

GOSFORD NORTHSIDE PRIVATE HOSPITAL

CIVIL & STORMWATER

SSDA



LOCALITY AERIAL
NOT TO SCALE

SOURCE : SIXMAPS

DRAWING SCHEDULE		
DRAWING No.	DRAWING TITLE	REV
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NSW
Plot Date: 31.07.2020 09:17:24:20
C:\p1\10001\NSP\LOCAL\Drawings\6422001\Drawings\AutoCAD\Drawings\01\Cover Sheet.dwg

CLIENT	
PROJECT MANAGER	DONALD CANT WATTS CORKE
ARCHITECT	

Warren
Smith &
Partners

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Consulting Engineers
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COVER SHEET				
SCALE	DRAWN	DESIGNED	CHECKED	APPROVED
AS SHOWN	M.T.	M.D.	L.S.	L.S.
JOB No.	DRAWING No.		ISSUE	
6422001	C1.01		1	
DATE	STATUS			
JULY 2020	SSDA			

Plot Date

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Pld Date: 30.07.2020@20:25:03
Caf File: T:\422001\Drawings\AutoCAD\Civil\Design\Civil_201\02_Specification Notes.dwg
Caf Name: mbylor
Login Name: mbylor

GENERAL

- G1.

DESIGN HEREIN HAS BEEN PREPARED BY WARREN SMITH AND PARTNERS PTY LTD CONSULTING CIVIL ENGINEERS, LEVEL 9, 233 CASTLEREAGH ST, SYDNEY NSW 2000. TEL:- (02) 9299 1312, FAX:- (02) 9290 1295.
- G2.

THE DRAWINGS HEREIN SHALL BE READ AS REQUIRED IN CONJUNCTION WITH ARCHITECT DRAWINGS BY: HEALTH PROJECTS INTERNATIONAL, SUITE 1, GROUND FLOOR, 68 ALFRED STREET MILSONS POINT, NSW, 1565 TEL: (02) 9460 4199
- G3.

ALL DIMENSIONS IN MILLIMETRES UNO. REDUCED LEVELS AND CHAINAGES ARE IN METRES. DO NOT SCALE DRAWINGS. USE FIGURED DIMENSIONS.
- G4.

THE PROPOSED WORKS DETAILED HEREIN SHALL BE CONSTRUCTED TO THE REQUIREMENTS OF COUNCIL GENERALLY AS DETAILED HEREUNDER.
- G5.

ALL EXISTING SERVICES SHALL BE VERIFIED FOR DEPTH AND HORIZONTAL POSITION BY PHYSICAL MEANS PRIOR TO EXCAVATION. ANY DISCREPANCIES SHALL BE BROUGHT FORTHWITH TO THE PROJECT MANAGER'S ATTENTION.
- G6.

REFERENCE SHALL BE MADE TO THE CONSTRUCTION MANAGEMENT PLAN FOR ALL SITE WORKS DETAILED HEREIN.

STORMWATER & SUB-SOIL DRAINAGE MATERIALS:

- STW1.

PIPES AND FITTINGS FOR STORMWATER DRAINAGE SHALL BE AS FOLLOWS UNO ON THE DRAWINGS:
A. SEWER GRADE uPVC (SN8) WITH SOLVENT WELDED JOINTS FOR BELOW GROUND DRAINAGE UP TO 225mm.
B. REINFORCED CONCRETE WITH RUBBER RINGS, CLASS 3, FOR PIPE DIA'S GREATER THAN 225mm OR WHERE REQUIRED BY AS3500 FOR EXCESSIVE DEPTH.
C. INSTALL IN ACCORDANCE WITH AUSTRALIAN STANDARD AS3500 EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.
- STW2.

PIPES & FITTINGS FOR SUBSOIL DRAINAGE SHALL BE SLOTTED POLYVINYL CHLORIDE (PVC) WITH SOLVENT WELDED JOINTS, MIN. 150mm DIAMETER.
- STW3.

IN GROUND DRAINAGE PIPEWORK SERVING DP's SHALL BE MINIMUM 150mm DIA. UNO.
- STW4.

GRATED DRAINS SHALL BE 150mm NOM.
A. 150mm NOM. WIDTH IN NON TRAFFICABLE AREAS.
B. 225mm NOM. WIDTH IN TRAFFICABLE AREAS.
- STW5.

STORMWATER PITS ARE AS SHOWN & SPECIFIED ON THE PLANS . PRECAST TYPE ACCEPTABLE WITH STEP IRONS FOR DEPTH GREATER THAN 1200. BENCH ALL PITS MIN. 30mm & FORM SMOOTH TRANSITION FROM INLET TO OUTLET.
- STW6.

SELECT FILL SHALL BE MATERIAL OBTAINED FROM EXCAVATION OF THE PIPE TRENCH OR IMPORTED WITH A PARTICLE SIZE FOR ROCK NOT GREATER THAN 75mm OR FOR OTHER THAN ROCK NOT GREATER THAN 150mm.
- STW7.

IMPORTED FILL SHALL BE EITHER, AND GENERALLY CONSIST OF SINGLE SIZED AGGREGATE WITH PARTICLE SIZE NOT GREATER THAN 5mm WRAPPED ALL ROUND WITH GEOTEXTILE FILTER FABRIC OR APPROVED HIGH COMPACTION SAND OR APPROVED CRUSHED ROAD GRAVEL CONFORMING TO RTA FORM 3051 OR SIMILAR.
- STW8.

STORMWATER PITS AND GRATES TO CONFORM WITH STANDARD COUNCIL REQUIREMENTS, WHERE ON PUBLIC LAND. GRATES TO BE SUPPLIED IN CLASS SHOWN ON THE DRAWINGS.

INSTALLATION REQUIREMENTS:

- STW9.

PIPES SHALL BE TRUE TO GRADES SHOWN AND ALIGNED SO THAT THE CENTRES OF THE INLET PIPES INTERSECT WITH THE CENTRE OF THE OUTLET PIPE AT THE DOWNSTREAM FACE OF THE PIT.
- STW10.

MINIMUM GRADES FOR GRAVITY STORMWATER DRAINAGE SHALL CONFORM TO AS3500 PART3 AS FOLLOWS, UNO:
1% FOR 100 AND 150 mm DIA.
0.5% FOR 225 mm DIA
0.5% FOR 300 mm DIA
0.4% FOR 375 mm DIA
- STW11.

MINIMUM DEPTH OF COVER SHALL BE :-
- 300mm IN PRIVATE PROPERTY (NON VEHICULAR TRAFFIC).
- 450mm IN PUBLIC AREAS.
- 600mm IN VEHICULAR TRAFFICABLE AREAS (FOOTWAY/ROADWAY)

- STW12.

BED ALL PIPES FIRMLY AND EVENLY ONTO IMPORTED BEDDING FILL MATERIAL.
- STW13.

LAY AND JOINT ALL PIPES IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS AND AS 3725-1989 BURIED FLEXIBLE PIPELINES AS 2566-1998 LOADS ON BURIED FLEXIBLE PIPELINES AS 1597.2-1996 PRECAST REINFORCED CONCRETE BOX CULVERTS.
AS 3500-1990 NATIONAL PLUMBING & DRAINAGE CODE. PART 2, SANITARY PLUMBING AND SANITARY DRAINAGE. SYDNEY WATER REQUIREMENTS.
- STW14.

ALLOW TO TEST ALL PIPES AND PITS TO MANUFACTURERS REQUIREMENTS.

CONCRETE WORKS

- C1.

ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS3600, THE STANDARDS ASSOCIATION AUSTRALIA, STANDARDS CITED IN AS3600, THE DRAWINGS AND THE SPECIFICATION.
- C2.

ALL CONCRETE SHALL BE 80mm NOMINAL SLUMP, 20mm MAXIMUM AGGREGATE WITH NO ADMIXTURES OR FLY ASH, UNLESS OTHERWISE APPROVED. ALL CONCRETE WORK IN CONTACT WITH SEWER TO HAVE TYPE SL PORTLAND CEMENT, OTHERWISE TYPE A CEMENT FOR BRIDGE WORKS, A MAXIMUM 56 DAYS SHRINKAGE OF 600 MICROSTRAIN, A MINIMUM CEMENT CONTENT 350kg/m3 AND MAXIMUM WATER:CEMENT RATIO OF 0.40.
- C3.

STRENGTH GRADE OF CONCRETE SHALL BE : 25 MPa (KERBS, EDGE STRIPS & CONCRETE ENCASEMENT) AND 32 MPa ELSEWHERE.
- C4.

CONSTRUCTION JOINTS SHALL BE PROPERLY FORMED AND USED ONLY WHERE SHOWN OR APPROVED. GENERALLY FOR HAND PLACED KERB & GUTTER 6mm THICK APPROVED BITUMINOUS MASTIC JOINTING MATERIAL SHALL BE PROVIDED AT INTERVALS NOT EXCEEDING 6m. FOR MACHINE PLACED KERB & GUTTER 6mm THICK APPROVED BITUMINOUS MASTIC JOINTING MATERIAL SHALL BE PROVIDED AT INTERVALS NOT EXCEEDING 12m & GUILLOTINED DUMMY GROOVED JOINTS, 25mm IN DEPTH, SHALL BE FORMED EVERY 3m OF GUTTER. JOINTS ARE ALSO REQUIRED AT EACH END OF GUTTER CROSSING AND GULLY PITS. JOINTS SHALL BE SET VERTICAL AND SQUARE TO THE KERB.
- C5.

REINFORCEMENT IS REPRESENTED DIAGRAMMATICALLY AND IS NOT NECESSARILY SHOWN IN TRUE PROJECTION.
- C6.

ELDING OR SPLICES IN REINFORCEMENT SHALL BE USED ONLY IN POSITIONS APPROVED BY THE ENGINEER.
- C7.

CONCRETE CURING SHALL BE IN ACCORDANCE WITH AS3600. CURING SHALL BE COMMENCED WITHIN TWO HOURS OF FINISHING OPERATIONS AND SHALL BE CONTINUED FOR A MINIMUM OF SEVEN DAYS BY AN APPROVED PROPRIETARY COMPOUND OR BY KEEPING CONTINUOUSLY WET.
- C8.

FORMWORK SHALL BE DESIGNED AND CONSTRUCTED IN ACCORDANCE WITH AS3610. FORMWORK SHALL NOT BE STRIPPED NOR PROPS REMOVED WITHOUT APPROVAL.
- C9.

FABRIC LAP DETAILS SHALL BE IN ACCORDANCE WITH FIG.13.2.4 OF AS3600.
- C10.

HOOKS, LAPS AND BENDS SHALL BE IN ACCORDANCE WITH AS3600 UNO.
- C11.

ALL CHEMICAL ANCHORS SHALL BE EITHER 'CHEMSET' BY 'RAMSET' WITH THE GLASS CAPSULE SYSTEM INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURERS INSTRUCTIONS OR HILTI HVU ADHESIVE ANCHOR WITH FOIL CAPSULE SYSTEM INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURERS INSTRUCTION.
- C12.

