

Results of Geotechnical Investigation

Project Atlas Data Centre, 10 Roberts Road Eastern Creek and Offsite Enabling Infrastructure located at Lenore Drive

PSM5953-006R REV A - SSDA Submission 25 March 2026

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Rev	Date	Issue Description
A	11/02/2026	
B	17/03/2026	To include additional investigation
A – SSDA Submission	25/03/2026	SSDA submission, with updated site plan

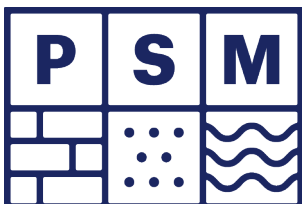


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Executive Summary

This report has been prepared by PSM to accompany a State Significant Development Application (SSDA) for the construction and ongoing operation of a data centre facility and associated Offsite Enabling Infrastructure (OEI) at 10 Roberts Road, Eastern Creek within in the Blacktown Local Government Area (“LGA”). The site is legally described as Lot 553 in Deposited Plan 1110447.

This report has been prepared to address the Secretary’s Environmental Assessment Requirements (SEARs) issued for the Project Atlas Data Centre (SSD - 101067971) date 16th January 2026.

The report has been updated to include the results of PSM additional investigation in January and March 2026.

1. Background

A State Significant Development Application (SSDA) has been prepared to support a data centre at 10 Roberts Road, Eastern Creek and associated offsite enabling infrastructure.

The proposal will include:

- Site preparation works including demolition, bulk excavation and removal of existing structures on the site, tree and vegetation clearing and bulk earthworks.
- Construction, fit-out and 24/7 operation of a Data Centre.
- Offsite enabling infrastructure.
- 2 Data Centre Buildings.
 - Building 1 – 2 Level + rooftop plant
 - Building 2 – 3 Level + rooftop plant
- Ancillary office, EOT and amenity.
- Provision of required utilities including:
 - diesel storage tanks.
 - water tanks.
 - substations on site.
- Vehicle access for 20m articulated vehicles (semi-trailers).
- Associated landscaping and site servicing.
- Installation of site services and drainage infrastructure.

This report has been prepared to address the Secretary’s Environmental Assessment Requirements (SEARs) and accompanying cover letter issued for the Project Atlas Data Centre (SSD-101067971) dated 16th January 2026.

Specifically, this report has been prepared to respond to the SEARs requirements issued below.

Item	Description of Requirement
Soil	An assessment of potential impacts on soil resources and riparian land on and near the site, including: <ul style="list-style-type: none">• Impacts on soil salinity• Details of earthworks, including cut and fill volumes
Water Management (Groundwater)	Provide an integrated water management plan for the development that includes: <ul style="list-style-type: none">• A description of groundwater and surface water conditions and all works/activities that may intercept, extract, use, divert or receive surface water and/or groundwater (both temporary and permanent)

The report presents the results of the geotechnical investigation for the proposed data centre development at 10 Roberts Road, Eastern Creek and Offsite Enabling Infrastructure located at Lenore Drive (‘the Site’). The works were undertaken in accordance with the PSM proposal (ref. PSM5953-001L REV 1 dated 07 October 2025).



2. Site Description

The proposed Data Centre is located at 10 Roberts Road, Eastern Creek, legally referred to as Lot 553 DP1110447 with Offsite Enabling Infrastructure (OEI) proposed across Lot 21 DP1246626, Lot 20 DP1157491, and Lot 22 DP1246626 to connect to TransGrid's Sydney West Substation & Switchyard (refer to Figure 1 below). The site is located on Country of the Dharug people within the local government area of Blacktown.

The Data Centre site has a land area of approximately 168,574m².

The site is identified as part of the Eastern Creek Precinct Stage 3 within the Western Sydney Employment Area. It is located approximately 35km of the Sydney Central Business District (CBD) and 5.1 km from Rooty Hill Train Station.

The site has a site area of approximately 17 hectares and is bounded by industrial development and has a primary frontage of 400m to Roberts Road. The existing site contains an industrial warehouse and surrounding hard-stand areas.

Surrounding land uses in the vicinity include:

- Northeast: A range of mixed-use industry & commercial buildings
- Southeast: Data Centre and substation
- South, southwest and west: land primarily owned and operated by TransGrid Sydney
- Northwest: Commercial industry and the Industrial Harvest café.

The site is zoned IN1 General Industrial within Chapter 2 Western Sydney Employment Area of State environmental Planning Policy (Industry and Employment) 2021 (IESEPP). The proposal is permissible with development consent in the IN1 zone as per the State Environmental Planning Policy (Transport and Infrastructure) 2021 (TISEPP).

A summary of key features for the site and surrounding context are also summarised in Table 1 below.

Table 1 – Summary of Key Features

Item	Description
Site Area	Data Centre - 168,574m ²
Ownership	Goodman
Legal Description	Data Centre - Lot 553 DP1110447 OEI - Lot 21 DP1246626, Lot 20 DP1157491, and Lot 22 DP1246626



Inset 1: Site Aerial

3. Desktop Study

3.1 Historical Aerial Imagery

PSM has reviewed available historical aerial imagery of the site sourced from the NSW Historical Aerial Imagery Database and Nearmap. Following the review, we understand the following:

- Prior to 1947, the site and surrounding areas were predominantly agricultural land consisting of grassed and vegetated areas. A basin was observed at the south-western portion of the current warehouse site.
- Between 1947 to 1991, the site appeared to remain relatively unchanged. However, construction activity commenced in the surrounding areas, including roads at the Transgrid site, earthworks commenced surrounding the current warehouse site and backfilling of the basin observed in the south-western area of the warehouse site.
- Between 1991 to 2005, warehouses were in construction at the southern end of the current warehouse site. The earthworks from the surrounding area have fully extended to the current warehouse site. Additional internal roads are constructed in the Transgrid site.
- Between 2005 to 2009, warehouses and paved car parks had been constructed on the current warehouse site. There is a further continuation of industrial developments in the surrounding vicinity of the Site.
- Between 2009 to 2014, the site remained relatively unchanged, further industrial developments were completed in the adjacent surrounding areas.
- Between 2014 to 2021, additional high voltage transmission towers are constructed within the Transgrid site, the rest of the site remained relatively unchanged.
- Between 2021 to presents, the overall configuration of the site, warehouse and hardstand areas remains relatively unchanged from the aerial taken in 2021 to the present day in 2025 with minimal observable changes to the surrounding landscape areas.

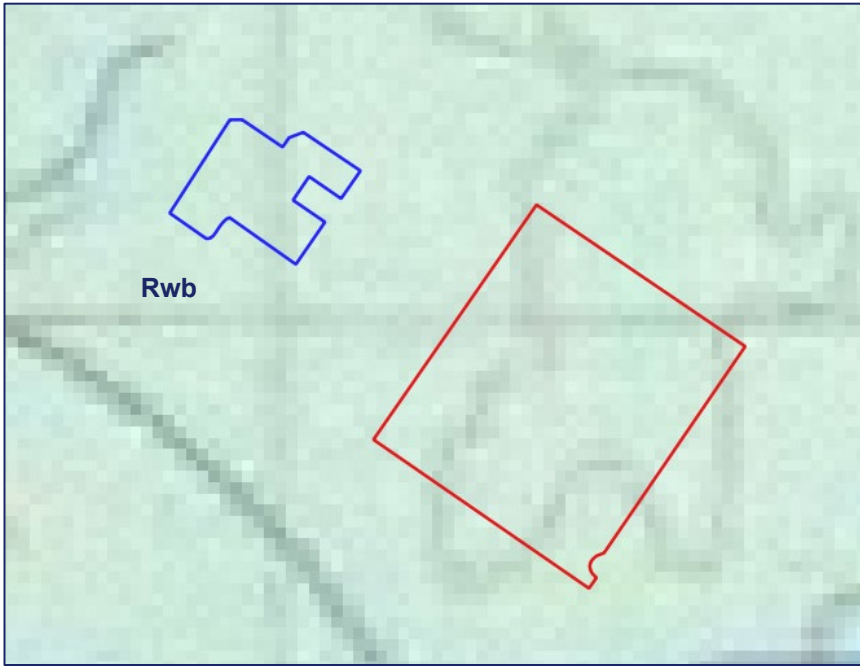
Appendix A presents a series of historical aerial imagery of the Site.

3.2 Geological Maps

The 1:10 000 Penrith Geological Map indicated the Site is underlain by:

- Bringelly Shale (Rwb) which comprise – Shale, carbonaceous claystone, laminate, coal in parts.

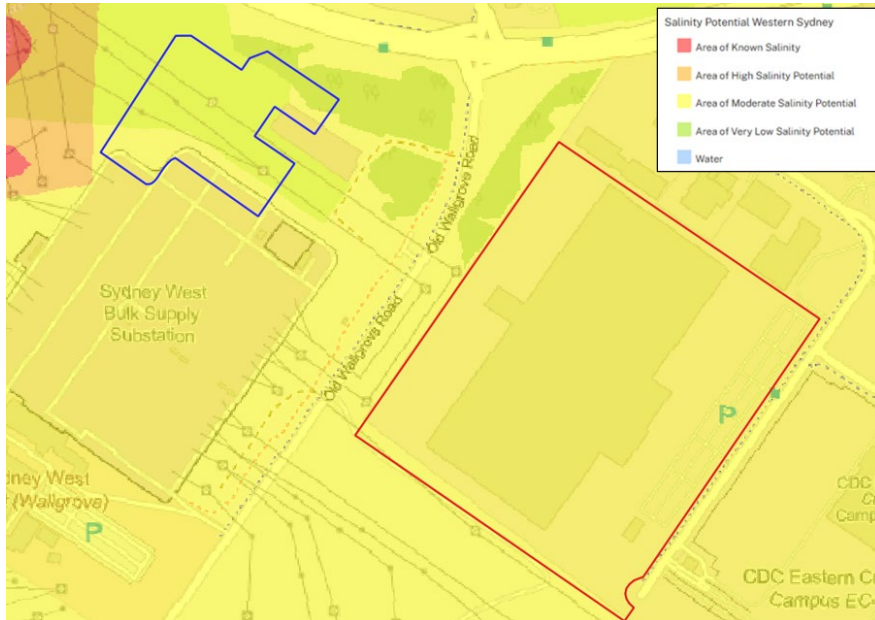
Inset 2 below presents the boundary of the site with respect to the geological settings.



Inset 2: Snippet of 1:100,000 Penrith Geological Map (approximate extent of the Site outline in red and blue)

3.3 Soil Salinity

Based on the Western Sydney Salinity Potential Map (2002), the Site falls within the area of moderate salinity potential, as indicated in Inset 3.



Inset 3: Western Sydney Salinity Potential Map (2002) on datasets.seed.nsw.gov.au approximate extent of the site outline in red and blue)

3.4 Acid Sulphate Soils

The Site is not located with the areas covered by the Acid Sulphate Soil Risk Map (v2.5.1) on datasets.seed.nsw.gov.au, as indicated in Inset 4.

Appendix C presents the results of point load test.

4.2 Fieldwork – January to March 2026

Additional geotechnical investigation was undertaken during two separate periods:

- 29 January 2026 to 30 January 2026 (2 days)
- 2 March 2026 to 5 March 2026 (4 days).

The fieldwork included the following:

- Two (2) non-cored boreholes of depths between 2.2 and 10 m
- Seven (7) cored boreholes of depths between 8.8 and 20.1 m.

The same approach in fieldwork (e.g. service locator, SPT, reinstatement etc) has been undertaken as per above. Note that no soil samples were collected on this round of investigation.

Figure 1 presents the locations of these additional boreholes.

4.3 Laboratory Testing – October 2025

4.3.1 California Bearing Ratio

PSM recovered three (3) bulk soil samples for CBR testing. The following sample preparation was undertaken prior to CBR testing:

- Compact to 98% Standard MDD, at optimum moisture content (OMC)
- Four (4) day-soaked sample
- 4.5 kg surcharge.

Table 2 presents a summary of the CBR test results. The laboratory test report is included in Appendix D.

Table 2 – CBR Test Results

Sample Location and Depth ⁽¹⁾ (m)	Sampled Date	Inferred Geotechnical Unit	Soaked CBR (%)	Optimum Moisture Content (%)	Standard Maximum Dry Density (t/m ³)	Swell (%)
BH10 (0.2 – 0.8 m)	23/10/2025	NATURAL SOIL	4.0*	17.8	1.68	2.0
BH03 (1.0 – 1.5)	31/10/2025	FILL	3.0**	19.5	1.69	1.5
BH12 (1.0 – 2.0)		FILL / NATURAL SOIL (CLAY)	1.5*	18.6	1.73	2.5
Notes:						
(1) m bgl – meters below existing ground level.						
(2) * = Soaked CBR value measured at 2.5 mm penetration						
(3) ** = Soaked CBR value measured at 5.0 mm penetration						

4.3.2 Aggressivity and Salinity Testing

Six (6) disturbed soil samples were retrieved for aggressivity and salinity testing at a NATA accredited laboratory. The following tests were undertaken:

- Cation Exchange Capacity (CEC)
- Exchange sodium percentage
- Salinity (EC 1:5, one part soil to five parts water)
- Soil pH



- Chlorides
- Sulphates
- Resistivity
- Moisture content.

Table 3 presents a summary of the results and location of the samples. The laboratory results sheets are presented in Appendix E.



Table 3 – Aggressivity Test Results

Sample ID (Depth)	Date	Inferred Geotechnical Unit	pH	Electrical Conductivity [μ S/cm]	Resistivity [ohm.cm]	Moisture Content [%]	Chloride by discrete analyser [mg/kg]	Soluble Sulfate by ICPAES [mg/kg]	Exchangeable Cations on Alkaline Soils (ED006)		Exchangeable Cations (ED008)	
									CEC [meq/100g]	ESP [%]	CEC [meq/100g]	ESP [%]
BH07 (1.0 – 1.5 m)	23/10/2025	NATURAL SOIL	5.7	356	2810	14.4	400	170	-	-	4.0	19.5
BH08 (0.5 – 1.0 m)		NATURAL SOIL	6.6	718	1390	14.4	1310	110	-	-	8.2	23.2
BH03 (0.5 – 1.0 m)	31/10/2025	FILL	9.4	299	3340	15.5	110	80	12.9	21.9	-	-
BH03 (1.5 – 1.95 m)		FILL/NATURAL SOIL (CLAY)	5.1	364	2750	16.3	440	200	-	-	5.9	23.4
BH05 (1.0 – 1.5 m)		FILL	6.3	315	3170	16.2	440	20	-	-	6.1	15.2
BH12 (0.5 – 0.95 m)		FILL	8.0	209	4780	17.0	150	160	11.3	25.5	-	-



5. Site Conditions

5.1 Surface Conditions

The Transgrid site consists of grassed and vegetated areas with a few gravel roads and high voltage transmission towers at the centre portion of the site.

The Warehouse site is currently occupied by a single storey warehouse located on the southern west portion of the site. Surrounding the warehouse are the following areas:

- Vehicular access and turning paths on concrete hardstand
- Laydown areas for pallets, truck containers, truck parking and other miscellaneous items
- A carpark area located at the western portion of the site.

Concrete within the hardstand presents on the site is noted to be in a good condition with no major visible cracks of the existing hardstand surfaces on site.

5.2 Inferred Subsurface Conditions

The subsurface conditions encountered within the boreholes are summarised in Table 4. Table 5 presents the inferred reduced level (RL) to the top of the inferred geotechnical units encountered. Note that for the non-cored boreholes, due to the limitations of the rotary auger drilling method using TC bit and disturbed nature of the retrieved samples, the low strength and medium strength rock units may not be differentiated.

Table 4 – Summary of Inferred Geotechnical Unit encountered in PSM boreholes

Unit Name	Approximate Depth to Top of Unit (m)	Description
CONCRETE / ASPHALT	0	CONCRETE: 160 to 450 mm thick. ASPHALT: 150 to 200 mm thick.
TOPSOIL	0	Silty CLAY: brown, low plasticity, rootlets observed, firm.
EXISTING FILL	0.2 – 0.5	Sandy CLAY to Sandy Gravelly CLAY: brown, dark grey and pale grey mottled red brown, medium plasticity, sand fine to medium grained, gravel subangular up to 5 mm, firm to stiff. Gravelly CLAY to CLAY with gravel: brown and dark grey, medium plasticity, gravel is of shale and sandstone origin, sub-angular up to 40 mm, stiff. Gravelly SAND to Gravelly Clayey SAND: grey and brown, fine grained, gravel subangular up to 25mm, some plastic observed.
NATURAL SOIL	0.2 – 1.7	CLAY to CLAY with gravel: red brown, pale grey and yellow brown, medium plasticity, grave fine-grained sub-angular up to 20 mm, stiff to very stiff.
BEDROCK A	0.2 – 7.6	SHALE (Extremely weathered to highly weathered SHALE): pale grey and brown, very low to low strength. SHALE: red brown, pale grey and dark grey, massive to thinly bedded, typically distinct rock fabric, extremely weathered to moderately weathered, typically very low to low strength, some clay seams and iron staining throughout, defect spacing typically less than 60 mm.

Unit Name	Approximate Depth to Top of Unit (m)	Description
		INTERBEDDED SANDSTONE AND SILTSTONE: brown, 50% SANDSTONE, 50% SILTSTONE, fine to medium grained, developed, laminated to thinly bedded, very low to low strength, iron stained.
BEDROCK B	2.2 – 14.2	<p>SHALE: dark grey and brown, thinly laminated to thinly bedded, developed, highly to slightly weathered, low to medium strength, minor iron staining, defect spacing typically less than 200 mm.</p> <p>SANDSTONE: grey to dark grey, medium grained, developed to well developed, thinly laminated to thinly bedded, occasionally massive, moderately weathered to fresh, low to medium strength, some SILTSTONE laminations and clasts, minor iron staining, defect spacing typically less than 200 mm.</p> <p>INTERBEDDED SANDSTONE AND SILTSTONE: pale grey to dark grey, 20-70% SANDSTONE, 30-80% SILTSTONE, slightly weathered to fresh, low to medium strength, defect spacing typically less than 200 mm.</p>
BEDROCK C	9.0 – 16.5	<p>SHALE: dark grey, developed, laminated, slightly weathered to fresh, medium to high strength, defect spacing typically larger than 200mm.</p> <p>SANDSTONE: dark grey, fine to medium grained, well developed, laminated to thinly bedded, slightly weathered to fresh, medium to high strength, defect spacing typically larger than 200mm.</p> <p>INTERBEDDED SANDSTONE AND SILTSTONE: pale grey to dark grey, 20-70% SANDSTONE, 30-80% SILTSTONE, developed, laminated to thinly bedded, medium to high strength, defect spacing typically larger than 200mm.</p>



Table 5 – RL’s of Top of Inferred Geotechnical Units Encountered

LOCATION	Borehole ID	Borehole Collar Level (m AHD)	RL at Top of Unit (m AHD)							
			CONCRETE/ ASPHALT	TOPSOIL	EXISTING FILL	NATURAL SOIL	BEDROCK A	BEDROCK B	BEDROCK C	EOH
WAREHOUSE SITE	October 2025									
	BH03	79.5	79.5	NE	79.2	77.8	76.2	75.9 ⁽³⁾	NE	75.9
	BH05*	79.9	79.9	NE	79.4	78.2	77.2	73.9	NE	71.6
	BH12	80.7	80.7	NE	80.3	79.1	75.2	NE	NE	74.7
	January & March 2026									
	BH-A*	79.7	79.7	NE	79.5	78.7	77.7	76.5	65.4	62.3
	BH-B*	79.4	79.4	NE	NE	NE	79.3	76.1	71.7	70.6
	BH-C*	80.4	80.4	NE	80.1	NE	79.9	72.8	71.4	70.4
	BH-D*	79.2	79.2	NE	78.8	78.2	73.0	66.4	63.2	59.1
	BH-E	80.3	80.3	NE	79.9	78.8	73.3	NE	NE	70.3
	BH-F*	80.7	80.7	NE	80.4	79.1	73.1	66.5	64.2	63.7
	BH-G*	77.2	77.2	NE	77.0	76.8	74.2	NE	66.2	62.2
	BH-H	78.9	78.9	NE	78.7	NE	78.2	76.8 ⁽³⁾	NE	76.8
	BH-I*	77.8	77.8	NE	77.7	77.5	76.9	74.9	70.3	63.8



LOCATION	Borehole ID	Borehole Collar Level (m AHD)	RL at Top of Unit (m AHD)							
			CONCRETE/ ASPHALT	TOPSOIL	EXISTING FILL	NATURAL SOIL	BEDROCK A	BEDROCK B	BEDROCK C	EOH
TRANSGRID SITE	BH07	68.0	NE	68.0	67.8	67.0	65.8	NE	NE	62.0
	BH08	66.9	NE	66.9	66.7	66.4	64.5	NE	NE	60.9
	BH10	69.9	NE	69.9	NE	69.7	69.1	NE	NE	62.9

- Notes:
- (1) EOH = End of Hole
 - (2) NE = Not Encountered
 - (3) Top of BEDROCK B was inferred by at TC bit refusal.
 - (4) * = Cored borehole



5.3 Groundwater

During the site investigation, groundwater seepage was only observed in BH10, Table 6 provides a summary of the observed groundwater.

