



Deicorp Projects (Tallawong Station) Pty Ltd

Traffic and Parking Impact Assessment Report

Proposed Mixed Use Development

Tallawong Station Precinct South

June 2020

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|-------------|----------|
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Planning Secretary's Environmental Assessment Requirements
Section 4.12(8) of the *Environmental Planning and Assessment Act 1979*
Schedule 2 of the Environmental Planning and Assessment Regulation 2000

| | |
|---------------------------|---|
| Application Number | SSD-10425 |
| Project Name | Stage 2 – Detailed Development Application - Tallawong Station Precinct South |
| Location | 1-15 and 2-12 Conferta Avenue, Rouse Hill within Blacktown City Council |
| Applicant | DEICORP PROJECTS (TALLAWONG STATION) PTY LTD |
| Date of Issue | 13/02/2020 |

| Requirement | Relevant Report Section |
|---|--|
| Item 6 - Traffic, Parking and Access (operation) | |
| The EIS must include a traffic, parking and access assessment providing: | |
| Details of a car parking strategy which includes provision of car and bicycle parking for residential and non-residential uses, and consideration of sharing use of the car spaces between land uses. | Section 3.6 (bicycle parking) and Section 4 (car parking). |
| Details on the likely estimated future mode share for the various users (residents, visitors, etc) accessing the proposed development measures to encourage users of the development to make sustainable travel choices, including a green travel plan, walking, cycling, public transport and car sharing, adequate provision of bicycle parking and end of trip facilities and the minimisation of private car trips. | Green Travel Plan prepared by Barker Ryan Stewart. |
| Measures to include street tree planting. | Section 3.4 |
| Impacts of the proposed development on the operation of existing and future transport networks, in particular bus corridors, including the public transport capacity and its ability to accommodate the forecast number of trips to and from the development. | Section 3.5 and the Green Travel Plan. |
| Modelling and analysis of pedestrian and cyclist access to the proposed development in consultation with TfNSW, together with an assessment of pedestrian and cyclist safety and consideration of the relationship with design and operation of the station. | Section 3.6 and the Green Travel Plan. |
| Detailed assessment of the existing and future performance of key intersections providing access to the site, supported by appropriate modelling and analysis to the satisfaction of TfNSW. | Section 5 |
| Measures to mitigate impacts of the proposed development on the capacity and operation of existing and future traffic, public transport, pedestrian and bicycle networks, including any required upgrades. | Sections 3.4, 3.5, 3.6 and 5. |

| | |
|--|--|
| Details of existing and proposed vehicle access arrangements, including parking, pedestrian safety management, loading dock and servicing management with consideration of precinct wide shared loading docks and/or remote or off-site loading zone hub facilities, ensuring all servicing and loading occurs on-site and does not rely on kerbside controls. | Sections 3.2, 3.3, 4.7 and Appendix D. |
| An assessment of pedestrian and cyclist safety with consideration of the relationship with design, access and operation of the station. | Section 3.6 |

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List of Abbreviations

Abbreviations

| | |
|------------------------|--|
| DCP | Blacktown Development Control Plan 2015 |
| LEP | Blacktown Local Environmental Plan 2015 |
| GFA | Gross Floor Area |
| AS/NZS2890.1 | Australian Standards, 'AS/NZS 2890.1:2004 Off-Street Car Parking' |
| AS2890.2..... | Australian Standards, 'AS 2890.2:2018 Off-Street Commercial Vehicle Facilities' |
| AS/NZS2890.6..... | Australian Standards, 'AS/NZS 2890.6:2002 Off-Street Parking for People with Disabilities' |
| RMS..... | Roads and Maritime Services |
| RMS Guide | RMS Guide to Traffic Generating Developments, Version 2.2, October 2002 |
| RMS Guide Update | RMS Guide to Traffic Generating Developments, Updated Traffic Surveys |

1 Introduction

Barker Ryan Stewart have been engaged by Deicorp Projects (Tallawong Station) Pty Ltd to prepare a Traffic and Parking Impact Assessment in accordance with the Planning Secretary's Environmental Assessment Requirements (SEARS), Blacktown Council's DCP and LEP and the Roads and Maritime Services (RMS) 'Guide to Traffic Generating Developments' to accompany a proposal for a mixed-use development comprised of residential units and retail and commercial uses.

The purpose of this report is to assess and address traffic, access, car parking and pedestrian and cycling impacts generated by the proposed development and recommend any mitigation measures where required. This can be briefly outlined as follows:

- The expected traffic generation to/from the proposed development.
- The impact of the proposed development on the road network.
- Intersection analysis based on traffic counts.
- Car parking strategy
- Vehicle parking provisions.
- Access design requirements.
- Vehicular requirements for delivery and waste collection.
- Safety of pedestrians and cyclists and the impact on existing pedestrian and cycling networks.
- Availability of public transport.

The assessment included a review of all known available traffic and transport documentation associated with the Tallawong South Precinct including but not limited to the following:

- Traffic and Transport Study prepared by SCT Consulting,
- The Sydney Metro Tallawong Station Precinct South Submissions Report, and
- The Technical Memorandum – Tallawong Station SSDA – Post Exhibition Responses to Traffic and Parking Comments prepared by SCT Consulting.

This Traffic and Parking Impact Assessment concludes that the subject site is suitable for the proposed development in relation to traffic impact, car parking provision, safety of and provision for pedestrians and cyclists.

2 Existing Conditions

2.1 Site Location

The site is located at 1-15 and 2-12 Conferta Avenue, Rouse Hill (Lots 293 and 294 DP 1213279). The two sites are currently unoccupied and have been cleared of vegetation. The sites are bisected by Conferta Avenue and bounded by Themeda Avenue and Tallawong Metro Station to the north, Cudgegong Road to the east, Schofields Road to the south, and the Tallawong Station commuter car park to the west.



Figure 1: Site Location (source: NearMap October 2019)

2.2 Existing Road Conditions

Intersections

The following four signalised intersections with turn bays are located at the corners of the Tallawong Station Precinct South.

- Cudgegong Road/ Schofields Road
- Tallawong Road / Schofields Road
- Themeda Avenue/ Cudgegong Road
- Themeda Avenue/ Tallawong Road

There are also two restricted left in left out intersections at Confeta Avenue and Tallawong Road and Confeta Avenue and Cudgegong Road.

Aristida Avenue has stop sign posted intersections with Confeta Avenue (T intersection) and Themeda Avenue.

Schofields Road

Schofields Road is an urban arterial road that provides a major connection between Rouse Hill town centre to the east and Schofields suburb to the west. It generally consists of two lanes in each direction (3.1m wide) separated by a central concrete median with additional turning lanes at intersections to increase turning capacity. Shared pedestrian / cyclist paths are provided on each side of the road. The posted speed limit is 70km/hr. Intermittent bus lanes are included within intersections.

Cudgegong Road

Cudgegong Road is a local road that provides access from Schofields Road at the southern end to Guntawong Road at the northern end of the road. It generally has two northbound lanes (3.1m wide each) from the intersection with Schofields Road and the rail overpass and one southbound lane (3.1m wide) from Themeda Avenue to Confeta Avenue and two southbound lanes from Confeta Avenue to Schofields Road at a major three-way signalised intersection. A shared pedestrian / cyclist path is provided along the western side of the road. The posted speed limit on Cudgegong Road is 60km/hr.

Tallawong Road

Tallawong Road is a local road that provides a connection between Schofields Road at the south and Guntawong Road to the north. Generally between Themeda Avenue and Schofields Road, it has three 3.1m wide lane northbound lanes and two 3.1m wide Southbound lanes. Shared pedestrian / cyclist paths are provided on each side of the road. The posted speed limit is 60km/hr.

Confeta Avenue

Confeta Avenue is a local road running parallel with Schofields Road along the northern edge of Lot 293 and the southern edge of Lot 294. It connects Cudgegong Road to the east and Tallawong Road to the west and also provides access to the southern section of the commuter carpark. It has a single 3.8m wide lane of traffic in each direction. Each carriageway has a parking lane delineated by an edge line. The on-street parking has a 2-Hour time limit outside of morning and afternoon peak periods. The posted speed limit is 50km/hr.

Themeda Avenue

Themeda Avenue is a two-way local road consisting of 3.8m wide single lanes with 2.3m on-street parking on both sides of the road. The on-street parking has a 2-Hour time limit outside of morning and afternoon peak periods. It is adjacent to Tallawong metro Station and connects at signalised intersections at Cudgegong Road to the east and Tallawong Road to the west. The posted speed limit is 50km/hr.

Aristida Street

Aristida Street is a two-way local road consisting of 3.8m wide lanes. It connects Implexa Parade to the north and Confeta Avenue to the south. It is subject to a speed limit of 50km/hr.

2.3 Traffic Flows and Volumes

Traffic counts were undertaken during the morning and afternoon peak periods to gauge the performance of the current road system. The traffic counts were undertaken between the hours of 7am – 9am and 4pm – 6pm on Thursday 12th December 2019 at the following intersections:

- Cudgegong Road/ Schofields Road
- Tallawong Road / Schofields Road

Traffic counts were also undertaken between the hours of 7am – 9am and 4pm – 6pm on Thursday 12th March 2020 at the following intersections:

- Themeda Avenue/ Cudgegong Road
- Themeda Avenue/ Tallawong Road

The location of the intersections where the traffic counts were undertaken are highlighted in Figure 20 in Section 5 of this report.

The peak hour periods, traffic volumes and layouts for each of these intersections are summarised below in **Figures 2 to 5**, the full results of the traffic counts are available in **Appendix E**.

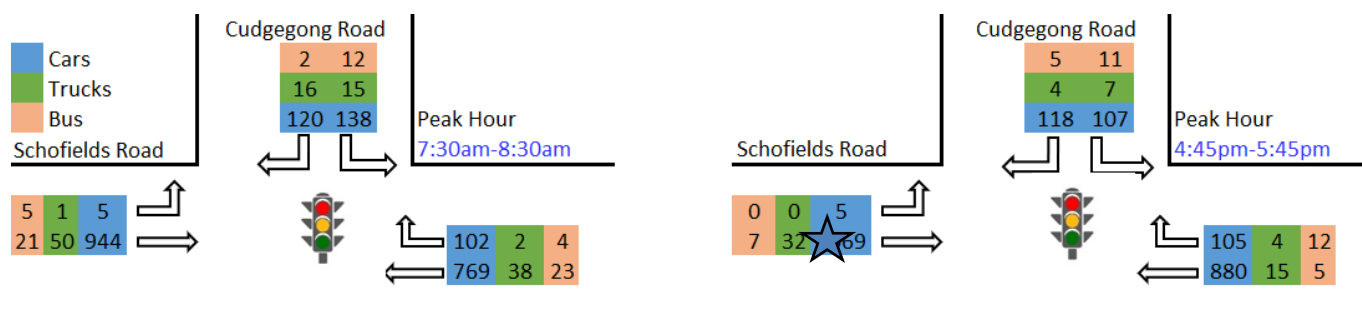


Figure 2: Cudgegong Road / Schofields Road Intersection Count

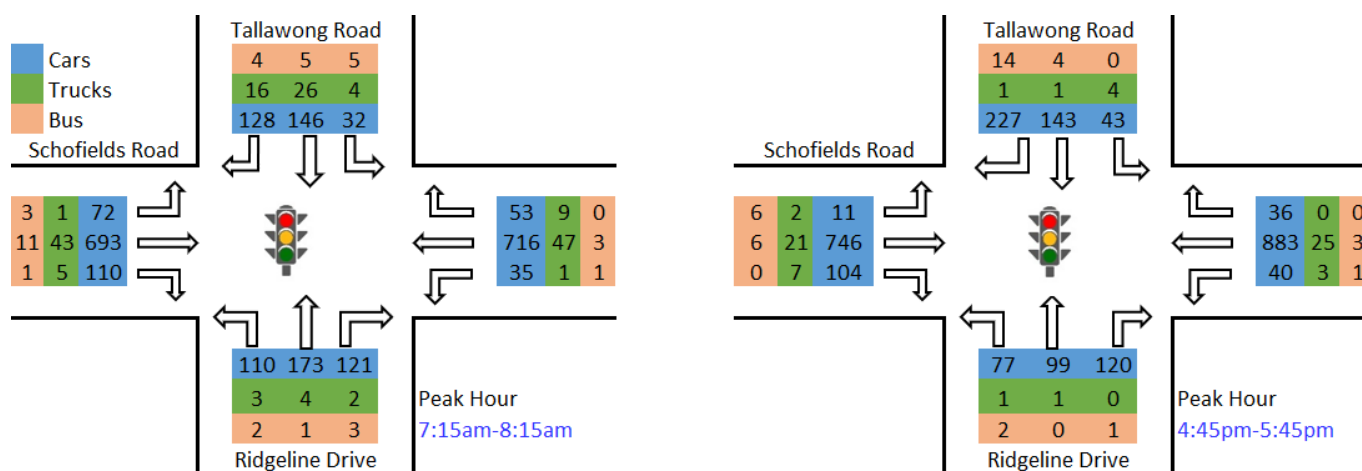


Figure 3: Tallawong Road / Schofields Road Intersection Count

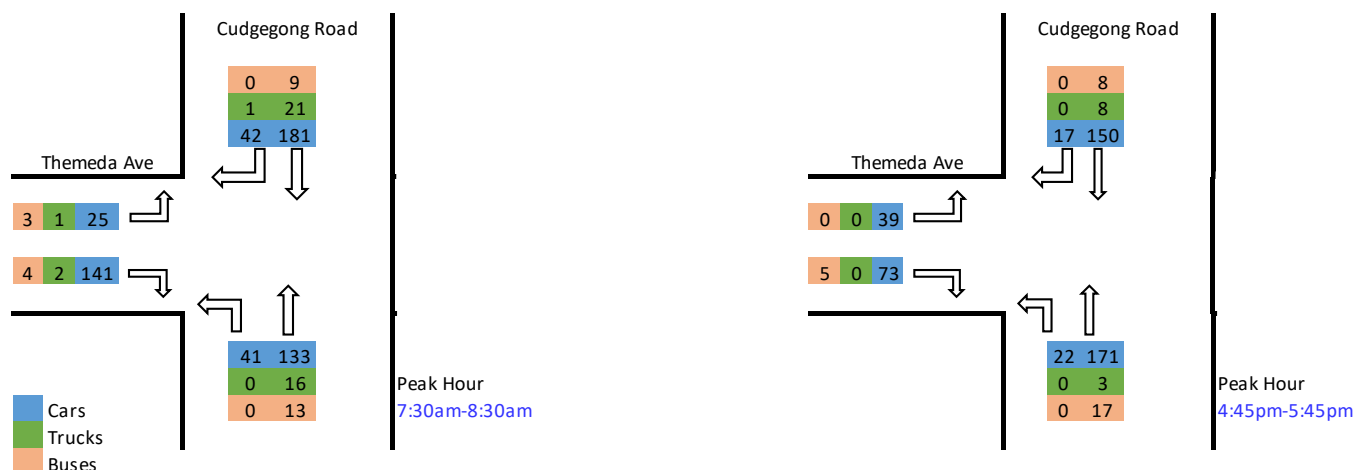


Figure 4: Themeda Avenue / Cudgegong Road Intersection Count

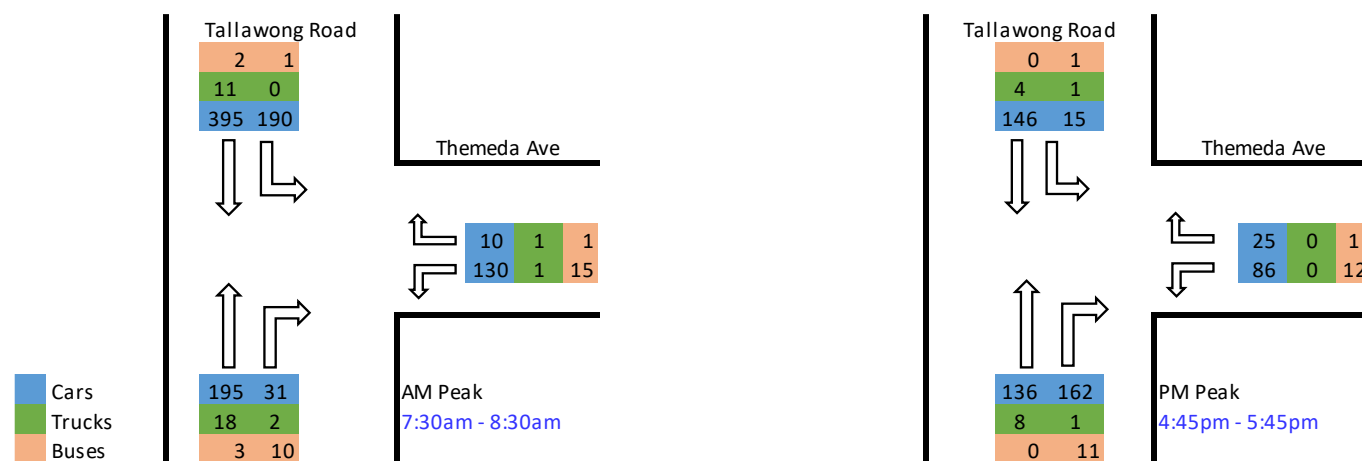


Figure 5: Themeda Avenue / Tallawong Road Intersection Count

2.4 Public Transport Facilities

The area is well serviced by public transport with four bus stops located within 400 metres of the site. These bus stops provide the following services:

- Route 607N Tallawong Station to City QVB via North West T-way and M2 Motorway
- Route 732 Rouse Hill to Blacktown via The Ponds
- Route 742 Marsden Park to Rouse Hill
- Route 747 Marsden Park to Rouse Hill via Riverstone
- Route 751 Rouse Hill Town Centre to Blacktown

The full bus network map is attached at **Appendix B**.

The Sydney Metro Northwest commenced services in June 2019. This rail link connects Sydney's north west region to the Sydney CBD with trains arriving every four minutes during peak hours. Accordingly, the Tallawong train station is located directly adjacent to the site (to the north) and provides connection to centres such as Castle Hill, Epping, Macquarie Park, and the Sydney CBD. **Appendix C** provides a context of the Sydney Metro Northwest with the greater Sydney Train Network.

As part of the Tallawong station facilities there are 1,000 commuter car parking spaces, 4 spaces for buses, 15 kiss-and-ride spaces, 9 taxi spaces and parking and storage for 35 bicycles.

Figure 6 is an extract from the Blacktown Growth Centres DCP and shows the public transport network map highlighting the proposed strategic bus corridors, bus routes, indicative bus stops and pedestrian catchments for bus stops (400m) and train stations (800m).

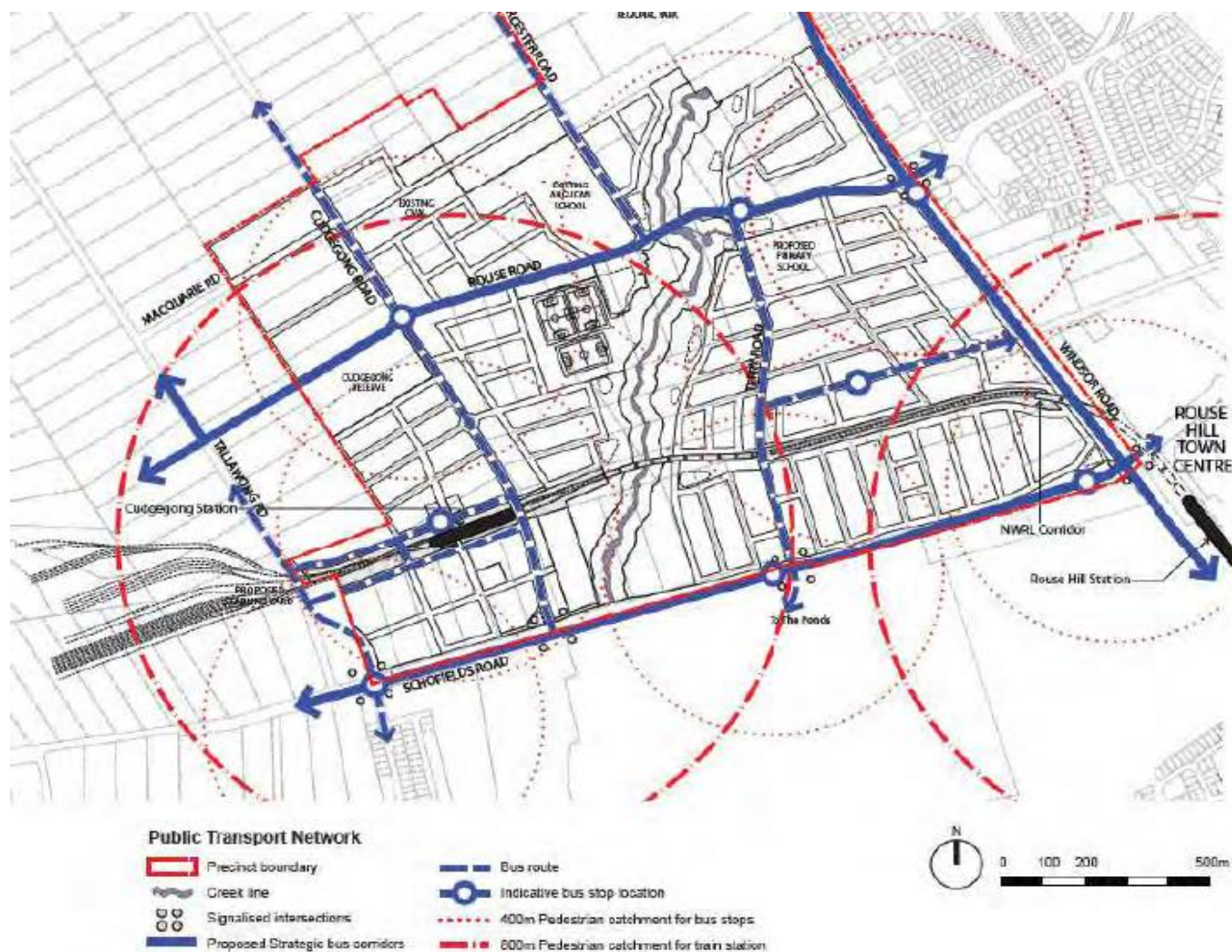


Figure 6: Blacktown Growth Centres DCP Schedule 4 Area 20 Precinct Public Transport Network Map 2011

Figure 7, below identifies existing public bus and transport options in close proximity to the site.

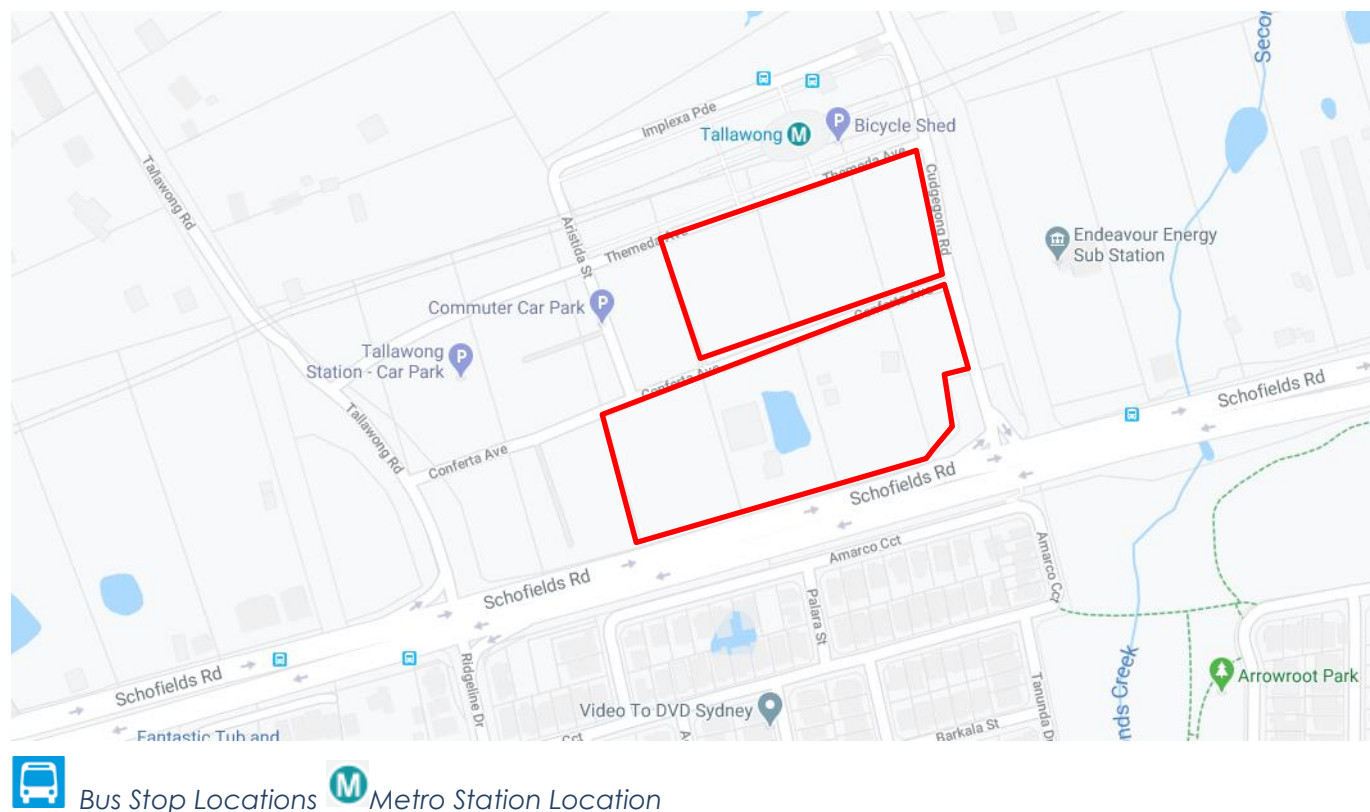


Figure 7: Location of bus stops and Sydney Metro Station to the site

2.5 Pedestrian and Bicycle Facilities

The pedestrian and cycling network extend within the road corridors of Schofields Road, Cudgegong Road and Tallawong Road.

In the immediate vicinity of the site, Themeda Avenue provides 3m wide pedestrian paths on both sides of the road. Conferta Avenue has 2.5m wide footpaths on the northern side of the road and a 1.9m footpaths on the southern side of the road. Aristida Street and Implexa Parade have footpaths on both sides of the road with a zebra crossing located on Themeda Avenue and Implexa Parade, directly in front of the Tallawong Metro Station.

Signalised pedestrian crossings are also available at the following signalised intersections:

- Cudgegong Road/ Schofields Road
- Tallawong Road / Schofields Road
- Themeda Avenue/ Cudgegong Road
- Themeda Avenue/ Tallawong Road

As mentioned above, at Tallawong Station there are bike racks available and a bicycle shed with parking for 35 bicycles.

An extract from the DCP Pedestrian and Cycle Network Map shows the site pedestrian and bicycle links that link the Tallawong Station Precinct South to the surrounding area.

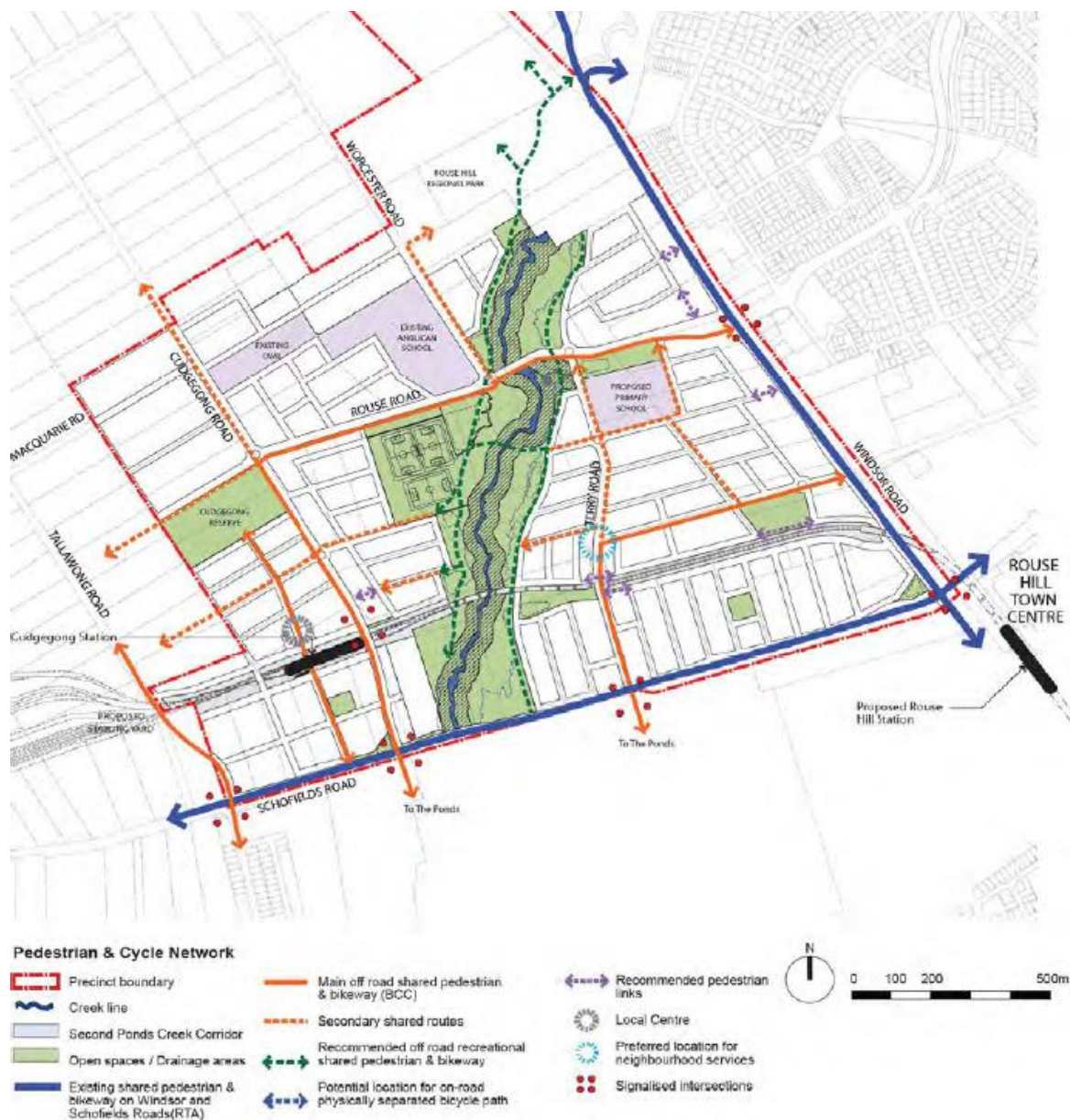


Figure 8: Blacktown Growth Centres DCP Schedule 4 Area 20 DCP Pedestrian and Cycle Network Map

In summary, the roads surrounding the site provide a high standard of pedestrian / cyclist facilities including shared paths and pedestrian and cyclist crossing facilities at each of the signalised intersections. The existing and future pedestrian and bicycle network surrounding the site is attached at **Appendix A**.

3 Proposed Development

3.1 Development Yield

The proposed development is comprised of residential and non-residential components as stipulated in the following table.

Table 1: Proposed Development Yield

| Land Use | | Yield |
|-------------|--------------|---------------------|
| Residential | 1 Bedroom | 252 units |
| | 2 Bedroom | 682 units |
| | 3 Bedroom | 53 units |
| | Total | 987 units |
| Retail | | 6,000m ² |
| Commercial | | 3,000m ² |

The basement level parking is spread over three levels and comprises of 1,368 spaces, comprising of 1,068 residential (including 28 visitor spaces), 100 commercial spaces and 200 retail spaces.

