed Site Plan - Level 1 / Street Level	B
CD-104	Proposed Site Plan - Level 2 / Courtyard Level	B
CD-105	Proposed Site Plan - Level 3	B
CD-106	Perspectives	C
CD-201	Level 1 / Street Level - Hall	C
CD-202	Level 1 / Street Level - Admin_Home Bases	C
CD-203	Level 2 / Courtyard Level - Hall	C
CD-204	Level 3 - Hall_Plant	C
CD-205	Level 3 - Home bases	C
CD-206	Proposed Plans - Building F	B
CD-207	Proposed Plans - Building F	B
CD-208	Proposed Plans - Building D	B
CD-209	Proposed Plans - Buildings A & G	B
CD-210	Roof Plan - Home Bases	B
CD-211	Roof Plan - Hall	B
CD-301	Elevations	B
CD-302	Elevations	B
CD-303	Elevations	B
CD-304	Elevations	B
CD-901	Level 1 - Administration FF+E Plan	B
CD-902	Home Base Cluster - FF+E Plan	B
CD-903	Library - FF+E Plan	C



**1 PLAN**  
**EXISTING SITE PLAN**  
 SCALE: 1:1000





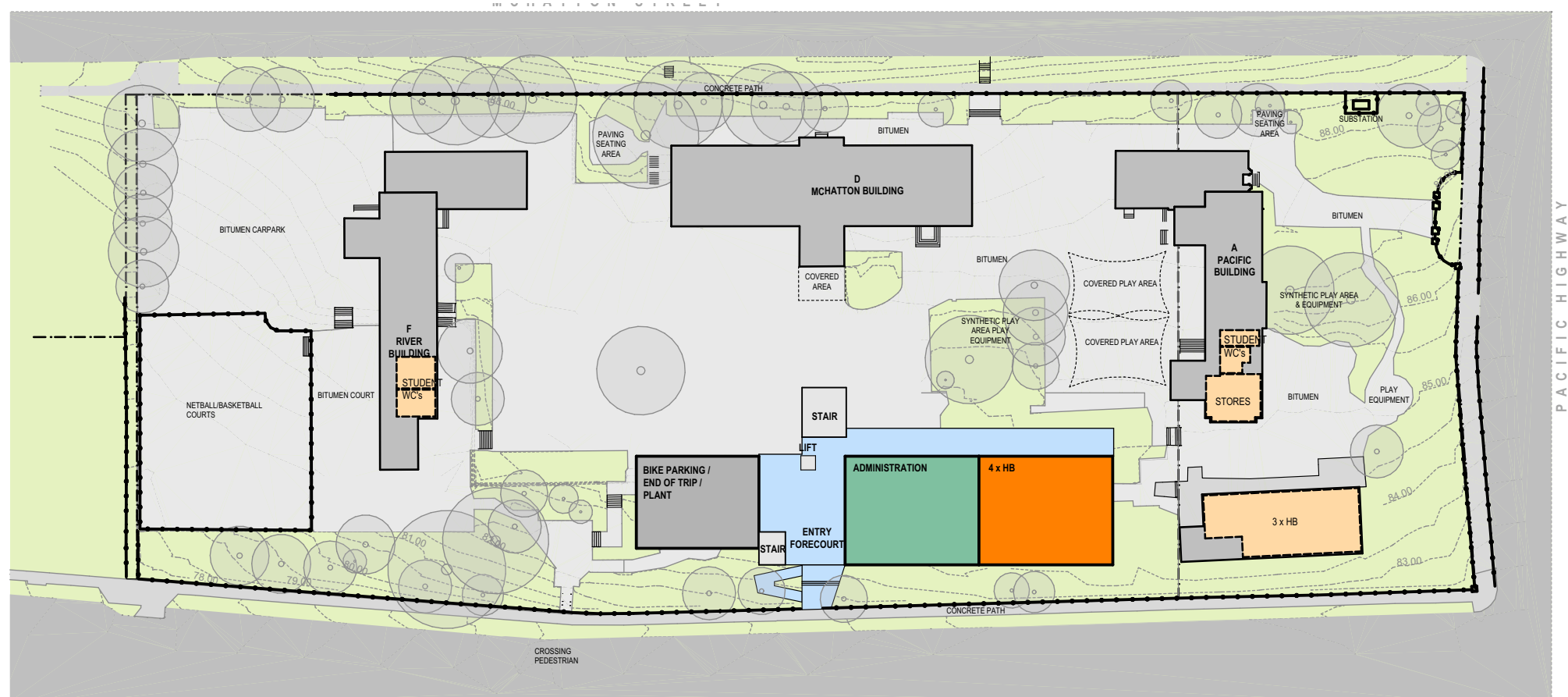


1 PLAN  
DEMOLITION SITE PLAN  
SCALE: 1:1000

SITE PLAN LEGEND

- EXISTING
- DEMOLISHED
- TREE TO BE REMOVED

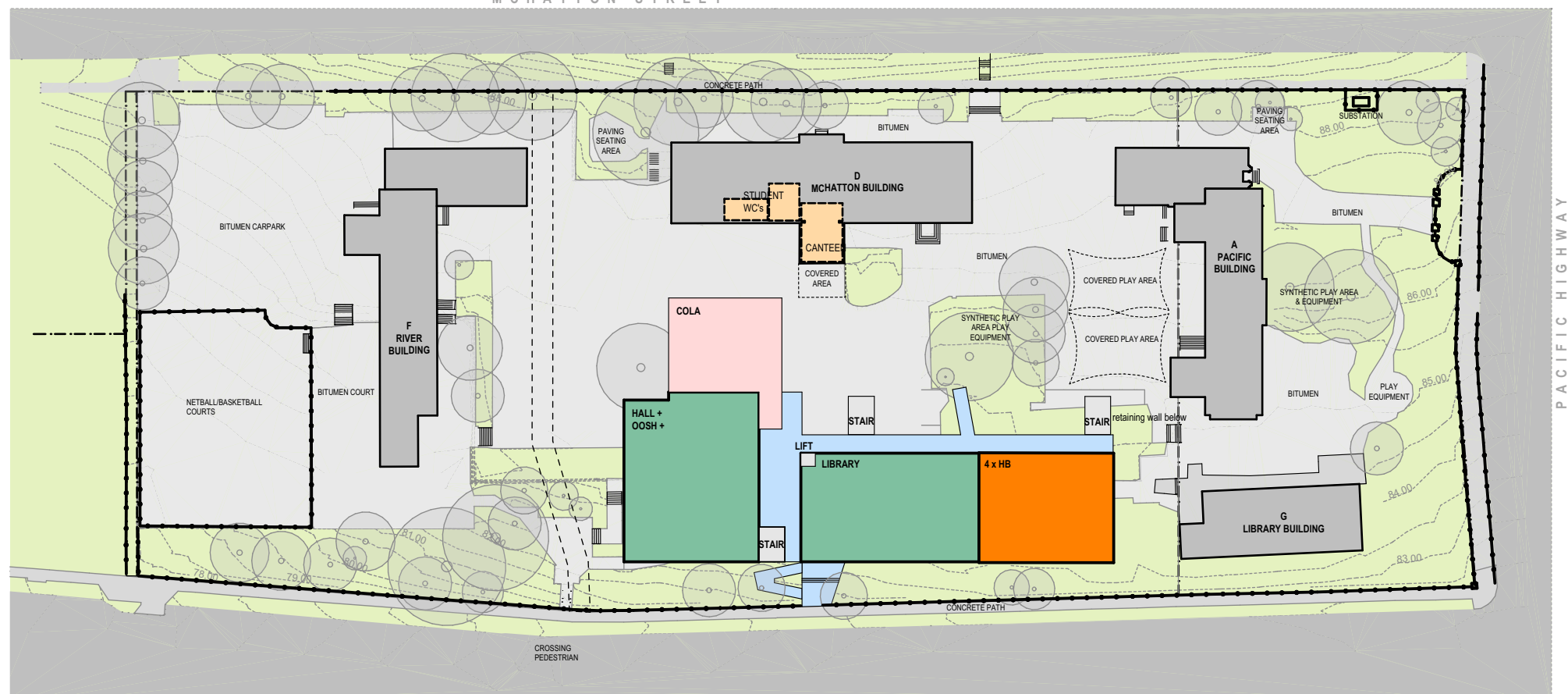




**1 PLAN**  
**PROPOSED SITE PLAN - LEVEL 1/STREET LEVEL**  
 SCALE: 1:1000

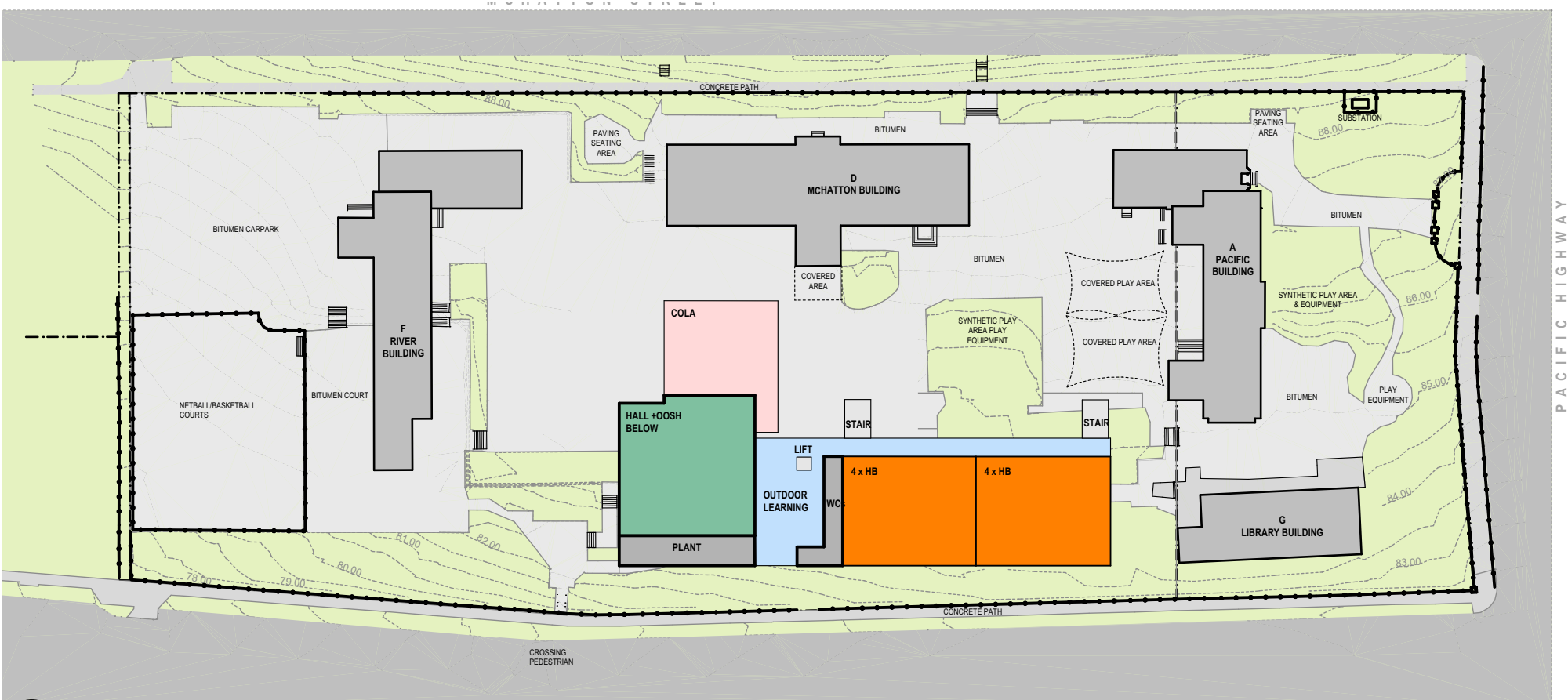






