SSD 10291 St Luke's Grammar School, Dee Why

Proposed Senior School Campus & Sports Centre

TRAFFIC AND PARKING ASSESSMENT REPORT

9 June 2020

Ref 19090



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1.0 INTRODUCTION

This report has been prepared to accompany an Environmental Impact Statement to the *NSW* Department of Planning for the proposed staged expansion of the school facilities at the *St* Luke's Grammar School, Dee Why (Figures 1 and 2).

The existing *St Luke's Grammar School* comprises a pre-school and K-Year 12 coeducational school located at No. 210 Headland Road. It currently has a maximum enrolment of 1022 students, with approval to increase to 1092 students and 125 full-time equivalent staff (MOD 2018/0412, approved 26 June 2019).

1.1 Project Summary

St Luke's Grammar School proposes to expand its existing facilities to incorporate No. 224 Headland Road as a *sports centre*, and No. 800 Pittwater Road as a designated *senior campus* for Year 10-12 students only. The proposed expansion will involve *adaptive re-use* of the existing buildings on the sites.

The proposed development will result in a significantly *less intensive* use of the site in traffic terms when compared with the existing commercial uses of the site. Surveys indicate that daily traffic flows generated by the site will be reduced from more than 2931 vehicles per day to 740 vehicles per day after the senior school campus is fully developed.

Car parking on the *senior campus* site will also be reduced as the commercial tenants vacate the site, from 182 parking spaces at present, to 131 spaces whilst Office Works remains on the site, and then down to 91 spaces when the senior school campus is fully developed.

The *sports centre* proposed at 224 Headland Road will also result in a *less intensive* use of the site in traffic terms, with daily traffic flows expected to be reduced from 201 vehicles per day to 78 vehicles per day, with car parking on the site to be reduced from 45 parking spaces to 39 parking spaces.

1.2 Projected Enrolment

The proposed expansion of *St Luke's Grammar School* has been designed to accommodate future-focused principles of learning and to meet projected enrolment growth, recognising increasing demand in the local catchment area. The design has been developed through comprehensive consultation with the school.

The expanded school will ultimately have a capacity of 1600 students comprising:

- 1000 pre-school to Year 9 students, with 120 staff at 210 Headland Road, and
- 600 Year 10-12 students, with 60 staff at 800 Pittwater Road.

The *sports centre* proposed at No. 224 Headland Road will be shared by both school campuses.

The proposed development will be undertaken in three stages as follows:

- Stage 1: sports centre at 224 Headland Road commencing mid-2022
- Stage 2: senior school campus at 800 Pittwater Road commencing mid-2025
- Stage 3: senior school campus at 800 Pittwater Road commencing mid-2029.

Enrolments at the new senior school campus are expected to increase incrementally as students progress through the school years towards Year 12, and to suit site availability.

1.3 Purpose of this Report

The purpose of this report is to assess the traffic and parking implications of the proposed development and to that end this report:

• describes the site and provides details of the development proposal

- reviews the road network in the vicinity of the site
- reviews the school bus and other public transport/bus services in the vicinity of the site
- reviews the results of traffic surveys conducted at the existing school, at the two commercial sites which are the subject of this application, and at the two nearby intersections nominated by TfNSW and the RMS
- estimates the traffic generation potential of the development proposal
- assesses the traffic implications of the development proposal in terms of road network capacity
- reviews the geometric design features of the proposed car parking facilities for compliance with the relevant codes and standards
- assesses the adequacy and suitability of the quantum of off-street car parking and loading provided on the site.





2.0 SECRETARY'S ENVIRONMENTAL ASSESSMENT REQUIREMENTS

2.1 SEARS

This report addresses the Secretary's Environmental Assessment Requirements (SEARs) for the State Significant Development (SSD), SSD 10291. Table 1 below details where the responses to each of the items raised in *Section 7: Transport and Accessibility* of the SEARs have been addressed in this report.

Table 1: Secretary's Environmental Assessment Requirements			
9. Transport & Accessibility	Report Section		
Accurate details of the current daily and peak hour vehicle, existing and future public	Chapter 2		
transport networks and pedestrian and cycle movement provided on the road network			
located adjacent to the proposed development			
Details of estimated total daily and peak hour trips generated by the proposal,	Chapter 6		
including vehicle, public transport, pedestrian and bicycle trips based on surveys of the			
existing and similar schools within the local area			
The adequacy of existing public transport or any future public transport infrastructure	Chapter 6		
to meet the likely future demand of the proposed development			
Measures to integrate the development with the existing/future public transport	Chapters 5 & 6		
network			
The impact of trips generated by the development on nearby intersections	Chapter 6		
The identification of infrastructure required to ameliorate any impacts on traffic	Chapters 5 & 6		
efficiency and road safety impacts associated with the proposed development,	See also Green Travel		
including details on improvements required to affected intersections, additional school	Plan		
bus routes along bus capable road, additional bus stops or bus bays			
Details of travel demand management measures to minimise the impact on general	Chapters 5 & 6		
traffic and bus operations, including details of a location-specific sustainable travel	See also Green Travel		
plan (Green Travel Plan and specific Workplace Travel Plan) and the provision of	Plan		
facilities to increase the non-car mode share for travel to and from the site			
The proposed walking and cycling access arrangements and connections to public	Chapter 5		
transport services			
The proposed access arrangements, including car and bus pick-up/drop-off facilities,	Chapters 5 & 6		
and measures to mitigate any associated traffic impacts and impacts on public			
transport, pedestrian and bicycle networks			
Proposed bicycle parking provision, including end-of-trip facilities, in secure,	Chapter 5		
convenient, accessible areas close to main entries incorporating lighting and passive			
surveillance			

Proposed number of on-site car parking spaces for teaching staff and visitors and	Chapters 5 & 7
corresponding compliance with existing parking codes and justification for the level of	
car parking provided on-site	
An assessment of the cumulative on-street parking impacts of cars and bus pick-	Chapter 7
up/drop-off, staff parking and any other parking demands associated with the	
development	
An assessment of road and pedestrian safety adjacent to the proposed development and	Chapter 5
the details of required road safety measures and personal safety in line with CPTED	
Emergency vehicle access, service vehicle access, delivery and loading arrangements	Chapter 5
and estimated service vehicle movements	
The preparation of a preliminary Construction Traffic and Pedestrian Management	See separate CTMP
Plan to demonstrate the proposed management of the impact in relation to construction	
traffic	
Investigation of the possibility of removing vehicle access to Harbord Road by	Chapter 5
accessing the development via Headland Road	
Provision of safe and convenient pedestrian access between the existing and future	Chapter 5
campus for all users	
Pick-up and Drop-off areas designed to minimise impact of vehicle movements on the	Chapter 5
intersection of Pittwater Road with Harbord Road	
Vehicle access design that does not require the implementation of a school zone along	Chapter 6
Pittwater Road	
Investigations into potential internal pedestrian and vehicle connectivity between 224	Chapter 5
Headland Road and 800 Pittwater Road	

2.2 RMS Requirements

Discussions were also held with officers of the Roads and Maritime Services at a pre-DA meeting held on 7 November 2019. The RMS raised a number of matters to be addressed. Table 2 below details where responses to each of the matters raised by the RMS have been addressed in this report.

Table 2: Roads and Maritime Services Requirements			
RMS Matters	Report Section		
Applicant to investigate the possibility of removing all vehicular access from Harbour	Chapter 5		
Road			
Roads and Maritime request that modelling is conducted to reflect the changes to the	Chapter 6		
proposed development. Daily and peak traffic movements likely to be generated by			
the proposed development including the impact of nearby intersections and the			
need/associated funding for upgrading or road improvement works (if required). Key			
intersections to be examined and modelled include:			

• Pittwater Road/Harbord Road – Existing, Proposed with no 40 km/h school zone	
and	
 Proposed with 40 km/h School Zone on all four legs of the signalised 	
intersection. Harbord Road/Headland Road – Existing and Proposed	
Details of the proposed accesses and the parking provisions associated with the	Chapter 7
proposed development including compliance with the requirements of the relevant	
Australian Standards (ie: turn paths, sight distance requirements, aisle widths, etc.) and	
relevant parking codes (if parking is to be changed)	
Details of service vehicle movements (including vehicle type and likely arrival and	Chapter 7
departure times)	
Roads and Maritime require justification for the large amount of onsite parking needed	Chapters 5 & 7
for St Luke's Grammar School and if car parking can be removed from 800 Pittwater	
Road, Dee Why	
Roads and Maritime require further clarification in relation to the mixed use of the site	Chapter 5
until 2026 with Office Works Dee Why. How is the school going to manage this and	
how it will operate?	
Roads and Maritime requires the environmental assessment report to assess the	Chapters 5 & 6
implications of the proposed development for non-car travel modes (including public	
transport use, walking and cycling); the potential for implementing a location-specific	
sustainable travel plan and the provision of facilities to increase the non-car mode	
share for travel to and from the site. This will entail an assessment of the accessibility	
of the development site by public transport.	
Further information in relation to school access points along Pittwater Road.	Chapter 5

2.3 Relevant Policies and Guidelines:

- Guide to Traffic Generating Developments (Roads and Maritime Services)
- EIS Guidelines Road and Related Facilities (DoPI)
- Cycling Aspects of Austroads Guides
- NSW Planning Guidelines for Walking and Cycling
- Austroads Guide to Traffic Management Part 12: Traffic Impacts of Development
- Standards Australia AS2890.3 (Bicycle Parking Facilities)
- Warringah DCP 2011 Appendix 1 Car Parking Requirements

3.0 EXISTING SITE DEVELOPMENTS

3.1 Existing School Site

St Luke's Grammar School (SLGS) is located in Dee Why and North Curl Curl on Sydney's Northern Beaches.

The existing school is located at 210 Headland Road and comprises a number of school buildings, including a junior block, middle-school block and senior block, as well as a resource centre, multi-purpose hall, playing courts, a playing field and grassed areas.

The existing school site has frontages to Headland Road, Quirk Street, Tango Avenue and the Stony Range Regional Botanic Garden.

A recent aerial image of the existing SLGS and its surroundings is reproduced below, showing the locations of the proposed *sports centre* and *senior campus* in the context of the existing school and the surrounding local road network.



3.2 Existing Developments at No. 224 Headland Road

No. 224 Headland Road is currently occupied by an existing industrial/warehouse unit building comprising 7 tenancies. A number of those tenancies are already occupied by SLGS, to accommodate existing sporting facilities used by the school and the school uniform shop. Off-street car parking is currently provided for a total of 45 cars located in an outdoor at-grade parking area.

Vehicular access to the site is provided via a two-way entry/exit driveway crossover located on the Headland Road site frontage.

Pedestrian access to No. 224 Headland Road is provided via a separate pedestrian path which connects the public footpath in Headland Road, with a dedicated pedestrian pathway separate from the driveway (which is to be provided as part of a recently approved DA) within the site which traverses the entire length of the building.

3.3 Existing Developments at No. 800 Pittwater Road

No. 800 Pittwater Road is located opposite the intersection of Pittwater Road, Harbord Road and Warringah Road. The existing heritage-significant building is currently occupied by an *Office Works* retail outlet, a *Fitness First* gymnasium and swimming pool (including swim school), and an *I-MED* radiology clinic.

Off-street parking is currently provided for 182 cars at various locations throughout the site, with the majority of the parking (i.e. 93 spaces) provided in a basement car parking area located beneath the existing building.

Vehicular access to the basement parking area is provided within the site via an entry-only ramp along the northern boundary of the site and an exit-only ramp located towards the south-western corner of the existing building.

Vehicular access to the site is provided via a two-way entry/exit driveway off the Harbord Road site frontage. The existing vehicular access driveway has been designed to accommodate 12.5m long HRV rigid trucks, and includes a splitter island inside the property

boundary which separates the entry and exit movements. A raised concrete central median island in Harbord Road restricts turning movements to Left-In/Left-Out (LILO) movements only.

Loading/servicing for the existing development is currently undertaken by a variety of trucks up to and including 12.5m long HRV rigid trucks. A dedicated loading dock is located in the south-eastern corner of the site.

Pedestrian access to the site is provided via a *pedestrian-only* gate located in the northwestern corner of the site which connects with the public footpath that extends along the eastern side of Pittwater Road. There is a no other pedestrian access provided to the site at present.

3.4 Vehicular & Pedestrian Site Access Constraints – 800 Pittwater Road

It is pertinent to note that vehicular and pedestrian access to the 800 Pittwater Road site is *constrained* by the topographic features of the site. These features include:

- a concrete retaining wall constructed immediately behind the kerb in Pittwater Road which extends between Harbord Road and the car park entrance to the Stony Range Regional Botanic Gardens approximately 100m to the north of the site. The concrete retaining wall reaches a height of approximately 2m and is topped by a post and rail/wire mesh fence
- a second concrete retaining wall which generally follows the site boundary in Pittwater Road between the existing vehicular access driveway and the pedestrian gate located at the north-western corner of the site. The second concrete retaining wall is typically in the order of 1.5m in height and is topped by a steel guard rail along the site boundary. A second pedestrian safety/security fence is proposed along the entire Pittwater Rd site boundary to replace the existing guard rail.

Thus the public footpath along the eastern side of Pittwater Road along the site frontage is located *between* the two concrete retaining walls, each topped by a fence or guard rail, such that there is no pedestrian access between the *senior campus* site and traffic on the road

pavement on Pittwater Road except at the signalised pedestrian crossing at the Pittwater Road/Harbord Road intersection.

The rear of the 800 Pittwater Road site is separated from the 224 Headland Road site by a 20m high cliff. Stairs and a lift are proposed to connect the two sites as detailed later in this report.

Consideration has also been given to providing a vehicular link between the two sites at this location as required by the RMS, however the 20m difference in height between the two sites has proved to be insurmountable.

4.0 EXISTING SCHOOL OPERATIONAL CHARACTERISTICS

The existing *St Luke's Grammar School* comprises a pre-school and K-Year 12 coeducational school located at No. 210 Headland Road. It currently has a maximum enrolment of 1022 students, with approval to increase to 1092 students and 125 full-time equivalent staff.

4.1 Existing Off-Street Car Parking Facilities

The school provides a total of 133 off-street car parking spaces on the existing school site, including 112 spaces for staff plus 17 staff for students, as follows:

Off-Street Car	Parking
----------------	---------

133 spaces:	TOTAL			
1 space:	school deliveries off Headland Road			
1 space:	school visitors off Headland Road			
2 spaces:	disabled parking off Headland Road			
17 spaces:	student car parking off Headland Road			
8 spaces: school hall undercroft staff car park off Headla				
104 spaces:	multi-storey staff car park off Tango Avenue			

Vehicular access to the multi-storey staff car park off Tango Avenue is provided via a single two-way driveway located at the northern-end of the Tango Avenue site frontage. Vehicular access to the remaining car parking facilities is provided via two separate two-way driveways located approximately midway along the Headland Road frontage of the site.

4.2 Existing Kerbside Drop-Off & Pick-Up Arrangements

There are a number of drop-off/pick-up areas and bus stops located around the perimeter of the existing SLGS site in Headland Road, Quirk Street and Tango Avenue. These comprise:

• an indented bus bay in Headland Road which has the capacity to accommodate 3 buses simultaneously

- a bus stop in Quirk Street which has the capacity to accommodate a further 2 buses, should the need ever arise
- a drop-off/pick-up zone in Headland Road which has the capacity to accommodate two cars, and
- an indented drop-off/pick-up bay in tango Avenue which has the capacity to accommodate up to 12 cars.

4.3 School Traffic Management Plan

The drop-off and pick-up operations at the school are intensively managed by school staff in accordance with the schools' *Traffic Management Plan* (Appendix 1). There are four staff rostered each morning to supervise drop-offs, and 12 staff are rostered each afternoon to supervise pick-ups.

Parents/carers using the Tango Avenue drop-off/pick-up bay are not permitted to exit their vehicle. Drivers must remain the driver's seat, and a member of St Lukes staff will open the passenger side doors and assist children to exit or enter the car. To assist this operation in the afternoon, parents/carers must place their family name *label* on the passenger side of the dashboard or suspended form the passenger side sunvisor so that it can be seen by St Lukes staff. Students await inside the school grounds until called to the kerb by staff before their parents' vehicle arrives at the pick-up point.

Five cars are loaded at any one time in the 12 spaces. The next 5 cars then move forward, by which time each student is waiting at their designated pick-up point. Staff will then assist children into their vehicle.

The successful operation of the *Traffic Management Plan* is further enhanced by the staggered operating hours of the school, as follows:

•	Cottage Class Pre-School:	8:30am – 2:45pm (21 students)
•	Kindergarten to Year 2 (Junior School):	8:30am – 2:55pm (138 students)
•	Year 3 to Year 6 (Junior School):	8:30am - 3:05pm (205 students)

8:30am – 3:20pm (355 students)

• Year 7 to Year 9 (Middle School):

Year 10 to Year 12 (Senior School): 8:30am – 3:20pm (303 students)

Peak traffic activity during the morning drop-off period typically occurs between 8:10am – 8:25am. Although this is a busy period, drop-offs are quick and efficient, there is no waiting and delays are minimal, if any.

Peak traffic activity in the afternoon typically occurs between 3:00pm and 3:15pm, and is associated with the junior school finishing times of 2:55pm (Kindergarten to Year-2), and 3:05pm (Years 3 to 6). There are typically no queues in Tango Avenue outside this 15-minute peak period in the afternoons, or at the high school finishing time of 3:20pm (Years 7 to 12).

The School Traffic Management Plan requires all school buses to use the indented bus bay in Headland Road, except for 1 bus which approaches from the east and turns right into Quirk Street from Headland Road. Occasionally a bus approaching the Headland Road indented bus bay from the west will be directed around the corner to the Quirk Street stop if the indented bus bay is fully occupied.

Students catching a bus in the afternoon must wait inside the school grounds, in the school courtyard, and are called forward when their bus arrives. Students board their buses under staff supervision.

If a bus is unable to be accommodated in the indented bus bay in Headland Road, staff will ensure that the bus moves around the corner to the Quirk Street bus stop. This issue will be resolved with the recently approved extension of the indented bus bay in Headland Road.

4.4 Existing School Traffic Generation

In order to gain an accurate appreciation of the traffic and parking demands generated by the existing school, surveys were undertaken during the school's *morning* drop-off and *afternoon* pick-up periods on 19th September, 2019. The surveys revealed that:

[•] during the 2-hour morning drop-off period; between 7:00am and 9:00am:

- a total of 144 students travelled in 9 buses, including a total of 50 senior students (Years 10-12).
- a total of 373 students were dropped-off by 275 cars, including a total of 68 senior students (Years 10-12) dropped-off by 60 cars.
- during the 2-hour afternoon pick-up period; between 2:30pm and 4:30pm there were:
 - a total of 271 students travelled in 10 buses, including a total of 50 senior students (Years 10-12).
 - a total of 208 students were picked up by 143 cars, including a total of 33 senior students (Years 10-12) picked-up by 30 cars.

In summary, the drop-off/pick-up surveys conducted at the existing SLGS revealed that the typical bus occupancy rate generated by the school was *18 students per bus* during the *morning* drop-off period and *27 students per bus* during the *afternoon* pick-up period.

The car occupancy rate generated by the school was approximately *1 vehicle per 1.55* students during the morning drop-off period and approximately *1 vehicle per 1.45 students* during the *afternoon* pick-up periods.

Traffic activity generated by the school outside the 2 hours surveyed in both the morning and the afternoon is absolutely minimal, if any, apart from the late departure of some senior staff.

Accordingly, the cumulative daily traffic generation potential of the existing school including staff is in the order of 1,126 vehicles per day (vpd) comprising 563 vpd TO the school and 563 vpd FROM the school.

During the *1-hour* school peak period the surveys revealed that:

- during the 1-hour morning drop-off period:
 - a total of 294 K-Y9 students arrived in 196 cars, and

- a total of 60 Y10-Y12 students arrived in 52 cars
- during the *1-hour* afternoon pick-up period:
 - a total of 156 K-Y9 students were picked-up in 115 cars, and
 - a total of 28 Y10-Y12 students were picked-up in 26 cars.

The peak hour traffic generation characteristics of the existing school on a *per student* basis (i.e. excluding staff) is set out in the table below:

Existing School Traffic Generation						
(vehicles per hour/student)						
	AM Peak Hour PM Peak Hour				•	
	IN	OUT	TOTAL	IN	OUT	TOTAL
K-Y9	0.273	0.273	0.545	0.160	0.160	0.319
Y10-Y12	0.172	0.172	0.343	0.086	0.086	0.172

The survey data confirms that the senior students in Years 10 to 12 generate substantially *less traffic* on a *per student* basis than younger students, and reinforces the conclusions reached in an independent traffic study¹ undertaken for the *Sydney North Planning Panel* prior to its approval of MOD 2018/0412 on 26 June 2019 that:

- the peak volumes at the site coincide with the junior school operating hours, and
- senior school drop-off/pick-up activities are relatively minor.

The survey results also confirm on-site observations that there are typically no queues in the drop-off/pick-up area at the high school finishing time of 3:20pm (Years 7 to 12).

¹ The Transport Planning Partnership St Luke's Grammar School – Planning Panel Deferral Traffic and Parking Survey Results (7 May 2019) undertaken for the Sydney North Planning Panel.

5.0 PROPOSED DEVELOPMENT

St Luke's Grammar School proposes to expand its existing Dee Why school campus to incorporate No. 224 Headland Road as a *sports centre* (for all Years) and No. 800 Pittwater Road as designated *senior campus* for up to 600 Year 10-12 school students.

The proposed development involves the internal alteration to the existing buildings located at both locations. Additional work is proposed at the rear of No. 800 Pittwater Road site to provide a passenger lift and stair access to the adjoining site at No. 224 Headland Road.

A staged redevelopment is proposed to accommodate the individual lease agreements for the current tenancies located within the two existing buildings. The first space to become available is the building at No.224 Headland Road.

Key features of the works proposed in each stage are detailed below.

5.1 Stage 1 – No.224 Headland Road (Sports Centre)

- Current lease agreement end date 30 June 2022
- Conversion of existing teacher training room to school clothing store
- Remove existing internal wall partitions and mezzanine offices
- Basketball courts will also be used as a temporary Dance Studio space / Multi-Purpose Hall until the completion of the entire project
- At-grade car parking to be reconfigured and reduced from 45 spaces to 39 spaces for Year 12 students.

There will also be *no change* to the existing ground floor amenities, the half-basketball court and the vehicular access arrangements.

5.2 Stage 2 – No.800 Pittwater Road (Senior Campus)

- I-MED lease agreement end date 31 August 2024
- Fitness First lease agreement end date 30 June 2025
- Conversion of I-MED and Fitness First tenancies to school facilities

- Office Works Tenancy to remain operational on-site until Stage 3
- Car parking areas reconfigured, with parking reduced from 182 spaces to 131 spaces, including 60 spaces for staff, 15 spaces for students, 40 spaces for Office Works in accordance with DA Consent No. 97/59, with the remaining car parking to be allocated to the builders and visitors
- The 40 spaces for Office Works will be located in the at-grade car park at the front of the site. The remaining spaces in the at-grade car park will be allocated to the *senior campus*
- Reconfiguration of the at-grade car parking area in front of the building to provide a dedicated on-site drop-off/pick-up bay comprising 7 car spaces for the senior students only
- The reconfigured at-grade car parking area will have the capacity to accommodate 12.5m long large buses to cater for any school excursions when the drop-off/pick-up bay is *not* in use
- New passenger lift/stair access to connect the buildings at No. 224 Headland Road and at No. 800 Pittwater Road
- Construct a new pedestrian access to the *senior campus* off Harbord Road, directly opposite the signalised pedestrian crossing at the Pittwater Road/Harbord Road intersection
- Projected senior school campus of 360 students in 2026 with 36 staff, increasing to 480 Year 10-12 students with 48 staff in 2030.

5.3 Stage 3 – No.800 Pittwater Road (Senior Campus)

- Office Works lease agreement end date 30 April 2029
- Conversion of existing commercial tenancy to school facilities
- New extension to the south-western side of existing building
- Temporary classroom spaces in Stage 2 reconfigured
- New entrance to basement car park, with the existing entrance to be closed
- Car parking area reconfigured, with parking reduced from 131 spaces in Stage 2 to 91 spaces, comprising 60 staff car parking spaces and 25 student parking spaces and 5 visitor car parking spaces

- Construct a new pedestrian access to the *senior campus* off Harbord Road, directly opposite the signalised pedestrian crossings at the Pittwater Road/Harbord Road intersection
- Projected senior school campus enrolments to increase to 600 students with 60 staff after completion of Stage 3.