3.2 Access

As shown in Figure 9 below, access to the site will be provided via the vehicle crossings arrangement as follows:

- Site 1A Combined car and loading dock vehicle access on the Northern side of Conferta Avenue
- Site 1B Separated car and loading dock vehicle access on the Northern side of Conferta Avenue
- Site 2A Separated car and loading dock vehicle access on the Northern side of the new proposed road
- Site 2B, 2C & 2E Separated car and loading dock vehicle access on the Southern side of Conferta Avenue
- Site 2D Separated car and loading dock vehicle access on the Southern side new proposed road

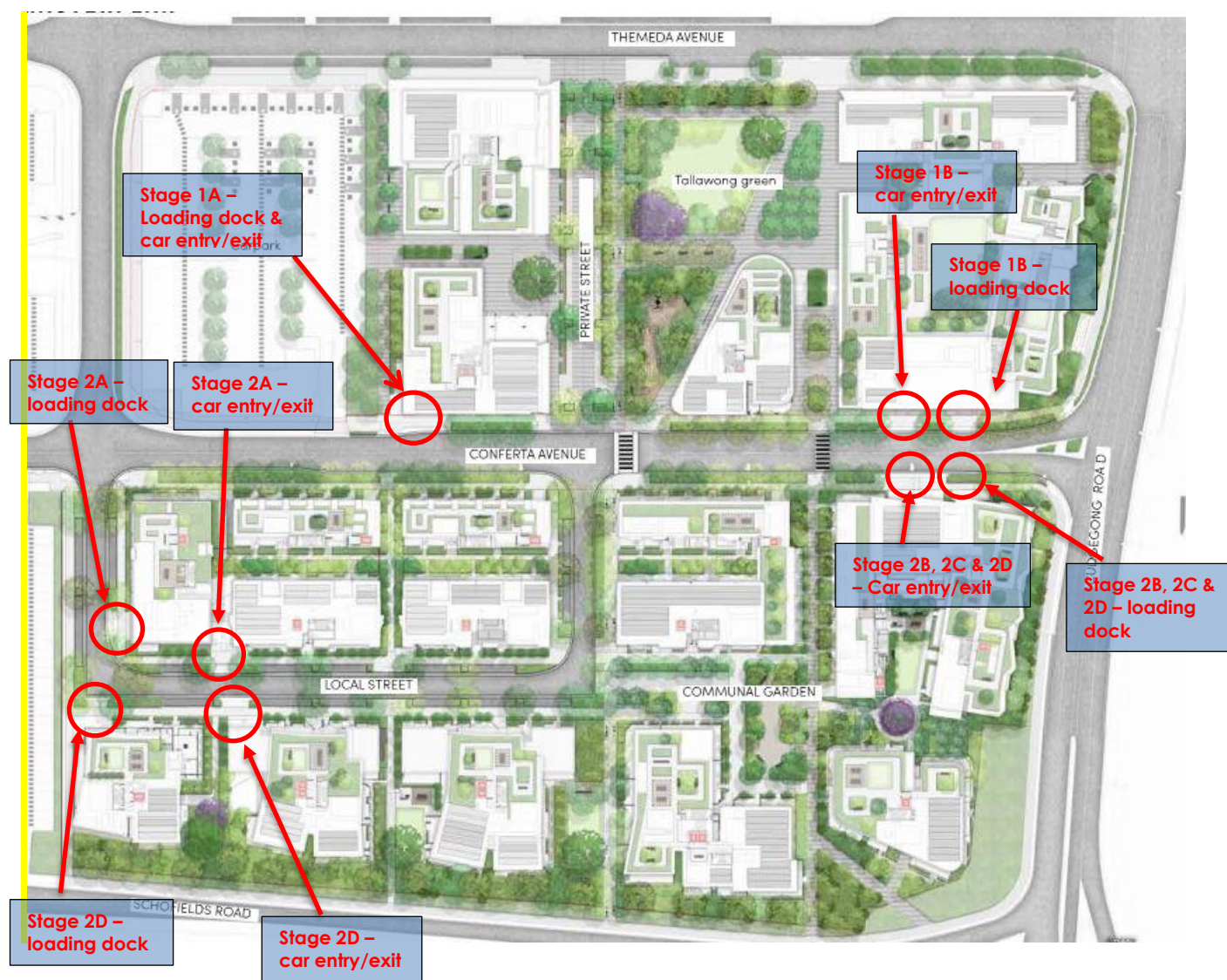


Figure 9: Extract from Landscape Plans showing the location of vehicle crossings within the development.

The entry/exit driveways comply with *AS/NZS 2890.1-2004 Parking Facilities – Off-Street Car Parking*, *AS 2890.2-2002 Parking Facilities – Off Street Commercial Vehicle Facilities*. More details are available in Section 4.7 of this report (Parking Compliance Check).

Car swept turning path plans of vehicles have been provided in **Appendix D** in accordance *AS/NZS 2890.1-2004 Parking Facilities – Off-Street Car Parking*.

3.3 Service Vehicles and Loading Requirements

Swept turning path plans of Blacktown Council's Waste vehicle have been provided in **Appendix D** in accordance with *AS 2890.2-2002 Parking Facilities – Off Street Commercial Vehicle Facilities* and the Waste Vehicle dimensions outlined in Blacktown City Council's DCP Part G Site Waste Management and Minimisation.

The plans demonstrate forward ingress and egress of an 11 metre Medium Rigid Vehicle (MRV) between the site and Conferta Avenue / proposed new road. Internally, the MRV is able to enter the site in a forward direction, satisfactorily reverse into the waste collection area and loading bay and exit in a forward direction.

The swept turning paths demonstrate that the proposed loading and waste vehicle access provisions comply with Blacktown Council's DCP (minimum 25m) and the requirements of AS 2890.2-2018.

3.4 Public Domain Improvements

Public domain improvements are proposed to the extent outlined in the extracts from the Civil Plans in Figure 10 below not only to accommodate vehicle, pedestrian and bicycle movements from the development to the surrounding road, pedestrian and cycle networks but to also improve the landscaping and overall amenity of the precinct for pedestrians.

Apart from upgrading Conferta Avenue to accommodate a safe movement of vehicles, pedestrians and cyclists, a new public road is also proposed to allow vehicle access to the basement parking and loading bay area and to provide for safe pedestrian movements.

To facilitate pedestrian movement from the precinct to Tallawong Station and the bus stops adjacent to the station, pedestrian and cycling links are provided through the site as shown in Figure 12 and two pedestrian crossings are proposed in Conferta Avenue as indicated in Figure 10 below.

To address concerns about pedestrian and cyclist safety while crossing Conferta Avenue it would be possible to provide a limited amount of on-street parking along both sides of the road as shown in Figure 10 below.

The presence of on-street parking has been shown to reduce traffic speeds as drivers are more aware of vehicle parking movements and adjust their speed accordingly. This on-street parking could most effectively be provided by line marking a parking lane on each side of Conferta Avenue and providing a combination of time-limited parking signs, No Stopping and No Parking signs as shown in Figure 10. The installation of the line marking and signposting can only be implemented with the approval of Blacktown City Council and, if approved, would be installed as part of the development proposal.

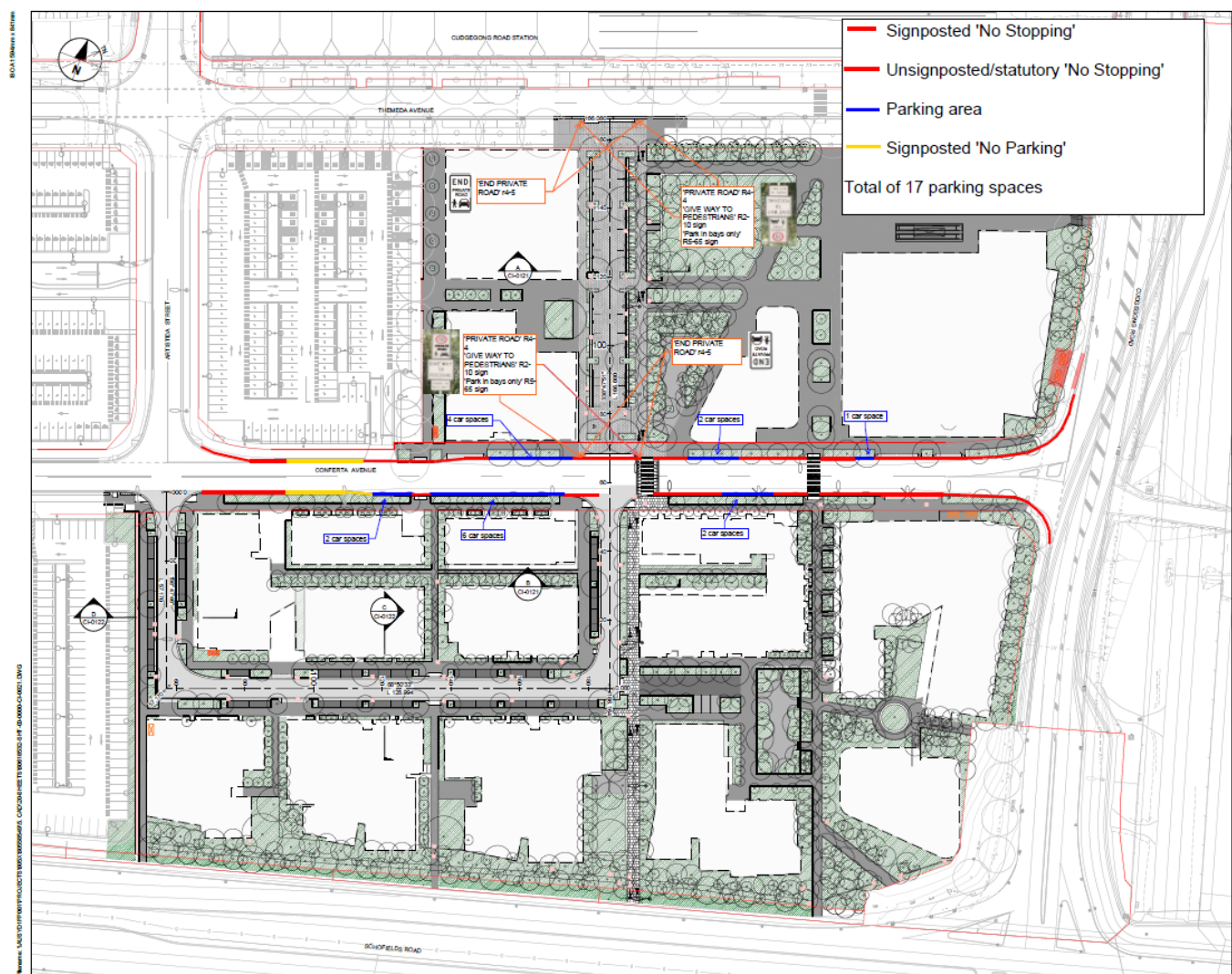


Figure 10: Extract from Civil Plans showing the proposed on-street parking in Conferta Avenue

According to the Landscape Plans, "The proposed development will deliver a new standard in public domain along the North West Priority Growth Area. Delivering high end public realm comprising of green open space with high end playground". And, "The streetscape is designed to be to create a vibrant, diverse and cool character".

With regards to accessibility the Landscape Plans state, "All residents and local, community may access and enjoy the public domain universal access to AS1428 has been incorporated into where possible connected walkways, the public spaces and building entries without compromising design quality."

Figure 11 below is an extract from Landscape Plans showing an overview of the proposed tree planting through the site, including street tree planting within the existing Conferta Avenue, Themeda Avenue and the proposed new public road.



Figure 11: Extract from Landscape Plans showing the proposed landscape works.

3.5 Public Transport network

Note that the proposed 987 residential units is less than the original 1,100 residential units approved in the concept proposal. Therefore, the Sydney Metro Northwest line and bus network would have been designed to accommodate a higher density on the site than what is currently approved.

From the Green Travel Plan prepared by Barker Ryan Stewart it was estimated that the precinct once fully completed and occupied would generate approximately 100 people per hour traveling by train in the morning peak between 7am and 9am and the evening peak between 4pm and 6pm. It is also estimated that approximately 50 people per hour would leave the precinct to travel by bus in the morning 7am to 9am peak and the evening peak between 4pm and 6pm.

The Sydney Metro Northwest can have trains departing in each direction every four minutes (15 trains per hour) and has the capacity of moving approximately 20,000 people in each direction every hour. Therefore, the additional commuters estimated to be generated by the development would be easily catered for by Sydney Metro Northwest in fact this is the reason the metro line was built in order to accommodate residents in the Northwest Growth area.

The number of people from the precinct likely to commute by bus would be able to be catered for by the buses leaving from Tallawong Station. Figure 6 shows the bus public transport map in the vicinity of the site.

3.6 Pedestrian and Bicycle Facilities

According to the 2016 Census, the average persons per household in the Rouse Hill / Schofields area is 3.07. As the proposed development consists of 987 apartments, it is assumed that the development will accommodate around 3,000 residents.

The census also indicated that 50% of the population in Rouse Hill / Schofields area is employed which equates to 1,500 people employed. In addition, 19.1% of the population travelled to work by public transport (train and bus) or cycled / walked (1.1%), which provides an estimated 300 people that will regularly walk between the site and Tallawong Metro Station and adjacent bus stops during the 2 hour peak periods.

Pedestrian and cyclist access modelling to the proposed development was not considered necessary as the off road pedestrian and cycle paths in Schofields Road and Cudgewong Road and the wider network would have been modelled on a regional scale and the network would be able to cater for the number of pedestrians and cyclists that would be generated by the development, particularly that the development yield proposed is a smaller residential unit yield than what was previously approved by the concept SSDA.

Site Pedestrian Access

Extensive public pedestrian paths exist in the precinct area to support the recently constructed Tallawong Station. The site is located immediately adjacent to the train station with a number of key public facilities within walking distance to the site.

The site is well connected with pedestrian footpaths which allow easy access from Tallawong station to and from the main pedestrian entries to the precinct.

In addition to the public domain works outlined in Section 3.4 above and the proposed through site cycle and pedestrian links shown in Figure 12 below, the existing pedestrian network of footpaths on surrounding public roads would encourage residents, staff and users of the precinct to walk from destination to destination or use a combination of public transport and walking. This would represent a significant mode shift toward active transport based on promotion of site permeability and north- south shared pedestrian links through the site.

The pedestrian site links proposed within the precinct together with the proposed upgrades to the public domain within Conferta Avenue including the two pedestrian crossings will provide safe pedestrian access to Tallawong Station and the surrounding pedestrian network.

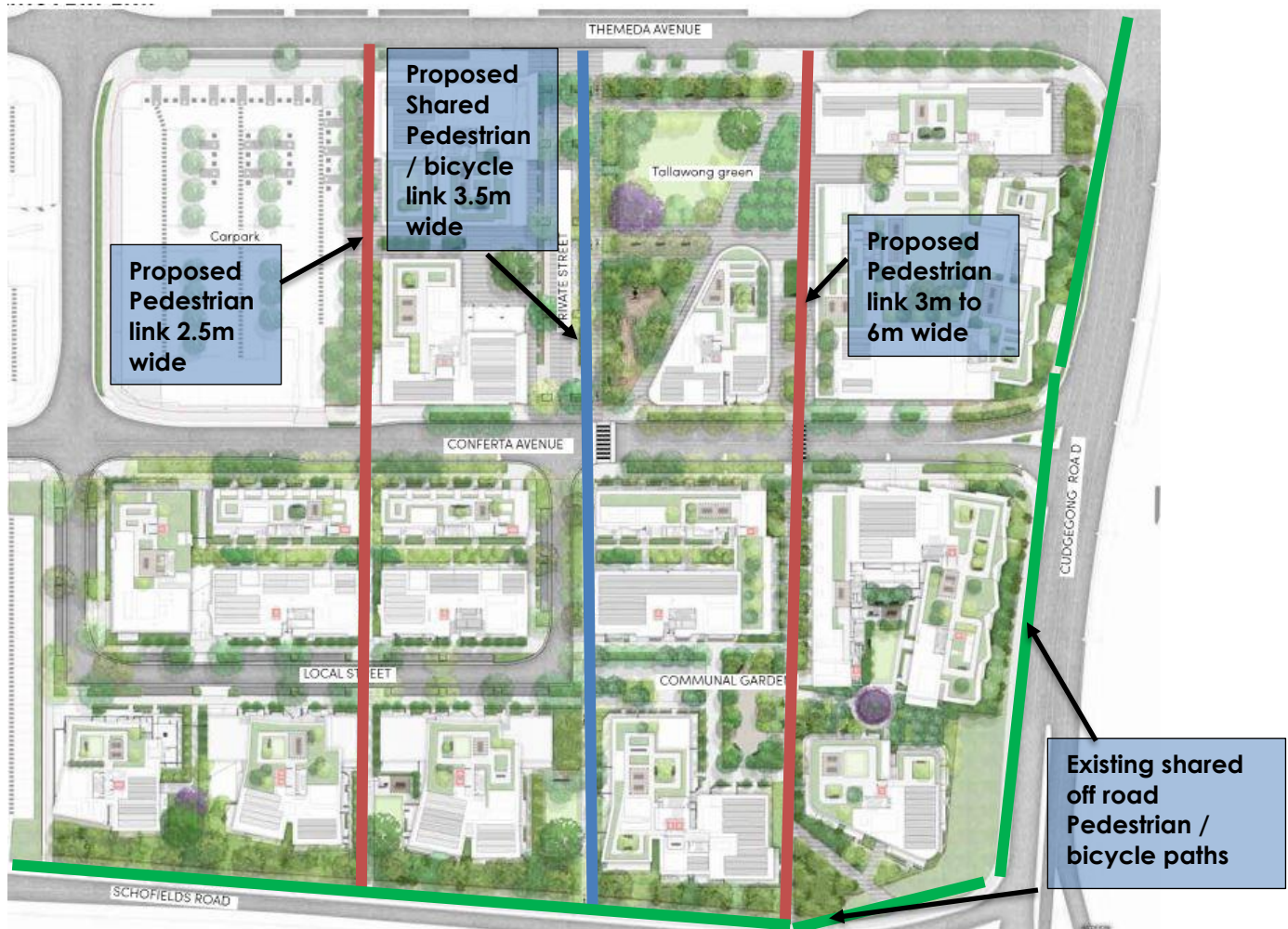


Figure 12: Extract from Landscape Plans with pedestrian and bicycle links added

Bicycle Facilities

In regard to cycling, the 2016 census showed that only 0.2% of the population cycled to and from work. However, assuming a more conservative estimate of 1% of the total residents cycling each day will give a total of 150 cyclists per day.

It is proposed to provide 1,086 bicycle spaces to the precinct which is considered more than sufficient to cater for the storage and facility needs of the residents and their visitors. Note that there is no requirement in the Concept Plan SSD 9063 to provide the non-residential uses with bicycle storage.

The bicycle storage spaces are proposed to be provided at a rate of one space for each residential unit for residents and one space for each 10 units for residential visitors in accordance with the Concept Plan SSD 9063 requirements. This equates to $987 \times 1 + 987/10 = 1,086$ bicycle parking spaces.

Bicycle parking is proposed to be provided in the following locations adjacent to lifts:

- Site 1A Basement 1 – 55.2m² storage area for 24 bicycles and 76.2m² for 31 bicycles.
- Site 1A Basement 2 – 144.5m² storage area for 69 bicycles.
- Site 1B Basement 3 – 179.8m² storage area for 111 bicycles, 148.2m² for 38 bicycles, 114.7m² for 50 bicycles, 43.6m² for 24 bicycles and 43.1m² for 19 bicycles.
- Site 2A Mezzanine – 359m² storage area and 182 bicycles.
- Site 2B, 2C & 2E Mezzanine – 167.6m² and 74 bicycles.

- Site 2B, 2C & 2E Basement 1 – storage area 51m² for 20 bicycles, 186.3m² for 80 bicycles, 164.7m² for 80 bicycles and 187.1m² for 95 bicycles.
- Site 2D Mezzanine – 127m² storage area for 60 bicycles and 258m² storage area for 129 bicycles.

The precinct will also incorporate dedicated cycle and pedestrian links as shown in Figure 12 above. This includes off road bicycle links in Schofields Road and Cudgegong Road and on road link in Themda Avenue and Conferta Avenue.

Access will be enhanced from Schofields Road through the precinct to Tallawong Station and proposed Cudgegong Town Centre in accordance with Blacktown Growth Centre Precincts DCP mapping. As the North West Growth Centre begins to support greater number of residents, it is anticipated that the provision of additional safe and connected bicycle infrastructure will encourage alternative modes of transport to be utilised by residents and visitors to the site.

The development will connect to Rouse Hill Town Centre via the existing shared path along Schofields Rd. **Appendix A** shows the cycleways along Cudgegong and Tallawong Roads that have been partially constructed by Sydney Metro.

Overall, the precinct will have safe and excellent access for cyclists to public transport and bicycle network and facilities which can accommodate the bicycle movements generated by the proposal.

4 Car Parking Assessment and Strategy

4.1 Parking Requirements

The basement level parking includes 1,368 spaces, comprising of 1,068 residential spaces (including 28 visitor spaces), 100 commercial spaces and 200 retail spaces.

The applicable parking rates for the development are provided below:

Concept SSD 9063 Approval Rates

- 0.6 space per 1-bedroom unit
- 0.9 spaces per 2-bedroom unit
- 1.4 spaces per 3-bedroom unit
- 1 visitor space per 10 units
- 1 space per 60m² retail space
- 1 space per 70m² commercial space

Blacktown Council DCP Rates

- 1 spaces per 1-bedroom unit
- 1 spaces per 2-bedroom unit
- 2 spaces per 3-bedroom unit
- 1 visitor space per 5 units
- 1 space per 22m² retail space (over 200m²)
- 1 space per 40m² office space

RMS Guide Rates (Metropolitan Sub-Regional)

- 0.6 space per 1-bedroom unit
- 0.9 spaces per 2-bedroom unit
- 1.4 spaces per 3-bedroom unit
- 1 visitor space per 5 units
- 4.5 spaces per 100m² retail space
- 1 space per 40m² commercial space

Proposed Rates

- 1 space per 1-bedroom unit
- 1 space per 2-bedroom unit
- 2 spaces per 3-bedroom unit
- 1 visitor space per 35 units
- 1 space per 30m² retail space
- 1 space per 30m² commercial space

A summary of these requirements in relation to the development yield is included in Table 2 below.

Table 2: Minimum car parking requirements

| Land Use | | Concept SSD 9063 Units | Concept SSD 9063 Approval | Proposed Units | SSD 9063 rates | RMS Guide | Council DCP | Proposed Parking |
|------------------------------|-----------|------------------------|---------------------------|------------------|----------------|--------------|--------------|------------------|
| Residential | 1-Bedroom | 220 units | 132 | 252 units | 152 | 151 | 252 | 252 |
| | 2-Bedroom | 660 units | 594 | 682 units | 594 | 614 | 682 | 682 |
| | 3-Bedroom | 220 units | 308 | 53 units | 74 | 74 | 106 | 106 |
| | Visitors | Total units: 1,100 | 110 | Total units: 987 | 99 | 198 | 198 | 28 |
| Residential sub total | | | 1,144 | | 919 | 1,037 | 1,238 | 1,068 |

| Land Use | Concept SSD 9063 GFA | Concept SSD 9063 Approval Requirement | Proposed GFA | SSD 9063 rates | RMS Guide | Council DCP | Proposed Parking |
|------------------------------|-----------------------------|---------------------------------------|-----------------------------|----------------|------------|-------------|------------------|
| Commercial | 3,000m ² | 43 | 3,000m ² | 43 | 75 | 75 | 100 |
| Retail | 6,000m ² | 100 | 6,000m ² | 100 | 270 | 273 | 200 |
| Non-Residential total | Total = 9,000m ² | 143 | Total = 9,000m ² | 143 | 345 | 348 | 300 |

| Land Use | Concept SSD 9063 Approval requirement | SSD 9063 rates | RMS Guide Requirement (based on proposal) | Council DCP Requirement (based on proposal) | Proposed Parking |
|------------------------------|--|-------------------|--|--|---------------------|
| Residential sub total | 1,144 | 919 | 1,037 | 1,238 | 1,068 |
| Non-Residential total | 143 | 143 | 345 | 348 | 300 |
| Total | 1,287 | 1,062 | 1,382 | 1,586 | 1,368 |

4.2 Transit orientated development parking guiding principal

'As a guiding principal, Sydney Metro and Landcom consider that a reduced car parking provision for Tallawong Station Precinct South is fundamental to achieving:

- An exemplar transit orientated development (maximising the benefits of fast frequent metro connections with services every 4 minutes in the peak and 10 minutes in off peak.
- A town centre where cars do not dominate
- Activation and life on the street
- Less congestion of precinct roads.

Providing high levels of car parking does not align with the key principles of transit orientated development'

The parking provision included in the concept approval for the development was determined by applying the car parking requirements of State Environmental Planning Policy No 65 - Design Quality of Residential Apartment Development (SEPP 65) and the Apartment Design Guide which states that:

"Car parking is provided based on proximity to public transport in metropolitan Sydney and centres in regional areas. For development on sites that are within 800m of a railway station or light rail stop in the Sydney Metropolitan Area the **minimum** car parking requirement for residents and visitors is set out in the Guide to Traffic Generating Developments, or the car parking requirement prescribed by the relevant council, whichever is less".

Given the location of the development close to Tallawong Station, the parking requirements in the RMS Guidelines for Metro Sub-Regional CBD Centres were adopted in the concept approval when determining the required number of parking spaces for the residential components of the site.

The parking rates applicable to the non-residential (commercial / retail) component of the proposed development adopted for the concept approval were based on the rates in the Parramatta DCP for Epping Town Centre.

4.3 Tallawong Station existing commuter car parks

As part of the Tallawong station facilities there are 1,000 car parking spaces (820 regular spaces, 30 accessible spaces and 150 compact car spaces) in the three commuter carparking areas. There are also 4 spaces for buses, 15 kiss-and-ride spaces and 9 taxi spaces provided in Themeda Avenue and Implexa Parade.

The results of the of the AM peak period on Thursday 12 March 2020 are indicated in Figure 13 and the PM peak period on the same day are indicated in Figure 14.

| Tallawong Station commuter car parks AM 200312 | | | | | | | | | | | | |
|--|-------------------------|-----|--------------|------------------|-------------------------|-----|--------------|------------------|-------------------------|-----|--------------|------------------|
| Time | Car Park 1 (147 spaces) | | | | Car Park 2 (454 spaces) | | | | Car Park 3 (397 spaces) | | | |
| | In | Out | Total parked | Available spaces | In | Out | Total parked | Available spaces | In | Out | Total parked | Available spaces |
| 6:45 | | | 147 | 0 | | | 239 | 215 | | | 5 | 392 |
| 7:00 | | | 147 | 0 | 69 | | 308 | 146 | | | 134 | 263 |
| 7:15 | | | 147 | 0 | 98 | | 406 | 48 | | | 196 | 201 |
| 7:30 | | | 147 | 0 | 39 | | 445 | 9 | | | 243 | 154 |
| 7:45 | | | 147 | 0 | 9 | | 454 | 0 | | | 317 | 80 |
| 8:00 | | | 147 | 0 | | | 454 | 0 | | | 354 | 43 |
| 8:15 | | | 147 | 0 | | | 454 | 0 | | | 377 | 20 |
| 8:30 | | | 147 | 0 | | | 454 | 0 | | | 396 | 1 |
| 8:45 | | | 147 | 0 | | | 454 | 0 | | | 397 | 0 |
| 4 motor bikes also entered Car Park 2 between 7am-7:30am | | | | | | | | | | | | |
| Car Park 1 was full at 6:15am except for the disabled spaces which were all free | | | | | | | | | | | | |
| Car Park 3 had electronic signage indicating the number of spaces that were free, this sign was checked every 15 minutes | | | | | | | | | | | | |

Figure 13: The results of the commuter car park survey AM

| Tallawong Station commuter car parks 200312 | | | | | | | | | | | | |
|---|-------------------------|-----|--------------|------------------|-------------------------|-----|--------------|------------------|-------------------------|-----|--------------|------------------|
| Time | Car Park 1 (147 spaces) | | | | Car Park 2 (454 spaces) | | | | Car Park 3 (397 spaces) | | | |
| | In | Out | Total parked | Available spaces | In | Out | Total parked | Available spaces | In | Out | Total parked | Available spaces |
| 4:00 | | | | | | | | | | | | |
| 4:15 | | | | | | | | | | | 354 | 43 |
| 4:30 | 3 | 6 | 119 | 28 | 3 | 17 | 380 | 74 | | | 350 | 47 |
| 4:45 | 2 | 16 | 105 | 42 | | 13 | 367 | 87 | | | 342 | 55 |
| 5:00 | 4 | 8 | 101 | 46 | 1 | 40 | 328 | 126 | | | 326 | 71 |
| 5:15 | 1 | 10 | 92 | 55 | 4 | 39 | 293 | 161 | | | 302 | 95 |
| 5:30 | 3 | 10 | 85 | 62 | | 38 | 255 | 199 | | | 269 | 128 |
| 5:45 | 4 | 7 | 82 | 65 | 1 | 31 | 225 | 229 | | | 239 | 158 |
| 6:00 | 3 | 4 | 81 | 66 | 1 | 45 | 181 | 273 | | | 200 | 197 |
| Informal car parking on opposite side of Cudgegong Road to development site - 108 cars parked at 4:15pm | | | | | | | | | | | | |
| On street parking in Cudgegong Road stretching from bridge over Sydney Metro west to Rouse Road (400m) - 64 cars parked at 4:15pm | | | | | | | | | | | | |
| On Street parking in Grassland Court - 58 cars parked at 4:15pm | | | | | | | | | | | | |
| Car Park 3 had electronic signage indicating the number of spaces that were free, this sign was checked every 15 minutes | | | | | | | | | | | | |

Figure 14: The results of the commuter car park survey PM

In summary we undertook a car park survey on Thursday 12 March 2020 with the following results:

- P1 (147 spaces) – Full by 6:15am
- P2 (454 spaces) – Full by 7:45am
- P3 (397 spaces) – Full by 8:30am
- P1 had 28 spaces available by 4:30pm and 66 available at 6pm
- P2 had 74 spaces available by 4:30pm and 273 available by 6pm
- P3 had 43 spaces available at 4:15pm and 197 available by 6pm.
- Note that the 30 accessible spaces in P1 remained available throughout the survey.
- Informal car parking on the opposite side of Cudgegong Road to the development site – 108 vehicles parked at 4:15pm
- On street parking in Cudgegong Road stretching from Sydney Metro to Rouse Road (400m) – 64 cars parked at 4:15pm.
- On Street parking in Grassland Court – 58 cars parked at 4:15pm.

The car parking survey reveals that the 1,000 existing car parking spaces located at the Tallawong Station are already at capacity (with the exception of the accessible spaces in P1 which remained vacant) with a substantial overflow of all-day parking currently occurring onto the surrounding street network and adjoining properties during the working week.

Note that this car park survey was also undertaken at a time when the coronavirus was already impacting the use of public transport.

Furthermore, NearMap satellite images taken once per month show that on consecutive months on the dates indicated below the car parks were full.

- Tuesday 29th Oct 2019
- Thursday 12th of Sep 2019
- Friday 16th Aug 2019
- Friday 19th of July 2019

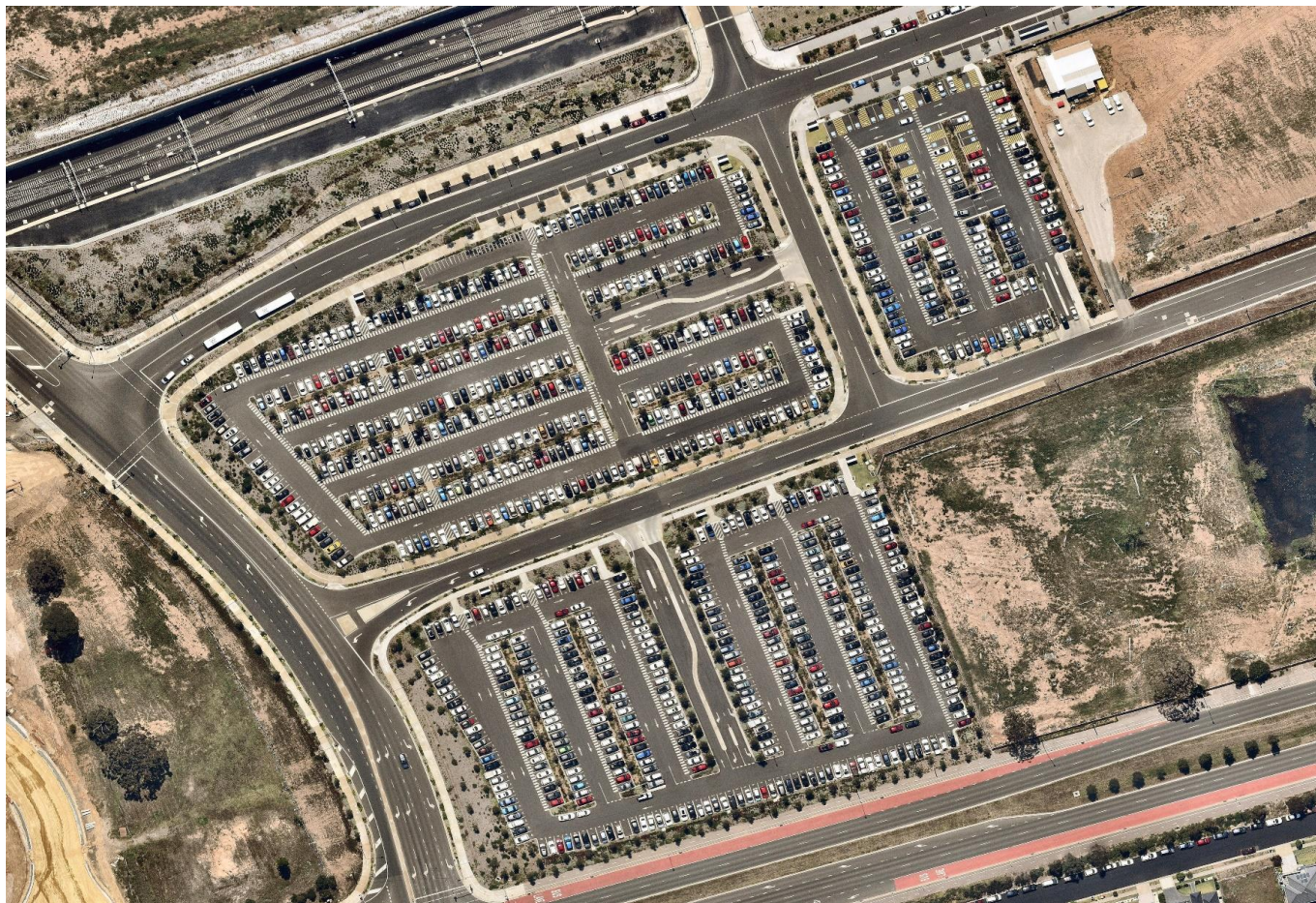


Figure 15: NearMap image 29 October 2019 showing the car parks full.