**1 PLAN**  
**PROPOSED SITE PLAN - LEVEL 2/COURTYARD LEVEL**  
 SCALE: 1:1000





**1 PLAN**  
**PROPOSED SITE PLAN - LEVEL 3**  
SCALE: 1:1000







OVERVIEW



EDWARD STREET

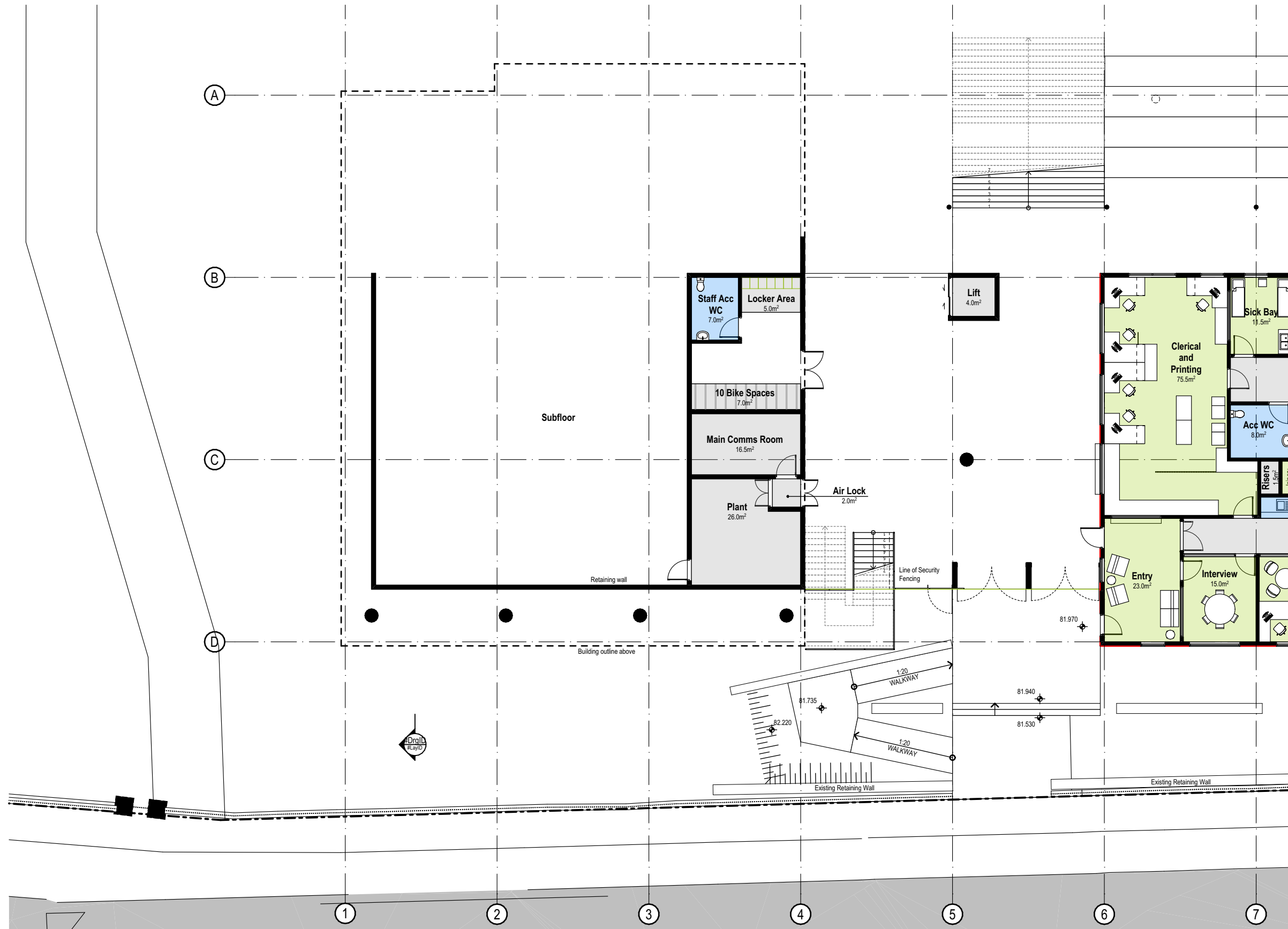


BAY ROAD



PACIFIC HIGHWAY





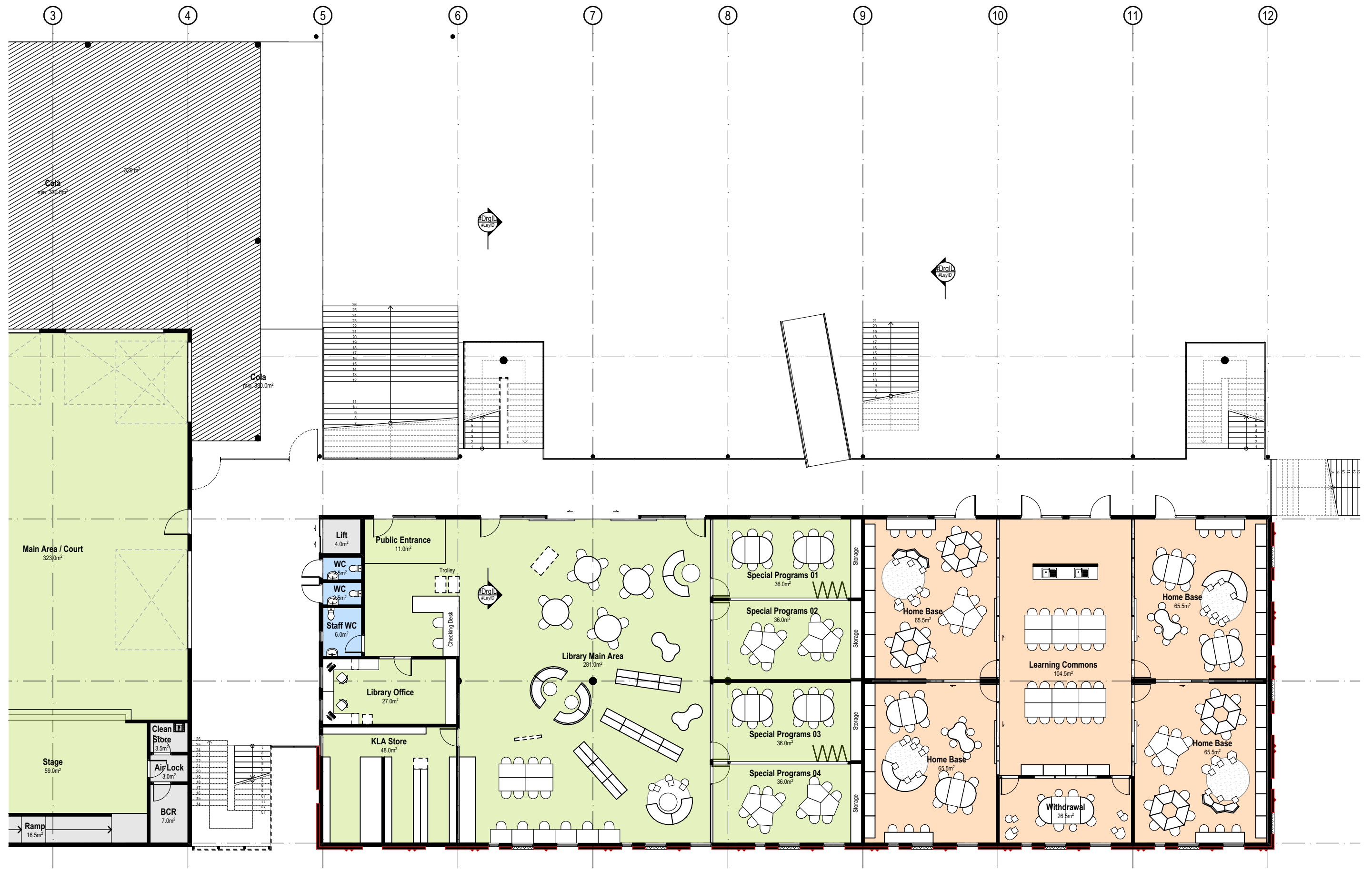
**1 PLAN**  
**LEVEL 1 - BIKE PARKING / END OF TRIP / PLANT**  
 SCALE: 1:200





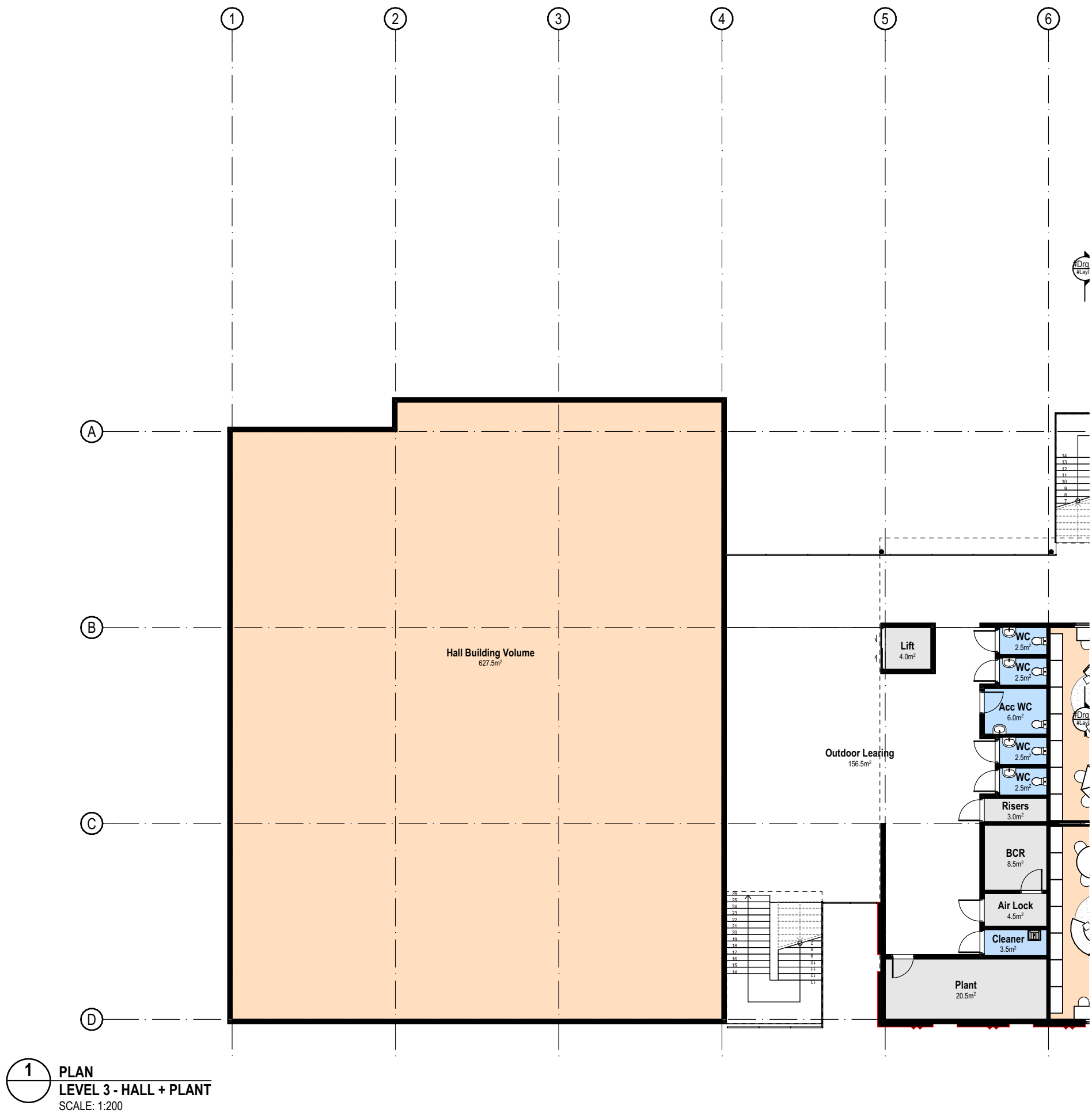






1 PLAN  
LEVEL 2 - LIBRARY  
SCALE: 1:200





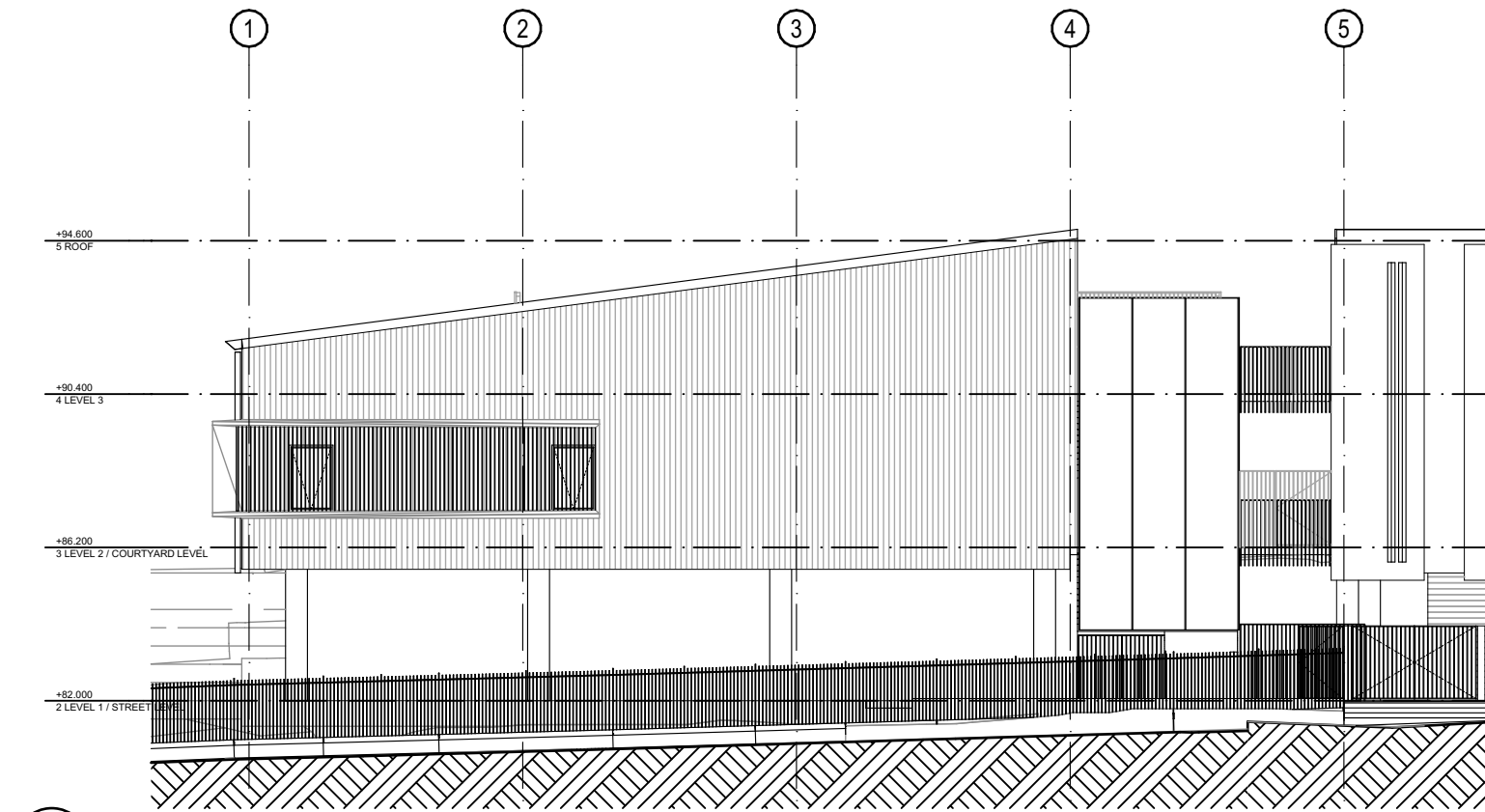
**1** PLAN  
LEVEL 3 - HALL + PLANT  
SCALE: 1:200