5.4 Projected Enrolment

As noted above, the proposed expansion seeks approval to increase student enrolments from 1,022 students to 1,600 students (i.e. 578 *additional* students) and a total of 160 full-time equivalent staff as follows:

- the new *senior campus* will ultimately accommodate 600 Year 10-12 senior school students only
- the existing school campus at 210 Headland Road will accommodate 1,000 pre-school to Year 9 students, with 100 full-time equivalent staff.

It is anticipated that the increase in student enrolment and staff numbers will occur progressively over a period of many years, with the *maximum* enrolment of 1,600 students not expected to be achieved earlier than 2030.

5.5 Proposed New On-Site Drop-Off/Pick-Up Bay

A new dedicated on-site drop-off/pick-up bay is proposed on the senior school campus, initially with a capacity of 7 spaces whilst Office Works remains on the site in the Stage 2, then increasing to 12 spaces in the drop-off/pick-up bay upon completion of Stage 3.

Parents with children in *both* campuses will drop-off/pick-up their children at the Tango Road indented bay only, and the older sibling will then *walk* down to the lower campus.

The proposed on-site drop-off/pick-up zone has been designed to accommodate the swept turning path of large vehicles up to and including the 12.5m HRV trucks – i.e. similar to a large bus or tourist coach which may be used for school excursions, when the drop-off/pick-up bay is not in use.

Emergency service vehicles, such as an ambulance, will be also able to enter and exit the new senior school site in a forward direction at all times, utilising the proposed drop-off/pick-up zone.

A new bicycle parking facility is proposed adjacent to the new drop-off/pick-up facility for the use of senior students and staff. End-of-trip facilities are proposed adjacent to the reconstructed swimming pool, with separate showers/change rooms for students and staff.

5.6 Proposed Internal Pedestrian Link Between the Two Sites

A new internal pedestrian link comprising stairs and a lift is proposed between 800 Pittwater Rod (proposed *senior campus*) and 224 Headland Road (proposed *sports centre*) to facilitate the safe and direct movement of students and staff between the two sites. The proposed new internal pedestrian link will enable students and staff to move directly between the two sites in a safe and efficient manner.

The proposed pedestrian link comprising stairs and a lift will overcome the physical barrier between the two sites presented by the 20m high cliff which currently separates the two sites.

Consideration has also been given to providing a vehicular link between the two sites at this location as required by the RMS, however the 20m difference in height between the two sites has proved to be insurmountable.

5.7 Improved Pedestrian Access Between Senior Campus & Public Transport Services

The only pedestrian access to the senior school campus at present is provided via a gate located at the north-western corner of the site. A new pedestrian entrance is therefore proposed at the south-western corner of the site, directly opposite the signalised pedestrian crossings at the Pittwater Road/Harbord Road intersection to better integrate the *senior campus* with public transport services on Pittwater Road, and to encourage active forms of transport such as walking to school.

The new pedestrian entrance will be constructed as an interim arrangement in Stage 2, and will then be upgraded in Stage 3. It will provide the safest and shortest possible route between the *senior campus* and the existing bi-directional bus stops located in Pittwater Road immediately to the south of the Harbord Road intersection.

The existing pedestrian gate located at the north-western corner of the site will be retained and will provide the shortest possible walking route between the *senior campus* and the B-Line service bus stops located further to the north in Pittwater Road, near Howard Street. It should be noted that this *pedestrian-only* gate is separated from road traffic on Pittwater Road by a 2m high concrete embankment topped by a pedestrian fence which extends along the kerb line in front of the site and further to the north beyond the site to the car park entrance to the *Stony Range Regional Botanic Gardens*.

Improved pedestrian accessibility to both the regular route bus services and also to the new B-Line Services will considerably enhance the sustainable public transport options available to the *senior campus* students and staff. The new *senior campus* will provide significantly improved pedestrian access to regular route bus services and the new B-Line services for Year 10-12 students and staff.

5.8 After School Hours uses of the Development

The school proposes to make available for *community uses* the swimming pool on the *senior campus* site, and the basketball courts on the *sports centre* site, on an after-school hours basis.

It is anticipated that 3 swim school classes and squad training sessions with 50 children may be held in the pool between 4pm-7pm.

The basketball courts may be used by up to 20 people with 2 training sessions anticipated between 4pm-7pm.

5.9 Plans of the Development Proposal

Plans of the proposed development have been prepared by *Tonkin Zulaikha Greer Architects* and are reproduced in the following pages.

Swept turning path diagrams which have been prepared in accordance with the requirements of *AS2890.1* and *AS2890.2* have been prepared using the *Autodesk Vehicle Tracking 2020* program are also reproduced in the following pages, confirming that all vehicles will be able to enter and exit the *senior campus* site whilst travelling in a forward direction at all times.

On some occasions it may be necessary to transport students from the junior school to the *sports centre* or the *senior campus*. This will be undertaken using a minibus such as a Toyota Coaster or similar sized community bus. Swept turning path analysis confirms that these minibuses can be accommodated at both sites, such that they will be able to enter and exit both sites while travelling in a forward direction at all times.









LEGEND







EXISTING ELEVATION EAST 1:200

5

DATE 92/0-320














REV

DATE at to 20













6.0 TRAFFIC ASSESSMENT

6.1 Road Hierarchy

The road hierarchy allocated to the road network in the vicinity of the site by the Roads and Maritime Authority is illustrated on Figure 3.

Pittwater Road is classified by the RMS as a *State Road* and provides the key north-south road link in the area, linking North Manly to Church Point. It typically carries two traffic lanes in each direction in the vicinity of the site, with opposing traffic flows separated by a central median island. Additional bus dedicated lanes are also located along both sides of Pittwater Road. Kerbside parking is generally permitted on both sides of the road outside of commuter peak periods.

Harbord Road is classified by the RMS as a *Regional Road* and follows a north-south alignment between Harbord and Brookvale. It typically carries two traffic lanes in each direction in the vicinity of the site, with kerbside parking permitted at selected locations.

Headland Road, Quirk Street and Tango Avenue are local, unclassified roads which are primarily used to provide vehicular and pedestrian access to frontage properties. Kerbside parking is generally permitted on both sides of all three roads. Headland Road performs the function of a *collector road*, connecting the local residential area to the *higher-order* classified RMS road network.

6.2 Existing Traffic Controls

The existing traffic controls which apply to the road network in the vicinity of the site are illustrated on Figure 4. Key features of those traffic controls are:

- a 60 km/h SPEED LIMIT which applies to Pittwater Road and Harbord Road
- a 50 km/h SPEED LIMIT which applies to Headland Road, Quirk Street, Tango Avenue and all other local roads in the area





- a 40 km/h SCHOOL ZONE SPEED LIMIT which applies to Harbord Road, Headland Road, Quirk Street and Tango Avenue in the vicinity of the *St Luke's Grammar School* and the *Northern Beaches Secondary College*
- a CENTRAL MEDIAN ISLAND in Pittwater Road which precludes right-turn movements into and out of the site
- TRAFFIC SIGNALS in Pittwater Road where it intersects with Warringah Road/Harbord Road
- RIGHT TURN HOLDING BAYS for all directions along the Pittwater Road and Warringah Road/Harbord Road intersection
- a STOP SIGN restriction in Headland Road where it intersects with Harbord Road.

6.3 Existing Public Transport Services

There are currently 13 bus routes which operate along Pittwater Road as illustrated on Figure 5.

The nearest bi-directional bus stops for these bus routes on Pittwater Road are located to the south of the Harbord Road intersection, approximately 100m walking distance from the proposed senior school campus.

The 132 bus service (Warringah Mall to Manly via North Balgowlah) is also located within easy walking distance, with the closest bus stop located along Harbord Road.

The typical service frequencies for services along Pittwater Road during morning/afternoon school times are summarised in Table 3.1 below.

In addition to the abovementioned regular bus route services, the new B-Line service also operates along Pittwater Road. The nearest B-Line bus stops are located near Howard Avenue approximately 700m to the north of the *pedestrian-only* gate located at the northwestern corner of the *senior campus* site.



Route	Bus Route Description	Туріса	I Service Freq	uency
Number		Weekday	Saturday	Sunday
146	Wheeler Heights to Manly	30min	30min	30min
151	Mona Vale to City QVB	30min	30min	30min
158	Cromer to Manly	1 service ¹	N/A	N/A
169	Manly to City Wynyard via Narraweena	30min	30min	60min
178	City Wynyard to Cromer Heights	30min	30min	30min
180	Collaroy Plateau to City Wynyard	30min	30min	30min
185	Mona Vale to Warringah Mall via Warriewood	30min	30min	30min
188	Mona Vale to City Wynyard	20min	30min	30-60min
193	Warringah Mall to Austlink via Frenchs Forest	30min	30min	30-60min
199	Palm Beach to Manly	15min	30min	30min
E69	Manly to City Wynyard via Narraweena (EXPRESS)	5-10min	N/A	N/A
E78	Cromer Heights to City Wynyard (EXPRESS)	15-20min	N/A	N/A
E79	Wheeler Heights to City Wynyard (EXPRESS)	15-20min	N/A	N/A

Table 3.1: Existing Bus Services along Pittwater Road

¹Morning Service at approx. 7am and Afternoon Service at approx. 4pm

The new B-Line service will primarily benefit *senior campus* students who can independently access the B-Line bus stops in Pittwater Road near Howard Street using the *pedestrian-only* gate located on the north-western corner of the site.

It should be noted that this *pedestrian-only* gate is separated from road traffic on Pittwater Road by a 2m high concrete embankment topped by a pedestrian fence which extends along the site frontage and northerly to the car park entrance to the Stony Range Regional Botanic Gardens, approximately 100m to the north of the site. The B-Line route is illustrated on Figure 5.1 below.

Figure 5.1 – B-Line Route



In addition to the abovementioned regular bus routes & B-Line services, there are also dedicated *school bus* services which stop directly outside the existing *St Luke's Grammar School* in Headland Road or in Quirk Street.

The school buses serving the existing St Luke's Grammar School Campus are provided by the NSW State Government and are listed on the Transport for NSW website as detailed below, with service timetables detailed in Appendix B.

- 668N St Luke's Dee Why to Balgowlah Heights
- 669N St Luke's Dee Why to Manly Wharf
- 670N St Luke's Dee Why to Bantry Bay
- 671N St Luke's Dee Why to Skyline Shops
- 672N St Luke's Dee Why to Collaroy Plateau
- 677N St Luke's Dee Why to Warriewood Square
- 678N St Luke's Dee Why to Newport
- 684N Collaroy Plateau to Balgowlah Boys' High
- 702N Frenchs Forest to St Pauls
- 732N Allambie & Rodborough Roads to Manly High School
- 759N North Curl Curl to North Curl Curl Public School
- 784N St Luke's to Frenchs Forest
- 787N St Luke's Grammar to Avalon

The location of the bus stops in the vicinity of the site are illustrated on Figure 5.2 below and are also shown on the Transport Access Guide which forms part of the *Green Travel Plan*.

Figure 5.2 Access and Movement Analysis of existing/proposed SLGS site, extracted from Tonkin Zulaikha Greer's Architectural Design Report (October 2019)



6.4 Existing Traffic Conditions

In order to gain an accurate appreciation of the existing traffic conditions on the road network in the vicinity of the site, peak period traffic surveys were undertaken at the Pittwater Road/Harbord Road and also the Harbord Road/Headland Road intersections on 19th September 2019. The surveys revealed that:

- the road network peak in the morning occurred between 7:30am and 8:30am which coincides with the morning *school peak*
- The road network peak in the afternoon occurred between 4:00pm and 5:00pm, after the afternoon *school peak* which occurs at 3:00pm and 4:00pm
- one-way southbound traffic flows in Pittwater Road along the site frontage are typically in the order of 2,400 vehicles per hour (vph) during the morning *school peak* period, and approximately 1,800 vph during the afternoon *school peak* period
- one-way northbound traffic flows in Pittwater Road in the vicinity of the Warringah Road/Harbord Road intersection are typically in the order of 1,350 vehicles per hour (vph) during the morning *school peak* period, and approximately 2,000 vph during the afternoon *school peak* period
- one-way southbound traffic flow in Harbord Road along the site frontage are typically in the order of 1,100 vehicles per hour (vph) during the morning *school peak* period, decreasing to approximately 900 vph during the afternoon *school peak* period
- southbound traffic flows in Harbord Road in the vicinity of the Headland Road intersection are typically in the order of 1,000 vph during the *school peak* periods
- two-way traffic flows in Headland Road in the vicinity of the Harbord Road intersection are typically in the order of 550 vehicles per hour (vph) during the morning *school peak* period, and approximately 450 vph during the afternoon *school peak* period.

The results of the traffic surveys are reproduced in full in Appendix C, and the traffic flows during school peak periods are summarised on Figures 6a and 6b, revealing that:

Traffic surveys were also undertaken at the site access driveways of No.800 Pittwater Road and also No.224 Headland Road to identify the traffic flows generated by the existing uses of those sites.

The results of the traffic surveys at the site access driveways are reproduced in full in Appendix C and are summarised on Figures 7a and 7b, revealing that:

No. 800 Pittwater Road

- two-way traffic flows using the existing site access driveway during the *morning* school peak period (7:30-8:30am) were 170 vph (i.e. 92 vph IN and 72 vph OUT)
- during the *afternoon* school peak period (3:00pm-4:00pm) the two-way traffic flows using the existing driveway was 244 vph (131 vph IN and 113 vph OUT)
- the total volume of traffic generated by the existing uses of the site throughout the 12hour duration of the surveys (6am-6pm) was 2,931 vehicle movements including 17 heavy vehicle truck movements.

No. 224 Headland Road

- there were 23 vehicle movements in and out of the existing site during the school peak period in the *morning* (i.e. 14 vph IN and 9 vph OUT)
- there were 16 vehicle movements in and out of the existing site (i.e. 6 trips IN and 10 trips OUT) during the school peak period in the *afternoon*
- there were a total of 201 vehicle movements (two-way) generated by the existing uses of the site throughout the 12-hour duration of the surveys between 6am-6pm.



Figure 6a – Existing Traffic Flow Traffic Assignment during morning school peak (7:30am-8:30am)



Figure 6b – Existing Traffic Flow Traffic Assignment during afternoon school peak (3:00pm-4:00pm)



Figure 7a – Existing Traffic Flow Traffic Assignment along the site access driveways of 800 Pittwater Road and 224 Headland Road during *morning* school peak (7:30am-8:30am)



Figure 7b – Existing Traffic Flow Traffic Assignment along the site access driveways of 800 Pittwater Road and 224 Headland Road during *afternoon* school peak (3:00pm-4:00pm)

6.5 Projected Future Traffic Generation Potential

The traffic implications of a development proposal primarily concern the effects of the additional traffic flows generated as a result of a development and its impact on the operational performance of the adjacent road network.

An indication of the traffic generation potential at most types of developments is usually provided by reference to Roads and Maritime Services Publication *Guide to Traffic Generating Developments (2002)*. However, the RMS *guidelines* do not nominate the traffic generation rate for schools. A number of surveys were therefore undertaken at SLGS to identify the level of traffic activity generated by the existing student numbers, as detailed in Chapter 4 of this report.

Based on a "per student" basis the survey results identified the following traffic generation rates:

Existing School Traffic Generation Rates							
	(vehicles per hour/student)						
AM Peak Hour PM				PM Peak Hour	•		
	IN	OUT	TOTAL	IN	OUT	TOTAL	
K-Y9	0.273	0.273	0.545	0.160	0.160	0.319	
Y10-Y12	0.172	0.172	0.343	0.086	0.086	0.172	

The survey data confirms that senior students in Years 10 to 12 generate substantially less *traffic* on a per student basis than younger students, and reinforces the conclusions reached in an independent traffic study² prior to its approval of MOD 2018/0412 on 26 June 2019. It also confirms on-site observations. There are typically no queues at the high school finishing time of 3:20pm (Years 7 to 12).

Proposed Senior Campus - No. 800 Pittwater Road

Application of the above traffic generation rates to the 600 students proposed on the senior school campus at 800 Pittwater Road yields a total of 103 drop-offs during the morning *school peak* hour, and a total of 52 pick-ups during the afternoon *school peak* hour. In

² The Transport Planning Partnership St Luke's Grammar School – Planning Panel Deferral Traffic and Parking Survey Results 7 May 2019 undertaken for the Sydney North Planning Panel.

addition, there will be 15 students and 75% of staff parking on-site who will also arrive and depart during the same *school peak* hour.

A comparison of existing and projected traffic flows during the 1-hour *school peak* period at the existing site access driveway in Harbor Road is set out in the table below, revealing that:

- traffic flows during the morning *school peak* hour will increase by 96 vph when compared with the existing uses of the site, and
- traffic flows during the afternoon *school peak* hour will decrease by 80 vph when compared with the existing uses of the site.

No. 800 Pittwater Road Comparison of Existing and Projected Future Traffic Flows During the 1-Hour <i>School Peak</i> Period (vehicles per hour)						
	Existing Traffic Flows Projected Future Traffic Flows					
	AM	PM	AM	PM		
IN	92 vph	131 vph	163 vph	52 vph		
OUT	78 vph	113 vph	103 vph	112 vph		
TOTAL	170 vph	244 vph	266 vph	164 vph		

As noted in the foregoing, the school proposes to make available for *community uses* the swimming pool on the *senior campus* site, and the basketball courts on the *sports centre* site on an after-school hours basis. Parking for both community uses will be allocated on the *service campus* site.

It is anticipated that 3 swim school classes and squad training sessions may be held in the pool. Assuming a vehicle occupancy of 1.2 swim/squad students per vehicle, the swim school/squad training could be expected to generate approximately 85 vehicles per hour (in and out combined) between 4pm-7pm. It is noted that the *existing* uses of the site at those times typically generate more than 300 vph (in and out combined) and the proposed community uses therefore represent a substantially *less intensive* use of the site in traffic terms than those existing uses.

The *community use* of the basketball courts on an after-school hours basis is expected to be used by 10 persons per court. With a vehicle occupancy of 1.2 persons per vehicle the basketball courts are expected to generate approximately 33 vph between 4pm-7pm.

The *daily* traffic generation potential of the proposed senior school campus upon completion of Stage 3 is expected to be 508 vehicles per day (including student and staff parking) plus a further 232 vehicles per day associated with the *community uses* of the swimming pool and basketball courts, yielding a cumulative *daily* traffic generation potential of 740 vehicles per day.

This compares with the existing *daily* traffic flows of 2,931 vehicles per day generated by the existing uses of the site as identified by surveys conducted between $6am-6pm^3$. The proposed senior school campus development is therefore expected to result in a very substantial reduction in the *daily* traffic generation potential of the site of at least 2,191 vehicles per day (vpd) as set out in the table below:

No. 800 Pittwater Road				
Comparison of Existing and Projected Future Daily Traffic Flows (6am-6pm)				
Existing Traffic Generation: 2,931 vpd				
Projected Traffic Generation:	740 vpd			
Nett Change:	-2,191 vpd			

Proposed Sports Centre - No. 224 Headland Road

The *sports centre* proposed at 224 Headland Road includes 39 car parking spaces for Year 12 students who will arrive in the morning and depart in the afternoon. There will not be any other traffic generated by the *sports centre* usage, with all students to walk to the site from the adjacent school campus.

A comparison of existing and projected future traffic flows during the 1-hour *school peak* period is set out in the table below, revealing that:

• traffic flows during the morning school peak hour will increase slightly by 18 vph, and

³ It is acknowledged that the existing uses of the site generates additional traffic *before* 6am and *after* 6pm associated with the Fitness First gymnasium and swimming pool.

No. 224 Headland Road Comparison of Existing and Projected Future Traffic Flows During the 1-Hour <i>School Peak</i> Period (vehicles per hour)						
	Existing Traffic Flows Projected Future Traffic Flows					
	AM	PM	AM	PM		
IN	14 vph	6 vph	39 vph	-		
OUT	9 vph	10 vph	-	39 vph		
TOTAL	AL 23 vph 16 vph 39 vph 39 vph					

traffic flows during the afternoon *school peak* hour will increase slightly by 25 vph.

The *daily* traffic generation potential of the proposed *sports centre* upon completion of Stage 1 is expected to be 78 vehicles per day. This compares with the *daily* traffic flows of 201 vehicles per day generated by the existing uses of the site as identified by surveys conducted between 6am-6pm. The proposed *sports centre* component of the development is therefore expected to result in a substantial reduction in the *daily* traffic generation potential of the site of approximately 123 vehicles per day (vpd) as set out in the table below:

No. 224 Headland Road				
Comparison of Existing and Projected Future Daily Traffic Flows (6am-6pm)				
Existing Traffic Generation:	201 vpd			
Projected Traffic Generation:	78 vpd			
Nett Change: -123 vpd				

Existing School Campus - No. 210 Headland Road

Application of the above traffic generation rates to the 1,000 students proposed on the existing school campus at 210 Headland Road indicates that there will be nett increase of 25 drop-offs during the morning *school peak* hour, and a nett increase of 22 pick-ups during the afternoon *school peak* hour, as set out in the table below.

Existing School Campus - No. 210 Headland Road Comparison of Existing and Projected Future Traffic Flows During the 1-Hour <i>School Peak</i> Period (vehicles per hour)						
	Existing Traffic Flows Projected Future Traffic Flows					
	AM	PM	AM	PM		
IN	297 vph	115 vph	322 vph	137 vph		
OUT	196 vph	216 vph	221 vph	238 vph		
TOTAL	TOTAL 493 vph 331 vph 543 vph 375 vph					

The *daily* traffic generation potential of the existing school campus upon completion of Stage 3 is expected to be 1,215 vehicles per day taking account the change in student mix proposed at the existing school campus. This compares with the *daily* traffic flows of 1,126 vehicles per day generated by the existing uses of the existing school campus. The change in student mix proposed at the existing school campus is therefore expected to result in an increase in the *daily* traffic generation potential of the site of 89 vpd as set out in the table below.

No. 210 Headland Road				
Comparison of Existing and Projected Future Daily Traffic Flows (6am-6pm)				
Existing Traffic Generation:	1,126 vpd			
Projected Traffic Generation:	1,215 vpd			
Nett Change:	+89 vpd			

6.6 Traffic Implications - Road Network Capacity

The traffic implications of development proposals primarily concern the effects that any changes in traffic flows may have on the operational performance of the nearby road network during school peak periods.

Those effects can be assessed using the SIDRA program which is widely used by the RMS and many LGA's for this purpose. Criteria for evaluating the results of SIDRA analysis are reproduced in the following pages.

The result of the SIDRA analysis of the two main intersections located around the perimeter of *St Luke's Grammar School* are summarised in the Tables 3.1, 3.2 & 3.3 below, revealing that:

Pittwater Road/Harbord Road Intersection

- the Pittwater Road/Harbord Road intersection currently operates at *Level of Service* "D" under the existing traffic demands with total average vehicle delays in the order of 49 seconds/vehicle
- under the projected future traffic demands expected to be generated by the development proposal, the Pittwater Road/Harbord Road intersection will continue to

operate at *Level of Service "D"* during the *morning* school peak and *afternoon* school peak period, with total average vehicle delays in the order of 50 second/vehicle.

The SIDRA capacity analysis of the Pittwater Road/Harbord Road intersection was then repeated, assuming a 40 km/h School Zone speed limit applied to all four approaches to the intersection. The results of that analysis are summarised in Table 3.2 below, revealing that the intersection will continue to operate at Level of Service "D" with total average vehicle delays in the order of 52 seconds/vehicle.

In essence, the analysis shows that, *if* a 40 km/h School Zone speed limit was applied, it would have little practical effect on the operating performance of the intersection.

It is noted in this regard that repeated observations of the intersection operation throughout the *school peak* periods that vehicle speeds traversing the intersection are in the order of approximately 40 km/h in any event, and thus the application of a 40 km/h School Zone speed limit is unlikely to have any practical or appreciable effects on intersection performance.