No piezometer has been installed. No long-term groundwater monitoring was undertaken for the Site.

Table 6 – Observed groundwater seepage during drilling

BH ID	Date	Collar RL (m AHD)	Depth of groundwater seepage (m)	RL of groundwater seepage (m AHD)
BH10	31/10/2025	69.9	6.0	63.9

6. Salinity and Aggressivity Assessment

6.1 Soil Chemistry

The laboratory test results summarised in Table 3 indicates the following:

- pH of the soil samples analysed was in the range of 5.1 to 9.4, with an average of 6.9.
- The 1:5 soil to water extraction and subsequent electrical conductivity (EC_{1:5}) of the soil samples analysed to be in the range of 209 µS/cm to 718 µS/cm.
- Concentrations of chlorides in samples analysed was in the range of 110 mg/kg to 1310 mg/kg.
- Concentrations of soluble sulphate in samples analysed was in the range of 20 mg/kg to 200 mg/kg.
- Cation Exchange Capacity (CEC) in samples analysed was in the range 4 meq/100g to 12.9 meq/100g.
- Exchange Sodium Percentage (ESP) in samples analysed was in the range of 15.2% to 25.5%.

6.2 Salinity

Site Investigations for Urban Salinity (DLWC 2002) classify soil salinity based on electrical conductivity (EC_e). The method of conversion from EC_{1:5} to EC_e (electrical conductivity of saturated extract) is based on DLWC (2002) and given by $EC_e = EC_{1:5} \times M$, where M is the multiplication factor based on “Soil Texture Group”.

The “Soil Texture Group” of the samples tested were assessed during our investigation. The salinity classification for the soil samples that were tested are presented in Table 7.

Table 7 – Salinity Classification

LOCATION	SAMPLE ID (Depth)	DATE	EC _{1:5}	SOIL TYPE	M	EC _e	SALINITY CLASS
			(dS/m)			(dS/m)	
Transgrid Site	BH07 (1.0 – 1.5 m)	23/10/2025	0.356	Light Medium Clay	8	2.85	Slightly Saline
	BH08 (0.5 – 1.0 m)		0.718	Light Medium Clay	8	5.74	Moderately Saline
Warehouse Site	BH03 (0.5 – 1.0 m)	31/10/2025	0.299	Light Clays	8.5	2.54	Slightly Saline
	BH03 (1.5 – 1.95 m)		0.364	Light Medium Clay	8	2.91	Slightly Saline
	BH05 (1.0 – 1.5 m)		0.315	Light Clays	8.5	2.68	Slightly Saline
	BH12		0.209	Light Clays	8.5	1.78	Non-Saline



LOCATION	SAMPLE ID (Depth)	DATE	EC1:5	SOIL TYPE	M	EC _e	SALINITY CLASS
			(dS/m)			(dS/m)	
	(0.5 – 0.95 m)						

It is assessed that the soils on site are classified as “Non-Saline to Moderately Saline”.

6.3 Aggressivity / Corrosivity

Table 4.8.1 of AS3600-2018 “Concrete Structures” provides criteria for exposure classification for concrete in sulphate soils based on sulphates in soil and groundwater, and pH of soil. On the basis of the sulphate and pH testing completed we assess the exposure classification for concrete in sulphate soils to be “A1” to “A2”.

Table 6.4.2(C) of Australian Standard AS2159:2009, Piling – Design and Installation provides criteria for exposure classification for concrete piles based on sulfates in the soil and groundwater, soil and groundwater pH, and chlorides in groundwater. On the basis of the soil sulfates and pH testing completed we assess the exposure classification for concrete piles in the soil to be “Non-aggressive” to “Mild”.

Table 6.5.2(C) of Australian Standard AS2159:2009, Piling – Design and Installation provides criteria for exposure classification for steel piles based on resistivity, soil and groundwater pH, and chlorides in soil and groundwater. On the basis of the soil chlorides and pH testing completed we assess the exposure classification for steel piles in the soil to be “Non-aggressive” to “Mild”.

6.4 Sodidity

Sodidity provides a measure of the likely soil dispersion on wetting. Soil sodidity is classified based on the Exchangeable Sodium Percentage (ESP) which is the amount of exchangeable sodium as a percentage of the Cation Exchange Capacity (DLWC, 2002).

The Exchangeable Sodium Percentages calculated from these laboratory results, ranging from 15.2% to 25.5%, indicates that the soils on Site are highly sodic when compared to criteria listed in DLWC (2002).

Table 8 – Jar Sample Soil Sodidity Results

LOCATION	SAMPLE ID (Depth)	ESP (%)	Rating (DLCW, 2002)
TRANSGRID SITE	BH07 (1.0 – 1.5 m)	19.5	Highly Sodic
	BH08 (0.5 – 1.0 m)	23.2	Highly Sodic
WAREHOUSE SITE	BH03 (0.5 – 1.0 m)	21.9	Highly Sodic
	BH03 (1.5 – 1.95 m)	23.4	Highly Sodic
	BH05 (1.0 – 1.5 m)	15.2	Highly Sodic
	BH12 (0.5 – 0.95 m)	25.5	Highly Sodic

7. Potential Geotechnical Risk

The following geotechnical risks should be considered. Note that these do not include any risks relating to contamination or environmental concerns.

7.1 Buried Structures

There are existing buildings, warehouses, roads and carparks situated within the 10 Roberts Road. The warehouse will be demolished for the proposed development.

It is thus likely that the following structures are presented below ground:

- Structure foundations including concrete slabs and concrete footings. It is PSMs opinion that these structures present risks to the proposed development including:
 - Risk of “hard spots” resulting in differential settlements of overlying structures. If the structure is not removed, this risk can be mitigated by providing minimum cover over such structures. Specific advice should be sought. In our experience minimum covers of 0.8 m are required to mitigate their effect on overlying structures.
 - Construction risks with regards to installation of new services, and shallow footings. That is, these could present as obstructions particularly.
 - Concrete slabs also reduce the ability of the underlying materials to be assessed as part of the site preparation works.
- Decommissioned services. A number of services including pipes of varying dimensions are likely to be present within the site. These present a low risk for future settlement caused by future collapse of the pipes, poor backfilling of pipe trenches and pits and preferential/uncontrolled transmittal of water. Review of historical photos, and subgrade preparation in accordance with the Specification can mitigate these risks to where they are acceptable. Water carrying services (e.g. potable water pipes, stormwater pipes, etc.) not intended to be used will need to be decommissioned either by removal or grouting. Grouting is recommended where these are located more than 2.0 m below ground level.

When selecting appropriate foundation systems particular consideration shall be given to the possible presence of such buried structures.

It should be noted that quantifying possible buried structures is difficult to achieve prior to the proposed development works and intensive investigations may be required. This site preparation will need to address the possible presence of buried slabs, footings, and services associated with historical land use.

Where footings or services are removed careful attention shall be given to limit the amount of ground that is disturbed and backfill the void in accordance with the Specification adjusted for the use of smaller compaction equipment. It would be prudent to backfill using material less susceptible to water and that can be compacted with smaller equipment. Sandstone VENM would be ideal material for completing this detailed backfilling.

7.2 Variable Depths to Top of BEDROCK Units

The investigation results of the Site show variable depths to the top of BEDROCK units. The range of depths to the top of BEDROCK units from the surface level is summarised:

- BEDROCK A – 0.16 m to 7.6 m
- BEDROCK B – 2.1 m to 14.2 m.
- BEDROCK C – 7.5 m to 16.5 m.

The designer should recognise this degree of variability in the planning, costing and design phases. The foundation conditions will need to be confirmed based on inspections during piling.

7.3 Localised Area of Deeper EXISTING FILL

EXISTING FILL which is considered as uncontrolled FILL was observed with highly variable thickness across the site. Localised areas of deeper EXISTING FILL may be encountered throughout the site (e.g., up to 1.7 m deep fill observed in BH03, BH05, BH-E and BH-F).

Based on PSM’s experience, EXISTING FILL can be managed through an effective bulk earthworks specification or be removed. We have prepared a bulk earthworks specification for the proposed development (see Appendix F). It is important that any earthworks on site are undertaken appropriately and in accordance with a proper bulk earthworks specification.

8. Discussion

8.1 General

The design advice in the following sections is provided on the basis that:



- The subsurface conditions are as those encountered in the geotechnical investigation reported in Section 4 of this report.
- The bulk earthworks will be undertaken in accordance with the cut/fill plan by BG&E (Ref. S25201-XX-CI-0210 REV A dated 2 February 2026).
- The earthworks (i.e., filling, cutting, testing, subgrade preparation, etc) will be undertaken in accordance with the PSM bulk earthworks specification PSM5953-007S attached in Appendix F.

If any of those bases are not applicable, PSM should be requested to confirm that the design advice below is still applicable.

8.2 Site Classification

We note that the proposed development (Data Centre) is outside the scope of AS2870-2011 “Residential slabs and footings”.

We recommend that structures which are within the scope of AS 2870 (2001) be designed for a site classification of Class “P”.

If all EXISTING FILL were to be treated or removed, structures within the scope of AS 2870 (2011) could be designed for a site classification of Class “H1” following reclassification. This includes consideration that any materials imported to the site will comprise typical VENM clay which based on PSM experience, could be highly reactive.

8.3 Site Sub Soil Classification

Throughout the Site, the depth to the top of BEDROCK varies from 0.16 to 7.6 m. We note, the proposed pad levels (bulk earthworks levels) of the warehouse site are approximately RL 80m.

In accordance with Clause 4.1 of AS1170.4-2007 (Earthquake actions in Australia):

- Where the depth to BEDROCK is:
 - Less than 3 m, a site can be classified as a Class B_e (rock) site and
 - Between 3 and 20 m, a site can be classified as a Class C_e (shallow soil) site.

There is generally a deeper soil profile at the warehouse site.

Based on the BEDROCK levels, we note that the buildings could span multiple span classifications. We note that for some structures a B classification can be more adverse than a C classification and given that particular regions of the Site are in between the boundary of both classifications, the designer should satisfy itself that its design is suitable for the borderline classification.

8.4 Permanent and Temporary Batters

The batter slope angles shown in Table 9 are recommended for the design of batters up to 2 m height and above groundwater: subject to the following recommendations:

1. The batters shall be protected from erosion.
2. Permanent batters shall be drained.
3. Temporary batters shall not be left unsupported for more than 1 month without further advice, and inspection by a geotechnical engineer should be undertaken following significant rain events.
4. Where loads are imposed or structures/services are located within one batter height of the crest of the batter, further advice should be sought.

Table 9 – Batter Slope Angles

Unit	Temporary	Permanent
EXISTING FILL / ENGINEERED FILL / NATURAL SOIL /	2.0H : 1.0V	2.5H : 1.0V
BEDROCK A	1.0H : 1.0V	2.0H : 1.0V
BEDROCK B & BEDROCK C	1.0H : 1.0V	1.0H : 1.0V



Steeper batters may be possible subject to further advice.

The batters should be inspected by an experienced geotechnical engineer or engineering geologist during excavation to confirm the batter advice provided and assess the need for localised support, such as rock bolting to control adverse jointing and mesh and/or shotcreting for overall face support or soft seams etc.

8.5 Excavation Support

Cuts in the Soil and Bedrock units steeper than the recommended permanent batter slopes in Table 9 will need to be supported by an appropriate retaining structure.

Any retaining structure or excavation support should be designed based on the following:

- Effective soil strength parameters in Table 10
- Water pressure (depending on the type of structure)
- Surcharge loads.

Note that design of retention systems may be based on either K_a or K_o earth pressures. Design using active earth pressures provides the minimum lateral earth pressure that must be supported to avoid failure and requires a wall that can rotate or translate to allow the pressures to reduce to these values (vertical and lateral movements up to 2% of height may occur, typical movements will be much less).

Where the design is based on K_o pressures, construction should be carefully controlled to avoid unwanted effects. It should be noted that designing for K_o pressures does not, of itself, ensure that movement does not occur. Movements are controlled by the construction method, especially sequence.

Both surface and sub-surface drainage needs to be designed and constructed properly to prevent pore water pressures from building up behind the retaining walls or appropriate water pressures must be included in the design.

8.6 Foundation

8.6.1 Shallow Footings

Pad footings may be proportioned on the basis of an allowable bearing pressure (ABP) for centric vertical loads provided in Table 10.

We note that an allowable bearing pressure (ABP) is not a soil property. It depends on many factors such as the size of the footings, the embedment depth, the load direction and eccentricity, the stiffness of the footing, the adopted factor of safety (FOS), the settlement limit, as well as the soil properties. As footings get bigger or deeper the capacity increases rapidly, as the load gains eccentricity or becomes inclined, the capacity reduces rapidly.

Settlements of shallow footings should be assessed and can be estimated using the elastic parameters provided in Table 10.

Table 10 – Engineering Parameters of Inferred Geotechnical Units

Inferred Unit	Bulk Unit Weight (kN/m ³)	Soil Effective Strength Parameters		Ultimate Bearing Pressure (UBP) under Vertical Centric Loading (kPa) ³	Allowable Bearing Pressure (ABP) under Vertical Centric Loading (kPa)	Ultimate Shaft Adhesion (kPa)	Elastic Parameters	
		c' (kPa)	φ' (deg)				Long Term Young's Modulus (MPa)	Poisson's Ratio
EXISTING FILL	18	0	27	285	100 ⁽¹⁾	N/A	8	0.3
ENGINEERED FILL / NATURAL SOIL	18	0	30	420	150 ⁽¹⁾	N/A	10	0.3
BEDROCK A	24	N/A	N/A	3,000	700 ^{(3),(4)}	50	200	0.2
BEDROCK B	24	N/A	N/A	12,000	2,000 ^{(3),(4)}	450	600	0.2
BEDROCK C	24	N/A	N/A	30,000	5,000 ^{(3),(4)}	800	900	0.2

- (1) Shallow footings (for ABP of 150 kPa) should have a minimum horizontal dimension of 1.0 m and an embedment depth of 0.5 m. Corresponds to FOS = 3. Existing Fill unit shall be inspected to assess the design parameters.
- (2) Ultimate values occur at large settlement (>5% of minimum footing dimensions).
- (3) End bearing pressure to cause settlement of <1% of minimum footing dimensions / pile dimension.
- (4) Assuming a sufficiently clean pile base.



8.6.2 Pile Footing

Piles should be designed in accordance with the requirements in AS 2159 (2009), Piling – Design and Installation. Selection of the pile system depends on many considerations and should be undertaken by the designer in conjunction with the Principal and contractor / builder.

We envisage that piles are likely to be founded within the BEDROCK units. Further advice should be sought, if piles are designed to be founded in soil units.

The advice provided herein is contingent on piles being vertically and centrally loaded. Further advice should be sought if the piles are not vertically centrally loaded.

With regards to the pile design, we recommend that:

- A basic geotechnical strength reduction factor, $\Phi_{gb} = 0.60$ (AS2159-2009 Clause. 4.3.2) be adopted for a high redundancy system for an assessed average risk rating (ARR) between 2.5 and 3.0. This should be reviewed to suit the specific design and appropriate pile testing proposed by the structural / pile designers in accordance with the requirements of AS2159-2009.
- It may be possible to increase the pile reduction factors, if the details of the proposed pile installation procedures indicate a high level of quality control with regards to concrete placement, base cleanliness, etc.
- If a geotechnical strength reduction factor, $\Phi_g = 0.40$ is adopted then no pile testing will be required (AS2159-2009 Clause 8.2.4 (b)).

The actual pile design will be heavily dependent on the pile type, installation method and required loadings. We recommend that piling contractors be provided with the information in this report as a basis for the design.

8.7 Slabs

The design of slabs on ground can be based on a:

- The subsurface conditions as described in Section 5
- A subgrade with a long-term Young's Modulus provided in Table 10.

We recommend that the exposed subgrade is inspected by a suitably qualified geotechnical engineer prior to the pouring of concrete. Softened/ loose areas will need to be boxed out and backfilled with engineered fill.

8.8 Pavements

A total of three (3) CBR tests were undertaken in the geotechnical investigation. The test results indicate a soaked CBR value of between 1.5% to 4%.

A design subgrade CBR of 1.5% can be adopted for pavement founded on the EXISTING FILL unit and earthworks completed in accordance with the Specification.

Further testing will be required to confirm the design value if the pavement will be constructed on new imported fill or if significant cut and fill will be carried out.

We recommend that specific CBR testing be undertaken at subgrade level when pavement layouts are finalised.

Should you have further queries, please do not hesitate to contact us.

Yours Sincerely



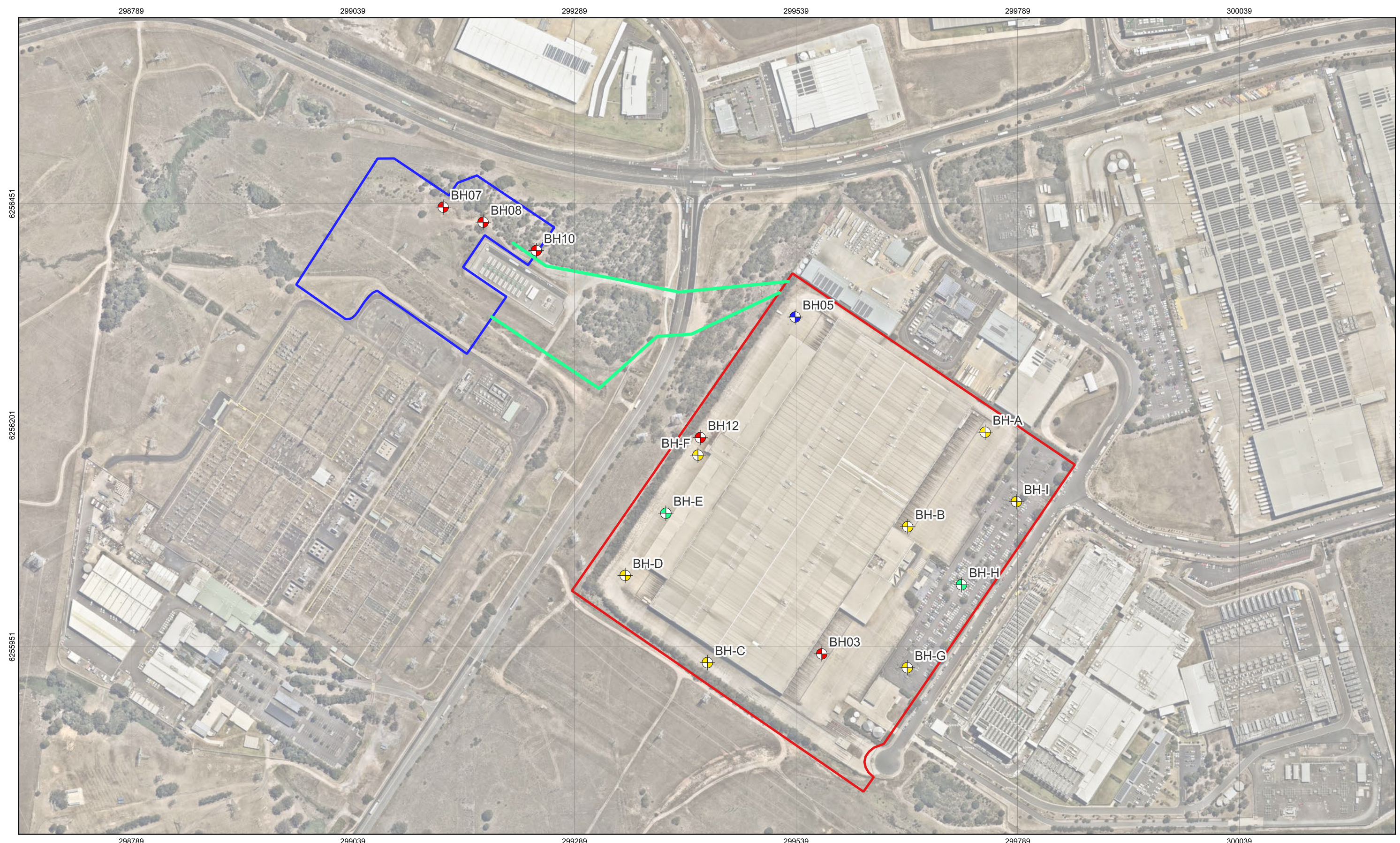
**TSZ IN WONG
GEOTECHNICAL ENGINEER**










**STEPHANIE SALIM
ASSOCIATE GEOTECHNICAL ENGINEER**



**AGUSTRIA SALIM
PRINCIPAL**



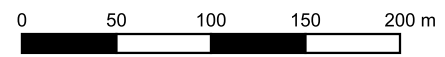
Legend

 Warehouse Site - 10 Roberts Road	 Non-cored boreholes (2025)
 Transgrid Site - Lenore Drive after Wallgrove Road	 Cored Boreholes (2025)
 HV Pathway	 Non-cored boreholes (2026)
	 Cored boreholes (2026)

Notes:

1. Aerial imagery sourced from nearmap.com dated 10 October 2025.
2. All boundaries and testing locations are approximate.
3. The 2025 site investigation was completed on 23 and 31 October 2025.
4. The additional site investigation was completed on 29 to 30 January 2026, and 2 March to 5 March 2026.