This indicates that the moment that the Tallawong Station car parks were completed and opened they were at capacity.

4.4 Basis of Proposed Parking Provision

As outlined in Section 4.1 the proposed parking provision is summarised in the following table.

Table 3: Proposed parking

| Land Use | Concept SSD 9063 Approval requirement | SSD 9063 rates | RMS Guide Requirement (based on proposal) | Council DCP Requirement (based on proposal) | Proposed Parking |
|------------------------------|--|-------------------|--|--|---------------------|
| Residential sub total | 1,144 | 919 | 1,037 | 1,238 | 1,068 |
| Non-Residential total | 143 | 143 | 345 | 348 | 300 |
| Total | 1,287 | 1,062 | 1,382 | 1,586 | 1,368 |

It should be noted that the primary purpose of the Sydney Metro Northwest is to connect the North-West area with major employment and education centres such as Epping, Macquarie University, Macquarie Park, North Ryde, Chatswood and the Sydney CBD primarily for those commuting to work and university.

It is acknowledged that planning for new developments in this area needs to consider the principles of transit-oriented development and the implementation of targeted travel demand management measures and initiatives to reduce the need and reliance on private vehicle travel.

However, the guiding principles do not take into consideration the unique location of Tallawong Precinct South being located at the end of the Sydney Metro Norwest Line and the extensive catchment area it serves. The 1,000 existing commuter car parking spaces located at the Tallawong Station are already at capacity with a substantial overflow of all-day parking currently occurring onto the surrounding street network during every workday. By 8:30am the existing car parks are full by commuters travelling on the Sydney Metro Norwest line, once full drivers of vehicles then proceed to park in the surrounding street network.

The catchment area beyond the North West Growth centre is predominantly rural in nature with poor or no public transport options available to the residents.

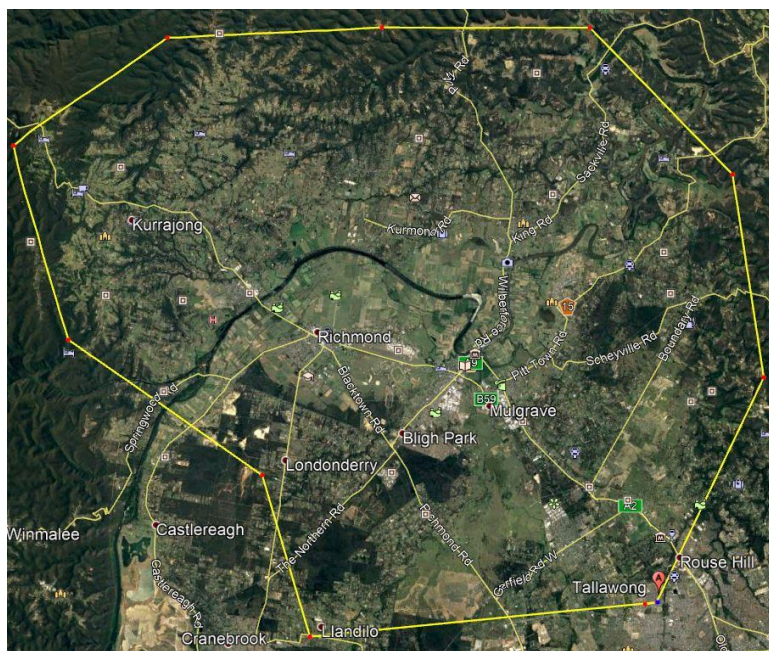


Figure 16: Estimated Catchment area of car users that currently park at Tallawong Station

Even once the Sydney Metro Norwest extends to the Western Sydney airport and connects with Schofields Train Station, this will only partially reduce this catchment area. Also, the surrounding North West Growth sector in the immediate vicinity of Tallawong Station and to the west and east of the station is only partially developed. Once this is fully developed this would put even more pressure on the at capacity Tallawong Station car parks.

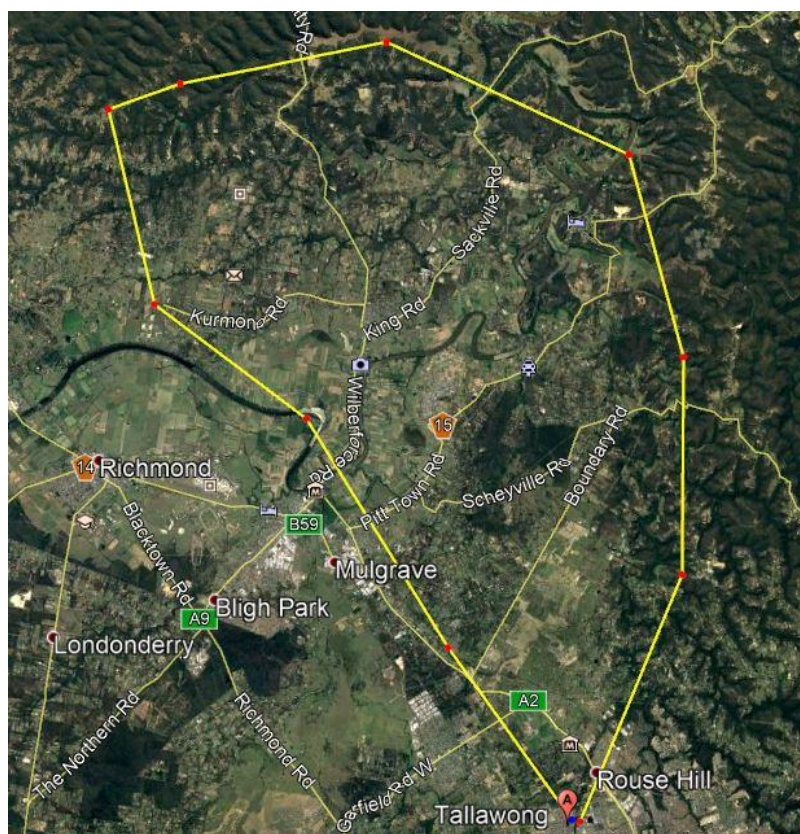


Figure 17: Estimated Catchment area of car users once Sydney Metro Northwest connects with Schofields Station

Furthermore, in the developing area around Tallawong Station there will also be a high demand for a range of local trips by residents on week days such as school / childcare centre pick up and drop off and on weekends for accessing family sporting activities, shopping and visiting family and friends at destinations that are not readily served by public transport. Consequently, there will continue to be a high demand for parking spaces associated with new developments in this area.

Evidence of this high demand for parking can be seen in a review of six residential developments recently approved by Blacktown City Council in the vicinity of Tallawong Station. This review has revealed that, in almost all cases, the parking rates adopted were 1 space per 1 and 2-bedroom units and 2 spaces per 3 bedroom units, with visitor parking at a rate of 1 space per 5 units. The parking rates proposed for the residential component of this development are consistent with the rates that have been adopted in these recently approved developments.

CBRE provided advice to the saleability of residential units in the area and they stated that units without a car parking space are difficult to sell compared to those that do. (See letter at **Appendix G**)

Strictly complying with the car parking rates outlined by the Concept Approval would result in a development that would have insufficient car parking to cater for residential component of the development and would put pressure on the at capacity Tallawong Station car parks and on-street parking in the surrounding road network.

The proposed parking rates are considered better able to meet the travel demands associated with the characteristics of the Tallawong area; are consistent with the parking provision of other approved developments in the vicinity of the Tallawong station and will prevent a detrimental impact on the already at capacity Tallawong Station commuter car parking areas and the surrounding road network.

Although not required to support the proposed parking rates, there is potential for four GoGet shared parking spaces to be provided in the new private road in the vicinity of the Tallawong Station.

In summary, it is proposed that the proposed parking rates of one space per one and two bedroom units, two spaces for each three bedroom unit, one space per 35 units for residential visitor parking and one space per 30m² of GFA for the retail and commercial component should be adopted. These rates are considered to be better able to meet the travel demands associated with the characteristics of the Tallawong area and consistent with the parking provision of other approved developments in the Blacktown City Council and The Hills Council LGA within vicinity of the Tallawong Metro station and to reduce the detrimental impact at the already at capacity Tallawong Station car parks and the surrounding road network.

The proposed parking provision of 1,368 spaces is only 81 spaces (or 5.9%) more than what was approved in the concept plan and provides a balance between the minimum requirements of the RMS Guide, Blacktown DCP and the maximum rates of transit orientated development guiding principles.

4.5 Residential Parking Provision

The proposed provision of 1,068 residential spaces for the 987 units proposed equates to approximately one space (1.08 spaces) per unit and is considered satisfactory to cater for the car parking requirements generated by the development.

The residential parking provision is based on the rates of one space per one and two-bedroom units, two spaces for each three bedroom unit, one space per 35 units for residential visitor parking.

The proposed 1,068 residential spaces are considered to be warranted and acceptable for the following reasons:

- The proposed 1,068 residential spaces comply as it is more than the SEPP65 **minimum** parking requirement of 1,037 spaces based on the car parking rates required by the RMS Guide.
- The residential component of 1,068 spaces proposed is 170 spaces less than the 1,238 spaces based on the **minimum** car parking rates required by the Blacktown DCP.
- The original concept approval for the site was for 1,100 residential units and the provision of 1,144 car parking spaces for those units. The current proposal is for 987 residential units and the provision of 1,068 car parking spaces. This is a net reduction of 76 total residential car parking spaces compared to that which was approved in the concept proposal.
- Adequate secure on-site parking is a necessity for all residents to enable them to have a car parking space available 24/7 so they can drive to destinations such as work, schools, sports grounds, recreational areas, visiting friends and relatives etc where there is inadequate public transport modes available from the site.
- The provision of the 28 visitor car parking spaces is considered adequate for the following reasons:
 - Minimum visitor parking would address, to some extent, the transit orientated development guiding principles to minimise the parking provision.
 - Minimising visitor parking would encourage visitors to use the public transport during peak periods.
 - There would be an opportunity for visitors to utilise the 23 car parking spaces to be provided in the new private road and 34 on street parking in the new road (see Figure 10).
 - Residential visitors tend to visit outside of peak working week hours and based on the car park survey and satellite images there would be plenty of parking available in the 1,000 space commuter car parks that would be underutilised after 4pm during weekdays and through the weekends which is the peak time for visitors.
- Strictly complying with the car parking rates outlined by the Concept Approval would result in a development that would have insufficient car parking to cater for the residential component of the development and would put pressure on the at capacity Sydney Metro commuter car park and on-street parking in the surrounding road network.

4.6 Non - Residential Parking Provision

The non-residential parking rates adopted for the concept approval were from the Parramatta DCP for Epping Town Centre. However, the travel demand, socio-economic and environmental characteristics of Tallawong and surrounding areas are vastly different to those at Epping being a well-established location close to employment and educational facilities with highly-developed road and public transport networks. The non-residential parking rates for the Epping Town Centre are therefore not considered to be appropriate for use at Tallawong.

In addition, the types of commercial / retail developments proposed have now been identified in more detail since the planning for the concept approval was conducted. Consequently, the parking rates required by RMS and Blacktown City Council are considered to be more appropriate as they relate to specific land use types rather than the generic commercial / retail land uses referred to in the concept approval.

The RMS rates in their Guide to Traffic Generating Development for retail and commercial uses are derived from undertaking numerous car parking surveys of similar retail and commercial developments and are considered appropriate in determining the demand for parking for these types of uses.

However, with a view to follow the guiding principles of transit oriented development with respect to the minimising the provision of parking adjacent Tallawong Station Precinct South, Deicorp has sought advice from an expert that negotiates leases/sale of properties to all the supermarket chains and other retail and commercial operators. They sought to determine the minimum amount of parking that could be provided for retail and commercial uses on site that would still make that component of the development viable and usable.

The expert Shiprock Advisory have stated that one space per 30m² would be the minimum rate at which parking should be provided for the retail and commercial uses. (letter is attached at **Appendix G**).

Strictly complying with the car parking rates outlined by the Concept approval would result in a development that would have insufficient car parking to cater for non-residential component of the development and would put pressure on the existing at capacity Sydney Metro commuter car park and on-street parking in the surrounding road network.

On this basis the proposed provision of 300 non-residential spaces based on the rate of one space per 30m² is considered satisfactory to cater for the car parking requirements generated by the retail and commercial component of the development.

The proposed provision of 300 spaces is 48 spaces less than the 348 spaces required by the Blacktown DCP car parking rates and 45 spaces less than the 345 spaces required by the RMS Guide to Traffic Generating Developments car parking rates.

4.7 Parking Compliance Check

Barker Ryan Stewart has reviewed the Architectural plans prepared by Turner. This review included the layout of car parking and internal roadways / ramps and overall, we are satisfied that the design is consistent with the requirements of Standards AS/NZS 2890.1, AS 2890.2-2002, AS 2890.3-2015 and AS/NZS 2890.6 and Blacktown Council DCP. It is anticipated that the car park will function in a satisfactory manner and in accordance with the original design intent. A summary of critical parameters assessed regarding the Australian Standards is included below.

Table 4: Compliance Table

| Control | Proposed | Compliance |
|---|---|------------|
| AS2890.1-2004 (Off-street Car Parking), AS2890.2-2002 (Off-street commercial vehicle facilities) and Council DCP | | |
| 2.4.1 Car Space Dimensions: Class 1A (residential) Class 3 (commercial and retail) | 2.4m x 5.4m with 5.8m aisle width 2.6m x 5.4m with 5.8m aisle width | Yes |
| 2.4.2 Blind aisle Extension & Clearance | 1 aisle extension provided for end of aisle parking spaces | Yes |
| 2.5.2 Layout Roadways/Ramps | Passenger vehicle: Minimum 5.5m wide for two-way flow Heavy vehicle: minimum 6.5m wide for two-way flow | Yes |
| 2.5.3 Roadway/Ramp Grades | Passenger vehicle: Max 1:4 (25%) with 2m 1:8 (12.5%) transition ramp Heavy vehicle: Max 1:6.5 (15.4%) with 1:16 (6.25%) in 7m of travel | Yes |
| 3.2.2 Driveway Width | Site 1A Category 3 (6m entry, 4-6m exit and a 1-3m median) Site 1B Category 3 (6m entry, 4-6m exit and a 1-3m median) Site 2A Category 2 (6-9m combined entry / exit) Site 2B, 2C and 2E Category 3 (6m entry, 4-6m exit and a 1-3m median) Site 2D Category 2 (6-9m combined entry / exit) Loading dock for all sites Confirmed via swept path analysis | Yes |
| 3.3 Gradients of Driveways | Maximum 1:20 for the first 6m within the property boundary. | Yes |
| 5.2 Column Location/Spacing | Columns and other obstructions kept clear of parking envelope (as depicted in Figure 5.2 of AS2890.1) | Yes |

| | | |
|---|--|-------------------|
| 5.3 Headroom | Passenger vehicles: Minimum 2.2m Heavy vehicles: 4.5m | Yes |
| Control | Proposed | Compliance |
| AS2890.6-2009 (Accessible Parking) | | |
| 2.2.1 Car Spaces Dimension | 2.4m x 5.4m adjacent to 2.4m wide shared area. | Yes |
| 2.4 Headroom | Minimum 2.5m | Yes |

As shown in the table above, the development car park and access design comply with the relevant Australian Standards.

5 Traffic Assessment

The impact of the proposed development on the surrounding road network was assessed using SIDRA Intersection modelling software. The traffic counts outlined in Section 2.3 and traffic generation estimated below in Section 5.1 were used to determine an overall traffic level for the area post-development. Section 5.2 describes how these additional trips were distributed amongst the critical intersections chosen for study.

Ultimately Section 5.3 outlines the SIDRA analysis undertaken which found that the increased traffic resulting from the proposed development will not have a significant impact on the efficiency of the surrounding road network.

5.1 Trip Generation

Currently the site is vacant and as such has no traffic generation associated with it. Trip generation rates for the proposal were determined using the *Guide to Traffic Generating Developments, Updated Traffic Surveys (TDT 2013/04a)* and the rates previously used by SCT Consulting in their Traffic and Transport Study for the Concept Approval for the site.

The *Updated Traffic Surveys* document states that the recommended trip generation rates were based on ten surveys conducted in 2012, eight within Sydney, and one each in the Hunter and Illawarra. All developments were close to public transport, greater than six storeys and almost exclusively residential in nature. Appendix B1 of the document also provides details of the locations where the surveys were conducted and, although located within Sydney, locations such as Strathfield and Pyrmont were classed as "Regional Areas" for the purposes of the trip rate assessment. Consequently, Tallawong would also be regarded as a regional area and the appropriate trip generation rates have been adopted for this assessment.

Due to the proximity of the site to rail and bus services, the primarily residential nature and scale of the development and the high standard and connectivity of pedestrian and cyclist facilities, it was determined that the average Regional residential trip rates were appropriate for this assessment.

The proposed estimated traffic volumes generated by the proposed development are outlined in the Table 6 using the following trip rate shown in Table 5:

Table 5: Trip generation rates

| Use | AM trip rates | PM trip rates |
|--------------------------------|---------------------------------|---------------------------------|
| Residential (Regional Average) | 0.53 trips per unit | 0.32 trips per unit |
| Retail | 1.94 per 100m ² | 2.7 trips per 100m ² |
| Commercial | 1.6 trips per 100m ² | 1.2 trips per 100m ² |

Table 6: Proposed development – traffic generation

| Land Use | Yield | AM Peak Hour Trips | In | Out | PM Peak Hour Trips | In | Out |
|--------------|----------------------|--------------------|-----|-----|--------------------|-----|-----|
| Residential | 987 units | 523 | 37 | 150 | 316 | 118 | 30 |
| Retail | 6,000 m ² | 117 | 58 | 59 | 162 | 81 | 81 |
| Commercial | 3,000 m ² | 48 | 29 | 19 | 36 | 14 | 22 |
| Total | - | 688 | 124 | 228 | 514 | 213 | 133 |

The retail trip rates used in Table 5 are considered satisfactory given the proximity of the site to Tallawong Station and the reduction in parking to be provided in the retail component of the site compared to the RMS 'Guide to Traffic Generating Development' rates. They were previously accepted/endorsed by the Department of Planning, Blacktown City Council, TfNSW and RMS as outlined in the Sydney Metro Northwest Places Program Tallawong Station Precinct South Submissions Report dated October 2018.

Note that when applying the retail area of 4,500 m² used by SCT Consulting in their analysis and the subject retail and commercial component of the proposal together with the reduction in the proposed number of residential units the overall trips generated by the development is less than that assessed as part of the Concept Approval.

The table above shows the proposed development generates approximately 688 trips during the AM peak hour and 514 trips during the PM peak hour.

5.2 Trip Distribution

This section outlines the methodology used to distribute the trips generated by the proposed development. The in / out distribution for the proposal was as follows:

- | | |
|---------------|--|
| • Residential | 20% in, 80% out (AM), 80% in, 20% out (PM) |
| • Retail | 50% in, 50% out (AM and PM) |
| • Commercial | 60% in, 40 %out (AM), 40%in, 60% out (PM) |

Trips were assigned based on a distribution of 10% each to the north and west, and 40% each to the east and south, based on the employment location of resident workers from the 2016 census which indicated that employment locations were largely located in the greater Sydney area.

Access to and from all basement carpark will be via Conferta Avenue, however, as traffic movements at the Conferta Avenue / Cudgegong Road and the Conferta Avenue / Tallawong Road intersections are restricted to left in / left out only several trips will need to be made via Themeda Avenue.

The following diagrams provide summaries of the routes that vehicles will use to enter and exit between the site and the surrounding road network.

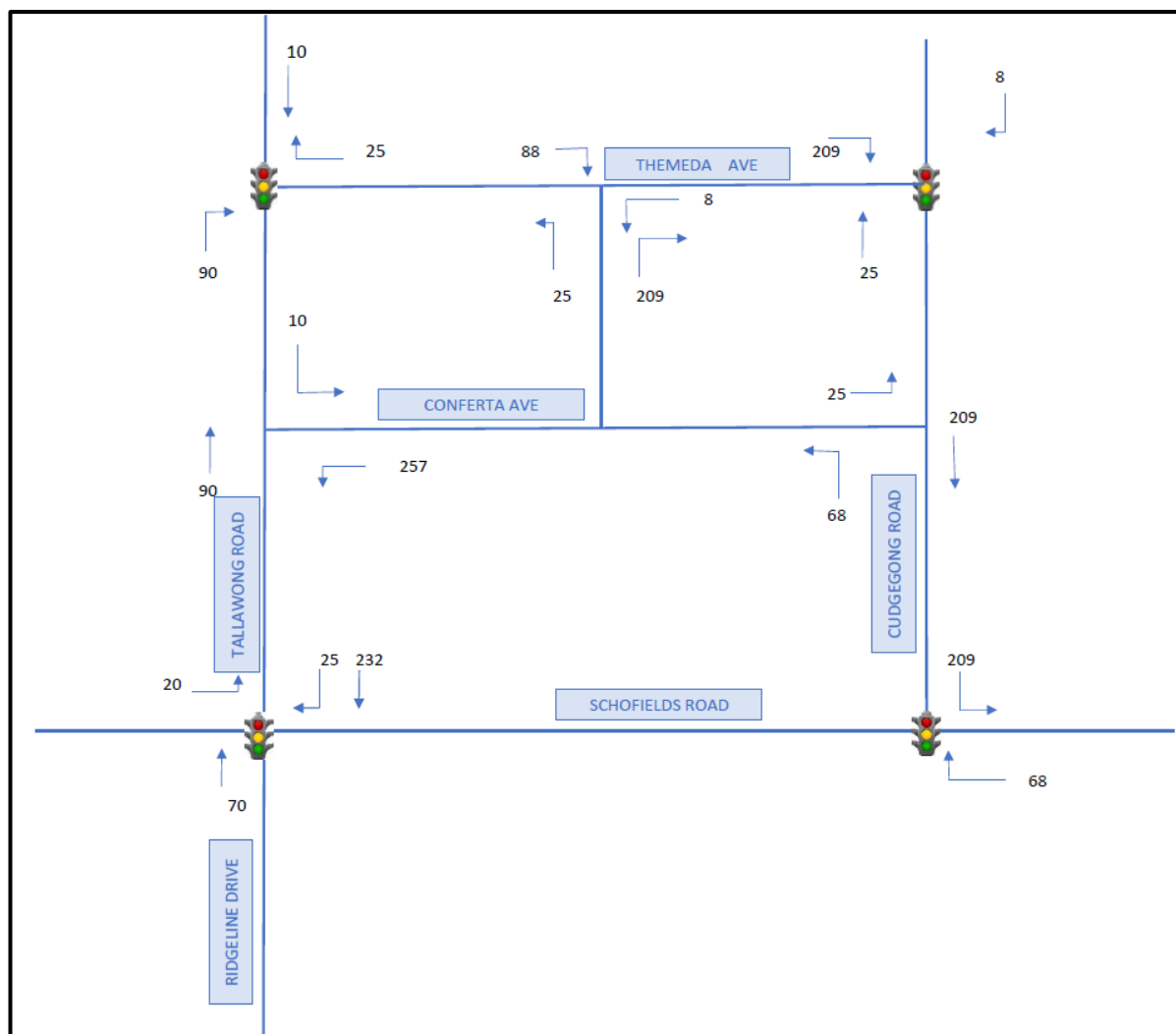


Figure 18: AM Trip Assignment

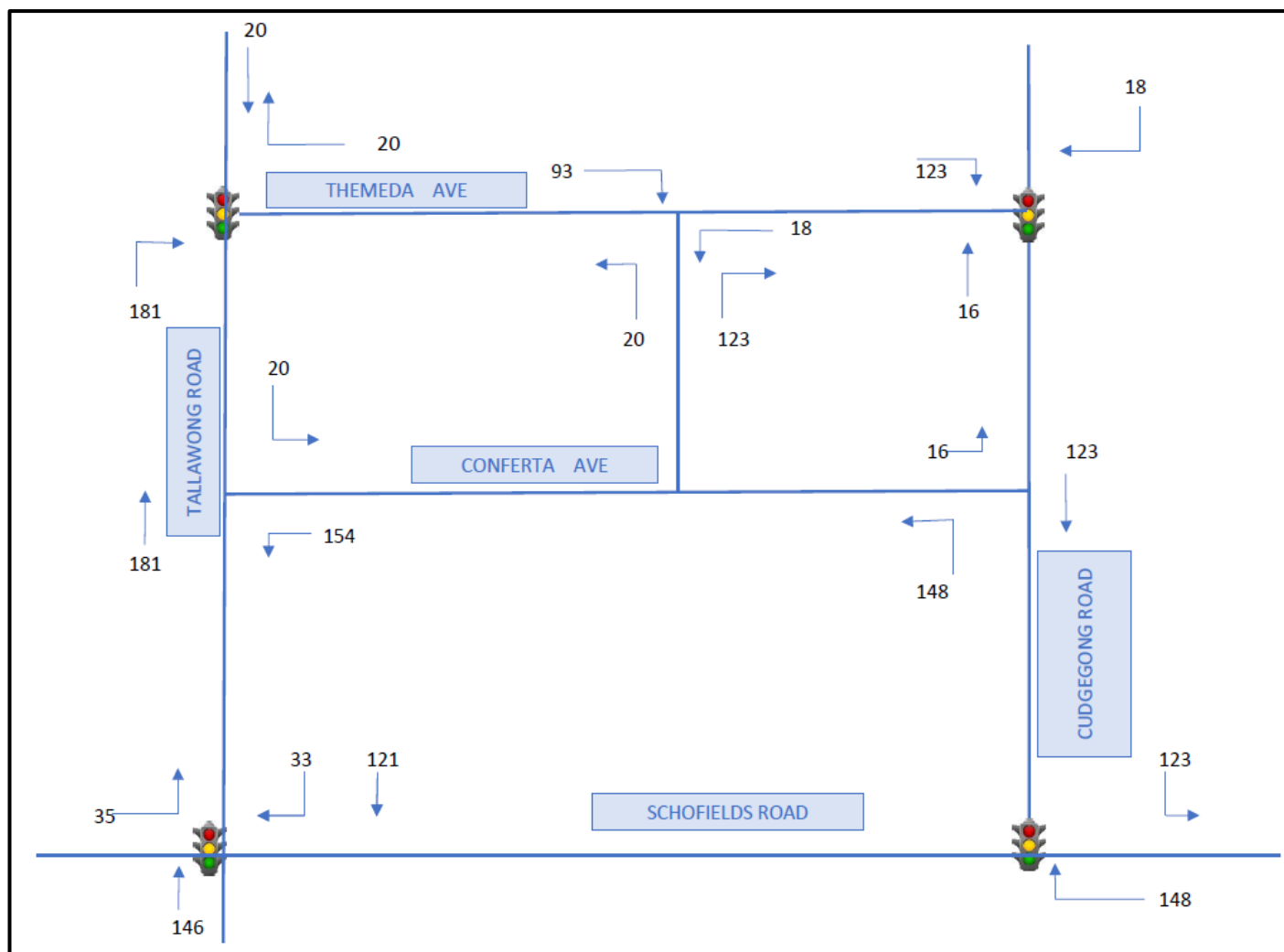


Figure 19: PM Trip Assignment

5.3 SIDRA Analysis and Impact of Generated Traffic

Intersection performance has been assessed using the SIDRA modelling software which uses the level of service (delay) model adopted by Transport for NSW to assess intersection performance.

Average delay is used to determine the level of service (LOS), which ranges from 'A' (excellent level of service) to 'F', with a LOS of 'D' being the minimum acceptable performance. The intersections outlined at the start of Section 2.3 have been assessed as a network for the existing and 10-year growth traffic volumes for AM and PM peak periods.

To assess the expected growth rate over the next 10 years, reference was made to the Transport for NSW vehicle counting station (ID No. 71024) located on Windsor Road between Merriville Road and Sanctuary Drive, Rouse Hill. The traffic volumes recorded at this station increased from an average daily volume of 47,589 vehicles per day to 53,958 vehicles per day, an increase of 6,369 or 13.4% over 8 years (1.67% per annum).

In order to ensure a robust and conservative assessment of the impact of the development on the road network, a growth rate of 2% per annum was applied to the traffic volumes at the surveyed intersections for the projected 10-year growth scenarios.



Figure 20: Intersections analysed using SIDRA

The differences in performance between these two scenarios are summarised in the tables below, with the full movement summary attached at **Appendix F**.

Table 7: Cudgegong Road / Schofields Road SIDRA Modelling Summary

| Cudgegong Road/ Schofields Road | | Existing Scenario | | 10-year growth scenario | |
|------------------------------------|-----------|---------------------|----------------------------|-------------------------|----------------------------|
| | | Existing Conditions | Post Development Condition | Existing Conditions | Post Development Condition |
| AM | Delay (s) | 17.3 | 17.7 | 26.3 | 23.1 *(25.6) |
| | LOS | B | B | B | B *(B) |
| PM | Delay (s) | 16.0 | 19.6 | 16.8 | 23.5 *(26.0) |
| | LOS | B | B | B | B *(B) |

Table 8: Tallawong Road / Schofields Road / Ridgeline Drive SIDRA Modelling Summary

| Tallawong Road/ Schofields Road / Ridgeline Dr | | Existing Scenario | | 10-year growth scenario | |
|--|-----------|---------------------|----------------------------|-------------------------|----------------------------|
| | | Existing Conditions | Post Development Condition | Existing Conditions | Post Development Condition |
| AM | Delay (s) | 27.7 | 36.8 | 31.7 | 47.0*(53.4) |
| | LOS | B | C | C | D *(D) |
| PM | Delay (s) | 28.2 | 32.2 | 36.4 | 36.7 *(46.5) |
| | LOS | C | C | C | C *(D) |

Table 9: Themeda Avenue/ Cudgegong Road SIDRA Modelling Summary

| Themeda Ave/ Cudgegong Road | | Existing Scenario | | 10-year growth scenario | |
|--------------------------------|-----------|---------------------|----------------------------|-------------------------|----------------------------|
| | | Existing Conditions | Post Development Condition | Existing Conditions | Post Development Condition |
| AM | Delay (s) | 14.1 | 20.9 | 15.4 | 21.2 *(34.1) |
| | LOS | A | B | B | B *(C) |
| PM | Delay (s) | 12.8 | 17.5 | 13.4 | 17.5 *(29.5) |
| | LOS | A | B | A | B *(C) |

Table 10: Themeda Avenue / Tallawong Road SIDRA Modelling Summary

| Themeda Ave/ Tallawong Road | | Existing Scenario | | 10-year growth scenario | |
|--------------------------------|-----------|---------------------|----------------------------|-------------------------|----------------------------|
| | | Existing Conditions | Post Development Condition | Existing Conditions | Post Development Condition |
| AM | Delay (s) | 17.5 | 23.0 | 18.7 | 24.5 *(37.6) |
| | LOS | B | B | B | B *(C) |
| PM | Delay (s) | 21.5 | 25.9 | 24.5 | 26.6 *(39.0) |
| | LOS | B | B | B | B *(C) |

* The Sidra model for this assessment adopted a network cycle time of 80 seconds based on this being the optimum cycle time to minimise delays at each intersection. However, feedback from Transport for NSW indicated that a cycle time of 140 seconds should be adopted in the modelling. Consequently, additional modelling was conducted for the critical "Post-development / 10-year Growth Scenario" based on a user-given cycle time of 140 seconds. The results are shown in brackets in the tables above indicating that the network will operate within acceptable parameters on a cycle time of 140 seconds, however, average delays will increase at some of the intersections, thereby reducing levels from B to C and C to D which is still well within accepted levels.