Harbord Road/Headland Road Intersection

- the Harbord Road/Headland Road intersection currently operates at *Level of Service* "A" during the morning and afternoon school peak period, under the existing traffic demands with total average vehicle delays in the order of 10.6 and 5.9 seconds/vehicle respectively.
- under the projected future traffic demands expected to be generated by the development proposal, the Harbord Road/Headland Road intersection will continue to operate at *Level of Service "A"*, with increases in average vehicle delays of between 0 and 2 second/vehicle.

The detailed SIDRA movement summaries are reproduced in Appendix D. The results indicate that the surrounding intersections are expected to continue to operate at current *Levels of Service*, with minimal change in total average vehicle delays.

In summary therefore, the capacity analysis confirms that the proposed expansion of St Luke's Grammar School will not have any unacceptable traffic implications in terms of road network capacity. Accordingly, there will not be any road upgrades or intersection improvements required as a consequence of the development proposal.

6.7 The Need for 40/h School Zone Speed Limits

The RMS requirements for 40 km/h School Zone speed limits on multi-lane roads and highspeed roads are specified in *Technical Direction No. TD2003/RS02*. The *Technical Direction* specifies the following:

- *multi-lane roads* are defined as roads with 2 or more lanes for traffic movement in the same direction of travel
- access points are defined as any point by which students can currently enter or exit a school site on foot, bicycle, wheel recreational device or wheeled toy. They include gates, driveways and delivery access points
- where access points exist on multi-lane roads and high-speed roads, every effort should be undertaken to negotiate with the school to relocate and/or close-off an access point, rather than install a 40 km/h school zone. If this is not possible, it may be necessary to consider other treatments in addition to the school zone, so that a safer environment is created for the students of the school in question
 - school zone installations on multi-lane roads of 70 km/h or less should be a minimum (length) of 200m

A 40 km/h school speed limit already exists around the perimeter of the existing SLGS site in Headland Road, Quirk Street and in Tango Avenue. It is anticipated that the 40 km/h school zone speed limit in Headland Road would be extended towards the west to incorporate the entrance to the vehicular and pedestrian entrances to the *sports centre* proposed at 224 Headland Road.

Another school zone speed limit exists in Harbord Road from 100m south of Pittwater Road to Brighton Street for Freshwater Secondary College.

The *senior campus* which is the subject of this application will have pedestrian entrances located at the north-western and south-western corners of the site, in addition to the vehicle entrance (left-in/left-out) also located at the south-western corner of the site.

The proposed *senior campus* does not have any other street frontage other than Pittwater Road and a short section of Harbord Road where the existing vehicular access driveway is located.

The two pedestrian entrances are a key component of the *senior campus* proposals to enable it to be integrated with the existing public transport services on Pittwater Road, because each pedestrian entrance provides the shortest and most direct walking route to:

- the B-Line bus stops to the north, and
- the regular bus route service stops to the south of the site in Pittwater Road.

It is noted in this regard that the *pedestrian-only* gate located at the north-western corner of the site is separated from road traffic on Pittwater Road by a 2m high concrete retaining wall topped by a pedestrian safety fence along the kerb line which extends along the majority of the site frontage and further to the north across the frontage of the adjacent Stony Range Regional Botanic Garden.

The location of the existing *pedestrian-only* gate at the north-western corner of the site is illustrated on Figure 7c below. It is pertinent to note that another retaining wall is located on the opposite (i.e. western) side of Pittwater Road, such that all pedestrian access to this section of Pittwater Road is impossible.



Figure 7c – Existing Pedestrian-Only Gate at North-Western Corner of the Senior Campus Site

The pedestrian entrance proposed near the south-western corner of the *senior campus* site is to be located directly opposite the signalised pedestrian crossings which provide direct pedestrian access to the bi-directional bus stops located in Pittwater Road immediately to the south of the intersection.

The location of the pedestrian access proposed at the south-western corner of the *senior campus* site is illustrated on Figure 7d below. The proposed pedestrian access will be located directly opposite the signalised pedestrian crossings and will provide the shortest possible walking route to the bi-directional bus stops located immediately to the south of the intersection.



Figure 7c - Existing Pedestrian-Only Gate at North-Western Corner of the Senior Campus Site

The provision of direct pedestrian access along the shortest possible routes to both the regular bus route stops and the B-Line bus stops form a key component of the school's proposal to integrate the *senior campus* with the existing public transport network, and is ideally suited to the needs of senior students who will be able to access these external bus stops in a safe and efficient manner.

The proposed pedestrian entrances will also extend the "range" of the *senior campus* for active transport users (i.e. walking and cycling) for students and staff travelling to/from the *senior campus* on foot or bicycle.

Strict application of *Technical Direction TDT2003/RS02* suggests that a 40 km/h School Zone speed limit would be required on all four approaches to the Pittwater Road/Harbor Road intersection.

However the need to implement a 40 km/h School Zone speed limit could be mitigated by the following factors:

- the *pedestrian-only* gate located at the north-western corner of the site frontage provides pedestrian access to the public footpath *only*. It is *not possible* for pedestrians on the footpath to access the road pavement due to the 2m high concrete retaining wall which is topped by a pedestrian fence
- the pedestrian entrance proposed on the south-western corner of the site in Harbord Road will be located directly opposite the signalised pedestrian crossings at the Pittwater Road/Harbord Road intersection which provide access to the bi-directional bus stops for regular bus services on Pittwater Road.

Key Indicators Level of Service		Existing Traffic Demand		Projected Developme Traffic Demand	
		AM	PM	AM	РМ
		D	D	D	D
Degree of Saturation		0.804	0.808	0.872	0.808
Average Vehicle Delay (secs/veh)				
Harbord Road (S)	L T R	85.6 78.0 72.1	77.7 73.1 76.8	84.4 75.1 75.0	78.1 73.2 75.4
Pittwater Road (NE)	L T R	19.2 35.5 67.6	17.0 38.1 77.3	23.6 47.4 78.4	17.7 40.0 77.3
Warringah Road (W)	L T R	37.7 73.8 80.8	44.6 69.6 86.8	41.4 78.4 89.5	44.6 71.0 86.8
Pittwater Road (SW)	L T R	7.2 36.8 87.9	7.2 35.6 48.8	7.2 31.3 86.9	7.2 36.8 49.1
TOTAL AVERAGE VEHICLE	DELAY	48.5	49.0	53.4	49.9

PITTWATER ROAD & WARRINGAH ROAD/HARBORD ROAD WITH 40 KM/H SPEED LIMIT					
Key Indicators		Existing Traffic Demand		Projected Development Traffic Demand	
		AM	РМ	AM	РМ
Level of Service		D	D	D	D
Degree of Saturation		0.804	0.808	0.872	0.808
Average Vehicle Delay (secs/vel	h)				
Harbord Road (S)	L T R	82.7 75.5 69.7	74.8 70.6 74.4	81.5 72.6 72.6	75.2 70.8 73.0
Pittwater Road (NE)	L T R	17.0 35.5 64.7	14.8 38.1 74.4	21.4 47.4 75.4	15.5 40.0 74.4
Warringah Road (W)	L T R	35.2 71.4 78.2	42.2 67.2 84.1	38.9 75.9 86.8	42.2 68.5 84.1
Pittwater Road (SW)	L T R	3.8 36.8 85.4	3.8 35.6 46.3	3.8 31.3 84.4	3.8 36.8 46.6
TOTAL AVERAGE VEHICLE	E DELAY	47.3	47.7	52.1	48.5
		PIT_	HARX	PIT_	HARP

TABLE 3.2 - RESULTS OF SIDRA ANALYSIS OF

HARBORD ROAD & HEADLAND ROAD						
Key Indicators		Exi Traffic	sting Demand	Projected Development		
		AM	РМ	AM	РМ	
Level of Service		А	A	A	А	
Degree of Saturation		0.905	0.742	0.930	0.851	
Average Vehicle Delay (secs/veh)						
Harbord Road (S)	T R	1.1 11.9	1.2 9.7	1.1 12.2	1.4 10.8	
Headland Road (E)	L R	61.9 137.6	40.1 103.0	71.0 149.2	65.6 138.9	
Harbord Road (N)	L T	3.5 0.0	3.5 0.0	3.5 0.0	3.5 0.0	
TOTAL AVERAGE VEHICLE	DELAY	10.6	5.9	11.7	7.9	
		HAR	HEAX	HAR	HEAP	

TABLE 3.3 - RESULTS OF SIDRA ANALYSIS OF

Criteria for Interpreting Results of Sidra Analysis

1. Level of Service (LOS)

LOS	Traffic Signals and Roundabouts	Give Way and Stop Signs
'A'	Good operation.	Good operation.
'B'	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
'C'	Satisfactory.	Satisfactory but accident study required.
'D'	Operating near capacity.	Near capacity and accident study required.
'E'	At capacity; at signals incidents will cause excessive	At capacity and requires other control mode.
	delays. Roundabouts require other control mode.	
'F'	Unsatisfactory and requires additional capacity.	Unsatisfactory and requires other control mode.

2. Average Vehicle Delay (AVD)

The AVD provides a measure of the operational performance of an intersection as indicated on the table below which relates AVD to LOS. The AVD's listed in the table should be taken as a guide only as longer delays could be tolerated in some locations (ie inner city conditions) and on some roads (ie minor side street intersecting with a major arterial route).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way and Stop Signs
А	less than 14	Good operation.	Good operation.
В	15 to 28	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
С	29 to 42	Satisfactory.	Satisfactory but accident study required.
D	43 to 56	Operating near capacity.	Near capacity and accident study required.
E	57 to 70	At capacity; at signals incidents will cause excessive delays. Roundabouts require other control mode.	At capacity and requires other control mode.

3. Degree of Saturation (DS)

The DS is another measure of the operational performance of individual intersections.

For intersections controlled by traffic signals⁴ both queue length and delay increase rapidly as DS approaches 1, and it is usual to attempt to keep DS to less than 0.9. Values of DS in the order of 0.7 generally represent satisfactory intersection operation. When DS exceeds 0.9 queues can be anticipated.

For intersections controlled by a roundabout or GIVE WAY or STOP signs, satisfactory intersection operation is indicated by a DS of 0.8 or less.

4

The values of DS for intersections under traffic signal control are only valid for cycle length of 120 secs.

7.0 PARKING IMPLICATIONS

7.1 Existing Kerbside Parking Restrictions

The existing kerbside parking restrictions which apply to the road network in the vicinity of the site are illustrated on Figure 8 and comprise:

- CLEARWAY and full-time NO STOPPING/NO PARKING restrictions along the entire Pittwater Road frontage of the proposed senior school campus site
- BUS LANE and NO STOPPING/NO PARKING restrictions along the western side of Pittwater Road, opposite the senior school campus site
- NO PARKING restrictions on the eastern side of Harbord Road in the vicinity of the senior school campus site
- a short section of UNRESTRICTED kerbside parking further to the south on the eastern side of Harbord Road
- NO STOPPING/NO PARKING restrictions along the western side of Harbord Road
- NO STOPPING restrictions along the northern side of Headland Road in the vicinity of the proposed *sports centre* site
- SCHOOL BUS ZONES on the northern side of Headland Road and the western side of Quirk Street adjacent to the existing school campus site
- NO PARKING restrictions in the drop-off bay located on the southern side of Tango Avenue adjacent to the existing school campus site.

7.2 Off-Street Parking Provisions

The off-street parking requirements applicable to the development proposal are specified in Warringah Council's *DCP 2011 – Appendix 1 – Car Parking Requirements* document in the following terms:


Educational Establishment

1 car space per staff member in attendance *plus* as relevant, adequate pick-up/set-down area on-site *plus* adequate provision of bicycle racks *plus* adequate provision for student parking *plus* provision of bus standing and turning area

Application of the above parking requirements to the 60 staff proposed at the new senior school campus site yields a staff car parking requirement of 60 parking spaces.

That staff car parking requirement is satisfied by the proposed provision of 60 car parking spaces in a reconfiguration of the existing basement car parking area.

In addition, a further 25 car parking spaces are proposed on the senior school campus site for the use of Year 12 students only.

A further 40 car parking spaces for Office Works will be provided on the senior school campus site in Stage 2, in accordance with DA Consent Number 97/59.

Additional student car parking comprising 39 spaces is proposed for Year 12 students on the *sports centre* site. The total Year 12 student parking to be provided at both sites is therefore 64 spaces.

In addition to the above car parking facilities, a new drop-off/pick-up bay is also proposed within the senior school campus site. The existing 90^{0} angle parking spaces in the outdoor car parking area along the site frontage are to be removed to create the dedicated drop-off/pick-up area, initially for 7 spaces in Stage 2, increasing to 12 spaces in Stage 3. Queueing for a further 5 cars can be accommodated on the driveway approaching the drop-off/pick-up bay in both stages should the need ever arise:

- without impeding other traffic entering the site, and
- without extending beyond the property boundary.

The proposed new drop-off/pick-up bay on the *senior campus* site will have the same capacity as the existing drop-off/pick-up bay in Tango Avenue, but will cater for

approximately half the number of students. In addition, survey data confirms that senior students in Years 10 to 12 generate substantially *less traffic* on a *per student* basis than younger students.

In particular, the surveys and observations undertaken in the existing drop-off/pick-up bay in Tango Avenue have revealed that:

- peak traffic activity during the morning drop-off period typically occurs between 8:10am-8:25am. Although this is a busy period, drop-offs are quick and efficient, and delays are minimal if any
- peak traffic activity in the afternoon typically occurs between 3:00pm-3:15pm, and is associated with the *junior school* finishing times of 2:55pm (for Kindergarten to Year 2) and 3:05pm (Years 3 to 6). There are typically no queues in Tango Avenue outside this 15 minute peak period in the afternoons
- the *high school* Years 7 to 12 (currently 658 students) finishes at 3:20pm but does not generate any delays or queues, with typically only 3 to 5 cars recorded in the Tango Avenue drop-off/pick-up bay at 3:25pm, then only 1 car after that time. On a pro-rata basis during the afternoon pick-up this equates to:
 - 2 to 4 cars waiting in Stage 2 (480 students), when a drop-off/pick-up capacity of 7 cars will be provided (plus a further 5 cars on the driveway), and
 - 3 to 5 cars waiting in Stage 3 (600 students) when a drop-off/pick-up capacity of 12 cars will be provided (plus a further 5 cars on the driveway).

Thus the drop-off/pick-up bays proposed on the *senior campus* site in Stages 2 and 3 will substantially exceed projected future demands at all times.

7.3 Loading/Servicing Provisions

Loading/servicing of the new senior school site is expected to be undertaken by a variety of light commercial vehicles and small to medium sized rigid trucks such as garbage trucks.

The servicing needs of the proposed senior school campus are expected to be minimal, and will primarily comprise light commercial vehicles such as stationary deliveries.

All deliveries will be undertaken outside drop-off and pick-up times, as currently occurs at the existing school campus at 210 Headland Road.

The largest truck expected to access the site will be a medium sized rigid truck for garbage collection, and will access the site on an after-hours basis only when the school is closed, consistent with the existing garbage collection arrangements at the existing school campus.

Deliveries to the senior school campus will be undertaken using the drop-off/pick-up bay in Stage 2 (at times when the bay is not in use), and will revert to the existing loading area when it is vacated by Office Works in Stage 3.

It is pertinent to note that all vehicular access and circulation arrangements proposed on the senior school campus site have been designed to accommodate the swept turning path/manoeuvring requirements of large 12.5m long HRV rigid trucks, consistent with the servicing arrangements for the existing uses of the site.

The geometric design layout of the proposed drop-off/pick-up and loading facilities will also ultimately be designed to comply with the relevant requirements specified in the Standards Australia publication *Parking Facilities Part 2 - Off-Street Commercial Vehicle Facilities AS2890.2* in respect of loading dock dimensions and service area requirements for HRV trucks.

8.0 CONCLUSION

St Luke's Grammar School at Dee Why proposes to expand its existing educational facilities to provide a new *sports centre* at No. 224 Headland Road (Stage 1) and a new senior school campus at No. 800 Pittwater Road which will cater for senior Year 10-12 students only, initially for 480 students (Stage 2) and increasing to 600 students in Stage 3.

The foregoing analysis has found that:

No. 800 Pittwater Road

- the existing uses of the site by Office Works, I-MED Imaging and Fitness First Gymnasium/Swimming Pool generates a *daily* traffic flow of 2,931 vehicles per day (between 6am-6pm)
- the proposed senior school campus development will result in a substantial reduction in the *daily* traffic generation potential of the site to approximately 740 vehicles per day
- traffic flows during the morning *school peak* hour will increase by 96 vph (two-way) when compared with the existing uses of the site, and
- traffic flows during the afternoon *school peak* hour will decrease by 80 vph (two-way) when compared with the existing uses of the site.

No. 224 Headland Road

- the existing uses of the site by the industrial/warehouse tenants as well as existing school sports facilities generate *daily* traffic flows of 201 vehicles per day (between 6am-6pm). The proposed school's *sports centre* will reduce the *daily* traffic generation potential of the site to approximately 78 vehicles per day
- traffic flows during the morning *school peak* hour will increase slightly by 18 vph (two-way), and

• traffic flows during the afternoon *school peak* hour will increase slightly by 25 vph (two-way).

Traffic Assessment

- before/after SIDRA capacity analysis of the Pittwater Road/Harbord Road/Warringah Road and Harbord Road/Headland Road intersections has found that both intersections will continue to operate at current *Levels of Service*, with minimal increases in total average delays
- further analysis has found that the Pittwater Road/Harbord Road/Warringah Road intersection would *also* continue to operate at current *Levels of Service* and with minimal delays, *if* a 40 km/h School Zone speed limit was implemented on all four approaches to the intersection
- SIDRA capacity analysis of the proposed site access driveways has confirmed that both driveways will operate at *Level of Service "A"* and with minimal delays during the *school peak* hours.

Proposed Car Parking Facilities

- Consistent with RMS requests, car parking on the *senior campus* site will be reduced from 182 spaces at present, to 131 spaces whilst Office Works remains on the site, then down to 91 spaces upon completion of the *senior campus*.
- Similarly, car parking on the *sports centre* site will also be reduced, from 45 spaces to 39 spaces.
- Car parking on the *senior campus* site and the *sports centre* site is proposed in accordance with the minimum parking requirements specified in Warringah Council's *DPCP 2011* as follows:
 - 60 spaces will be provided for staff in accordance with DCP 2011 requirements
 - 5 spaces for visitors

- 25 spaces for Year 12 students on the *senior campus* site plus a further 39 spaces for Year 12 students on the *sports centre* site
- The proposed car parking represents a balance between the RMS request to reduce car parking on the *senior campus* site and the need to achieve compliance with Council's DCP parking requirements which seek to minimise the impacts of overflow car parking in the surrounding residential streets.
- It is noted in this regard that the amount of car parking on the 2 sites will be reduced from 227 spaces to 130 spaces whilst achieving *compliance* with Council's DCP parking requirements. Most of that reduction of 97 spaces was removed from the *senior campus* site.

Proposed On-Site Drop-off/Pick-Up Facility

- Analysis of the survey data indicates that a drop-off/pick-up capacity of 2 to 4 cars will be required in Stage 2, increasing to 3 to 5 cars in Stage 3.
- The drop-off/pick-up facility proposed on the *senior campus* will have a capacity of 7 spaces in Stage 2, increasing to 12 spaces in Stage 3.
- Queueing for *a further 5 cars* is provided on the driveway approaching the dropoff/pick-up bay in both stages *without* impeding other traffic flows entering the site, and *without* extending beyond the property boundary.
- Thus the drop-off/pick-up bay proposed on the *senior campus* site in Stages 2 and 3 will substantially exceed projected future demands at all times.
- It is pertinent to note in this regard that the proposed new drop-off/pick-up facility will have the *same capacity* as the existing drop-off/pick-up bay in Tango Avenue, but will cater for *half the number of students*, noting also that the senior students in Years 10 to 12 generate substantially *less traffic* on a *per student* basis than younger students.

APPENDIX A

SCHOOL TRAFFIC MANAGEMENT PLAN



This plan reflects changes to be undertaken in Headland Road and Tango Avenue in accordance with DA Modification Application 2018/0412 approved by Sydney North Planning Panel on 26 June 2019. The school intends to have the additional works in Council's road reserve finalised before commencement of Term 1 2020

ST LUKES GRAMMAR SCHOOL TRAFFIC MANAGEMENT PLAN

APPENDIX B

SCHOOL BUS TIMETABLE

scнооl вus Timetable



Effective 12 February 2019



- 679 Nowport (2.36 pm)
- 678 Newport (3.26 pm)
- 672 Collaroy Plateau (3.32 pm)
- 677 Narrabeen (3.40 pm)
- 784 Frenchs Forest (3.33 pm)
- 787 Avalon / Clareville (3.25 pm)