Scale 1:4,000



Map Projection:
GDA 2020 / MGA zone 56
Grid: EPSG:7856

PSM	Created By:	TW	Revision:	A
	Date:	24 Mar 2026	Paper Size:	A3

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Project Atlas 10 Roberts Road
Eastern Creek NSW

**GEOTECHNICAL INVESTIGATION
LOCALITY PLAN**

PSM5953-006R	Figure 1
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M:\PSM5953\Eng\1. GIS\02_WorkSpace\01_MXD\PSM5953-006R Figure 1 REV A SSDA Submission



Photo 1 - General site photo looking south from BH08



Photo 2 - General site photo looking north from BH10



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10 Roberts Rd, Eastern Creek
2025 SITE INVESTIGATION

SELECTED SITE PHOTOGRAPHS (1 OF 9)

PSM5953-006R

Figure 2



Photo 3 - General site photo looking south from BH12



Photo 4 - General site photo looking north from BH03



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2025 SITE INVESTIGATION

SELECTED SITE PHOTOGRAPHS (2 OF 9)

PSM5953-006R

Figure 3



Photo 5 - Machine used for drilling (JK309)



Photo 6 - Machine used for drilling (MCT200)



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 2025 SITE INVESTIGATION
 SELECTED SITE PHOTOGRAPHS (3 OF 9)

PSM5953-006R

Figure 4



Photo 7 - Typical recovered FILL material



Photo 8 - Typical recovered NATURAL material



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2025 SITE INVESTIGATION

SELECTED SITE PHOTOGRAPHS (4 OF 9)

PSM5953-006R

Figure 5



Photo 9 - Typical recovered BEDROCK



Photo 10 - Typical core of concrete pavement (BH03)



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2025 SITE INVESTIGATION

SELECTED SITE PHOTOGRAPHS (5 OF 9)

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Figure 6



Photo 11 - Typical borehole after concrete coring (BH03)



Photo 12 - Typical backfilled borehole (BH03)



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2025 SITE INVESTIGATION

SELECTED SITE PHOTOGRAPHS (6 OF 9)

PSM5953-006R

Figure 7



Photo 13 - General site photo looking north from BH-G



Photo 14 - General site photo looking east from BH-H



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2026 SITE INVESTIGATION

SELECTED SITE PHOTOGRAPHS (7 OF 9)

PSM5953-006R

Figure 8



Photo 15 - Machine used for drilling (JK400)



Photo 16 - Typical backfilled borehole (BH-B)



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2026 SITE INVESTIGATION

SELECTED SITE PHOTOGRAPHS (8 OF 9)

PSM5953-006R

Figure 9



Photo 17 - Typical recovered BEDROCK from SPT



Photo 18 - Typical recovered BEDROCK



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10 Roberts Rd, Eastern Creek
2026 SITE INVESTIGATION

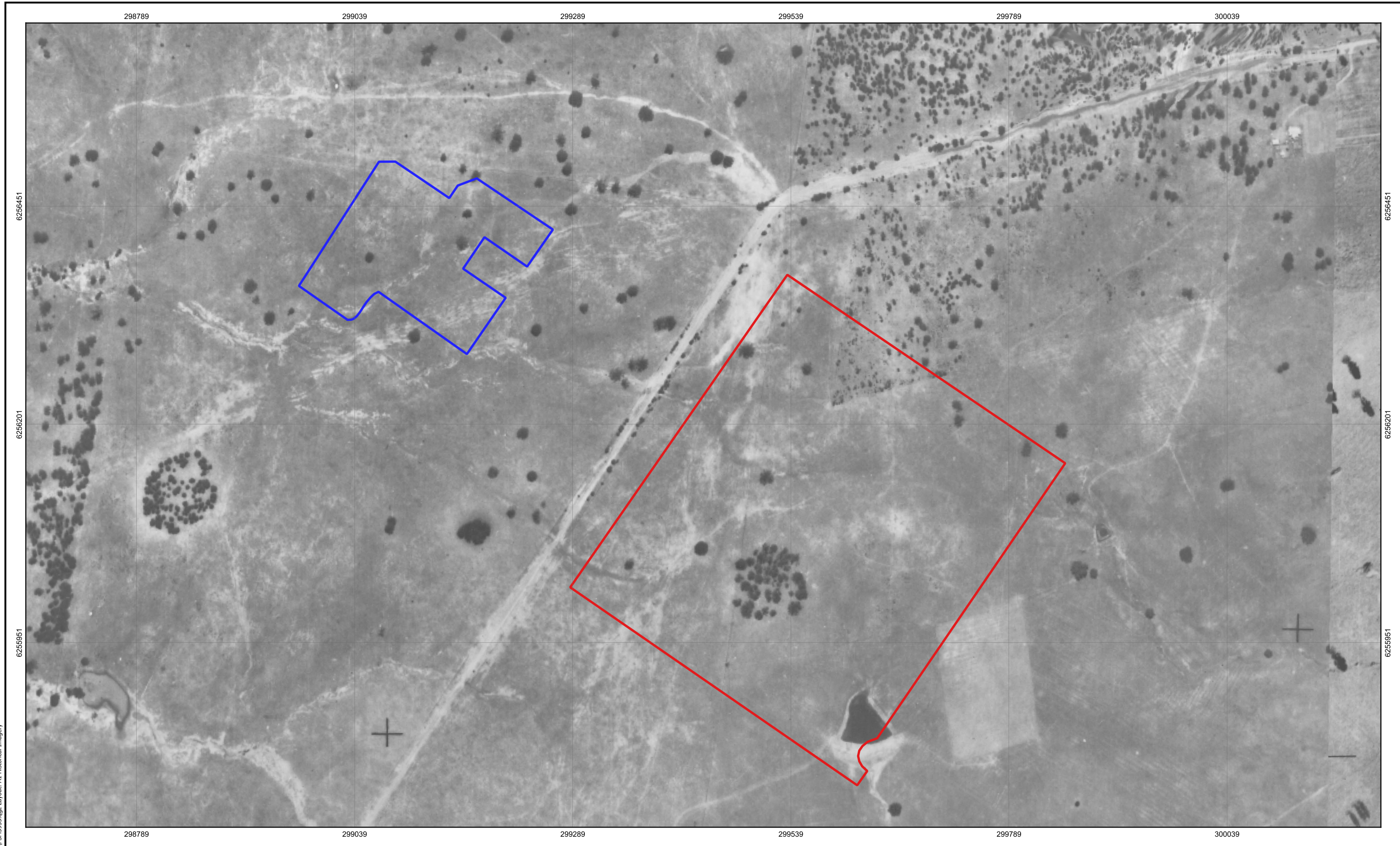
SELECTED SITE PHOTOGRAPHS (9 OF 9)

PSM5953-006R

Figure 10

Appendix A Historical Aerial Imagery





M:\PSM5953\Eng\1. GIS\02_Workspace\01_MXD\PSM5953.dwg Layout: A1 Historical Imagery

- Legend**
- Warehouse Site - 10 Roberts Road
 - Transgrid Site - Lenore Drive after Wallgrove Road

Notes:
 1. Aerial imagery sourced from NSW Spatial Services Historical Imagery Viewer dated 1947.
 2. All site boundaries are approximate.

Scale 1:4,000

Map Projection:
GDA 2020 / MGA zone 56
Grid: EPSG:7856

P S M 	Created By:	TW	Revision:	A
	Date:	24 Mar 2026	Paper Size:	A3

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 Eastern Creek NSW
HISTORICAL AERIAL IMAGERY
 1947

PSM5953-006R	Appendix A
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298789

299039

299289

299539

299789

300039

6256451

6256201

6255951

6256451

6256201

6255951



298789

299039

299289

299539

299789

300039

Legend

- Warehouse Site - 10 Roberts Road
- Transgrid Site - Lenore Drive after Wallgrove Road

Notes:

1. Aerial imagery sourced from NSW Spatial Services Historical Imagery Viewer dated 1991.
2. All site boundaries are approximate.

Scale 1:4,000

Map Projection:
GDA 2020 / MGA zone 56
Grid: EPSG:7856

P S M	Created By: TW	Revision: A
Date: 24 Mar 2026	Paper Size: A3	

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 Project Atlas 10 Roberts Road
 Eastern Creek NSW
HISTORICAL AERIAL IMAGERY
 1991

PSM5953-006R Appendix A

298789

299039

299289

299539

299789

300039

6256451

6256201

6255951

6256451

6256201

6255951



298789

299039

299289

299539

299789

300039

Legend

- Warehouse Site - 10 Roberts Road
- Transgrid Site - Lenore Drive after Wallgrove Road

Notes:

1. Aerial imagery sourced from NSW Spatial Services Historical Imagery Viewer dated 2005.
2. All site boundaries are approximate.

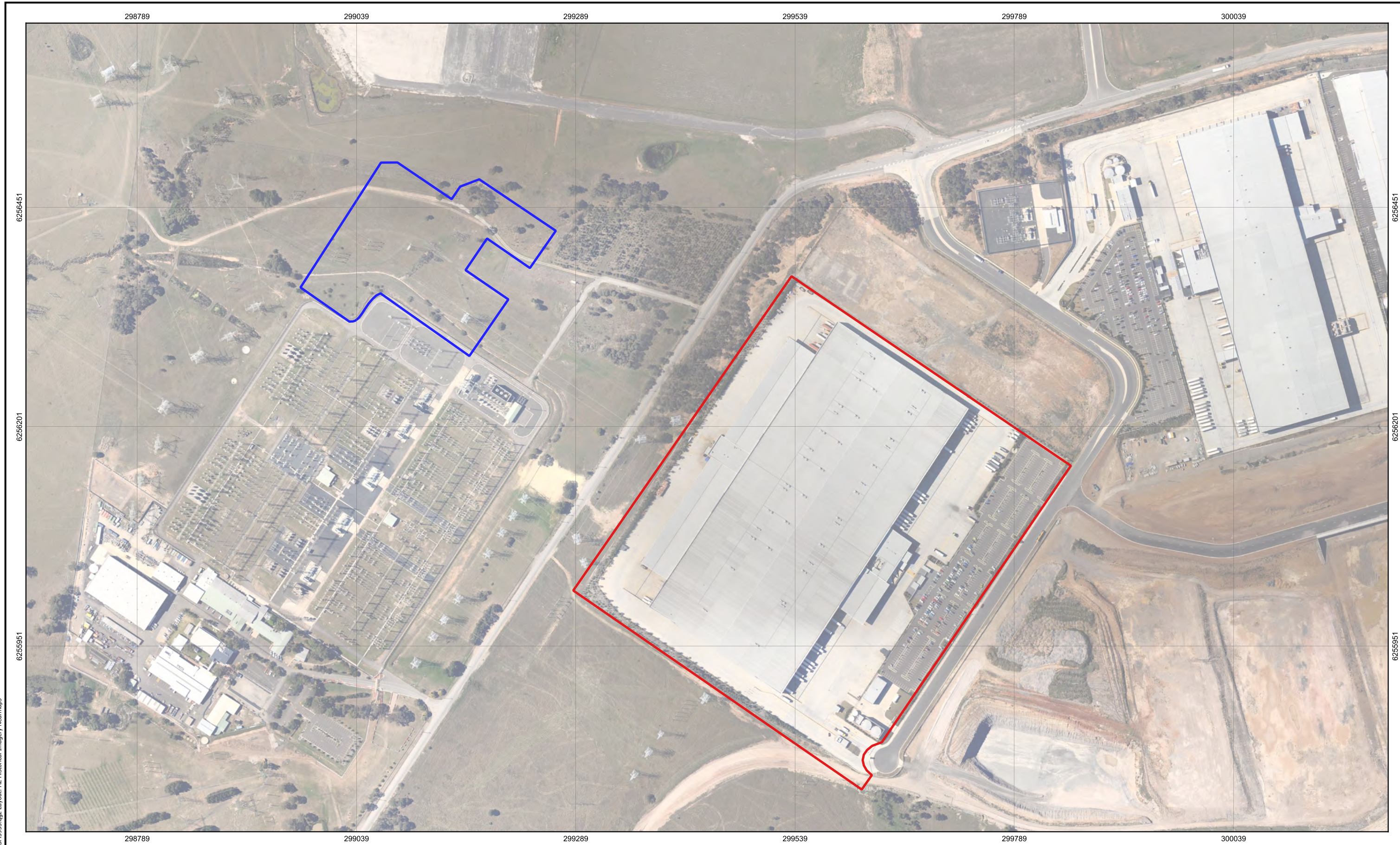
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Date: 24 Mar 2026			Paper Size: A3	

Goodman
Project Atlas 10 Roberts Road
Eastern Creek NSW
HISTORICAL AERIAL IMAGERY
2005

PSM5953-006R Appendix A

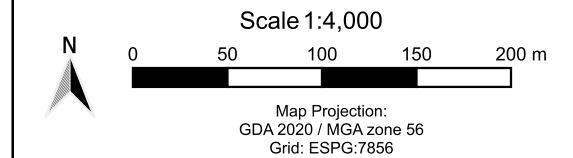


Legend

- Warehouse Site - 10 Roberts Road
- Transgrid Site - Lenore Drive after Wallgrove Road

Notes:

1. Aerial imagery sourced from Nearmaps dated 14 November 2009.
2. All site boundaries are approximate.



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	Date:	24 Mar 2026	Paper Size:	A3

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 Project Atlas 10 Roberts Road
 Eastern Creek NSW
HISTORICAL AERIAL IMAGERY
 14 November 2009

PSM5953-006R Appendix A



- Legend**
- Warehouse Site - 10 Roberts Road
 - Transgrid Site - Lenore Drive after Wallgrove Road

Notes:
 1. Aerial imagery sourced from Nearmaps dated 21 November 2014,
 2. All site boundaries are approximate.

Scale 1:4,000

0 50 100 150 200 m

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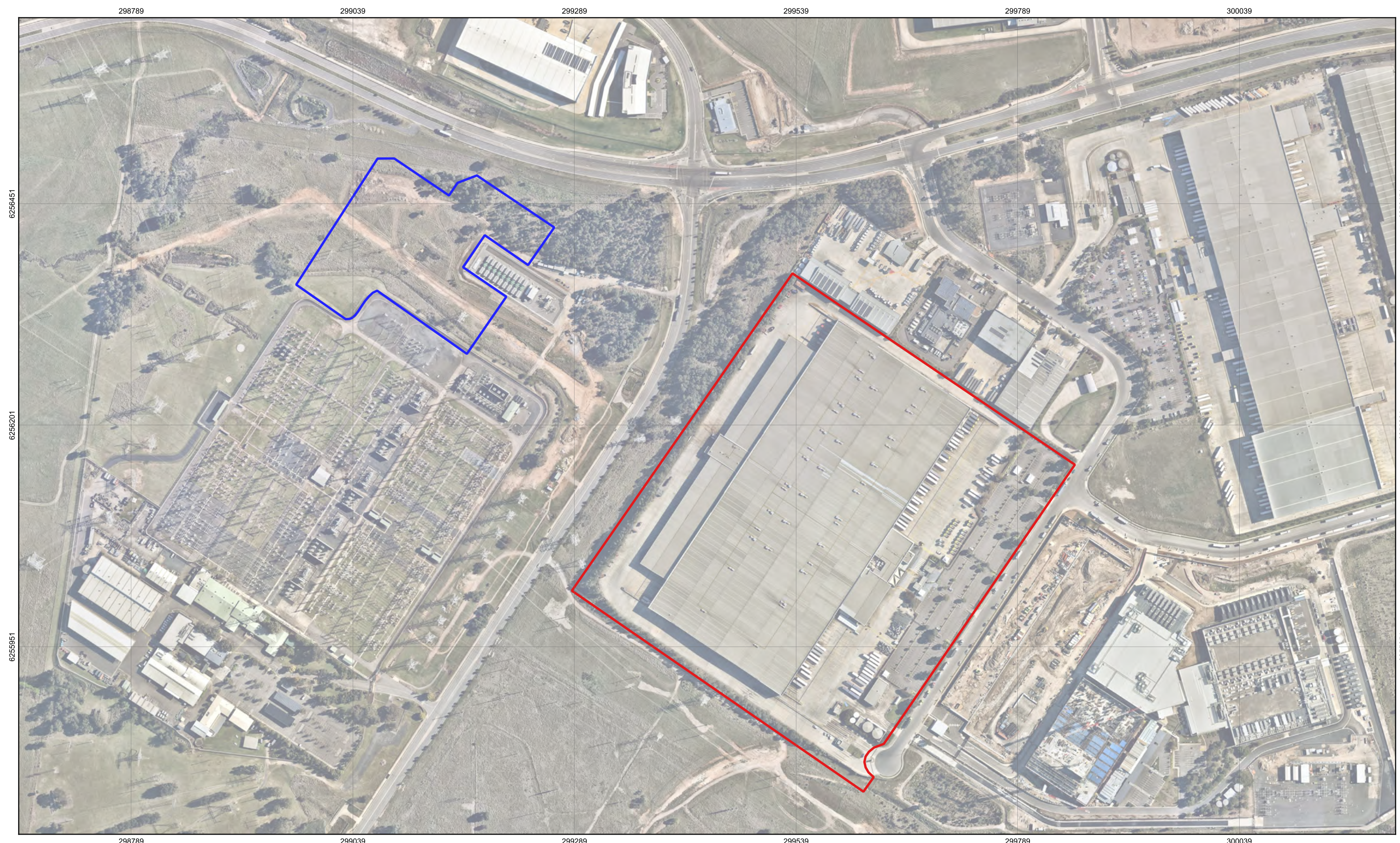
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 Project Atlas 10 Roberts Road
 Eastern Creek NSW

HISTORICAL AERIAL IMAGERY
 21 November 2014

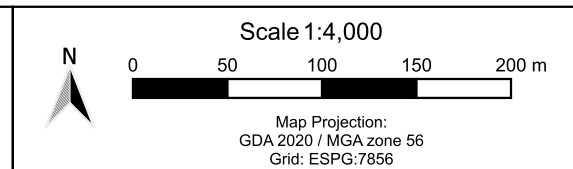
PSM5953-006R	Appendix A
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M:\PSM5953\Eng\1. GIS\02_Workspace\01_MXD\PSM5953.dwg Layout: A2 Historical Imagery Nearmaps



- Legend**
- Warehouse Site - 10 Roberts Road
 - Transgrid Site - Lenore Drive after Wallgrove Road

Notes:
 1. Aerial imagery sourced from Nearmaps dated 17 October 2021.
 2. All site boundaries are approximate.