ALL CHEMICAL ANCHORS SHALL BE HOT DIPPED GALVANIZED AND BE MIN M16 DIA. U.N.O.

GENERAL EARTHWORKS, SITEWORKS & FILLING: FILLING:

- SGE1.

THESE CLAUSES SHALL BE READ IN CONJUNCTION WITH THE DOUGLAS PARTNERS GEOTECHNICAL INVESTIGATION REPORT, DATED 14TH MARCH 2019.
- SGE2.

ANY CONTRADICTIONS BETWEEN THE DESIGN DOCUMENTS AND THE RECOMMENDATIONS CONTAINED IN THE GEOTECH REPORT SHALL BE REPORTED TO WS+P.
- SGE3.

STRIP ALL TOPSOIL AND UNDERLYING FILL AND STOCKPILE TOPSOIL FOR LATER REUSE FOR LANDSCAPING PURPOSES.
- SGE4.

NEW FILL REQUIRED TO REINSTATE CUT LEVELS TO PROPOSED BENCHING LEVELS SHALL BE SOURCED FROM OTHER PARTS OF THE EXCAVATION AS SELECT FILL OR IMPORTED FILL AS SPECIFIED BELOW IN SGE 5 AND SGE 6.

- SGE5.

SELECT FILL SHALL CONSIST OF LOCALLY DERIVED OR CUT NATURAL CLAYS.
- SGE6.

IMPORTED FILL SHALL CONSIST OF RIPPED SANDSTONE OR SHALE OR SIMILAR MATERIAL WITH MAXIMUM PARTICLE SIZE NOT GREATER THAN 120mm AND A MOISTURE CONTENT WITHIN 2-3% OF STANDARD OPTIMUM.
- SGE7.

ALL FILL (COHESIVE SOIL) SHALL BE PLACED IN LAYERS OF 200mm MAXIMUM THICKNESS, COMPACTED BY MACHINE ROLLING TO ACHIEVE A DRY DENSITY RATIO OF NOT LESS THAN 98% STANDARD MAXIMUM AT A CORRESPONDING MOISTURE CONTENT WITHIN 2-3% OF STANDARD OPTIMUM.
- SGE8.

IN AREAS WHERE HIGH IMPACT ROLLING IS USED TEST EACH FINAL LAYER OF NOT GREATER THAN 300mm TO 400mm TO ACHIEVE A DRY DENSITY SGE8. RATIO OF NOT LESS THAN 98% STANDARD MAXIMUM AT A CORRESPONDING MOISTURE CONTENT WITHIN 2-3% OF STANDARD OPTIMUM.
- EXCAVATION BATTERS:

SGE9.

ALL TEMPORARY BATTERS CUT IN CLAY SUBSTRATE SHALL BE 1 HORIZ : 1 VERT. ALL LONG TERM EXPOSED BATTERS CUT IN CLAY SUBSTRATE SHALL BE 2 HORIZ : 1 VERT. ALL DETENTION BASIN BATTERS IN CLAY SUBSTRATE SHALL BE 3 HORIZ : 1 VERT. ALL DETENTION BASIN BATTERS IN ROCK SUBSTRATE SHALL BE NEAR VERTICAL.
- SGE10.

GEOTECHNICAL TESTING IS TO BE UNDERTAKEN TO AT LEAST LEVEL 1 CONTROL OF FILL COMPACTION STANDARD, AS DEFINED IN AS. 3738 AS FOLLOWS
- FOR GENERAL FILL OR CUT AREAS OVER THE AREA PROVIDE ONE (1) TEST PER 200mm LAYER, OVER AN AREA NOT GREATER THAN 500 m²
- FOR GENERAL FILL AREAS IN CONCENTRATED AREAS ADJACENT TO AND BEHIND THE STRUCTURE AND ADJACENT TO AND BEHIND RETAINING WALLS PROVIDE ONE (1) TEST PER 200mm LAYER, OVER AN AREA NOT GREATER THAN 50m²
- SGE11.

SUBMIT ALL GEOTECHNICAL TEST RESULTS TO WARREN SMITH & PARTNERS FOR REVIEW PRIOR TO CONTINUATION WITH SUBSEQUENT SECTION OF WORK.

EARTH WORKS FOR SERVICES

- E1.

EXCAVATE TRENCHES AND STOCKPILE ALL MATERIAL FOR INSPECTION WITH REGARD TO RE-USE FOR TRENCH BACKFILL. REMAINING MATERIAL TO BE REMOVED FROM SITE.
- E2.

BEDDING MATERIAL SHALL CONSIST OF IMPORTED FILL ONLY. THICKNESS OF BEDDING LAYER SHALL BE 75mm IN O.T.R. AND 200mm IN ROCK.
- E3.

EMBED ALL PIPES WITH IMPORTED FILL. PROVIDE 200mm SIDE SUPPORT AND 150mm OVERLAY ABOVE PIPE CROWN.
- E4.

TRENCH FILL ABOVE THE EMBEDMENT ZONE TO THE UNDERSIDE OF THE ROAD PAVEMENT OR FOOTWAY FILL MATERIAL SHALL BE AS FOLLOWS :
- UNDER ROADWAY

TRENCH FILL MATERIAL SHALL CONSIST OF IMPORTED FILL AS SPECIFIED HEREIN OF EITHER HIGH GRADE COMPACTION SAND OR APPROVED CRUSHED ROAD GRAVEL CONFORMING TO RTA FORM 3051 OR SIMILAR.

OTHER THAN ROADWAY

TRENCH FILL MATERIAL EXCAVATED SHALL CONSIST OF SELECT FILL AS SPECIFIED HEREIN AND SHALL NOT CONTAIN MORE THAN 20% OF STONES OF SIZE BETWEEN 75mm & 150mm AND NONE LARGER THAN 150mm. PRIOR TO THE USE OF THE EXCAVATED MATERIAL IT SHALL BE INSPECTED AND APPROVED BY THE CONSULTANT.

- E5.

COMPACT BEDDING, EMBEDMENT AND TRENCH FILL MATERIALS AS FOLLOWS:

EMBEDMENT:-
FOR GRANULAR FILL MATERIAL (NON-COHESIVE SOILS) E.G. COARSE AGGREGATE FILL, HIGH GRADE COMPACTION SAND, THE DENSITY INDEX (ID) SHALL BE NOT LESS THAN 70%.

TRENCH FILL:-
FOR GRANULAR MATERIAL (NON-COHESIVE SOILS), THE DENSITY INDEX (ID) SHALL BE NOT LESS THAN 70%.

FOR NON-GRANULAR FILL MATERIAL (COHESIVE SOILS), THE DRY DENSITY RATIO (RD) SHALL BE NOT LESS THAN 95%.

- E6.

MEASURE OF COMPACTION:-
THE DEGREE OF COMPACTION SHALL BE MEASURED BY ONE OF THE FOLLOWING PARAMETERS :-

GRANULAR FILL (NON-COHESIVE SOILS). THE DENSITY INDEX (ID) DETERMINED IN ACCORDANCE WITH AS 1289.E6.1 BASED ON THE MAXIMUM AND MINIMUM DRY DENSITIES IN ACCORDANCE WITH AS 1289.E5.1 AND THE FIELD DRY DENSITY IN ACCORDANCE WITH AS 1289.5.3.2, AS 1289.E3.5 OR AS 1289.E8.1.

NON-GRANULAR FILL (COHESIVE SOILS). THE DRY DENSITY RATION (RD) DETERMINED IN ACCORDANCE WITH AS 1289.5.4.1 BASED ON THE FIELD DRY DENSITY IN ACCORDANCE WITH AS 1289.5.3.2 AND THE MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS 1289.5.1.1
- E7.

GEOTECHNICAL TESTING IS TO BE UNDERTAKEN TO AT LEAST LEVEL 1 CONTROL OF FILL COMPACTION STANDARD, AS DEFINED IN AS. 3738 AS FOLLOWS:

- TEST EACH 300mm LAYER ABOVE PIPE CROWN.
- TEST BASE & SUB-BASE LAYERS WHERE APPLICABLE. TESTS SHALL BE REQUIRED AT EACH 50m CENTRES WHERE THE LENGTH OF TRENCH IS WITHIN THE 50m REQUIREMENT.

E8. SUBMIT ALL GEOTECHNICAL TEST RESULTS TO WARREN SMITH & PARTNERS FOR REVIEW PRIOR TO CONTINUATION WITH SUBSEQUENT SECTION OF WORK.

RESTORATION:

- RES1.

RESTORE ALL TRAFFIC AREAS TO PRE EXISTING CONDITION.
- RES2.

FOR ALL SURFACES OTHER THAN IN TRAFFIC AREAS RESTORE DISTURBED SURFACES TO PRE-EXISTING CONDITIONS AND COMPACT AS SPECIFIED.
- RES3.

RESTORE ALL AUTHORITY OWNED AREAS TO COUNCIL STANDARDS

ROAD WORKS, DRIVEWAYS & CARPARKS

- R1.

ALLOW FOR LEVEL 2 TESTING AND SUB-GRADE CONDITIONS & PAVEMENT THICKNESS TO BE VERIFIED BY GEOTECHNICAL CONSULTANT AFTER INSPECTION OF PRELIMINARY BOXING.
- R2.

ALLOW FOR ANY SUB-GRADE REPLACEMENT WORK TO BE DETERMINED AS REQUIRED BY GEOTECHNICAL CONSULTANT AT THE TIME OF PAVEMENT CONSTRUCTION.
- R3.

MINIMUM DRY DENSITY RATIOS (AS 1289 3.4.1-1993) TO BE:
BASE COURSE: 98% MODIFIED
SUB-BASE: 95% MODIFIED
SUB-GRADE: 100% STANDARD
SUB-GRADE REPLACEMENT: 100% STANDARD
- R4.

PAVEMENT MATERIALS TO COMPLY WITH RMS SPECIFICATION No. 3051 OR SIMILAR AS APPROVED BY GEOTECHNICAL CONSULTANT.
- R5.

PROVIDE (1) TEST FOR EACH LAYER NOT EXCEEDING 250mm THICK BEING BASECOURSE, SUB-BASE & SUB-GRADE OVER AN AREA NOT GREATER THAN 500m²
- R6.

SUBMIT ALL GEOTECHNICAL TEST RESULTS TO WARREN SMITH & PARTNERS FOR REVIEW PRIOR TO CONTINUATION WITH SUBSEQUENT SECTION OF WORK.

APPROVALS

- A1.

THE AS CONSTRUCTED WORKS SHALL BE INSPECTED BY DESIGN CONSULTANT. MINIMUM 48 HOURS NOTICE SHALL APPLY TO ALL INSPECTIONS.
- A2.

THE DESIGN PLANS HEREIN ARE SUBJECT TO LOCAL COUNCIL APPROVAL PRIOR TO CONSTRUCTION. OBTAIN EXPRESS (WRITTEN) ADVICE TO PROCEED FROM PROJECT MANAGER PRIOR TO COMMENCEMENT.
- A3.