These results therefore indicate that the optimum cycle time for this network is 80 seconds as adopted by the original modelling.

In summary, the intersections surrounding the site currently operate at a high level of service with acceptable delays and will continue to operate satisfactorily with spare capacity based on the expected background growth in traffic volumes over the next 10 years and the additional traffic that will be generated by the proposed development. The development traffic will not have any significant impacts on level of service and delays currently experienced within the road network. Consequently, the development can be supported based on traffic grounds without the warrant to upgrade the existing local road network.

The SIDRA analysis and our conclusions are in line with the analysis undertaken in the Traffic and Transport Study prepared for the concept proposal for this development by SCT Consulting, dated 18 May 2018.

5.4 Impact of Construction Traffic

The construction phase of the development will require the delivery of machinery, equipment and materials to the site by a range of heavy vehicles up to a 19 metre articulated vehicle. Access to and from the site will be restricted primarily to the major roads in the area, Schofields Road which has links to the wider state road network at Windsor Road in the east and Richmond Road in the west. The only local roads that will be used for access to and from the site will be Cudegong Road and Tallawong Road which have direct connections with the Schofields Road via signalized intersections.

The management of construction traffic on the surrounding road network is detailed in a Construction Pedestrian and Traffic Management Plan prepared for this development by Barker Ryan Stewart.

5.5 Impact on Road Safety

The additional traffic volumes that will be generated by the proposed development is not expected to have any impact on road safety on the surrounding road network.

The main intersections providing access to and from the site are all controlled by traffic signals with pedestrian crossing facilities on all approaches. The streets directly surrounding and through the site have been designed for low speed environments with raised crossings for pedestrians and cyclists.

A review of all existing pedestrian and cyclist infrastructure has identified a high level of safety and connectivity in all streets surrounding the site and the proposed internal pedestrian / cyclist infrastructure has been designed to connect seamlessly with this existing infrastructure. All shared paths will be identified with the required signage and line marking to minimise any potential pedestrian / cyclist conflicts.

Driveway widths have been designed to minimise crossing distances for pedestrians, while adequately providing for vehicle swept paths. Driveways will also be designed to provide appropriate sight lines for pedestrian safety and to provide the required sight distances to approaching traffic in accordance with AS/NZS 2890.1.

A Road Safety Audit for the Construction phase has been prepared by Barker Ryan Stewart.

6 Conclusion

This Traffic and Parking Impact Assessment has been prepared in accordance with the Planning Secretary's Environmental Assessment Requirements (SEARS), Blacktown Council's DCP and LEP, the Road and Maritime Services (RMS) 'Guide to Traffic Generating Developments' to accompany a Development Application to the Blacktown City Council for the development of a mixed-use development.

Parking requirements have been assessed in accordance with the approved concept parking rates, Blacktown City Council DCP, the RMS Guide, and proposed rates.

The proposed parking provision provides a balance between the minimum requirements of the RMS Guide, Blacktown DCP, the transit orientated development maximum rates and to alleviate the potential detrimental impact on the existing Sydney Metro commuter car park and existing road network which are already at capacity.

The proposed provision of 1,068 residential spaces for the 987 units is considered satisfactory to cater for the car parking requirements generated by the development.

The proposed provision of 300 spaces is considered satisfactory to cater for the retail and commercial car parking requirements generated by the development.

It is proposed to provide 1,086 bicycle spaces to the precinct which is considered more than sufficient to cater for the storage and facility needs of the residents, visitors, employees and customers.

Traffic surveys and modelling were undertaken on the relevant intersections as follows:

- Cudgegong Road/ Schofields Road
- Tallawong Road / Schofields Road
- Themeda Avenue/ Cudgegong Road
- Themeda Avenue/ Tallawong Road

The SIDRA analysis and our conclusions are in line with the analysis undertaken in the Traffic and Transport Study prepared by SCT Consulting, dated 18 May 2018. The development traffic would have only a minor impact on existing, delays and Level of Service along this traffic network. Accordingly, any network upgrades will be required due solely to the existing conditions and the general growth in traffic that will be expected to occur over future years and not as a result of the additional traffic that will be generated by the proposed development.

The proposed parking and loading facilities have been designed in accordance with the requirements of AS/NZS 2890.1 – *Off Street Car Parking*, AS 2890.2 – *Off-Street Commercial Vehicle Facilities* and AS/NZS 2890.6 – *Off-street Parking for People with Disabilities*. These facilities are also considered practical and safe ensuring that all traffic generated by the development can enter and exit the site in a forward direction.

The Traffic and Parking Impact Assessment concludes that the subject site is suitable for the proposed mixed-use development in relation to traffic impact, car parking provision and the provision for pedestrian and bicycle facilities. The development is considered to have negligible effect on the safety and operating outcome of the surrounding transport network.

7 References

Australian Standards, 'AS/NZS 2890.1:2004 Off-Street Car Parking'.

Australian Standards, 'AS 2890.2:2018 Off-Street Commercial Vehicle Facilities'.

Australian Standards, 'AS/NZS 2890.6:2002 Off-Street Parking for People with Disabilities'.

Blacktown city Council DCP.

NSW Department of Planning, 'SEPP (Infrastructure) 2007'.

Roads and Maritime Services, 'Guide to Traffic Generating Developments' Version 2.2 dated October 2002.

Roads and Maritime Services, 'Guide to Traffic Modelling' Version 1.0 dated February 2013.

Traffic and Transport Study prepared by SCT Consulting, dated 18 May 2018.

Sydney Metro - Tallawong Station Precinct South Submissions Report, dated October 2018.

Technical Memorandum – Tallawong Station SSDA – Post Exhibition Responses to Traffic and Parking Comments prepared by SCT Consulting.

Appendix A

Pedestrian and bicycle network

BLACKTOWN CITY COUNCIL

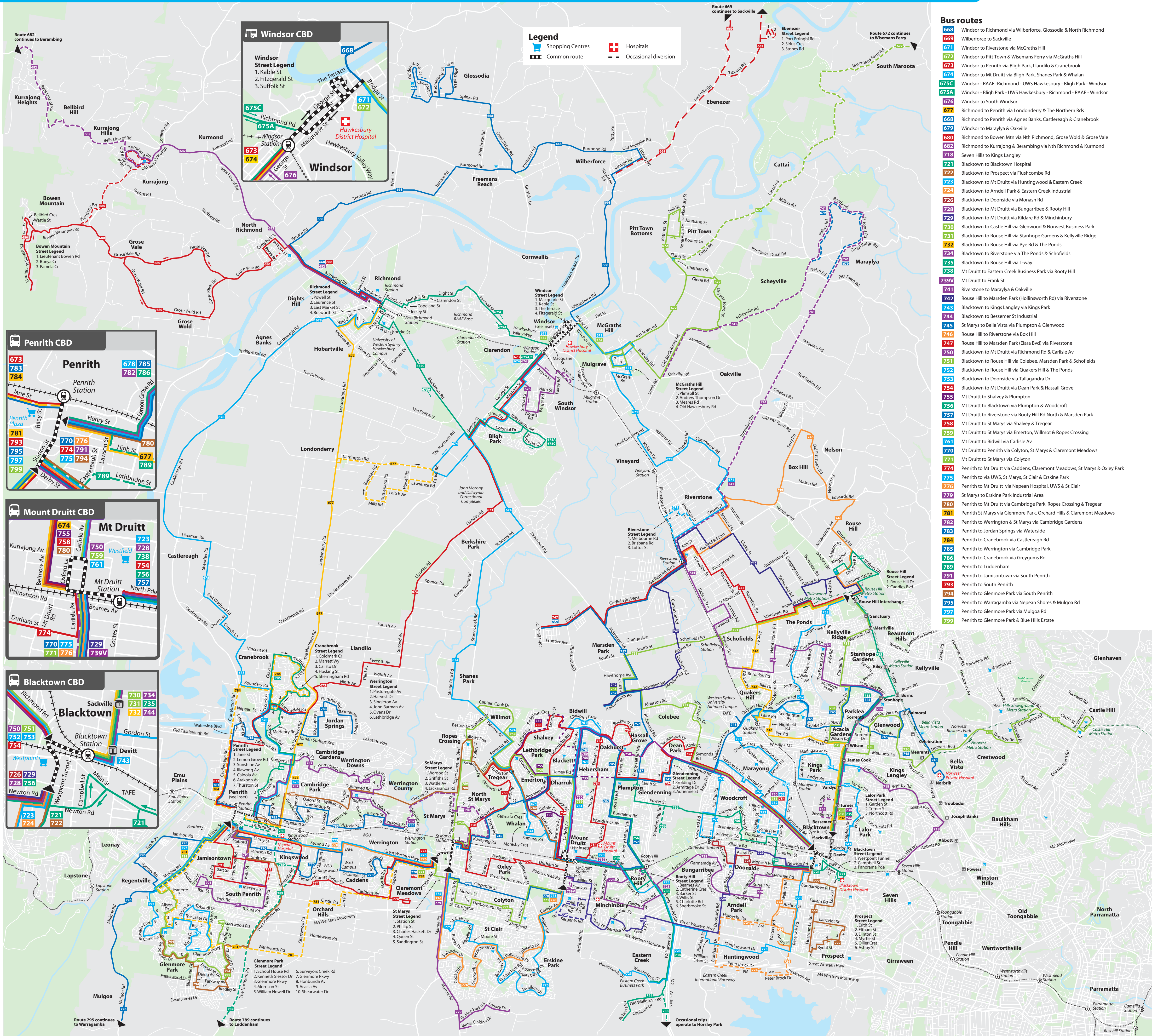
2016 BIKE PLAN

EXISTING & FUTURE PROPOSED ROUTES



Appendix B

Bus network



Appendix C

Sydney Train Map

Sydney rail network



M Metro **T** Trains



Sydney metro and train lines

M Metro North West Line
Chatswood
Tallawong

T1 North Shore & Western Line
North Shore
Western
Richmond

T2 Inner West & Leppington Line
Inner West
Leppington
City

T3 Bankstown Line
Liverpool
Lidcombe
City

T4 Eastern Suburbs & Illawarra Line
Eastern Suburbs
Illawarra
Cronulla

T5 Cumberland Line
Leppington
Richmond

T6 Carlingford Line
Carlingford
Clyde

T7 Olympic Park Line
Olympic Park
Lidcombe

T8 Airport & South Line
Airport
South
City

T9 Northern Line
Northern
Gordon

Check timetables and trip planners for train services and connections

Visit transportnsw.info

Appendix D

Swept Path Analysis

A1

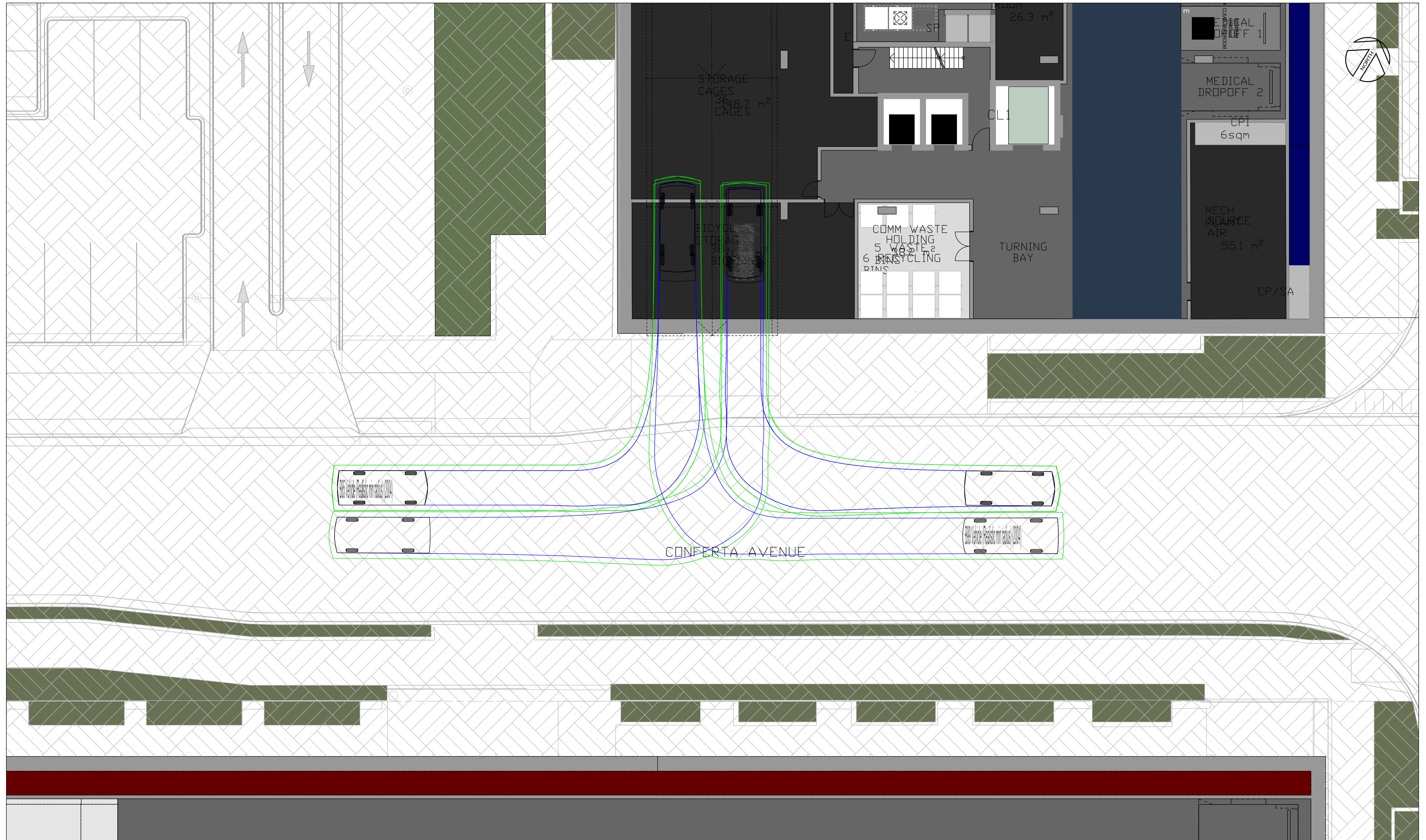



IMAGE SOURCED FROM NEARMAP AUSTRALIA PTY LTD

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| K | 16/04/2020 | ELEVENTH ISSUE | | | DEICORP | | | | | |
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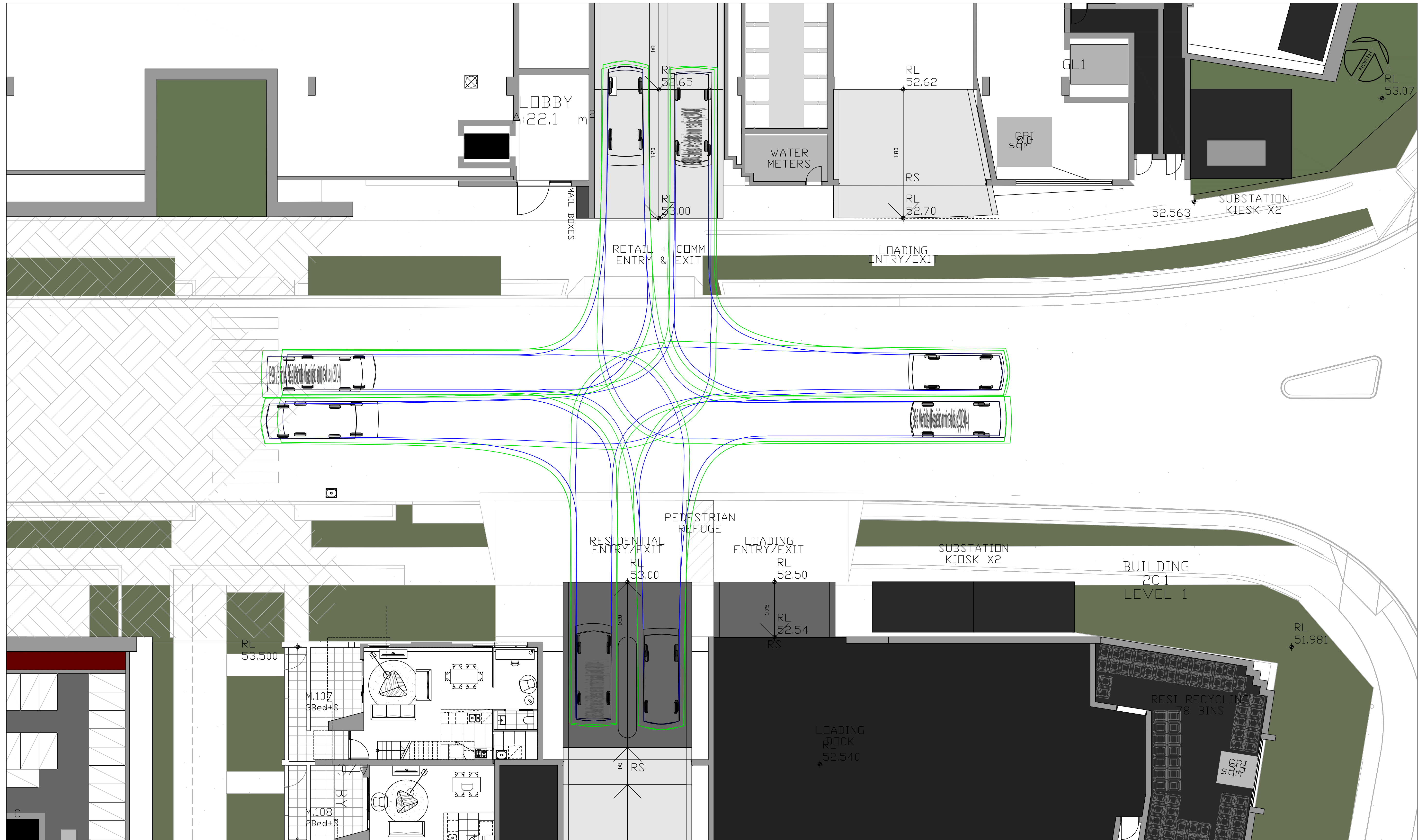


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| K | 16/04/2020 | ELEVENTH ISSUE | | DEICORP | Drawn: AAJ | | Horiz. | | |
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| No | DATE | AMENDMENT |  <div><div>BARKER RYAN STEWART</div><div>TOTAL PROJECT SOLUTIONS ENGINEERING PLANNING PROJECT MANAGEMENT SURVEYING CERTIFICATION</div></div> | <div><div>SYDNEY P: 02 9659 0005 CENTRAL COAST P: 02 4325 5255 HUNTER P: 02 4966 8388 ABN: 26 134 067 842 www.brs.com.au mail@brs.com.au</div><div>Client: DEICORP</div></div> | <div>TALLAWONG STATION PRECINCT SOUTH</div> <div>SITE 2A BASEMENT ACCESS SWEEP PATH ANALYSIS - B85 / B99</div> | Designed: AAJ | Scales: Plan | Plan No. |
| K | 16/04/2020 | ELEVENTH ISSUE | | | | Drawn: AAJ | Horiz. | SY190226TR03 |
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| | | | | | | | X-Sect. | REV. |
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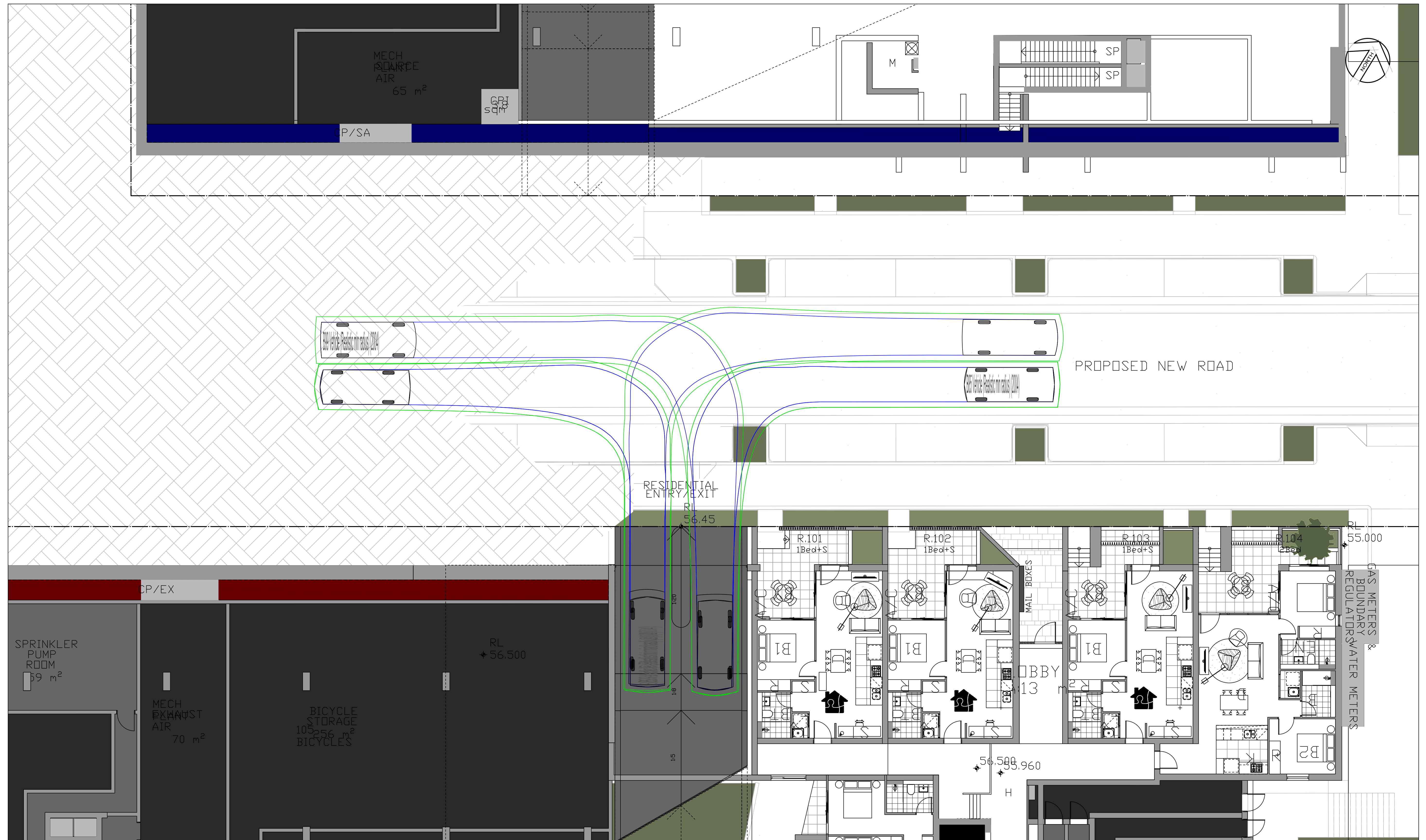



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| No | DATE | AMENDMENT |  <div><div>BARKER RYAN STEWART</div><div>TOTAL PROJECT SOLUTIONS ENGINEERING PLANNING PROJECT MANAGEMENT SURVEYING CERTIFICATION</div></div> | SYDNEY P: 02 3659 0005 CENTRAL COAST P: 02 4325 5255 HUNTER P: 02 4960 8388 A&N: 26 134 067 842 www.brs.com.au mail@brs.com.au | Client: | TALLAWONG STATION PRECINCT SOUTH SITE 2D BASEMENT ACCESS SWEEP PATH ANALYSIS - B99 / B85 | Designed: AAJ Drawn: AAJ Checked: AAJ | Scales: Plan Horiz. Vert. X-Sect. | Plan No. SY190226TR04 File Ref. SY190226D01K SHEET 4 OF 8 SHEETS | REV. K |
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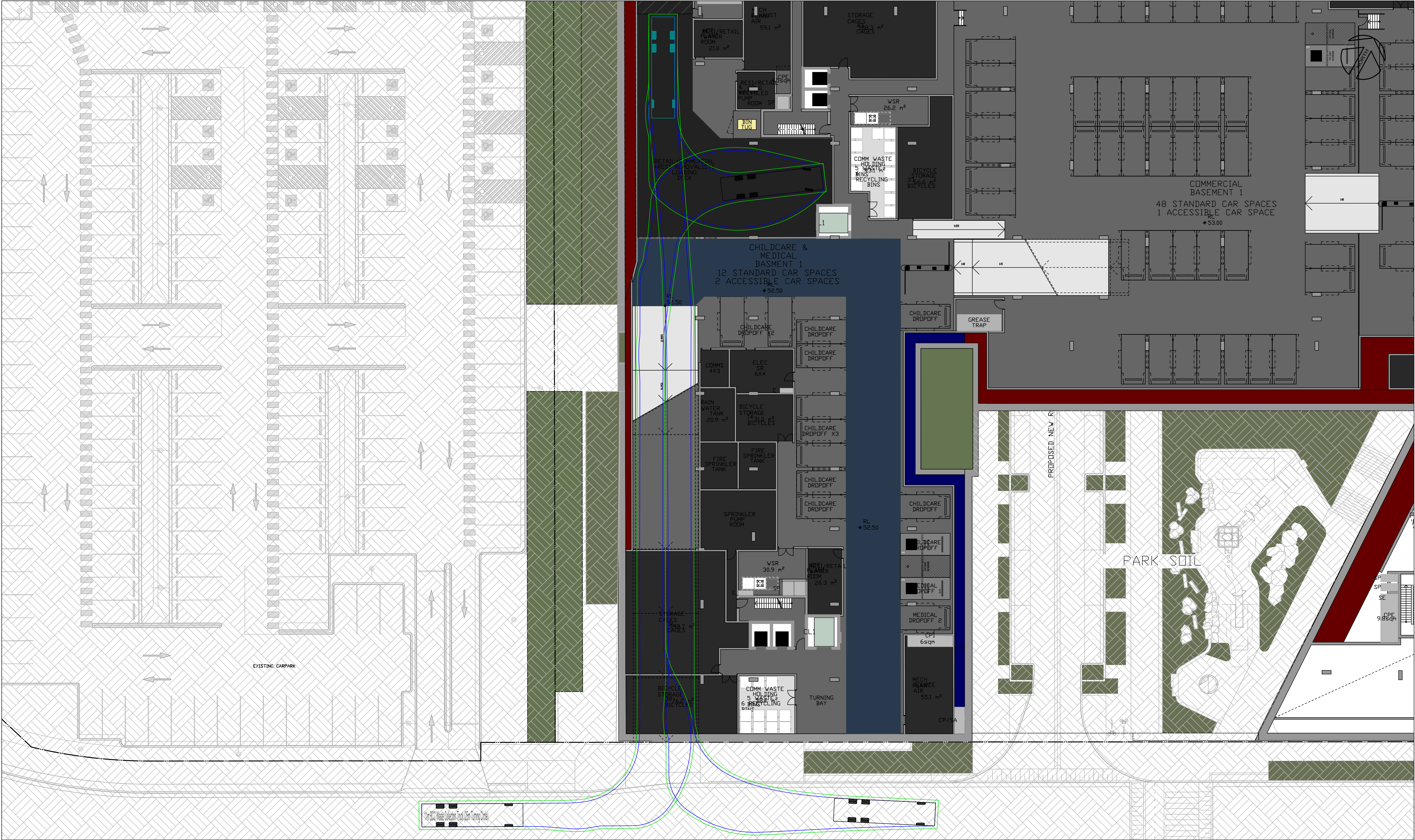


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HUNTER
P: 02 4966 8388
ABN: 26 134 067 842
www.brs.com.au
mail@brs.com.au

Client:

DEICORP

TALLAWONG STATION PRECINCT SOUTH

SITE 2A BASEMENT ACCESS SWEEP PATH ANALYSIS - 11M BCC WASTE COLLECTION VEHICLE

Designed: AAJ
Drawn: AAJ
Checked: AAJ

Scales: Plan
Horiz.
Vert.
X-Sect.

Datum: A.H.D.