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 Project Atlas 10 Roberts Road
 Eastern Creek NSW
HISTORICAL AERIAL IMAGERY
 17 October 2021

PSM	Created By:	TW	Revision:	A
	Date:	24 Mar 2026	Paper Size:	A3

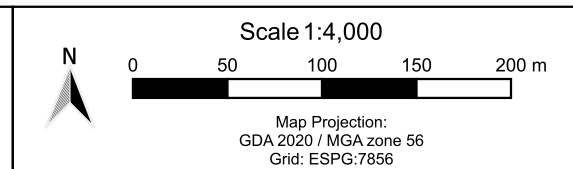
PSM5953-006R Appendix A

M:\PSM5953\Eng\1. GIS\02_Workspace\01_MXD\PSM5953.dwg Layout: A2 Historical Imagery Nearmaps



- Legend**
- Warehouse Site - 10 Roberts Road
 - Transgrid Site - Lenore Drive after Wallgrove Road

Notes:
 1. Aerial imagery sourced from Nearmaps dated 10 October 2025.
 2. All site boundaries are approximate.



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 Project Atlas 10 Roberts Road
 Eastern Creek NSW
HISTORICAL AERIAL IMAGERY
 10 October 2025

PSM	Created By:	TW	Revision:	A
	Date:	24 Mar 2026	Paper Size:	A3

PSM5953-006R Appendix A

M:\PSM5953\Eng\1. GIS\02_Workspace\01_MXD\PSM5953.dwg Layout: A2 Historical Imagery Nearmaps

Appendix B Engineering Borehole Logs





Engineering Log - Non Cored Borehole

Project No.: PSM5953

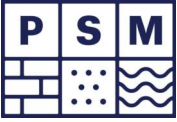
Client: Goodman	Commenced: 31/10/2025
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 31/10/2025
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299568 m E 6255943 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: 7.5 tonne truck mounted drill rig	Inclination: -90°	RL Surface: 102.60 m
Hole Diameter: 150 mm	Bearing:	Datum: AHD
		Operator: Terratest

Drilling Information				Soil Description						Observations				
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
DT		N								CONCRETE: 200 mm thick.				0.00: PAVEMENT
										CONCRETE: 100 mm thick.				0.30: Possibly FILL
				D 0.50-1.00 m						FILL: Gravelly CLAY; brown and dark grey, medium plasticity, gravel fine-grained sub-angular up to 40 mm.				
				CBR 1.00-1.50 m		101.6	1			1.0m: becomes red brown and pale grey, gravel sub-angular up to 5mm.	M (<PL)	St		
				SPT 1.50-1.95 m 3.5,7 N=12 D 1.50-1.95 m										1.70: Possibly NATURAL
ADV		N				100.6	2		CI	CLAY; pale grey mottled red, medium plasticity.	M (>PL)	St		
									CI	CLAY with gravel; yellow brown, medium plasticity, gravel fine-grained sub-angular up to 10 mm.				
											M (<PL)	St		
				SPT 3.00-3.45 m 2,3,6/60 mm HB N=ref		99.6	3							3.00: Some SHALE fragments observed
AD/T		N								SHALE; brown, extremely weathered to highly weathered, very low to low strength.				3.30: V-bit refusal, possibly BEDROCK. Rock strength and weathering inferred from drilling (penetration resistance and cuttings)
										Hole Terminated at 3.60 m Refusal				3.60: TC-bit refusal
						98.6	4							

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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Engineering Log - Non Cored Borehole

Project No.: PSM5953

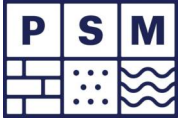
Client: Goodman	Commenced: 31/10/2025
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 31/10/2025
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299538 m E 6256323 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: 7.5 tonne truck mounted drill rig	Inclination: -90°	RL Surface: 103.00 m
Hole Diameter: 150 mm	Bearing:	Datum: AHD
		Operator: Terratest

Drilling Information				Soil Description						Observations				
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
DT	[Hatched]	N						[Grid]		CONCRETE: 220 mm thick.				0.00: PAVEMENT
										CONCRETE: 230 mm thick.				
AD/V	[Hatched]	N		D 1.00-1.50 m SPT 1.50-1.95 m 5.5,9 N=14	[Hatched]	102.0	1	[Hatched]	CI	FILL: CLAY with gravel; dark brown, medium plasticity, gravel fine-grained sub-angular up to 10 mm.	M (<PL)	St		0.45: Possibly FILL
						101.0	2	[Hatched]	CI	CLAY; pale grey and red, medium plasticity.	M (>PL)	St		1.70: Possibly NATURAL
						101.0	2	[Hatched]	CI	CLAY with gravel; brown, medium plasticity, gravel fine-grained sub-angular up to 5 mm.	M (<PL)	St		2.00: Some SHALE fragments observed
						100.0	3			Continued on cored borehole sheet				
						99.0	4							

Method	Penetration	Water	Samples and Tests	Moisture Condition	Consistency/Relative Density
AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	[No resistance] [Hatched] Refusal	▽ Inflow △ Partial Loss ◀ Complete Loss	U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	D - Dry M - Moist W - Wet	VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact

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Engineering Log - Cored Borehole

Project No.: PSM5953

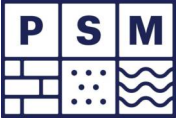
Client: Goodman	Commenced: 31/10/2025
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 31/10/2025
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299538 m E 6256323 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: 7.5 tonne truck mounted drill rig	Inclination: -90°	RL Surface: 103.00 m
Barrel Type and Length: NMLC 1.5 m	Bearing:	Datum: AHD
		Operator: Terratest

Drilling Information					Rock Substance					Rock Mass Defects		
Method	Water	TCR (%)	ROD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering	Strength Is(50)	Defect Spacing (mm)	Defect Descriptions / Comments
								ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XW HW MW SW FR	● - Axial ○ - Diametral VL 0.1 L 0.3 M 1 H 3 VH 10 EH	<20 60 200 600 1000	Description, alpha/beta, infilling or coating, shape, roughness, thickness, other
					102.0	1						
					101.0	2						
					100.0	3		Continued from non-cored borehole sheet SHALE; brown, massive, iron staining observed.				2.66: V-bit refusal
					100.0	3		SHALE; red brown and pale grey, distinct rock fabric, hard consistency to very low strength, massive, some clay seams up to depth 3.7m.				SM, CL, 250 mm
					99.0	4		3.7m: becomes grey and brown, massive, very low strength.				SM, CL, 200 mm BP, 0°, CN, PR, RF BP, 0°, CN, PR, RF BP, 5°, CN, PR, RF JT, 80°, CN, PR, RF BP, 0°, CN, PR, RF FZ, CL & RF, IR, RF BP, 0°, CN, PR, RF CZ, CL, IR, RF CZ, CL & RF, IR, RF BP, 0°, CN, PR, RF JT, 45°, RF, IR, RF

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT- Standard penetration test PT - Push tube WPT - Water pressure test	Water ▽ Inflow △ Partial Loss ▲ Complete Loss Graphic Log/Core Loss 	Weathering XW - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron OZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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PSM 3.02.2, LIB (HLZ), GLB Log PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:36 10.03.00.09 Dangle Fence and Map Tool | Lib: PSM 3.02.1 2019-03-06 PSM 3.02.1 2019-03-06



Engineering Log - Cored Borehole

Project No.: PSM5953

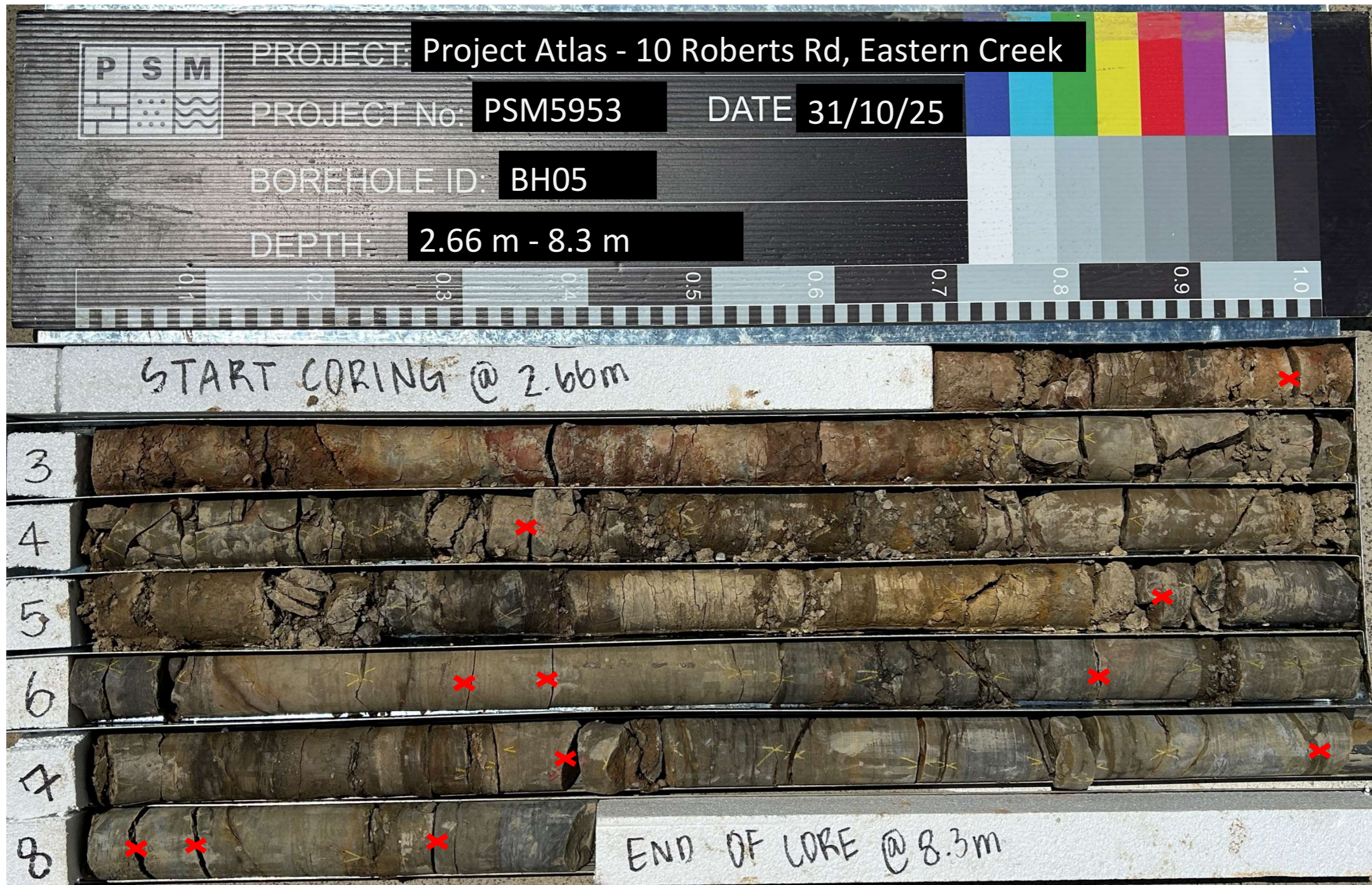
Client: Goodman	Commenced: 31/10/2025
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 31/10/2025
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299538 m E 6256323 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: 7.5 tonne truck mounted drill rig	Inclination: -90°	RL Surface: 103.00 m
Barrel Type and Length: NMLC 1.5 m	Bearing:	Datum: AHD
		Operator: Terratest

Drilling Information					Rock Substance					Rock Mass Defects																	
Method	Water	TCR (%)	ROD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering				Strength Is(50)			Defect Spacing (mm)			Defect Descriptions / Comments								
								ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XW	HW	MW	SW	FR	VL	L	M	H	VH	EH	<20	20	60	200	600	1000		
NMLC	100% Water RETURN	100	47	Is(50) d=0.04 a=0.14 MPa	97.0	6		SHALE; grey and brown, massive, distinct rock fabric. 5.2m: becomes developed, thinly laminated, some iron staining observed.																		SS, CL & RF, IR, S JT, 80°, RF, PR, RF BP, 0°, RF, PR, RF BP, 0°, CN, PR, RF FZ, CL & RF, IR, RF JT, 40°, CN, PR, RF SM, CL, 50 mm FZ, RF, IR, RF BP, 0°, RF, PR, RF BP, 0°, RF, PR, RF BP, 2°, CN, PR, RF BP, 10°, CN, PR, RF BP, 10°, RF, CU, RF CZ, CL & RF, IR, RF CZ, CL & RF, IR, RF BP, 10°, RF, PR, RF BP, 0°, CL, PR, RF JT, 30°, CL & Fe, PR, S SM, CL, 50 mm BP, 0°, RF, PR, RF BP, 0°, RF, PR, RF FZ, CL & RF, IR, RF BP, 10°, CN, PR, RF BP, 20°, CN, CU, RF BP, 0°, FE, PR, RF BP, 0°, CN, PR, RF BP, 20°, RF, PR, RF BP, 20°, CN, PR, RF JT, 35°, FE, PR, RF BP, 0°, CL & RF, PR, RF	
		100	46	Is(50) d=0.28 a=0.16 MPa	96.0	7																					
		100	62	Is(50) a=0.3 MPa Is(50) a=0.23 MPa	95.0	8																					
				Is(50) d=0.28 a=0.32 MPa	94.0	9		Hole Terminated at 8.30 m Target depth																			

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT- Standard penetration test PT - Push tube WPT - Water pressure test	Water Inflow Partial Loss Complete Loss Graphic Log/Core Loss Core recovered (hatching indicates material) No core recovery	Weathering XW - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron OZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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PSM 3.02.2 LIB (HLZ) GLB Log PSM5953 EASTERN CREEK GPJ <<DrawingFile>> 17/03/2026 11:36 10.03.00.09 Digital Fence and Map Tool Lib: PSM 3.02.1 2019-03-06 PSM 3.02.1 2019-03-06



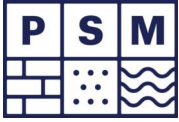
Note: Drill breaks are marked by red cross



Goodman
Project Atlas
10 Roberts Road, Eastern Creek
Core Photograph (BH05)

PSM5953-002R

APPENDIX B



Engineering Log - Non Cored Borehole

Project No.: PSM5953

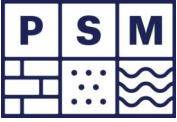
Client: Goodman	Commenced: 23/10/2025
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 23/10/2025
Hole Location: See Figure 1 (Transgrid Site)	Logged By: TW
Hole Position: 299141 m E 6256447 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: 6 tonne track mounted drill rig	Inclination: -90°	RL Surface: 91.10 m
Hole Diameter: 150 mm	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information				Soil Description						Observations				
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
AD/V		N		SPT 0.50-0.95 m 3,3,5 N=8		90.1	1		OL	TOPSOIL: Silty CLAY; brown, low plasticity.	D	F		0.00: Possibly TOPSOIL, rootlets observed from 0 to 0.2 m
				D 1.00-1.50 m					CI	FILL: Sandy CLAY; brown and pale grey mottled red brown, medium plasticity; sand fine to medium grained.	M (>PL)	St		0.20: Possibly FILL
				SPT 1.50-1.95 m 9,7,7 N=14		89.1	2		CI	CLAY with gravel; red brown, medium plasticity, gravel fine-grained sub-angular up to 5mm.	M (<PL)	St		1.00: Possibly NATURAL
AD/T		N				88.1	3			SHALE; brown, extremely weathered to highly weathered, very low to low strength.				2.20: V-bit refusal, possibly BEDROCK. Rock strength and weathering inferred from drilling (penetration resistance and cuttings)
						87.1	4							
							4.5m			4.5m: becomes highly weathered, low strength.				

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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PSM 3.02.2, LIB (HLZ), G.L.B. Log PSM AU NONCORE BH NZ AU PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:18 10.03.00.09 Dageel Fence and Map Tool [Lib: PSM 3.02.1 2019-03-06 Proj: PSM 3.02.1 2019-03-06]



Borehole ID
BH07
Page 2 of 2

Engineering Log - Non Cored Borehole

Project No.: PSM5953

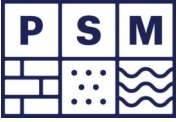
Client: Goodman	Commenced: 23/10/2025
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 23/10/2025
Hole Location: See Figure 1 (Transgrid Site)	Logged By: TW
Hole Position: 299141 m E 6256447 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: 6 tonne track mounted drill rig	Inclination: -90°	RL Surface: 91.10 m
Hole Diameter: 150 mm	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Soil Description					Observations					
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description SOIL NAME: Plasticity, behaviour or particle characteristics of primary component, colour, secondary components, additional observations	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations	
AD/T		N				85.1	6			SHALE; brown, highly weathered, low strength.					
						84.1	7			Hole Terminated at 6.00 m Target depth					
						83.1	8								
						82.1	9								

PSM 3.02.2 LIB (HLZ) G.L.B. Log PSM AU NONCORE BH NZ AU PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:18 10.03.00.09 Dageel Fence and Map Tool Lib: PSM 3.02.1 2019-03-06 Pj: PSM 3.02.1 2019-03-06

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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Engineering Log - Non Cored Borehole

Project No.: PSM5953

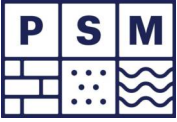
Client: Goodman	Commenced: 23/10/2025
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 23/10/2025
Hole Location: See Figure 1 (Transgrid Site)	Logged By: TW
Hole Position: 299186 m E 6256430 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: 6 tonne track mounted drill rig	Inclination: -90°	RL Surface: 90.10 m
Hole Diameter: 150 mm	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information				Soil Description						Observations				
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
ADV	N			SPT 0.50-0.95 m 3,3,5 N=8 D 0.50-1.00 m	89.1	1	0.00		OL	TOPSOIL: Silty CLAY; brown, low plasticity.	D	F		0.00: Possibly TOPSOIL, rootlets observed from 0 to 0.2 m
									FILL: Sandy CLAY; brown, medium plasticity, sand fine to medium grained.	M (>PL)	F to St		0.20: Possibly FILL	
									CLAY; red brown and pale grey mottled red, medium plasticity.				0.50: Possibly NATURAL	
									1.9m: becomes pale grey.					
AD/T	N			SPT 1.50-1.95 m 5,5,8 N=13	88.1	2	0.50		CI	CLAY with gravel; brown, medium plasticity, gravel fine-grained sub-angular up to 5 mm.	M (<PL)	St		2.00: Some SHALE fragments observed
									SHALE; brown and grey, extremely weathered to highly weathered, very low to low strength.				2.40: V-bit refusal, possibly BEDROCK. Rock strength and weathering inferred from drilling (penetration resistance and cuttings)	
					87.1	3								
					86.1	4								

Method	Penetration	Water	Samples and Tests	Moisture Condition	Consistency/Relative Density
AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	No resistance Refusal	Inflow Partial Loss Complete Loss	U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	D - Dry M - Moist W - Wet	VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact

PSM 3.02.2 LIB (HLZ) G.L.B. Log PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:18 10.03.00.09 Dageel Fence and Map Tool Lib: PSM 3.02.1 2019-03-06 Pj: PSM 3.02.1 2019-03-06



Borehole ID
BH08
Page 2 of 2

Engineering Log - Non Cored Borehole

Project No.: PSM5953

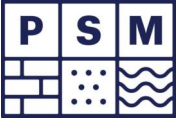
Client: Goodman	Commenced: 23/10/2025
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 23/10/2025
Hole Location: See Figure 1 (Transgrid Site)	Logged By: TW
Hole Position: 299186 m E 6256430 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: 6 tonne track mounted drill rig	Inclination: -90°	RL Surface: 90.10 m
Hole Diameter: 150 mm	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Soil Description						Observations				
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations	
AD/T		N				84.1	6			SHALE: brown and grey, highly weathered, low strength.					
						83.1	7			Hole Terminated at 6.00 m Target depth					
						82.1	8								
						81.1	9								

PSM 3.02.2 LIB (HLZ) G.L.B. Log PSM AU NONCORE BH NZ AU PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:18 10.03.00.09 Dageel Fence and Map Tool Lib: PSM 3.02.1 2019-03-06 Pj: PSM 3.02.1 2019-03-06

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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Borehole ID
BH10
Page 1 of 2

Engineering Log - Non Cored Borehole

Project No.: PSM5953

Client: Goodman	Commenced: 23/10/2025
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 23/10/2025
Hole Location: See Figure 1 (Transgrid Site)	Logged By: TW
Hole Position: 299246 m E 6256398 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: 6 tonne track mounted drill rig	Inclination: -90°	RL Surface: 93.10 m
Hole Diameter: 150 mm	Bearing:	Datum: AHD Operator: JK Drilling

Drilling Information				Soil Description					Observations					
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
AD/V		N		CBR 0.20-0.80 m SPT 0.50-0.95 m 4,4,5 HB N=9		92.1	0		OL	TOPSOIL: Silty CLAY; brown, low plasticity.	D	F		0.00: Possibly TOPSOIL, rootlets observed from 0 to 0.2 m
									CI	CLAY; red brown, medium plasticity.	M (>PL)	St		0.20: Possibly NATURAL
AD/T		N				91.1	1			SHALE; brown, extremely weathered to highly weathered, very low to low strength.				0.80: V-bit refusal, possibly BEDROCK. Rock strength and weathering inferred from drilling (penetration resistance and cuttings)
						90.1	3							
						89.1	4							4.50: Some medium strength SHALE fragments up to 20 mm observed.