SUBMIT WORK-AS-EXECUTED DRAWINGS IN CIVILCAD OR DXF DIGITAL FORMAT AND HARD COPY FORMAT. VERIFY ALL CONSTRUCTION WORKS SHOWN HEREON.
- A4.

CERTIFY THAT THE AS CONSTRUCTED SYSTEM HAS BEEN BUILT IN ACCORDANCE WITH THE APPROVED PLANS ISSUED FOR CONSTRUCTION.

SERVICES UNDER ROAD SURFACES

- S1.

ALL OTHER SERVICES INCLUDING BUT NOT LIMITED TO WATER, HYDRANT, GAS, SEWER, ELECTRICAL AND COMMUNICATIONS CONDUITS OR CABLES SHALL BE LAID WITH MINIMUM 600mm U.N.O. COVER BELOW PROPOSED ROAD SURFACE OR APPROVED OTHER MEANS TO PROTECT DURING CONSTRUCTION.

ROAD SIGNS & LINE MARKING

- RS1.

ALL SIGNS AND LINEMARKING SHALL BE TO ROADS & TRAFFIC AUTHORITY STANDARDS AND SPECIFICATIONS AND AS.1742, MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES
- RS2.

ALL LINEMARKING SHALL BE AUGMENTED BY RETROREFLECTIVE RAISED PAVEMENT MARKERS (RRPMs) AND ALL SHALL BE TO AS 1742.2 - 1994 AND AS 1742.2 /AMDT 1/1997-10-05
- RS3.

ALL ROAD SIGNS AND POSTS SHALL BE TO AS 1742.2 - 1994 AND AS 1742.2 /AMDT 1/1997-10-05

PROTECTION OF FLORA - REFER SPECIFICATION

1.

ANY TRENCHES WITHIN 3m OF TREES SHALL BE HAND DUG TO AVOID DAMAGE TO TREE ROOTS.
2.

IF IT IS CONSIDERED NECESSARY TO PERFORM ANY WORK ON TREES, INCLUDING TRIMMING, LOPPING, ROOT CUTTING, REPAIR AND REMOVAL, APPLICATION IN WRITING SHALL BE MADE BY THE CONTRACTOR TO THE SUPERINTENDENT. ANY WORK PERMITTED TO BE DONE ON TREES TO BE RETAINED SHALL BE PERFORMED IN ACCORDANCE WITH THE ARBORISTS REPORT.
3.

MATURE TREES AND SHRUBS ARE TO BE REMOVED IN ACCORDANCE WITH THE ARBORISTS REPORT.

AUTHORITY STANDARDS

- LGA 1.

THE DRAWINGS HEREIN SHALL BE READ IN CONJUNCTION WITH CENTRAL COAST COUNCIL STANDARDS & SPECIFICATIONS.

TRAFFIC NOTE:

1.

A TRAFFIC CONTROL PLAN IS TO BE PREPARED BY AN ACCREDITED RMS 1. TRAFFIC CONTROLLER AND SUBMITTED TO COUNCIL. THIS TRAFFIC PLAN IS TO BE CERTIFIED BY AND IMPLEMENTED TO THE SATISFACTION OF AN ACCREDITED RMS TRAFFIC CONTROLLER

PRIOR TO COMMENCEMENT OF WORK

2.

ALL TRAFFIC CONTROL WORKS SHALL ONLY BE CARRIED OUT BY 2. ACCREDITED RMS TRAFFIC CONTROLLERS.

CLOSED CIRCUIT COLOUR TV (CCTV)

- CCTV 1.

UNDERTAKE A CCTV INSPECTION OF ALL THE COMPLETED DRAINAGE IN ACCORDANCE WITH THE GUIDELINES OF THE AUSTRALIAN CONDUIT CONDITION EVALUATION MANUAL (ACCEM)
- CCTV 2.

APPLY THE FOLLOWING REQUIREMENTS TO THE CCTV INSPECTION:-
A. USE DATA CAPTURE SOFTWARE APPROVED BY SYDNEY WATER

B. USE CERTIFIED CCTV OPERATORS
C. THE CCTV FOOTAGE SHALL BE OF QUALITY TO ALLOW ACCURATE ASSESSMENT OF THE INTERNAL CONDITION OF THE PIPE. FURNISH TO THE DESIGN CONSULTANT:-

CCTV 3. A. VIDEOS IN MPG FORMAT
B. CCTV REPORT AND SURVEY DATA IN PDF FORMAT

PROTECTION OF TREES

- T1.

WHERE STORMWATER DRAINAGE IS LAID WITHIN THE VICINITY OF TREES / CANOPIES OF TREES, WORK IS TO BE COMPLETED TO THE PROJECT ARBORIST'S REQUIREMENTS AND SPECIFICATIONS.

PRELIMINARY - NOT FOR CONSTRUCTION

DO NOT SCALE FROM DRAWINGS. CHECK & VERIFY ALL DIMENSIONS & LEVELS BEFORE COMMENCEMENT OF ANY WORK.

THIS DRAWING IS NOT TO BE COPIED IN PART OR WHOLE WITHOUT WRITTEN PERMISSION FROM WARREN SMITH AND PARTNERS.

REVISION	AMENDMENT	DATE	REVISION	AMENDMENT	DATE
1	DEVELOPMENT APPLICATION	31/07/20			

CLIENT

 **Health Infrastructure**

PROJECT

GOSFORD NORTHSIDE PRIVATE HOSPITAL

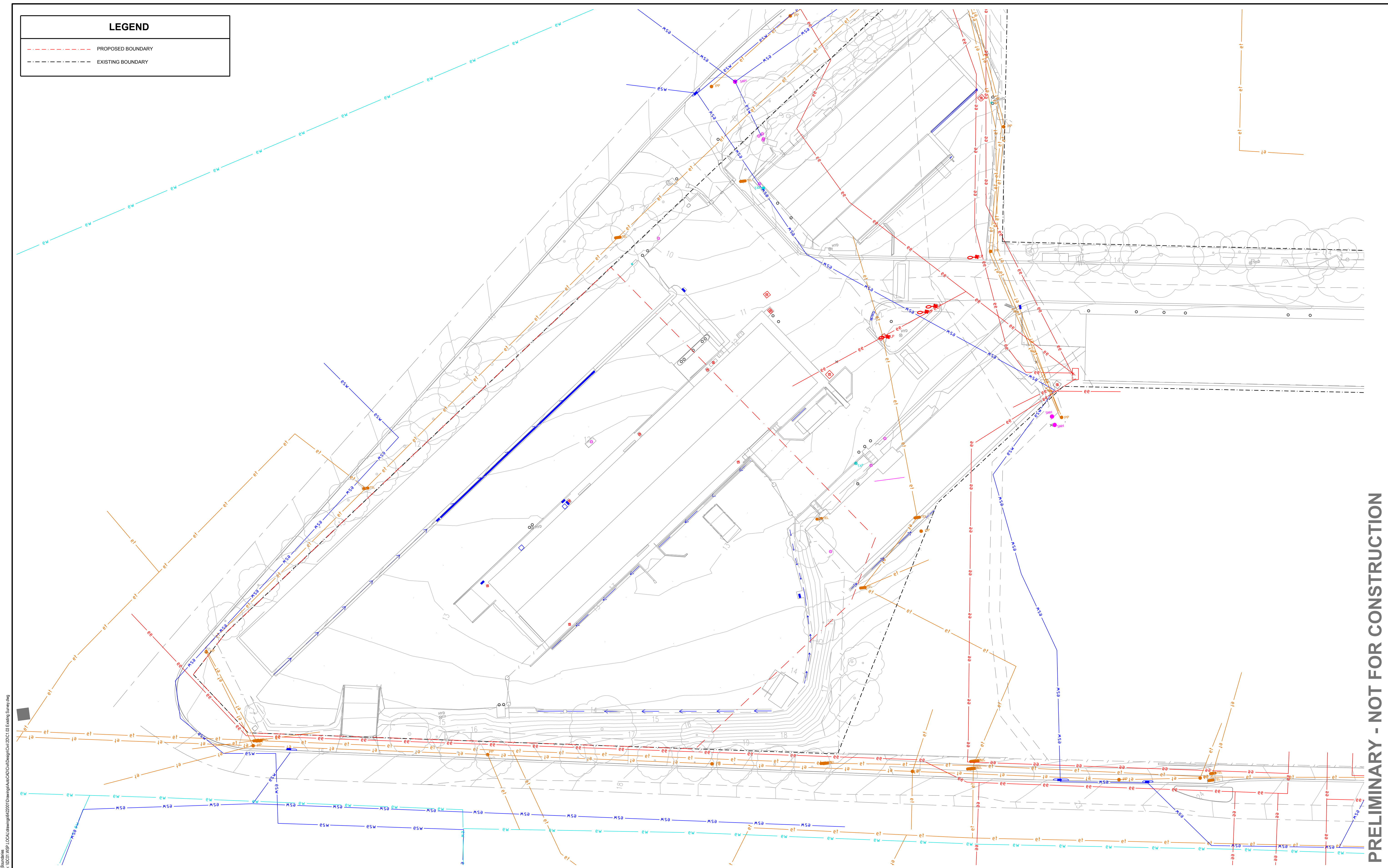
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TITLE						
SPECIFICATION NOTES						
SCALE	DRAWN	DESIGNED	CHECKED	APPROVED		
AS SHOWN	M.T.	M.D.	--	--		
JOB No.	DRAWING No.		ISSUE			
6422001	C1.02		1			
DATE	STATUS					
JULY 2020	SSDA					



LEGEND

PROPOSED BOUNDARY

EXISTING BOUNDARY

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NORTH

50m

0

5

10

15

20m

PLAN

SCALE 1:400

A1 SHEET

REVISION	AMENDMENT	DATE	REVISION	AMENDMENT	DATE
1	DEVELOPMENT APPLICATION	31/07/20			

Health Infrastructure

PROJECT

GOSFORD NORTHSIDE PRIVATE HOSPITAL

PREPARED BY

Warren Smith & Partners Pty Ltd
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TITLE

EXISTING SURVEY

SCALE	AS SHOWN	DRAWN	M.T.	DESIGNED	M.D.	CHECKED	L.S.	APPROVED	L.S.
JOB No.	6422001	DRAWING No.	C1.03	ISSUE	1				
DATE	JULY 2020	STATUS	SSDA						

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NSW GOVERNMENT

Health Infrastructure

CLIENT

GOSFORD NORTHSIDE PRIVATE HOSPITAL

PROJECT

Warren Smith & Partners

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TITLE

SEDIMENT AND EROSION CONTROL DETAILS SHEET 2

SCALE	DRAWN	DESIGNED	CHECKED	APPROVED
AS SHOWN	M.T.	M.D.	L.S.	L.S.

JOB No.

6422001

DRAWING No.