Plan No.
SY190226TR05
File Ref.
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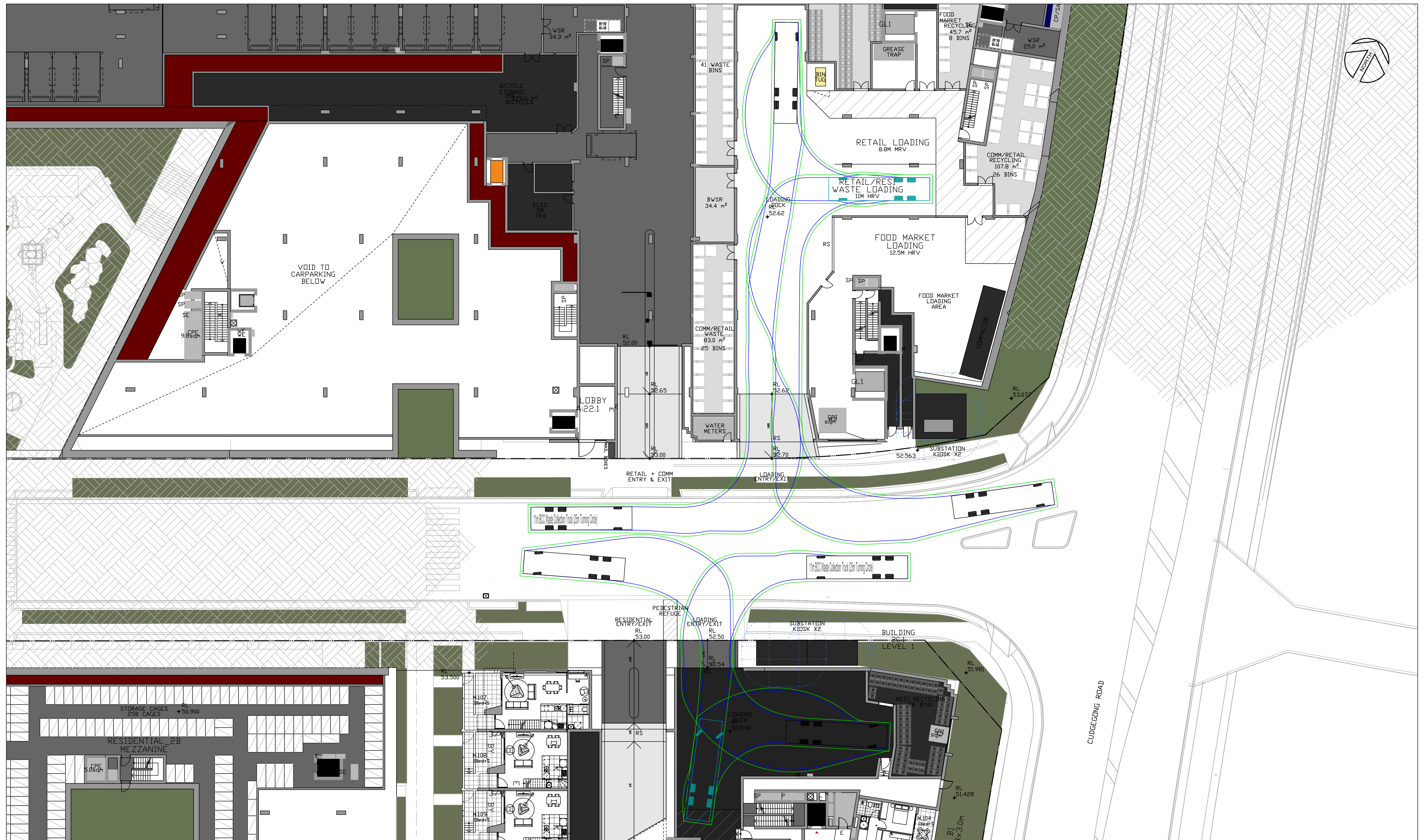



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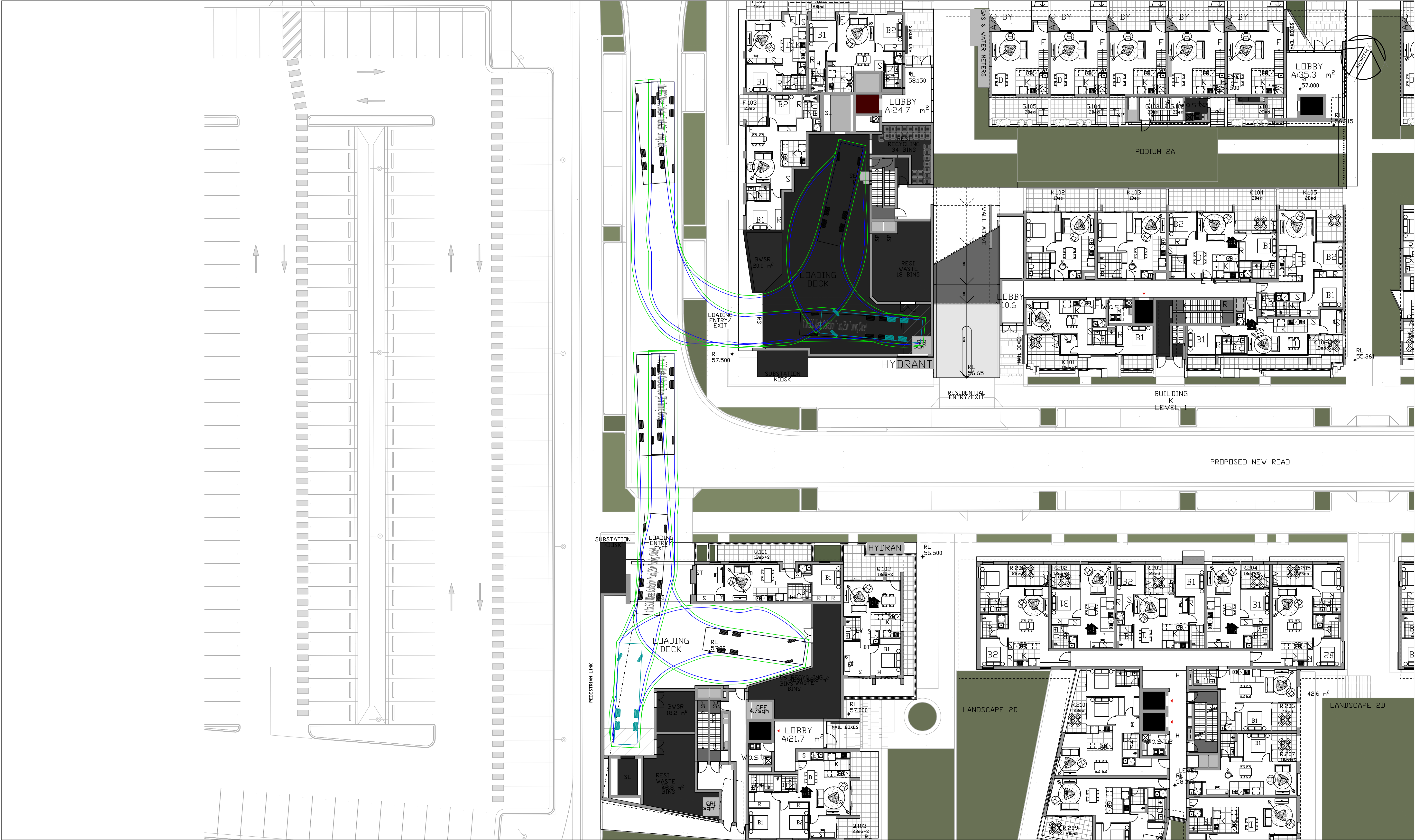



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| K | 16/04/2020 | ELEVENTH ISSUE | | | | SITE 2A AND 2D BASEMENT ACCESS SWEEP PATH ANALYSIS - 11M BCC WASTE COLLECTION | Drawn: AAJ | Horiz. | SY190226TR07 |
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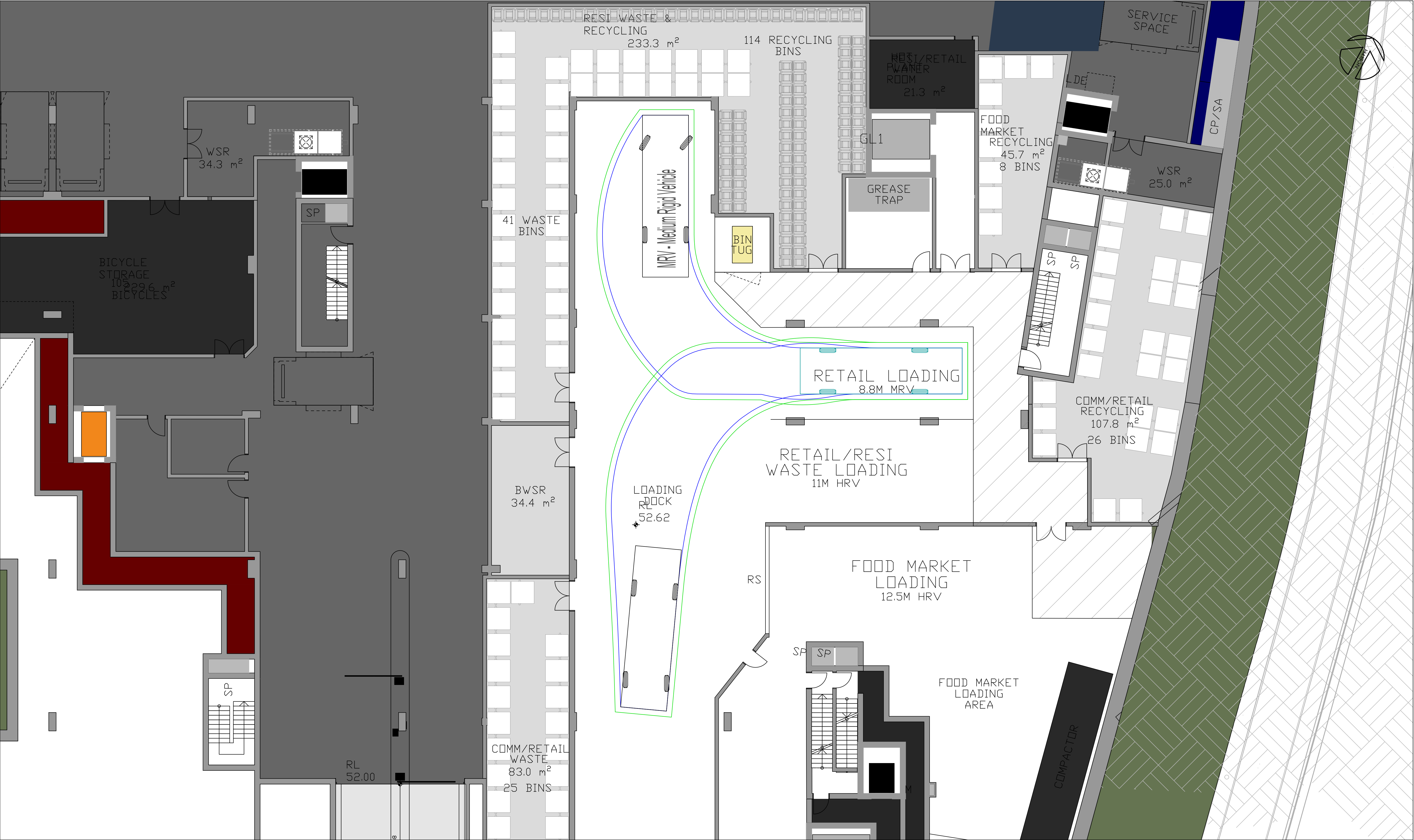


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HUNTER
P: 02 4966 8388
ABN: 26 134 067 842
www.brs.com.au
mail@brs.com.au

Client:

DEICORP

TALLAWONG STATION PRECINCT SOUTH

SITE 1B BASEMENT ACCESS SWEEP PATH ANALYSIS - 8.8M MRV

Designed: AAJ
Drawn: AAJ
Checked: AAJ

Scales: Plan
Horiz.
Vert.
X-Sect.

Datum: A.H.D.

Plan No.
SY190226TR08
File Ref.
SY190226D01K
SHEET 8 OF 8 SHEETS
REV.
K

Appendix E

Traffic Counts

Date #####
 Project Site Cudgegong Road/Themeda Avenue
 Project Number SY190226
 Client Combined Projects (tallawong) Pty. Ltd.

Cars

| Time | Western Approach (EB) | | | | Southern Approach (NB) | | | | Northern Approach (SB) | | | | 15 min Total | 1hr Total |
|------|-----------------------|----------|-------|--------|------------------------|----------|-------|--------|------------------------|----------|-------|--------|--------------|-----------|
| | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | | |
| 7:00 | 10 | | | 5 | | 4 | 41 | | | 13 | 20 | | 93 | |
| 7:15 | 9 | | | 19 | | 11 | 41 | | | 24 | 24 | | 128 | |
| 7:30 | 6 | | | 23 | | 5 | 29 | | | 26 | 10 | | 99 | |
| 7:45 | 4 | | | 39 | | 10 | 38 | | | 51 | 15 | | 157 | 477 |
| 8:00 | 6 | | | 42 | | 12 | 25 | | | 54 | 12 | | 151 | 535 |
| 8:15 | 9 | | | 37 | | 14 | 41 | | | 50 | 5 | | 156 | 563 |
| 8:30 | 7 | | | 45 | | 4 | 58 | | | 44 | 5 | | 163 | 627 |
| 8:45 | 14 | | | 38 | | | 72 | | | 72 | 2 | | 198 | 668 |

Trucks

| Time | Eastern Approach | | | | Western Approach | | | | Northern Approach | | | | Southern Approach | | | | 15 min Total | 1hr Total |
|------|------------------|----------|-------|--------|------------------|----------|-------|--------|-------------------|----------|-------|--------|-------------------|----------|-------|--------|--------------|-----------|
| | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | | |
| 7:00 | | | | 1 | | | | | | 5 | | | | 1 | | | 7 | |
| 7:15 | | | | | | | | | | 10 | | | | | | | 10 | |
| 7:30 | | | | 1 | | | | | | | | | | 3 | | | 4 | |
| 7:45 | 1 | | | | | | | | | 10 | | | | 5 | | | 16 | 37 |
| 8:00 | | | | | | | | | | 2 | | | | 7 | 1 | | 10 | 40 |
| 8:15 | | | | 1 | | | | | | 4 | | | | 6 | | | 11 | 41 |
| 8:30 | | | | | | | | | | 2 | | | | 3 | | | 5 | 42 |
| 8:45 | | | | | | | | | | 2 | | | | 4 | | | 6 | 32 |



BARKER
RYAN
STEWART

TOTAL PROJECT SOLUTIONS

| | |
|----------------|---|
| Date | 12-Mar-20 |
| Project Site | Cudgegong Road/Themeda Avenue |
| Project Number | SY190226 |
| Client | Combined Projects (tallawong) Pty. Ltd. |

Cars

| Time | Western Approach (EB) | | | | | | | | Southern Approach (NB) | | | | Northern Approach (SB) | | | | 15 min Total | 1hr Total |
|------|-----------------------|----------|-------|--------|------|----------|-------|--------|------------------------|----------|-------|--------|------------------------|----------|-------|--------|--------------|-----------|
| | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | | |
| 4:00 | 6 | | | 13 | | | | | | | 52 | | | | 90 | 6 | 167 | |
| 4:15 | 14 | | | 20 | | | | | 7 | 32 | | | | | 69 | 2 | 144 | |
| 4:30 | 6 | | | 16 | | | | | 11 | 31 | | | | | 45 | 3 | 112 | |
| 4:45 | 13 | | | 13 | | | | | 5 | 42 | | | | | 30 | 1 | 104 | 527 |
| 5:00 | 12 | | | 24 | | | | | 6 | 49 | | | | | 43 | 4 | 138 | 498 |
| 5:15 | 4 | | | 9 | | | | | 5 | 39 | | | | | 43 | 3 | 103 | 457 |
| 5:30 | 10 | | | 27 | | | | | 6 | 41 | | | | | 34 | 9 | 127 | 472 |
| 5:45 | 18 | | | 25 | | | | | 6 | 34 | | | | | 46 | 5 | 134 | 502 |
| | 39 | | | 73 | | | | | 22 | 171 | | | | | 150 | 17 | | |

Trucks

| Time | Eastern Approach | | | | Western Approach | | | | Northern Approach | | | | Southern Approach | | | | 15 min Total | 1hr Total |
|------|------------------|----------|-------|--------|------------------|----------|-------|--------|-------------------|----------|-------|--------|-------------------|----------|-------|--------|--------------|-----------|
| | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | | |
| 4:00 | | | | | | | | | | | | | | | | | 0 | |
| 4:15 | | | | | | | | | | | | | | | 3 | | 3 | |
| 4:30 | | | | | | | | | | | 4 | | | | 3 | | 7 | |
| 4:45 | | | | | | | | | | | 1 | | | | 3 | | 4 | 14 |
| 5:00 | | | | | | | | | | | | | | | 1 | | 1 | 15 |
| 5:15 | | | | | | | | | | | 1 | | | | 2 | | 3 | 15 |
| 5:30 | | | | | | | | | | | 1 | | | | 2 | | 3 | 11 |
| 5:45 | | | | | | | | | | | | | | | | | 0 | |

Date 5-Dec-19
 Project Site Cudgegong Road/Schofields Road
 Project Number SY190226
 Client Combined Projects (tallawong) Pty. Ltd.

Cars

| Time | Eastern Approach (WB) | | | | Western Approach (EB) | | | | Northern Approach (SB) | | | | Southern Approach (NB) | | | | 15 min Total | 1hr Total |
|------|-----------------------|----------|-------|--------|-----------------------|----------|-------|--------|------------------------|----------|-------|--------|------------------------|----------|-------|--------|--------------|-----------|
| | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | | |
| 7:00 | 2 | 232 | | | | 170 | 38 | | | | | | 13 | | 9 | | 464 | |
| 7:15 | 1 | 213 | | | | 156 | 30 | | | | | | 15 | | 28 | | 443 | |
| 7:30 | | 227 | | | | 211 | 29 | | | | | | 37 | | 33 | | 537 | |
| 7:45 | | 228 | | | | 178 | 29 | | | | | | 26 | | 24 | | 485 | 1929 |
| 8:00 | 2 | 256 | | | | 192 | 23 | | | | | | 41 | | 37 | | 551 | 2016 |
| 8:15 | 3 | 233 | | | | 188 | 21 | | | | | | 34 | | 26 | | 505 | 2078 |
| 8:30 | 4 | 249 | | | | 147 | 11 | | | | | | 23 | | 25 | | 459 | 2000 |
| 8:45 | 1 | 230 | | | | 174 | 16 | | | | | | 24 | | 21 | | 466 | 1981 |

Trucks

| Time | Eastern Approach | | | | Western Approach | | | | Northern Approach | | | | Southern Approach | | | | 15 min Total | 1hr Total |
|------|------------------|----------|-------|--------|------------------|----------|-------|--------|-------------------|----------|-------|--------|-------------------|----------|-------|--------|--------------|-----------|
| | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | | |
| 7:00 | | 20 | | | | 16 | 5 | | | | | | 2 | | 3 | | 46 | |
| 7:15 | | 24 | | | | 11 | 5 | | | | | | 5 | | 1 | | 46 | |
| 7:30 | | 14 | | | | 13 | 2 | | | | | | 5 | | 2 | | 36 | |
| 7:45 | | 26 | | | | 13 | | | | | | | 6 | | 4 | | 49 | 177 |
| 8:00 | 1 | 23 | | | | 15 | 1 | | | | | | 9 | | 8 | | 57 | 188 |
| 8:15 | | 18 | | | | 13 | 1 | | | | | | 7 | | 4 | | 43 | 185 |
| 8:30 | 1 | 19 | | | | 10 | 3 | | | | | | 7 | | 4 | | 44 | 193 |
| 8:45 | 1 | 16 | | | | 17 | 5 | | | | | | 5 | | 3 | | 47 | 191 |



TOTAL PROJECT SOLUTIONS

Date 5-Dec-19
 Project Site Cudgegong Road/Rouse Road
 Project Number SY190226
 Client Combined Projects (tallawong) Pty. Ltd.

Cars

| Time | Eastern Approach (WB) | | | | Western Approach (EB) | | | | Northern Approach (SB) | | | | Southern Approach (NB) | | | | 15 min Total | 1hr Total |
|------|-----------------------|----------|-------|--------|-----------------------|----------|-------|--------|------------------------|----------|-------|--------|------------------------|----------|-------|--------|--------------|-----------|
| | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | | |
| 4:00 | 4 | 204 | | | | 172 | 26 | | | | | | 16 | | 18 | | 440 | |
| 4:15 | 3 | 224 | | | | 176 | 18 | | | | | | 34 | | 26 | | 481 | |
| 4:30 | 3 | 256 | | | | 192 | 15 | | | | | | 41 | | 37 | | 544 | |
| 4:45 | 1 | 271 | | | | 236 | 22 | | | | | | 27 | | 25 | | 582 | 2047 |
| 5:00 | 2 | 263 | | | | 167 | 16 | | | | | | 31 | | 41 | | 520 | 2127 |
| 5:15 | 1 | 236 | | | | 212 | 31 | | | | | | 26 | | 28 | | 534 | 2180 |
| 5:30 | 1 | 299 | | | | 265 | 36 | | | | | | 23 | | 24 | | 648 | 2284 |
| 5:45 | 4 | 200 | | | | 162 | 11 | | | | | | 13 | | 17 | | 407 | 2109 |

Trucks

| Time | Eastern Approach | | | | Western Approach | | | | Northern Approach | | | | Southern Approach | | | | 15 min Total | 1hr Total |
|------|------------------|----------|-------|--------|------------------|----------|-------|--------|-------------------|----------|-------|--------|-------------------|----------|-------|--------|--------------|-----------|
| | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | | |
| 4:00 | 1 | 8 | | | | 10 | 3 | | | | | | 2 | | 1 | | 25 | |
| 4:15 | | 8 | | | | 3 | 1 | | | | | | 2 | | 4 | | 18 | |
| 4:30 | 1 | 23 | | | | 15 | 1 | | | | | | 9 | | 8 | | 57 | |
| 4:45 | | 7 | | | | 15 | 3 | | | | | | 4 | | 4 | | 33 | 133 |
| 5:00 | | 9 | | | | 9 | 1 | | | | | | 1 | | 2 | | 22 | 130 |
| 5:15 | | 14 | | | | 15 | 7 | | | | | | 3 | | | | 39 | 151 |
| 5:30 | | 11 | | | | 13 | 4 | | | | | | 4 | | 1 | | 33 | 127 |
| 5:45 | 1 | 7 | | | | 8 | 1 | | | | | | 1 | | | | 18 | 112 |

Date 5-Dec-19
 Project Site Schofields Road/Tallawong Road
 Project Number SY190226
 Client Combined Projects (tallawong) Pty. Ltd.

Cars

| Time | Eastern Approach (WB) | | | | Western Approach (EB) | | | | Northern Approach (SB) | | | | Southern Approach (NB) | | | | 15 min Total | 1hr Total |
|------|-----------------------|----------|-------|--------|-----------------------|----------|-------|--------|------------------------|----------|-------|--------|------------------------|----------|-------|--------|--------------|-----------|
| | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | | |
| 7:00 | 3 | 152 | 4 | | 29 | 163 | 12 | | 8 | 24 | 10 | | 9 | 53 | 14 | | 481 | |
| 7:15 | 14 | 171 | 2 | | 33 | 159 | 9 | | 10 | 28 | 24 | | 24 | 62 | 18 | | 554 | |
| 7:30 | 13 | 188 | 14 | | 26 | 195 | 31 | | 9 | 42 | 23 | | 22 | 42 | 33 | | 638 | |
| 7:45 | 7 | 206 | 15 | | 7 | 195 | 28 | | 4 | 52 | 67 | | 45 | 41 | 38 | | 705 | 2378 |
| 8:00 | 1 | 151 | 22 | | 6 | 144 | 42 | | 9 | 24 | 14 | | 19 | 28 | 32 | | 492 | 2389 |
| 8:15 | 2 | 143 | 13 | | 7 | 184 | 15 | | 5 | 12 | 7 | | 30 | 22 | 41 | | 481 | 2316 |
| 8:30 | 7 | 114 | 19 | | 3 | 127 | 20 | | 2 | 17 | 11 | | 14 | 9 | 21 | | 364 | 2042 |
| 8:45 | 5 | 117 | 7 | | 17 | 148 | 14 | | 17 | 25 | 18 | | 25 | 35 | 28 | | 456 | 1793 |

Trucks

| Time | Eastern Approach | | | | Western Approach | | | | Northern Approach | | | | Southern Approach | | | | 15 min Total | 1hr Total |
|------|------------------|----------|-------|--------|------------------|----------|-------|--------|-------------------|----------|-------|--------|-------------------|----------|-------|--------|--------------|-----------|
| | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | | |
| 7:00 | 1 | 7 | 0 | | 2 | 15 | | | 1 | 0 | 3 | | 1 | 3 | 3 | | 36 | |
| 7:15 | 3 | 14 | 3 | | 1 | 20 | 5 | | 1 | 3 | 6 | | 3 | 1 | | | 60 | |
| 7:30 | | 15 | 1 | | 5 | 22 | | | 1 | 4 | 1 | | 2 | 3 | 1 | | 55 | |
| 7:45 | | 17 | 4 | | 1 | 15 | 4 | | 1 | 2 | 6 | | | 3 | 3 | | 56 | 207 |
| 8:00 | 1 | 17 | 3 | | 1 | 14 | 3 | | 1 | 17 | 3 | | 1 | 1 | 1 | | 63 | 234 |
| 8:15 | | 6 | 2 | | 2 | 19 | 6 | | | 1 | 3 | | 2 | 1 | 3 | | 45 | 219 |
| 8:30 | 1 | 19 | 3 | | 1 | 12 | 2 | | | 3 | 4 | | 3 | 1 | 1 | | 50 | 214 |
| 8:45 | 1 | 9 | 3 | | 3 | 4 | | | 2 | 4 | 4 | | 1 | 1 | 1 | | 33 | 191 |



| | |
|----------------|---|
| Date | 5-Dec-19 |
| Project Site | Tallawong Road/Cudgegong Road |
| Project Number | SY190226 |
| Client | Combined Projects (tallawong) Pty. Ltd. |

Cars

[illegible]

Trucks

[illegible]

Date 12-Mar-20
 Project Site Tallawong Road/Themeda Av
 Project Number SY190226
 Client Combined Projects (tallawong) Pty. Ltd.

Cars

| Time | Eastern Approach (WB) | | | | Western Approach (EB) | | | | Northern Approach (SB) | | | | Southern Approach (NB) | | | | 15 min Total | 1hr Total |
|------|-----------------------|----------|-------|--------|-----------------------|----------|-------|--------|------------------------|----------|-------|--------|------------------------|----------|-------|--------|--------------|-----------|
| | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | | |
| 7:00 | | | | | 23 | | | 2 | | 38 | 140 | | 19 | 36 | | | 258 | |
| 7:15 | | | | | 25 | | | 3 | | 42 | 167 | | 12 | 47 | | | 296 | |
| 7:30 | | | | | 43 | | | 2 | | 41 | 124 | | 7 | 59 | | | 276 | |
| 7:45 | | | | | 29 | | | 5 | | 43 | 94 | | 14 | 56 | | | 241 | 1071 |
| 8:00 | | | | | 38 | | | 1 | | 44 | 92 | | 4 | 40 | | | 219 | 1032 |
| 8:15 | | | | | 20 | | | 2 | | 62 | 85 | | 6 | 40 | | | 215 | 951 |
| 8:30 | | | | | 10 | | | 2 | | 51 | 43 | | 5 | 40 | | | 151 | 826 |
| 8:45 | | | | | 15 | | | 2 | | 27 | 32 | | 3 | 36 | | | 115 | 700 |
| | | | | | 130 | | | 10 | | 190 | 395 | | 31 | 195 | | | | |

Trucks

| Time | Eastern Approach | | | | Western Approach | | | | Northern Approach | | | | Southern Approach | | | | 15 min Total | 1hr Total |
|------|------------------|----------|-------|--------|------------------|----------|-------|--------|-------------------|----------|-------|--------|-------------------|----------|-------|--------|--------------|-----------|
| | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | | |
| 7:00 | | | | | | | | | | 6 | | | | 5 | | | 11 | |
| 7:15 | | | | | | | | | | 4 | | | | 6 | | | 10 | |
| 7:30 | | | | | | | 1 | | | 4 | | | | 3 | | | 8 | |
| 7:45 | | | | | 1 | | | | | 2 | 1 | | | 3 | | | 7 | 36 |
| 8:00 | | | | | | | | | | 4 | | | | 3 | | | 7 | 32 |
| 8:15 | | | | | | | | | | 4 | 1 | | | 2 | | | 7 | 29 |
| 8:30 | | | | | | | | | | 4 | | | | 5 | | | 9 | 30 |
| 8:45 | | | | | | | | | | 5 | | | | | | | 5 | 28 |



BARKER
RYAN
STEWART

| | |
|----------------|---|
| Date | 5-Dec-19 |
| Project Site | Tallawong Road/Themeda Av |
| Project Number | SY190226 |
| Client | Combined Projects (tallawong) Pty. Ltd. |

Cars

| Time | Eastern Approach (WB) | | | | Western Approach (EB) | | | | Northern Approach (SB) | | | | Southern Approach (NB) | | | | 15 min Total | 1hr Total |
|------|-----------------------|----------|-------|--------|-----------------------|----------|-------|--------|------------------------|----------|-------|--------|------------------------|----------|-------|--------|--------------|-----------|
| | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | Left | Straight | Right | U-turn | | |
| 4:00 | | | | | 19 | | 5 | | 31 | 25 | | | 5 | 29 | | | 114 | |
| 4:15 | | | | | 7 | | 2 | | 34 | 38 | | | 4 | 31 | | | 116 | |
| 4:30 | | | | | 15 | | 6 | | 32 | 30 | | | 1 | 39 | | | 123 | |
| 4:45 | | | | | 17 | | 4 | | 31 | 35 | | | 3 | 41 | | | 131 | 484 |
| 5:00 | | | | | 27 | | 7 | | 31 | 42 | | | 3 | 44 | | | 154 | 524 |
| 5:15 | | | | | 15 | | 4 | | 31 | 41 | | | 6 | 26 | | | 123 | 531 |
| 5:30 | | | | | 27 | | 10 | | 43 | 44 | | | 3 | 35 | | | 162 | 570 |
| 5:45 | | | | | 28 | | 9 | | 33 | 58 | | | 2 | 30 | | | 160 | 599 |
| 6:00 | | | | | | | | | | | | | | | | | 0 | 445 |
| | | | | | 86 | | 25 | | 136 | 162 | | | 15 | 146 | | | | |

Trucks

[illegible]

Appendix F1
SIDRA Movement Summary
Existing

MOVEMENT SUMMARY

 Site: 101 [Cudgegong / Themeda Existing AM]

 Network: N101 [Tallawong Existing AM]

Cudgegong / Conferta Existing AM

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 60 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| South: Cudgegong Road | | | | | | | | | | | | | | |
| 1 | L2 | 43 | 0.0 | 43 | 0.0 | 0.151 | 19.1 | LOS B | 1.4 | 11.2 | 0.77 | 0.67 | 0.77 | 28.7 |
| 2 | T1 | 171 | 17.9 | 171 | 17.9 | 0.151 | 16.7 | LOS B | 1.8 | 13.7 | 0.88 | 0.73 | 0.88 | 31.3 |
| Approach | | 214 | 14.3 | 214 | 14.3 | 0.151 | 17.2 | LOS B | 1.8 | 13.7 | 0.86 | 0.72 | 0.86 | 30.8 |
| North: Cudgegong Road | | | | | | | | | | | | | | |
| 8 | T1 | 222 | 14.2 | 222 | 14.2 | 0.208 | 13.1 | LOS A | 1.8 | 13.9 | 0.70 | 0.59 | 0.70 | 16.4 |
| 9 | R2 | 45 | 2.3 | 45 | 2.3 | 0.208 | 18.4 | LOS B | 1.5 | 11.1 | 0.71 | 0.64 | 0.71 | 14.9 |
| Approach | | 267 | 12.2 | 267 | 12.2 | 0.208 | 14.0 | LOS A | 1.8 | 13.9 | 0.70 | 0.60 | 0.70 | 16.1 |
| West: Themeda Ave | | | | | | | | | | | | | | |
| 10 | L2 | 31 | 13.8 | 31 | 13.8 | 0.045 | 11.3 | LOS A | 0.2 | 1.5 | 0.36 | 0.60 | 0.36 | 37.7 |
| 12 | R2 | 155 | 4.1 | 155 | 4.1 | 0.214 | 10.3 | LOS A | 0.9 | 6.5 | 0.33 | 0.62 | 0.33 | 38.6 |
| Approach | | 185 | 5.7 | 185 | 5.7 | 0.214 | 10.5 | LOS A | 0.9 | 6.5 | 0.34 | 0.62 | 0.34 | 38.4 |
| All Vehicles | | 666 | 11.1 | 666 | 11.1 | 0.214 | 14.1 | LOS A | 1.8 | 13.9 | 0.65 | 0.64 | 0.65 | 30.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|----------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian | Prop. Queued | Effective Stop Rate | | |
| | | | | | ped | m | | | |
| P1 | South Full Crossing | 11 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 | |
| P3 | North Full Crossing | 13 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 | |
| P4 | West Full Crossing | 21 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 | |
| All Pedestrians | | 44 | 24.3 | LOS C | | | 0.90 | 0.90 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: CT [Cudgegong / Themeda Existing PM]

 Network: N101 [Tallawong Existing PM]

Cudgegong / Conferta Existing PM

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 60 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------------|------------------|-------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Flows Total | Arrival Flows HV | Flows Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| South: Cudgegong Road | | | | | | | | | | | | | | |
| 1 | L2 | 23 | 0.0 | 23 | 0.0 | 0.141 | 17.8 | LOS B | 1.3 | 10.9 | 0.74 | 0.63 | 0.74 | 31.4 |
| 2 | T1 | 201 | 10.5 | 201 | 10.5 | 0.141 | 15.2 | LOS B | 1.9 | 13.5 | 0.86 | 0.70 | 0.86 | 33.0 |
| Approach | | 224 | 9.4 | 224 | 9.4 | 0.141 | 15.4 | LOS B | 1.9 | 13.5 | 0.85 | 0.69 | 0.85 | 32.9 |
| North: Cudgegong Road | | | | | | | | | | | | | | |
| 8 | T1 | 175 | 9.6 | 175 | 9.6 | 0.129 | 11.1 | LOS A | 1.1 | 8.4 | 0.63 | 0.52 | 0.63 | 18.7 |
| 9 | R2 | 18 | 0.0 | 18 | 0.0 | 0.129 | 15.8 | LOS B | 1.0 | 7.3 | 0.63 | 0.54 | 0.63 | 18.0 |
| Approach | | 193 | 8.7 | 193 | 8.7 | 0.129 | 11.5 | LOS A | 1.1 | 8.4 | 0.63 | 0.52 | 0.63 | 18.6 |
| West: Themeda Ave | | | | | | | | | | | | | | |
| 10 | L2 | 41 | 0.0 | 41 | 0.0 | 0.060 | 9.5 | LOS A | 0.2 | 1.3 | 0.26 | 0.59 | 0.26 | 39.0 |
| 12 | R2 | 82 | 6.4 | 82 | 6.4 | 0.126 | 10.1 | LOS A | 0.4 | 3.1 | 0.29 | 0.60 | 0.29 | 38.8 |
| Approach | | 123 | 4.3 | 123 | 4.3 | 0.126 | 9.9 | LOS A | 0.4 | 3.1 | 0.28 | 0.60 | 0.28 | 38.9 |
| All Vehicles | | 540 | 8.0 | 540 | 8.0 | 0.141 | 12.8 | LOS A | 1.9 | 13.5 | 0.64 | 0.61 | 0.64 | 32.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|----------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian | Prop. Queued | Effective Stop Rate | | |
| | | | | | ped | m | | | |
| P1 | South Full Crossing | 14 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 | |
| P3 | North Full Crossing | 9 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 | |
| P4 | West Full Crossing | 16 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 | |
| All Pedestrians | | 39 | 24.3 | LOS C | | | 0.90 | 0.90 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: SC [SCHOFIELDS / CUDGEGONG - AM]

 Network: N101 [Tallawong Existing AM]