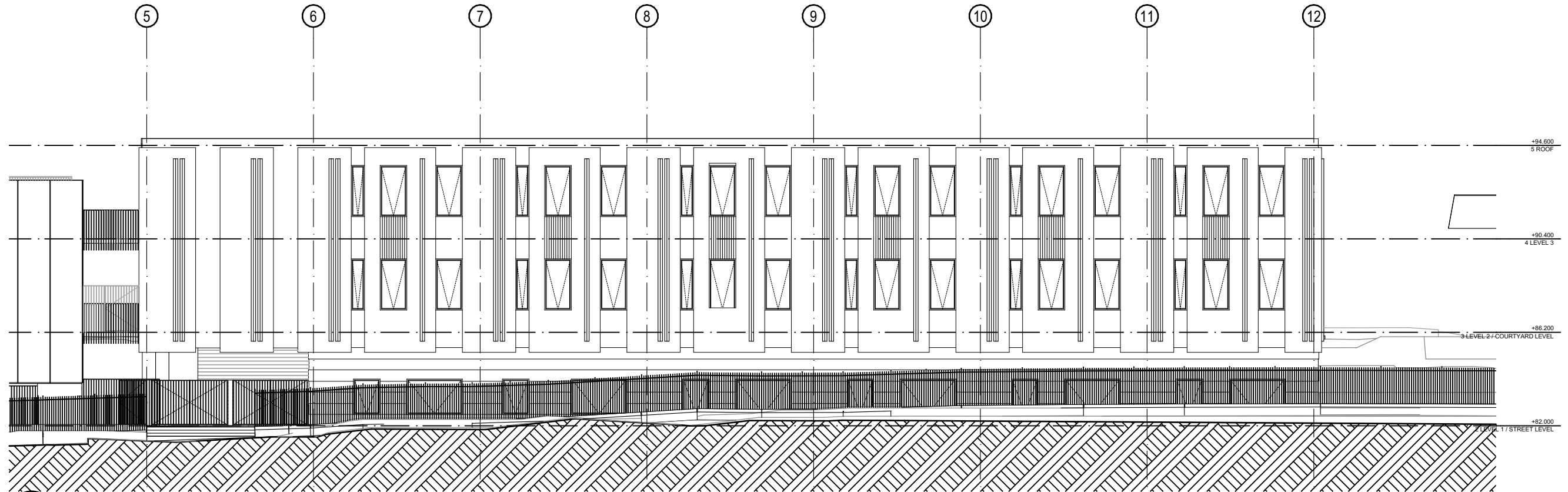


**1 PLAN**  
**LEVEL 3 - HOME BASE**  
SCALE: 1:200

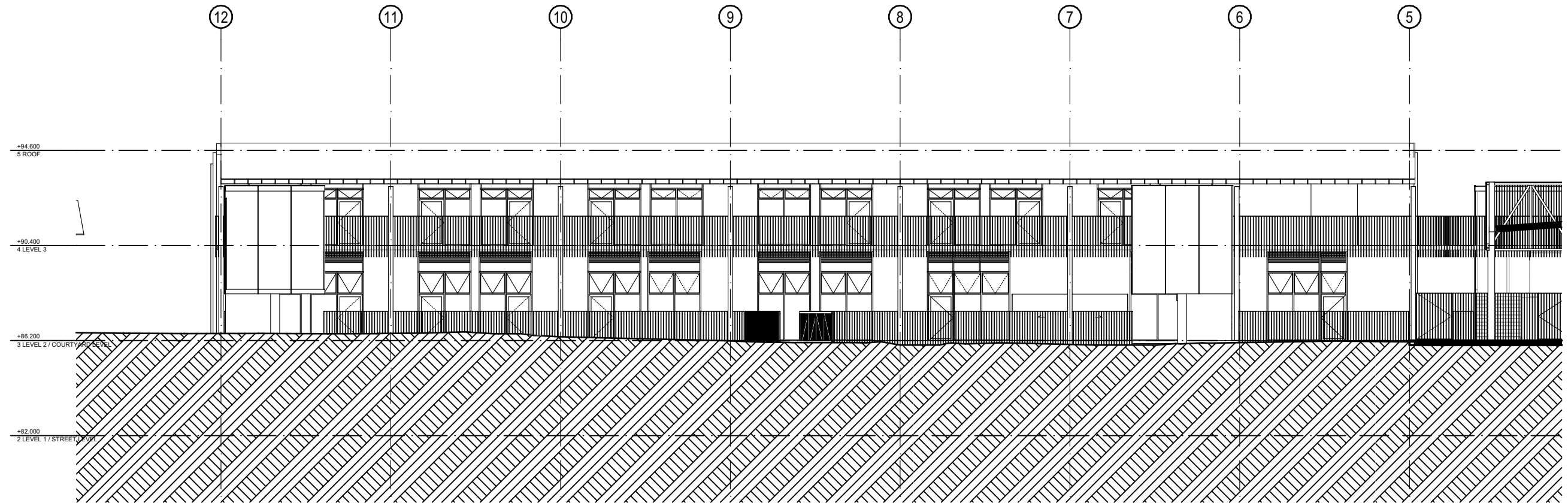
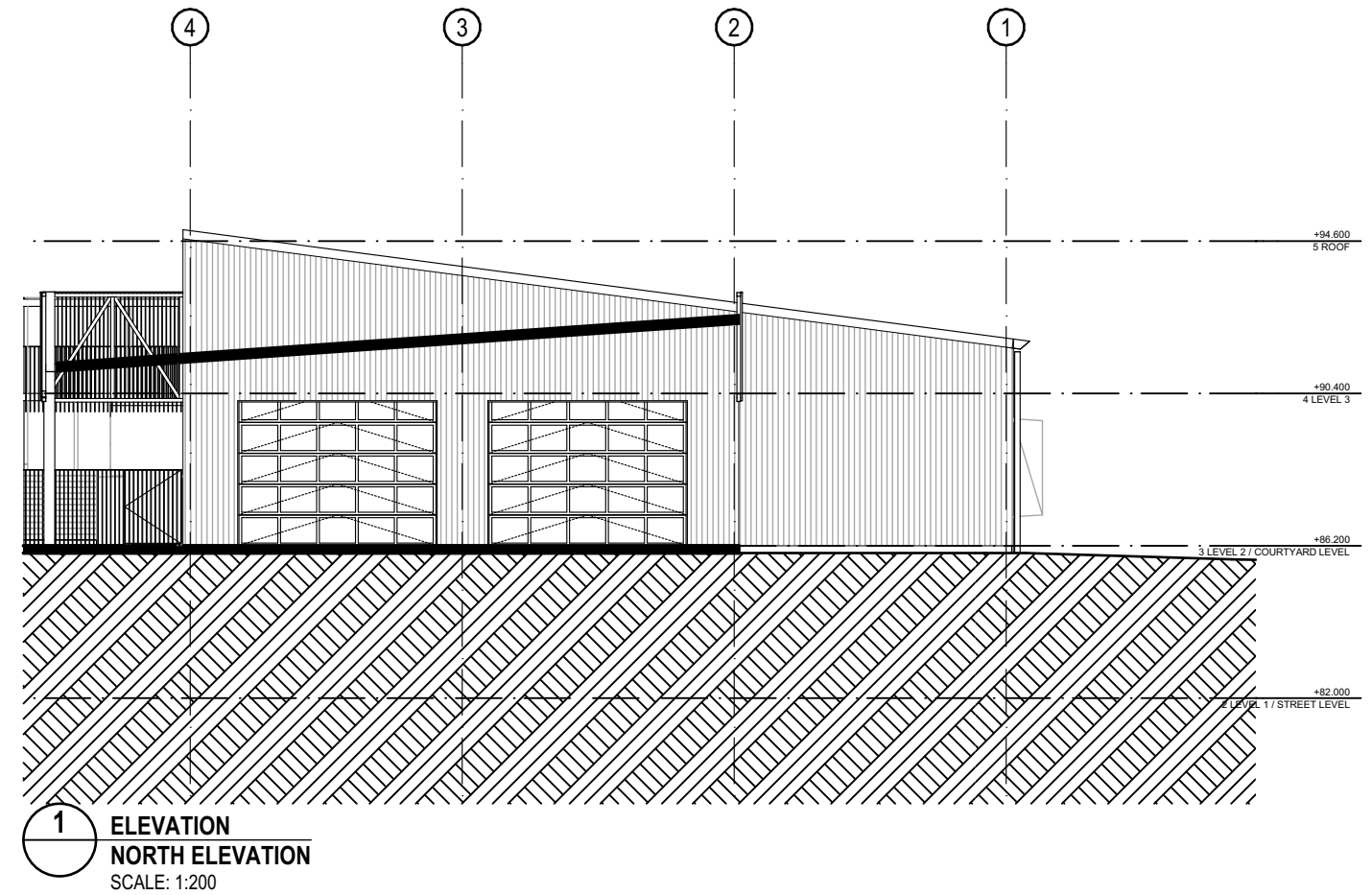


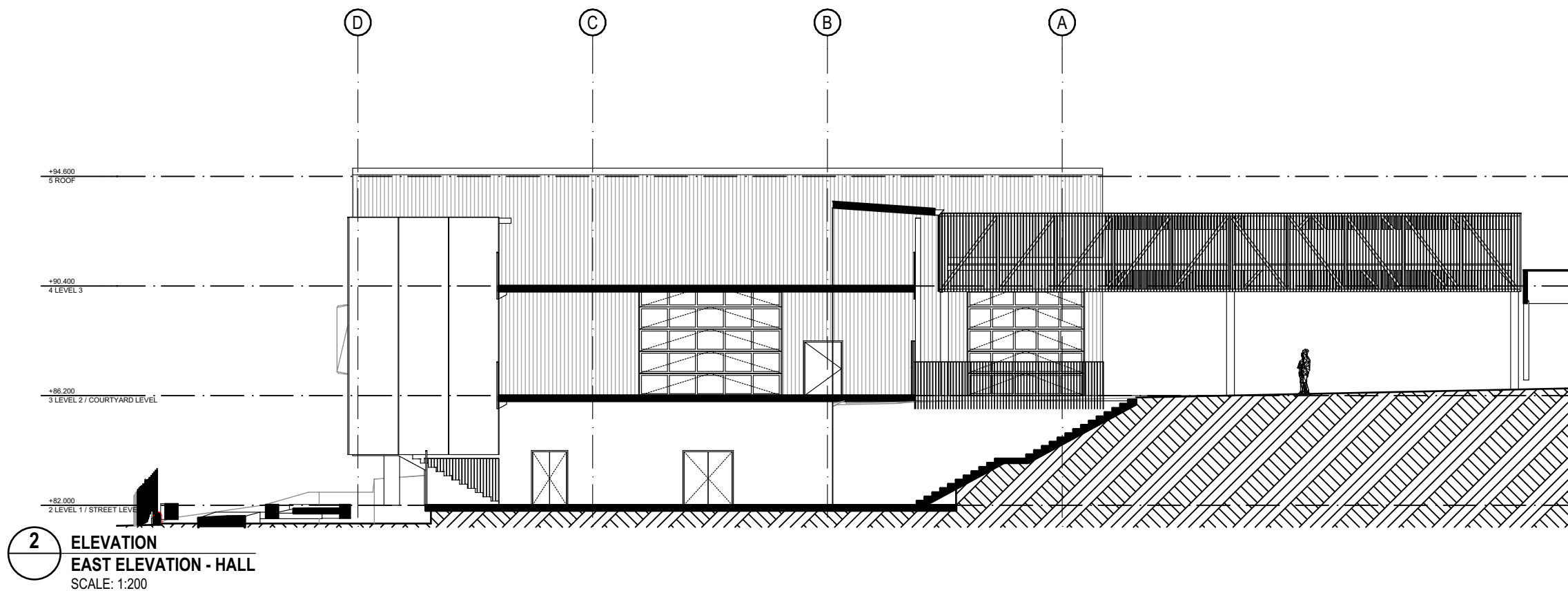
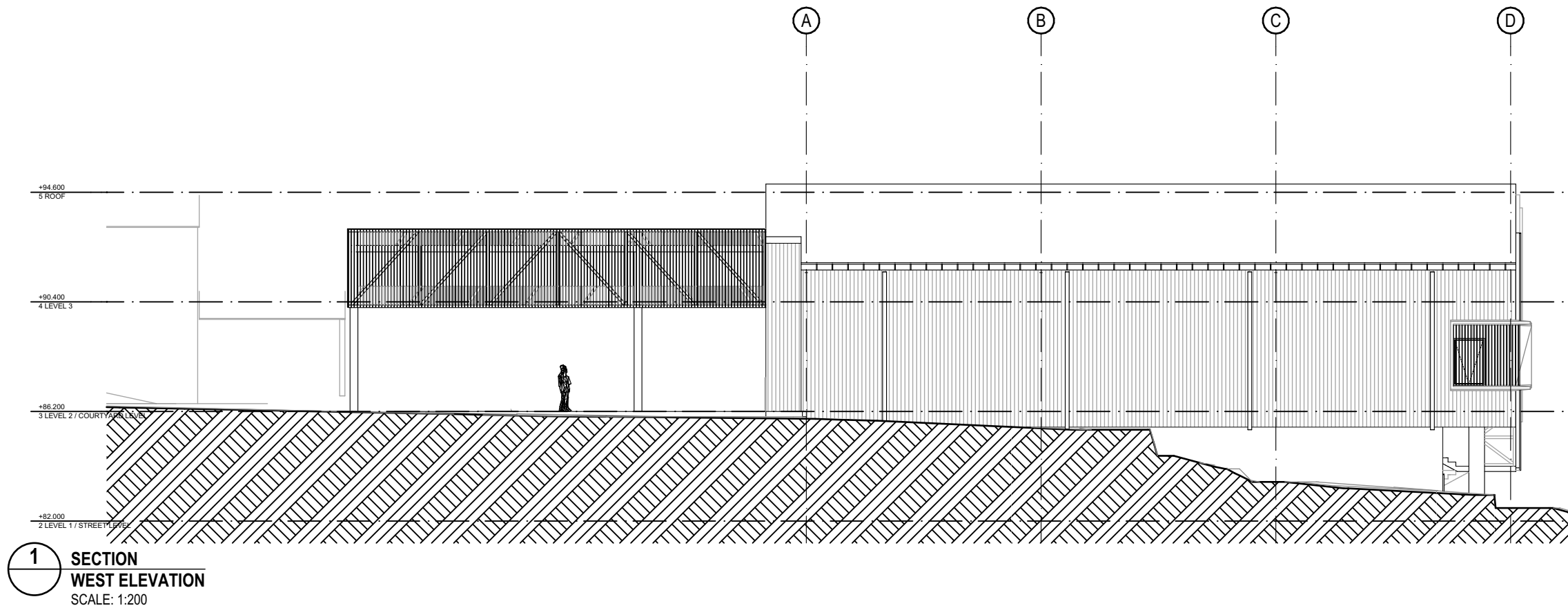