Morning Buses

631	8.05 am	Manly Wharf (Gilbert Park)	FROMMANLY
	8:11 am	Stella Maris Manly	
	8:20 am	NBSC Manly Campusl	Dep. Stand F Gilbert Park Via Belgrave, Pittwater, Collingwood Route
	8:24 am	St Luke's Grammar School	150, Harbord, Abbort, Fitt, Fibylan, Fatt, Headiand, Qank
668	7:42 am	Balgowlah Heights [Ernest Street]	FROM BALGOWLAH HEIGHTS
	7:51 am	Panorama & Ponsonby Pdes	
	7:59 am	Seaforth - Dudley St	
	8:02 am	Balgowlah Shops	
	8:07 am	Manly Vale Shops	
	8:11 am	Warringah Mall - Pittwater Rd	
	8:16 am	NBSC Manly Campus	
_	8:19 am	St Luke's Grammar School	
675	7:37 am	Balgowlah Heights [Ernest Street]	FROM BALGOWLAH HEIGHTS
	7:49 am	Balgowlah Shops	
	7:54 am	Manly Vale Shops	Via Woodland, Abbott, Lewis, Ernest, Beatrice, Seaview, Upper Beach, Maretimo, Violet, Wangapella, Sydney, Condamine, Pittwater, Mitchell
	7:58 am	Warringah Mall [Pittwater Rd]	Winbourne, Harbord, Headland
	8:04 am	NBSC Manly Campus	
	8:08 am	St Luke's Grammar School	
702	7:45 am	Rabbett St & Forest Way	FROM FRENCHS FOREST
107	7:52 am	Skyline Shops	
	7:56 am	Warringah & Willandra Roads	Via Rabbett, Warringah, Wakehurst Parkway, F. Forest East (Skyline
	8:01 am	Moorilla St at Victor Rd, Dee Why	Shops), warnigan, Harbord, Headiand Quirk, Tango
	8:06 am	St Luke's Grammar School	
673	7:33 am	Pittwater High	FROM MONA VALE
Express	7:38 am	Mona Vale Junction	
	7:46 am	Narrabeen [Pittwater Rd]	Via Mona, Bassett, Barrenjoey, Route 190, Harbord, Headland Rd
from	7:51 am	Collaroy	
Mona	7:58 am	Dee Why [Howard Ave]	
Vale	8:02 am	St Luke's Grammar School	
673n	7:29 am	Careel Head Road	FROM AVALON
All	7:34 am	Avalon	
Stops	7:39 am	Newport [Neptune Road]	Via Barrenjoey, Route 190, Harbord, Headland
	7:44 am	Newport Hotel	
from	7:51 am	Mona Vale Junction	
Avalon	7:57 am	Narrabeen [Pittwater Road]	
	8:01 am	Collaroy	
	8:07 am	Dee Why [Howard Ave]	
_	8:10 am	St Luke's Grammar School	
677	7:55 am	Warriewood Square	FROMWARIEWOOD
~~~	8:01 am	Narrabeen [Pittwater Road]	
	8:04 am	Collaroy	Via Jacksons, Pittwater Harbord, Headland
	8:10 am	Dee Why [Howard Ave]	
	8:15 am	St Luke's Grammar School	



684	684 7:35 am	Collaroy Plateau	FROM COLLAROY PLATEAU					
	7:45 am	Cromer Heights						
	7:51 am	Carawa & Carcoola Roads	Toronto, Route 173 to Cromer Heights, Route 173 to Carawa, Fishe					
	8:02 am	Dee Why [Howard Ave]	Road North, Lynwood, Fisher, McIntosh, 180 degree turn at Victor					
	8:05 am	Cnr Headland & Harbord	Road roundabout, McIntosh, Fisher, St Davids, Pittwater, Harbord,					
	8:07 am	St Luke's Grammar School	Headland					

# Afternoon Buses

669	3:34 pm	Departs Quirk St Stand	TOMANLY
	3:37 pm	NBSC Manly Campus	
	3:47 pm	Crown & Dowling [Queenscliff]	Via Quirk, Tango, Quirk, Headland, Harbord, Brighton, Bennett, Oliver,
	3:51 pm	Queenscliff Bridge	Lawence, bowing
	3:58 pm	Manly Wharf	
	Te could be		
668	3:37 pm	Departs Headland Rd Stand	TO SEAFORTH (via Manly Wharf)
	3:41 pm	NBSC Manly Campus	Via Tango, Quirk Headland, Harbord, Windborne, Pittwater, Belgrave
	3:46 pm	Brookvale Shops	West Esplanade, Commonwealth, Lauderdale, White, Woodland,
	3:47 pm	Brookvale Depot	Lower Beach, Bungaloe, New, Curban, Ernest, Woodland, Abbott,
	3:53 pm	Manly Lagoon [P'water & E'bin Ave]	Lewis, Ernest, Beatrice, Seaview, Upper Beach, Maretimo, Sydney,
	3:59 pm	Manly Wharf	Ross, Panorama, Ponsby, Paimerston, Alan, Salisbury, Ponsonby, Seaforth, Princes
	4:03 pm	Condamine St & White St	Senorm, rinces
	4:11 pm	Balgowlah Heights [Ernest St]	
	4:18 pm	Seaforth Shops	
_	4:23 pm	The Bluff	
670	3:36 pm	Departs Headland Rd Stand	TO SEAFORTH
	3:39 pm	NBSC Manly Campus	
	3:46 pm	Warringah Mall	Via Quirk, Headland, Harbord, Miles, Ada, Orchard, Pittwater,
	3:56 pm	Kitchener Road & Wanganella St	Frenchs Forest, Wakehurst Parkway/Judith
	4:00 pm	North Balgowlah	
	4:09 pm	Seaforth Oval [Wakehurst & Judith]	
671	3:35 pm	Departs Headland Rd Stand	TO FRENCHS FOREST (End Skyline Shops via Allambie Heights)
	3:40 pm	NBSC Manly Campus	
	3:44 pm	St Augustines Brookvale	Via Winbourne Road, Pittwater Road, Old Pittwater Road, Kentwell
	3:49 pm	Brookvale Depot	Troad, Anamole Road, Aquatic Dive
	3:54 pm	Condamine St & Kentwell Road	
	4:00 pm	Allambie Heights Shops	
	4:05 pm	Warringah Aquatic Centre	
	4:08 pm	Skyline Shops	
784	3:33 pm	Departs Headland Rd Stand	TO FRENCHS FOREST (End Forest Way Shops)
	3:49 pm	Warringah & Willandra Roads	
	3:54 pm	Skyline Shops, Frenchs Forest	Via Quirk, Carew, Headland, Wheeler, The Crescent, Pacific, Pittwater,
784	4:01 pm	Frenchs Forest	Warringah, Forest Way, Naree, Rabbett
	4:06 pm	Forest Way & Rabbett St	

# Afternoon Buses

672	3:32 pm	Departs Headland Rd Stand	TO COLLAROY PLATEAU
	3:40 pm	Dee Why [Howard Ave]	
672 677	3:45 pm	Narraweena [McIntosh & Alfred]	Via Headland, Grimn, The Strand, Howard, St Davids, Fisher, McIntosh, Alfred, Route 173, House, McNamara, Badcoe, Truman, Toronto, South
	3:50 pm	Carawa & Carcoola Roads	Creek, Ambleside, Rose, Veterans, Telopea, Fuller, Hall
	3:55 pm	Cromer Heights	
-	4:04 pm	Collaroy Plateau	
677	3:40 pm	Departs Headland Rd Stand	TO NARRABEEN
	3:48 pm	Dee Why [Howard Avenue]	
	3:57 pm	Collaroy	Via Headland Road, The Strand, Howard Avenue, Pittwater Road, Waterloo Narrabeen
	4:03 pm	Narrabeen [Waterloo Street]	Hatchios Humbeen
678	3:26 pm	Departs Headland Rd Stand	TO NEWPORT
	3:52 pm	Mona Vale Junction	
	4:01 pm	Newport Hotel	First set down Mona Vale. Via Headland, Harbord, Wheeler, The
4	4:07 pm	Newport [Neptune Road] First set down Mona Vale	Hotel Loop
787	3:26 pm	Departs Headland Rd Stand	TO AVALON
	3:33 pm	NBSC Manly Campus	
	3:35 pm	Freshwater Senior Campus	Via Headland Road, Harbord, Abbott, Pitt, Playfair, Parr, Headland, Wheeler The Crescent Pacific Avon Howard Pittwater Barrenioev
	3:48 pm	Dee Why [Howard Ave]	Beaconsfield, Kalinya, Gladstone, Barrenjoey, Plateau, Bilwara, Lower
	3:55 pm	Collaroy	Plateau, Wandeen, Hudson, Avalon
	3:59 pm	Narrabeen [Pittwater Road]	
	4:09 pm	Mona Vale Junction	
	4:15 pm	Newport Hotel	
	4:20 pm	Newport [Neptune Road]	
	4:24 pm	Plateau and Barrenjoey Roads	
	4:27 pm	Bilgola Plateau Shops	
	4:31 pm	Taylors Point	
	4:34 pm	Clareville [Hudson and Hilltop]	
	4:39 pm	Avalon	

### BUS TIMETABLES ONLINE:

www.sydneybuses.info/schools/school-services www.transport.info www.b-line.transport.nsw.gov.au

Locate "ST LUKE'S GRAMMAR SCHOOL" in the "Select School" option and identify the routes available.

Timetable Correct as of 12 February 2019

# **APPENDIX C**

# TRAFFIC SURVEY DATA

Client	Varga Traffic Engineering
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Location St Lukes Dee Why

Date Thu, 19th September 2019

- **Survey Time** 07:30 09:30 & 14:30 16:30 (4hrs)
- **Description** St Lukes Dee Why Pick Up & Drop Off Surveys



# MATRIX Traffic and Transport Data

St Lukes Dee Why

[Location]

Bus Stop 1 - Headland Rd(North Side) Bus Stop 2 - Quirk St(West Side) Car Drop Off 1 - Headland Rd(North Side) Car Drop Off 2 - Tango Ave(South Side) Car Drop Off 3 - Tango Ave(West Side) Car Drop Off 4 - Tango Ave(East Side)

# Client Varga Traffic Engineering Location Bus Stop 1 - Headland Rd(North Side) Date Thu, 19th September 2019 Survey Time 07:30 - 09:30 & 14:30 - 16:30 (4hrs)

Description St Lukes Dee Why Pick Up & Drop Off Surveys



Arrival	Departure	Duration of	Class	Pick Up			Drop Off			
Time	Time	Stay	Class	Grade 10-12	Others	Total	Grade 10-12	Others	Total	
7:48:20	7:49:57	0:01:37	Bus	0	0	0	4	17	21	
7:51:56	7:53:32	0:01:36	Bus	0	0	0	16	17	33	
7:53:44	7:54:09	0:00:25	Bus	0	1	1	1	3	4	
8:01:14	8:01:41	0:00:27	Bus	0	0	0	6	8	14	
8:03:14	8:04:34	0:01:20	Bus	0	0	0	4	4	8	
8:03:20	8:04:10	0:00:50	Bus	0	0	0	9	17	26	
8:08:36	8:09:18	0:00:42	Bus	0	0	0	2	6	8	
15:15:09	15:16:09	0:01:00	Car	0	1	1	0	0	0	
15:37:55	15:43:35	0:05:40	Bus	6	35	41	0	0	0	
15:40:35	15:43:39	0:03:04	Bus	9	23	32	0	0	0	
15:47:15	15:48:57	0:01:42	Bus	6	19	25	0	0	0	
15:49:44	15:51:41	0:01:57	Bus	3	6	9	0	0	0	
15:47:54	15:52:16	0:04:22	Bus	4	41	45	0	0	0	
15:52:33	15:54:18	0:01:45	Bus	6	20	26	0	0	0	
15:55:31	15:56:35	0:01:04	Bus	1	17	18	0	0	0	
15:57:43	15:59:35	0:01:52	Bus	2	23	25	0	0	0	
16:01:07	16:02:40	0:01:33	Bus	5	14	19	0	0	0	

[Peak Hr Summary]											
A		Pick Up			ind tal						
Approach	Grade 10-12	Others	Total	Grade 10-12	Others	Total	Tot B				
7:15 - 8:15	0	1	1	42	72	114	115				
15:15 - 16:15	42	199	241	0	0	0	241				

Hourly Summa	ary]						
6		Pick Up				nd tal	
Approach	Grade 10-12	Others	Total	Grade 10-12	Others	Total	Gra
7:00 - 8:00	0	1	1	21	37	58	59
7:15 - 8:15	0	1	1	42	72	114	115
7:30 - 8:30	0	1	1	42	72	114	115
7:45 - 8:45	0	1	1	42	72	114	115
8:00 - 9:00	0	0	0	21	35	56	56
8:15 - 9:15	0	0	0	0	0	0	0
8:30 - 9:30	0	0	0	0	0	0	0
AM Total	0	1	1	42	72	114	115
14:30 - 15:30	0	1	1	0	0	0	1
14:45 - 15:45	15	59	74	0	0	0	74
15:00 - 16:00	37	185	222	0	0	0	222
15:15 - 16:15	42	199	241	0	0	0	241
15:30 - 16:30	42	198	240	0	0	0	240
PM Total	42	199	241	0	0	0	241

[15mins interval	]						
•		Pick Up			al d		
Approach	Grade 10-12	Others	Total	Grade 10-12	Others	Total	10 E
7:00 - 7:15	0	0	0	0	0	0	Ó
7:15 - 7:30	0	0	0	0	0	0	0
7:30 - 7:45	0	0	0	0	0	0	0
7:45 - 8:00	0	1	1	21	37	58	59
8:00 - 8:15	0	0	0	21	35	56	56
8:15 - 8:30	0	0	0	0	0	0	0
8:30 - 8:45	0	0	0	0	0	0	0
8:45 - 9:00	0	0	0	0	0	0	0
9:00 - 9:15	0	0	0	0	0	0	0
9:15 - 9:30	0	0	0	0	0	0	0
AM Total	0	1	1	42	72	114	115
14:30 - 14:45	0	0	0	0	0	0	0
14:45 - 15:00	0	0	0	0	0	0	0
15:00 - 15:15	0	0	0	0	0	0	0
15:15 - 15:30	0	1	1	0	0	0	1
15:30 - 15:45	15	58	73	0	0	0	73
15:45 - 16:00	22	126	148	0	0	0	148
16:00 - 16:15	5	14	19	0	0	0	19
16:15 - 16:30	0	0	0	0	0	0	0
PM Total	42	199	241	0	0	0	241

Client	Varga Traffic Engineering
Location	Bus Stop 2 - Quirk St(West Side)
Date	Thu, 19th September 2019
Survey Time	07:30 - 09:30 & 14:30 - 16:30 (4hrs)
Description	St Lukes Dee Why Pick Up & Drop Off Surveys



Arrival	Departure	Duration of	Class	Pick Up			Drop Off		
Time	Time Time Stay	Stay	Class	Grade 10-12	Others	Total	Grade 10-12	Others	Total
8:04:55	8:05:16	0:00:21	Car	0	0	0	0	1	1
8:05:04	8:05:14	0:00:10	Car	0	0	0	0	3	3
8:11:35	8:12:32	0:00:57	Bus	0	0	0	8	18	26
15:36:19	15:36:43	0:00:24	Car	0	1	1	0	0	0
15:38:22	15:38:55	0:00:33	Car	0	1	1	0	5	5
15:45:09	15:48:10	0:03:01	Bus	8	23	31	0	2	2
15:59:18	16:00:37	0:01:19	Bus	0	0	0	0	4	4
16:20:52	16:21:10	0:00:18	Car	0	1	1	0	0	0
16:24:35	16:30:00	0:05:25	Car	0	0	0	0	1	1

[Peak Hr Summary]							
Annaach		Pick Up			ind tal		
Approach	Grade 10-12	Others	Total	Grade 10-12	Others	Total	Gra To
7:15 - 8:15	0	0	0	8	22	30	30
15:30 - 16:30	8	26	34	0	12	12	46

[Hourly Summa	ary]						
A		Pick Up			Drop Off		tal nd
Approacn	Grade 10-12	Others	Total	Grade 10-12	Others	Total	Toi Toi
7:00 - 8:00	0	0	0	0	0	0	0
7:15 - 8:15	0	0	0	8	22	30	30
7:30 - 8:30	0	0	0	8	22	30	30
7:45 - 8:45	0	0	0	8	22	30	30
8:00 - 9:00	0	0	0	8	22	30	30
8:15 - 9:15	0	0	0	0	0	0	0
8:30 - 9:30	0	0	0	0	0	0	0
AM Total	0	0	0	8	22	30	30
14:30 - 15:30	0	0	0	0	0	0	0
14:45 - 15:45	0	2	2	0	5	5	7
15:00 - 16:00	8	25	33	0	11	11	44
15:15 - 16:15	8	25	33	0	11	11	44
15:30 - 16:30	8	26	34	0	12	12	46
PM Total	8	26	34	0	12	12	46

0		Pick Up			Drop Off		ta d
Approach	Grade 10-12	Others	Total	Grade 10-12	Others	Total	5 Gra
7:00 - 7:15	0	0	0	0	0	0	0
7:15 - 7:30	0	0	0	0	0	0	0
7:30 - 7:45	0	0	0	0	0	0	0
7:45 - 8:00	0	0	0	0	0	0	0
8:00 - 8:15	0	0	0	8	22	30	30
8:15 - 8:30	0	0	0	0	0	0	0
8:30 - 8:45	0	0	0	0	0	0	0
8:45 - 9:00	0	0	0	0	0	0	0
9:00 - 9:15	0	0	0	0	0	0	0
9:15 - 9:30	0	0	0	0	0	0	0
AM Total	0	0	0	8	22	30	30
14:30 - 14:45	0	0	0	0	0	0	0
14:45 - 15:00	0	0	0	0	0	0	0
15:00 - 15:15	0	0	0	0	0	0	0
15:15 - 15:30	0	0	0	0	0	0	0
15:30 - 15:45	0	2	2	0	5	5	7
15:45 - 16:00	8	23	31	0	6	6	37
16:00 - 16:15	0	0	0	0	0	0	0
16:15 - 16:30	0	1	1	0	1	1	2
PM Total	8	26	34	0	12	12	46

#### Client Varga Traffic Engineering Location Car Drop Off 1 - Headland Rd(North Side) Thu, 19th September 2019

Date

Survey Time 07:30 - 09:30 & 14:30 - 16:30 (4hrs)

### Description St Lukes Dee Why Pick Up & Drop Off Surveys

Arrival	Departure	Duration of	Class	Pick Up		Drop Off			
Time	Time	Stay	Class	Grade 10-12	Others	Total	Grade 10-12	Others	Total
6:58:35	7:00:48	0:02:13	Car	0	0	0	2	0	2
7:00:00	7:27:30	0:27:30	Car	0	1	1	0	0	0
7:18:09	7:18:22	0:00:13	Car	0	0	0	0	1	1
7:24:57	7:25:27	0:00:30	Car	0	0	0	0	1	1
7:37:25	7:37:38	0:00:13	Car	0	0	0	1	1	2
7:38:18	7:38:32	0:00:14	Car	0	0	0	0	1	1
7:44:39	7:44:49	0:00:10	Car	0	0	0	1	0	1
7:46:45	7:47:01	0:00:16	Car	0	0	0	2	0	2
8:00:41	8:00:55	0:00:14	Car	0	0	0	0	2	2
8:02:23	8:02:38	0:00:15	Car	0	0	0	0	2	2
8:04:00	8:04:05	0:00:05	Car	0	0	0	0	1	1
8:04:46	8:05:16	0:00:30	Car	0	1	1	1	11	12
8:07:28	8:07:35	0:00:07	Car	0	0	0	0	1	1
8:08:27	8:08:51	0:00:24	Car	0	0	0	0	3	3
8:19:54	8:20:09	0:00:15	Car	0	0	0	0	1	1
8:32:52	8:33:11	0:00:19	Car	0	0	0	0	1	1
8:53:49	8:54:31	0:00:42	Car	0	0	0	0	1	1
14:30:01	14:32:00	0:01:59	Car	0	1	1	0	0	0
14:33:40	14:40:13	0:06:33	Car	0	1	1	0	0	0
14:40:42	14:54:45	0:14:03	Car	0	2	2	0	0	0

•	Pick Up					tal	
Approach	Grade 10-12	Others	Total	Grade 10-12	Others	Total	Gra
7:30 - 8:30	0	1	1	5	23	28	29
15:00 - 16:00	0	0	0	0	0	0	0

MATRIX

[Hourly Summa	ary]						
Annroach		Pick Up			Drop Off		tal
Approach	Grade 10-12	Others	Total	Grade 10-12	Others	Total	Gra
7:00 - 8:00	0	1	1	6	4	10	11
7:15 - 8:15	0	1	1	5	24	29	30
7:30 - 8:30	0	1	1	5	23	28	29
7:45 - 8:45	0	1	1	3	22	25	26
8:00 - 9:00	0	1	1	1	23	24	25
8:15 - 9:15	0	0	0	0	3	3	3
8:30 - 9:30	0	0	0	0	2	2	2
AM Total	0	2	2	7	27	34	31
14:30 - 15:30	0	4	4	0	0	0	4
14:45 - 15:45	0	0	0	0	0	0	0
15:00 - 16:00	0	0	0	0	0	0	0
15:15 - 16:15	0	0	0	0	0	0	0
15:30 - 16:30	0	0	0	0	0	0	0
PM Total	0	4	4	0	0	0	4

#### [15mins interval]

•		Pick Up			nd		
Approach	Grade 10-12	Others	Total	Grade 10-12	Others	Total	Gra
7:00 - 7:15	0	1	1	2	0	2	3
7:15 - 7:30	0	0	0	0	2	2	2
7:30 - 7:45	0	0	0	2	2	4	4
7:45 - 8:00	0	0	0	2	0	2	2
8:00 - 8:15	0	1	1	1	20	21	22
8:15 - 8:30	0	0	0	0	1	1	1
8:30 - 8:45	0	0	0	0	1	1	1
8:45 - 9:00	0	0	0	0	1	1	1
9:00 - 9:15	0	0	0	0	0	0	0
9:15 - 9:30	0	0	0	0	0	0	0
AM Total	0	2	2	7	27	34	31
14:30 - 14:45	0	4	4	0	0	0	4
14:45 - 15:00	0	0	0	0	0	0	0
15:00 - 15:15	0	0	0	0	0	0	0
15:15 - 15:30	0	0	0	0	0	0	0
15:30 - 15:45	0	0	0	0	0	0	0
15:45 - 16:00	0	0	0	0	0	0	0
16:00 - 16:15	0	0	0	0	0	0	0
16:15 - 16:30	0	0	0	0	0	0	0
PM Total	0	4	4	0	0	0	4

Client	Varga Traffic Engineering
Location	Car Drop Off 2 - Tango Ave(South Side)
Date	Thu, 19th September 2019
Survey Time	07:30 - 09:30 & 14:30 - 16:30 (4hrs)
Description	St Lukes Dee Why Pick Up & Drop Off Surveys

#### [15mins interval]

Annreach		Pick Up			Drop Off		nd tal	
Approach	Grade 10-12	Others	Total	Grade 10-12	Others	Total	Gra	
7:00 - 7:15	0	0	0	2	3	5	5	
7:15 - 7:30	0	0	0	4	13	17	17	
7:30 - 7:45	0	0	0	8	20	28	28	
7:45 - 8:00	0	0	0	24	57	81	81	
8:00 - 8:15	0	0	0	14	132	146	146	
8:15 - 8:30	0	0	0	9	51	60	60	
8:30 - 8:45	0	0	0	0	2	2	2	
8:45 - 9:00	0	0	0	0	0	0	0	
9:00 - 9:15	0	0	0	0	0	0	0	
9:15 - 9:30	0	0	0	0	0	0	0	
AM Total	0	0	0	61	278	339	317	
14:30 - 14:45	0	0	0	0	1	1	1	
14:45 - 15:00	0	3	3	0	2	2	5	
15:00 - 15:15	0	19	19	0	0	0	19	
15:15 - 15:30	11	106	117	0	0	0	117	
15:30 - 15:45	16	24	40	0	0	0	40	
15:45 - 16:00	1	1	2	0	2	2	4	
16:00 - 16:15	2	3	5	0	0	0	5	
16:15 - 16:30	3	1	4	0	0	0	4	
PM Total	33	157	190	0	5	5	195	

#### [Peak Hr Summary]

ſ	Annroach		Pick Up			ind tal		
	Approach	Grade 10-12	Others	Total	Grade 10-12	Others	Total	Gra To
ſ	7:30 - 8:30	0	0	0	55	260	315	315
Ī	15:00 - 16:00	28	150	178	0	2	2	181

#### [Hourly Summary]

Annash		Pick Up			Drop Off		ind tal
Approach	Grade 10-12	Others	Total	Grade 10-12	Others	Total	Gra
7:00 - 8:00	0	0	0	38	93	131	131
7:15 - 8:15	0	0	0	50	222	272	272
7:30 - 8:30	0	0	0	55	260	315	315
7:45 - 8:45	0	0	0	47	242	289	289
8:00 - 9:00	0	0	0	23	185	208	208
8:15 - 9:15	0	0	0	9	53	62	62
8:30 - 9:30	0	0	0	0	2	2	2
AM Total	0	0	0	61	278	339	317
14:30 - 15:30	11	128	139	0	3	3	142
14:45 - 15:45	27	152	179	0	2	2	181
15:00 - 16:00	28	150	178	0	2	2	180
15:15 - 16:15	30	134	164	0	2	2	166
15:30 - 16:30	22	29	51	0	2	2	53
PM Total	33	157	190	0	5	5	195



Client	Varga Traffic Engineering
Location	Car Drop Off 3 - Tango Ave(West Side)
Date	Thu, 19th September 2019
Survey Time	07:30 - 09:30 & 14:30 - 16:30 (4hrs)
Description	St Lukes Dee Why Pick Up & Drop Off Surveys



Arrival	Departure	Duration of	Class		Pick Up		Drop Off		
Time	Time	Stay	Class	Grade 10-12	Others	Total	Grade 10-12	Others	Total
7:00:00	7:07:57	0:07:57	Car	0	1	1	0	0	0
7:41:32	9:00:00	1:18:28	Car	0	0	0	0	1	1
7:57:48	8:09:14	0:11:26	Car	0	0	0	0	1	1
7:57:56	7:58:22	0:00:26	Car	0	1	1	0	0	0
8:04:27	8:23:36	0:19:09	Car	0	0	0	0	1	1
8:06:29	8:07:04	0:00:35	Car	0	0	0	0	1	1
8:17:00	9:00:00	0:43:00	Car	0	0	0	0	1	1
8:21:39	9:00:00	0:38:21	Car	0	0	0	0	1	1