C2.03

ISSUE

1

DATE

JULY 2020

STATUS

SSDA

FOR CONTINUATION REFER TO INSERT

LEGEND

- PROPOSED BOUNDARY
- BEL 2.99 BULK EARTHWORKS LEVEL
- FFL 3.19 FINISHED FLOOR LEVEL
- 95 NATURAL SURFACE CONTOUR
- 78.0 PROPOSED CONTOUR
- 2.0 DEPTH CONTOUR

NOTES:

- THIS IS A PRELIMINARY VOLUMES CALCULATION AND DOES NOT TAKE INTO ACCOUNT:
 - PIPE TRENCH EXCAVATIONS
 - STRUCTURAL BUILDING FOOTINGS/ EDGE BEAMS
 - RETAINING WALL BACKFILLING
 - ON-SITE DETENTION TANK
- A PAVEMENT THICKNESS OF 330mm HAS BEEN ASSUMED FOR TRAFFICABLE AREAS AND 250mm FOR NON-TRAFFICABLE AREAS.
- AN INTERNAL BASEMENT SLAB OF 200mm HAS BEEN ASSUMED IN ACCORDANCE WITH STRUCTURAL CONSULTANT ADVICE.
- CONTRACTOR TO CONFIRM BULK EARTHWORK VOLUMES AND LEVELS BASED ON THEIR ASSUMPTIONS AND CALCULATIONS. ALL TEMPORARY CONSTRUCTION BATTERS TO BE A MAX SLOPE OF 1.5:1
- NO CUT AND FILL FACTORS ARE INCLUDED IN THESE VOLUMES.
- ALLOW A +/- 15% TOLERANCE FOR THESE VOLUMES.
- VOLUMES DO NOT INCLUDE BULKING FACTORS.

CUT 87,198 m³
FILL 1,829 m³
NET 85,369 m³ (CUT TO SPOIL)

INSERT

BASEMENT
LEVEL 2
FFL 3.19
BEL 2.99

CARPARK
(230 SPACES)
RL 3.190

PLANT ROOM

RAMP TO B2

BASEMENT
LEVEL 1
FFL 7.19
BEL 6.99

PLANT

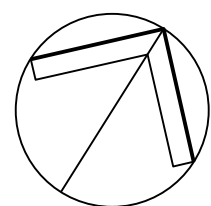
MANAGEMENT

DIESEL BULK
STORE

SUB-STATION
DECANT PLATFORM

LIFT PIT

SEDIMENT BASIN



NORTH

5.0 0 2.5 5.0 7.5 10.0m
PLAN SCALE 1:250 A1 SHEET

REVISION	AMENDMENT	DATE	REVISION	AMENDMENT	DATE
1	DEVELOPMENT APPLICATION	31/07/20			

CLIENT
 **Health Infrastructure**

PROJECT
GOSFORD NORTHSIDE PRIVATE HOSPITAL

PREPARED BY
 **Warren Smith & Partners**
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Consulting Engineers
■ Hydraulic ■ Fire ■ Civil ■ Utilities Infrastructure

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TITLE				
BULK EARTHWORKS PLAN				
SCALE 1:250 @ A1	DRAWN M.T.	DESIGNED M.D.	CHECKED L.S.	APPROVED L.S.
JOB No. 6422001	DRAWING No. C3.01		ISSUED 1	
DATE JULY 2020	STATUS SSDA			

PRELIMINARY - NOT FOR CONSTRUCTION

Levels Table				
No.	From Depth	To Depth	Colour	Depth Range Volume
1	-14.792	-8.000	Red	15998.63m
2	-8.000	-4.000	Orange	34380.75m
3	-4.000	-2.000	Yellow	17465.11m
4	-2.000	0.000	Light Green	19353.71m
5	0.000	2.000	Green	1264.31m
6	2.000	5.207	Dark Green	564.33m

- NOTES:**
- THIS IS A PRELIMINARY VOLUMES CALCULATION AND DOES NOT TAKE INTO ACCOUNT:
 - PIPE TRENCH EXCAVATIONS
 - STRUCTURAL BUILDING FOOTINGS/ EDGE BEAMS
 - RETAINING WALL BACKFILLING
 - ON-SITE DETENTION TANK
 - A PAVEMENT THICKNESS OF 330mm HAS BEEN ASSUMED FOR TRAFFICABLE AREAS AND 250mm FOR NON-TRAFFICABLE AREAS.
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 - VOLUMES DO NOT INCLUDE BULKING FACTORS.
- CUT 87,198 m³
FILL 1,829 m³
NET 85,369 m³ (CUT TO SPOIL)

LEGEND	
BEL 2.99	BULK EARTHWORKS LEVEL
FFL 3.19	FINISHED FLOOR LEVEL
95	NATURAL SURFACE CONTOUR
78.0	PROPOSED CONTOUR
20	DEPTH CONTOUR

FOR CONTINUATION REFER TO INSERT



INSERT



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NORTH

5.0 0 2.5 5.0 7.5 10.0m
PLAN SCALE 1:250 A1 SHEET

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1	DEVELOPMENT APPLICATION	31/07/20			

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NSW GOVERNMENT **Health Infrastructure**

PROJECT

GOSFORD NORTHSIDE PRIVATE HOSPITAL

PREPARED BY

Warren Smith & Partners

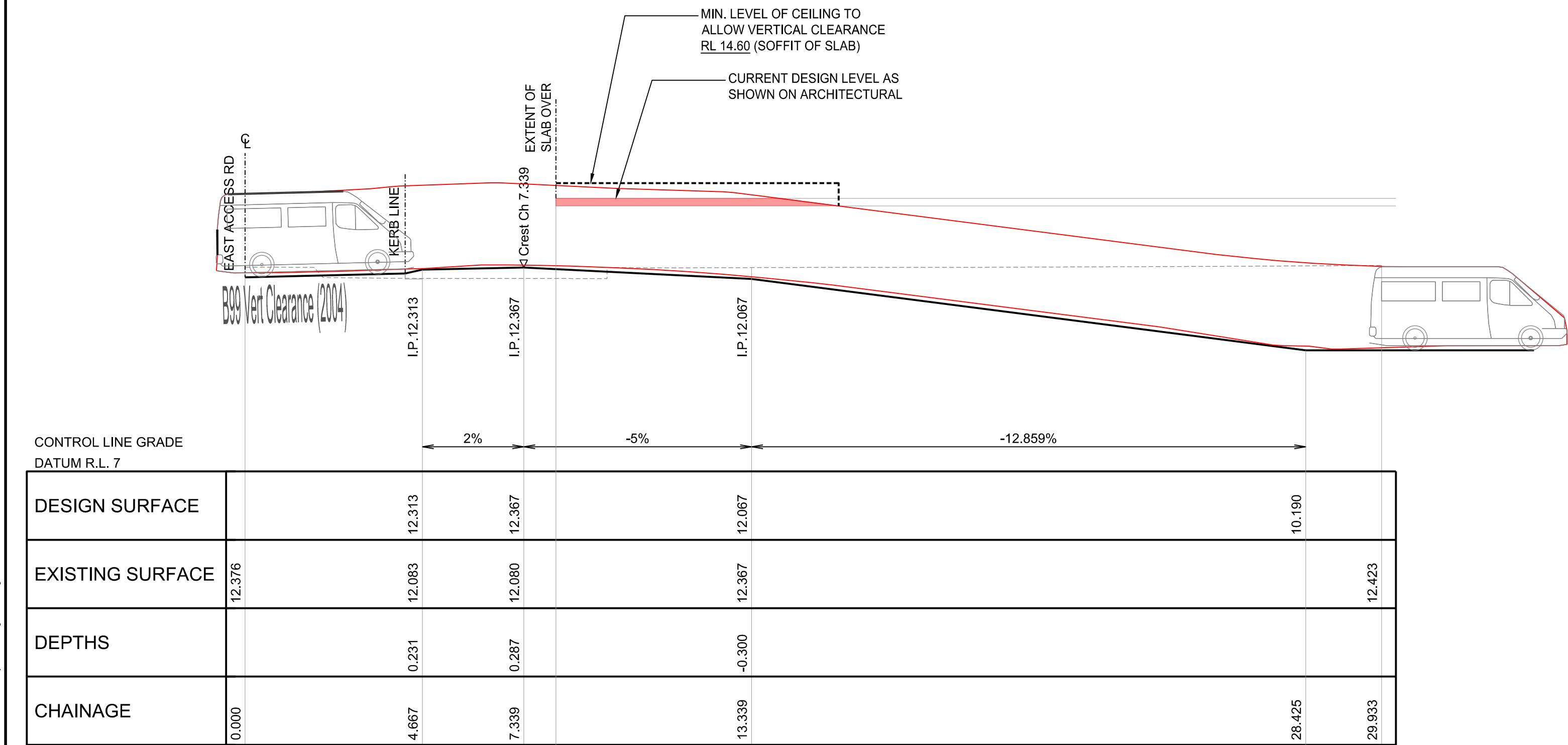
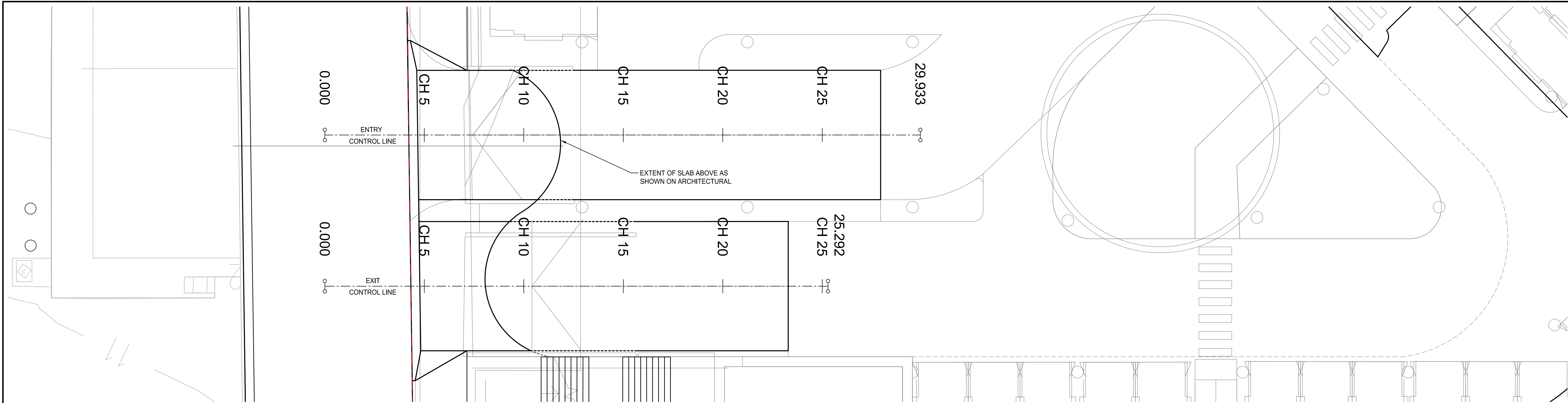
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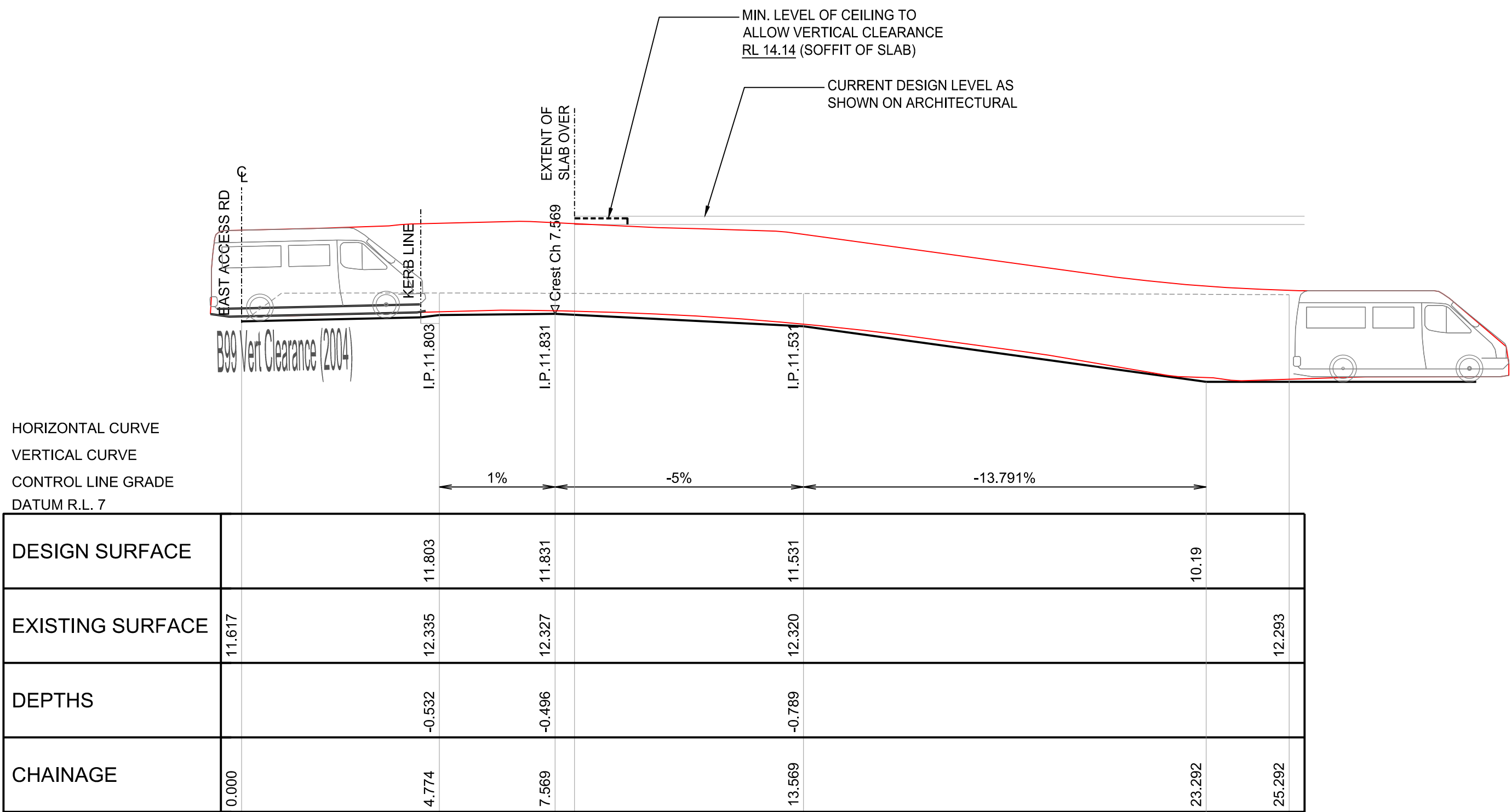
Hydraulic Fire Civil Utilities Infrastructure