**1** ELEVATION  
SOUTH ELEVATION  
SCALE: 1:200

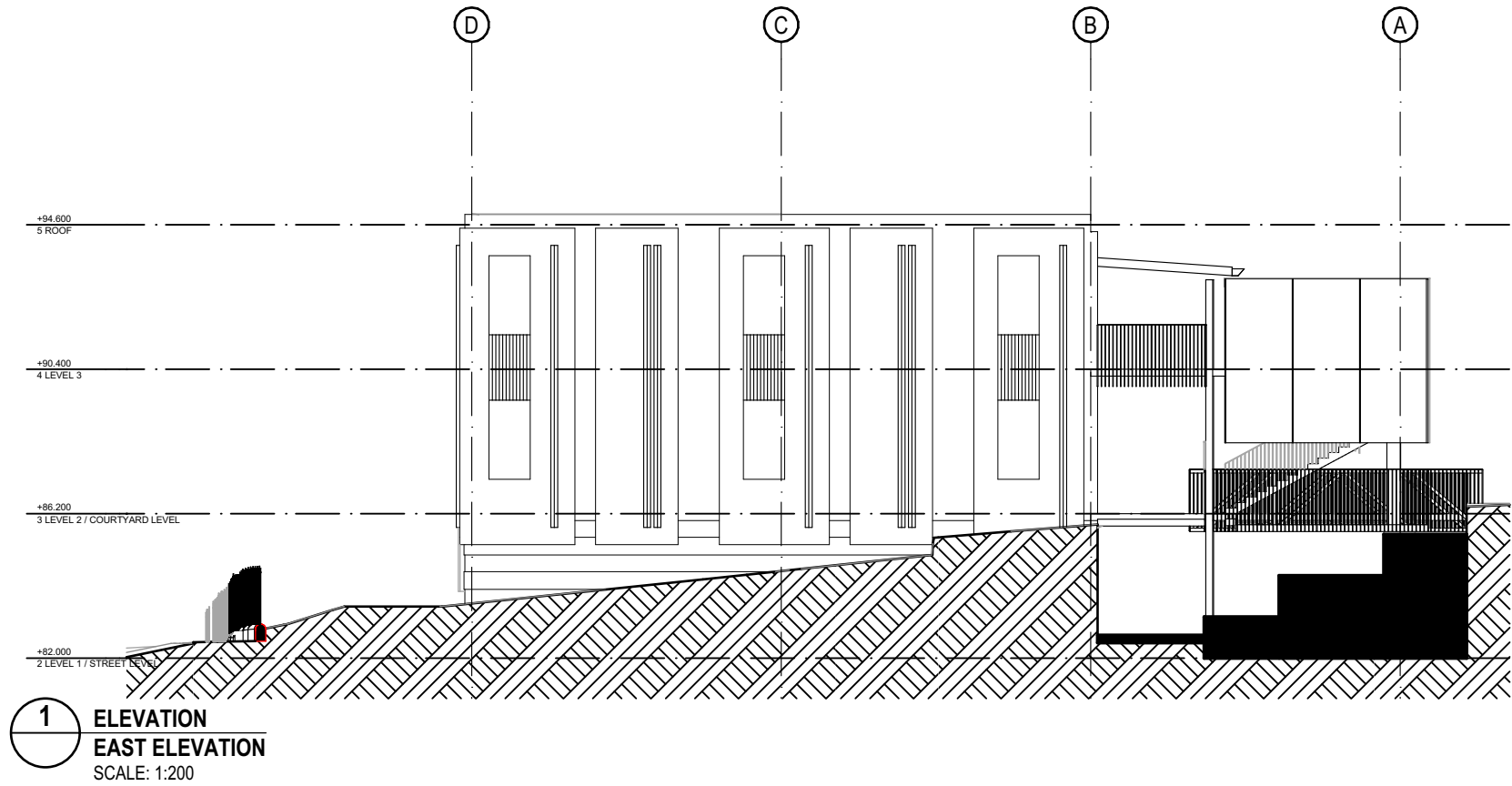


**2** ELEVATION  
SOUTH ELEVATION  
SCALE: 1:200





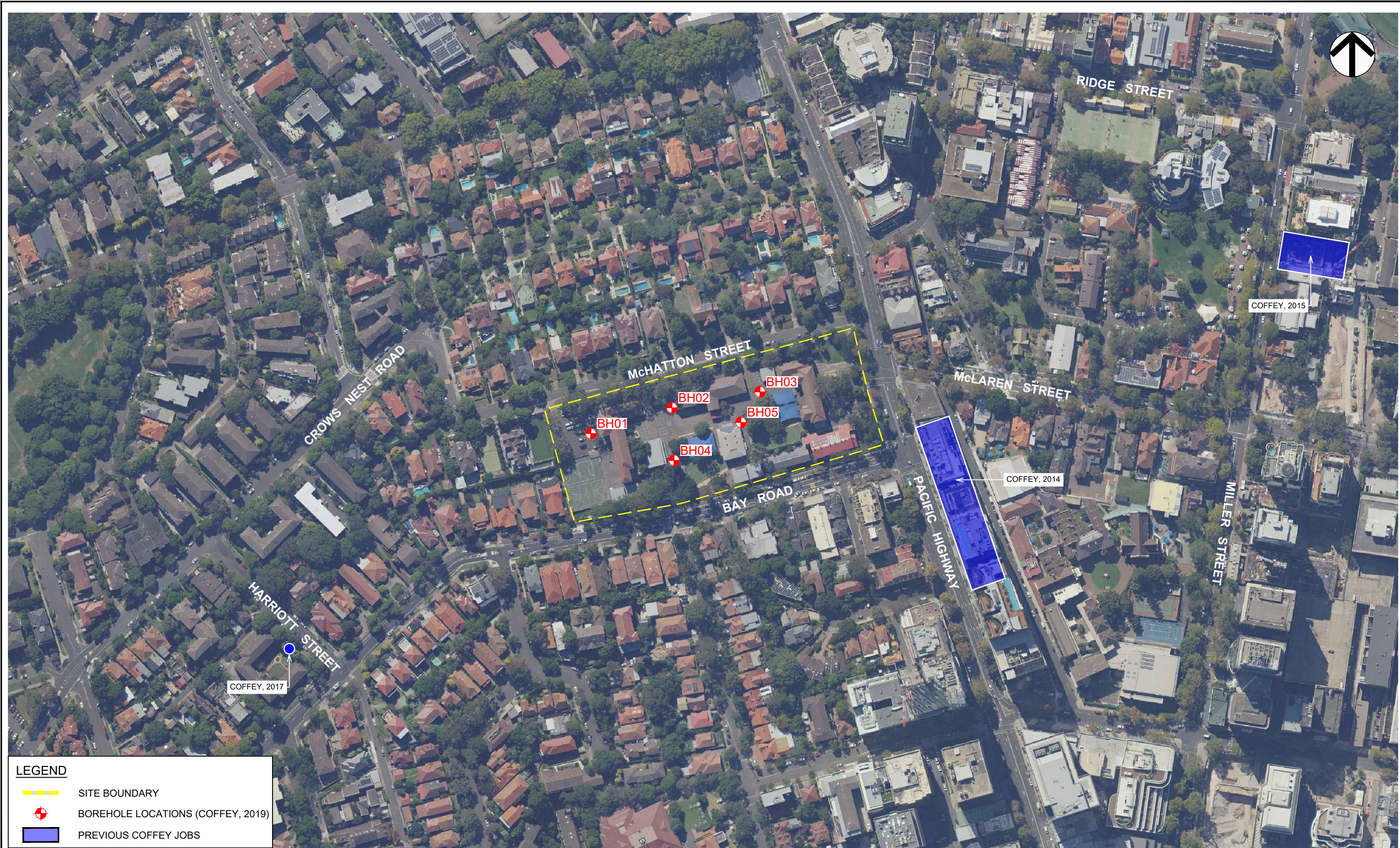




## APPENDIX B: SITE PLAN

---





revision	no.	description	drawn	approved	date	<div><div><div><div>25</div><div>0</div><div>50</div><div>100</div></div><div>Scale (metres) 1:2500</div></div><div>AERIAL IMAGERY COPYRIGHT: ©Land and Property Information (7/04/2018) SOURCED FROM WEBSITE: <a href="http://www.lpi.nsw.gov.au/mapping_and_imagery/lpi_web_services">http://www.lpi.nsw.gov.au/mapping_and_imagery/lpi_web_services</a> LICENSED UNDER CC BY 3.0 AU (<a href="https://creativecommons.org/licenses/by/3.0/au/legalcode">https://creativecommons.org/licenses/by/3.0/au/legalcode</a>)</div></div>	drawn	AM / AW	<div><div><div>Tt</div><div>TETRA TECH</div><div>COFFEY</div></div></div>	client: SCHOOL INFRASTRUCTURE NSW		
	A	ORIGINAL ISSUE	AW	AM	30/07/2021		approved	AM		project: NORTH SYDNEY PUBLIC SCHOOL DESKTOP STUDY PACIFIC HIGHWAY, NORTH SYDNEY, NSW		
							date	28/07/2021		title: SITE PLAN		
							scale	AS SHOWN		project no: 754-SYDGE290593		
							original size	A3		figure no: FIGURE 1		rev: A

PLOT DATE: 28/07/2021 3:50:12 PM DWG FILE: \NTS\776F51.TT\LOCAL\A11 PROJECTS\4 SYDGE290593 NORTH SYD PS\12 CAD\754SYDGE290593.DWG



## APPENDIX C: PREVIOUS INVESTIGATION (2019) BOREHOLE LOGS

---

# Engineering Log - Borehole

client: **NSW Department of Education**

principal: **Coffey Services Australia Pty Ltd**

project: **North Sydney Public School**

location: **North Sydney**

Borehole ID. **BH01**

sheet: 1 of 2

project no. **SYDGE232786**

date started: **02 Oct 2019**


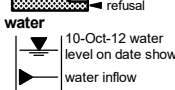
date completed: **02 Oct 2019**

logged by: **RN**

checked by: **RR**

position: Not Specified surface elevation: Not Specified angle from horizontal: 90°  
drill model: Delta Base, Track mounted drilling fluid: hole diameter : 100 mm

drilling information					material substance									
method & support	penetration	water	samples & field tests	RL (m)	depth (m)	graphic log	soil group symbol	material description	moisture condition	consistency / relative density	hand penetrometer (kPa)	structure and additional observations		
<div>AD/T</div> <div>CASING</div>	1	Not Encountered	E		1.0			CONCRETE.	~Wp	S	100	CONCRETE		
	2		E					FILL: ROAD BASE.				S	200	FILL
	3		E					CLAY: medium plasticity, brown, dark brown, with fine to coarse grained sand, trace fine grained, sub-rounded gravel. 0.7 m: becoming medium to high plasticity, pale brown.				F	300	RESIDUAL SOIL
			SPT 5, 13, 10/70mm HB N=R				CL-CH	CLAY: medium - high plasticity, pale brown, grey, trace fine to coarse grained sand.				St - VSt	400	
					2.0			SHALE: pale brown, pale grey, recovered as sandy clay, estimated very low to low strength.				INFERRED WEATHERED BEDROCK		
					3.0			Borehole BH01 continued as cored hole						
					4.0									
					5.0									
					6.0									
					7.0									