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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Borehole ID
BH10
Page 2 of 2

Engineering Log - Non Cored Borehole

Project No.: PSM5953

Client: Goodman	Commenced: 23/10/2025
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 23/10/2025
Hole Location: See Figure 1 (Transgrid Site)	Logged By: TW
Hole Position: 299246 m E 6256398 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: 6 tonne track mounted drill rig	Inclination: -90°	RL Surface: 93.10 m
Hole Diameter: 150 mm	Bearing:	Datum: AHD Operator: JK Drilling

Drilling Information					Soil Description					Observations				
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
AD/T		N	▽			87.1	6			SHALE; pale grey, highly weathered, medium strength.				6.00: Depth to groundwater inferred from encountering wet and saturated spoil material from auger.
						86.1	7			Hole Terminated at 7.00 m Target depth				
						85.1	8							
						84.1	9							

PSM 3.02.2 LIB (HLZ) G.L.B. Log PSM 5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:18 10.03.00.09 Dageel Fence and Map Tool Lib: PSM 3.02.1 2019-03-06

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water ▽ Inflow ▽ Partial Loss ▲ Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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Engineering Log - Non Cored Borehole

Project No.: PSM5953

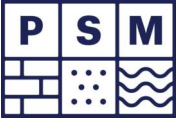
Client: Goodman	Commenced: 31/10/2025
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 31/10/2025
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299431 m E 6256187 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: 7.5 tonne truck mounted drill rig	Inclination: -90°	RL Surface: 102.80 m
Hole Diameter: 150 mm	Bearing:	Datum: AHD
		Operator: Terratest

Drilling Information				Soil Description						Observations				
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
AD/V	N	N	N	SPT 0.50-0.95 m 2,4,4 N=8 D 0.50-0.95 m	101.8	1	1	[Cross-hatched]	CI	CONCRETE: 200 mm thick. CONCRETE: 150 mm thick. FILL: Gravelly CLAY; dark brown, medium plasticity, gravel fine-grained sub-angular up to 20mm. 1.0m: gravel sub-angular up to 10mm.	M (<PL)	St	100 200 300 400 500	0.00: PAVEMENT 0.35: Possibly FILL
				SPT 1.50-1.95 m 3,3,5 N=8	100.8	2	2	[Diagonal lines]	CLAY; pale grey and red, medium plasticity. 1.8m: becomes dark grey	M (>PL)	St	1.60: Possibly NATURAL		
				SPT 3.00-3.45 m 7,6,6 N=12	99.8	3	3	[Diagonal lines]	3.1m: becomes pale grey and red	M (>PL)	St	3.45: Some SHALE fragments observed at the bottom part of SPT recovery		
				SPT 4.50-4.95 m 8,10,8 N=18	98.8	4	4	[Diagonal lines]	CLAY with gravel; brown, medium plasticity, gravel fine to medium grained sub-angular up to 20mm. 4.8m: becomes pale grey	M (<PL)	VSt			

Method	Penetration	Water	Samples and Tests	Moisture Condition	Consistency/Relative Density
AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	No resistance Refusal	▽ Inflow △ Partial Loss ◀ Complete Loss	U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	D - Dry M - Moist W - Wet	VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact

PSM 3.02.2 LIB (HLZ) G.L.B Log PSM 5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:18 10.03.00.09 Dageel Fence and Map Tool Lib: PSM 3.02.1 2019-03-06 Pj: PSM 3.02.1 2019-03-06



Engineering Log - Non Cored Borehole

Project No.: PSM5953

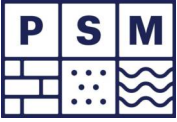
Client: Goodman	Commenced: 31/10/2025
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 31/10/2025
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299431 m E 6256187 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: 7.5 tonne truck mounted drill rig	Inclination: -90°	RL Surface: 102.80 m
Hole Diameter: 150 mm	Bearing:	Datum: AHD
		Operator: Terratest

Drilling Information					Soil Description						Observations			
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
ADV		N							CI	CLAY with gravel; pale grey, medium plasticity, gravel fine to medium grained sub-angular up to 20mm.	M (<PL)	VSt		5.50: V-bit refusal, possibly BEDROCK. Rock strength and weathering inferred from drilling (penetration resistance and cuttings)
AD/T		N				96.8	6			SHALE; brown, extremely weathered to highly weathered, very low to low strength.				
						95.8	7			Hole Terminated at 6.00 m Target depth				
						94.8	8							
						93.8	9							

PSM 3.02.2 LIB (HLZ) G.L.B. Log PSM AU NONCORE BH NZ AU PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:18 10.03.00.09 Dageel Fence and Map Tool Lib: PSM 3.02.1 2019-03-06

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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Engineering Log - Non Cored Borehole

Project No.: PSM5953

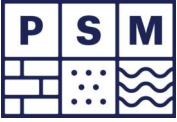
Client: Goodman	Commenced: 02/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 02/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299752 m E 6256193 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 79.70 m	
Hole Diameter: 150 mm	Bearing:	Datum: AHD	Operator: JK Drilling

Drilling Information					Soil Description					Observations				
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description SOIL NAME: Plasticity, behaviour or particle characteristics of primary component, colour, secondary components, additional observations	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
DT		N								CONCRETE: 190mm thick.				0.00: Pavement
						78.7	1			FILL: Gravelly CLAY: pale brown, medium plasticity, gravel is of shale and sandstone origin, subangular up to 40mm.	M (<PL)	St		0.19: V bit refusal, possibly FILL
AD/T		N				77.7	2		CI	CLAY with gravel: pale brown, medium plasticity, gravel is of shale and sandstone origin, subangular up to 25mm.	M (>PL)	St		1.00: Possibly NATURAL
						76.7	3			SHALE: pale brown, extremely weathered to highly weathered, very low to low strength.				2.00: Possibly BEDROCK
						75.7	4			SHALE: pale grey, highly weathered, low strength.				
										Continued on cored borehole sheet				

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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PSM 3.02.2, LIB (HLZ), G.L.B. Log PSM/AU NONCORE BH NZ AU PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:14 10.03.00.09 Dageel Fence and Map Tool | Lib: PSM 3.02.1 2019-03-06 Proj: PSM 3.02.1 2019-03-06



Engineering Log - Cored Borehole

Project No.: PSM5953

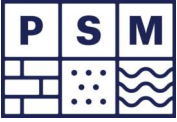
Client: Goodman	Commenced: 02/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 02/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299752 m E 6256193 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 79.70 m	Operator: JK Drilling
Barrel Type and Length: NMLC	Bearing:	Datum: AHD	

Drilling Information					Rock Substance					Rock Mass Defects						
Method	Water	TCR (%)	ROD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering				Strength Is(50)		Defect Spacing (mm)	Defect Descriptions / Comments
									XW	HW	MW	SW	FR	VL		
NMLC	100% RETURN	100	80	Is(50) d=0.88 a=4.52 MPa	78.7	1										
					77.7	2										
					76.7	3	X	Continued from non-cored borehole sheet NO CORE: 180 mm.								
				Is(50) d=0.61 a=1.21 MPa	75.7	4		SANDSTONE: grey, medium grained, well developed, thinly laminated to thinly bedded, some SILTSTONE laminations less than 10%, minor iron staining of rock mass associated in bedding parting.								BP, 0°, CL & RF, PR, RF BP, 2°, CN, PR, RF BP, 0°, CN, PR, RF BP, 0°, CN, PR, RF BP, 5°, CN, PR, RF BP, 0°, CN, PR, RF BP, 0°, CL & RF, PR, RF BP, 0°, CN, PR, RF BP, 2°, CN, PR, RF
				Is(50) d=0.63 a=0.98												

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT- Standard penetration test PT - Push tube WPT - Water pressure test	Water Inflow Partial Loss Complete Loss Graphic Log/Core Loss Core recovered (hatching indicates material) No core recovery	Weathering XW - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron OZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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PSM 3.02.2 LIB (HLZ) G.L.B. Log PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:36 10.03.00.09 Digital Fence and Map Tool Lib: PSM 3.02.1 2019-03-06 PSM 3.02.1 2019-03-06



Engineering Log - Cored Borehole

Project No.: PSM5953

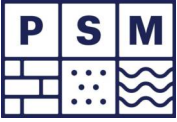
Client: Goodman	Commenced: 02/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 02/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299752 m E 6256193 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 79.70 m
Barrel Type and Length: NMLC	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information				Rock Substance						Rock Mass Defects		
Method	Water	TCR (%)	ROD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering	Strength Is(50)	Defect Spacing (mm)	Defect Descriptions / Comments
				MPa				ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XW HW MW SW FR	● - Axial ○ - Diametral	<20 60 200 600 1000	Description, alpha/beta, infilling or coating, shape, roughness, thickness, other
NMLC	100% RETURN	100	80	Is(50) d=0.66 a=1.77 MPa	73.7	6		SANDSTONE: grey, medium grained, well developed, thinly laminated to thinly bedded, some SILTSTONE laminations less than 10%, minor iron staining of rock mass associated in bedding parting. (continued)				JT, 40°, FE, PR, RF BP, 0°, FE, PR, RF BP, 0°, CN, PR, RF BP, 0°, CL & RF, PR, RF BP, 0°, CL & RF, CU, RF
	100% RETURN	100	75	Is(50) d=0.29 a=0.95 MPa Is(50) d=0.14 a=1.06 MPa	72.7	7		6.0m: SHALE clasts from 6.0m to 6.5m SHALE: dark grey, developed, thinly bedded, minor iron staining.				BP, 0°, CL & RF, IR, RF BSH, 10°, CL, CU, RF CZ, CL & RF, 40mm
	100% RETURN	100		Is(50) a=0.39 MPa	71.7	8		NO CORE: 120 mm.				BP, 0°, CL, PR, RF XWS, CL & RF, 240mm
	100% RETURN	96	87	Is(50) d=0.02 a=0.2 MPa Is(50) d=0.34 a=0.45 MPa Is(50) d=0.14 a=0.23 MPa	70.7	9		SHALE: dark grey, developed, thinly bedded.				BP, 0°, CN, PR, RF BP, 0°, CN, PR, RF BP, 2°, CL, PR, RF FZ, CL & RF, 60mm FZ, CL & RF, 50mm BP, 2°, CN, PR, RF

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT- Standard penetration test PT - Push tube WPT - Water pressure test	Water Inflow Partial Loss Complete Loss Graphic Log/Core Loss Core recovered (hatching indicates material) No core recovery	Weathering XW - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron QZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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PSM 3.02.2, LIB (HLZ), GLB Log PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:36 10.03.00.09 Digital Fence and Map Tool | Lib: PSM 3.02.1 2019-03-06 PSM 3.02.1 2019-03-06



Engineering Log - Cored Borehole

Project No.: PSM5953

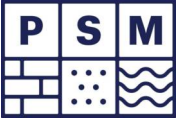
Client: Goodman	Commenced: 02/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 02/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299752 m E 6256193 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 79.70 m
Barrel Type and Length: NMLC	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Rock Substance					Rock Mass Defects		
Method	Water	TCR (%)	ROD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering	Strength Is(50)	Defect Spacing (mm)	Defect Descriptions / Comments
								ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XW HW MW SW FR	● - Axial ○ - Diametral		
	100% RETURN	96	87	Is(50) d=0.24 a=0.3 MPa	68.7	11		SHALE: dark grey, developed, thinly bedded.				BP, 0°, CN, PR, RF SM, CL & RF, 50mm
	100% RETURN	100	84	Is(50) d=0.12 MPa	67.7	12		SANDSTONE: dark grey, fine to medium grained, well developed, thinly laminated to thinly bedded, some SILTSTONE laminations less than 20%.				BP, 0°, CN, PR, RF BP, 0°, CN, PR, RF
	100% RETURN	100	84	Is(50) d=0.33 a=1.58 MPa	66.7	13		SHALE: dark grey, developed, thinly bedded to thickly bedded.				BP, 0°, CL & RF, PR, RF BP, 0°, CN, PR, RF
	100% RETURN	100	84	Is(50) d=0.6 a=1.97 MPa	65.7	14						BP, 10°, CL & RF, PR, RF BP, 0°, CN, PR, RF BP, 0°, CN, PR, RF BP, 5°, CN, PR, RF BP, 0°, CL & RF, PR, RF
	100% RETURN	100	84	Is(50) a=0.03 MPa								BP, 2°, CL, PR, RF BP, 0°, CN, PR, S
	100% RETURN	100	84	Is(50) d=0.42 a=0.63 MPa								BP, 5°, CL, PR, S BP, 5°, CL, PR, S BP, 0°, CN, PR, S
	100% RETURN	100	84	Is(50) d=0.42 a=0.63 MPa								BP, 2°, CN, PR, S
	100% RETURN	100	84	Is(50) d=0.42 a=0.63 MPa								BP, 0°, CN, PR, S
	100% RETURN	100	84	Is(50) d=0.42 a=0.63 MPa								BP, 2°, CN, PR, S

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT- Standard penetration test PT - Push tube WPT - Water pressure test	Water Inflow Partial Loss Complete Loss Graphic Log/Core Loss Core recovered (hatching indicates material) No core recovery	Weathering XW - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron OZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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PSM 3.02.2 LIB (HLZ) G.L.B. Log PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:36 10.03.00.09 Digital Fence and Map Tool | Lib: PSM 3.02.1 2019-03-06 PSM 3.02.1 2019-03-06



Engineering Log - Cored Borehole

Project No.: PSM5953

Client: Goodman	Commenced: 02/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 02/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299752 m E 6256193 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 79.70 m
Barrel Type and Length: NMLC	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Rock Substance					Rock Mass Defects		
Method	Water	TCR (%)	ROD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering	Strength Is(50)	Defect Spacing (mm)	Defect Descriptions / Comments
								ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XW HW MW SW FR	● - Axial ○ - Diametral		
NMLC	100% RETURN	100	84	Is(50) d=3 a=2.36 MPa	63.7	16	SHALE: dark grey, developed, thinly bedded to thickly bedded. (continued)					FZ, RF, 80mm BP, 0°, CN, PR, S BP, 10°, CN, CU, S
				Is(50) d=1.04 a=0.74 MPa	62.7	17	SANDSTONE: dark grey, fine to medium grained, well developed, laminated to thinly bedded.					BP, 0°, CN, PR, S BP, 0°, CN, PR, RF BP, 0°, CN, PR, RF BP, 0°, CN, ST, RF
				Is(50) d=0.53 a=1.81 MPa	60.7	18	Hole Terminated at 17.40 m Target depth					
					61.7	19						

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT- Standard penetration test PT - Push tube WPT - Water pressure test	Water Inflow Partial Loss Complete Loss Graphic Log/Core Loss Core recovered (hatching indicates material) No core recovery	Weathering XW - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron OZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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PSM 3.02.2. LIB (HLZ) G.L.B. Log PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:36 10.03.00.09 Digital Fence and Map Tool Lib: PSM 3.02.1 2019-03-06 PSM 3.02.1 2019-03-06

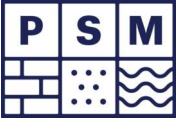


Note: Drill breaks are marked by white crosses

Goodman
Project Atlas
10 Roberts Road, Eastern Creek
Core Photograph (BH-A)



PSM5953-006R | APPENDIX B



Borehole ID
BH B
Page 1 of 3

Engineering Log - Non Cored Borehole

Project No.: PSM5953

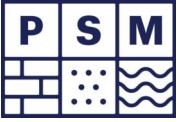
Client: Goodman	Commenced: 04/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 04/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299665 m E 6256086 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 79.42 m
Hole Diameter: 150 mm	Bearing:	Datum: AHD Operator: JK Drilling

Drilling Information					Soil Description					Observations				
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
DT		N		SPT 0.50-0.52 m 6/20mm HB N=R		78.4	1			CONCRETE: 160 mm thick.				0.00: Pavement
AD/T		N				77.4	2			SHALE: brown and grey, extremely weathered to highly weathered, very low to low strength.				0.16: V bit refusal, possibly BEDROCK
						76.4	3			SHALE: brown and grey, highly weathered, low strength.				0.50: Some medium strength SHALE fragments up to 20mm observed.
						75.4	4			Continued on cored borehole sheet				

PSM 3.02.2 LIB (HLZ) G.L.B Log PSM AU NONCORE BH NZ AU PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:14 10.03.00.09 Dageel Fence and Map Tool Lib: PSM 3.02.1 2019-03-06 Pj: PSM 3.02.1 2019-03-06

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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Borehole ID
BH B
Page 2 of 3

Engineering Log - Cored Borehole

Project No.: PSM5953

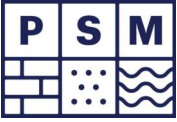
Client: Goodman	Commenced: 04/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 04/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299665 m E 6256086 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 79.42 m
Barrel Type and Length: NMLC	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Rock Substance					Rock Mass Defects		
Method	Water	TCR (%)	ROD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering	Strength Is(50)	Defect Spacing (mm)	Defect Descriptions / Comments
								ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XW HW MW SW FR	● - Axial ○ - Diametral VL 0.1 L 0.3 M 1 H 3 VH 10 EH	<20 60 200 600 1000	Description, alpha/beta, infilling or coating, shape, roughness, thickness, other
					78.4	1						
					77.4	2						
					76.4	3		Continued from non-cored borehole sheet NO CORE: 200 mm.				
NMLC	100% RETURN	93	86	Is(50) d=0.36 a=1.29 MPa	75.4	4		SANDSTONE: dark grey, medium grained, developed, laminated to thinly bedded, with SILTSTONE laminations and clasts of less than 20%.				SM, CL, 40mm BP, 0°, RF, PR, RF BP, 0°, RF, PR, RF
				Is(50) d=1.35 a=1.81 MPa				SHALE: dark grey, developed, thinly bedded.				BP, 5°, CN, CU, RF CZ, CL & RF, 60mm
				Is(50) d=0.02 MPa								

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT- Standard penetration test PT - Push tube WPT - Water pressure test	Water Inflow Partial Loss Complete Loss Graphic Log/Core Loss Core recovered (hatching indicates material) No core recovery	Weathering XW - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron QZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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PSM 3.02.2. LIB (HLZ) G.L.B. Log PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:36 10,03,00,09 Digital Fence and Map Tool | Lib: PSM 3.02.1 2019-03-06 PSM 3.02.1 2019-03-06



Engineering Log - Cored Borehole

Project No.: PSM5953

Client: Goodman	Commenced: 04/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 04/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299665 m E 6256086 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 79.42 m
Barrel Type and Length: NMLC	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Rock Substance					Rock Mass Defects		
Method	Water	TCR (%)	ROD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering	Strength Is(50)	Defect Spacing (mm)	Defect Descriptions / Comments
								ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XV HW MW SW FR	● - Axial ○ - Diametral		
NMLC	100% RETURN	93	86	Is(50) d=0.28 a=0.49 MPa	73.4	6		SHALE: dark grey, developed, thinly bedded.				BP, 0°, CN, PR, S
	100% RETURN	100	83	Is(50) d=0.14 a=0.36 MPa	72.4	7		6.7m: becomes massive to thinly bedded.				BP, 0°, CN, PR, S
	100% RETURN	100	83	Is(50) d=0.15 MPa	71.4	8		7.2m: becomes laminated to thinly bedded.				FZ, CL & RF, 50mm
	100% RETURN	100	83	Is(50) d=0.37 a=0.57 MPa	71.4	8						BP, 0°, RF, PR, S
				Is(50) d=0.04 a=0.3 MPa	70.4	9		Hole Terminated at 8.80 m Target depth				FZ, CL & RF, 50mm BP, 0°, CL & RF, PR, S CZ, CL & RF, 20mm
					70.4	9						BP, 0°, CN, PR, S BP, 2°, CN, PR, S BP, 0°, CL, PR, S BP, 0°, CL, PR, S