14:30:00	15:16:42	0:46:42	Car	0	2	2	0	0	0
14:30:00	15:31:04	1:01:04	Car	0	1	1	0	0	0
14:30:00	16:30:00	2:00:00	Car	0	1	1	0	0	0
14:30:00	16:28:29	1:58:29	Car	0	1	1	0	0	0
14:56:02	15:15:00	0:18:58	Car	0	2	2	0	0	0
15:08:41	15:28:08	0:19:27	Car	0	2	2	0	0	0
15:17:04	15:32:53	0:15:49	Car	0	1	1	0	0	0
15:26:20	15:37:32	0:11:12	Car	0	1	1	0	0	0
15:31:42	15:46:03	0:14:21	Car	0	1	1	0	0	0
15:34:08	15:44:34	0:10:26	Car	0	1	1	0	0	0

[Peak Hr Summ	ary]						
Annroach		Pick Up			Drop Off		ind tal
Approach	Grade 10-12	Others	Total	Grade 10-12	Others	Total	Gra
7:30 - 8:30	0	1	1	0	6	6	7
14:45 - 15:45	0	8	8	0	0	0	8

[Hourly Summa	ary]							
A		Pick Up			Drop Off		nd	
Approach	Grade 10-12	Others	Total	Grade 10-12	Others	Total	Gra	
7:00 - 8:00	0	2	2	0	2	2	4	
7:15 - 8:15	0	1	1	0	4	4	5	
7:30 - 8:30	0	1	1	0	6	6	7	
7:45 - 8:45	0	1	1	0	5	5	6	
8:00 - 9:00	0	0	0	0	4	4	4	
8:15 - 9:15	0	0	0	0	2	2	2	
8:30 - 9:30	0	0	0	0	0	0	0	
AM Total	0	2	2	0	6	6	7	
14:30 - 15:30	0	11	11	0	0	0	11	
14:45 - 15:45	0	8	8	0	0	0	8	
15:00 - 16:00	0	6	6	0	0	0	6	
15:15 - 16:15	0	4	4	0	0	0	4	
15:30 - 16:30	0	2	2	0	0	2		
PM Total	0	13	13	0	0	0	13	

#### [15mins interval]

1		Pick Up			Drop Off	Drop Off						
Approach	Grade 10-12	Others	Total	Grade 10-12	Others	Total	Gra					
7:00 - 7:15	0	1	1	0	0	0	1					
7:15 - 7:30	0	0	0	0	0	0	0					
7:30 - 7:45	0	0	0	0	1	1	1					
7:45 - 8:00	0	1	1	0	1	1	2					
8:00 - 8:15	0	0	0	0	2	2	2					
8:15 - 8:30	0	0	0	0	2	2	2					
8:30 - 8:45	0	0	0	0	0	0	0					
8:45 - 9:00	0	0	0	0	0	0	0					
9:00 - 9:15	0	0	0	0	0	0	0					
9:15 - 9:30	0	0	0	0	0	0	0					
AM Total	0	2	2	0	6	6	7					
14:30 - 14:45	0	5	5	0	0	0	5					
14:45 - 15:00	0	2	2	0	0	0	2					
15:00 - 15:15	0	2	2	0	0	0	2					
15:15 - 15:30	0	2	2	0	0	0	2					
15:30 - 15:45	0	2	2	0	0	0	2					
15:45 - 16:00	0	0	0	0	0	0	0					
16:00 - 16:15	0	0	0	0	0	0	0					
16:15 - 16:30	0	0	0	0	0	0	0					
PM Total	0	13	13	0	0	0	13					

Client	Varga Traffic Engineering
Location	Car Drop Off 4 - Tango Ave(East Side)
Date	Thu, 19th September 2019
Survey Time	07:30 - 09:30 & 14:30 - 16:30 (4hrs)
Description	St Lukes Dee Why Pick Up & Drop Off Surveys

Arrival	Departure	Duration of	Class		Pick Up	Drop Off							
Time	Time	Stay	Class	Grade 10-12	Others	Total	Grade 10-12	Others	Total				
7:00:00	7:12:23	0:12:23	Car	0	1	1	0	0	0				
7:00:00	7:40:16	0:40:16	Car	0	1	1	0	0	0				
7:56:29	9:00:00	1:03:31	Car	0	0	0	0	2	2				
7:56:36	9:00:00	1:03:24	Car	0	0	0	0	3	3				
14:30:00	16:14:14	1:44:14	Car	0	5	5	0	0	0				

MATRIX

Drop Off

Others

5

0

Total

5

0

Grade 10-12

0

0

Total

0

0

Grand Total

5

0

Annroach		Pick Up			Drop Off		tal ud
Арргоасп	Grade 10-12	Others	Total	Grade 10-12	Others	Total	Gra
7:00 - 8:00	0	2	2	0	5	5	7
7:15 - 8:15	0	0	0	0	5	5	5
7:30 - 8:30	0	0	0	0	5	5	5
7:45 - 8:45	0	0	0	0	5	5	5
8:00 - 9:00	0	0	0	0	0	0	0
8:15 - 9:15	0	0	0	0	0	0	0
8:30 - 9:30	0	0	0	0	0	0	0
AM Total	0	2	2	0	5	5	5
L4:30 - 15:30	0	5	5	0	0	0	5
L4:45 - 15:45	0	0	0	0	0	0	0
15:00 - 16:00	0	0	0	0	0	0	0
5:15 - 16:15	0	0	0	0	0	0	0
5:30 - 16:30	0	0	0	0	0	0	0
PM Total	0	5	5	0	0	0	5

[Peak Hr Summary]

Grade 10-12

0

0

Approach

7:30 - 8:30

14:45 - 15:45

Pick Up

Others

0

0

#### [15mins interval]

A			ind tal				
Approach	Grade 10-12	Others	Total	Grade 10-12	Others	Total	Gra
7:00 - 7:15	0	2	2	0	0	0	2
7:15 - 7:30	0	0	0	0	0	0	0
7:30 - 7:45	0	0	0	0	0	0	0
7:45 - 8:00	0	0	0	0	5	5	5
8:00 - 8:15	0	0	0	0	0	0	0
8:15 - 8:30	0	0	0	0	0	0	0
8:30 - 8:45	0	0	0	0	0	0	0
8:45 - 9:00	0	0	0	0	0	0	0
9:00 - 9:15	0	0	0	0	0	0	0
9:15 - 9:30	0	0	0	0	0	0	0
AM Total	0	2	2	0	5	5	5
14:30 - 14:45	0	5	5	0	0	0	5
14:45 - 15:00	0	0	0	0	0	0	0
15:00 - 15:15	0	0	0	0	0	0	0
15:15 - 15:30	0	0	0	0	0	0	0
15:30 - 15:45	0	0	0	0	0	0	0
15:45 - 16:00	0	0	0	0	0	0	0
16:00 - 16:15	0	0	0	0	0	0	0
16:15 - 16:30	0	0	0	0	0	0	0
PM Total	0	5	5	0	0	0	5

Job No.	: N5401		
Client	: Varga Traffic Plar	nning	
Suburb	: Dee Why		
Location	: 1. Warringah Rd	/ Pittwater Rd / Ha	rbord Rd
Day/Date	: Tue, 29th Octobe	er 2019	
Weather	: Fine		
Description	: Classified Interse	ction Count	
	: 15 mins Data		
	Class 1	Class 2	
Classifications	Lights	Heavies	





Approach						Harbo	ord Rd					Pittwater Rd												
Direction		Direction (Left Turn	1		Direction 2 (Through)	2	(	Direction 3 Right Turr	3 1)	C	Direction 3 (U Turn)	U		Direction (Left Turn	4 )	1	Direction 5 (Through)	i		Direction ( Right Turr	5 1)	I	Direction 6 (U Turn)	J
Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total
7:00 to 7:15	2	1	3	74	6	80	60	5	65	0	0	0	93	2	95	318	29	347	76	0	76	0	0	0
7:15 to 7:30	3	1	4	88	9	97	58	6	64	0	0	0	127	7	134	369	27	396	99	6	105	0	0	0
7:30 to 7:45	4	0	4	109	3	112	69	5	74	0	0	0	116	6	122	374	27	401	94	0	94	0	0	0
7:45 to 8:00	7	2	9	81	8	89	71	4	75	0	0	0	123	1	124	368	30	398	82	1	83	0	0	0
8:00 to 8:15	7	2	9	102	4	106	58	4	62	0	0	0	122	6	128	370	36	406	65	1	66	0	0	0
8:15 to 8:30	4	1	5	94	6	100	52	3	55	0	0	0	98	4	102	400	19	419	64	1	65	0	0	0
8:30 to 8:45	13	1	14	96	5	101	86	5	91	0	0	0	93	2	95	353	28	381	82	2	84	0	0	0
8:45 to 9:00	14	1	15	94	10	104	74	2	76	0	0	0	118	8	126	338	22	360	66	2	68	0	0	0
AM Totals	54	9	63	738	51	789	528	34	562	0	0	0	890	36	926	2,890	218	3,108	628	13	641	0	0	0
15:00 to 15:15	8	0	8	104	3	107	71	7	78	0	0	0	78	5	83	265	29	294	85	5	90	0	0	0
15:15 to 15:30	5	1	6	107	4	111	99	4	103	0	0	0	95	6	101	245	15	260	100	1	101	0	0	0
15:30 to 15:45	7	0	7	120	4	124	82	3	85	0	0	0	84	4	88	219	21	240	95	1	96	0	0	0
15:45 to 16:00	13	1	14	111	3	114	89	3	92	0	0	0	94	4	98	261	20	281	66	5	71	0	0	0
16:00 to 16:15	8	1	9	117	4	121	84	1	85	0	0	0	85	5	90	220	22	242	79	2	81	0	0	0
16:15 to 16:30	10	0	10	107	4	111	87	3	90	0	0	0	112	2	114	263	25	288	65	5	70	0	0	0
16:30 to 16:45	13	0	13	112	4	116	102	1	103	0	0	0	97	2	99	182	13	195	74	3	77	0	0	0
16:45 to 17:00	10	1	11	123	4	127	81	2	83	0	0	0	110	0	110	259	29	288	53	2	55	0	0	0
PM Totals	74	4	78	901	30	931	695	24	719	0	0	0	755	28	783	1,914	174	2,088	617	24	641	0	0	0

Approach						Warrin	igah Rd											Pittwa	ater Rd						Crossing								
Direction		Direction (Left Turn	7 )		Direction (Through	B		Direction (Right Tur	9 n)		Direction 9 (U Turn)	U		Direction 1 (Left Turn	LO )	C	Direction 1 (Through)	1		Direction 1 Right Turr	2 1)	(	Direction 12 (U Turn)	U	Pedestrians								
Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	B to A	A to B	D to C	C to D	F to E	E to F	H to G	G to H	Total
7:00 to 7:1	i 25	1	26	96	5	101	67	4	71	0	0	0	5	7	12	152	29	181	32	0	32	0	0	0	0	6	1	4	3	2	0	0	16
7:15 to 7:30	9	1	10	97	3	100	78	6	84	0	0	0	4	7	11	169	27	196	19	2	21	0	0	0	3	6	3	4	3	8	3	1	31
7:30 to 7:4	5 27	0	27	118	3	121	71	6	77	0	0	0	9	9	18	198	26	224	26	0	26	0	0	0	1	5	2	3	6	6	4	0	27
7:45 to 8:00	17	1	18	123	2	125	99	4	103	0	0	0	7	8	15	242	29	271	22	3	25	0	0	0	4	16	2	8	11	6	7	6	60
8:00 to 8:1	i 18	3	21	111	3	114	67	6	73	0	0	0	15	6	21	240	21	261	21	2	23	0	0	0	2	6	2	8	4	14	1	4	41
8:15 to 8:30	17	3	20	116	9	125	49	5	54	0	0	0	20	10	30	227	17	244	24	2	26	0	0	0	6	10	2	3	16	3	9	0	49
8:30 to 8:4	i 28	5	33	141	7	148	59	12	71	0	0	0	10	8	18	188	19	207	35	0	35	0	0	0	2	2	1	7	5	5	4	1	27
8:45 to 9:00	21	4	25	115	5	120	86	8	94	0	0	0	18	8	26	212	14	226	46	1	47	0	0	0	1	1	2	2	6	6	3	0	21
AM Totals	162	18	180	917	37	954	576	51	627	0	0	0	88	63	151	1,628	182	1,810	225	10	235	0	0	0	19	52	15	39	54	50	31	12	272
15:00 to 15:1	5 34	1	35	101	2	103	39	6	45	0	0	0	21	14	35	369	14	383	43	1	44	0	0	0	7	1	2	1	3	3	1	0	18
15:15 to 15:3	0 35	3	38	101	4	105	50	7	57	0	0	0	19	9	28	317	17	334	32	4	36	0	0	0	5	14	11	2	13	7	3	2	57
15:30 to 15:4	5 49	3	52	83	3	86	55	7	62	0	0	0	17	18	35	359	16	375	26	1	27	0	0	0	7	4	9	2	10	7	4	1	44
15:45 to 16:0	0 50	1	51	106	9	115	52	9	61	0	0	0	26	12	38	328	30	358	29	1	30	0	0	0	5	2	4	7	1	8	2	1	30
16:00 to 16:1	5 51	1	52	92	2	94	42	5	47	0	0	0	22	5	27	405	17	422	15	2	17	0	0	0	10	4	3	11	5	7	3	5	48
16:15 to 16:3	0 47	4	51	114	4	118	35	6	41	0	0	0	28	3	31	400	24	424	35	2	37	0	0	0	4	1	2	6	6	12	0	2	33
16:30 to 16:4	5 62	2	64	132	5	137	55	2	57	0	0	0	29	8	37	379	22	401	32	0	32	0	0	0	7	5	5	3	5	9	5	4	43
16:45 to 17:0	0 46	6	52	128	2	130	56	2	58	0	0	0	22	3	25	385	18	403	35	1	36	0	0	0	3	5	2	0	4	5	0	5	24
PM Totals	374	21	395	857	31	888	384	44	428	0	0	0	184	72	256	2,942	158	3,100	247	12	259	0	0	0	48	36	38	32	47	58	18	20	297

Job No.	: N5401	
Client	: Varga Traffic Planning	
Suburb	: Dee Why	
Location	: 1. Warringah Rd / Pittwater Rd / Ha	rbord Rd
Day/Date	: Tue, 29th October 2019	
Weather	: Fine	
Description	: Classified Intersection Count	
	: Hourly Summary	





Approa	ach	Harbord Rd																Pittwa	ater Rd						
Directi	on	-	Direction 1 (Left Turn)	1 )		Direction 2 (Through)	2	Direction 3 (Right Turn)			[	Direction 3 (U Turn)	U		Direction 4 (Left Turn)	4 )	1	Direction 5 (Through)	i	(	Direction ( Right Turr	; )	1	virection 6U (U Turn)	J
Time Pe	riod	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total
7:00	to 8:00	16	4	20	352	26	378	258	20	278	0	0	0	459	16	475	1,429	113	1,542	351	7	358	0	0	0
7:15	to 8:15	21	5	26	380	24	404	256	19	275	0	0	0	488	20	508	1,481	120	1,601	340	8	348	0	0	0
7:30	to 8:30	22	5	27	386	21	407	250	16	266	0	0	0	459	17	476	1,512	112	1,624	305	3	308	0	0	0
7:45	to 8:45	31	6	37	373	23	396	267	16	283	0	0	0	436	13	449	1,491	113	1,604	293	5	298	0	0	0
8:00	to 9:00	38	5	43	386	25	411	270	14	284	0	0	0	431	20	451	1,461	105	1,566	277	6	283	0	0	0
AM Tot	als	54	9	63	738	51	789	528	34	562	0	0	0	890	36	926	2,890	218	3,108	628	13	641	0	0	0
15:00	to 16:00	33	2	35	442	14	456	341	17	358	0	0	0	351	19	370	990	85	1,075	346	12	358	0	0	0
15:15	to 16:15	33	3	36	455	15	470	354	11	365	0	0	0	358	19	377	945	78	1,023	340	9	349	0	0	0
15:30	to 16:30	38	2	40	455	15	470	342	10	352	0	0	0	375	15	390	963	88	1,051	305	13	318	0	0	0
15:45	to 16:45	44	2	46	447	15	462	362	8	370	0	0	0	388	13	401	926	80	1,006	284	15	299	0	0	0
16:00	to 17:00	41	2	43	459	16	475	354	7	361	0	0	0	404	9	413	924	89	1,013	271	12	283	0	0	0
PM Tot	als	74	4	78	901	30	931	695	24	719	0	0	0	755	28	783	1,914	174	2,088	617	24	641	0	0	0

Appro	ach						Warri	ngah Rd											Pittwa	ater Rd						Crossing								
Direct	on		Direction (Left Turn	7		Direction (Through	8 )		Direction (Right Turr	9 1)	I	Direction 9 (U Turn)	U	1	Direction 1 (Left Turn	10 1)	C	irection 1 (Through)	1		Direction 1 (Right Turr	L2 n)	D	irection 12 (U Turn)	U					Pedestria	ıs			
Time Pe	riod	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	B to A	A to B	D to C	C to D	F to E	E to F	H to G	G to H	Total
7:00	to 8:00	78	3	81	434	13	447	315	20	335	0	0	0	25	31	56	761	111	872	99	5	104	0	0	0	8	33	8	19	23	22	14	7	134
7:15	to 8:15	71	5	76	449	11	460	315	22	337	0	0	0	35	30	65	849	103	952	88	7	95	0	0	0	10	33	9	23	24	34	15	11	159
7:30	to 8:30	79	7	86	468	17	485	286	21	307	0	0	0	51	33	84	907	93	1,000	93	7	100	0	0	0	13	37	8	22	37	29	21	10	177
7:45	to 8:45	80	12	92	491	21	512	274	27	301	0	0	0	52	32	84	897	86	983	102	7	109	0	0	0	14	34	7	26	36	28	21	11	177
8:00	to 9:00	84	15	99	483	24	507	261	31	292	0	0	0	63	32	95	867	71	938	126	5	131	0	0	0	11	19	7	20	31	28	17	5	138
AM To	als	162	18	180	917	37	954	576	51	627	0	0	0	88	63	151	1,628	182	1,810	225	10	235	0	0	0	19	52	15	39	54	50	31	12	272
15:00	to 16:00	168	8	176	391	18	409	196	29	225	0	0	0	83	53	136	1,373	77	1,450	130	7	137	0	0	0	24	21	26	12	27	25	10	4	149
15:15	to 16:15	185	8	193	382	18	400	199	28	227	0	0	0	84	44	128	1,409	80	1,489	102	8	110	0	0	0	27	24	27	22	29	29	12	9	179
15:30	to 16:30	197	9	206	395	18	413	184	27	211	0	0	0	93	38	131	1,492	87	1,579	105	6	111	0	0	0	26	11	18	26	22	34	9	9	155
15:45	to 16:45	210	8	218	444	20	464	184	22	206	0	0	0	105	28	133	1,512	93	1,605	111	5	116	0	0	0	26	12	14	27	17	36	10	12	154
16:00	to 17:00	206	13	219	466	13	479	188	15	203	0	0	0	101	19	120	1,569	81	1,650	117	5	122	0	0	0	24	15	12	20	20	33	8	16	148
PM Tot	als	374	21	395	857	31	888	384	44	428	0	0	0	184	72	256	2,942	158	3,100	247	12	259	0	0	0	48	36	38	32	47	58	18	20	297







	Approach		Approach Harbord Rd			Pittwater Rd			Wa	arringah	Rd	Pit	twater	Rd	otal	
	Time Period		riod	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Grand T
AM	7:30	to	8:30	658	42	700	2,276	132	2,408	833	45	878	1,051	133	1,184	5,170
PM	16:00	to	17:00	854	25	879	1,599	110	1,709	860	41	901	1,787	105	1,892	5,381

Ap	proa	ich	H	arbord F	۲d	Pit	ttwater	Rd	Wa	arringah	Rd	Pit	ttwater	Rd	otal
Tim	e Pe	riod	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Grand T
7:00	to	8:00	626	50	676	2,239	136	2,375	827	36	863	885	147	1,032	4,946
7:15	to	8:15	657	48	705	2,309	148	2,457	835	38	873	972	140	1,112	5,147
7:30	to	8:30	658	42	700	2,276	132	2,408	833	45	878	1,051	133	1,184	5,170
7:45	to	8:45	671	45	716	2,220	131	2,351	845	60	905	1,051	125	1,176	5,148
8:00	to	9:00	694	44	738	2,169	131	2,300	828	70	898	1,056	108	1,164	5,100
AN	/I Tot	als	1,320	94	1,414	4,408	267	4,675	1,655	106	1,761	1,941	255	2,196	10,046
15:00	to	16:00	816	33	849	1,687	116	1,803	755	55	810	1,586	137	1,723	5,185
15:15	to	16:15	842	29	871	1,643	106	1,749	766	54	820	1,595	132	1,727	5,167
15:30	to	16:30	835	27	862	1,643	116	1,759	776	54	830	1,690	131	1,821	5,272
15:45	to	16:45	853	25	878	1,598	108	1,706	838	50	888	1,728	126	1,854	5,326
16:00	to	17:00	854	25	879	1,599	110	1,709	860	41	901	1,787	105	1,892	5,381
PN	/I Tot	als	1,670	58	1,728	3,286	226	3,512	1,615	96	1,711	3,373	242	3,615	10,566



Job No.	: N5401
Client	: Varga Traffic Planning
Suburb	: Dee Why
Location	: 1. Warringah Rd / Pittwater Rd / Harbord Rd
Day/Date	: Tue, 29th October 2019
Weather	: Fine
Description	: Classified Intersection Count
	: Pedestrian Data





Di	recti	on	Pedestrians											
Tim	e Pe	riod	B to A	A to B	D to C	C to D	F to E	E to F	H to G	G to H				
7:00	to	7:15	0	6	1	4	3	2	0	0				
7:15	to	7:30	3	6	3	4	3	8	3	1				
7:30	to	7:45	1	5	2	3	6	6	4	0				
7:45	to	8:00	4	16	2	8	11	6	7	6				
8:00	to	8:15	2	6	2	8	4	14	1	4				
8:15 to 8:30		6	10	2	3	16	3	9	0					
8:30	to	8:45	2	2	1	7	5	5	4	1				
8:45	to	9:00	1	1	2	2	6	6	3	0				
AN	/I Tot	als	19	52	15	39	54	50	31	12				
15:00	to	15:15	7	1	2	1	3	3	1	0				
15:15	to	15:30	5	14	11	2	13	7	3	2				
15:30	to	15:45	7	4	9	2	10	7	4	1				
15:45	to	16:00	5	2	4	7	1	8	2	1				
16:00	to	16:15	10	4	3	11	5	7	3	5				
16:15	to	16:30	4	1	2	6	6	12	0	2				
16:30	to	16:45	7	5	5	3	5	9	5	4				
16:45	to	17:00	3	5	2	0	4	5	0	5				
PN	PM Totals		48	36	38	32	47	58	18	20				

Job No.	: N5401
Client	: Varga Traffic Planning
Suburb	: Dee Why
Location	: 2. 800 Pittwater Rd Driveway / Harbord Rd
Day/Date	: Tue, 29th October 2019
Weather	: Fine
Description	: Mid Block Count
	: 15 mins Data
	Class 1 Class 2

Heavies

Class 1 Classifications

_ _ _ _

Ар	proa	ach	800 Pittwater Rd Driveway									
Dir	ecti	on	w	estbour	nd	E	astboun	d				
Tim	e Pe	riod	Lights	Heavies	Total	Lights	Heavies	Total				
6:00	to	6:15	14	0	14	18	2	20				
6:15	to	6:30	21	2	23	14	0	14				
6:30	to	6:45	24	0	24	19	0	19				
6:45	to	7:00	50	0	50	15	0	15				
7:00	to	7:15	26	0	26	17	0	17				
7:15	to	7:30	25	0	25	17	0	17				
7:30	to	7:45	18	0	18	24	0	24				
7:45	to	8:00	22	0	22	26	0	26				
8:00	to	8:15	18	0	18	23	0	23				
8:15	to	8:30	20	0	20	19	0	19				
8:30	to	8:45	18	0	18	34	0	34				
8:45	to	9:00	22	0	22	58	0	58				
9:00	to	9:15	30	0	30	38	0	38				
9:15	to	9:30	29	0	29	53	0	53				
9:30	to	9:45	36	0	36	28	0	28				
9:45	to	10.00	25	0	25	20	0	20				
10:00	+0	10:00	20	0	20	32 4E	0	32				
10:00		10.15	25 EE	0	23	45	1	43				
10.15	to	10:30	33	0	33	40	-	47				
10.50	t0	11:00	94	0	44 25	20	0	20				
10.45		11.00	35	-	35	25	-	25				
11:00	to	11:15	41	1	42	30	1	31				
11:15	to	11:30	36	0	36	27	0	27				
11:30	to	11:45	35	0	35	20	0	20				
11:45	to	12:00	35	0	35	31	0	31				
12:00	to	12:15	39	0	39	27	0	27				
12:15	to	12:30	30	0	30	27	1	28				
12:30	to	12:45	33	0	33	31	0	31				
12:45	to	13:00	34	1	35	27	0	27				
13:00	to	13:15	29	1	30	18	2	20				
13:15	to	13:30	27	1	28	39	0	39				
13:30	to	13:45	36	0	36	23	0	23				
13:45	to	14:00	18	0	18	20	0	20				
14:00	to	14:15	27	0	27	21	0	21				
14:15	to	14:30	20	0	20	25	0	25				
14:30	to	14:45	19	0	19	29	0	29				
14:45	to	15:00	32	0	32	31	0	31				
15:00	to	15:15	24	0	24	39	0	39				
15:15	to	15:30	32	0	32	31	0	31				
15:30	to	15:45	30	0	30	33	1	34				
15:45	to	16:00	27	0	27	27	0	27				
16:00	to	16:15	24	0	24	43	0	43				
16:15	to	16:30	42	0	42	50	0	50				
16:30	to	16:45	38	0	38	46	0	46				
16:45	to	17:00	38	0	38	35	1	36				
17:00	to	17:15	32	2	34	54	0	54				
17:15	to	17:30	20	0	20	47	0	47				
17:30	to	17:45	33	0	33	47	0	47				
17:45	to	18:00	32	0	32	33	0	33				
				_	1 442	1 499		4 407				





Job No.	: N5401
Client	: Varga Traffic Planning
Suburb	: Dee Why
Location	: 2. 800 Pittwater Rd Driveway / Harbord Rd
Day/Date	: Tue, 29th October 2019
Weather	: Fine
Description	: Mid Block Count
	: Hourly Summary

Ар	proa	ach	800 Pittwater Rd Driveway									
Di	recti	on	N	/estbour	nd	E	astboun	d				