SCHOFIELDS ROAD / CUDGEGONG ROAD

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 60 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|------|--------------------|------------------|-------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows Total | Arrival Flows HV | Flows Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h |
| East: SCHOFIELDS ROAD | | | | | | | | | | | | | |
| 5 | T1 | 874 | 7.3 | 874 | 7.3 | 0.484 | 11.9 | LOS A | 5.3 | 38.9 | 0.73 | 0.63 | 43.3 |
| 6 | R2 | 114 | 5.6 | 114 | 5.6 | 0.322 | 34.9 | LOS C | 1.0 | 7.5 | 0.97 | 0.74 | 28.3 |
| Approach | | 987 | 7.1 | 987 | 7.1 | 0.484 | 14.6 | LOS B | 5.3 | 38.9 | 0.76 | 0.65 | 40.8 |
| North: CUDGEGONG ROAD | | | | | | | | | | | | | |
| 7 | L2 | 174 | 16.4 | 174 | 16.4 | 0.313 | 26.8 | LOS B | 2.9 | 22.8 | 0.94 | 0.81 | 37.0 |
| 9 | R2 | 145 | 13.0 | 145 | 13.0 | 0.638 | 35.0 | LOS C | 2.7 | 21.2 | 1.00 | 0.84 | 18.3 |
| Approach | | 319 | 14.9 | 319 | 14.9 | 0.638 | 30.5 | LOS C | 2.9 | 22.8 | 0.97 | 0.82 | 30.2 |
| West: SCHOFIELDS ROAD | | | | | | | | | | | | | |
| 10 | L2 | 12 | 54.5 | 12 | 54.5 | 0.018 | 15.6 | LOS B | 0.1 | 1.3 | 0.61 | 0.63 | 38.0 |
| 11 | T1 | 1068 | 7.0 | 1068 | 7.0 | 0.614 | 15.9 | LOS B | 8.5 | 63.3 | 0.92 | 0.81 | 46.6 |
| Approach | | 1080 | 7.5 | 1080 | 7.5 | 0.614 | 15.9 | LOS B | 8.5 | 63.3 | 0.91 | 0.80 | 46.5 |
| All Vehicles | | 2386 | 8.3 | 2386 | 8.3 | 0.638 | 17.3 | LOS B | 8.5 | 63.3 | 0.86 | 0.74 | 42.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|----------------------------------|--------------|---------------------|------|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian | Prop. Queued | Effective Stop Rate | |
| | | | | | ped | m | | |
| P2 | East Full Crossing | 8 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| P3 | North Full Crossing | 16 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| P4 | West Full Crossing | 13 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| All Pedestrians | | 37 | 24.3 | LOS C | | | 0.90 | 0.90 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: SC [SCHOFIELDS / CUDGEGONG - PM]

 Network: N101 [Tallawong Existing PM]

SCHOFIELDS ROAD / CUDGEGONG ROAD

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 60 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h |
| East: SCHOFIELDS ROAD | | | | | | | | | | | | | |
| 5 | T1 | 947 | 2.2 | 947 | 2.2 | 0.508 | 11.5 | LOS A | 5.9 | 41.8 | 0.73 | 0.64 | 43.7 |
| 6 | R2 | 127 | 13.2 | 127 | 13.2 | 0.379 | 35.3 | LOS C | 1.2 | 9.1 | 0.97 | 0.75 | 28.1 |
| Approach | | 1075 | 3.5 | 1075 | 3.5 | 0.508 | 14.4 | LOS A | 5.9 | 41.8 | 0.76 | 0.65 | 41.0 |
| North: CUDGEGONG ROAD | | | | | | | | | | | | | |
| 7 | L2 | 132 | 14.4 | 132 | 14.4 | 0.247 | 26.6 | LOS B | 2.1 | 16.8 | 0.93 | 0.79 | 37.1 |
| 9 | R2 | 134 | 7.1 | 134 | 7.1 | 0.645 | 35.7 | LOS C | 2.5 | 18.5 | 0.99 | 0.83 | 18.0 |
| Approach | | 265 | 10.7 | 265 | 10.7 | 0.645 | 31.2 | LOS C | 2.5 | 18.5 | 0.96 | 0.81 | 29.2 |
| West: SCHOFIELDS ROAD | | | | | | | | | | | | | |
| 10 | L2 | 5 | 0.0 | 5 | 0.0 | 0.006 | 14.3 | LOS A | 0.1 | 0.4 | 0.59 | 0.61 | 38.6 |
| 11 | T1 | 1166 | 3.5 | 1166 | 3.5 | 0.633 | 14.2 | LOS A | 8.9 | 64.4 | 0.88 | 0.78 | 47.7 |
| Approach | | 1172 | 3.5 | 1172 | 3.5 | 0.633 | 14.2 | LOS A | 8.9 | 64.4 | 0.88 | 0.78 | 47.7 |
| All Vehicles | | 2512 | 4.3 | 2512 | 4.3 | 0.645 | 16.0 | LOS B | 8.9 | 64.4 | 0.84 | 0.73 | 43.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | |
|------------------------------------|---------------------|-------------|---------------|------------------|-----------------------|--------------|---------------------|------|
| Mov ID | Description | Demand Flow | Average Delay | Level of Service | Average Back of Queue | Prop. Queued | Effective Stop Rate | |
| | | ped/h | sec | | ped | m | | |
| P2 | East Full Crossing | 18 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| P3 | North Full Crossing | 21 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| P4 | West Full Crossing | 13 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| All Pedestrians | | 52 | 24.3 | LOS C | | | 0.90 | 0.90 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY



Site: 101 [SCHOFIELDS / TALLAWONG / RIDGELINE - AM]



Network: N101 [Tallawong Existing AM]

SCHOFIELDS / TALLAWONG / RIDGELINE

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 60 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|------|--------------|------|---------------|------|-----------|---------------|------------------|---------------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows | | Arrival Flows | | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | Total veh/h | HV % | Total veh/h | HV % | v/c | sec | | Vehicles veh | Distance m | | | km/h |
| South: RIDGELINE DRIVE | | | | | | | | | | | | | |
| 1 | L2 | 126 | 3.3 | 126 | 3.3 | 0.226 | 22.9 | LOS B | 1.7 | 12.5 | 0.79 | 0.75 | 42.7 |
| 2 | T1 | 143 | 2.2 | 143 | 2.2 | 0.737 | 32.3 | LOS C | 2.8 | 20.1 | 1.00 | 0.88 | 29.8 |
| 3 | R2 | 158 | 4.0 | 158 | 4.0 | 0.742 | 37.4 | LOS C | 3.1 | 22.4 | 1.00 | 0.90 | 27.5 |
| Approach | | 427 | 3.2 | 427 | 3.2 | 0.742 | 31.4 | LOS C | 3.1 | 22.4 | 0.94 | 0.85 | 33.5 |
| East: SCHOFIELDS ROAD | | | | | | | | | | | | | |
| 4 | L2 | 24 | 0.0 | 24 | 0.0 | 0.025 | 14.0 | LOS A | 0.2 | 1.7 | 0.59 | 0.66 | 47.3 |
| 5 | T1 | 772 | 6.1 | 772 | 6.1 | 0.722 | 26.2 | LOS B | 7.2 | 52.9 | 0.99 | 0.87 | 40.8 |
| 6 | R2 | 78 | 13.5 | 78 | 13.5 | 0.462 | 35.7 | LOS C | 1.5 | 11.4 | 1.00 | 0.76 | 25.1 |
| Approach | | 874 | 6.6 | 874 | 6.6 | 0.722 | 26.7 | LOS B | 7.2 | 52.9 | 0.98 | 0.85 | 39.8 |
| North: TALLAWONG ROAD | | | | | | | | | | | | | |
| 7 | L2 | 35 | 18.2 | 35 | 18.2 | 0.070 | 27.1 | LOS B | 0.6 | 4.7 | 0.95 | 0.74 | 18.2 |
| 8 | T1 | 146 | 6.5 | 146 | 6.5 | 0.774 | 34.8 | LOS C | 2.9 | 21.6 | 1.00 | 0.85 | 33.0 |
| 9 | R2 | 128 | 9.0 | 128 | 9.0 | 0.312 | 33.8 | LOS C | 1.1 | 8.5 | 0.95 | 0.75 | 33.0 |
| Approach | | 309 | 8.8 | 309 | 8.8 | 0.774 | 33.5 | LOS C | 2.9 | 21.6 | 0.98 | 0.80 | 32.1 |
| West: SCHOFIELDS ROAD | | | | | | | | | | | | | |
| 10 | L2 | 49 | 2.1 | 49 | 2.1 | 0.051 | 13.9 | LOS A | 0.5 | 3.2 | 0.54 | 0.66 | 41.7 |
| 11 | T1 | 808 | 6.5 | 808 | 6.5 | 0.752 | 23.9 | LOS B | 7.2 | 52.8 | 0.97 | 0.90 | 34.1 |
| 12 | R2 | 125 | 2.5 | 125 | 2.5 | 0.697 | 37.5 | LOS C | 2.4 | 17.5 | 1.00 | 0.86 | 36.8 |
| Approach | | 983 | 5.8 | 983 | 5.8 | 0.752 | 25.2 | LOS B | 7.2 | 52.8 | 0.95 | 0.88 | 34.9 |
| All Vehicles | | 2594 | 6.0 | 2594 | 6.0 | 0.774 | 27.7 | LOS B | 7.2 | 52.9 | 0.96 | 0.86 | 36.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | |
|------------------------------------|---------------------|-------------|---------------|------------------|-----------------------|--------------|---------------------|
| Mov ID | Description | Demand Flow | Average Delay | Level of Service | Average Back of Queue | Prop. Queued | Effective Stop Rate |
| | | ped/h | sec | | Pedestrian ped | Distance m | |
| P1 | South Full Crossing | 11 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 |
| P2 | East Full Crossing | 13 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 |
| P3 | North Full Crossing | 14 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 |
| P4 | West Full Crossing | 16 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 |
| All Pedestrians | | 53 | 24.3 | LOS C | | | 0.90 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY



Site: 101 [SCHOFIELDS / TALLAWONG / RIDGELINE - PM]

Network: N101 [Tallawong Existing PM]

SCHOFIELDS / TALLAWONG / RIDGELINE

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 60 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h |
| South: RIDGELINE DRIVE | | | | | | | | | | | | | |
| 1 | L2 | 84 | 3.8 | 84 | 3.8 | 0.151 | 22.5 | LOS B | 1.1 | 8.2 | 0.77 | 0.73 | 42.9 |
| 2 | T1 | 105 | 1.0 | 105 | 1.0 | 0.538 | 30.0 | LOS C | 2.0 | 13.8 | 0.99 | 0.78 | 30.9 |
| 3 | R2 | 127 | 0.8 | 127 | 0.8 | 0.683 | 37.2 | LOS C | 2.5 | 17.4 | 1.00 | 0.85 | 27.5 |
| Approach | | 317 | 1.7 | 317 | 1.7 | 0.683 | 30.9 | LOS C | 2.5 | 17.4 | 0.94 | 0.80 | 33.4 |
| East: SCHOFIELDS ROAD | | | | | | | | | | | | | |
| 4 | L2 | 46 | 9.1 | 46 | 9.1 | 0.038 | 8.2 | LOS A | 0.3 | 1.9 | 0.44 | 0.64 | 51.1 |
| 5 | T1 | 959 | 3.1 | 959 | 3.1 | 0.836 | 29.3 | LOS C | 9.5 | 67.8 | 1.00 | 0.95 | 39.2 |
| 6 | R2 | 38 | 0.0 | 38 | 0.0 | 0.205 | 34.3 | LOS C | 0.7 | 4.8 | 0.97 | 0.72 | 25.6 |
| Approach | | 1043 | 3.2 | 1043 | 3.2 | 0.836 | 28.6 | LOS C | 9.5 | 67.8 | 0.97 | 0.92 | 39.3 |
| North: TALLAWONG ROAD | | | | | | | | | | | | | |
| 7 | L2 | 49 | 8.5 | 49 | 8.5 | 0.093 | 26.3 | LOS B | 0.8 | 6.0 | 0.92 | 0.74 | 18.6 |
| 8 | T1 | 156 | 3.4 | 156 | 3.4 | 0.808 | 35.4 | LOS C | 3.2 | 22.8 | 1.00 | 0.91 | 32.8 |
| 9 | R2 | 255 | 6.2 | 255 | 6.2 | 0.709 | 37.4 | LOS C | 2.5 | 18.6 | 1.00 | 0.85 | 31.5 |
| Approach | | 460 | 5.5 | 460 | 5.5 | 0.808 | 35.6 | LOS C | 3.2 | 22.8 | 0.99 | 0.86 | 31.3 |
| West: SCHOFIELDS ROAD | | | | | | | | | | | | | |
| 10 | L2 | 20 | 42.1 | 20 | 42.1 | 0.018 | 7.3 | LOS A | 0.1 | 0.6 | 0.28 | 0.59 | 49.0 |
| 11 | T1 | 814 | 3.5 | 814 | 3.5 | 0.706 | 21.7 | LOS B | 6.9 | 49.2 | 0.94 | 0.85 | 35.5 |
| 12 | R2 | 117 | 6.3 | 117 | 6.3 | 0.668 | 37.2 | LOS C | 2.3 | 16.7 | 1.00 | 0.85 | 36.9 |
| Approach | | 951 | 4.7 | 951 | 4.7 | 0.706 | 23.3 | LOS B | 6.9 | 49.2 | 0.94 | 0.84 | 36.0 |
| All Vehicles | | 2771 | 3.9 | 2771 | 3.9 | 0.836 | 28.2 | LOS B | 9.5 | 67.8 | 0.96 | 0.87 | 36.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | |
| P1 | South Full Crossing | 13 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| P2 | East Full Crossing | 18 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| P3 | North Full Crossing | 21 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| P4 | West Full Crossing | 15 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| All Pedestrians | | 66 | 24.3 | LOS C | | | 0.90 | 0.90 |

MOVEMENT SUMMARY

 Site: 101 [Tallawong / Themeda Existing AM]

 Network: N101 [Tallawong Existing AM]

Tallawong / Themeda Existing AM

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 60 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|------|--------------------|------------------|-------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows Total | Arrival Flows HV | Flows Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h |
| South: Tallawong Road | | | | | | | | | | | | | |
| 2 | T1 | 227 | 9.7 | 227 | 9.7 | 0.106 | 8.2 | LOS A | 1.6 | 11.9 | 0.63 | 0.51 | 39.7 |
| 3 | R2 | 45 | 27.9 | 45 | 27.9 | 0.292 | 37.4 | LOS C | 0.9 | 7.5 | 1.00 | 0.74 | 14.4 |
| Approach | | 273 | 12.7 | 273 | 12.7 | 0.292 | 13.0 | LOS A | 1.6 | 11.9 | 0.69 | 0.55 | 32.4 |
| East: Themeda Ave | | | | | | | | | | | | | |
| 4 | L2 | 154 | 11.0 | 154 | 11.0 | 0.412 | 28.4 | LOS B | 2.5 | 19.0 | 0.90 | 0.79 | 30.4 |
| 6 | R2 | 13 | 16.7 | 13 | 16.7 | 0.035 | 26.1 | LOS B | 0.2 | 1.5 | 0.81 | 0.67 | 32.2 |
| Approach | | 166 | 11.4 | 166 | 11.4 | 0.412 | 28.2 | LOS B | 2.5 | 19.0 | 0.90 | 0.78 | 30.6 |
| North: Tallawong Road | | | | | | | | | | | | | |
| 7 | L2 | 201 | 0.5 | 201 | 0.5 | 0.435 | 19.8 | LOS B | 4.2 | 29.9 | 0.79 | 0.75 | 13.3 |
| 8 | T1 | 429 | 3.2 | 429 | 3.2 | 0.435 | 15.0 | LOS B | 4.3 | 31.0 | 0.79 | 0.69 | 14.8 |
| Approach | | 631 | 2.3 | 631 | 2.3 | 0.435 | 16.5 | LOS B | 4.3 | 31.0 | 0.79 | 0.71 | 14.3 |
| All Vehicles | | 1069 | 6.4 | 1069 | 6.4 | 0.435 | 17.5 | LOS B | 4.3 | 31.0 | 0.78 | 0.68 | 23.9 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|----------------------------------|--------------|---------------------|------|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian | Prop. Queued | Effective Stop Rate | |
| | | | | | ped | m | | |
| P1 | South Full Crossing | 18 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| P2 | East Full Crossing | 13 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| P3 | North Full Crossing | 6 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 |
| All Pedestrians | | 37 | 24.3 | LOS C | | | 0.90 | 0.90 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: TT [Tallawong / Themeda Existing PM]

 Network: N101 [Tallawong Existing PM]

Tallawong / Themeda Existing PM

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 60 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------------|------------------|-------------|----------|-----------|---------------|------------------|------------------------------|----------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows Total | Arrival Flows HV | Flows Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Queue Distance | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | | km/h |
| South: Tallawong Road | | | | | | | | | | | | | | |
| 2 | T1 | 152 | 5.6 | 152 | 5.6 | 0.067 | 7.8 | LOS A | 1.0 | 7.1 | 0.65 | 0.51 | 0.65 | 40.3 |
| 3 | R2 | 183 | 6.9 | 183 | 6.9 | 0.296 | 23.2 | LOS B | 2.8 | 21.0 | 0.89 | 0.80 | 0.89 | 20.2 |
| Approach | | 335 | 6.3 | 335 | 6.3 | 0.296 | 16.2 | LOS B | 2.8 | 21.0 | 0.78 | 0.67 | 0.78 | 27.5 |
| East: Themeda Ave | | | | | | | | | | | | | | |
| 4 | L2 | 103 | 12.2 | 103 | 12.2 | 0.302 | 28.7 | LOS C | 1.6 | 12.8 | 0.89 | 0.76 | 0.89 | 30.3 |
| 6 | R2 | 27 | 3.8 | 27 | 3.8 | 0.076 | 27.2 | LOS B | 0.4 | 3.0 | 0.84 | 0.70 | 0.84 | 31.7 |
| Approach | | 131 | 10.5 | 131 | 10.5 | 0.302 | 28.4 | LOS B | 1.6 | 12.8 | 0.88 | 0.75 | 0.88 | 30.6 |
| North: Tallawong Road | | | | | | | | | | | | | | |
| 7 | L2 | 18 | 11.8 | 18 | 11.8 | 0.309 | 30.5 | LOS C | 1.5 | 10.7 | 0.93 | 0.73 | 0.93 | 9.7 |
| 8 | T1 | 158 | 2.7 | 158 | 2.7 | 0.309 | 25.8 | LOS B | 1.5 | 10.7 | 0.93 | 0.72 | 0.93 | 9.9 |
| Approach | | 176 | 3.6 | 176 | 3.6 | 0.309 | 26.3 | LOS B | 1.5 | 10.7 | 0.93 | 0.72 | 0.93 | 9.9 |
| All Vehicles | | 641 | 6.4 | 641 | 6.4 | 0.309 | 21.5 | LOS B | 2.8 | 21.0 | 0.84 | 0.70 | 0.84 | 24.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|----------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian | Prop. Queued | Effective Stop Rate | | |
| | | | | | ped | | | | |
| P1 | South Full Crossing | 21 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 | |
| P2 | East Full Crossing | 16 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 | |
| P3 | North Full Crossing | 18 | 24.3 | LOS C | 0.0 | 0.0 | 0.90 | 0.90 | |
| All Pedestrians | | 55 | 24.3 | LOS C | | | 0.90 | 0.90 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Appendix F2
SIDRA Movement Summary
Existing + Development

MOVEMENT SUMMARY

 Site: CT [Cudgegong / Themeda Existing AM + Devt]

 Network: N101 [Tallawong Existing AM + Devt]

Cudgegong / Conferta Existing AM + Devt

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------------|------------------|-------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Flows Total | Arrival Flows HV | Flows Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| South: Cudgegong Road | | | | | | | | | | | | | | |
| 1 | L2 | 43 | 0.0 | 43 | 0.0 | 0.261 | 33.5 | LOS C | 2.3 | 18.6 | 0.98 | 0.79 | 0.98 | 19.9 |
| 2 | T1 | 197 | 15.5 | 197 | 15.5 | 0.261 | 29.2 | LOS C | 2.6 | 19.6 | 0.99 | 0.80 | 0.99 | 23.4 |
| Approach | | 240 | 12.7 | 240 | 12.7 | 0.261 | 30.0 | LOS C | 2.6 | 19.6 | 0.99 | 0.79 | 0.99 | 22.8 |
| North: Cudgegong Road | | | | | | | | | | | | | | |
| 8 | T1 | 222 | 14.2 | 222 | 14.2 | 0.347 | 24.0 | LOS B | 2.8 | 21.9 | 0.87 | 0.71 | 0.87 | 10.4 |
| 9 | R2 | 54 | 2.0 | 54 | 2.0 | 0.347 | 30.1 | LOS C | 2.1 | 16.0 | 0.88 | 0.74 | 0.88 | 9.4 |
| Approach | | 276 | 11.8 | 276 | 11.8 | 0.347 | 25.2 | LOS B | 2.8 | 21.9 | 0.87 | 0.72 | 0.87 | 10.2 |
| West: Themeda Ave | | | | | | | | | | | | | | |
| 10 | L2 | 31 | 13.8 | 31 | 13.8 | 0.032 | 11.7 | LOS A | 0.3 | 2.3 | 0.49 | 0.64 | 0.49 | 37.4 |
| 12 | R2 | 375 | 1.7 | 375 | 1.7 | 0.357 | 12.8 | LOS A | 4.0 | 28.6 | 0.55 | 0.72 | 0.55 | 36.6 |
| Approach | | 405 | 2.6 | 405 | 2.6 | 0.357 | 12.7 | LOS A | 4.0 | 28.6 | 0.54 | 0.71 | 0.54 | 36.7 |
| All Vehicles | | 921 | 8.0 | 921 | 8.0 | 0.357 | 20.9 | LOS B | 4.0 | 28.6 | 0.76 | 0.73 | 0.76 | 27.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|----------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian | Prop. Queued | Effective Stop Rate | | |
| | | | | | ped | m | | | |
| P1 | South Full Crossing | 13 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P3 | North Full Crossing | 15 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P4 | West Full Crossing | 18 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| All Pedestrians | | 45 | 29.3 | LOS C | | | 0.91 | 0.91 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: CT [Cudgegong / Themeda Existing PM + DEVT]

 Network: N101 [Tallawong Existing PM + Devt]

Cudgegong / Conferta Existing PM + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------------|------------------|-------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Flows Total | Arrival Flows HV | Flows Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| South: Cudgegong Road | | | | | | | | | | | | | | |
| 1 | L2 | 23 | 0.0 | 23 | 0.0 | 0.219 | 25.6 | LOS B | 1.7 | 13.9 | 0.75 | 0.63 | 0.75 | 24.8 |
| 2 | T1 | 218 | 9.7 | 218 | 9.7 | 0.219 | 24.7 | LOS B | 2.6 | 18.8 | 0.89 | 0.73 | 0.89 | 26.0 |
| Approach | | 241 | 8.7 | 241 | 8.7 | 0.219 | 24.8 | LOS B | 2.6 | 18.8 | 0.88 | 0.72 | 0.88 | 25.9 |
| North: Cudgegong Road | | | | | | | | | | | | | | |
| 8 | T1 | 175 | 9.6 | 175 | 9.6 | 0.220 | 20.6 | LOS B | 1.9 | 14.6 | 0.80 | 0.65 | 0.80 | 11.8 |
| 9 | R2 | 37 | 0.0 | 37 | 0.0 | 0.220 | 26.6 | LOS B | 1.5 | 11.1 | 0.81 | 0.69 | 0.81 | 10.7 |
| Approach | | 212 | 8.0 | 212 | 8.0 | 0.220 | 21.7 | LOS B | 1.9 | 14.6 | 0.80 | 0.66 | 0.80 | 11.6 |
| West: Themeda Ave | | | | | | | | | | | | | | |
| 10 | L2 | 41 | 0.0 | 41 | 0.0 | 0.042 | 6.7 | LOS A | 0.1 | 0.7 | 0.13 | 0.56 | 0.13 | 41.3 |
| 12 | R2 | 212 | 2.5 | 212 | 2.5 | 0.219 | 7.1 | LOS A | 0.7 | 4.9 | 0.17 | 0.58 | 0.17 | 41.6 |
| Approach | | 253 | 2.1 | 253 | 2.1 | 0.219 | 7.1 | LOS A | 0.7 | 4.9 | 0.16 | 0.58 | 0.16 | 41.5 |
| All Vehicles | | 705 | 6.1 | 705 | 6.1 | 0.220 | 17.5 | LOS B | 2.6 | 18.8 | 0.60 | 0.65 | 0.60 | 29.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|----------------------------------|--------------|---------------------|------|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian | Prop. Queued | Effective Stop Rate | |
| | | | | | ped | m | | |
| P1 | South Full Crossing | 14 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 |
| P3 | North Full Crossing | 24 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 |
| P4 | West Full Crossing | 19 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 |
| All Pedestrians | | 57 | 29.3 | LOS C | | | 0.92 | 0.92 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: SC [SCHOFIELDS / CUDGEGONG - EXISTING AM + DEVT]

 Network: N101 [Tallawong Existing AM + Devt]

SCHOFIELDS ROAD / CUDGEGONG ROAD EXISTING AM +DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| East: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 5 | T1 | 874 | 7.3 | 874 | 7.3 | 0.439 | 11.4 | LOS A | 5.6 | 40.9 | 0.67 | 0.58 | 0.67 | 43.8 |
| 6 | R2 | 185 | 3.4 | 185 | 3.4 | 0.602 | 42.1 | LOS C | 2.1 | 14.8 | 1.00 | 0.80 | 1.09 | 25.4 |
| Approach | | 1059 | 6.7 | 1059 | 6.7 | 0.602 | 16.8 | LOS B | 5.6 | 40.9 | 0.72 | 0.62 | 0.74 | 38.9 |
| North: CUDGEGONG ROAD | | | | | | | | | | | | | | |
| 7 | L2 | 394 | 7.2 | 394 | 7.2 | 0.709 | 29.9 | LOS C | 7.3 | 54.5 | 0.90 | 0.85 | 0.94 | 35.7 |
| 9 | R2 | 145 | 13.0 | 145 | 13.0 | 0.595 | 32.4 | LOS C | 2.8 | 21.6 | 0.90 | 0.78 | 0.91 | 19.3 |
| Approach | | 539 | 8.8 | 539 | 8.8 | 0.709 | 30.6 | LOS C | 7.3 | 54.5 | 0.90 | 0.83 | 0.93 | 32.6 |
| West: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 10 | L2 | 12 | 54.5 | 12 | 54.5 | 0.016 | 15.0 | LOS B | 0.1 | 1.1 | 0.46 | 0.60 | 0.46 | 38.5 |
| 11 | T1 | 1068 | 7.0 | 1068 | 7.0 | 0.557 | 12.1 | LOS A | 7.8 | 58.2 | 0.70 | 0.62 | 0.70 | 49.2 |
| Approach | | 1080 | 7.5 | 1080 | 7.5 | 0.557 | 12.2 | LOS A | 7.8 | 58.2 | 0.70 | 0.62 | 0.70 | 49.1 |
| All Vehicles | | 2678 | 7.4 | 2678 | 7.4 | 0.709 | 17.7 | LOS B | 7.8 | 58.2 | 0.75 | 0.66 | 0.76 | 42.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | | |
| P2 | East Full Crossing | 13 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P3 | North Full Crossing | 21 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| P4 | West Full Crossing | 19 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| All Pedestrians | | 53 | 29.3 | LOS C | | | 0.91 | 0.91 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: SC [SCHOFIELDS / CUDGEGONG - EXISTING PM + DEVT]

 Network: N101 [Tallawong Existing PM + Devt]

SCHOFIELDS ROAD / CUDGEGONG ROAD EXISTING PM +DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| East: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 5 | T1 | 947 | 2.2 | 947 | 2.2 | 0.505 | 13.3 | LOS A | 6.8 | 48.2 | 0.73 | 0.64 | 0.73 | 42.0 |
| 6 | R2 | 283 | 5.9 | 283 | 5.9 | 0.625 | 39.3 | LOS C | 3.0 | 22.3 | 0.99 | 0.83 | 1.07 | 26.5 |
| Approach | | 1231 | 3.1 | 1231 | 3.1 | 0.625 | 19.2 | LOS B | 6.8 | 48.2 | 0.79 | 0.68 | 0.81 | 37.0 |
| North: CUDGEGONG ROAD | | | | | | | | | | | | | | |
| 7 | L2 | 261 | 7.3 | 261 | 7.3 | 0.431 | 32.7 | LOS C | 5.3 | 39.2 | 1.00 | 0.84 | 1.00 | 34.4 |
| 9 | R2 | 134 | 7.1 | 134 | 7.1 | 0.585 | 42.0 | LOS C | 2.9 | 21.8 | 1.00 | 0.80 | 1.02 | 16.0 |
| Approach | | 395 | 7.2 | 395 | 7.2 | 0.585 | 35.8 | LOS C | 5.3 | 39.2 | 1.00 | 0.83 | 1.01 | 29.5 |
| West: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 10 | L2 | 5 | 0.0 | 5 | 0.0 | 0.006 | 15.1 | LOS B | 0.1 | 0.4 | 0.51 | 0.60 | 0.51 | 37.9 |
| 11 | T1 | 1166 | 3.5 | 1166 | 3.5 | 0.630 | 14.4 | LOS A | 9.4 | 67.9 | 0.81 | 0.72 | 0.81 | 47.6 |
| Approach | | 1172 | 3.5 | 1172 | 3.5 | 0.630 | 14.4 | LOS A | 9.4 | 67.9 | 0.81 | 0.72 | 0.81 | 47.5 |
| All Vehicles | | 2797 | 3.8 | 2797 | 3.8 | 0.630 | 19.6 | LOS B | 9.4 | 67.9 | 0.83 | 0.72 | 0.84 | 40.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | | |
| P2 | East Full Crossing | 19 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| P3 | North Full Crossing | 24 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| P4 | West Full Crossing | 21 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| All Pedestrians | | 64 | 29.3 | LOS C | | | 0.92 | 0.92 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: STR [SCHOFIELDS / TALLAWONG / RIDGELINE - AM + Devt]

 Network: N101 [Tallawong Existing AM + Devt]

SCHOFIELDS / TALLAWONG / RIDGELINE Existing AM + Devt

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| South: RIDGELINE DRIVE | | | | | | | | | | | | | | |
| 1 | L2 | 126 | 3.3 | 126 | 3.3 | 0.170 | 20.2 | LOS B | 1.7 | 12.3 | 0.68 | 0.73 | 0.68 | 44.1 |
| 2 | T1 | 217 | 1.5 | 217 | 1.5 | 0.486 | 26.0 | LOS B | 4.1 | 28.8 | 0.92 | 0.75 | 0.92 | 33.0 |
| 3 | R2 | 158 | 4.0 | 158 | 4.0 | 0.865 | 47.9 | LOS D | 3.9 | 28.2 | 1.00 | 1.01 | 1.53 | 23.8 |
| Approach | | 501 | 2.7 | 501 | 2.7 | 0.865 | 31.4 | LOS C | 4.1 | 28.8 | 0.88 | 0.83 | 1.05 | 33.0 |
| East: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 4 | L2 | 24 | 0.0 | 24 | 0.0 | 0.030 | 17.9 | LOS B | 0.3 | 2.1 | 0.63 | 0.67 | 0.63 | 44.8 |
| 5 | T1 | 772 | 6.1 | 772 | 6.1 | 0.842 | 35.4 | LOS C | 8.8 | 64.7 | 1.00 | 0.94 | 1.14 | 36.6 |
| 6 | R2 | 78 | 13.5 | 78 | 13.5 | 0.539 | 41.9 | LOS C | 1.7 | 13.6 | 1.00 | 0.78 | 1.03 | 22.7 |
| Approach | | 874 | 6.6 | 874 | 6.6 | 0.842 | 35.5 | LOS C | 8.8 | 64.7 | 0.99 | 0.92 | 1.12 | 35.9 |
| North: TALLAWONG ROAD | | | | | | | | | | | | | | |
| 7 | L2 | 35 | 18.2 | 35 | 18.2 | 0.052 | 25.3 | LOS B | 0.6 | 5.0 | 0.89 | 0.73 | 0.89 | 19.1 |
| 8 | T1 | 408 | 2.3 | 408 | 2.3 | 0.921 | 44.9 | LOS D | 10.2 | 72.7 | 1.00 | 1.03 | 1.30 | 29.2 |
| 9 | R2 | 155 | 7.5 | 155 | 7.5 | 0.434 | 40.0 | LOS C | 1.6 | 12.2 | 0.98 | 0.76 | 0.98 | 30.6 |
| Approach | | 598 | 4.6 | 598 | 4.6 | 0.921 | 42.5 | LOS C | 10.2 | 72.7 | 0.99 | 0.94 | 1.20 | 29.3 |
| West: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 10 | L2 | 71 | 1.5 | 71 | 1.5 | 0.085 | 18.3 | LOS B | 0.9 | 6.2 | 0.62 | 0.69 | 0.62 | 37.9 |
| 11 | T1 | 808 | 6.5 | 808 | 6.5 | 0.878 | 37.6 | LOS C | 9.9 | 72.7 | 1.00 | 1.07 | 1.36 | 27.4 |
| 12 | R2 | 125 | 2.5 | 125 | 2.5 | 0.814 | 46.0 | LOS D | 3.0 | 21.4 | 1.00 | 0.94 | 1.41 | 33.9 |
| Approach | | 1004 | 5.7 | 1004 | 5.7 | 0.878 | 37.3 | LOS C | 9.9 | 72.7 | 0.97 | 1.03 | 1.32 | 29.1 |
| All Vehicles | | 2977 | 5.2 | 2977 | 5.2 | 0.921 | 36.8 | LOS C | 10.2 | 72.7 | 0.97 | 0.94 | 1.19 | 32.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | | |
| P1 | South Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P2 | East Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P3 | North Full Crossing | 26 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| P4 | West Full Crossing | 26 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| All Pedestrians | | 84 | 29.3 | LOS C | | | 0.92 | 0.92 | |