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT- Standard penetration test PT - Push tube WPT - Water pressure test	Water Inflow Partial Loss Complete Loss Graphic Log/Core Loss Core recovered (hatching indicates material) No core recovery	Weathering XV - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein H - High FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron OZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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PSM 3.02.2. LIB (HLZ) G.L.B. Log PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:36 10.03.00.09 Digital Fence and Map Tool | Lib: PSM 3.02.1 2019-03-06 PSM 3.02.1 2019-03-06

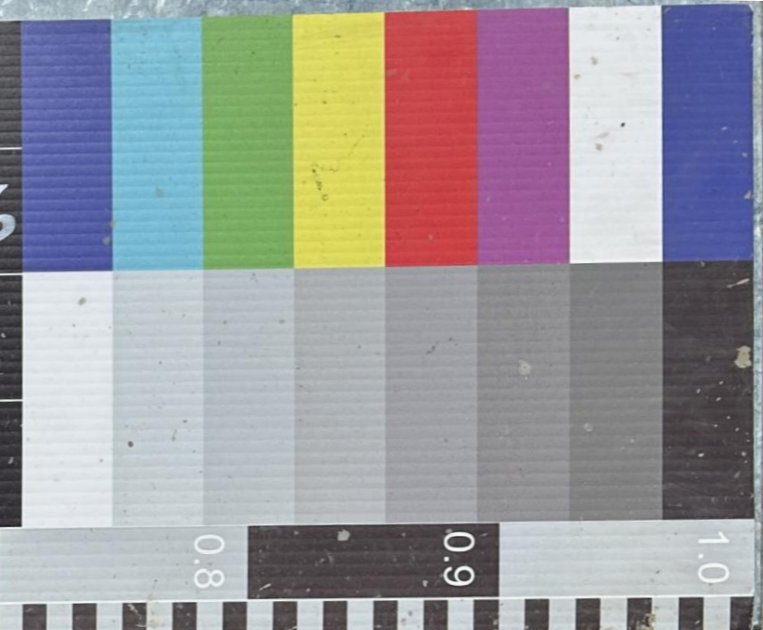


PROJECT: Project Atlas

PROJECT No: PSM5953 DATE: 04/03/26

BOREHOLE ID: BH-B

DEPTH: 3.1m - 8.8m



Note: Drill breaks are marked by white crosses

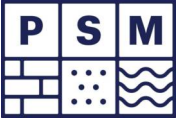
Goodman
Project Atlas
10 Roberts Road, Eastern Creek

Core Photograph (BH-B)



PSM5953-006R

APPENDIX B



Engineering Log - Non Cored Borehole

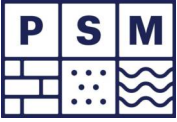
Project No.: PSM5953

Client: Goodman	Commenced: 04/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 04/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299439 m E 6255933 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 80.39 m	Operator: JK Drilling
Hole Diameter: 150 mm	Bearing:	Datum: AHD	

Drilling Information					Soil Description						Observations			
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
AD/T		N		SPT 0.50-0.51 m 5/10mm HB N=R		79.4	1			CONCRETE: 200mm thick.				0.00: Pavement
ADV		N								CONCRETE: 120 mm thick.				0.32: Possibly FILL
AD/T		N				78.4	2			FILL: Gravelly CLAY: brown, medium plasticity, gravel subangular up to 20 mm.	M (<PL)	F to St		0.50: V bit refusal. Possibly BEDROCK
										SHALE: pale brown, extremely weathered to highly weathered, very low to low strength.				
										1.5m: becomes pale grey				
										2.0m: becomes pale brown				
						77.4	3							
						76.4	4			Continued on cored borehole sheet				

Method AD/T - Auger drilling TC bit ADV - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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Borehole ID
BH C
Page 2 of 3

Engineering Log - Cored Borehole

Project No.: PSM5953

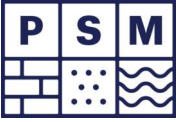
Client: Goodman	Commenced: 04/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 04/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299439 m E 6255933 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 80.39 m
Barrel Type and Length: NMLC	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Rock Substance						Rock Mass Defects														
Method	Water	TCR (%)	ROD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering			Strength Is(50)	Defect Spacing (mm)	Defect Descriptions / Comments											
								ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XW	HW	MW	SW	FR	VL 0.1	L 0.3	M 1	H 3	VH 10	EH	<20	60	200	600	1000	Description, alpha/beta, infilling or coating, shape, roughness, thickness, other
					79.4	1																			
					78.4	2																			
					77.4	3																			
					76.4	4		Continued from non-cored borehole sheet																	
NMLC	100% RETURN	95	63					NO CORE: 150 mm.																	
								SHALE: pale grey to brown, massive to thinly bedded, iron stained.																	

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT- Standard penetration test PT - Push tube WPT - Water pressure test	Water Inflow Partial Loss Complete Loss Graphic Log/Core Loss Core recovered (hatching indicates material) No core recovery	Weathering XW - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron OZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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PSM 3.02.2 LIB (HLZ) GLB Log PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:36 10.03.00.09 Digital Fence and Map Tool Lib: PSM 3.02.1 2019-03-06



Engineering Log - Cored Borehole

Project No.: PSM5953

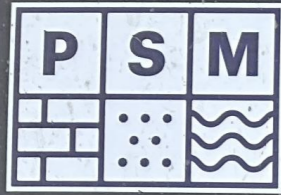
Client: Goodman	Commenced: 04/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 04/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299439 m E 6255933 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 80.39 m
Barrel Type and Length: NMLC	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Rock Substance					Rock Mass Defects		
Method	Water	TCR (%)	ROD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering	Strength Is(50)	Defect Spacing (mm)	Defect Descriptions / Comments
								ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XV HW MW SW FR	● - Axial ○ - Diametral		Description, alpha/beta, infilling or coating, shape, roughness, thickness, other
	100% RETURN	95	63	Is(50) d=0.27 a=0.47 MPa	74.4	6	SHALE: pale grey to brown, massive to thinly bedded, iron stained.					CZ, CL & RF, 50mm
				Is(50) d=0.5 a=0.69 MPa			5.6m: becomes thinly bedded					SM, CL, 90mm
												SM, CL, 10mm
												BP, 0°, CN, PR, S
												BP, 0°, CN, PR, S
												BP, 2°, CN, PR, S
												BP, 0°, CN, PR, S
				Is(50) d=0.26 MPa	73.4	7						SM, CL, 10mm
				Is(50) d=0.02 a=0.21 MPa								BP, 0°, CN, PR, S
												CZ, RF, 50mm
								SHALE: dark grey, developed, laminated to thinly bedded.				BP, 0°, CL, PR, S
												BP, 0°, CN, PR, S
				Is(50) d=0.43 a=0.62 MPa	72.4	8						SM, CL & RF, 70mm
												BP, 0°, CL, PR, S
												BP, 0°, RF, PR, S
												BP, 0°, CN, PR, S
												BP, 0°, CL, PR, S
												CZ, CL & RF, 30mm
												BP, 0°, CN, PR, S
												CZ, CL & RF, 30mm
												BP, 0°, CN, PR, S
				Is(50) d=0.24 a=0.86 MPa	71.4	9						BP, 0°, CN, PR, S
				Is(50) d=0.48 a=0.79 MPa								BP, 2°, CN, PR, S
												SM, CL, 10mm
				Is(50) d=0.4 a=0.5 MPa								BP, 0°, CN, PR, S

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT- Standard penetration test PT - Push tube WPT - Water pressure test	Water ▽ Inflow ▽ Partial Loss ▲ Complete Loss Graphic Log/Core Loss 	Weathering XV - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron OZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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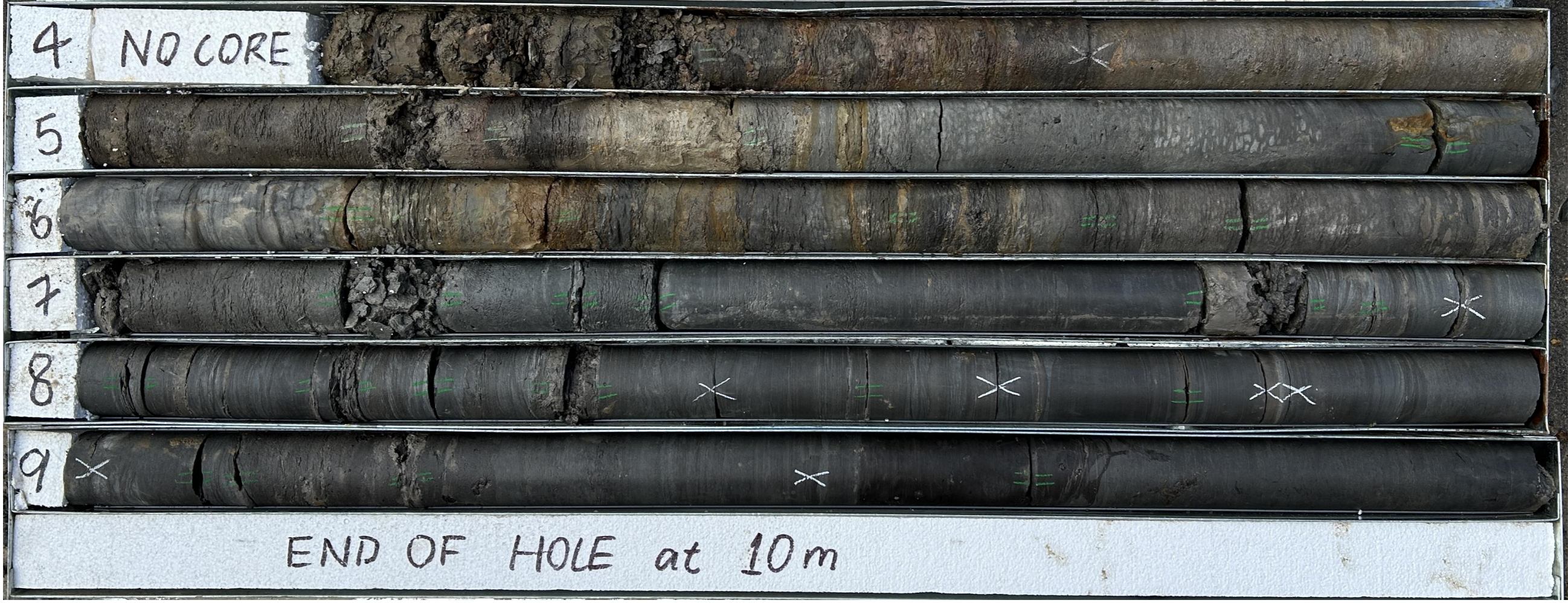
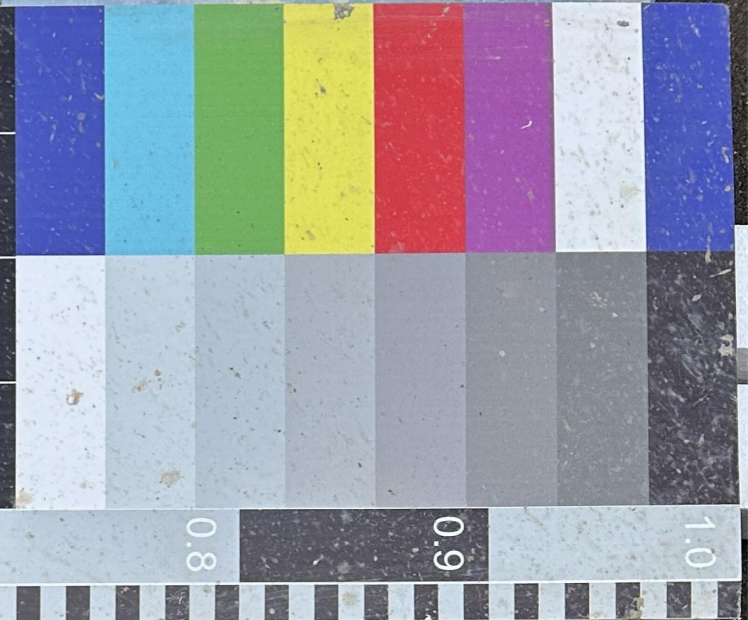


PROJECT: Project Atlas

PROJECT No: PSM5953 DATE: 04/03/26

BOREHOLE ID: BH-C

DEPTH: 4.0m - 10.0m



Note: Drill breaks are marked by white crosses

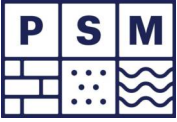
Goodman
Project Atlas
10 Roberts Road, Eastern Creek

Core Photograph (BH-C)



PSM5953-006R

APPENDIX B



Engineering Log - Non Cored Borehole

Project No.: PSM5953

Client: Goodman	Commenced: 29/01/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 29/01/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299346 m E 6256031 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 79.20 m
Hole Diameter: 150 mm	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Soil Description					Observations				
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
AD/V		N		SPT 6.00-6.20 m 10,10/50mm HB N=R		73.2	6			CLAY with gravel; pale grey, medium plasticity, gravel fine-grained sub-angular up to 5mm.	M (<PL)	St	100 200 300 400 500	
						72.2	7			SHALE; brown and pale grey, extremely weathered to highly weathered, very low to low strength.				6.20: Possibly BEDROCK 6.50: Shale fragments observed
						71.2	8							
						70.2	9							

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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Engineering Log - Non Cored Borehole

Project No.: PSM5953

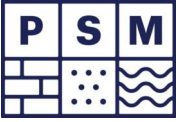
Client: Goodman	Commenced: 29/01/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 29/01/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299346 m E 6256031 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 79.20 m	
Hole Diameter: 150 mm	Bearing:	Datum: AHD	Operator: JK Drilling

Drilling Information					Soil Description						Observations			
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
AD/V		N				68.2	11			SHALE; brown and pale grey, extremely weathered to highly weathered, very low to low strength.				
						67.2	12			Continued on cored borehole sheet				
						66.2	13							
						65.2	14							

PSM 3.02.2, LIB (HLZ), G.L.B. Log PSM 5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:14 10.03.00.09 Daqgel Fence and Map Tool Lib: PSM 3.02.1 2019-03-06

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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Engineering Log - Cored Borehole

Project No.: PSM5953

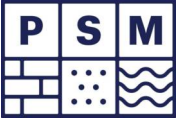
Client: Goodman	Commenced: 29/01/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 29/01/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299346 m E 6256031 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 79.20 m
Barrel Type and Length: NMLC	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Rock Substance					Rock Mass Defects		
Method	Water	TCR (%)	ROD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering	Strength Is(50)	Defect Spacing (mm)	Defect Descriptions / Comments
								ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XW HW MW SW FR	● - Axial ○ - Diametral	<20 60 200 600 1000	Description, alpha/beta, infilling or coating, shape, roughness, thickness, other
					68.2	11		Continued from non-cored borehole sheet				
	70% RETURN	100	46	Is(50) d=2.48 MPa	67.2	12		SHALE; brown and grey, laminated to thinly bedded, developed, planar, some iron staining.				SM, CL, 50mm BP, 0°, CN, PR, RF JT, 45°, CN, PR, RF BP, 0°, CN, PR, RF BP, 0°, CN, PR, RF FZ, RF & CL, IR, RF
	70% RETURN	100	46	Is(50) d=0.26 a=0.6 d=0.12 MPa	66.2	13						BP, 5°, CN, PR, RF BP, 5°, CN, IR, RF
	80% RETURN	100	50	Is(50) a=1.59 d=0.81 MPa Is(50) d=0.15 MPa Is(50) a=0.62 d=0.15 MPa	65.2	14		INTERBEDDED SANDSTONE AND SILTSTONE with bands of carbonaceous silts and clay; pale grey to dark grey, 70% SANDSTONE, 30% SILTSTONE, fine grained, developed.				BP, 20°, CN, PR, RF BP, 0°, RF, PR, RF FZ, RF, IR, RF BP, 5°, CN, PR, RF BP, 0°, CN, PR, RF BP, 0°, CN, PR, RF BP, 0°, CN, PR, RF BP, 0°, CN, PR, RF
				Is(50) a=0.3 d=0.13 MPa				SHALE; pale grey, extremely weathered, very low strength, recovered as Gravelly CLAY, medium plasticity, fine to coarse gravel, sub-angular				CZ, RF & CL, IR, RF
								INTERBEDDED SANDSTONE AND SILTSTONE with bands of carbonaceous silts and clay; pale grey				

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT- Standard penetration test PT - Push tube WPT - Water pressure test	Water ▽ Inflow △ Partial Loss ▲ Complete Loss	Weathering XW - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron OZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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Borehole ID
BH D
Page 6 of 6

Engineering Log - Cored Borehole

Project No.: PSM5953

Client: Goodman	Commenced: 29/01/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 29/01/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299346 m E 6256031 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 79.20 m
Barrel Type and Length: NMLC	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Rock Substance					Rock Mass Defects															
Method	Water	TCR (%)	ROD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering			Strength Is(50)	Defect Spacing (mm)	Defect Descriptions / Comments											
			79					ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XW	HW	MW	SW	FR	VL 0.1	L 0.3	M 1	H 3	VH 10	EH	<20	60	200	600	1000	Description, alpha/beta, infilling or coating, shape, roughness, thickness, other
					58.2	21		Hole Terminated at 20.10 m Target depth																	
					57.2	22																			
					56.2	23																			
					55.2	24																			

<p>Method</p> <p>AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT- Standard penetration test PT - Push tube</p> <p>WPT - Water pressure test</p>	<p>Water</p> <p>▽ Inflow △ Partial Loss ◼ Complete Loss</p> <p>Graphic Log/Core Loss</p> <p>▨ Core recovered (hatching indicates material) ▩ No core recovery</p>	<p>Weathering</p> <p>XW - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh</p> <p>Strength</p> <p>VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High</p>	<p>Defect Type</p> <p>FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break</p>	<p>Infilling/Coating</p> <p>CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron OZ - Quartz X - Carbonaceous</p>	<p>Roughness</p> <p>SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough</p> <p>Shape</p> <p>PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular</p>
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PSM 3.02.2 LIB (HLZ) GLB Log PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:36 10.03.00.09 Digital Fence and Map Tool | Lib: PSM 3.02.1 2019-03-06 PSM 3.02.1 2019-03-06



Note: Drill breaks are marked by red cross

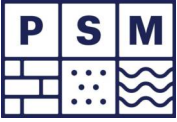


Goodman
Project Atlas
10 Roberts Road, Eastern Creek

Core Photograph (BH-D)

PSM5953-006R

APPENDIX B



Engineering Log - Non Cored Borehole

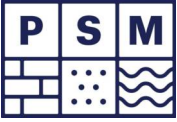
Project No.: PSM5953

Client: Goodman	Commenced: 04/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 04/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299392 m E 6256102 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 80.27 m	Operator: JK Drilling
Hole Diameter: 150 mm	Bearing:	Datum: AHD	

Drilling Information					Soil Description					Observations				
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description SOIL NAME: Plasticity, behaviour or particle characteristics of primary component, colour, secondary components, additional observations	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
DT		N				79.3	1			CONCRETE: 200mm thick.				0.00: Pavement
										CONCRETE: 140mm thick.				
										FILL: Gravelly CLAY: brown, medium plasticity, gravel subangular up to 5mm.	M (<PL)	F to St		0.34: Possibly FILL
AD/V		N				78.3	2		CI	CLAY: red brown and dark grey, medium plasticity.				1.50: Possibly NATURAL
						77.3	3				M (>PL)	St		
						76.3	4							

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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Borehole ID
BH E
Page 2 of 2

Engineering Log - Non Cored Borehole

Project No.: PSM5953

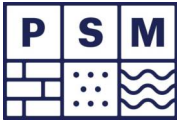
Client: Goodman	Commenced: 04/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 04/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299392 m E 6256102 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 80.27 m
Hole Diameter: 150 mm	Bearing:	Datum: AHD Operator: JK Drilling

Drilling Information				Soil Description						Observations				
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
AD/V						74.3	6		CI	CLAY: pale grey and red brown, medium plasticity.	M (>PL)			
						73.3	7		CI	CLAY trace gravel: brown, medium plasticity, gravel subangular up to 10mm.	M (<PL)			
						72.3	8			SHALE: brown, extremely weathered to highly weathered, very low to low strength.				7.00: Possibly BEDROCK
						71.3	9			9.5m: becomes pale brown.				
Hole Terminated at 10.00 m Target depth														