Tim	e Pe	riod	Lights	Heavies	<b>Fotal</b>	Lights	Heavies	<b>Fotal</b>				
6:00	to	7:00	109	2	111	66	2	68				
6:15	to	7:15	121	2	123	65	0	65				
6:30	to	7:30	125	0	125	68	0	68				
6:45	to	7:45	119	0	119	73	0	73				
7:00	to	8:00	91	0	91	84	0	84				
7:15	to	8:15	83	0	83	90	0	90				
7:30	to	8:30	78	0	78	92	0	92				
7:45	to	8:45	78	0	78	102	0	102				
8:00	to	9:00	78	0	78	134	0	134				
8:15	to	9:15	90	0	90	149	0	149				
8:30	to	9:30	99	0	99	183	0	183				
8:45	to	9:45	117	0	117	177	0	177				
9:00	to	10:00	130	0	130	151	0	151				
9:15	to	10:15	129	0	129	158	0	158				
9:30	to	10:30	155	0	155	151	1	152				
9:45	to	10:45	163	0	163	149	1	150				
10:00	to	11:00	163	0	163	142	1	143				
10:15	to	11:15	175	1	176	127	2	129				
10:30	to	11:30	156	1	157	108	1	109				
10:45	to	11:45	147	1	148	102	1	103				
11:00	to	12:00	147	1	148	108	1	109				
11:15	to	12:15	145	0	145	105	0	105				
11:30	to	12:30	139	0	139	105	1	106				
11:45	to	12:45	137	0	137	116	1	117				
12:00	to	13:00	136	1	137	112	1	113				
12:15	to	13:15	126	2	128	103	3	106				
12:30	to	13:30	123	3	126	115	2	117				
12:45	to	13:45	126	3	129	107	2	109				
13:00	to	14:00	110	2	112	100	2	102				
13:15	to	14:15	108	1	109	103	0	103				
13:30	to	14:30	101	0	101	89	0	89				
13:45	to	14:45	84	0	84	95	0	95				
14:00	to	15:00	98	0	98	106	0	106				
14:15	to	15:15	95	0	95	124	0	124				
14:30	to	15:30	107	0	107	130	0	130				
14:45	to	15:45	118	0	118	134	1	135				
15:00	to	16:00	113	0	113	130	1	131				
15:15	to	16:15	113	0	113	134	1	135				
15:30	to	16:30	123	0	123	153	1	154				
15:45	to	16:45	131	0	131	166	0	166				
16:00	to	17:00	142	0	142	174	1	175				
16:15	to	17:15	150	2	152	185	1	186				
16:30	to	17:30	128	2	130	182	1	183				
16:45	to	17:45	123	2	125	183	1	184				
17:00	to	18:00	117	2	119	181	0	181				
121	nr To	tals	1,434	8	1,442	1,488	9	1,497				





Annroach	Masthound	Fostbound -
		_
	: Peak Hour Summary	
Description	: Mid Block Count	
Weather	: Fine	
Day/Date	: Tue, 29th October 2019	
Location	: 2. 800 Pittwater Rd Drivewa	ay / Harbord Rd
Suburb	: Dee Why	
Client	: Varga Traffic Planning	
Job No.	: N5401	

	Ap	proa	ich	w	estbour	nd	E	d	otal	
	Tim	ie Pe	riod	Lights	Heavies	Total	Lights	Heavies	Total	Grand To
AM	9:45	to	10:45	163	0	163	149	1	150	313
PM	16:15	to	17:15	150	2	152	185	1	186	338

Ар	proa	ch	w	estbour	nd	E	Ital		
Tim	e Pei	riod	Lights	Heavies	Total	Lights	Heavies	Total	Grand To
6:00	to	7:00	109	2	111	66	2	68	179
6:15	to	7:15	121	2	123	65	0	65	188
6:30	to	7:30	125	0	125	68	0	68	193
6:45	to	7:45	119	0	119	73	0	73	192
7:00	to	8:00	91	0	91	84	0	84	175
7:15	to	8:15	83	0	83	90	0	90	173
7:30	to	8:30	78	0	78	92	0	92	170
7:45	to	8:45	78	0	78	102	0	102	180
8:00	to	9:00	78	0	78	134	0	134	212
8:15	to	9:15	90	0	90	149	0	149	239
8:30	to	9:30	99	0	99	183	0	183	282
8:45	to	9:45	117	0	117	177	0	177	294
9:00	to	10:00	130	0	130	151	0	151	281
9:15	to	10:15	129	0	129	158	0	158	287
9:30	to	10:30	155	0	155	151	1	152	307
9:45	to	10:45	163	0	163	149	1	150	313
10:00	to	11:00	163	0	163	142	1	143	306
10:15	to	11:15	175	1	176	127	2	129	305
10:30	to	11:30	156	1	157	108	1	109	266
10:45	to	11:45	147	1	148	102	1	103	251
11:00	to	12:00	147	1	148	108	1	109	257
11:15	to	12:15	145	- 0	145	105	0	105	250
11.30	to	12:30	139	0	139	105	1	105	245
11:45		12:00	137	0	137	116	- 1	117	254
12:00	to	12:45	136	1	137	117	1	112	254
12:00	to	13.00	130	-	137	102	-	106	230
12.15	to	12.20	120	2	120	105	3	100	234
12.50	10	13.30	125	3	120	107	2	100	243
12:45		13:45	120	د م	129	107	2	109	238
13:00	10	14:00	100	2	100	100	2	102	214
13:15	to	14:15	108	1	109	103	0	103	212
13:30	to	14:30	101	0	101	89	-	89	190
13:45	to	14:45	84	0	84	95	0	95	179
14:00	to	15:00	98	0	98	106	0	106	204
14:15	to	15:15	95	0	95	124	0	124	219
14:30	to	15:30	107	0	107	130	0	130	237
14:45	to	15:45	118	0	118	134	1	135	253
15:00	to	16:00	113	0	113	130	1	131	244
15:15	to	16:15	113	0	113	134	1	135	248
15:30	to	16:30	123	0	123	153	1	154	277
15:45	to	16:45	131	0	131	166	0	166	297
16:00	to	17:00	142	0	142	174	1	175	317
16:15	to	17:15	150	2	152	185	1	186	338
16:30	to	17:30	128	2	130	182	1	183	313
16:45	to	17:45	123	2	125	183	1	184	309
17:00	to	18:00	117	2	119	181	0	181	300
12h	nr Tot	als	1,434	8	1,442	1,488	9	1,497	2,939

800 Pittwater Rd Driveway











Approach				Harbo	ord Rd									Headla	and Rd					
Disection		D	Direction	2	D	Direction	3	Di	rection	3U	D	irection	4		0	irection	6	Di	rection 6	5U
Direction		(	(Through	)	(F	Right Tur	n)		(U Turn)		(	Left Turr	ו)		(F	Right Tur	n)		(U Turn)	1
Time Period		Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total		Lights	Heavies	Total	Lights	Heavies	Total
7:00 to 7:15		119	13	132	34	0	34	0	0	0	43	0	43		13	0	13	0	0	0
7:15 to 7:30		138	11	149	27	0	27	0	0	0	52	0	52		18	0	18	0	0	0
7:30 to 7:45		132	8	140	31	1	32	0	0	0	37	1	38		15	0	15	0	0	0
7:45 to 8:00		165	9	174	25	0	25	0	0	0	46	1	47		7	1	8	0	0	0
8:00 to 8:15		169	13	182	33	0	33	0	0	0	55	0	55		9	0	9	0	0	0
8:15 to 8:30		156	10	166	38	2	40	0	0	0	73	0	73		9	1	10	0	0	0
8:30 to 8:45		145	9	154	13	0	13	0	0	0	42	0	42		8	0	8	0	0	0
8:45 to 9:00		148	14	162	12	0	12	0	0	0	35	1	36		7	0	7	0	0	0
AM Totals		1,172	87	1,259	213	3	216	0	o	0	383	3	386		86	2	88	0	0	0
15:00 to 15:15		189	10	199	44	1	45	0	0	0	40	0	40		11	0	11	0	0	0
15:15 to 15:30		195	7	202	19	0	19	0	0	0	30	1	31		12	1	13	0	0	0
15:30 to 15:45		183	8	191	22	1	23	0	0	0	19	7	26		10	1	11	0	0	0
15:45 to 16:00		174	10	184	24	0	24	0	0	0	23	2	25		11	1	12	0	0	0
16:00 to 16:15	]	204	4	208	29	0	29	0	0	0	24	0	24	]	6	0	6	0	0	0
16:15 to 16:30	]	196	6	202	25	0	25	0	0	0	29	0	29	]	11	1	12	0	0	0
16:30 to 16:45		199	2	201	31	0	31	0	0	0	35	0	35		7	1	8	0	0	0
16:45 to 17:00		163	4	167	36	0	36	0	0	0	35	0	35		12	0	12	0	0	0
PM Totals		1.503	51	1.554	230	2	232	0	0	0	235	10	245	Ī	80	5	85	0	0	0

Approach		Harbord Rd								
Direction	C (	Direction Left Turi	ı7 n)	0	irection (Through	8 1)		Di	irection (U Turn)	θU
Time Period	Lights	Heavies	Total	Lights	Heavies	Total		Lights	Heavies	Total
7:00 to 7:15	56	2	58	158	5	163		0	0	0
7:15 to 7:30	36	3	39	197	10	207		0	0	0
7:30 to 7:45	44	3	47	200	9	209		0	0	0
7:45 to 8:00	42	2	44	208	4	212		0	0	0
8:00 to 8:15	49	6	55	195	5	200		0	0	0
8:15 to 8:30	49	2	51	183	13	196		0	0	0
8:30 to 8:45	40	1	41	195	9	204		0	0	0
8:45 to 9:00	46	0	46	168	14	182		0	0	0
AM Totals	362	19	381	1,504	69	1,573		0	0	0
15:00 to 15:15	57	3	60	152	7	159		0	0	0
15:15 to 15:30	43	4	47	170	10	180		0	0	0
15:30 to 15:45	34	3	37	149	5	154		0	0	0
15:45 to 16:00	36	2	38	185	14	199		0	0	0
16:00 to 16:15	28	0	28	153	7	160		0	0	0
16:15 to 16:30	42	1	43	207	5	212		0	0	0
16:30 to 16:45	53	0	53	198	8	206		0	0	0
16:45 to 17:00	51	0	51	225	3	228		0	0	0
PM Totals	344	13	357	1,439	59	1,498		0	0	0

Job No.	: N5401
Client	: Varga Traffic Planning
Suburb	: Dee Why
Location	: 3. Harbord Rd / Headland Rd
Day/Date	: Tue, 29th October 2019
Weather	: Fine
Description	: Classified Intersection Count
	: Hourly Summary





Approach			Harb	ord Rd						Headland Rd								
Direction		Direction (Through	12 1)	C (F	irection Right Tui	3 m)	D	irection (U Turn)	3U )	D (	irection Left Turr	4 1)		C (F	irection Right Tur	6 'n)		D
Time Period	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total		Lights	Heavies	Total		Lights
7:00 to 8:00	554	41	595	117	1	118	0	0	0	178	2	180		53	1	54		0
7:15 to 8:15	604	41	645	116	1	117	0	0	0	190	2	192		49	1	50		0
7:30 to 8:30	622	40	662	127	3	130	0	0	0	211	2	213		40	2	42		0
7:45 to 8:45	635	41	676	109	2	111	0	0	0	216	1	217		33	2	35		0
8:00 to 9:00	618	46	664	96	2	98	0	0	0	205	1	206		33	1	34		0
AM Totals	1,172	87	1,259	213	3	216	0	0	0	383	3	386		86	2	88		0
15:00 to 16:00	741	35	776	109	2	111	0	0	0	112	10	122		44	3	47		0
15:15 to 16:15	756	29	785	94	1	95	0	0	0	96	10	106		39	3	42		0
15:30 to 16:30	757	28	785	100	1	101	0	0	0	95	9	104		38	3	41		0
15:45 to 16:45	773	22	795	109	0	109	0	0	0	111	2	113		35	3	38		0
16:00 to 17:00	762	16	778	121	0	121	0	0	0	123	0	123	1	36	2	38		0
PM Totals	1,503	51	1,554	230	2	232	0	0	0	235	10	245	1	80	5	85		0

Approach		Harbord Rd								
Direction	Direction 7         Direction 8         Direction 9U           (Left Turn)         (Through)         (U Turn)						9U )			
Time Period	Lights	Heavies	Total	Lights	Heavies	Total		Lights	Heavies	Total
7:00 to 8:00	178	10	188	763	28	791		0	0	0
7:15 to 8:15	171	14	185	800	28	828		0	0	0
7:30 to 8:30	184	13	197	786	31	817		0	0	0
7:45 to 8:45	180	11	191	781	31	812		0	0	0
8:00 to 9:00	184	9	193	741	41	782		0	0	0
AM Totals	362	19	381	1,504	69	1,573		0	0	0
15:00 to 16:00	170	12	182	656	36	692		0	0	0
15:15 to 16:15	141	9	150	657	36	693		0	0	0
15:30 to 16:30	140	6	146	694	31	725		0	0	0
15:45 to 16:45	159	3	162	743	34	777		0	0	0
16:00 to 17:00	174	1	175	783	23	806		0	0	0
PM Totals	344	13	357	1,439	59	1,498		0	0	0







	Ap	oproa	ach	Н	arbord F	۲d	He	adland	Rd	н	arbord I	Rd	
	Tim	ne Pe	riod	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total	
АМ	7:30	to	8:30	749	43	792	251	4	255	970	44	1,014	
PM	16:00	to	17:00	883	16	899	159	2	161	957	24	981	

Ар	proa	ch	Ha	arbord F	۲d	He	adland	Rd	Н	arbord F	Rd
Tim	e Pe	riod	Lights	Heavies	Total	Lights	Heavies	Total	Lights	Heavies	Total
7:00	to	8:00	671	42	713	231	3	234	941	38	979
7:15	to	8:15	720	42	762	239	3	242	971	42	1,013
7:30	to	8:30	749	43	792	251	4	255	970	44	1,014
7:45	to	8:45	744	43	787	249	3	252	961	42	1,003
8:00	to	9:00	714	48	762	238	2	240	925	50	975
AN	1 Tot	als	1,385	90	1,475	469	5	474	1,866	88	1,954
15:00	to	16:00	850	37	887	156	13	169	826	48	874
15:15	to	16:15	850	30	880	135	13	148	798	45	843
15:30	to	16:30	857	29	886	133	12	145	834	37	871
15:45	to	16:45	882	22	904	146	5	151	902	37	939
16:00	to	17:00	883	16	899	159	2	161	957	24	981
PN	1 Tot	als	1,733	53	1,786	315	15	330	1,783	72	1,855



Job No.	: N5401									
Client	: Varga Traffic Planning									
Suburb	: Dee Why									
Location	: 4. 224 Headland Rd									
Day/Date	: Tue, 29th October 2019									
Weather	: Fine									
Description	: Mid Block Co	unt								
	: 15 mins Data									
	Class 1	Class 2								
Classifications	Lights	Heavies								





Job No. : N5401 Client : Varga Traffic Planning Suburb : Dee Why Location : 4. 224 Headland Rd Day/Date : Tue, 29th October 2019 Weather : Fine

Description

### : Mid Block Count : Hourly Summary

Ар	proa	ich	224 Headland Rd										
Dii	recti	on	No	orthbou	nd	So	uthbound						
Tim	e Pe	riod	Lights	Heavies	Total	Lights	Heavies	Total					
6:00	to	7:00	8	2	10	3	2	5					
6:15	to	7:15	21	2	23	17	2	19					
6:30	to	7:30	25	1	26	22	2	24					
6:45	to	7:45	26	0	26	24	1	25					
7:00	to	8:00	23	0	23	22	0	22					
7:15	to	8:15	13	0	13	10	0	10					
7:30	to	8:30	14	0	14	9	0	9					
7:45	to	8:45	16	0	16	11	0	11					
8:00	to	9:00	15	0	15	11	0	11					
8:15	to	9:15	14	0	14	9	0	9					
8:30	to	9:30	13	0	13	6	0	6					
8:45	to	9:45	12	0	12	3	0	3					
9:00	to	10:00	12	0	12	4	0	4					
9:15	to	10:15	10	0	10	5	0	5					
9:30	to	10:30	7	0	7	7	0	7					
9:45	to	10:45	6	0	6	8	0	8					
10:00	to	11:00	5	0	5	7	0	7					
10:15	to	11:15	6	0	6	7	0	7					
10.30	to	11:30			8	6		6					
10:45		11:45			7	6		6					
11:00		12:00	5	0	,	5	0	5					
11.00	+0	12:00			5	6		6					
11.15	to	12:15			2	4	0	4					
11.50	to	12.50	3		2	4	0	4					
12.00	**	12.45			,			-					
12.00		13.00	*		•			,					
12:15	to	13:15	4	0	4	,	0	,					
12:30	t0	13:30	4		4	9		9					
12:45	to	13:45	-	0	-	8	0	8					
13:00	to	14:00	/	0	7	6	0	6					
13:15	to	14:15	9	0	9	4	0	4					
13:30	to	14:30	12	0	12	3	0	3					
13:45	to	14:45	12	0	12	4	0	4					
14:00	to	15:00	10	0	10	8	0	8					
14:15	to	15:15	9	0	9	11	0	11					
14:30	to	15:30	7	0	7	16	0	16					
14:45	to	15:45	6	0	6	14	0	14					
15:00	to	16:00	6	0	6	10	0	10					
15:15	to	16:15	3	1	4	8	0	8					
15:30	to	16:30	3	1	4	3	1	4					
15:45	to	16:45	3	1	4	4	1	5					
16:00	to	17:00	2	1	3	6	1	7					
16:15	to	17:15	3	0	3	9	1	10					
16:30	to	17:30	1	0	1	8	0	8					
16:45	to	17:45	2	0	2	9	0	9					
17:00	to	18:00	3	0	3	6	0	6					
12	nr To	tals	100	3	103	95	3	98					




Job No.	: N5401
Client	: Varga Traffic Planning
Suburb	: Dee Why
Location	: 4. 224 Headland Rd
Day/Date	: Tue, 29th October 2019
Weather	: Fine
Description	: Mid Block Count
	: Peak Hour Summary

	Ар	proa	ich	No	orthbou	nd	So	otal		
	Tim	e Pe	riod	Lights	Heavies	Total	Lights	Heavies	Total	Grand To
AM	6:45	to	7:45	26	0	26	24	1	25	51
PM	14:30	to	15:30	7	0	7	16	0	16	23

Ар	proa	ch	No	orthbou	nd	So	uthbou	nd	otal	
Time	e Pe	riod	Lights	Heavies	Total	Lights	Heavies	Total	Grand To	
6:00	to	7:00	8	2	10	3	2	5	15	
6:15	to	7:15	21	2	23	17	2	19	42	
6:30	to	7:30	25	1	26	22	2	24	50	
6:45	to	7:45	26	0	26	24	1	25	51	
7:00	to	8:00	23	0	23	22	0	22	45	
7:15	to	8:15	13	0	13	10	0	10	23	
7:30	to	8:30	14	0	14	9	0	9	23	
7:45	to	8:45	16	0	16	11	0	11	27	
8:00	to	9:00	15	0	15	11	0	11	26	
8:15	to	9:15	14	0	14	9	0	9	23	
8:30	to	9:30	13	0	13	6	0	6	19	
8:45	to	9:45	12	0	12	3	0	3	15	
9:00	to	10:00	12	0	12	4	0	4	16	
9:15	to	10:15	10	0	10	5	0	5	15	
9:30	to	10:30	7	0	7	7	0	7	14	
9:45	to	10:45	6	-	6	8	-	8	14	
10.00	to	11:00	5	0	5	7	0	7	17	
10:15	to	11.00	۔ د	0	6	, 7	0	,	12	
10.13		11.15		0		, ,	0	,	13	
10:50	10	11:50		0	•	6	0	6	14	
10:45	to	11:45		0	-	6	0	6	13	
11:00	to	12:00	-	-	5	5	-	5	10	
11:15	to	12:15	5	0	5	6	0	6	11	
11:30	to	12:30	3	0	3	4	0	4	7	
11:45	to	12:45	3	0	3	4	0	4	7	
12:00	to	13:00	4	0	4	7	0	7	11	
12:15	to	13:15	4	0	4	7	0	7	11	
12:30	to	13:30	4	0	4	9	0	9	13	
12:45	to	13:45	5	0	5	8	0	8	13	
13:00	to	14:00	7	0	7	6	0	6	13	
13:15	to	14:15	9	0	9	4	0	4	13	
13:30	to	14:30	12	0	12	3	0	3	15	
13:45	to	14:45	12	0	12	4	0	4	16	
14:00	to	15:00	10	0	10	8	0	8	18	
14:15	to	15:15	9	0	9	11	0	11	20	
14:30	to	15:30	7	0	7	16	0	16	23	
14:45	to	15:45	6	0	6	14	0	14	20	
15:00	to	16:00	6	0	6	10	0	10	16	
15:15	to	16:15	3	1	4	8	0	8	12	
15:30	to	16:30	3	1	4	3	1	4	8	
15:45	to	16:45	3	1	4	4	1	5	9	
16:00	to	17:00	2	1	3	6	1	7	10	
16:15	to	17:15	3	0	3	9	1	10	13	
16:30	to	17:30	1	0	1	8	0	8	9	
16:45	to	17:45	2	0	2	9	0	9	11	
17:00	to	18:00	3	0	3	6	0	6	9	
12h	r To	tals	100	3	103	95	3	98	201	





Client Suburb Location	: N5401 : Varga Traffic Planning : Dee Why : 4. 224 Headland Rd									N	ΛΑ	ΓR		
Day/Date Weather Description	: Tue, 29th October 2019 : Fine : Mid Block Count										Trat	ific and	Transport Dat	a
••••	: Intersection Diagram	1							1					
Hour Sta	arting Vehicle Type Is  All Vehicles		<b>103</b>		Northb	ound				Ń				
			26 37% 7 21%	AM Peak PM Peak	(Vol) (%) (Vol) (%)					T				
			NB								224 Headland	Rd		
										AM Peak	6:45	to	7:45	
										PM Peak	14:30	to	15:30	
						AM Peak PM Peak	(Vol) (%) (Vol) (%)	SB 25 46% 16 36% 98 100%						

Job No.	: N5401					
Client	: Varga Traffic	Planning				
Suburb	: Dee Why					
Location	: 5. 222 Headland Rd					
Day/Date	: Tue, 29th October 2019					
Weather	: Fine					
Description	: Mid Block Co	unt				
	: 15 mins Data					
	Class 1	Class 2				
Classifications	Lights	Heavies				

Ар	proach 222 Headland Rd					adland Rd				
Di	recti	on	No	orthbou	nd	So	uthbou	nd		
Tim	e Pe	riod	Lights	Heavies	Total	Lights	Heavies	Total		
6:00	to	6:15	2	0	2	0	0	0		
6:15	to	6:30	0	0	0	0	0	0		
6:30	to	6:45	0	0	0	1	0	1		
6:45	to	7:00	1	0	1	3	0	3		
7:00	to	7:15	2	0	2	0	0	0		
7:15	to	7:30	1	0	1	2	0	2		
7:30	to	7:45	1	0	1	0	0	0		
7:45	to	8:00	4	0	4	2	0	2		
8:00	to	8:15	3	0	3	2	0	2		
8:15	to	8:30	2	0	2	0	0	0		
8:30	to	8:45	3	0	3	1	0	1		
8:45	to	9:00	0	0	0	1	0	1		
9:00	to	9:15	1	0	1	0	0	0		
9:15	to	9:30	3	0	3	1	0	1		
9:30	to	9:45	0	0	0	0	0	0		
9:45	to	10:00	3	0	3	0	0	0		
10:00	to	10:15	1	0	1	5	0	5		
10:15	to	10:30	0	0	0	0	0	0		
10:30	to	10:45	1	0	1	2	0	2		
10:45	to	11:00	4	0	4	2	0	2		
11:00	to	11:15	2	0	2	2	0	2		
11:15	to	11:30	0	0	0	2	0	2		
11:30	to	11:45	2	0	2	3	0	3		
11:45	to	12:00	2	1	3	0	0	0		
12:00	to	12:15	2	0	2	2	1	3		
12:15	to	12:30	3	0	3	2	0	2		
12:30	to	12:45	1	0	1	3	0	3		
12:45	to	13:00	1	0	1	2	0	2		
13:00	to	13:15	2	0	2	1	0	1		
13:15	to	13:30	1	0	1	1	0	1		
13:30	to	13:45	1	0	1	1	0	1		
13:45	to	14:00	2	0	2	0	0	0		
14:00	to	14:15	2	1	3	1	0	1		
14:15	to	14:30	0	1	1	1	2	3		
14:30	to	14:45	2	0	2	1	0	1		
14:45	to	15:00	2	0	2	1	0	1		
15:00	to	15:15	0	0	0	2	0	2		
15:15	to	15:30	3	0	3	2	0	2		
15:30	to	15:45	1	0	1	3	0	3		
15:45	to	16:00	2	0	2	1	0	1		
16:00	to	16:15	0	0	0	4	0	4		
16:15	to	16:30	0	0	0	1	0	1		
16:30	to	16:45	1	0	1	0	0	0		
16:45	to	17:00	2	0	2	0	0	0		
17:00	to	17:15	3	0	3	4	0	4		
17:15	to	17:30	1	0	1	2	0	2		
17:30	to	17:45	3	0	3	3	0	3		
17:45	to	18:00	2	0	2	1	0	1		
12ŀ	nr Tot	als	75	3	78	68	3	71		





Job No. Client	: N5401 : Varga Traffic Planning : Doo Why
Location	: 5. 222 Headland Rd
Day/Date	: Tue, 29th October 2019
Weather	: Fine
Description	: Mid Block Count
	: Hourly Summary

Ар	proa	ich	222 Headland Rd							
Diı	ecti	on	No	orthbou	nd	So	uthbou	nd		
Tim	- D	riad	ghts	eavies	otal	ghts	eavies	otal		