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TITLE				
BULK EARTHWORKS CUT & FILL PLAN				
SCALE	DRAWN	DESIGNED	CHECKED	APPROVED
1:250 @ A1	M.T.	M.D.	L.S.	L.S.
JOB No.	DRAWING No.		ISSUE	
6422001	C3.02		1	
DATE	STATUS			
JULY 2020	SSDA			



LONGITUDINAL SECTION OF ENTRY
SCALE H1:100
SCALE V1:100



LONGITUDINAL SECTION OF EXIT
SCALE H1:100
SCALE V1:100

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1	DEVELOPMENT APPLICATION	31/07/20			

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GOSFORD NORTHSIDE PRIVATE HOSPITAL

PREPARED BY

Consulting Engineers

Hydraulic Fire Civil Utilities Infrastructure

TITLE				
DRIVEWAY PLAN AND LONGSECTIONS				
SCALE	DRAWN	DESIGNED	CHECKED	APPROVED
AS SHOWN	J.W.	J.W.	L.S.	L.S.
JOB No.	DRAWING No.		ISSUE	
6422001	C4.01		1	
DATE	STATUS			
JULY 2020	SSDA			

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Y:\6422001_TB_A\H Login Name: mdsz Cad File: T:\6422001\Drawings\AutoCAD\Design\CH\2D\C6.02 Stormwater Pit Schedule.dwg Plot Date: 31/07/2020@17:02:08

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1	DEVELOPMENT APPLICATION	31/07/20			

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PROJECT

GOSFORD NORTHSIDE PRIVATE HOSPITAL

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TITLE

STORMWATER PIT SCHEDULE

SCALE

AS SHOWN

DRAWN

M.T.

DESIGNED

M.D.

CHECKED

L.S.

APPROVED

L.S.

JOB No.

6422001

DRAWING No.

C6.02

ISSUE

1

DATE

JULY 2020

STATUS

SSDA

Name	Surface Elev. (m)	Pit Depth (m)	Pit Size and Type	From	To	Length (m)	U/S IL (m)	D/S IL (m)	Slope (%)	Pipe Type	Dia (mm)	No. Pipes
Pit 1/1	18.14	0.68	900SQ PIT, CLASS B GRATE	Pit 1/1	Pit 2/1	23.00	17.46	17.23	1.00	uPVC (SN8)	225	1
Pit 2/1	19.41	2.21	900SQ PIT, CLASS B COVER	Pit 2/1	Pit 3/1	24.04	17.20	16.96	1.00	uPVC (SN8)	225	1
Pit 3/1	18.02	1.17	900SQ PIT, CLASS B HEELSAFE GRATE	Pit 3/1	Pit 4/1	10.71	16.85	14.42	22.69	RCP (CLASS 3)	300	1
Pit 4/1	15.32	0.93	900SQ PIT, CLASS B HEELSAFE GRATE	Pit 4/1	Pit 5/1	14.62	14.39	13.59	5.47	RCP (CLASS 3)	300	1
Pit 5/1	14.49	0.93	600 x 900 PIT, CLASS D GRATE	Pit 5/1	Pit 6/1	29.71	13.56	12.99	1.92	RCP (CLASS 3)	300	1
Pit 6/1	13.89	0.93	1800 x 900 PIT WITH 450 x 900 CLASS D DOUBLE GRATE WITH 1.8m LINTEL OPENING	Pit 6/1	OSD Tank	30.63	12.96	12.08 dropping to 9.50. Refer to C6.05	2.87	RCP (CLASS 3)	300	1
Pit 1/2	9.90	0.83	1800 x 900 PIT WITH 450 x 900 CLASS D DOUBLE GRATE WITH 1.8m LINTEL OPENING	Pit 1/2	Pit 2/2	18.65	9.07	8.88	1.00	uPVC (SN8)	225	1
Pit 2/2	11.15	2.30	1800 x 900 PIT WITH 450 x 900 CLASS D DOUBLE GRATE WITH 1.8m LINTEL OPENING	Pit 1/2	Pit 3/2	18.65	8.85	8.66	1.00	uPVC (SN8)	225	1
Pit 3/2	12.42	3.79	1800 x 900 PIT WITH 450 x 900 CLASS D DOUBLE GRATE WITH 1.8m LINTEL OPENING	Pit 3/2	OSD Tank	8.31	8.63	8.55	1.00	uPVC (SN8)	225	1
Pit 1/3	13.51	0.67	900SQ PIT, CLASS B GRATE	Pit 1/3	Pit 2/3	33.14	12.84	11.95	2.67	uPVC (SN8)	225	1
Pit 2/3	12.63	0.71	900SQ PIT, CLASS B COVER	Pit 2/3	Pit 3/3	38.36	11.92	10.41	3.94	uPVC (SN8)	225	1
Pit 3/3	11.09	0.71	900SQ PIT, CLASS B COVER	Pit 3/3	Pit 4/3	26.34	10.38	9.55	3.15	uPVC (SN8)	225	1
Pit 4/3	10.23	0.71	900SQ PIT, CLASS B COVER	Pit 4/3	OSD Tank	46.00	9.52	8.20	2.87	uPVC (SN8)	225	1
				OSD	Pit 1/4	34.98	7.19	6.93	0.74	RCP (CLASS 3)	375	1
Pit 1/4	8.87	1.97	900SQ PIT, CLASS B COVER	Pit 1/4	EXISTING PIT	30.45	6.90	6.70	0.66	RCP (CLASS 3)	375	1
Pit 1/5	9.07	0.82	900SQ PIT WITH 450 x 900 CLASS D GRATE WITH 1.2m LINTEL OPENING	Pit 1/5	Pit 1/4	3.21	8.25	7.92	10.28	uPVC (SN8)	225	1