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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PSM 3.02.2, LIB (HLZ), G.L.B. Log PSM AU NONCORE BH NZ AU PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:14 10.03.00.09 Dageel Fence and Map Tool Lib: PSM 3.02.1 2019-03-06 Pj: PSM 3.02.1 2019-03-06



Engineering Log - Non Cored Borehole

Project No.: PSM5953

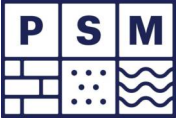
Client: Goodman	Commenced: 30/01/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 30/01/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299428 m E 6256167 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 80.73 m
Hole Diameter: 150 mm	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information				Soil Description						Observations					
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations	
AD/V	N	N	N	SPT 0.50-0.95 m 2,3,6 N=9	79.7	1	1	[Cross-hatched]	CI	CONCRETE: 200mm thick. CONCRETE: 150mm thick. FILL: Gravelly CLAY; brown and dark grey, medium plasticity, gravel fine-grained sub-angular up to 20mm	M (<PL)	F - St	100 200 300 400 500	0.00: PAVEMENT 0.35: Possibly FILL	
				SPT 1.50-1.95 m 3,5,6 N=11	78.7	2	2	[Diagonal lines]	CI	CLAY trace gravel; red and pale grey, medium plasticity, gravel fine-grained sub-angular up to 5mm. 2.5m: becomes brown	M (>PL)	St		1.60: Possibly NATURAL	
				SPT 3.00-3.45 m 3,5,6 N=11	77.7	3	3	[Diagonal lines]	CI	CLAY; red and pale grey, medium plasticity	M (>PL)	St			
				SPT 4.50-4.95 m 4,4,7 N=11	76.7	4	4	[Diagonal lines]	CI	CLAY trace gravel; brown, medium plasticity, gravel fine-grained sub-angular up to 10mm 4.8m: becomes brown	M (<PL)	St			

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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Engineering Log - Non Cored Borehole

Project No.: PSM5953

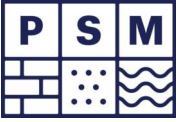
Client: Goodman	Commenced: 30/01/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 30/01/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299428 m E 6256167 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 80.73 m	Operator: JK Drilling
Hole Diameter: 150 mm	Bearing:	Datum: AHD	

Drilling Information				Soil Description						Observations				
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
AD/V				SPT 6.00-6.45 m 11,10,13 N=23		74.7	6		CI	CLAY trace gravel; brown, medium plasticity, gravel fine-grained sub-angular up to 10mm	M (<PL)	St		
						73.7	7		CI	CLAY trace gravel; red and pale grey, medium plasticity, gravel fine-grained sub-angular up to 5mm, some ironstones	M (<PL)	VSt		
				SPT 7.50-7.90 m 9,16,12/100mm HB N=R		72.7	8			SHALE; brown and pale grey, extremely weathered to highly weathered, very low to low strength				7.60: Possibly BEDROCK
						71.7	9							

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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PSM 3.02.2 LIB (HLZ) G.L.B. Log PSM AU NONCORE BH NZ AU PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:14 10.03.00.09 Dageel Fence and Map Tool Lib: PSM 3.02.1 2019-03-06 Pj: PSM 3.02.1 2019-03-06



Borehole ID
BH F
Page 3 of 5

Engineering Log - Non Cored Borehole

Project No.: PSM5953

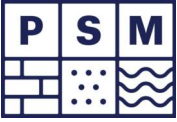
Client: Goodman	Commenced: 30/01/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 30/01/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299428 m E 6256167 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 80.73 m
Hole Diameter: 150 mm	Bearing:	Datum: AHD Operator: JK Drilling

Drilling Information					Soil Description						Observations			
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
AD/V		N				69.7	11			SHALE; brown and pale grey, extremely weathered to highly weathered, very low to low strength (<i>continued</i>)				
						68.7	12			SHALE; grey, highly weathered, low strength.				
						67.7	13			Continued on cored borehole sheet				
						66.7	14							

PSM 3.02.2 LIB (HLZ) G.L.B. Log PSM AU NONCORE BH NZ AU PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:14 10.03.00.09 Dageel Fence and Map Tool Lib: PSM 3.02.1 2019-03-06

<p>Method</p> <p>AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter</p>	<p>Penetration</p> <p> No resistance Refusal</p>	<p>Water</p> <p> Inflow Partial Loss Complete Loss</p>	<p>Samples and Tests</p> <p>U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample</p>	<p>Moisture Condition</p> <p>D - Dry M - Moist W - Wet</p>	<p>Consistency/Relative Density</p> <p>VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact</p>
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Engineering Log - Cored Borehole

Project No.: PSM5953

Client: Goodman	Commenced: 30/01/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 30/01/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299428 m E 6256167 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 80.73 m
Barrel Type and Length: NMLC 3.6 m	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Rock Substance					Rock Mass Defects		
Method	Water	TCR (%)	ROD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering	Strength Is(50)	Defect Spacing (mm)	Defect Descriptions / Comments
								ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XW HW MW SW FR	● - Axial ○ - Diametral VL 0.1 L 0.3 M 1 H 3 VH 10 EH	<20 60 200 600 1000	Description, alpha/beta, infilling or coating, shape, roughness, thickness, other
					69.7	11		Continued from non-cored borehole sheet				
				Is(50) a=0.39 d=0.07 MPa	68.7	12		SHALE; dark grey, laminated to thinly bedded, developed, planar to undulating, some clay bands observed up to 12.4m, minor iron staining.				BP, 0°, CN, PR, S FZ, RF, IR, RF BP, 0°, RF & CL, PR, S BP, 0°, RF & CL, PR, S BP, 5°, CL, CU, S BP, 0°, CL, PR, S SM, RF & CL, 160mm SM, CL, 20mm BP, 0°, CN, PR, S
	80% RETURN	78	61	Is(50) a=0.13 d=0.34 MPa	67.7	13						JT, 30°, CN, PR, RF BP, 0°, CN, PR, S
				Is(50) a=0.17 MPa				NO CORE: 500 mm thick.				
	100% RETURN	100	75	Is(50) a=0.28 d=0.23 MPa	66.7	14		INTERBEDDED SANDSTONE AND SILTSTONE; dark grey, 80% SILTSTONE, 20% SANDSTONE, fine to medium grained, developed, planar				BP, 2°, RF & CL, PR, RF

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT - Standard penetration test PT - Push tube WPT - Water pressure test	Water Inflow Partial Loss Complete Loss Graphic Log/Core Loss Core recovered (hatching indicates material) No core recovery	Weathering XW - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron OZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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Engineering Log - Cored Borehole

Project No.: PSM5953

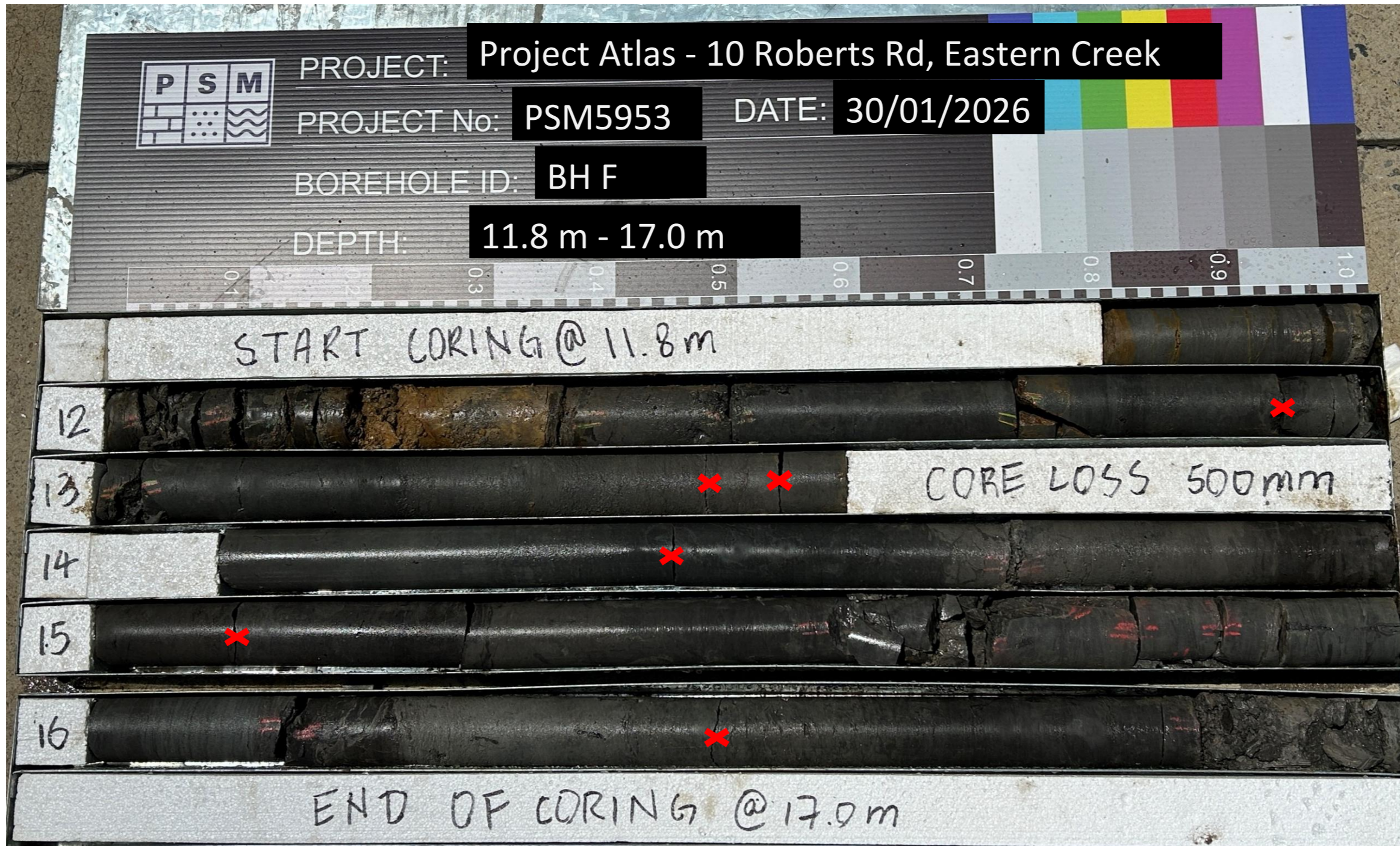
Client: Goodman	Commenced: 30/01/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 30/01/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299428 m E 6256167 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 80.73 m
Barrel Type and Length: NMLC 3.6 m	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Rock Substance					Rock Mass Defects		
Method	Water	TCR (%)	RQD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering	Strength Is(50)	Defect Spacing (mm)	Defect Descriptions / Comments
								ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XV HW MW SW FR	● - Axial ○ - Diametral	<20 60 200 600 1000	Description, alpha/beta, infilling or coating, shape, roughness, thickness, other
NMLC	100% RETURN	100	75	Is(50) a=0.32 MPa Is(50) a=0.27 d=0.08 MPa Is(50) a=0.2 d=0.15 MPa Is(50) a=0.73 d=0.53 MPa	64.7	16		INTERBEDDED SANDSTONE AND SILTSTONE; dark grey, 80% SILTSTONE, 20% SANDSTONE, fine to medium grained, developed, planar 16.0m: becomes laminated to thinly bedded, planar to undulating.				JT, 45°, RF & CL, PR, RF FZ, RF, IR, RF JT, 45°, CN, PR, RF BP, 0°, CN, PR, RF JT, 45°, CN, PR, RF BP, 0°, CN, PR, RF BP, 5°, CN, IR, RF
					63.7	17		Hole Terminated at 17.00 m Target depth				FZ, RF, IR, RF
					62.7	18						
					61.7	19						

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT- Standard penetration test PT - Push tube WPT - Water pressure test	Water Inflow Partial Loss Complete Loss Graphic Log/Core Loss Core recovered (hatching indicates material) No core recovery	Weathering XW - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron OZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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PSM 3.02.2, LIB (HLZ), GLB Log PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:36 10.03.00.09 Digital Fence and Map Tool | Lib: PSM 3.02.1 2019-03-06



Note: Drill breaks are marked by red cross



Goodman
Project Atlas
10 Roberts Road, Eastern Creek
Core Photograph (BH-F)
PSM5953-006R APPENDIX B



Engineering Log - Non Cored Borehole

Project No.: PSM5953

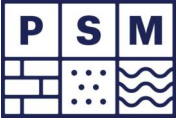
Client: Goodman	Commenced: 03/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 03/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299664 m E 6255927 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 77.18 m	Operator: JK Drilling
Hole Diameter: 150 mm	Bearing:	Datum: AHD	

Drilling Information				Soil Description						Observations				
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
AD/T		N								ASPHALT: 200mm thick.				0.00: WEARING COURSE
										FILL: Gravelly SAND: pale grey, fine grained, gravel subangular up to 25mm.	M			0.20: Possibly BASE COURSE / FILL
				SPT 0.50-0.95 m 2,2,1 N=3		76.2	1		CI	CLAY with gravel: brown, medium plasticity, gravel subangular up to 30mm. 0.7m: becomes pale grey and red brown	S to F			0.40: Possibly NATURAL
				SPT 1.50-1.95 m 5,6,7 N=13		75.2	2		CI	CLAY: red brown and pale grey, medium plasticity. 2.0m: becomes yellow brown and pale grey	M (>PL)	F to St		1.50: Shale fragments at bottom of SPT recovery, up to 20mm
				SPT 3.00-3.08 m 18/75mm HB N=R		74.2	3			SHALE: pale brown, extremely weathered to highly weathered, very low to low strength.		St		3.00: Possibly BEDROCK
						73.2	4			Continued on cored borehole sheet				

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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PSM 3.02.2, LIB (HLZ), G.L.B. Log PSM AU NONCORE BH NZ AU PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:14 10.03.00.09 Dageel Fence and Map Tool | Lib: PSM 3.02.1 2019-03-06 Proj: PSM 3.02.1 2019-03-06



Engineering Log - Cored Borehole

Project No.: PSM5953

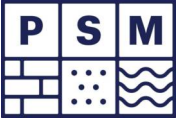
Client: Goodman	Commenced: 03/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 03/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299664 m E 6255927 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 77.18 m
Barrel Type and Length: NMLC	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Rock Substance					Rock Mass Defects				
Method	Water	TCR (%)	ROD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering	Strength Is(50)	Defect Spacing (mm)	Defect Descriptions / Comments		
								ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XW HW MW SW FR	● - Axial ○ - Diametral	<20 60 200 600 1000	Description, alpha/beta, infilling or coating, shape, roughness, thickness, other		
100% RETURN	100% RETURN	84	44					INTERBEDDED SANDSTONE AND SILTSTONE: brown, 50% SANDSTONE, 50% SILTSTONE, fine to medium grained, developed, laminated to thinly bedded, iron stained. (continued)				CZ, RF, 10mm BP, 2°, CL, CU, RF CZ, CL & RF, 40mm JT, 90°, CL & RF, PR, RF BP, 0°, CL & RF, PR, RF BP, 0°, CL & RF, PR, RF JT, 80°, CL, CU, RF CZ, RF, 40mm CZ, CL, 30mm		
					71.2	6	X	NO CORE: 300 mm.						
100% RETURN	100% RETURN	90	65	Is(50) d=0.32 a=0.33 MPa	70.2	7		SHALE: pale grey to dark grey, developed, thinly bedded, bands of iron staining from 6.3m to 6.75m.				BP, 20°, CN, PR, S BP, 5°, CN, CU, S BP, 0°, CN, PR, S BP, 2°, CN, PR, S CZ, CL & RF, 50mm BP, 2°, CL & RF, PR, S BP, 0°, CL & RF, IR, S		
100% RETURN	100% RETURN	100	89	Is(50) d=0.01 a=0.38 MPa	69.2	8						BP, 0°, CN, PR, S BP, 0°, CN, ST, S BP, 0°, CN, PR, S CZ, CL & RF, 20mm JT, 45°, CL, PR, S BP, 0°, CN, PR, S BP, 5°, CN, PR, S		
100% RETURN	100% RETURN			Is(50) d=0.04 MPa	68.2	9						BP, 0°, CN, PR, S BP, 2°, CN, PR, S JT, 45°, CL & RF, IR, S BP, 2°, CN, CU, S		

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT - Standard penetration test PT - Push tube WPT - Water pressure test	Water ▽ Inflow ▴ Partial Loss ▲ Complete Loss Graphic Log/Core Loss Core recovered (hatching indicates material) No core recovery	Weathering XW - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron OZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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PSM 3.02.2 LIB (HLZ) GLB Log PSM5953 EASTERN CREEK GPJ <<DrawingFile>> 17/03/2026 11:36 10.03.00.09 Dargal Fence and Map Tool | Lib: PSM 3.02.1 2019-03-06 PSM 3.02.1 2019-03-06



Borehole ID
BH G
Page 4 of 4

Engineering Log - Cored Borehole

Project No.: PSM5953

Client: Goodman	Commenced: 03/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 03/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299664 m E 6255927 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 77.18 m
Barrel Type and Length: NMLC	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Rock Substance					Rock Mass Defects		
Method	Water	TCR (%)	RQD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering	Strength Is(50)	Defect Spacing (mm)	Defect Descriptions / Comments
								ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XW HW MW SW FR	● - Axial ○ - Diametral VL 0.1 L 0.3 M 1 H 3 VH 10 EH	<20 60 200 600 1000	Description, alpha/beta, infilling or coating, shape, roughness, thickness, other
NMLC								SHALE: pale grey to dark grey, developed, laminated to thinly bedded, occasionally massive.				BP, 0°, CL, CU, S
100% RETURN		100	89	Is(50) d=0.06 MPa	66.2	11						XWS, CL & RF, 160mm BP, 0°, CN, PR, S BP, 0°, CL, PR, S
				Is(50) a=0.47 MPa								BP, 2°, CN, PR, S
				Is(50) d=0.4 a=0.53 MPa	65.2	12						BP, 0°, RF, PR, S BP, 0°, RF, PR, S BP, 10°, RF, IR, S
				Is(50) a=0.47 MPa								
100% RETURN		100	90	Is(50) d=0.3 a=0.52 MPa	64.2	13						BP, 0°, RF, PR, S
				Is(50) d=0.39 a=0.34 MPa								
				Is(50) d=0.03 MPa	63.2	14						BP, 0°, RF, PR, S
				Is(50) d=0.3 a=0.35 MPa								BP, 2°, RF, PR, S
				Is(50) d=0.24 a=0.5 MPa								
								Hole Terminated at 15.00 m Target depth				

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT- Standard penetration test PT - Push tube WPT - Water pressure test	Water ▽ Inflow ▴ Partial Loss ▲ Complete Loss Graphic Log/Core Loss 	Weathering XW - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron OZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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PSM 3.02.2. LIB (HLZ) G.L.B. Log PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:36 10.03.00.09 Digital Fence and Map Tool | Lib: PSM 3.02.1 2019-03-06 PSM 3.02.1 2019-03-06



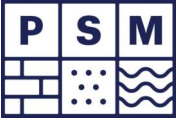
Note: Drill breaks are marked by white crosses

Goodman
 Project Atlas
 10 Roberts Road, Eastern Creek

Core Photograph (BH-G)



PSM5953-006R | APPENDIX B



Engineering Log - Non Cored Borehole

Project No.: PSM5953

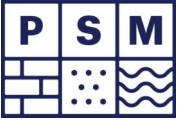
Client: Goodman	Commenced: 05/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 05/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299726 m E 6256021 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 78.87 m
Hole Diameter: 150 mm	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information				Soil Description						Observations				
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
AD/T		N								ASPHALT: 200 mm thick.				0.00: WEARING COURSE
AD/V		N		SPT 0.50-0.66 m 3,6/10mm HB N=R		77.9	1			FILL: Sandy Gravelly CLAY: brown, medium plasticity, fine to medium grained sand, gravel subangular up to 10mm.	M	St		0.20: Possibly BASE COURSE / FILL
AD/T		N				76.9	2			SHALE: pale grey to pale brown, extremely weathered to highly weathered, very low to low strength.				0.50: Shale fragments at bottom of SPT recovery, up to 25mm
										SHALE: pale brown, highly weathered, low strength.				0.70: Possibly BEDROCK
						76.9				Hole Terminated at 2.10 m TC-Bit Refusal				1.50: Some medium strength SHALE fragments observed
						75.9	3							
						74.9	4							