6:00	to	7:00		<u>т</u>	.≓ 3	4	H O	- - 4		
6:15	to	7:15	3	0	3	4	0	4		
6:30		7:30	4		4	-	0	-		
6:4E	*0	7:45	-		-			-		
7:00		8.00		0	•	4		3		
7.15		0.15	0		•	4 6		4		
7:20		0.15	10		10	4		4		
7.45		0.30	10	0	10	-	0	•		
0.00		0.00		0	12	3		3		
8:00	to	9:00	8	0	8	4	0	4		
8:15	to	9:15			6	2	0	2		
8:30	to	9:30	7	0	7	3	U	3		
8:45	to	9:45	4	0	4	2	0	2		
9:00	to	10:00	7	0	7	1	0	1		
9:15	to	10:15	7	0	7	6	0	6		
9:30	to	10:30	4	0	4	5	0	5		
9:45	to	10:45	5	0	5	7	0	7		
10:00	to	11:00	6	0	6	9	0	9		
10:15	to	11:15	7	0	7	6	0	6		
10:30	to	11:30	7	0	7	8	0	8		
10:45	to	11:45	8	0	8	9	0	9		
11:00	to	12:00	6	1	7	7	0	7		
11:15	to	12:15	6	1	7	7	1	8		
11:30	to	12:30	9	1	10	7	1	8		
11:45	to	12:45	8	1	9	7	1	8		
12:00	to	13:00	7	0	7	9	1	10		
12:15	to	13:15	7	0	7	8	0	8		
12:30	to	13:30	5	0	5	7	0	7		
12:45	to	13:45	5	0	5	5	0	5		
13:00	to	14:00	6	0	6	3	0	3		
13:15	to	14:15	6	1	7	3	0	3		
13:30	to	14:30	5	2	7	3	2	5		
13:45	to	14:45	6	2	8	3	2	5		
14:00	to	15:00	6	2	8	4	2	6		
14:15	to	15:15	4	1	5	5	2	7		
14:30	to	15:30	7	0	7	6	0	6		
14:45	to	15:45	6	0	6	8	0	8		
15:00	to	16:00	6	0	6	8	0	8		
15:15	to	16:15	- 6	0	-	10	0	10		
15:30	to	16:30	3	0	3	9	0	9		
15:45		16:45	3	0	3	6	0	6		
16:00	to	17:00	2		,	6		5		
10:00	t0	17:00	3	0	3	-				
16:15	to	17:15	6	0	6	5	0	5		
16:30	to	17:30	7	0	7	6	U	6		
16:45	to	17:45	9	0	9	9	0	9		
17:00	to	18:00	9	0	9	10	0	10		
12h	r To	tals	75	3	78	68	3	71		





Weather	: Fine
Description	: Mid Block Count

: Peak Hour Summary

	Approach			No	Northbound			Southbound			
	Tim	e Pe	riod	Lights	Heavies	Total	Lights Heavies Total			Grand To	
AM	7:45	to	8:45	12	0	12	5	0	5	17	
PM	17:00	to	18:00	9	0	9	10	0	10	19	

Approach			No	orthbou	nd	So	uthbou	nd	otal	
Tim	e Pei	riod	ights	leavies	<b>Fotal</b>	ights	leavies	<b>Fotal</b>	Grand To	
6:00	to	7:00	3	0	3	4	0	4	7	
6:15	to	7:15	3	0	3	4	0	4	7	
6:30	to	7:30	4	0	4	6	0	6	10	
6:45	to	7:45	5	0	5	5	0	5	10	
7:00	to	8:00	8	0	8	4	0	4	12	
7:15	to	8:15	9	0	9	6	0	6	15	
7:30	to	8:30	10	0	10	4	0	4	14	
7:45	to	8:45	12	0	12	5	0	5	17	
8:00	to	9:00	8	0	8	4	0	4	12	
8:15	to	9:15	6	0	6	2	0	2	8	
8:30	to	9:30	7	0	7	3	0	3	10	
8:45	to	9:45	4	0	4	2	0	2	6	
9:00	to	10:00	7	0	7	1	0	1	8	
9:15	to	10:15	7	0	7	6	0	6	13	
9:30	to	10:30	4	0	4	5	0	5	9	
9:45	to	10:45	5	0	5	7	0	7	12	
10:00	to	11:00	6	0	6	9	0	9	15	
10:15	to	11:15	7	0	7	6	0	6	13	
10:30	to	11:30	7	- 0	7	- 8	0	8	15	
10:45	to	11:45	8	- 0	8	9	0	9	17	
11:00	to	12:00	6		7	7	0	7	14	
11:15	to	12:15	6	- 1	7	7	- 1	8	15	
11.30		12:20	0	- 1	10	7	1	8	18	
11:45	to	12:30	8	1	- 10	7	1		17	
12:00		12:45	0 7	-	7	,	1	10	17	
12.00	to	13.00	, ,	0	7	•	-		17	
12.15	to	13.13	, E	0	,	~ ~	0	~ ~	13	
12:50		13:50	5	0	-		0	,	12	
12:45		13:45			3	د م		2	10	
13:00	10	14:00	6	1	-	2	0	3	9 10	
13:15	to	14:15	6	1	-	3	0	3	10	
13:30	to	14:30	5	2	/	3	2	5	12	
13:45	to	14:45	6	2	8	3	2	5	13	
14:00	to	15:00	6	2	8	4	2	6	14	
14:15	to	15:15	4	1	5	5	2	7	12	
14:30	to	15:30	7	0	7	6	0	6	13	
14:45	to	15:45	6	0	6	8	0	8	14	
15:00	to	16:00	6	0	6	8	0	8	14	
15:15	to	16:15	6	0	6	10	0	10	16	
15:30	to	16:30	3	0	3	9	0	9	12	
15:45	to	16:45	3	0	3	6	0	6	9	
16:00	to	17:00	3	0	3	5	0	5	8	
16:15	to	17:15	6	0	6	5	0	5	11	
16:30	to	17:30	7	0	7	6	0	6	13	
16:45	to	17:45	9	0	9	9	0	9	18	
17:00	to	18:00	9	0	9	10	0	10	19	
121	nr Tot	tals	75	3	78	68	3	71	149	





Job No. Client Suburb Location	: N5401 : Varga Traffic Planning : Dee Why : 5. 222 Headland Rd						N	ΛΑ	ΓR		
Day/Date Weather Description Hour Sta	: Tue, 29th October 2019 : Fine : Mid Block Count : Intersection Diagram			Northbound				——— Trai	fic and	Transport Da	ta
12hr Total	s	78 100% 12 31% 9 23%	AM Peak PM Peak	(Vol) (%) (Vol) (%)			Ņ	222 Headland	Rd		
		NB				SB	AM Peak PM Peak	7:45 17:00	to to	8:45 18:00	
				AM Peak PM Peak	(Vol) (%) (Vol) (%)	5 17% 10 24%					
				Southbound		<b>71</b> 100%					

## **APPENDIX D**

## SIDRA CAPACITY ANALYSIS RESULTS

## SITE LAYOUT

#### Site: 101 [Existing AM]

PIT_HARX Site Category: (None) Signals - Fixed Time Isolated



SIDRA INTERSECTION 8.0 | Copyright © 2000-2019 Akcelik and Associates Pty Ltd | sidrasolutions.com Organisation: VARGA TRAFFIC PLANNING | Created: Thursday, 9 April 2020 1:31:22 PM Project: Z:\DATA\Data\Jobs01\Jobs\19work\19090_800PittwaterRdDeeWhy\SIDRA\SIDRA 200409\PIT_HARX.sip8

## Site: 101 [Existing AM]

PIT_HARX

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Site User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Move	lovement Performance - Vehicles lov Turn Demand Flows Deg. Average Level of 95% Back of Queue Prop. Effective Aver. No. Average												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South	Harbo	ord Road (S)											
1b	L3	27	18.5	0.761	85.6	LOS F	16.8	124.4	1.00	0.92	1.54	25.1	
1	L2	407	5.2	0.761	78.0	LOS F	16.8	124.4	0.99	0.89	1.31	26.2	
3a	R1	266	6.0	0.572	72.1	LOS F	9.4	68.9	0.99	0.80	0.99	27.4	
Appro	ach	700	6.0	0.761	76.0	LOS F	16.8	124.4	0.99	0.86	1.20	26.6	
North	East: P	ittwater Road	l (NE)										
24a	L1	476	3.6	0.613	19.2	LOS B	17.0	122.9	0.54	0.72	0.54	44.8	
25	T1	1625	7.0	0.803	35.5	LOS C	42.5	315.4	0.85	0.78	0.87	38.0	
26a	R1	308	1.0	0.529	67.6	LOS E	10.5	74.2	0.97	0.80	0.97	28.9	
Appro	ach	2409	5.5	0.803	36.4	LOS C	42.5	315.4	0.81	0.77	0.82	37.6	
West:	Warrin	igah Road (V	√)										
10a	L1	86	8.1	0.130	37.7	LOS C	4.1	30.8	0.70	0.70	0.70	36.9	
12	R2	485	3.5	0.793	73.8	LOS F	18.4	132.9	1.00	0.89	1.11	27.1	
12b	R3	307	6.8	0.779	80.8	LOS F	11.7	86.5	1.00	0.88	1.15	25.8	
Appro	ach	878	5.1	0.793	72.7	LOS F	18.4	132.9	0.97	0.87	1.08	27.3	
South	West: F	Pittwater Roa	id (SW)										
30b	L3	84	39.3	0.065	7.2	LOS A	0.0	0.0	0.00	0.55	0.00	53.4	
31	T1	1001	9.4	0.516	36.8	LOS C	20.3	153.4	0.81	0.70	0.81	37.5	
32b	R3	100	7.0	0.804	87.9	LOS F	7.9	58.6	1.00	0.89	1.24	24.5	
Appro	ach	1185	11.3	0.804	39.0	LOS C	20.3	153.4	0.77	0.71	0.79	36.6	
All Vel	nicles	5172	6.8	0.804	48.5	LOS D	42.5	315.4	0.85	0.78	0.91	33.4	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedestria	ins						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of . Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P6	NorthEast Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P6B	NorthEast Slip/Bypass Lane Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P4	West Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P4B	West Slip/Bypass Lane Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P8	SouthWest Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
All Peo	All Pedestrians		69.3	LOS F			0.96	0.96

## Site: 101 [Existing PM]

PIT_HARX

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Site User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Move	<b>Novement Performance - Vehicles</b> Mov Turn Demand Flows Deg. Average Level of 95% Back of Queue Prop. Effective Aver. No. Average												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South	: Harbo	ord Road (S)											
1b	L3	36	8.3	0.788	77.7	LOS F	20.4	147.9	1.00	0.93	1.44	26.6	
1	L2	470	3.2	0.788	73.1	LOS F	20.4	147.9	0.98	0.90	1.27	27.2	
3a	R1	365	3.0	0.770	76.8	LOS F	13.6	97.8	1.00	0.88	1.11	26.5	
Appro	ach	871	3.3	0.788	74.8	LOS F	20.4	147.9	0.99	0.89	1.21	26.9	
North	East: P	ittwater Road	d (NE)										
24a	L1	377	5.0	0.379	17.0	LOS B	11.2	81.5	0.62	0.73	0.62	46.0	
25	T1	1024	7.7	0.743	38.1	LOS C	34.9	260.3	0.88	0.78	0.88	37.0	
26a	R1	349	2.6	0.774	77.3	LOS F	13.1	93.7	1.00	0.89	1.12	26.8	
Appro	ach	1750	6.1	0.774	41.4	LOS C	34.9	260.3	0.85	0.79	0.87	35.8	
West:	Warrin	gah Road (V	V)										
10a	L1	193	4.1	0.318	44.6	LOS D	10.5	76.0	0.80	0.76	0.80	34.5	
12	R2	400	4.5	0.667	69.6	LOS E	13.9	101.2	0.99	0.83	0.99	27.9	
12b	R3	227	12.3	0.808	86.8	LOS F	8.9	69.2	1.00	0.90	1.23	24.8	
Appro	ach	820	6.6	0.808	68.5	LOS E	13.9	101.2	0.95	0.83	1.01	28.2	
South	West: F	Pittwater Roa	ad (SW)										
30b	L3	128	34.4	0.096	7.2	LOS A	0.0	0.0	0.00	0.55	0.00	53.6	
31	T1	1490	5.4	0.759	35.6	LOS C	31.2	228.4	0.84	0.75	0.84	37.9	
32b	R3	110	7.3	0.760	48.8	LOS D	4.9	36.2	1.00	0.85	1.16	33.1	
Appro	ach	1728	7.7	0.760	34.4	LOS C	31.2	228.4	0.79	0.74	0.80	38.4	
All Ve	hicles	5169	6.2	0.808	49.0	LOS D	34.9	260.3	0.87	0.80	0.93	33.3	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedestria	ins						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of . Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P6	NorthEast Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P6B	NorthEast Slip/Bypass Lane Crossing	53	32.1	LOS D	0.1	0.1	0.92	0.92
P4	West Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P4B	West Slip/Bypass Lane Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P8	SouthWest Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
All Peo	All Pedestrians		63.1	LOS F			0.96	0.96

## Site: 101 [Proposed AM]

PIT_HARP

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Site User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Move	<mark>Novement Performance - Vehicles</mark> Mov Turn Demand Flows Deg. Average Level of 95% Back of Queue Prop. Effective Aver. No. Average												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South	: Harbo	rd Road (S)											
1b	L3	30	16.7	0.696	84.4	LOS F	16.0	118.0	0.99	0.89	1.52	25.3	
1	L2	412	5.1	0.696	75.1	LOS F	16.0	118.0	0.98	0.86	1.25	26.8	
3a	R1	271	5.9	0.651	75.0	LOS F	9.8	72.0	1.00	0.82	1.02	26.8	
Appro	ach	713	5.9	0.696	75.5	LOS F	16.0	118.0	0.99	0.85	1.18	26.7	
North	East: P	ittwater Road	l (NE)										
24a	L1	554	3.1	0.732	23.6	LOS B	23.3	167.2	0.63	0.76	0.63	42.5	
25	T1	1625	7.0	0.872	47.4	LOS D	50.1	371.2	0.90	0.89	1.01	33.8	
26a	R1	308	1.0	0.760	78.4	LOS F	11.6	81.7	1.00	0.87	1.12	26.7	
Appro	ach	2487	5.3	0.872	46.0	LOS D	50.1	371.2	0.86	0.86	0.94	34.2	
West:	Warrin	gah Road (V	√)										
10a	L1	86	8.1	0.143	41.4	LOS C	4.3	32.5	0.74	0.71	0.74	35.6	
12	R2	563	3.0	0.863	78.4	LOS F	22.9	164.5	1.00	0.94	1.19	26.2	
12b	R3	307	6.8	0.871	89.5	LOS F	12.5	92.5	1.00	0.95	1.31	24.4	
Appro	ach	956	4.7	0.871	78.6	LOS F	22.9	164.5	0.98	0.92	1.19	26.2	
South	West: F	Pittwater Roa	id (SW)										
30b	L3	84	39.3	0.065	7.2	LOS A	0.0	0.0	0.00	0.55	0.00	53.4	
31	T1	1001	9.4	0.459	31.3	LOS C	18.7	141.0	0.75	0.65	0.75	39.7	
32b	R3	141	5.0	0.840	86.9	LOS F	11.2	81.8	1.00	0.92	1.26	24.7	
Appro	ach	1226	10.9	0.840	36.1	LOS C	18.7	141.0	0.72	0.67	0.75	37.7	
All Vel	hicles	5382	6.6	0.872	53.4	LOS D	50.1	371.2	0.86	0.83	0.97	32.0	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedestria	ins						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of . Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P6	NorthEast Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P6B	NorthEast Slip/Bypass Lane Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P4	West Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P4B	West Slip/Bypass Lane Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P8	SouthWest Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
All Peo	All Pedestrians		69.3	LOS F			0.96	0.96

## Site: 101 [Proposed PM]

PIT_HARP

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Site User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Move	<b>Novement Performance - Vehicles</b> Nov Turn Demand Flows Deg. Average Level of 95% Back of Queue Prop. Effective Aver. No. Average												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
South	: Harbo	ord Road (S)											
1b	L3	43	7.0	0.799	78.1	LOS F	21.5	155.4	1.00	0.94	1.46	26.5	
1	L2	482	3.1	0.799	73.2	LOS F	21.5	155.4	0.98	0.91	1.29	27.2	
3a	R1	378	2.9	0.757	75.4	LOS F	14.0	100.2	1.00	0.87	1.09	26.7	
Appro	ach	903	3.2	0.799	74.4	LOS F	21.5	155.4	0.99	0.90	1.21	27.0	
North	East: P	ittwater Road	d (NE)										
24a	L1	403	4.7	0.409	17.7	LOS B	12.4	90.3	0.64	0.74	0.64	45.6	
25	T1	1024	7.7	0.766	40.0	LOS C	35.7	265.9	0.90	0.80	0.90	36.3	
26a	R1	349	2.6	0.774	77.3	LOS F	13.1	93.7	1.00	0.89	1.12	26.8	
Appro	ach	1776	6.0	0.774	42.3	LOS C	35.7	265.9	0.86	0.81	0.88	35.4	
West:	Warrin	gah Road (V	V)										
10a	L1	193	4.1	0.318	44.6	LOS D	10.5	76.0	0.80	0.76	0.80	34.5	
12	R2	425	4.2	0.713	71.0	LOS F	15.2	110.4	1.00	0.85	1.03	27.6	
12b	R3	227	12.3	0.808	86.8	LOS F	8.9	69.2	1.00	0.90	1.23	24.8	
Appro	ach	845	6.4	0.808	69.2	LOS E	15.2	110.4	0.95	0.84	1.03	28.0	
South	West: F	Pittwater Roa	ad (SW)										
30b	L3	128	34.4	0.096	7.2	LOS A	0.0	0.0	0.00	0.55	0.00	53.6	
31	T1	1490	5.4	0.775	36.8	LOS C	31.3	229.3	0.85	0.76	0.85	37.5	
32b	R3	122	6.6	0.783	49.1	LOS D	5.4	39.8	1.00	0.86	1.18	33.0	
Appro	ach	1740	7.6	0.783	35.4	LOS C	31.3	229.3	0.80	0.75	0.81	38.0	
All Ve	hicles	5264	6.1	0.808	49.9	LOS D	35.7	265.9	0.88	0.81	0.94	33.0	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedestria	ins						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of . Service	Average Bacł Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P6	NorthEast Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P6B	NorthEast Slip/Bypass Lane Crossing	53	32.0	LOS D	0.1	0.1	0.92	0.92
P4	West Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P4B	West Slip/Bypass Lane Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P8	SouthWest Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
All Peo	All Pedestrians		63.1	LOS F			0.96	0.96

## SITE LAYOUT

# Site: 101 [Existing AM]

HAR_SITE Site Category: (None) Stop (Two-Way)



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#### Site: 101 [Existing AM]

HAR_SITE Site Category: (None) Stop (Two-Way)

Move	Movement Performance - Vehicles												
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
East: S	SITE AC	CESS (NO.8	300)										
4	L2	78	0.0	0.092	10.5	LOS A	0.4	2.5	0.50	0.92	0.50	50.7	
Approa	ach	78	0.0	0.092	10.5	LOS A	0.4	2.5	0.50	0.92	0.50	50.7	
North:	Harbord	l Road (N)											
7	L2	92	0.0	0.297	3.9	LOS A	0.0	0.0	0.00	0.09	0.00	56.1	
8	T1	1061	0.0	0.297	0.0	LOS A	0.0	0.0	0.00	0.04	0.00	59.4	
Appro	ach	1153	0.0	0.297	0.3	NA	0.0	0.0	0.00	0.05	0.00	59.1	
All Vel	nicles	1231	0.0	0.297	1.0	NA	0.4	2.5	0.03	0.10	0.03	58.1	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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#### Site: 101 [Existing PM]

HAR_SITE Site Category: (None) Stop (Two-Way)

Move	Movement Performance - Vehicles												
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
East: \$	SITE AC	CESS (NO.8	800)										
4	L2	113	0.0	0.118	9.9	LOS A	0.5	3.3	0.46	0.90	0.46	51.1	
Approa	ach	113	0.0	0.118	9.9	LOS A	0.5	3.3	0.46	0.90	0.46	51.1	
North:	Harbord	d Road (N)											
7	L2	131	0.0	0.270	3.9	LOS A	0.0	0.0	0.00	0.15	0.00	55.5	
8	T1	916	0.0	0.270	0.0	LOS A	0.0	0.0	0.00	0.06	0.00	59.2	
Appro	ach	1047	0.0	0.270	0.5	NA	0.0	0.0	0.00	0.07	0.00	58.7	
All Vel	nicles	1160	0.0	0.270	1.4	NA	0.5	3.3	0.04	0.15	0.04	57.3	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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### Site: 101 [Proposed AM]

HAR_SITE Site Category: (None) Stop (Two-Way)

Move	Movement Performance - Vehicles												
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h	
East: \$	SITE AC	CESS (NO.	800)										
4	L2	181	0.0	0.191	10.1	LOS A	0.8	5.6	0.48	0.92	0.48	51.0	
Appro	ach	181	0.0	0.191	10.1	LOS A	0.8	5.6	0.48	0.92	0.48	51.0	
North:	Harbord	d Road (N)											
7	L2	255	0.0	0.341	3.9	LOS A	0.0	0.0	0.00	0.22	0.00	54.7	
8	T1	1061	0.0	0.341	0.0	LOS A	0.0	0.0	0.00	0.08	0.00	58.9	
Appro	ach	1316	0.0	0.341	0.8	NA	0.0	0.0	0.00	0.11	0.00	58.0	
All Vel	hicles	1497	0.0	0.341	1.9	NA	0.8	5.6	0.06	0.21	0.06	56.5	

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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#### Site: 101 [Proposed PM]

HAR_SITE Site Category: (None) Stop (Two-Way)

Move	ment P	erformanc	e - Vel	hicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: SITE ACCESS (NO.800)												
4	L2	225	0.0	0.228	9.9	LOS A	1.0	6.9	0.47	0.91	0.47	51.1
Approach		225	0.0	0.228	9.9	LOS A	1.0	6.9	0.47	0.91	0.47	51.1
North:	Harbord	d Road (N)										
7	L2	183	0.0	0.284	3.9	LOS A	0.0	0.0	0.00	0.19	0.00	55.0
8	T1	916	0.0	0.284	0.0	LOS A	0.0	0.0	0.00	0.08	0.00	59.0
Appro	ach	1099	0.0	0.284	0.7	NA	0.0	0.0	0.00	0.10	0.00	58.3
All Vel	hicles	1324	0.0	0.284	2.2	NA	1.0	6.9	0.08	0.23	0.08	56.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## SITE LAYOUT

# Site: 102 [Existing AM]

HAR_HEAX Site Category: (None) Stop (Two-Way)



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#### Site: 102 [Existing AM]

HAR_HEAX Site Category: (None) Stop (Two-Way)

Move	ment Pe	rformanc	e - Veł	nicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	Harbord	Road (S)										
2	T1	662	6.0	0.306	1.1	LOS A	1.8	12.8	0.10	0.07	0.12	38.8
3	R2	130	2.3	0.306	11.9	LOS A	1.8	12.8	0.75	0.56	0.89	34.6