<b>method</b> AD auger drilling* AS auger screwing* HA hand auger W washbore  * bit shown by suffix e.g. AD/T B blank bit T TC bit V V bit	<b>support</b> M mud N nil C casing  <b>penetration</b>  <b>water</b> 10-Oct-12 water level on date shown 	<b>samples &amp; field tests</b> B bulk disturbed sample D disturbed sample E environmental sample SS split spoon sample U## undisturbed sample ##mm diameter HP hand penetrometer (kPa) N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone VS vane shear; peak/remoulded (kPa) R refusal HB hammer bouncing	<b>soil group symbol &amp; soil description</b> based on AS 1726:2017  <b>moisture condition</b> D dry M moist W wet Wp plastic limit Wl liquid limit	<b>consistency / relative density</b> VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense
--	--	--	---	--

# Engineering Log - Cored Borehole

client: **NSW Department of Education**  
principal: **Coffey Services Australia Pty Ltd**  
project: **North Sydney Public School**  
location: **North Sydney**

Borehole ID: **BH01**  
sheet: 2 of 2  
project no: **SYDGE232786**  
date started: **02 Oct 2019**  
date completed: **02 Oct 2019**  
logged by: **RN**  
checked by: **RR**

position: Not Specified surface elevation: Not Specified angle from horizontal: 90°  
drill model: Delta Base, Track mounted drilling fluid: hole diameter: 100 mm

drilling information				material substance				rock mass defects			
method & support	water	RL (m)	depth (m)	graphic log	material description ROCK TYPE: grain characteristics, colour, structure, minor components	weathering & alteration	estimated strength & Is50 X = axial O = diametral a = axial d = diametral	samples, field tests & Is(50) (MPa) a = axial d = diametral	core run & RQD	defect spacing (mm)	additional observations and defect descriptions (type, inclination, planarity, roughness, coating, thickness, other)
			1.0								
			2.0								
			3.0		started coring at 2.50m						
			4.0		SHALE: pale brown, pale grey, distinctly laminated at 0° - 10°, with iron staining.	HW - MW					CS, 0°, IR, RO, CN CS, 0°, IR, RO, CN CS, 0°, IR, RO, CN CS, 0°, IR, RO, CN CS, 0°, IR, RO, CN
			5.0		4.0 m: becoming dark grey, pale grey.	MW - SW		a=0.11 d=0.06			SZ, IR, RO, CN CS, 0°, IR, RO, CN CS, 0°, IR, RO, CN
			6.0		Borehole BH01 terminated at 5.89 m Target depth			a=0.10 d=0.12			CS, 0°, IR, RO, CN
			7.0					a=0.35 d=0.44			CS, 0°, IR, RO, CN CS, 0°, IR, RO, CN

<b>method &amp; support</b> AS auger screwing AD auger drilling CB claw or blade bit W washbore RR rock roller NMLCNMLC core (51.9 mm) NQ wireline core (47.6mm) HQ wireline core (63.5mm) PQ wireline core (85.0mm)	<b>support</b> C casing M mud N none <b>water</b> 10/10/12, water level on date shown water inflow complete drilling fluid loss partial drilling fluid loss water pressure test result (lugeons) for depth interval shown	<b>graphic log / core recovery</b> core recovered (graphic symbols indicate material) no core recovered <b>core run &amp; RQD</b> barrel withdrawn RQD = Rock Quality Designation (%)	<b>weathering &amp; alteration*</b> RS residual soil XW extremely weathered HW highly weathered MW moderately weathered SW slightly weathered FR fresh *W replaced with A for alteration <b>strength</b> VL very low L low M medium H high VH very high EH extremely high	<b>defect type</b> PT parting JT joint CU curved SS shear surface SZ shear zone CO contact CS crushed seam SM seam <b>roughness</b> VR very rough RO rough SO smooth POL polished SL slickensided	<b>planarity</b> PL planar CU curved UN undulating ST stepped IR irregular <b>coating</b> CN clean SN stained VN veneer CO coating
---	--	--	---	---	--

CDF 0.9\_06\_LIBRARY.GLB Gfcttl COF PHOTO CORE PHOTO 1 PER PAGE SYDGE232786 (NORTH SYDNEY).GPI <-DrawingFile> 17-10-2019 13:24



BH01 2.50 - 5.89 m

drawn			client:	NSW Department of Education		
approved			project:	North Sydney Public School North Sydney		
date	17-10-2019		title:	<b>CORE PHOTOGRAPH BH01</b>		
scale	N.T.S.		project no:	SYDGE232786	fig no:	<b>FIGURE 1</b>
original size	A4				rev:	



# Engineering Log - Borehole

client: **NSW Department of Education**

principal: **Coffey Services Australia Pty Ltd**

project: **North Sydney Public School**

location: **North Sydney**

Borehole ID. **BH02**

sheet: 1 of 2

project no. **SYDGE232786**



date started: **02 Oct 2019**


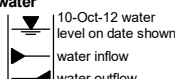
date completed: **02 Oct 2019**

logged by: **RN**

checked by: **RR**

position: Not Specified surface elevation: Not Specified angle from horizontal: 90°  
drill model: Delta Base, Track mounted drilling fluid: hole diameter : 100 mm

drilling information					material substance							
method & support	penetration	water	samples & field tests	RL (m)	depth (m)	graphic log	soil group symbol	material description	moisture condition	consistency / relative density	hand penetrometer (kPa)	structure and additional observations
<div>AD/T</div> <div>CASING</div>	1	Not Encountered	E				CL-CI	ASPHALT.	<Wp	S	100 200 300 400	ASPHALT
	2		D + E	CI	FILL: ROAD BASE.		FILL					
	3		SPT 19, HB N=R	1.0	CI		CLAY: low - medium plasticity, brown, grey, with fine to coarse grained sand, trace fine to medium sub-angular to sub-rounded gravel. CLAY: medium plasticity, brown, pale brown, with fine to coarse grained sand, trace fine to medium sub-rounded gravel.	F	RESIDUAL SOIL			
				2.0	CI		CLAY: medium plasticity, brown, pale brown, grey, trace fine to coarse grained sand.	~Wp St - VSt				
			SPT 11, 18/110mm, HB N=R					SHALE: grey to dark grey, recovered as sandy clay, estimated very low to low strength.				INFERRED WEATHERED BEDROCK
					3.0			Borehole BH02 continued as cored hole				
					4.0							
					5.0							
					6.0							
					7.0							

<b>method</b> AD auger drilling* AS auger screwing* HA hand auger W washbore  * bit shown by suffix e.g. AD/T B blank bit T TC bit V V bit	<b>support</b> M mud N nil C casing  <b>penetration</b>  <b>water</b> 	<b>samples &amp; field tests</b> B bulk disturbed sample D disturbed sample E environmental sample SS split spoon sample U## undisturbed sample ##mm diameter HP hand penetrometer (kPa) N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone VS vane shear; peak/remoulded (kPa) R refusal HB hammer bouncing	<b>soil group symbol &amp; soil description</b> based on AS 1726:2017  <b>moisture condition</b> D dry M moist W wet Wp plastic limit Wl liquid limit	<b>consistency / relative density</b> VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense
--	---	--	---	--

# Engineering Log - Cored Borehole

client: **NSW Department of Education**  
principal: **Coffey Services Australia Pty Ltd**  
project: **North Sydney Public School**  
location: **North Sydney**

Borehole ID: **BH02**  
sheet: 2 of 2  
project no: **SYDGE232786**  
date started: **02 Oct 2019**  
date completed: **02 Oct 2019**  
logged by: **RN**  
checked by: **RR**

drilling information		material substance		rock mass defects						
method & support	water	RL (m)	depth (m)	material description ROCK TYPE: grain characteristics, colour, structure, minor components	weathering & alteration	estimated strength & Is(50) X = axial O = diametral a = axial d = diametral	samples, field tests & Is(50) (MPa)	core run & RQD	defect spacing (mm)	additional observations and defect descriptions (type, inclination, planarity, roughness, coating, thickness, other)
			1.0							
			2.0							
			3.0	started coring at 2.80m						
			4.0	SHALE: grey to dark grey, distinctly laminated at 0° - 10°, with some iron staining.	MW - SW		a=0.03 d=0.01			
			5.0		SW		a=0.34 d=0.07			CS, 0°, IR, RO, CN
			6.0		FR		a=0.59 d=1.02			CS, 0°, IR, RO, CN JT, 45°, PL, SO, CN JT, 30 - 40°, PL, SO, CN JT, 20 - 25°, PL, SO, CN
			7.0	Borehole BH02 terminated at 5.65 m Target depth						

**method & support**

AS auger screwing  
AD auger drilling  
CB claw or blade bit  
W washbore  
RR rock roller  
NMLCNMLC core (51.9 mm)  
NQ wireline core (47.6mm)  
HQ wireline core (63.5mm)  
PQ wireline core (85.0mm)

**support**

C casing M mud N none

**water**

10/10/12, water level on date shown  
water inflow  
complete drilling fluid loss  
partial drilling fluid loss

water pressure test result (lugeons) for depth interval shown

**graphic log / core recovery**

core recovered (graphic symbols indicate material)

no core recovered

**core run & RQD**

barrel withdrawn

RQD = Rock Quality Designation (%)

**weathering & alteration\***

RS residual soil  
XW extremely weathered  
HW highly weathered  
MW moderately weathered  
SW slightly weathered  
FR fresh  
\*W replaced with A for alteration

**strength**

VL very low  
L low  
M medium  
H high  
VH very high  
EH extremely high

**defect type**

PT parting  
JT joint  
SS shear surface  
SZ shear zone  
CO contact  
CS crushed seam  
SM seam