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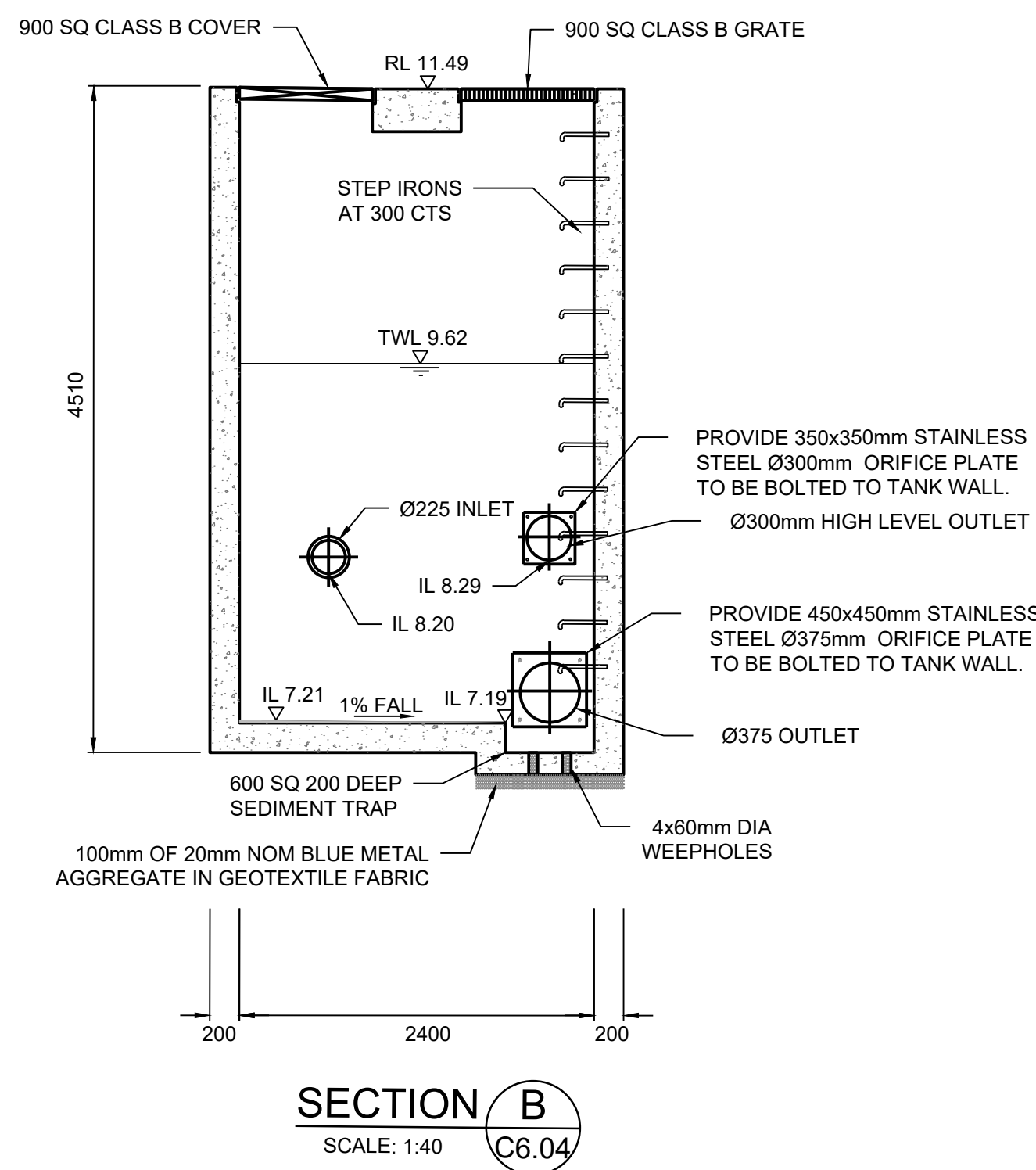
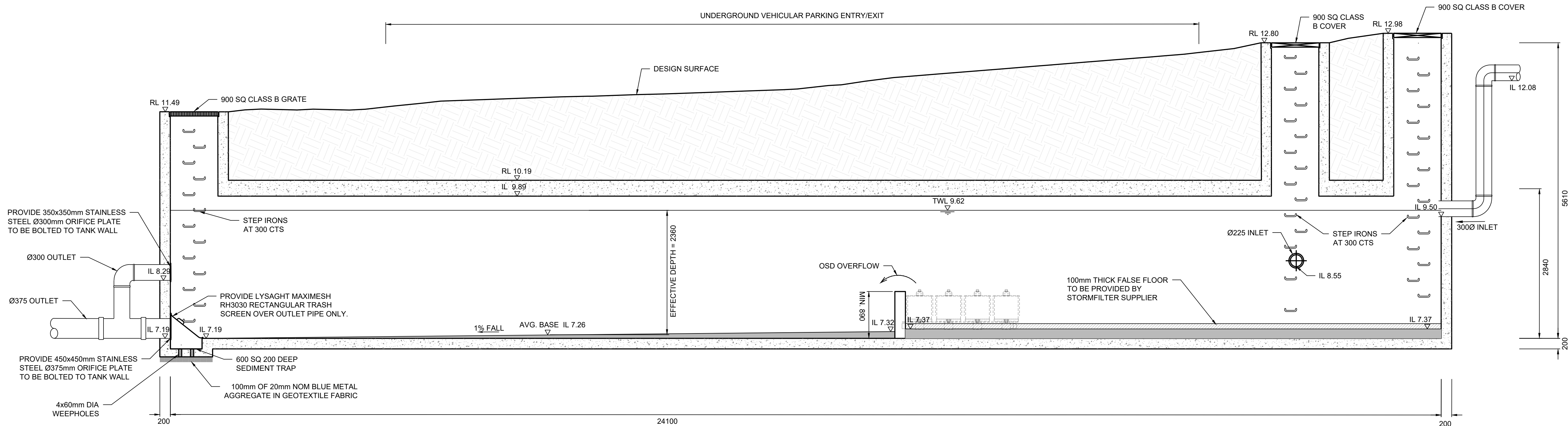


SCALE: 1:40

TITLE				
ON-SITE DETENTION (OSD) TANK PLAN				
SCALE	DRAWN	DESIGNED	CHECKED	APPROVED
AS SHOWN	M.T.	M.Cz.	L.S.	L.S.
JOB No.	DRAWING No.		ISSUE	
6422001		C6.04		1
DATE	STATUS			
JULY 2020	SSDA			

refs: X-6422001-TB_A1H	Cad File: T:\6422001\Drawings\AutoCAD\Civil\Design\Civil_2D\C6.04 OSD Planning
of Date: 31.07.2020@18:39:42	Login Name: mdiaz

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Plot Date: 31/07/2020@18:43:04
Caf File: T:\6422001\Drawings\AutoCAD\Civil\Design\CH\2D\06.04 OSD Plan.dwg
Login Name: mndaz



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REVISION	AMENDMENT	DATE	REVISION	AMENDMENT	DATE
1	DEVELOPMENT APPLICATION	31/07/20			

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 **Health Infrastructure**

PROJECT

GOSFORD NORTHSIDE PRIVATE HOSPITAL

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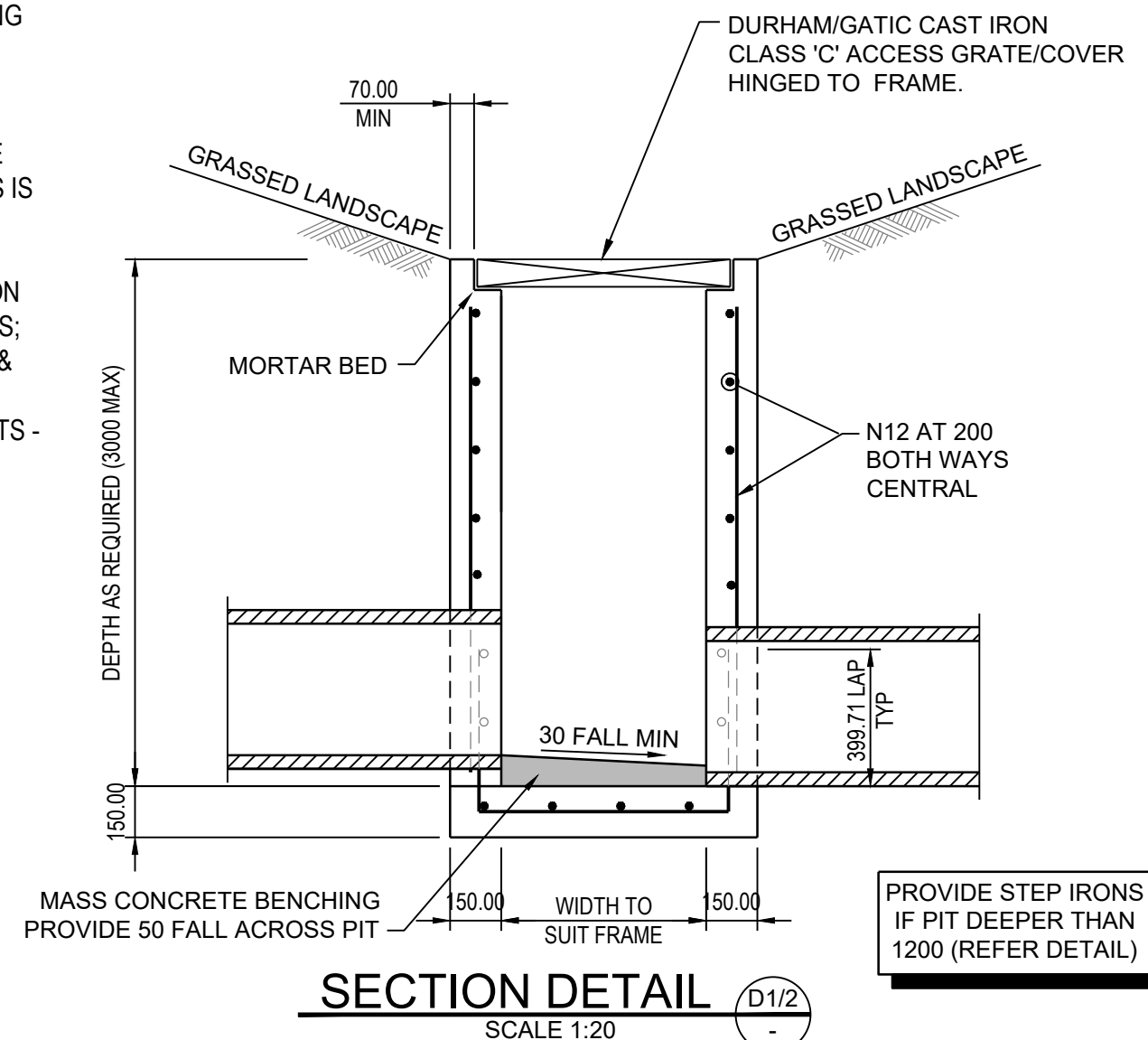
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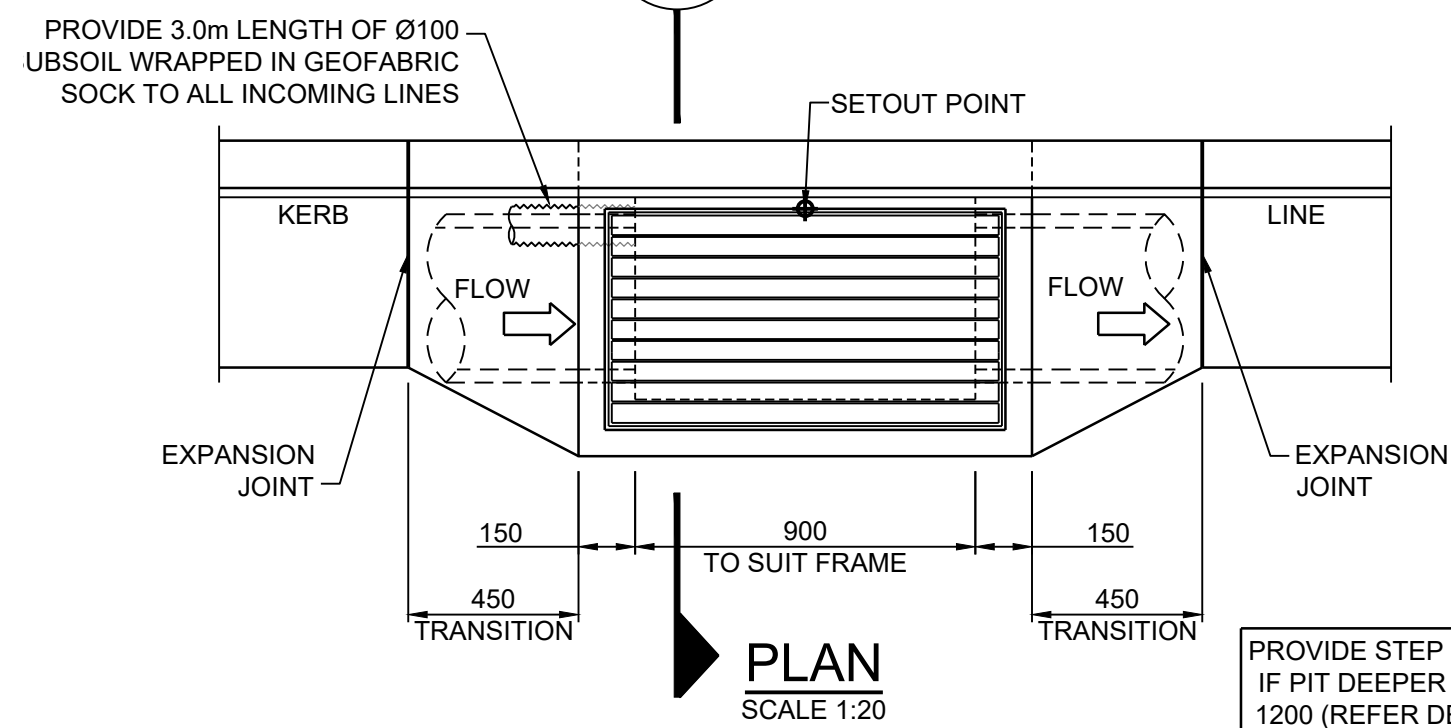
TITLE				
ON-SITE DETENTION (OSD) TANK SECTIONS				
SCALE	DRAWN	DESIGNED	CHECKED	APPROVED
AS SHOWN	M.T.	M.Cz	L.S.	L.S.
JOB No.	DRAWING No.		ISSUE	
6422001	C6.05		1	
DATE	STATUS			
JULY 2020	SSDA			