Approa	ach	792	5.4	0.306	2.9	NA	1.8	12.8	0.21	0.15	0.25	37.7
East: I	Headland	Road (E)										
4	L2	213	0.9	0.905	61.9	LOS E	14.5	102.9	0.75	2.31	3.39	19.2
6	R2	42	4.8	0.905	137.6	LOS F	14.5	102.9	0.75	2.31	3.39	19.4
Approa	ach	255	1.6	0.905	74.4	LOS F	14.5	102.9	0.75	2.31	3.39	19.3
North:	Harbord	Road (N)										
7	L2	197	6.6	0.270	3.5	LOS A	0.0	0.0	0.00	0.18	0.00	39.4
8	T1	817	3.8	0.270	0.0	LOS A	0.0	0.0	0.00	0.07	0.00	39.5
Approa	ach	1014	4.3	0.270	0.7	NA	0.0	0.0	0.00	0.09	0.00	39.5
All Vel	nicles	2061	4.4	0.905	10.6	NA	14.5	102.9	0.17	0.39	0.51	33.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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#### Site: 102 [Existing PM]

HAR_HEAX Site Category: (None) Stop (Two-Way)

Move	ment Pe	erformanc	e - Vel	nicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	Harbord	l Road (S)										
2	T1	785	3.7	0.284	1.2	LOS A	1.7	12.3	0.16	0.07	0.18	38.5
3	R2	95	1.1	0.284	9.7	LOS A	1.7	12.3	0.51	0.22	0.59	36.9
Approa	ach	880	3.4	0.284	2.1	NA	1.7	12.3	0.20	0.09	0.23	38.2
East: I	Headland	Road (E)										
4	L2	106	9.4	0.742	40.1	LOS C	5.7	42.7	0.72	1.49	1.89	21.8
6	R2	42	7.1	0.742	103.0	LOS F	5.7	42.7	0.72	1.49	1.89	22.0
Approa	ach	148	8.8	0.742	57.9	LOS E	5.7	42.7	0.72	1.49	1.89	21.9
North:	Harbord	Road (N)										
7	L2	150	6.0	0.226	3.5	LOS A	0.0	0.0	0.00	0.17	0.00	39.4
8	T1	693	5.2	0.226	0.0	LOS A	0.0	0.0	0.00	0.06	0.00	39.6
Approa	ach	843	5.3	0.226	0.6	NA	0.0	0.0	0.00	0.08	0.00	39.5
All Vel	nicles	1871	4.7	0.742	5.9	NA	5.7	42.7	0.15	0.20	0.26	35.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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#### Site: 102 [Proposed AM]

HAR_HEAP Site Category: (None) Stop (Two-Way)

Move	ment Po	erformanc	e - Vel	nicles								
Mov ID	Turn	Demand I Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	: Harbord	Road (S)										
2	T1	662	6.0	0.343	0.3	LOS A	1.6	11.2	0.02	0.02	0.03	39.6
3	R2	143	2.1	0.343	14.3	LOS A	1.6	11.2	0.81	0.86	0.99	33.1
Appro	ach	805	5.3	0.343	2.8	NA	1.6	11.2	0.16	0.17	0.20	37.8
East: I	Headland	d Road (E)										
4	L2	218	0.9	1.381	392.1	LOS F	59.2	419.8	1.00	6.06	10.97	5.6
6	R2	55	3.6	1.381	479.2	LOS F	59.2	419.8	1.00	6.06	10.97	5.7
Approa	ach	273	1.5	1.381	409.7	LOS F	59.2	419.8	1.00	6.06	10.97	5.6
North:	Harbord	Road (N)										
7	L2	231	5.6	0.306	3.5	LOS A	0.0	0.0	0.00	0.19	0.00	39.4
8	T1	920	3.4	0.306	0.0	LOS A	0.0	0.0	0.00	0.07	0.00	39.5
Approa	ach	1151	3.8	0.306	0.7	NA	0.0	0.0	0.00	0.09	0.00	39.5
All Vel	hicles	2229	4.1	1.381	51.6	NA	59.2	419.8	0.18	0.85	1.42	19.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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#### Site: 102 [Proposed PM]

HAR_HEAP Site Category: (None) Stop (Two-Way)

Move	ment P	erformanc	e - Vel	nicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	: Harbord	d Road (S)										
2	T1	785	3.7	0.298	1.5	LOS A	2.0	14.5	0.17	0.07	0.20	38.2
3	R2	99	1.0	0.298	11.3	LOS A	2.0	14.5	0.62	0.27	0.73	36.0
Appro	ach	884	3.4	0.298	2.6	NA	2.0	14.5	0.22	0.10	0.26	37.8
East: I	Headland	d Road (E)										
4	L2	118	8.5	1.431	442.2	LOS F	45.0	333.6	1.00	4.92	9.55	5.0
6	R2	74	4.1	1.431	502.8	LOS F	45.0	333.6	1.00	4.92	9.55	5.1
Appro	ach	192	6.8	1.431	465.5	LOS F	45.0	333.6	1.00	4.92	9.55	5.0
North:	Harbord	Road (N)										
7	L2	161	5.6	0.257	3.5	LOS A	0.0	0.0	0.00	0.16	0.00	39.5
8	T1	805	4.5	0.257	0.0	LOS A	0.0	0.0	0.00	0.06	0.00	39.6
Appro	ach	966	4.7	0.257	0.6	NA	0.0	0.0	0.00	0.08	0.00	39.6
All Vel	hicles	2042	4.3	1.431	45.2	NA	45.0	333.6	0.19	0.54	1.01	20.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## SITE LAYOUT

#### Site: 101 [Existing AM]

HEA_SITE (No.224) Site Category: (None) Stop (Two-Way)



Headland Road (E)

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#### Site: 101 [Existing AM]

HEA_SITE (No.224) Site Category: (None) Stop (Two-Way)

Move	ment P	erformanc	e - Vel	nicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East:	Headland	d Road (E)										
5	T1	14	0.0	0.011	0.0	LOS A	0.0	0.2	0.04	0.16	0.04	39.2
6	R2	7	0.0	0.011	3.6	LOS A	0.0	0.2	0.04	0.16	0.04	39.2
Appro	ach	21	0.0	0.011	1.2	NA	0.0	0.2	0.04	0.16	0.04	39.2
North:	SITE AC	CCESS (No.	.224)									
7	L2	7	0.0	0.007	6.7	LOS A	0.0	0.2	0.04	0.96	0.04	37.4
9	R2	2	0.0	0.007	6.4	LOS A	0.0	0.2	0.04	0.96	0.04	36.3
Appro	ach	9	0.0	0.007	6.6	LOS A	0.0	0.2	0.04	0.96	0.04	37.2
West:	Headlan	d Road (W)	1									
10	L2	7	0.0	0.008	3.4	LOS A	0.0	0.0	0.00	0.20	0.00	39.3
11	T1	9	0.0	0.008	0.0	LOS A	0.0	0.0	0.00	0.20	0.00	39.1
Appro	ach	16	0.0	0.008	1.5	NA	0.0	0.0	0.00	0.20	0.00	39.2
All Vel	hicles	46	0.0	0.011	2.4	NA	0.0	0.2	0.03	0.33	0.03	38.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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#### Site: 101 [Existing PM]

HEA_SITE (No.224) Site Category: (None) Stop (Two-Way)

Move	ment P	erformanc	e - Vel	nicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East:	Headlan	d Road (E)										
5	T1	6	0.0	0.005	0.0	LOS A	0.0	0.1	0.04	0.16	0.04	39.2
6	R2	3	0.0	0.005	3.6	LOS A	0.0	0.1	0.04	0.16	0.04	39.2
Appro	ach	9	0.0	0.005	1.2	NA	0.0	0.1	0.04	0.16	0.04	39.2
North:	SITE AC	CCESS (No.	.224)									
7	L2	6	0.0	0.008	6.7	LOS A	0.0	0.2	0.05	0.96	0.05	37.5
9	R2	4	0.0	0.008	6.4	LOS A	0.0	0.2	0.05	0.96	0.05	36.3
Appro	ach	10	0.0	0.008	6.6	LOS A	0.0	0.2	0.05	0.96	0.05	37.1
West:	Headlan	nd Road (W)	)									
10	L2	3	0.0	0.007	3.4	LOS A	0.0	0.0	0.00	0.11	0.00	39.7
11	T1	10	0.0	0.007	0.0	LOS A	0.0	0.0	0.00	0.11	0.00	39.5
Appro	ach	13	0.0	0.007	0.8	NA	0.0	0.0	0.00	0.11	0.00	39.6
All Vel	hicles	32	0.0	0.008	2.7	NA	0.0	0.2	0.03	0.39	0.03	38.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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#### Site: 101 [Proposed AM]

HEA_SITE (No.224) Site Category: (None) Stop (Two-Way)

Move	ment P	erformanc	e - Vel	nicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East: I	Headland	d Road (E)										
5	T1	32	0.0	0.028	0.1	LOS A	0.1	0.7	0.11	0.18	0.11	39.0
6	R2	19	0.0	0.028	3.7	LOS A	0.1	0.7	0.11	0.18	0.11	39.1
Appro	ach	51	0.0	0.028	1.4	NA	0.1	0.7	0.11	0.18	0.11	39.0
North:	SITE AC	CESS (No.	.224)									
7	L2	7	0.0	0.007	6.8	LOS A	0.0	0.2	0.09	0.93	0.09	37.4
9	R2	2	0.0	0.007	6.6	LOS A	0.0	0.2	0.09	0.93	0.09	36.3
Appro	ach	9	0.0	0.007	6.7	LOS A	0.0	0.2	0.09	0.93	0.09	37.2
West:	Headlan	d Road (W)	1									
10	L2	36	0.0	0.033	3.4	LOS A	0.0	0.0	0.00	0.26	0.00	39.1
11	T1	27	0.0	0.033	0.0	LOS A	0.0	0.0	0.00	0.26	0.00	38.9
Appro	ach	63	0.0	0.033	1.9	NA	0.0	0.0	0.00	0.26	0.00	39.0
All Vel	hicles	123	0.0	0.033	2.1	NA	0.1	0.7	0.05	0.28	0.05	38.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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#### Site: 101 [Proposed PM]

HEA_SITE (No.224) Site Category: (None) Stop (Two-Way)

Move	ment P	erformanc	e - Vel	nicles								
Mov ID	Turn	Demand F Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
East:	Headland	d Road (E)										
5	T1	21	0.0	0.020	0.0	LOS A	0.1	0.5	0.07	0.20	0.07	39.0
6	R2	15	0.0	0.020	3.6	LOS A	0.1	0.5	0.07	0.20	0.07	39.1
Appro	ach	36	0.0	0.020	1.5	NA	0.1	0.5	0.07	0.20	0.07	39.0
North:	SITE AC	CCESS (No.	.224)									
7	L2	18	0.0	0.043	6.8	LOS A	0.1	1.0	0.10	0.93	0.10	37.5
9	R2	33	0.0	0.043	6.5	LOS A	0.1	1.0	0.10	0.93	0.10	36.4
Appro	ach	51	0.0	0.043	6.6	LOS A	0.1	1.0	0.10	0.93	0.10	36.8
West:	Headlan	d Road (W)	1									
10	L2	3	0.0	0.014	3.4	LOS A	0.0	0.0	0.00	0.05	0.00	40.0
11	T1	25	0.0	0.014	0.0	LOS A	0.0	0.0	0.00	0.05	0.00	39.8
Appro	ach	28	0.0	0.014	0.4	NA	0.0	0.0	0.00	0.05	0.00	39.8
All Vel	hicles	115	0.0	0.043	3.5	NA	0.1	1.0	0.07	0.49	0.07	38.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Minor Road Approach LOS values are based on average delay for all vehicle movements.

NA: Intersection LOS and Major Road Approach LOS values are Not Applicable for two-way sign control since the average delay is not a good LOS measure due to zero delays associated with major road movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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## Site: 101 [Existing AM]

PIT_HARX

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Site User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Move	ment l	Performan	ce - Vel	nicles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	: Harbo	rd Road (S)										
1b	L3	27	18.5	0.761	82.7	LOS F	16.8	124.4	1.00	0.95	1.54	21.5
1	L2	407	5.2	0.761	75.5	LOS F	16.8	124.4	0.99	0.92	1.31	22.2
3a	R1	266	6.0	0.572	69.7	LOS E	9.4	68.9	0.99	0.80	0.99	22.9
Appro	ach	700	6.0	0.761	73.6	LOS F	16.8	124.4	0.99	0.87	1.20	22.4
North	East: Pi	ttwater Road	1 (NE)									
24a	L1	476	3.6	0.613	17.0	LOS B	17.0	122.9	0.54	0.64	0.54	33.9
25	T1	1625	7.0	0.803	35.5	LOS C	42.5	315.4	0.85	0.78	0.87	28.9
26a	R1	308	1.0	0.529	64.7	LOS E	10.5	74.2	0.97	0.79	0.97	24.0
Appro	ach	2409	5.5	0.803	35.6	LOS C	42.5	315.4	0.81	0.75	0.82	29.0
West:	Warring	gah Road (V	√)									
10a	L1	86	8.1	0.130	35.2	LOS C	4.1	30.8	0.70	0.64	0.70	29.3
12	R2	485	3.5	0.793	71.4	LOS F	18.4	132.9	1.00	0.91	1.11	22.7
12b	R3	307	6.8	0.779	78.2	LOS F	11.7	86.5	1.00	0.91	1.15	21.9
Appro	ach	878	5.1	0.793	70.2	LOS E	18.4	132.9	0.97	0.88	1.08	22.9
South	West: F	Pittwater Roa	id (SW)									
30b	L3	84	39.3	0.065	3.8	LOS A	0.0	0.0	0.00	0.39	0.00	40.0
31	T1	1001	9.4	0.516	36.8	LOS C	20.3	153.4	0.81	0.70	0.81	28.6
32b	R3	100	7.0	0.804	85.4	LOS F	7.9	58.6	1.00	0.93	1.24	21.0
Appro	ach	1185	11.3	0.804	38.6	LOS C	20.3	153.4	0.77	0.70	0.79	28.3
All Vel	hicles	5172	6.8	0.804	47.3	LOS D	42.5	315.4	0.85	0.78	0.91	26.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ment Performance - Pedestria	ins						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of . Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P6	NorthEast Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P6B	NorthEast Slip/Bypass Lane Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P4	West Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P4B	West Slip/Bypass Lane Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
P8	SouthWest Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96
All Peo	destrians	316	69.3	LOS F			0.96	0.96

## Site: 101 [Existing PM]

PIT_HARX

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Site User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Move	ment	Performan	ce - Vel	hicles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	: Harbo	ord Road (S)										
1b	L3	36	8.3	0.788	74.8	LOS F	20.4	147.9	1.00	0.95	1.44	22.5
1	L2	470	3.2	0.788	70.6	LOS F	20.4	147.9	0.98	0.93	1.27	22.8
3a	R1	365	3.0	0.770	74.4	LOS F	13.6	97.8	1.00	0.90	1.11	22.3
Appro	ach	871	3.3	0.788	72.4	LOS F	20.4	147.9	0.99	0.92	1.21	22.6
North	East: P	ittwater Road	d (NE)									
24a	L1	377	5.0	0.379	14.8	LOS B	11.2	81.5	0.62	0.67	0.62	34.6
25	T1	1024	7.7	0.743	38.1	LOS C	34.9	260.3	0.88	0.78	0.88	28.3
26a	R1	349	2.6	0.774	74.4	LOS F	13.1	93.7	1.00	0.92	1.12	22.6
Approach		1750	6.1	0.774	40.3	LOS C	34.9	260.3	0.85	0.79	0.87	28.0
West:	Warrin	gah Road (V	V)									
10a	L1	193	4.1	0.318	42.2	LOS C	10.5	76.0	0.80	0.72	0.80	27.7
12	R2	400	4.5	0.667	67.2	LOS E	13.9	101.2	0.99	0.83	0.99	23.3
12b	R3	227	12.3	0.808	84.1	LOS F	8.9	69.2	1.00	0.94	1.23	21.2
Appro	ach	820	6.6	0.808	66.0	LOS E	13.9	101.2	0.95	0.83	1.01	23.5
South	West: F	Pittwater Roa	ad (SW)									
30b	L3	128	34.4	0.096	3.8	LOS A	0.0	0.0	0.00	0.39	0.00	40.0
31	T1	1490	5.4	0.759	35.6	LOS C	31.2	228.4	0.84	0.75	0.84	28.8
32b	R3	110	7.3	0.760	46.3	LOS D	4.9	36.2	1.00	0.87	1.16	26.9
Appro	ach	1728	7.7	0.760	34.0	LOS C	31.2	228.4	0.79	0.73	0.80	29.3
All Ve	hicles	5169	6.2	0.808	47.7	LOS D	34.9	260.3	0.87	0.80	0.93	26.5

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Bacł Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate				
P1	South Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96				
P6	NorthEast Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96				
P6B	NorthEast Slip/Bypass Lane Crossing	53	32.1	LOS D	0.1	0.1	0.92	0.92				
P4	West Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96				
P4B	West Slip/Bypass Lane Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96				
P8	SouthWest Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96				
All Pedestrians		316	63.1	LOS F			0.96	0.96				

## Site: 101 [Proposed AM]

PIT_HARP

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Site User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/ <u>c</u>	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance <u>m</u>	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/ <u>h</u>
South	: Harbo	rd Road (S)										
1b	L3	30	16.7	0.696	81.5	LOS F	16.0	118.0	0.99	0.91	1.52	21.6
1	L2	412	5.1	0.696	72.6	LOS F	16.0	118.0	0.98	0.87	1.25	22.6
3a	R1	271	5.9	0.651	72.6	LOS F	9.8	72.0	1.00	0.82	1.02	22.5
Appro	ach	713	5.9	0.696	73.0	LOS F	16.0	118.0	0.99	0.86	1.18	22.5
North	East: P	ittwater Road	l (NE)									
24a	L1	554	3.1	0.732	21.4	LOS B	23.3	167.2	0.63	0.70	0.63	32.6
25	T1	1625	7.0	0.872	47.4	LOS D	50.1	371.2	0.90	0.89	1.01	26.4
26a	R1	308	1.0	0.760	75.4	LOS F	11.6	81.7	1.00	0.90	1.12	22.5
Appro	ach	2487	5.3	0.872	45.1	LOS D	50.1	371.2	0.86	0.85	0.94	26.9
West:	Warrin	gah Road (V	√)									
10a	L1	86	8.1	0.143	38.9	LOS C	4.3	32.5	0.74	0.66	0.74	28.4
12	R2	563	3.0	0.863	75.9	LOS F	22.9	164.5	1.00	0.97	1.19	22.1
12b	R3	307	6.8	0.871	86.8	LOS F	12.5	92.5	1.00	1.00	1.31	20.9
Appro	ach	956	4.7	0.871	76.1	LOS F	22.9	164.5	0.98	0.95	1.19	22.1
South	West: F	Pittwater Roa	id (SW)									
30b	L3	84	39.3	0.065	3.8	LOS A	0.0	0.0	0.00	0.39	0.00	40.0
31	T1	1001	9.4	0.459	31.3	LOS C	18.7	141.0	0.75	0.65	0.75	29.8
32b	R3	141	5.0	0.840	84.4	LOS F	11.2	81.8	1.00	0.96	1.26	21.1
Appro	ach	1226	10.9	0.840	35.5	LOS C	18.7	141.0	0.72	0.67	0.75	29.0
All Vel	hicles	5382	6.6	0.872	52.1	LOS D	50.1	371.2	0.86	0.83	0.97	25.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of . Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate				
P1	South Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96				
P6	NorthEast Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96				
P6B	NorthEast Slip/Bypass Lane Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96				
P4	West Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96				
P4B	West Slip/Bypass Lane Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96				
P8	SouthWest Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96				
All Pedestrians		316	69.3	LOS F			0.96	0.96				

## Site: 101 [Proposed PM]

PIT_HARP

Site Category: (None)

Signals - Fixed Time Isolated Cycle Time = 150 seconds (Site User-Given Cycle Time) Variable Sequence Analysis applied. The results are given for the selected output sequence.

Movement Performance - Vehicles												
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	: Harbo	ord Road (S)										
1b	L3	43	7.0	0.799	75.2	LOS F	21.5	155.4	1.00	0.97	1.46	22.4
1	L2	482	3.1	0.799	70.8	LOS F	21.5	155.4	0.98	0.94	1.29	22.8
3a	R1	378	2.9	0.757	73.0	LOS F	14.0	100.2	1.00	0.89	1.09	22.5
Appro	ach	903	3.2	0.799	71.9	LOS F	21.5	155.4	0.99	0.92	1.21	22.6
North	East: P	ittwater Road	d (NE)									
24a	L1	403	4.7	0.409	15.5	LOS B	12.4	90.3	0.64	0.69	0.64	34.4
25	T1	1024	7.7	0.766	40.0	LOS C	35.7	265.9	0.90	0.80	0.90	27.9
26a	R1	349	2.6	0.774	74.4	LOS F	13.1	93.7	1.00	0.92	1.12	22.6
Appro	ach	1776	6.0	0.774	41.2	LOS C	35.7	265.9	0.86	0.80	0.88	27.8
West:	Warrin	gah Road (V	V)									
10a	L1	193	4.1	0.318	42.2	LOS C	10.5	76.0	0.80	0.72	0.80	27.7
12	R2	425	4.2	0.713	68.5	LOS E	15.2	110.4	1.00	0.85	1.03	23.1
12b	R3	227	12.3	0.808	84.1	LOS F	8.9	69.2	1.00	0.94	1.23	21.2
Appro	ach	845	6.4	0.808	66.7	LOS E	15.2	110.4	0.95	0.85	1.03	23.4
South	West: F	Pittwater Roa	ad (SW)									
30b	L3	128	34.4	0.096	3.8	LOS A	0.0	0.0	0.00	0.39	0.00	40.0
31	T1	1490	5.4	0.775	36.8	LOS C	31.3	229.3	0.85	0.76	0.85	28.6
32b	R3	122	6.6	0.783	46.6	LOS D	5.4	39.8	1.00	0.89	1.18	26.9
Appro	ach	1740	7.6	0.783	35.0	LOS C	31.3	229.3	0.80	0.74	0.81	29.1
All Vel	hicles	5264	6.1	0.808	48.5	LOS D	35.7	265.9	0.88	0.81	0.94	26.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of . Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate				
P1	South Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96				
P6	NorthEast Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96				
P6B	NorthEast Slip/Bypass Lane Crossing	53	32.0	LOS D	0.1	0.1	0.92	0.92				
P4	West Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96				
P4B	West Slip/Bypass Lane Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96				
P8	SouthWest Full Crossing	53	69.3	LOS F	0.2	0.2	0.96	0.96				
All Pedestrians		316	63.1	LOS F			0.96	0.96				