**roughness**

VR very rough  
RO rough  
SO smooth  
POL polished  
SL slickensided

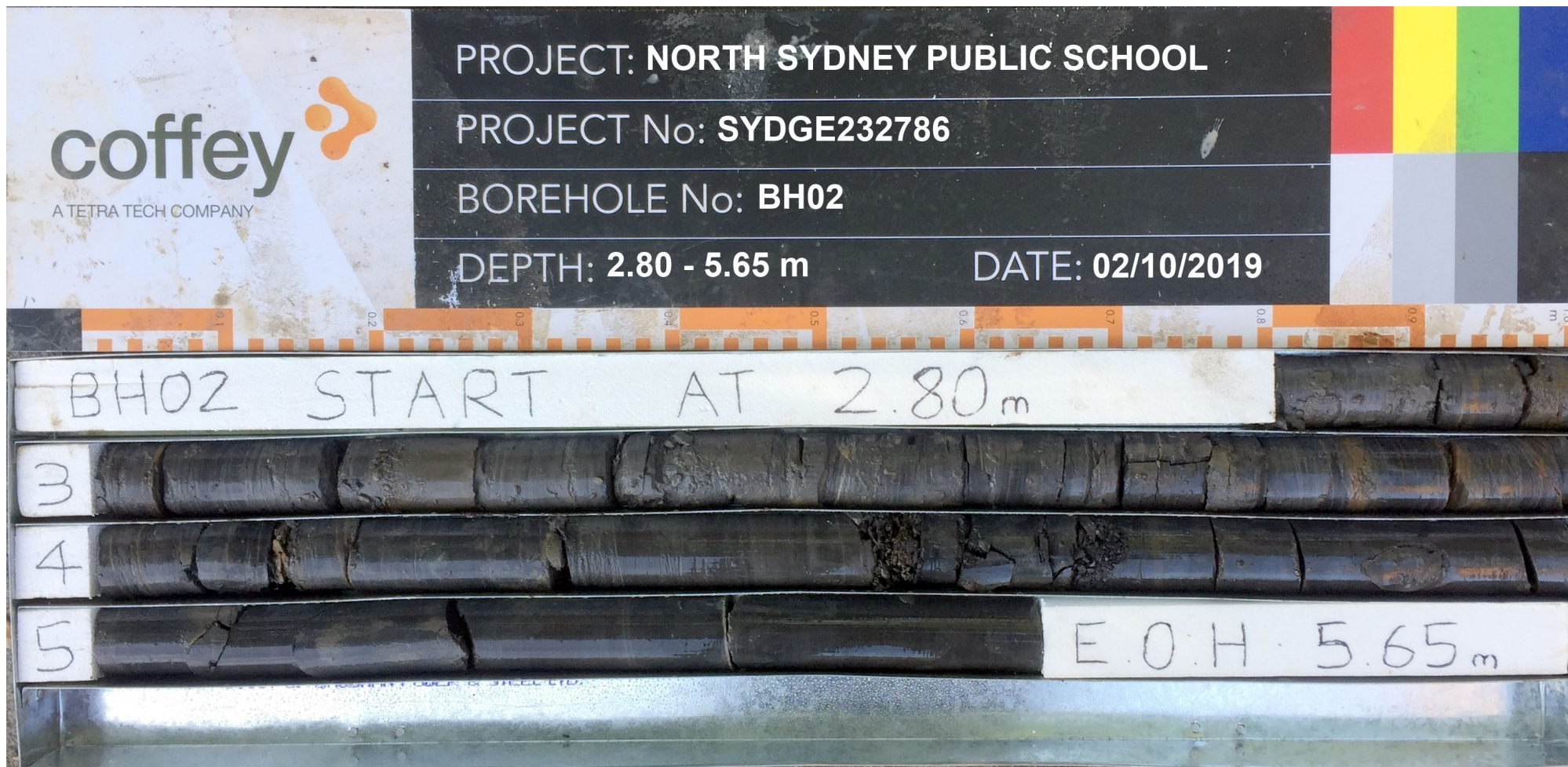
**planarity**

PL planar  
CU curved  
UN undulating  
ST stepped  
IR irregular


**coating**

CN clean  
SN stained  
VN veneer  
CO coating

CDF 0.9.06\_LIBRARY.GLB Gfcttl COF PHOTO CORE PHOTO 1 PER PAGE SYDGE232786 (NORTH SYDNEY).GPI <-DrawingFile> 17-10-2019 13:27



BH02 2.80 - 5.65 m

drawn		 A TETRA TECH COMPANY	client:	NSW Department of Education		
approved			project:	North Sydney Public School North Sydney		
date	17-10-2019		title:	<b>CORE PHOTOGRAPH BH02</b>		
scale	N.T.S.		project no:	SYDGE232786	fig no:	<b>FIGURE 1</b>
original size	A4				rev:	



# Engineering Log - Borehole

client: **NSW Department of Education**

principal: **Coffey Services Australia Pty Ltd**

project: **North Sydney Public School**

location: **North Sydney**

Borehole ID. **BH03**

sheet: 1 of 2

project no. **SYDGE232786**


date started: **04 Oct 2019**

date completed: **04 Oct 2019**

logged by: **RN**

checked by: **RR**

position: Not Specified surface elevation: Not Specified angle from horizontal: 90°  
drill model: Delta Base, Track mounted drilling fluid: hole diameter : 100 mm

drilling information					material substance								
method & support	penetration	water	samples & field tests	RL (m)	depth (m)	graphic log	soil group symbol	material description	moisture condition	consistency / relative density	hand penetrometer (kPa)	structure and additional observations	
<div><div>AD/T</div><div>CASING</div></div>	1	Not Encountered	E				CL	ASPHALT.	<WI	S	100 200 300 400	ASPHALT	
	2		D + E		CL-CI		FILL: ROAD BASE.	~WI				S - F	FILL
	3		E	1.0	CI		CLAY: low plasticity, brown, with fine to coarse grained sand, trace fine to medium, sub-angular to sub-rounded gravel. Sandy CLAY: low - medium plasticity, brown, red, grey, with fine to medium grained gravel; sand is fine to medium grained. CLAY: medium plasticity, brown, grey, trace fine to coarse sand.	F				RESIDUAL SOIL	
			SPT 4, 5, 15 N=20	2.0				St					
			SPT 14, 5/120mm, HB N=R		3.0			SHALE: grey, dark grey, recovered as sandy clay, estimated very low to low strength. Borehole BH03 continued as cored hole				INFERRED WEATHERED BEDROCK	
					4.0								
					5.0								
					6.0								
					7.0								

<b>method</b> AD auger drilling* AS auger screwing* HA hand auger W washbore  * bit shown by suffix e.g. AD/T B blank bit T TC bit V V bit	<b>support</b> M mud N nil C casing  <b>penetration</b>  <b>water</b> 10-Oct-12 water level on date shown water inflow water outflow	<b>samples &amp; field tests</b> B bulk disturbed sample D disturbed sample E environmental sample SS split spoon sample U## undisturbed sample ##mm diameter HP hand penetrometer (kPa) N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone VS vane shear; peak/remoulded (kPa) R refusal HB hammer bouncing	<b>soil group symbol &amp; soil description</b> based on AS 1726:2017  <b>moisture condition</b> D dry M moist W wet Wp plastic limit WI liquid limit	<b>consistency / relative density</b> VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense
--	---	--	---	--

# Engineering Log - Cored Borehole

client: **NSW Department of Education**  
principal: **Coffey Services Australia Pty Ltd**  
project: **North Sydney Public School**  
location: **North Sydney**

Borehole ID: **BH03**  
sheet: 2 of 2  
project no: **SYDGE232786**  
date started: **04 Oct 2019**  
date completed: **04 Oct 2019**  
logged by: **RN**  
checked by: **RR**

position: Not Specified surface elevation: Not Specified angle from horizontal: 90°  
drill model: Delta Base, Track mounted drilling fluid: hole diameter : 100 mm

drilling information			material substance			rock mass defects		
method & support	water	RL (m)	depth (m)	graphic log	material description ROCK TYPE: grain characteristics, colour, structure, minor components	weathering & alteration	estimated strength & Is(50) X = axial O = diametral a = axial d = diametral	samples, field tests & Is(50) (MPa) a = axial d = diametral
			1.0					
			2.0					
			3.0		started coring at 2.80m			
			4.0		SHALE: grey, dark grey, indistinctly laminated at 0° - 10°.	MW - SW		
			5.0			SW		
			6.0		Borehole BH03 terminated at 5.78 m Target depth			
			7.0					


<b>method &amp; support</b> AS auger screwing AD auger drilling CB claw or blade bit W washbore RR rock roller NMLCNMLC core (51.9 mm) NQ wireline core (47.6mm) HQ wireline core (63.5mm) PQ wireline core (85.0mm)	<b>support</b> C casing M mud N none <b>water</b> 10/10/12, water level on date shown water inflow complete drilling fluid loss partial drilling fluid loss water pressure test result (lugeons) for depth interval shown	<b>graphic log / core recovery</b> core recovered (graphic symbols indicate material) no core recovered <b>core run &amp; RQD</b> barrel withdrawn RQD = Rock Quality Designation (%)	<b>weathering &amp; alteration*</b> RS residual soil XW extremely weathered HW highly weathered MW moderately weathered SW slightly weathered FR fresh *W replaced with A for alteration <b>strength</b> VL very low L low M medium H high VH very high EH extremely high	<b>defect type</b> PT parting JT joint SS shear surface SZ shear zone CO contact CS crushed seam SM seam <b>roughness</b> VR very rough RO rough SO smooth POL polished SL slickensided	<b>planarity</b> PL planar CU curved UN undulating ST stepped IR irregular <b>coating</b> CN clean SN stained VN veneer CO coating
---	--	--	---	--	--



CDF 0.9.06\_LIBRARY.GLB Gfcttl COF PHOTO CORE PHOTO 1 PER PAGE SYDGE232786 (NORTH SYDNEY).GPI <<DrawingFile>> 17-10-2019 13:30



BH03 2.80 - 5.78 m

drawn			client:	NSW Department of Education		
approved			project:	North Sydney Public School North Sydney		
date	17-10-2019		title:	<b>CORE PHOTOGRAPH BH03</b>		
scale	N.T.S.		project no:	SYDGE232786	fig no:	<b>FIGURE 1</b>
original size	A4				rev:	


# Engineering Log - Borehole

client: **NSW Department of Education**  
principal: **Coffey Services Australia Pty Ltd**  
project: **North Sydney Public School**  
location: **North Sydney**

Borehole ID. **BH04**  
sheet: 1 of 2  
project no. **SYDGE232786**  
date started: **03 Oct 2019**  
date completed: **03 Oct 2019**  
logged by: **RN**  
checked by: **RR**

position: Not Specified surface elevation: Not Specified angle from horizontal: 90°  
drill model: Delta Base, Track mounted drilling fluid: hole diameter : 100 mm

drilling information					material substance				
method & support	penetration	water	samples & field tests	RL (m)	depth (m)	graphic log	soil group symbol	material description	structure and additional observations
AD/T Casing Not Encountered	1		E		1.0		CL	ASPHALT. FILL: ROAD BASE.	ASPHALT
	2		D + E				CL-CI	CLAY: low plasticity, brown, grey, with fine to coarse grained sand, trace fine to medium, sub-angular to sub-rounded gravel.	FILL
	3		E				CI	CLAY: low - medium plasticity, brown, with fine to coarse grained sand, trace fine to medium sub-rounded gravel.	RESIDUAL SOIL
			SPT 4, 4, 7 N=11		2.0		CI	CLAY: medium plasticity, brown, grey, trace fine sand.	
			SPT 6 HB N=R		3.0			SHALE: grey, pale grey, recovered as sandy clay, estimated very low to low strength.	INFERRED WEATHERED BEDROCK
					4.0			Borehole BH04 continued as cored hole	
					5.0				
					6.0				
					7.0				