1. TRENCH WIDTH MAY NEED TO BE INCREASED SUBJECT TO ACHIEVING ADEQUATE COMPACTION.
2. MINIMUM PIPE COVER NOT UNDER ROADS = 450mm UNO.
3. THE CONTRACTOR SHALL ENSURE THAT THE SHORING OF TRENCHES IS INSTALLED AS REQUIRED BY STATUTORY REQUIREMENTS.
4. ENSURE BACKFILLING COMPACTION MEETS THE RELEVANT STANDARDS;
 - A) TRENCHES UNDER PAVED AREAS & BUILDINGS - 100% SMDD.
 - B) TRENCHES NOT UNDER PAVEMENTS - 95% SMDD.

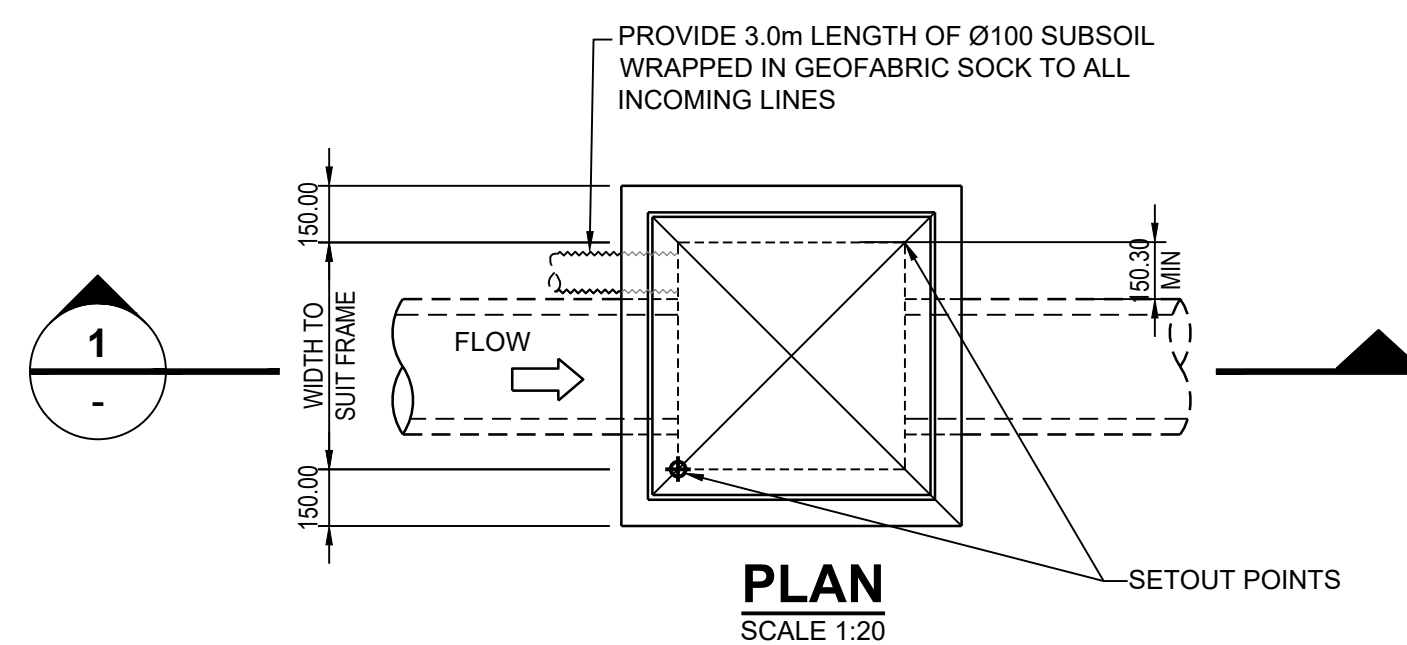


PROVIDE STEP IRONS
IF PIT DEEPER THAN
1200 (REFER DETAIL)



PROVIDE STEP IRONS
IF PIT DEEPER THAN
1200 (REFER DETAIL)

- STORMWATER GRATES LOCATED ADJACENT CONCRETE PAVEMENTS SHALL BE CAST IN-SITU WITH THE CONCRETE SLAB IN ACCORDANCE WITH THE GRATE MANUFACTURER'S SPECIFICATIONS.



Technical drawing of a 300mm diameter manhole with a 150mm diameter outlet. The drawing shows a cross-section of the manhole structure, including the concrete surround, the manhole body, and the outlet pipe. The outlet pipe is labeled "MIN. Ø100mm uPVC (Sf8) SEWER GRADE WITH SOLVENT WELDED JOINTS FOR BELOW GROUND DRAINAGE". The manhole body is labeled "300 x 300 x 150 MASS CONCRETE SURROUND". The top of the manhole is labeled "LIGHT DUTY CAST IRON OR BRASS SCREW OR BOLT DOWN CAP". The outlet pipe is shown with a 90-degree bend and a vertical section. A dimension line indicates a height of 600mm from the ground level to the top of the manhole body. An arrow labeled "FLOW" indicates the direction of flow into the manhole.

The figure consists of two parts: a sectional view and a plan view. The sectional view shows a rectangular pit with a width of 1.2 m and a depth of 1.2 m. The pit is shown in cross-section, with the top slab and bottom slab both having a width of 1.2 m. The plan view shows the pit from above, with a width of 1.2 m and a depth of 1.2 m. The pit is shown in plan, with the top slab and bottom slab both having a width of 1.2 m.

Technical drawing of a rectangular duct showing three views: Side Elevation, Plan, and Front Elevation.

SIDE ELEVATION: Shows the duct's profile with a total width of 125mm, a top flange width of 105mm, and a height of 25mm. The duct is labeled as Ø20 GALVANISED MILD STEEL.

PLAN: Shows the duct's top view with a width of 230mm.

FRONT ELEVATION: Shows the duct's front view, highlighting the rounded ends.

The diagram illustrates a cross-section of a typical pipe trench. The trench is defined by a top surface level and a bottom bed zone. The trench width is specified as $\text{TRENCH WIDTH} = \text{O.D.} + 300\text{mm}$. The vertical dimensions are as follows: a 300.00 unit layer of backfill above the pipe, a 0.30 O.D. unit overlay zone of compacted ordinary fill surrounding the pipe, and a 100.00 unit bed zone at the bottom. The pipe itself is a reinforced concrete pipe with an outside diameter of O.D. The layers are labeled: SURFACE LEVEL, BACKFILL, OVERLAY ZONE. COMPACTED ORDINARY FILL (MAX. 75mm), REINFORCED CONCRETE PIPE (OUTSIDE DIAMETER = O.D.), HAUNCH ZONE AS PER SPECIFICATION NOTES, and BED ZONE AS PER SPECIFICATION NOTES. The diagram is noted as NOT TO SCALE.