MOVEMENT SUMMARY

 Site: 101 [SCHOFIELDS / TALLAWONG / RIDGELINE - EXISTING PM + Devt]

 Network: N101 [Tallawong Existing PM + Devt]

SCHOFIELDS / TALLAWONG / RIDGELINE EXIST PM + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| South: RIDGELINE DRIVE | | | | | | | | | | | | | | |
| 1 | L2 | 84 | 3.8 | 84 | 3.8 | 0.132 | 22.7 | LOS B | 1.2 | 8.8 | 0.72 | 0.73 | 0.72 | 42.7 |
| 2 | T1 | 259 | 0.4 | 259 | 0.4 | 0.769 | 33.7 | LOS C | 5.7 | 40.2 | 1.00 | 0.92 | 1.19 | 29.1 |
| 3 | R2 | 127 | 0.8 | 127 | 0.8 | 0.683 | 41.9 | LOS C | 2.8 | 20.0 | 1.00 | 0.85 | 1.16 | 25.8 |
| Approach | | 471 | 1.1 | 471 | 1.1 | 0.769 | 34.0 | LOS C | 5.7 | 40.2 | 0.95 | 0.87 | 1.10 | 31.1 |
| East: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 4 | L2 | 46 | 9.1 | 46 | 9.1 | 0.041 | 9.3 | LOS A | 0.3 | 2.2 | 0.40 | 0.63 | 0.40 | 50.4 |
| 5 | T1 | 959 | 3.1 | 959 | 3.1 | 0.836 | 33.6 | LOS C | 10.9 | 78.0 | 1.00 | 0.94 | 1.11 | 37.4 |
| 6 | R2 | 38 | 0.0 | 38 | 0.0 | 0.239 | 42.5 | LOS C | 0.8 | 5.9 | 1.00 | 0.73 | 1.00 | 22.5 |
| Approach | | 1043 | 3.2 | 1043 | 3.2 | 0.836 | 32.8 | LOS C | 10.9 | 78.0 | 0.97 | 0.92 | 1.08 | 37.4 |
| North: TALLAWONG ROAD | | | | | | | | | | | | | | |
| 7 | L2 | 49 | 8.5 | 49 | 8.5 | 0.082 | 25.1 | LOS B | 0.9 | 6.5 | 0.87 | 0.74 | 0.87 | 19.2 |
| 8 | T1 | 283 | 1.9 | 283 | 1.9 | 0.848 | 37.1 | LOS C | 6.7 | 47.5 | 1.00 | 0.97 | 1.27 | 32.1 |
| 9 | R2 | 289 | 5.5 | 289 | 5.5 | 0.801 | 44.0 | LOS D | 3.4 | 24.8 | 1.00 | 0.90 | 1.27 | 29.2 |
| Approach | | 622 | 4.1 | 622 | 4.1 | 0.848 | 39.3 | LOS C | 6.7 | 47.5 | 0.99 | 0.92 | 1.24 | 30.1 |
| West: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 10 | L2 | 57 | 14.8 | 57 | 14.8 | 0.047 | 8.1 | LOS A | 0.3 | 2.3 | 0.34 | 0.62 | 0.34 | 47.7 |
| 11 | T1 | 814 | 3.5 | 814 | 3.5 | 0.706 | 24.8 | LOS B | 7.9 | 56.5 | 0.94 | 0.84 | 0.99 | 33.5 |
| 12 | R2 | 117 | 6.3 | 117 | 6.3 | 0.779 | 45.1 | LOS D | 2.7 | 20.3 | 1.00 | 0.91 | 1.34 | 34.2 |
| Approach | | 987 | 4.5 | 987 | 4.5 | 0.779 | 26.3 | LOS B | 7.9 | 56.5 | 0.92 | 0.84 | 1.00 | 34.2 |
| All Vehicles | | 3123 | 3.5 | 3123 | 3.5 | 0.848 | 32.2 | LOS C | 10.9 | 78.0 | 0.95 | 0.89 | 1.09 | 34.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | | |
| P1 | South Full Crossing | 24 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| P2 | East Full Crossing | 15 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P3 | North Full Crossing | 32 | 29.3 | LOS C | 0.1 | 0.1 | 0.92 | 0.92 | |
| P4 | West Full Crossing | 18 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| All Pedestrians | | 88 | 29.3 | LOS C | | | 0.92 | 0.92 | |

MOVEMENT SUMMARY

 Site: 101 [Tallawong / Themeda Existing AM + Devt]

 Network: N101 [Tallawong Existing AM + Devt]

Tallawong / Themeda Existing AM + Devt

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| South: Tallawong Road | | | | | | | | | | | | | | |
| 2 | T1 | 165 | 13.4 | 165 | 13.4 | 0.073 | 8.5 | LOS A | 1.2 | 9.7 | 0.66 | 0.52 | 0.66 | 39.1 |
| 3 | R2 | 140 | 9.0 | 140 | 9.0 | 0.468 | 39.7 | LOS C | 3.0 | 22.6 | 1.00 | 0.80 | 1.00 | 13.7 |
| Approach | | 305 | 11.4 | 305 | 11.4 | 0.468 | 22.9 | LOS B | 3.0 | 22.6 | 0.81 | 0.65 | 0.81 | 22.9 |
| East: Themeda Ave | | | | | | | | | | | | | | |
| 4 | L2 | 154 | 11.0 | 154 | 11.0 | 0.446 | 34.5 | LOS C | 3.2 | 24.2 | 0.99 | 0.81 | 0.99 | 27.5 |
| 6 | R2 | 39 | 5.4 | 39 | 5.4 | 0.109 | 34.2 | LOS C | 0.8 | 5.6 | 0.95 | 0.74 | 0.95 | 28.5 |
| Approach | | 193 | 9.8 | 193 | 9.8 | 0.446 | 34.4 | LOS C | 3.2 | 24.2 | 0.98 | 0.79 | 0.98 | 27.7 |
| North: Tallawong Road | | | | | | | | | | | | | | |
| 7 | L2 | 201 | 0.5 | 201 | 0.5 | 0.456 | 22.9 | LOS B | 5.1 | 36.1 | 0.80 | 0.76 | 0.80 | 11.7 |
| 8 | T1 | 440 | 3.1 | 440 | 3.1 | 0.456 | 18.2 | LOS B | 5.2 | 37.3 | 0.81 | 0.71 | 0.81 | 12.9 |
| Approach | | 641 | 2.3 | 641 | 2.3 | 0.456 | 19.6 | LOS B | 5.2 | 37.3 | 0.81 | 0.72 | 0.81 | 12.5 |
| All Vehicles | | 1139 | 6.0 | 1139 | 6.0 | 0.468 | 23.0 | LOS B | 5.2 | 37.3 | 0.84 | 0.71 | 0.84 | 20.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | | |
| P1 | South Full Crossing | 26 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| P2 | East Full Crossing | 31 | 29.3 | LOS C | 0.1 | 0.1 | 0.92 | 0.92 | |
| P3 | North Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| All Pedestrians | | 73 | 29.3 | LOS C | | | 0.92 | 0.92 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: TT [Tallawong / Themeda Existing PM + DEVT]

 Network: N101 [Tallawong Existing PM + Devt]

Tallawong / Themeda Existing PM + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|------|--------------------|------------------|-------------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Flows Total | Arrival Flows HV | Flows Total | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | | veh | | | | km/h | |
| South: Tallawong Road | | | | | | | | | | | | | |
| 2 | T1 | 152 | 5.6 | 152 | 5.6 | 0.059 | LOS A | 1.2 | 8.9 | 0.74 | 0.59 | 0.74 | 39.7 |
| 3 | R2 | 346 | 3.6 | 346 | 3.6 | 0.406 | LOS B | 6.4 | 46.0 | 0.93 | 0.84 | 0.93 | 18.9 |
| Approach | | 498 | 4.2 | 498 | 4.2 | 0.406 | LOS B | 6.4 | 46.0 | 0.87 | 0.77 | 0.87 | 23.5 |
| East: Themeda Ave | | | | | | | | | | | | | |
| 4 | L2 | 103 | 12.2 | 103 | 12.2 | 0.423 | LOS C | 2.2 | 16.7 | 0.99 | 0.78 | 0.99 | 26.6 |
| 6 | R2 | 48 | 2.2 | 48 | 2.2 | 0.185 | LOS C | 1.0 | 7.0 | 0.97 | 0.74 | 0.97 | 28.1 |
| Approach | | 152 | 9.0 | 152 | 9.0 | 0.423 | LOS C | 2.2 | 16.7 | 0.98 | 0.77 | 0.98 | 27.1 |
| North: Tallawong Road | | | | | | | | | | | | | |
| 7 | L2 | 18 | 11.8 | 18 | 11.8 | 0.403 | LOS C | 2.0 | 14.3 | 0.96 | 0.75 | 0.96 | 8.2 |
| 8 | T1 | 179 | 2.4 | 179 | 2.4 | 0.403 | LOS C | 2.0 | 14.4 | 0.96 | 0.75 | 0.96 | 8.3 |
| Approach | | 197 | 3.2 | 197 | 3.2 | 0.403 | LOS C | 2.0 | 14.4 | 0.96 | 0.75 | 0.96 | 8.3 |
| All Vehicles | | 846 | 4.9 | 846 | 4.9 | 0.423 | LOS B | 6.4 | 46.0 | 0.91 | 0.76 | 0.91 | 21.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|----------------------------------|--------------|---------------------|------|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian | Prop. Queued | Effective Stop Rate | |
| | | | | | ped | | | |
| P1 | South Full Crossing | 25 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 |
| P2 | East Full Crossing | 29 | 29.3 | LOS C | 0.1 | 0.1 | 0.92 | 0.92 |
| P3 | North Full Crossing | 13 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 |
| All Pedestrians | | 67 | 29.3 | LOS C | | | 0.92 | 0.92 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Appendix F3
SIDRA Movement Summary
Existing + Growth

MOVEMENT SUMMARY



Site: CT [Cudgegong / Themeda Existing AM + GROWTH]



Network: N101 [Tallawong Existing AM + GROWTH]

Cudgegong / Conferta Existing AM + GROWTH

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------------|------------------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|------|------|
| Mov ID | Turn | Demand Flows Total | Arrival Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | | | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | | km/h |
| South: Cudgegong Road | | | | | | | | | | | | | | |
| 1 | L2 | 52 | 0.0 | 52 | 0.0 | 0.175 | 20.8 | LOS B | 1.9 | 15.3 | 0.76 | 0.68 | 0.76 | 27.3 |
| 2 | T1 | 205 | 17.9 | 205 | 17.9 | 0.175 | 18.9 | LOS B | 2.5 | 19.1 | 0.88 | 0.74 | 0.88 | 29.5 |
| Approach | | 257 | 14.3 | 257 | 14.3 | 0.175 | 19.3 | LOS B | 2.5 | 19.1 | 0.86 | 0.72 | 0.86 | 29.1 |
| North: Cudgegong Road | | | | | | | | | | | | | | |
| 8 | T1 | 266 | 14.2 | 266 | 14.2 | 0.245 | 15.0 | LOS B | 2.5 | 19.5 | 0.70 | 0.60 | 0.70 | 15.0 |
| 9 | R2 | 54 | 2.0 | 54 | 2.0 | 0.245 | 20.7 | LOS B | 2.0 | 15.2 | 0.72 | 0.65 | 0.72 | 13.4 |
| Approach | | 320 | 12.2 | 320 | 12.2 | 0.245 | 15.9 | LOS B | 2.5 | 19.5 | 0.70 | 0.61 | 0.70 | 14.7 |
| West: Themeda Ave | | | | | | | | | | | | | | |
| 10 | L2 | 37 | 14.3 | 37 | 14.3 | 0.053 | 11.3 | LOS A | 0.2 | 1.8 | 0.32 | 0.60 | 0.32 | 37.7 |
| 12 | R2 | 185 | 4.0 | 185 | 4.0 | 0.248 | 10.0 | LOS A | 1.1 | 7.6 | 0.29 | 0.61 | 0.29 | 38.9 |
| Approach | | 222 | 5.7 | 222 | 5.7 | 0.248 | 10.2 | LOS A | 1.1 | 7.6 | 0.29 | 0.61 | 0.29 | 38.7 |
| All Vehicles | | 799 | 11.1 | 799 | 11.1 | 0.248 | 15.4 | LOS B | 2.5 | 19.5 | 0.64 | 0.64 | 0.64 | 29.1 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | |
| P1 | South Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 |
| P3 | North Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 |
| P4 | West Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 |
| All Pedestrians | | 47 | 29.3 | LOS C | | | 0.91 | 0.91 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: CT [Cudgegong / Themeda Existing PM + GROWTH]

 Network: N101 [Tallawong Existing PM + GROWTH]

Cudgegong / Conferta Existing PM + GROWTH

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|------|--------------------|------------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Flows Total | Arrival Flows HV | Total HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | | veh | | | | km/h | |
| South: Cudgegong Road | | | | | | | | | | | | | |
| 1 | L2 | 27 | 0.0 | 27 | 0.0 | 0.161 | LOS B | 1.9 | 15.1 | 0.74 | 0.64 | 0.74 | 30.0 |
| 2 | T1 | 241 | 10.5 | 241 | 10.5 | 0.161 | LOS B | 2.6 | 18.7 | 0.86 | 0.71 | 0.86 | 31.4 |
| Approach | | 268 | 9.4 | 268 | 9.4 | 0.161 | LOS B | 2.6 | 18.7 | 0.85 | 0.70 | 0.85 | 31.3 |
| North: Cudgegong Road | | | | | | | | | | | | | |
| 8 | T1 | 211 | 10.0 | 211 | 10.0 | 0.150 | LOS A | 1.5 | 11.6 | 0.62 | 0.52 | 0.62 | 17.5 |
| 9 | R2 | 22 | 0.0 | 22 | 0.0 | 0.150 | LOS B | 1.3 | 10.1 | 0.63 | 0.55 | 0.63 | 16.5 |
| Approach | | 233 | 9.0 | 233 | 9.0 | 0.150 | LOS A | 1.5 | 11.6 | 0.62 | 0.52 | 0.62 | 17.4 |
| West: Themeda Ave | | | | | | | | | | | | | |
| 10 | L2 | 49 | 0.0 | 49 | 0.0 | 0.072 | LOS A | 0.1 | 0.9 | 0.12 | 0.56 | 0.12 | 40.8 |
| 12 | R2 | 99 | 6.4 | 99 | 6.4 | 0.150 | LOS A | 0.3 | 2.3 | 0.16 | 0.57 | 0.16 | 40.9 |
| Approach | | 148 | 4.3 | 148 | 4.3 | 0.150 | LOS A | 0.3 | 2.3 | 0.14 | 0.56 | 0.14 | 40.9 |
| All Vehicles | | 649 | 8.1 | 649 | 8.1 | 0.161 | LOS A | 2.6 | 18.7 | 0.61 | 0.61 | 0.61 | 31.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.


| Movement Performance - Pedestrians | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|----------------------------------|--------------|---------------------|------|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian | Prop. Queued | Effective Stop Rate | |
| | | | | | ped | | | |
| P1 | South Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 |
| P3 | North Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 |
| P4 | West Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 |
| All Pedestrians | | 47 | 29.3 | LOS C | | | 0.91 | 0.91 |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: SC [SCHOFIELDS / CUDGEGONG - AM EXIST + GROWTH]

 Network: N101 [Tallawong Existing AM + GROWTH]

SCHOFIELDS ROAD / CUDGEGONG ROAD AM EXIST + GROWTH

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| East: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 5 | T1 | 1049 | 7.4 | 1049 | 7.4 | 0.678 | 18.7 | LOS B | 9.0 | 65.2 | 0.87 | 0.77 | 0.87 | 37.4 |
| 6 | R2 | 136 | 5.4 | 136 | 5.4 | 0.224 | 33.6 | LOS C | 1.3 | 9.3 | 0.90 | 0.75 | 0.90 | 28.8 |
| Approach | | 1185 | 7.2 | 1185 | 7.2 | 0.678 | 20.4 | LOS B | 9.0 | 65.2 | 0.88 | 0.76 | 0.88 | 36.2 |
| North: CUDGEGONG ROAD | | | | | | | | | | | | | | |
| 7 | L2 | 208 | 16.2 | 208 | 16.2 | 0.292 | 22.4 | LOS B | 3.5 | 27.6 | 0.83 | 0.79 | 0.83 | 39.3 |
| 9 | R2 | 175 | 13.3 | 175 | 13.3 | 0.598 | 28.0 | LOS B | 3.1 | 23.9 | 0.84 | 0.77 | 0.84 | 21.2 |
| Approach | | 383 | 14.8 | 383 | 14.8 | 0.598 | 25.0 | LOS B | 3.5 | 27.6 | 0.83 | 0.78 | 0.84 | 33.1 |
| West: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 10 | L2 | 14 | 53.8 | 14 | 53.8 | 0.024 | 19.9 | LOS B | 0.2 | 1.9 | 0.68 | 0.65 | 0.68 | 34.3 |
| 11 | T1 | 1282 | 7.0 | 1282 | 7.0 | 0.863 | 32.1 | LOS C | 15.0 | 111.4 | 1.00 | 1.02 | 1.18 | 37.9 |
| Approach | | 1296 | 7.5 | 1296 | 7.5 | 0.863 | 31.9 | LOS C | 15.0 | 111.4 | 1.00 | 1.01 | 1.17 | 37.9 |
| All Vehicles | | 2864 | 8.3 | 2864 | 8.3 | 0.863 | 26.3 | LOS B | 15.0 | 111.4 | 0.93 | 0.88 | 1.00 | 36.8 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.


| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | | |
| P2 | East Full Crossing | 37 | 29.3 | LOS C | 0.1 | 0.1 | 0.92 | 0.92 | |
| P3 | North Full Crossing | 37 | 29.3 | LOS C | 0.1 | 0.1 | 0.92 | 0.92 | |
| P4 | West Full Crossing | 37 | 29.3 | LOS C | 0.1 | 0.1 | 0.92 | 0.92 | |
| All Pedestrians | | 111 | 29.3 | LOS C | | | 0.92 | 0.92 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: SC [SCHOFIELDS / CUDGEGONG - PM EXIST + GROWTH]

 Network: N101 [Tallawong Existing PM + GROWTH]

SCHOFIELDS ROAD / CUDGEGONG ROAD PM EXIST + GROWTH

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| East: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 5 | T1 | 1137 | 2.2 | 1137 | 2.2 | 0.557 | 11.9 | LOS A | 8.0 | 56.6 | 0.72 | 0.64 | 0.72 | 43.4 |
| 6 | R2 | 153 | 13.1 | 153 | 13.1 | 0.530 | 41.8 | LOS C | 1.7 | 13.0 | 1.00 | 0.77 | 1.03 | 25.6 |
| Approach | | 1289 | 3.5 | 1289 | 3.5 | 0.557 | 15.4 | LOS B | 8.0 | 56.6 | 0.75 | 0.65 | 0.75 | 40.1 |
| North: CUDGEGONG ROAD | | | | | | | | | | | | | | |
| 7 | L2 | 158 | 14.7 | 158 | 14.7 | 0.313 | 32.4 | LOS C | 3.1 | 24.6 | 0.98 | 0.81 | 0.98 | 34.4 |
| 9 | R2 | 161 | 7.2 | 161 | 7.2 | 0.705 | 43.4 | LOS D | 3.6 | 26.8 | 1.00 | 0.84 | 1.08 | 15.6 |
| Approach | | 319 | 10.9 | 319 | 10.9 | 0.705 | 38.0 | LOS C | 3.6 | 26.8 | 0.99 | 0.82 | 1.03 | 26.4 |
| West: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 10 | L2 | 6 | 0.0 | 6 | 0.0 | 0.006 | 13.8 | LOS A | 0.1 | 0.4 | 0.46 | 0.60 | 0.46 | 39.1 |
| 11 | T1 | 1399 | 3.5 | 1399 | 3.5 | 0.695 | 13.2 | LOS A | 11.4 | 82.0 | 0.81 | 0.73 | 0.81 | 48.4 |
| Approach | | 1405 | 3.5 | 1405 | 3.5 | 0.695 | 13.2 | LOS A | 11.4 | 82.0 | 0.81 | 0.73 | 0.81 | 48.4 |
| All Vehicles | | 3014 | 4.3 | 3014 | 4.3 | 0.705 | 16.8 | LOS B | 11.4 | 82.0 | 0.80 | 0.71 | 0.81 | 42.9 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | | |
| P2 | East Full Crossing | 24 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| P3 | North Full Crossing | 15 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P4 | West Full Crossing | 19 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| All Pedestrians | | 58 | 29.3 | LOS C | | | 0.92 | 0.92 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: STR [SCHOFIELDS / TALLAWONG / RIDGELINE AM EXIST + GROWTH]

 Network: N101 [Tallawong Existing AM + GROWTH]

SCHOFIELDS / TALLAWONG / RIDGELINE AM EXIST + GROWTH

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| South: RIDGELINE DRIVE | | | | | | | | | | | | | | |
| 1 | L2 | 152 | 2.8 | 152 | 2.8 | 0.270 | 26.0 | LOS B | 2.5 | 17.6 | 0.80 | 0.77 | 0.80 | 41.2 |
| 2 | T1 | 172 | 1.8 | 172 | 1.8 | 0.771 | 37.0 | LOS C | 3.9 | 27.8 | 1.00 | 0.91 | 1.26 | 27.7 |
| 3 | R2 | 188 | 3.4 | 188 | 3.4 | 0.800 | 43.1 | LOS D | 4.4 | 31.5 | 1.00 | 0.94 | 1.30 | 25.4 |
| Approach | | 512 | 2.7 | 512 | 2.7 | 0.800 | 36.0 | LOS C | 4.4 | 31.5 | 0.94 | 0.88 | 1.14 | 31.5 |
| East: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 4 | L2 | 24 | 0.0 | 24 | 0.0 | 0.024 | 13.6 | LOS A | 0.2 | 1.4 | 0.41 | 0.63 | 0.41 | 47.6 |
| 5 | T1 | 772 | 6.1 | 772 | 6.1 | 0.651 | 27.8 | LOS B | 8.1 | 59.6 | 0.98 | 0.84 | 0.98 | 40.0 |
| 6 | R2 | 78 | 13.5 | 78 | 13.5 | 0.462 | 43.4 | LOS D | 1.7 | 13.6 | 1.00 | 0.77 | 1.00 | 22.3 |
| Approach | | 874 | 6.6 | 874 | 6.6 | 0.651 | 28.8 | LOS C | 8.1 | 59.6 | 0.96 | 0.83 | 0.96 | 38.8 |
| North: TALLAWONG ROAD | | | | | | | | | | | | | | |
| 7 | L2 | 41 | 17.9 | 41 | 17.9 | 0.082 | 26.6 | LOS B | 0.7 | 5.8 | 0.87 | 0.73 | 0.87 | 18.5 |
| 8 | T1 | 176 | 6.6 | 176 | 6.6 | 0.814 | 40.5 | LOS C | 4.1 | 30.1 | 1.00 | 0.88 | 1.18 | 30.8 |
| 9 | R2 | 154 | 8.9 | 154 | 8.9 | 0.339 | 37.4 | LOS C | 1.6 | 11.7 | 0.95 | 0.76 | 0.95 | 31.5 |
| Approach | | 371 | 8.8 | 371 | 8.8 | 0.814 | 37.7 | LOS C | 4.1 | 30.1 | 0.96 | 0.81 | 1.05 | 30.4 |
| West: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 10 | L2 | 59 | 1.8 | 59 | 1.8 | 0.058 | 14.2 | LOS A | 0.6 | 4.2 | 0.51 | 0.66 | 0.51 | 41.4 |
| 11 | T1 | 971 | 6.5 | 971 | 6.5 | 0.814 | 28.8 | LOS C | 10.5 | 77.2 | 0.98 | 0.97 | 1.15 | 31.3 |
| 12 | R2 | 149 | 2.1 | 149 | 2.1 | 0.830 | 45.9 | LOS D | 3.6 | 25.5 | 1.00 | 0.96 | 1.43 | 34.0 |
| Approach | | 1179 | 5.7 | 1179 | 5.7 | 0.830 | 30.3 | LOS C | 10.5 | 77.2 | 0.96 | 0.95 | 1.16 | 32.2 |
| All Vehicles | | 2935 | 5.8 | 2935 | 5.8 | 0.830 | 31.7 | LOS C | 10.5 | 77.2 | 0.96 | 0.88 | 1.08 | 34.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | | |
| P1 | South Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P2 | East Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P3 | North Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P4 | West Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| All Pedestrians | | 63 | 29.3 | LOS C | | | 0.91 | 0.91 | |

MOVEMENT SUMMARY

 Site: 101 [SCHOFIELDS / TALLAWONG / RIDGELINE PM EXIST + GROWTH]

 Network: N101 [Tallawong Existing PM + GROWTH]

SCHOFIELDS / TALLAWONG / RIDGELINE PM EXIST + GROWTH

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| South: RIDGELINE DRIVE | | | | | | | | | | | | | | |
| 1 | L2 | 101 | 4.2 | 101 | 4.2 | 0.153 | 22.2 | LOS B | 1.4 | 10.5 | 0.71 | 0.73 | 0.71 | 43.0 |
| 2 | T1 | 126 | 0.8 | 126 | 0.8 | 0.376 | 28.7 | LOS C | 2.4 | 17.2 | 0.93 | 0.74 | 0.93 | 31.5 |
| 3 | R2 | 153 | 0.7 | 153 | 0.7 | 0.817 | 45.2 | LOS D | 3.6 | 25.4 | 1.00 | 0.95 | 1.38 | 24.7 |
| Approach | | 380 | 1.7 | 380 | 1.7 | 0.817 | 33.6 | LOS C | 3.6 | 25.4 | 0.90 | 0.82 | 1.05 | 32.2 |
| East: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 4 | L2 | 35 | 15.2 | 35 | 15.2 | 0.033 | 9.8 | LOS A | 0.3 | 2.1 | 0.46 | 0.63 | 0.46 | 49.7 |
| 5 | T1 | 905 | 4.0 | 905 | 4.0 | 0.831 | 33.7 | LOS C | 10.3 | 74.0 | 1.00 | 0.94 | 1.11 | 37.3 |
| 6 | R2 | 81 | 0.0 | 81 | 0.0 | 0.439 | 39.8 | LOS C | 1.7 | 12.2 | 1.00 | 0.77 | 1.00 | 23.4 |
| Approach | | 1021 | 4.0 | 1021 | 4.0 | 0.831 | 33.3 | LOS C | 10.3 | 74.0 | 0.98 | 0.91 | 1.08 | 36.8 |
| North: TALLAWONG ROAD | | | | | | | | | | | | | | |
| 7 | L2 | 60 | 8.8 | 60 | 8.8 | 0.095 | 22.2 | LOS B | 0.9 | 6.9 | 0.76 | 0.73 | 0.76 | 20.8 |
| 8 | T1 | 251 | 27.7 | 251 | 27.7 | 0.875 | 39.5 | LOS C | 6.2 | 53.4 | 1.00 | 1.02 | 1.37 | 31.1 |
| 9 | R2 | 305 | 6.2 | 305 | 6.2 | 0.849 | 45.7 | LOS D | 3.7 | 27.0 | 1.00 | 0.95 | 1.38 | 28.6 |
| Approach | | 616 | 15.2 | 616 | 15.2 | 0.875 | 40.9 | LOS C | 6.2 | 53.4 | 0.98 | 0.96 | 1.31 | 29.3 |
| West: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 10 | L2 | 24 | 43.5 | 24 | 43.5 | 0.022 | 7.5 | LOS A | 0.1 | 0.9 | 0.27 | 0.59 | 0.27 | 48.8 |
| 11 | T1 | 976 | 3.5 | 976 | 3.5 | 0.889 | 37.3 | LOS C | 12.3 | 87.8 | 1.00 | 1.09 | 1.36 | 27.5 |
| 12 | R2 | 140 | 6.0 | 140 | 6.0 | 0.799 | 44.8 | LOS D | 3.3 | 24.3 | 1.00 | 0.93 | 1.36 | 34.3 |
| Approach | | 1140 | 4.6 | 1140 | 4.6 | 0.889 | 37.6 | LOS C | 12.3 | 87.8 | 0.98 | 1.06 | 1.34 | 28.9 |
| All Vehicles | | 3157 | 6.1 | 3157 | 6.1 | 0.889 | 36.4 | LOS C | 12.3 | 87.8 | 0.97 | 0.96 | 1.22 | 32.2 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | | |
| P1 | South Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P2 | East Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P3 | North Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P4 | West Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| All Pedestrians | | 63 | 29.3 | LOS C | | | 0.91 | 0.91 | |

MOVEMENT SUMMARY

 Site: TT [Tallawong / Themeda Existing AM + GROWTH]

 Network: N101 [Tallawong Existing AM + GROWTH]

Tallawong / Themeda Existing AM + GROWTH

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------------|------------------|-------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Flows Total | Arrival Flows HV | Flows Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| South: Tallawong Road | | | | | | | | | | | | | | |
| 2 | T1 | 274 | 10.0 | 274 | 10.0 | 0.125 | 9.3 | LOS A | 2.2 | 16.6 | 0.65 | 0.54 | 0.65 | 38.0 |
| 3 | R2 | 54 | 27.5 | 54 | 27.5 | 0.403 | 43.6 | LOS D | 1.2 | 10.4 | 1.00 | 0.75 | 1.00 | 12.8 |
| Approach | | 327 | 12.9 | 327 | 12.9 | 0.403 | 14.9 | LOS B | 2.2 | 16.6 | 0.71 | 0.57 | 0.71 | 30.4 |
| East: Themeda Ave | | | | | | | | | | | | | | |
| 4 | L2 | 184 | 10.9 | 184 | 10.9 | 0.468 | 31.7 | LOS C | 3.5 | 26.9 | 0.93 | 0.80 | 0.93 | 28.8 |
| 6 | R2 | 15 | 14.3 | 15 | 14.3 | 0.038 | 28.6 | LOS C | 0.3 | 2.0 | 0.83 | 0.68 | 0.83 | 31.0 |
| Approach | | 199 | 11.1 | 199 | 11.1 | 0.468 | 31.4 | LOS C | 3.5 | 26.9 | 0.92 | 0.79 | 0.92 | 29.0 |
| North: Tallawong Road | | | | | | | | | | | | | | |
| 7 | L2 | 241 | 0.4 | 241 | 0.4 | 0.466 | 20.2 | LOS B | 5.6 | 39.9 | 0.76 | 0.75 | 0.76 | 13.0 |
| 8 | T1 | 515 | 3.1 | 515 | 3.1 | 0.466 | 15.5 | LOS B | 5.8 | 41.3 | 0.76 | 0.68 | 0.76 | 14.5 |
| Approach | | 756 | 2.2 | 756 | 2.2 | 0.466 | 17.0 | LOS B | 5.8 | 41.3 | 0.76 | 0.70 | 0.76 | 14.0 |
| All Vehicles | | 1282 | 6.3 | 1282 | 6.3 | 0.468 | 18.7 | LOS B | 5.8 | 41.3 | 0.77 | 0.68 | 0.77 | 22.9 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|----------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian | Prop. Queued | Effective Stop Rate | | |
| | | | | | ped | | | | |
| P1 | South Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P2 | East Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P3 | North Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| All Pedestrians | | 47 | 29.3 | LOS C | | | 0.91 | 0.91 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: TT [Tallawong / Themeda Existing PM + growth]

 Network: N101 [Tallawong Existing PM + GROWTH]