<b>method</b> AD auger drilling* AS auger screwing* HA hand auger W washbore  * bit shown by suffix e.g. AD/T B blank bit T TC bit V V bit	<b>support</b> M mud N nil C casing  <b>penetration</b>  no resistance ranging to refusal  <b>water</b> 10-Oct-12 water level on date shown water inflow water outflow	<b>samples &amp; field tests</b> B bulk disturbed sample D disturbed sample E environmental sample SS split spoon sample U## undisturbed sample ##mm diameter HP hand penetrometer (kPa) N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone VS vane shear; peak/remoulded (kPa) R refusal HB hammer bouncing	<b>soil group symbol &amp; soil description</b> based on AS 1726:2017  <b>moisture condition</b> D dry M moist W wet Wp plastic limit Wl liquid limit	<b>consistency / relative density</b> VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense
--	--	--	---	--

# Engineering Log - Cored Borehole

client: **NSW Department of Education**  
principal: **Coffey Services Australia Pty Ltd**  
project: **North Sydney Public School**  
location: **North Sydney**

Borehole ID: **BH04**  
sheet: 2 of 2  
project no: **SYDGE232786**  
date started: **03 Oct 2019**  
date completed: **03 Oct 2019**  
logged by: **RN**  
checked by: **RR**

position: Not Specified surface elevation: Not Specified angle from horizontal: 90°  
drill model: Delta Base, Track mounted drilling fluid: hole diameter: 100 mm

drilling information				material substance				rock mass defects			
method & support	water	RL (m)	depth (m)	graphic log	material description ROCK TYPE: grain characteristics, colour, structure, minor components	weathering & alteration	estimated strength & Is50 X = axial; O = diametral a = axial; d = diametral	samples, field tests & Is(50) (MPa)	core run & RQD	defect spacing (mm)	additional observations and defect descriptions (type, inclination, planarity, roughness, coating, thickness, other)
							VL J M H VH EH			30 100 300 1000 3000	particular general
			1.0								
			2.0								
			3.0		started coring at 2.80m						
			4.0		SHALE: grey, pale grey, indistinctly laminated at 0° - 10°.	HW - MW  SW		a=0.01 d=0.00  a=0.02 d=0.03			CS, 0°, IR, RO, CN  CS, 0°, IR, RO, CN
			5.0		Borehole BH04 terminated at 4.92 m Target depth						
			6.0								
			7.0								


<b>method &amp; support</b> AS auger screwing AD auger drilling CB claw or blade bit W washbore RR rock roller NMLCNMLC core (51.9 mm) NQ wireline core (47.6mm) HQ wireline core (63.5mm) PQ wireline core (85.0mm)	<b>support</b> C casing M mud N none <b>water</b> 10/10/12, water level on date shown water inflow complete drilling fluid loss partial drilling fluid loss water pressure test result (lugeons) for depth interval shown	<b>graphic log / core recovery</b> core recovered (graphic symbols indicate material) no core recovered <b>core run &amp; RQD</b> barrel withdrawn RQD = Rock Quality Designation (%)	<b>weathering &amp; alteration*</b> RS residual soil XW extremely weathered HW highly weathered MW moderately weathered SW slightly weathered FR fresh *W replaced with A for alteration <b>strength</b> VL very low L low M medium H high VH very high EH extremely high	<b>defect type</b> PT parting JT joint SS shear surface SZ shear zone CO contact CS crushed seam SM seam <b>roughness</b> VR very rough RO rough SO smooth POL polished SL slickensided	<b>planarity</b> PL planar CU curved UN undulating ST stepped IR irregular <b>coating</b> CN clean SN stained VN veneer CO coating
---	--	--	---	--	--



CDF 0.9.06\_LIBRARY.GLB Gfcttl COF PHOTO CORE PHOTO 1 PER PAGE SYDGE232786 (NORTH SYDNEY).GPI <-DrawingFiles> 17-10-2019 13:32



BH04 2.80 - 4.92 m

drawn		 A TETRA TECH COMPANY	client:	NSW Department of Education		
approved			project:	North Sydney Public School North Sydney		
date	17-10-2019		title:	<b>CORE PHOTOGRAPH BH04</b>		
scale	N.T.S.		project no:	SYDGE232786	fig no:	<b>FIGURE 1</b>
original size	A4				rev:	

# Engineering Log - Borehole

client: **NSW Department of Education**

principal: **Coffey Services Australia Pty Ltd**

project: **North Sydney Public School**

location: **North Sydney**

Borehole ID. **BH05**

sheet: 1 of 2

project no. **SYDGE232786**

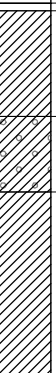
date started: **03 Oct 2019**


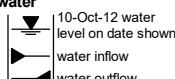
date completed: **03 Oct 2019**

logged by: **RN**

checked by: **RR**

position: Not Specified surface elevation: Not Specified angle from horizontal: 90°  
drill model: Delta Base, Track mounted drilling fluid: hole diameter : 100 mm

drilling information					material substance							
method & support	penetration	water	samples & field tests	RL (m)	depth (m)	graphic log	soil group symbol	material description	moisture condition	consistency / relative density	hand penetrometer (kPa)	structure and additional observations
<div><div>AD/T</div><div>CASING</div></div>	1	Not Encountered	E				CL	ASPHALT.	<Wp	S	<div><div>100</div><div>200</div><div>300</div><div>400</div></div>	ASPHALT
	2							FILL: ROAD BASE.				FILL
	3							CLAY: low plasticity, brown, with fine to coarse grained sand, trace fine to coarse, sub-angular to sub-rounded gravel.				RESIDUAL SOIL
								CLAY: medium plasticity, brown, pale brown, with fine to coarse grained sand, trace fine grained, sub-rounded gravel.				
								CLAY: medium plasticity, pale brown, grey.				
			SPT 5, 7, 17 N=24		1.0		CI		~Wp	F		
					2.0		CI					
			SPT 12, 14/200mm HB N=R		3.0			SHALE: grey, dark grey, recovered as sandy clay, estimated very low to low strength.		St - VSt		INFERRED WEATHERED BEDROCK
								Borehole BH05 continued as cored hole				
					4.0							
					5.0							
					6.0							
					7.0							

<b>method</b> AD auger drilling* AS auger screwing* HA hand auger W washbore  * bit shown by suffix e.g. AD/T B blank bit T TC bit V V bit	<b>support</b> M mud N nil C casing  <b>penetration</b>  <b>water</b> 	<b>samples &amp; field tests</b> B bulk disturbed sample D disturbed sample E environmental sample SS split spoon sample U## undisturbed sample ##mm diameter HP hand penetrometer (kPa) N standard penetration test (SPT) N* SPT - sample recovered Nc SPT with solid cone VS vane shear; peak/remoulded (kPa) R refusal HB hammer bouncing	<b>soil group symbol &amp; soil description</b> based on AS 1726:2017  <b>moisture condition</b> D dry M moist W wet Wp plastic limit Wl liquid limit	<b>consistency / relative density</b> VS very soft S soft F firm St stiff VSt very stiff H hard Fb friable VL very loose L loose MD medium dense D dense VD very dense
--	---	--	---	--



# Engineering Log - Cored Borehole

client: **NSW Department of Education**  
principal: **Coffey Services Australia Pty Ltd**  
project: **North Sydney Public School**  
location: **North Sydney**

Borehole ID: **BH05**  
sheet: 2 of 2  
project no: **SYDGE232786**  
date started: **03 Oct 2019**  
date completed: **03 Oct 2019**  
logged by: **RN**  
checked by: **RR**

position: Not Specified surface elevation: Not Specified angle from horizontal: 90°  
drill model: Delta Base, Track mounted drilling fluid: hole diameter : 100 mm


drilling information				material substance				rock mass defects			
method & support	water	RL (m)	depth (m)	graphic log	material description ROCK TYPE: grain characteristics, colour, structure, minor components	weathering & alteration	estimated strength & Is50 X = axial O = diametral a = axial d = diametral	samples, field tests & Is(50) (MPa)	core run & RQD	defect spacing (mm)	additional observations and defect descriptions (type, inclination, planarity, roughness, coating, thickness, other)
			1.0								
			2.0								
			3.0		started coring at 2.80m						
			4.0		SHALE: grey, dark grey, indistinctly laminated at 0° - 10°.	MW SW		a=0.21 d=0.31			CS, 0°, IR, RO, CN CS, 0°, IR, RO, CN CS, 0°, IR, RO, CN
			5.0		Borehole BH05 terminated at 5.00 m Target depth			a=0.03 d=0.05			PT, 25°, PL, SO, CN CS, 0°, IR, RO, CN JT, 35°, PL, SO, CN
			6.0								
			7.0								

<b>method &amp; support</b> AS auger screwing AD auger drilling CB claw or blade bit W washbore RR rock roller NMLCNMLC core (51.9 mm) NQ wireline core (47.6mm) HQ wireline core (63.5mm) PQ wireline core (85.0mm)	<b>support</b> C casing M mud N none <b>water</b> 10/10/12, water level on date shown water inflow complete drilling fluid loss partial drilling fluid loss water pressure test result (lugeons) for depth interval shown	<b>graphic log / core recovery</b> core recovered (graphic symbols indicate material) no core recovered <b>core run &amp; RQD</b> barrel withdrawn RQD = Rock Quality Designation (%)	<b>weathering &amp; alteration*</b> RS residual soil XW extremely weathered HW highly weathered MW moderately weathered SW slightly weathered FR fresh *W replaced with A for alteration <b>strength</b> VL very low L low M medium H high VH very high EH extremely high	<b>defect type</b> PT parting JT joint SS shear surface SZ shear zone CO contact CS crushed seam SM seam <b>roughness</b> VR very rough RO rough SO smooth POL polished SL slickensided	<b>planarity</b> PL planar CU curved UN undulating ST stepped IR irregular <b>coating</b> CN clean SN stained VN veneer CO coating
---	--	--	---	--	--

CDF 0 9 06 LIBRARY.GLB Gfcttl COF PHOTO CORE PHOTO 1 PER PAGE SYDGE232786 (NORTH SYDNEY).GPI <-DrawingFiles> 17-10-2019 13:35



BH05 2.80 - 5.00 m

drawn		 A TETRA TECH COMPANY	client:	NSW Department of Education		
approved			project:	North Sydney Public School North Sydney		
date	17-10-2019		title:	<b>CORE PHOTOGRAPH BH05</b>		
scale	N.T.S.		project no:	SYDGE232786	fig no:	<b>FIGURE 1</b>
original size	A4				rev:	

This page has been left intentionally blank