GENERAL AREAS

TYPICAL PIPE TRENCH

NOT TO SCALE

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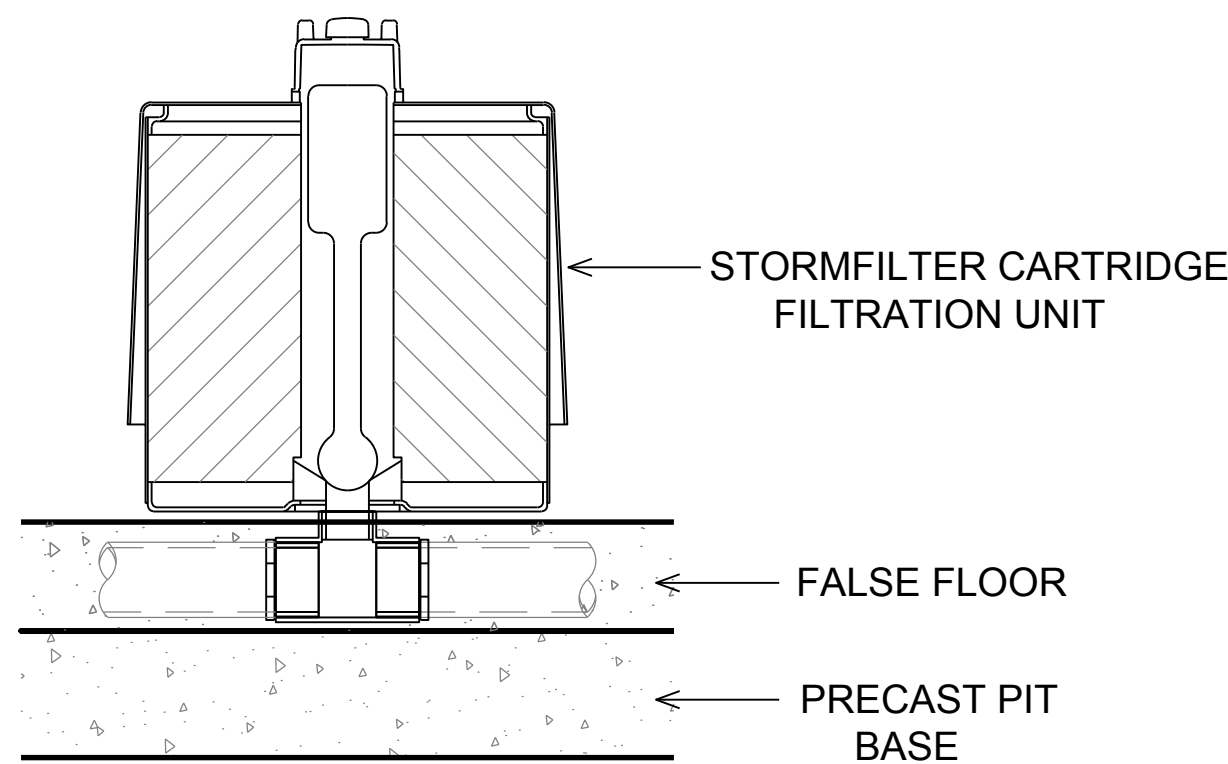
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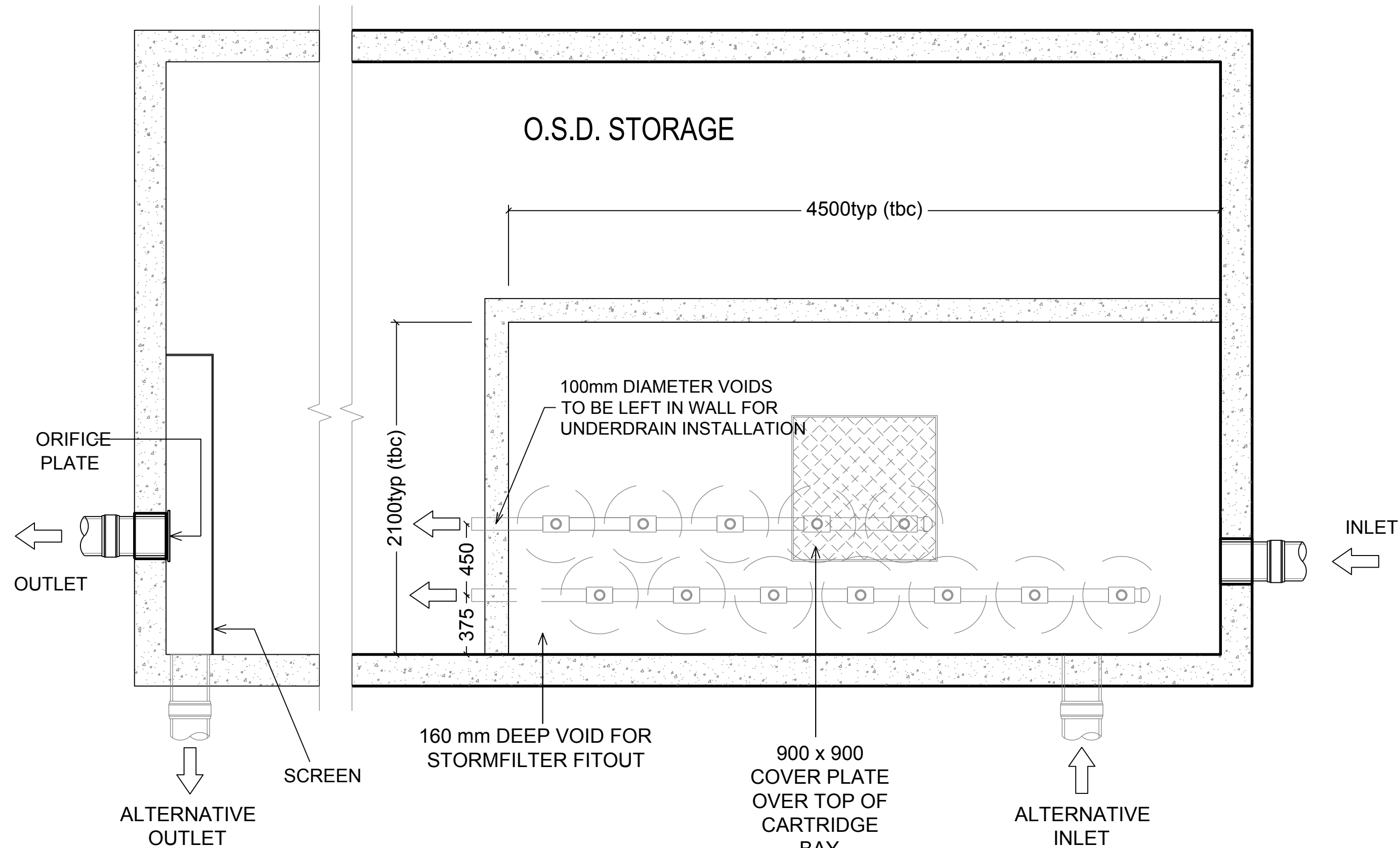
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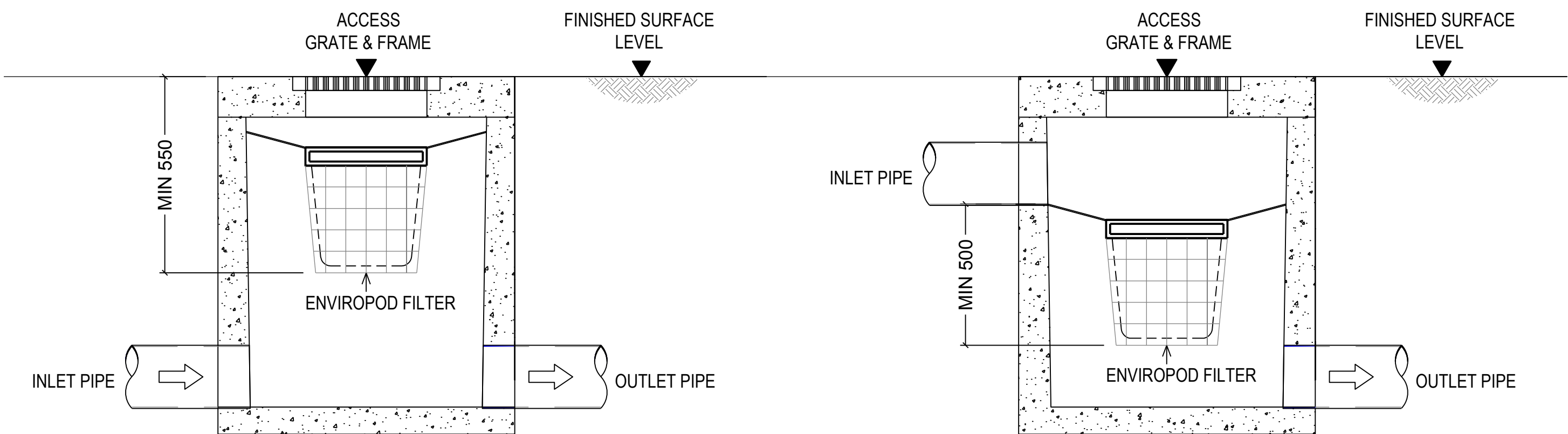
TITLE				
STORMWATER DRAINAGE DETAILS				
SCALE	DRAWN	DESIGNED	CHECKED	APPROVED
AS SHOWN	M.T.	M.C.Z.	L.S.	L.S.
JOB No.		DRAWING No.		ISSUE
6422001		C6.06		1
DATE	STATUS			
JULY 2020	SSDA			



STORMFILTER
CARTRIDGE DETAIL

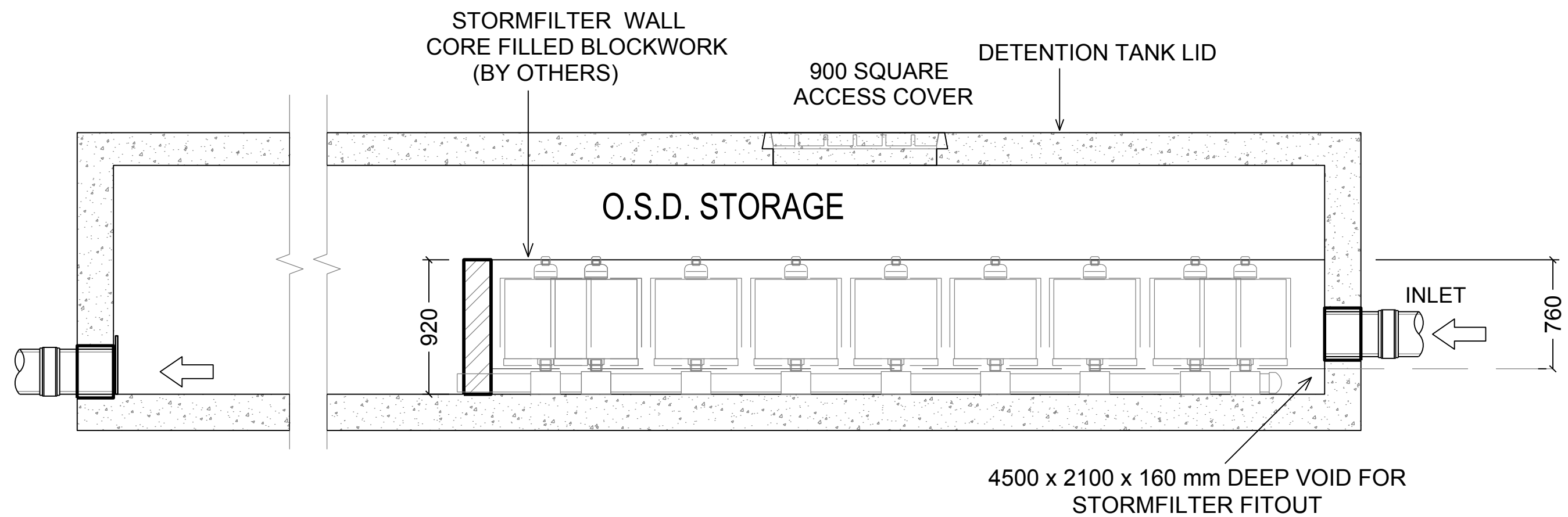


PLAN LAYOUT



STANDARD ENVIROPOD CONFIGURATION
SECTION

DROP PIPE ENVIROPOD CONFIGURATION
SECTION



SECTION

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1	DEVELOPMENT APPLICATION	31/07/20			



Health
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GOSFORD NORTHSIDE
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TITLE				
OCEAN PROTECT TREATMENT SYSTEM STANDARD DETAILS				
SCALE AS SHOWN	DRAWN M.T.	DESIGNED M.Cz	CHECKED L.S.	APPROVED L.S.
JOB No. 6422001		DRAWING No. C6.07		ISSUE 1
DATE JULY 2020	STATUS SSDA			