Tallawong / Themeda Existing PM + growth

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------------|------------------|-------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Flows Total | Arrival Flows HV | Flows Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| South: Tallawong Road | | | | | | | | | | | | | | |
| 2 | T1 | 182 | 5.8 | 182 | 5.8 | 0.079 | 8.5 | LOS A | 1.4 | 10.2 | 0.63 | 0.50 | 0.63 | 39.1 |
| 3 | R2 | 219 | 6.7 | 219 | 6.7 | 0.346 | 28.7 | LOS C | 4.1 | 30.2 | 0.93 | 0.82 | 0.93 | 17.4 |
| Approach | | 401 | 6.3 | 401 | 6.3 | 0.346 | 19.5 | LOS B | 4.1 | 30.2 | 0.79 | 0.67 | 0.79 | 24.7 |
| East: Themeda Ave | | | | | | | | | | | | | | |
| 4 | L2 | 123 | 12.0 | 123 | 12.0 | 0.336 | 31.6 | LOS C | 2.3 | 17.4 | 0.89 | 0.77 | 0.89 | 28.8 |
| 6 | R2 | 33 | 3.2 | 33 | 3.2 | 0.084 | 29.7 | LOS C | 0.6 | 4.0 | 0.83 | 0.71 | 0.83 | 30.5 |
| Approach | | 156 | 10.1 | 156 | 10.1 | 0.336 | 31.2 | LOS C | 2.3 | 17.4 | 0.88 | 0.76 | 0.88 | 29.2 |
| North: Tallawong Road | | | | | | | | | | | | | | |
| 7 | L2 | 21 | 10.0 | 21 | 10.0 | 0.324 | 33.2 | LOS C | 2.0 | 14.4 | 0.92 | 0.73 | 0.92 | 9.0 |
| 8 | T1 | 189 | 2.8 | 189 | 2.8 | 0.324 | 28.5 | LOS B | 2.0 | 14.5 | 0.92 | 0.73 | 0.92 | 9.1 |
| Approach | | 211 | 3.5 | 211 | 3.5 | 0.324 | 28.9 | LOS C | 2.0 | 14.5 | 0.92 | 0.73 | 0.92 | 9.1 |
| All Vehicles | | 767 | 6.3 | 767 | 6.3 | 0.346 | 24.5 | LOS B | 4.1 | 30.2 | 0.84 | 0.71 | 0.84 | 22.4 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|----------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian | Prop. Queued | Effective Stop Rate | | |
| | | | | | ped | m | | | |
| P1 | South Full Crossing | 21 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| P2 | East Full Crossing | 16 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| P3 | North Full Crossing | 13 | 29.3 | LOS C | 0.0 | 0.0 | 0.91 | 0.91 | |
| All Pedestrians | | 49 | 29.3 | LOS C | | | 0.91 | 0.91 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Appendix F4
SIDRA Movement Summary

Existing + Growth + Development

MOVEMENT SUMMARY

 **Site: CT [Cudgegong / Themeda Existing AM + GROWTH + DEVT]**

 **Network: N101 [Tallawong Existing AM + GROWTH + DEVT]**

Cudgegong / Conferta Existing AM + GROWTH + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 80 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| South: Cudgegong Road | | | | | | | | | | | | | | |
| 1 | L2 | 52 | 0.0 | 52 | 0.0 | 0.267 | 34.6 | LOS C | 2.9 | 23.0 | 0.94 | 0.78 | 0.94 | 19.4 |
| 2 | T1 | 225 | 13.6 | 225 | 13.6 | 0.267 | 31.1 | LOS C | 3.3 | 25.0 | 0.98 | 0.80 | 0.98 | 22.6 |
| Approach | | 277 | 11.0 | 277 | 11.0 | 0.267 | 31.7 | LOS C | 3.3 | 25.0 | 0.97 | 0.79 | 0.97 | 22.0 |
| North: Cudgegong Road | | | | | | | | | | | | | | |
| 8 | T1 | 266 | 14.2 | 266 | 14.2 | 0.376 | 25.7 | LOS B | 3.7 | 29.4 | 0.85 | 0.71 | 0.85 | 9.9 |
| 9 | R2 | 62 | 1.7 | 62 | 1.7 | 0.376 | 32.3 | LOS C | 2.8 | 20.8 | 0.87 | 0.74 | 0.87 | 8.8 |
| Approach | | 328 | 11.9 | 328 | 11.9 | 0.376 | 26.9 | LOS B | 3.7 | 29.4 | 0.86 | 0.72 | 0.86 | 9.7 |
| West: Themeda Ave | | | | | | | | | | | | | | |
| 10 | L2 | 37 | 14.3 | 37 | 14.3 | 0.039 | 9.7 | LOS A | 0.2 | 1.6 | 0.25 | 0.58 | 0.25 | 38.8 |
| 12 | R2 | 405 | 1.8 | 405 | 1.8 | 0.393 | 10.2 | LOS A | 2.9 | 20.3 | 0.32 | 0.64 | 0.32 | 38.7 |
| Approach | | 442 | 2.9 | 442 | 2.9 | 0.393 | 10.2 | LOS A | 2.9 | 20.3 | 0.31 | 0.63 | 0.31 | 38.7 |
| All Vehicles | | 1047 | 7.8 | 1047 | 7.8 | 0.393 | 21.1 | LOS B | 3.7 | 29.4 | 0.66 | 0.70 | 0.66 | 26.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------|---------------|------------------|-----------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow | Average Delay | Level of Service | Average Back of Queue | Prop. Queued | Effective Stop Rate | | |
| | | ped/h | sec | | Pedestrian ped | Distance m | | | |
| P1 | South Full Crossing | 53 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| P3 | North Full Crossing | 53 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| P4 | West Full Crossing | 53 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| All Pedestrians | | 158 | 34.3 | LOS D | | | 0.93 | 0.93 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: CT [Cudgegong / Themeda Existing PM + GROWTH + DEVT]

 Network: N101 [Tallawong Existing PM + GROWTH + DEVT]

Cudgegong / Conferta Existing PM + GROWTH + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| South: Cudgegong Road | | | | | | | | | | | | | | |
| 1 | L2 | 27 | 0.0 | 27 | 0.0 | 0.236 | 24.2 | LOS B | 1.9 | 15.4 | 0.70 | 0.61 | 0.70 | 25.8 |
| 2 | T1 | 258 | 9.8 | 258 | 9.8 | 0.236 | 23.6 | LOS B | 3.1 | 22.2 | 0.88 | 0.73 | 0.88 | 26.6 |
| Approach | | 285 | 8.9 | 285 | 8.9 | 0.236 | 23.7 | LOS B | 3.1 | 22.2 | 0.86 | 0.71 | 0.86 | 26.6 |
| North: Cudgegong Road | | | | | | | | | | | | | | |
| 8 | T1 | 211 | 10.0 | 211 | 10.0 | 0.242 | 19.2 | LOS B | 2.2 | 17.1 | 0.78 | 0.64 | 0.78 | 12.4 |
| 9 | R2 | 41 | 0.0 | 41 | 0.0 | 0.242 | 25.2 | LOS B | 1.7 | 12.7 | 0.79 | 0.68 | 0.79 | 11.3 |
| Approach | | 252 | 8.4 | 252 | 8.4 | 0.242 | 20.2 | LOS B | 2.2 | 17.1 | 0.78 | 0.65 | 0.78 | 12.2 |
| West: Themeda Ave | | | | | | | | | | | | | | |
| 10 | L2 | 49 | 0.0 | 49 | 0.0 | 0.053 | 8.1 | LOS A | 0.2 | 1.4 | 0.20 | 0.58 | 0.20 | 40.2 |
| 12 | R2 | 227 | 2.3 | 227 | 2.3 | 0.249 | 8.7 | LOS A | 1.1 | 8.1 | 0.25 | 0.61 | 0.25 | 40.1 |
| Approach | | 277 | 1.9 | 277 | 1.9 | 0.249 | 8.5 | LOS A | 1.1 | 8.1 | 0.24 | 0.60 | 0.24 | 40.1 |
| All Vehicles | | 814 | 6.3 | 814 | 6.3 | 0.249 | 17.5 | LOS B | 3.1 | 22.2 | 0.62 | 0.66 | 0.62 | 29.0 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | | |
| P1 | South Full Crossing | 53 | 29.3 | LOS C | 0.1 | 0.1 | 0.92 | 0.92 | |
| P3 | North Full Crossing | 53 | 29.3 | LOS C | 0.1 | 0.1 | 0.92 | 0.92 | |
| P4 | West Full Crossing | 53 | 29.3 | LOS C | 0.1 | 0.1 | 0.92 | 0.92 | |
| All Pedestrians | | 158 | 29.3 | LOS C | | | 0.92 | 0.92 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: SC [SCHOFIELDS / CUDGEGONG - AM EXIST + GROWTH + DEVT]

 Network: N101 [Tallawong Existing AM + GROWTH + DEVT]

SCHOFIELDS ROAD / CUDGEGONG ROAD AM EXIST + GROWTH + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 80 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| East: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 5 | T1 | 1049 | 7.4 | 1049 | 7.4 | 0.612 | 17.8 | LOS B | 9.4 | 68.4 | 0.80 | 0.71 | 0.80 | 38.1 |
| 6 | R2 | 206 | 3.1 | 206 | 3.1 | 0.353 | 38.9 | LOS C | 2.3 | 16.4 | 0.93 | 0.77 | 0.93 | 26.6 |
| Approach | | 1256 | 6.7 | 1256 | 6.7 | 0.612 | 21.3 | LOS B | 9.4 | 68.4 | 0.82 | 0.72 | 0.82 | 35.6 |
| North: CUDGEGONG ROAD | | | | | | | | | | | | | | |
| 7 | L2 | 428 | 7.9 | 428 | 7.9 | 0.609 | 26.4 | LOS B | 8.4 | 62.9 | 0.87 | 0.83 | 0.87 | 37.4 |
| 9 | R2 | 175 | 13.3 | 175 | 13.3 | 0.630 | 37.4 | LOS C | 3.9 | 30.5 | 0.94 | 0.81 | 0.96 | 17.5 |
| Approach | | 603 | 9.4 | 603 | 9.4 | 0.630 | 29.6 | LOS C | 8.4 | 62.9 | 0.89 | 0.82 | 0.89 | 32.9 |
| West: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 10 | L2 | 14 | 53.8 | 14 | 53.8 | 0.022 | 19.2 | LOS B | 0.2 | 1.8 | 0.58 | 0.63 | 0.58 | 34.9 |
| 11 | T1 | 1282 | 7.0 | 1282 | 7.0 | 0.767 | 21.8 | LOS B | 14.0 | 103.8 | 0.92 | 0.85 | 0.96 | 43.0 |
| Approach | | 1296 | 7.5 | 1296 | 7.5 | 0.767 | 21.8 | LOS B | 14.0 | 103.8 | 0.91 | 0.85 | 0.95 | 42.9 |
| All Vehicles | | 3155 | 7.5 | 3155 | 7.5 | 0.767 | 23.1 | LOS B | 14.0 | 103.8 | 0.87 | 0.79 | 0.89 | 38.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.


| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | | |
| P2 | East Full Crossing | 37 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| P3 | North Full Crossing | 37 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| P4 | West Full Crossing | 37 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| All Pedestrians | | 111 | 34.3 | LOS D | | | 0.93 | 0.93 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: SC [SCHOFIELDS / CUDGEGONG - PM EXIST + GROWTH + DEVT]

 Network: N101 [Tallawong Existing PM + GROWTH + DEVT]

SCHOFIELDS ROAD / CUDGEGONG ROAD PM EXIST + GROWTH + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| East: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 5 | T1 | 1137 | 2.2 | 1137 | 2.2 | 0.648 | 15.8 | LOS B | 9.2 | 65.4 | 0.82 | 0.73 | 0.82 | 39.7 |
| 6 | R2 | 308 | 6.5 | 308 | 6.5 | 0.768 | 43.0 | LOS D | 3.5 | 26.2 | 1.00 | 0.91 | 1.27 | 25.2 |
| Approach | | 1445 | 3.1 | 1445 | 3.1 | 0.768 | 21.6 | LOS B | 9.2 | 65.4 | 0.86 | 0.77 | 0.92 | 35.3 |
| North: CUDGEGONG ROAD | | | | | | | | | | | | | | |
| 7 | L2 | 287 | 8.1 | 287 | 8.1 | 0.441 | 31.1 | LOS C | 5.7 | 42.6 | 0.99 | 0.85 | 0.99 | 35.1 |
| 9 | R2 | 161 | 7.2 | 161 | 7.2 | 0.529 | 40.1 | LOS C | 3.5 | 25.7 | 1.00 | 0.81 | 1.00 | 16.6 |
| Approach | | 448 | 7.7 | 448 | 7.7 | 0.529 | 34.3 | LOS C | 5.7 | 42.6 | 0.99 | 0.83 | 0.99 | 29.8 |
| West: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 10 | L2 | 6 | 0.0 | 6 | 0.0 | 0.007 | 17.1 | LOS B | 0.1 | 0.6 | 0.63 | 0.62 | 0.63 | 36.2 |
| 11 | T1 | 1399 | 3.5 | 1399 | 3.5 | 0.804 | 22.0 | LOS B | 14.3 | 103.0 | 0.97 | 0.92 | 1.04 | 42.9 |
| Approach | | 1405 | 3.5 | 1405 | 3.5 | 0.804 | 22.0 | LOS B | 14.3 | 103.0 | 0.97 | 0.92 | 1.04 | 42.8 |
| All Vehicles | | 3299 | 3.9 | 3299 | 3.9 | 0.804 | 23.5 | LOS B | 14.3 | 103.0 | 0.92 | 0.84 | 0.98 | 38.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | | |
| P2 | East Full Crossing | 37 | 29.3 | LOS C | 0.1 | 0.1 | 0.92 | 0.92 | |
| P3 | North Full Crossing | 21 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| P4 | West Full Crossing | 21 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| All Pedestrians | | 79 | 29.3 | LOS C | | | 0.92 | 0.92 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: STR [SCHOFIELDS / TALLAWONG / RIDGELINE AM
EXIST + GROWTH + DEVT]

 Network: N101 [Tallawong
Existing AM + GROWTH +
DEVT]

SCHOFIELDS / TALLAWONG / RIDGELINE AM EXIST + GROWTH + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 80 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h |
| South: RIDGELINE DRIVE | | | | | | | | | | | | | |
| 1 | L2 | 154 | 4.1 | 154 | 4.1 | 0.214 | 23.2 | LOS B | 2.5 | 17.8 | 0.71 | 0.75 | 42.5 |
| 2 | T1 | 246 | 1.7 | 246 | 1.7 | 0.562 | 30.4 | LOS C | 5.4 | 38.1 | 0.94 | 0.78 | 30.7 |
| 3 | R2 | 189 | 3.9 | 189 | 3.9 | 0.922 | 59.0 | LOS E | 5.7 | 41.1 | 1.00 | 1.10 | 20.9 |
| Approach | | 589 | 3.0 | 589 | 3.0 | 0.922 | 37.7 | LOS C | 5.7 | 41.1 | 0.90 | 0.87 | 30.4 |
| East: SCHOFIELDS ROAD | | | | | | | | | | | | | |
| 4 | L2 | 29 | 0.0 | 29 | 0.0 | 0.033 | 18.0 | LOS B | 0.3 | 2.3 | 0.49 | 0.65 | 44.8 |
| 5 | T1 | 926 | 6.1 | 926 | 6.1 | 0.893 | 45.1 | LOS D | 13.1 | 95.7 | 1.00 | 1.04 | 33.1 |
| 6 | R2 | 94 | 13.5 | 94 | 13.5 | 0.635 | 50.2 | LOS D | 2.4 | 18.9 | 1.00 | 0.80 | 20.3 |
| Approach | | 1049 | 6.6 | 1049 | 6.6 | 0.893 | 44.8 | LOS D | 13.1 | 95.7 | 0.99 | 1.01 | 32.4 |
| North: TALLAWONG ROAD | | | | | | | | | | | | | |
| 7 | L2 | 41 | 17.9 | 41 | 17.9 | 0.064 | 28.1 | LOS B | 0.8 | 6.6 | 0.87 | 0.74 | 17.8 |
| 8 | T1 | 420 | 2.8 | 420 | 2.8 | 0.969 | 61.1 | LOS E | 13.1 | 94.0 | 1.00 | 1.13 | 24.7 |
| 9 | R2 | 180 | 7.6 | 180 | 7.6 | 0.449 | 43.5 | LOS D | 2.1 | 15.9 | 0.98 | 0.77 | 29.3 |
| Approach | | 641 | 5.1 | 641 | 5.1 | 0.969 | 54.1 | LOS D | 13.1 | 94.0 | 0.99 | 1.00 | 25.7 |
| West: SCHOFIELDS ROAD | | | | | | | | | | | | | |
| 10 | L2 | 80 | 1.3 | 80 | 1.3 | 0.089 | 18.5 | LOS B | 1.1 | 7.5 | 0.59 | 0.69 | 37.8 |
| 11 | T1 | 971 | 6.5 | 971 | 6.5 | 0.930 | 49.7 | LOS D | 15.1 | 110.2 | 1.00 | 1.17 | 23.3 |
| 12 | R2 | 151 | 2.8 | 151 | 2.8 | 0.960 | 67.6 | LOS E | 4.9 | 34.9 | 1.00 | 1.15 | 28.3 |
| Approach | | 1201 | 5.7 | 1201 | 5.7 | 0.960 | 49.8 | LOS D | 15.1 | 110.2 | 0.97 | 1.14 | 24.8 |
| All Vehicles | | 3481 | 5.4 | 3481 | 5.4 | 0.969 | 47.0 | LOS D | 15.1 | 110.2 | 0.97 | 1.03 | 28.3 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | |
| P1 | South Full Crossing | 21 | 34.3 | LOS D | 0.0 | 0.0 | 0.93 | 0.93 |
| P2 | East Full Crossing | 21 | 34.3 | LOS D | 0.0 | 0.0 | 0.93 | 0.93 |
| P3 | North Full Crossing | 21 | 34.3 | LOS D | 0.0 | 0.0 | 0.93 | 0.93 |
| P4 | West Full Crossing | 21 | 34.3 | LOS D | 0.0 | 0.0 | 0.93 | 0.93 |

MOVEMENT SUMMARY

 Site: STR [SCHOFIELDS / TALLAWONG / RIDGELINE PM
EXIST + GROWTH + DEVT]

 Network: N101 [Tallawong
Existing PM + GROWTH + DEVT]

SCHOFIELDS / TALLAWONG / RIDGELINE PM EXIST + GROWTH + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| South: RIDGELINE DRIVE | | | | | | | | | | | | | | |
| 1 | L2 | 101 | 4.2 | 101 | 4.2 | 0.159 | 22.9 | LOS B | 1.5 | 10.7 | 0.73 | 0.74 | 0.73 | 42.6 |
| 2 | T1 | 280 | 0.4 | 280 | 0.4 | 0.907 | 44.0 | LOS D | 7.3 | 51.1 | 1.00 | 1.10 | 1.55 | 25.2 |
| 3 | R2 | 153 | 0.7 | 153 | 0.7 | 0.715 | 41.5 | LOS C | 3.4 | 24.0 | 1.00 | 0.87 | 1.18 | 25.9 |
| Approach | | 534 | 1.2 | 534 | 1.2 | 0.907 | 39.3 | LOS C | 7.3 | 51.1 | 0.95 | 0.97 | 1.29 | 29.1 |
| East: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 4 | L2 | 35 | 15.2 | 35 | 15.2 | 0.031 | 9.6 | LOS A | 0.3 | 2.1 | 0.49 | 0.64 | 0.49 | 49.9 |
| 5 | T1 | 905 | 4.0 | 905 | 4.0 | 0.831 | 33.7 | LOS C | 10.3 | 74.0 | 1.00 | 0.94 | 1.11 | 37.3 |
| 6 | R2 | 81 | 0.0 | 81 | 0.0 | 0.439 | 42.9 | LOS D | 1.8 | 12.6 | 1.00 | 0.77 | 1.00 | 22.4 |
| Approach | | 1021 | 4.0 | 1021 | 4.0 | 0.831 | 33.6 | LOS C | 10.3 | 74.0 | 0.98 | 0.91 | 1.08 | 36.7 |
| North: TALLAWONG ROAD | | | | | | | | | | | | | | |
| 7 | L2 | 60 | 8.8 | 60 | 8.8 | 0.099 | 27.5 | LOS B | 1.1 | 8.2 | 0.90 | 0.75 | 0.90 | 18.0 |
| 8 | T1 | 222 | 2.8 | 222 | 2.8 | 0.731 | 36.5 | LOS C | 4.9 | 35.4 | 1.00 | 0.88 | 1.11 | 32.3 |
| 9 | R2 | 337 | 4.7 | 337 | 4.7 | 0.812 | 43.5 | LOS D | 3.9 | 28.7 | 1.00 | 0.92 | 1.27 | 29.4 |
| Approach | | 619 | 4.4 | 619 | 4.4 | 0.812 | 39.4 | LOS C | 4.9 | 35.4 | 0.99 | 0.89 | 1.18 | 29.9 |
| West: SCHOFIELDS ROAD | | | | | | | | | | | | | | |
| 10 | L2 | 61 | 17.2 | 61 | 17.2 | 0.053 | 8.8 | LOS A | 0.4 | 2.8 | 0.37 | 0.63 | 0.37 | 47.0 |
| 11 | T1 | 977 | 3.6 | 977 | 3.6 | 0.889 | 37.3 | LOS C | 12.3 | 87.8 | 1.00 | 1.09 | 1.36 | 27.5 |
| 12 | R2 | 140 | 6.0 | 140 | 6.0 | 0.799 | 44.8 | LOS D | 3.3 | 24.3 | 1.00 | 0.93 | 1.36 | 34.3 |
| Approach | | 1178 | 4.6 | 1178 | 4.6 | 0.889 | 36.7 | LOS C | 12.3 | 87.8 | 0.97 | 1.05 | 1.31 | 29.2 |
| All Vehicles | | 3352 | 3.8 | 3352 | 3.8 | 0.907 | 36.7 | LOS C | 12.3 | 87.8 | 0.97 | 0.96 | 1.21 | 31.9 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | | |
| P1 | South Full Crossing | 21 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| P2 | East Full Crossing | 21 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| P3 | North Full Crossing | 21 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| P4 | West Full Crossing | 21 | 29.3 | LOS C | 0.0 | 0.0 | 0.92 | 0.92 | |
| All Pedestrians | | 84 | 29.3 | LOS C | | | 0.92 | 0.92 | |

MOVEMENT SUMMARY

 Site: TT [Tallawong / Themeda Existing AM + GROWTH + DEVT]

 Network: N101 [Tallawong Existing AM + GROWTH + DEVT]

Tallawong / Themeda Existing AM + GROWTH + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 80 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | | |
|---------------------------------|------|--------------|----------|---------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|------|
| Mov ID | Turn | Demand Total | Flows HV | Arrival Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed | |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h | |
| South: Tallawong Road | | | | | | | | | | | | | | |
| 2 | T1 | 274 | 10.0 | 274 | 10.0 | 0.117 | 9.8 | LOS A | 2.4 | 18.4 | 0.67 | 0.55 | 0.67 | 37.2 |
| 3 | R2 | 148 | 9.9 | 148 | 9.9 | 0.489 | 44.4 | LOS D | 3.6 | 27.4 | 1.00 | 0.81 | 1.00 | 12.6 |
| Approach | | 422 | 10.0 | 422 | 10.0 | 0.489 | 22.0 | LOS B | 3.6 | 27.4 | 0.79 | 0.64 | 0.79 | 23.9 |
| East: Themeda Ave | | | | | | | | | | | | | | |
| 4 | L2 | 184 | 10.9 | 184 | 10.9 | 0.503 | 38.2 | LOS C | 4.3 | 32.9 | 1.00 | 0.82 | 1.00 | 26.0 |
| 6 | R2 | 41 | 5.1 | 41 | 5.1 | 0.108 | 37.4 | LOS C | 0.9 | 6.7 | 0.95 | 0.74 | 0.95 | 27.3 |
| Approach | | 225 | 9.8 | 225 | 9.8 | 0.503 | 38.1 | LOS C | 4.3 | 32.9 | 0.99 | 0.81 | 0.99 | 26.2 |
| North: Tallawong Road | | | | | | | | | | | | | | |
| 7 | L2 | 241 | 0.4 | 241 | 0.4 | 0.525 | 25.2 | LOS B | 7.1 | 50.0 | 0.82 | 0.78 | 0.82 | 10.8 |
| 8 | T1 | 527 | 3.4 | 527 | 3.4 | 0.525 | 20.4 | LOS B | 7.2 | 51.8 | 0.82 | 0.73 | 0.82 | 11.8 |
| Approach | | 768 | 2.5 | 768 | 2.5 | 0.525 | 21.9 | LOS B | 7.2 | 51.8 | 0.82 | 0.74 | 0.82 | 11.5 |
| All Vehicles | | 1416 | 5.9 | 1416 | 5.9 | 0.525 | 24.5 | LOS B | 7.2 | 51.8 | 0.84 | 0.72 | 0.84 | 19.6 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|--------------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian ped | Prop. Queued | Effective Stop Rate | | |
| P1 | South Full Crossing | 53 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| P2 | East Full Crossing | 53 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| P3 | North Full Crossing | 53 | 34.3 | LOS D | 0.1 | 0.1 | 0.93 | 0.93 | |
| All Pedestrians | | 158 | 34.3 | LOS D | | | 0.93 | 0.93 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

MOVEMENT SUMMARY

 Site: TT [Tallawong / Themeda Existing PM + growth + DEVT]  Network: N101 [Tallawong Existing PM + GROWTH + DEVT]

Tallawong / Themeda Existing PM + growth + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Optimum Cycle Time - Minimum Delay)

| Movement Performance - Vehicles | | | | | | | | | | | | | |
|---------------------------------|------|--------------------|------------------|-------------|----------|-----------|---------------|------------------|------------------------------|--------------|---------------------|------------------|---------------|
| Mov ID | Turn | Demand Flows Total | Arrival Flows HV | Flows Total | Flows HV | Deg. Satn | Average Delay | Level of Service | Aver. Back of Queue Vehicles | Prop. Queued | Effective Stop Rate | Aver. No. Cycles | Average Speed |
| | | veh/h | % | veh/h | % | v/c | sec | | veh | m | | | km/h |
| South: Tallawong Road | | | | | | | | | | | | | |
| 2 | T1 | 182 | 5.8 | 182 | 5.8 | 0.072 | 8.3 | LOS A | 1.5 | 10.7 | 0.72 | 0.58 | 39.5 |
| 3 | R2 | 409 | 3.6 | 409 | 3.6 | 0.495 | 27.4 | LOS B | 7.8 | 56.1 | 0.96 | 0.86 | 18.0 |
| Approach | | 592 | 4.3 | 592 | 4.3 | 0.495 | 21.5 | LOS B | 7.8 | 56.1 | 0.89 | 0.77 | 22.7 |
| East: Themeda Ave | | | | | | | | | | | | | |
| 4 | L2 | 124 | 12.7 | 124 | 12.7 | 0.464 | 36.1 | LOS C | 2.6 | 20.0 | 0.98 | 0.79 | 26.8 |
| 6 | R2 | 54 | 2.0 | 54 | 2.0 | 0.187 | 34.2 | LOS C | 1.1 | 7.6 | 0.95 | 0.75 | 28.5 |
| Approach | | 178 | 9.5 | 178 | 9.5 | 0.464 | 35.5 | LOS C | 2.6 | 20.0 | 0.97 | 0.78 | 27.4 |
| North: Tallawong Road | | | | | | | | | | | | | |
| 7 | L2 | 21 | 10.0 | 21 | 10.0 | 0.474 | 37.0 | LOS C | 2.4 | 17.1 | 0.97 | 0.77 | 8.1 |
| 8 | T1 | 211 | 2.5 | 211 | 2.5 | 0.474 | 32.2 | LOS C | 2.4 | 17.2 | 0.97 | 0.76 | 8.2 |
| Approach | | 232 | 3.2 | 232 | 3.2 | 0.474 | 32.7 | LOS C | 2.4 | 17.2 | 0.97 | 0.76 | 8.2 |
| All Vehicles | | 1001 | 4.9 | 1001 | 4.9 | 0.495 | 26.6 | LOS B | 7.8 | 56.1 | 0.92 | 0.77 | 20.9 |

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

| Movement Performance - Pedestrians | | | | | | | | | |
|------------------------------------|---------------------|-------------------|-------------------|------------------|----------------------------------|--------------|---------------------|------|--|
| Mov ID | Description | Demand Flow ped/h | Average Delay sec | Level of Service | Average Back of Queue Pedestrian | Prop. Queued | Effective Stop Rate | | |
| | | | | | ped | | | | |
| P1 | South Full Crossing | 53 | 29.3 | LOS C | 0.1 | 0.1 | 0.92 | 0.92 | |
| P2 | East Full Crossing | 53 | 29.3 | LOS C | 0.1 | 0.1 | 0.92 | 0.92 | |
| P3 | North Full Crossing | 53 | 29.3 | LOS C | 0.1 | 0.1 | 0.92 | 0.92 | |
| All Pedestrians | | 158 | 29.3 | LOS C | | | 0.92 | 0.92 | |

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Appendix G

Car Park Scheme Advice

SHIPROCK ADVISORY

PO Box 1020 Caringbah NSW 1495 · Phone: 0439 646 168

Email: peter@shirprockadvisory.com.au

15 April 2020

Greg Colbran
Development and Planning Executive
Deicorp
Level 3,
161 Redfern Street
Redfern NSW 2016

Dear Greg

**Proposed Mixed Use Development incorporating Retail and Commercial
Tallawong Station Precinct South- Parking allocation for Retail/Commercial**

We refer to the proposed mixed-use site at the above location.

We commenced a retail/commercial marketing campaign in November 2019 to seek interest to lease out the retail and commercial at the above site.

This has involved presentations and meetings with Coles, Woolworths and other Supermarket operators (Aldi, Metcash, Independents) and associated support specialty shops such as; Fresh Food, Food and Beverage, Restaurants, Services, Gym, Child Care, medical, pharmacy and office tenants.

As these conversations commenced last year, we have found some major concerns being raised from our prospective tenants that the proposed parking allocation of 147 retail/commercial car spaces.

The comments from tenants and from my experience is that the parking allocation as proposed is inadequate to meet the customer needs of the Retail/Commercial precinct which will impact the viability of the Centre.

In my experience and if we are unable to achieve adequate parking this will impact on being able to secure retailers and or the appropriate retailers for the site to be able to have a viable business in this location.

Tallawong is an emerging greenfield location which will be a densely populated location in the future however in the meantime the community currently drives to all the retail centers within the catchment.

Whilst I note we have an adjoining commuter car park for the metro station which has been found to be at capacity by 6am in the morning.

Therefore, the proposed amended retail/commercial of 300 car spaces would be adequate to accommodate the parking requirements for a 9000 sqm Retail/Commercial Centre as proposed at Tallawong.

I would welcome the opportunity to discuss this further as to concerns raised on the parking allocation.

Any questions please contact me.

Yours Sincerely

A handwritten signature in blue ink, appearing to read 'Peter Mokas', with a large loop at the top and a cross-like mark at the bottom.

Peter Mokas



14th April 2020

CBRE (RP) Pty Ltd
ABN 92 127 174 207

Level 29, 177 Pacific Hwy
North Sydney NSW 2060

T 61 2 8969 8500
F 61 2 8969 8599

www.cbre.com.au

Att: John Vamvakaris
Chief Operations Officer-Development
DEICORP
Level 4, 161 Redfern Street
Redfern NSW 2016

Dear John,

Re: Tallawong Car Park Scheme

As one of Australia's largest property services companies, CBRE has a deep understanding of the drivers of demand for residential properties across the country.

Our extensive experience in the sales and marketing of residential properties in new release areas like Sydney's North West enable us to accurately advise on the expectations and preferences of purchasers.

This experience confirms that purchasers of apartments in new development areas across the North West, including purchasers of 1-bedroom apartments, have a strong preference for car parking linked to their apartment. Whilst the major appeal of these areas is affordability and greatly improved transport as a result of the new train line, buyers rely on personal transport i.e. their own car for access to schools sporting amenities visiting friends and family etc. They rely on transport by train mainly for access to and from work, also other city-based events.

Sales of 1-bedroom apartments that do not include parking is significantly slower and results in a disproportionately lower price than comparable apartments with parking provided.

Our strong advice to developers of projects in the North West of Sydney is that a decision to not provide parking for all 1-bedroom apartments will dramatically reduce the saleability and demand for these dwellings.

We believe the correct scheme for Tallawong apartments needs to be as tabled below:

| | |
|-------|-------------|
| 1 bed | 1 car space |
| 2 bed | 1 car space |
| 3 bed | 2 car space |

If you have any questions, please do not hesitate to contact me on 0418200236.

Yours sincerely,



David Milton
CBRE
Managing Director
Residential Projects