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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PSM 3.02.2 LIB (HLZ) G.L.B Log PSM AU NONCORE BH NZ AU PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:14 10.03.00.09 Dageel Fence and Map Tool Lib: PSM 3.02.1 2019-03-06 Pj: PSM 3.02.1 2019-03-06



Engineering Log - Non Cored Borehole

Project No.: PSM5953

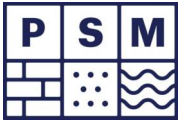
Client: Goodman	Commenced: 05/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 05/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299788 m E 6256115 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 77.83 m	Operator: JK Drilling
Hole Diameter: 150 mm	Bearing:	Datum: AHD	

Drilling Information				Soil Description						Observations				
Method	Penetration	Support	Water	Samples Tests Remarks	Recovery	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture Condition	Consistency / Relative Density	Hand Penetrometer UCS (kPa)	Structure, Zoning, Origin, Additional Observations
AD/T		N						ASPHALT: 150mm thick.						0.00: WEARING COURSE
AD/V		N		SPT 0.50-0.81 m 3,20,5/10mm HB N=R		76.8	1	FILL: Gravely Clayey SAND: brown, medium grained, gravel subrounded to subangular up to 15mm, medium grained, some plastic observed.	CI		M			0.15: Possibly BASE COURSE / FILL
		N						CLAY: pale grey and red brown, medium plasticity.			M (>PL)	S to VSt		0.35: Possibly NATURAL
AD/T		N				75.8	2	SHALE: pale brown, extremely weathered to highly weathered, very low to low strength.						0.90: Possibly BEDROCK
								2.0m: becomes pale grey to pale brown						1.10: V bit refusal, swap to TC bit
						74.8	3	Continued on cored borehole sheet						2.00: Some SANDSTONE fragments up to 15mm observed
						73.8	4							

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore SPT - Standard penetration test PT - Push tube AS - Auger screwing CT - Continuous push tube 1.5m long 76mm diameter	Penetration No resistance Refusal	Water Inflow Partial Loss Complete Loss	Samples and Tests U - Undisturbed Sample D - Disturbed Sample SPT - Standard Penetration Test ES - Environmental Sample TW - Thin Walled LB - Large Disturbed Sample	Moisture Condition D - Dry M - Moist W - Wet	Consistency/Relative Density VS - Very soft S - Soft F - Firm St - Stiff VSt - Very stiff H - Hard VL - Very loose L - Loose MD - Medium dense D - Dense VD - Very dense Ce - Cemented C - Compact
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Borehole ID
BH I
Page 2 of 4

Engineering Log - Cored Borehole

Project No.: PSM5953

Client: Goodman	Commenced: 05/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 05/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299788 m E 6256115 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 77.83 m
Barrel Type and Length: NMLC	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Rock Substance					Rock Mass Defects		
Method	Water	TCR (%)	ROD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering	Strength Is(50)	Defect Spacing (mm)	Defect Descriptions / Comments
								ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XW HW MW SW FR	● - Axial ○ - Diametral	<20 60 200 600 1000	Description, alpha/beta, infilling or coating, shape, roughness, thickness, other
					76.8	1						
					75.8	2						
					74.8	3		Continued from non-cored borehole sheet				
					73.8	4		SANDSTONE: brown and pale grey, medium grained, developed, laminated to thinly bedded, occasionally massive, with SILTSTONE laminations less than 10%, iron stained.				BP, 5°, CL, IR, RF CZ, CL & RF, 30mm CZ, CL & RF, 50mm BP, 0°, CL, PR, RF BP, 0°, CL, PR, RF BP, 0°, CL, PR, RF BP, 0°, CL, PR, RF BP, 5°, CL, CU, RF BP, 5°, CL, CU, RF BP, 5°, CN, PR, RF BP, 5°, CN, PR, RF BP, 0°, CN, PR, RF BP, 2°, CN, PR, RF BP, 5°, CN, PR, RF BP, 5°, CN, PR, RF BP, 0°, CN, PR, RF BP, 0°, CN, PR, RF BP, 0°, CL, PR, RF

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT- Standard penetration test PT - Push tube WPT - Water pressure test	Water ▽ Inflow ▴ Partial Loss ▲ Complete Loss Graphic Log/Core Loss 	Weathering XW - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - Infilled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron OZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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PSM 3.02.2. LIB (HLZ) G.L.B. Log PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:36 10.03.00.09 Digital Fence and Map Tool | Lib: PSM 3.02.1 2019-03-06



Engineering Log - Cored Borehole

Project No.: PSM5953

Client: Goodman	Commenced: 05/03/2026
Project Name: Project Atlas - 10 Roberts Road, Eastern Creek	Completed: 05/03/2026
Hole Location: See Figure 1 (Warehouse Site)	Logged By: TW
Hole Position: 299788 m E 6256115 m N MGA2020 Zone 56	Checked By: AS

Drill Model and Mounting: JK400 truck mounted drill rig	Inclination: -90°	RL Surface: 77.83 m
Barrel Type and Length: NMLC	Bearing:	Datum: AHD
		Operator: JK Drilling

Drilling Information					Rock Substance					Rock Mass Defects		
Method	Water	TCR (%)	ROD (%)	Samples and Field Tests	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering	Strength Is(50)	Defect Spacing (mm)	Defect Descriptions / Comments
								ROCK NAME: particle/grain characteristics, colour, fabric/texture, inclusions or minor components, moisture, mineral composition, alteration	XW HW MW SW FR	● - Axial ○ - Diametral		
NMLC	100% RETURN	100	56	Is(50) d=0.78 a=1.15 MPa	66.8	11		SANDSTONE: pale grey, medium grained, developed, massive, occasionally thinly bedded.				BP, 0°, CN, PR, RF
				Is(50) d=4.23 a=3.88 MPa	65.8	12						BP, 0°, CN, PR, RF
	100% RETURN	100	77	Is(50) d=1.03 a=1.71 MPa	64.8	13		SHALE: dark grey, developed, laminated.				BP, 0°, RF, PR, RF BP, 5°, CN, PR, RF BP, 0°, CN, IR, RF CZ, RF, 10mm SM, CL, 40mm CZ, CL & RF, 40mm JT, 45°, CN, CU, S XWS, CL & RF, 150mm
				Is(50) d=0.46 a=0.26 MPa	63.8	14						BP, 0°, CL & RF, PR, S
								Hole Terminated at 14.00 m Target depth				

Method AD/T - Auger drilling TC bit AD/V - Auger drilling V bit WB - Washbore HQ3- Wireline core (63.5 mm) PQ3- Wireline core (85.0 mm) SPT- Standard penetration test PT - Push tube WPT - Water pressure test	Water Inflow Partial Loss Complete Loss Graphic Log/Core Loss Core recovered (hatching indicates material) No core recovery	Weathering XW - Extremely Weathered HW - Highly Weathered MW - Moderately Weathered SW - Slightly Weathered FR - Fresh Strength VL - Very Low L - Low M - Medium H - High VH - Very High EH - Extremely High	Defect Type FT - Fault SS - Shear Surface SZ - Shear Zone BP - Bedding parting SM - Seam IS - In-filled Seam JT - Joint CO - Contact CZ - Crushed Zone VN - Vein FZ - Fracture Zone BSH - Bedding Shear DB - Drilling Break	Infilling/Coating CN - Clean SN - Stain VN - Veneer CO - Coating RF - Rock fragments G - Gravel S - Sand Z - Silt CA - Calcite CL - Clay FE - Iron OZ - Quartz X - Carbonaceous	Roughness SL - Slickensided POL - Polished S - Smooth RF - Rough VR - Very Rough Shape PR - Planar CU - Curved UN - Undulating ST - Stepped IR - Irregular
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PSM 3.02.2. LIB (HLZ) G.L.B. Log PSM5953 EASTERN CREEK.GPJ <<DrawingFile>> 17/03/2026 11:36 10.03.00.09 Digital Fence and Map Tool | Lib: PSM 3.02.1 2019-03-06 PSM 3.02.1 2019-03-06



Note: Drill breaks are marked by white crosses

Goodman
Project Atlas
10 Roberts Road, Eastern Creek

Core Photograph (BH-I)



PSM5953-006R

APPENDIX B

Appendix C Point Load Test Results





POINT LOAD STRENGTH INDEX TEST RESULTS

Job No. **PSM5953**

Sheet **1** of **4**

Project **10 Roberts Road, Eastern Creek**

Test Method **AS 4133.4.1-2007 Methods of testing rocks for engineering purposes - Determination of point load strength index**

Sampling Technique **NLMC**
Storage History **North Ryde office storage**

Sampling Date **2/03/2026 & 4/03/2026**
Testing Date **6/03/2026**

Test Machine **GSA 6510-0692**

Moisture Condition **Natural**

Tested By **TW**

Calibration Date **30/6/2025**

Loading Rate **< 30 seconds**

Rock Type	Location	Depth (m)	Diametral Tests					Axial Tests					AS 1726:2017 Strength Class	
			D (mm)	L (mm)	P (kN)	I _{s(50)} (MPa)	Failure Mode	W (mm)	D (mm)	P (kN)	I _s (MPa)	I _{s(50)} (MPa)		Failure Mode
SANDSTONE	BH-A	3.60	50	40	2.2	0.88	Parallel to bedding	50	40	11.5	4.5	4.52	Along defect	M / VH
SANDSTONE	BH-A	4.20	50	45	1.5	0.61	Parallel to bedding	50	45	3.4	1.2	1.21	Along defect	M / H
SANDSTONE	BH-A	4.80	50	30	1.6	0.63	Parallel to bedding	50	30	2.0	1.0	0.98	Along defect	M
SANDSTONE	BH-A	5.25	50	33	1.7	0.66	Parallel to bedding	50	33	3.9	1.8	1.77	Along defect	M / H
SANDSTONE	BH-A	3.09	50	30	0.7	0.29	Parallel to bedding	50	30	1.9	1.0	0.95	Along defect	L / M
SANDSTONE	BH-A	6.48	50	35	0.4	0.14	Parallel to bedding	50	35	2.4	1.1	1.06	Along defect	L / H
SHALE	BH-A	6.98	50	30				50	30	0.8	0.4	0.39	Along defect	M
SHALE	BH-A	7.78	50	32	0.0	0.02	Parallel to bedding	50	32	0.6	0.3	0.29	Along defect	#N/A
SHALE	BH-A	8.60	50	25	0.1	0.02	Parallel to bedding	50	38	0.5	0.2	0.20	Along defect	#N/A
SHALE	BH-A	8.90	50	30	0.9	0.34	Parallel to bedding	50	30	0.9	0.5	0.45	Along defect	M
SHALE	BH-A	9.95	50	30	0.3	0.14	Parallel to bedding	50	30	0.5	0.2	0.23	Along defect	L
SHALE	BH-A	10.70	50	32	0.6	0.24	Parallel to bedding	50	32	0.6	0.3	0.30	Along defect	L
SHALE	BH-A	11.10	50	33	0.3	0.12	Parallel to bedding	50	33	> 0.5	> 0.2	> 0.24	Bad break	L
SANDSTONE	BH-A	11.95	50	30	0.2	0.08	Parallel to bedding	50	30	1.7	0.9	0.82	Along defect	VL / M
SHALE	BH-A	12.45	50	42	0.8	0.33	Parallel to bedding	50	42	4.2	1.6	1.58	Along defect	M / H
SHALE	BH-A	13.45	50	30	1.5	0.60	Parallel to bedding	50	30	4.0	2.1	1.97	Along defect	M / H
SHALE	BH-A	14.40	50	33	1.0	0.42	Parallel to bedding	50	33	1.4	0.7	0.63	Along defect	M
SHALE	BH-A	15.75	50	35	7.5	3.00	Parallel to bedding	50	35	5.4	2.4	2.36	Along defect	H
SANDSTONE	BH-A	16.40	50	40	2.6	1.04	Parallel to bedding	50	40	1.9	0.7	0.74	Along defect	M / H
SANDSTONE	BH-A	17.20	50	35	1.3	0.53	Parallel to bedding	50	35	4.1	1.9	1.81	Along defect	M / H
SANDSTONE	BH-B	3.47	50	35	0.9	0.36	Parallel to bedding	50	50	3.9	1.2	1.29	Along defect	M / H
SANDSTONE	BH-B	4.33	50	70	3.4	1.35	Parallel to bedding	50	40	4.6	1.8	1.81	Along defect	H
SHALE	BH-B	4.97	50	35	0.1	0.02	Parallel to bedding	50	35	> 0.5	> 0.2	> 0.21	Bad break	#N/A
SHALE	BH-B	5.84	50	60	0.7	0.28	Parallel to bedding	50	40	1.2	0.5	0.49	Along defect	L / M
SHALE	BH-B	6.72	50	80	0.3	0.14	Parallel to bedding	50	35	0.8	0.4	0.36	Along defect	L / M
SHALE	BH-B	7.18	50	40	0.4	0.15	Parallel to bedding	50	35	> 0.7	> 0.3	> 0.31	Bad break	L / M

By: **TW**

Checked: **AS**

Date: **11/3/2026**



POINT LOAD STRENGTH INDEX TEST RESULTS

Job No. **PSM5953**

Sheet **2** of **4**

Project **10 Roberts Road, Eastern Creek**

Test Method **AS 4133.4.1-2007 Methods of testing rocks for engineering purposes - Determination of point load strength index**

Sampling Technique **NLMC**
Storage History **North Ryde office storage**

Sampling Date **4/03/2026 & 29/01/2026**
Testing Date **6/03/2026 & 30/01/2026**

Test Machine **GSA 6510-0692**

Moisture Condition **Natural**

Tested By **TW**

Calibration Date **30/6/2025**

Loading Rate **< 30 seconds**

Rock Type	Location	Depth (m)	Diametral Tests					Axial Tests					AS 1726:2017 Strength Class	
			D (mm)	L (mm)	P (kN)	I _{s(50)} (MPa)	Failure Mode	W (mm)	D (mm)	P (kN)	I _s (MPa)	I _{s(50)} (MPa)		Failure Mode
SHALE	BH-B	7.85	50	50	0.9	0.37	Parallel to bedding	50	40	1.4	0.6	0.57	Along defect	M
SHALE	BH-B	8.47	50	90	0.1	0.04	Parallel to bedding	50	40	0.8	0.3	0.30	Along defect	VL / M
SHALE	BH-C	5.65	50	65	0.7	0.27	Parallel to bedding	50	45	1.3	0.5	0.47	Along defect	L / M
SHALE	BH-C	6.05	50	31	1.3	0.50	Parallel to bedding	50	31	1.4	0.7	0.69	Along defect	M
SHALE	BH-C	6.65	50	40	0.7	0.26	Parallel to bedding	50	40	> 0.1	> 0.1	> 0.06	Bad break	L
SHALE	BH-C	7.04	50	40	0.1	0.02	Parallel to bedding	50	35	0.5	0.2	0.21	Along defect	#N/A
SHALE	BH-C	7.64	50	38	1.1	0.43	Parallel to bedding	50	38	1.5	0.6	0.62	Along defect	M
SHALE	BH-C	8.47	50	45	0.6	0.24	Parallel to bedding	50	42	2.3	0.8	0.86	Along defect	L / M
SHALE	BH-C	8.96	50	43	1.2	0.48	Parallel to bedding	50	40	2.0	0.8	0.79	Along defect	M
SHALE	BH-C	9.62	50	40	1.0	0.40	Parallel to bedding	50	40	1.3	0.5	0.50	Along defect	M
SHALE	BH-D	12.52	50	55	6.2	2.48	Parallel to bedding	50	48	> 1.5	> 0.5	> 0.52	Bad break	H
SHALE	BH-D	12.85	50	45	0.6	0.26	Parallel to bedding							L
SHALE	BH-D	12.85	50	35	0.3	0.12	Parallel to bedding	50	40	1.8	0.7	0.71	Along defect	L / M
SHALE	BH-D	13.27	50	65	0.4	0.15	Parallel to bedding							L
SHALE	BH-D	13.23	50	35	2.0	0.81	Parallel to bedding	50	45	4.4	1.5	1.59	Along defect	M / H
SHALE	BH-D	13.58	50	75	0.4	0.15	Parallel to bedding	50	48	1.9	0.6	0.64	Along defect	L / M
INTERBEDDED SANDSTONE AND SILTSTONE	BH-D	14.32	50	60	1.3	0.53	Parallel to bedding	50	50	> 3.4	> 1.1	> 1.12	Bad break	M / H
INTERBEDDED SANDSTONE AND SILTSTONE	BH-D	14.93	50	75	0.3	0.13	Parallel to bedding	50	35	0.9	0.4	0.40	Along defect	L / M
INTERBEDDED SANDSTONE AND SILTSTONE	BH-D	15.97	50	25	1.5	0.62	Parallel to bedding							M
INTERBEDDED SANDSTONE AND SILTSTONE	BH-D	16.25	50	50	4.4	1.74	Parallel to bedding	50	35	5.6	2.5	2.44	Along defect	H
INTERBEDDED SANDSTONE AND SILTSTONE	BH-D	16.34	50	40	2.3	0.91	Parallel to bedding	50	40	6.1	2.4	2.40	Along defect	M / H
INTERBEDDED SANDSTONE AND SILTSTONE	BH-D	17.81	50	85	6.9	2.75	Parallel to bedding							H
INTERBEDDED SANDSTONE AND SILTSTONE	BH-D	17.79	50	60	6.7	2.66	Parallel to bedding	50	45	10.2	3.6	3.67	Along defect	H / VH
INTERBEDDED SANDSTONE AND SILTSTONE	BH-D	18.25	50	45	3.2	1.30	Parallel to bedding	50	40	4.5	1.8	1.77	Along defect	H
INTERBEDDED SANDSTONE AND SILTSTONE	BH-D	18.76	50	70	1.5	0.58	Parallel to bedding							M
INTERBEDDED SANDSTONE AND SILTSTONE	BH-D	19.65	50	70	2.0	0.79	Parallel to bedding	50	45	> 2.7	> 0.9	> 0.97	Bad break	M

By: **TW**

Checked:

AS

Date: **11/3/2026**



POINT LOAD STRENGTH INDEX TEST RESULTS

Job No. **PSM5953**

Sheet **3** of **4**

Project **10 Roberts Road, Eastern Creek**

Test Method *AS 4133.4.1-2007 Methods of testing rocks for engineering purposes - Determination of point load strength index*

Sampling Technique *NLMC*
Storage History *North Ryde office storage*

Sampling Date *30/01/2026 & 03/03/2026*
Testing Date *30/01/2026 & 06/03/2026*

Test Machine *GSA 6510-0692*

Moisture Condition *Natural*

Tested By *TW*

Calibration Date *30/6/2025*

Loading Rate *< 30 seconds*

Rock Type	Location	Depth (m)	Diametral Tests					Axial Tests					AS 1726:2017 Strength Class	
			D (mm)	L (mm)	P (kN)	I _{s(50)} (MPa)	Failure Mode	W (mm)	D (mm)	P (kN)	I _s (MPa)	I _{s(50)} (MPa)		Failure Mode
SHALE	BH-F	11.82	50	50	0.2	0.07	Parallel to bedding	50	45	1.1	0.4	0.39	Along defect	VL / M
SHALE	BH-F	12.68	50	50	0.8	0.34	Parallel to bedding	50	45	0.4	0.1	0.13	Along defect	L / M
SHALE	BH-F	13.26	50	45	> 0.5	> 0.2	Bad break	50	45	> 0.5	> 0.2	> 0.17	Bad break	≥ L
SHALE	BH-F	13.57	50	40	> 0.5	> 0.21	Bad break	50	45	0.6	0.2	0.20	Along defect	L
INTERBEDDED SANDSTONE AND SILTSTONE	BH-F	14.49	50	50	0.6	0.23	Parallel to bedding	50	45	0.8	0.3	0.28	Along defect	L
INTERBEDDED SANDSTONE AND SILTSTONE	BH-F	15.07	50	45	0.9	0.3	Parallel to bedding	50	45	0.9	0.3	0.32	Along defect	M
INTERBEDDED SANDSTONE AND SILTSTONE	BH-F	15.96	50	40	0.2	0.08	Parallel to bedding	50	40	0.8	0.3	0.30	Along defect	VL / L
INTERBEDDED SANDSTONE AND SILTSTONE	BH-F	16.06	50	55	0.4	0.15	Parallel to bedding	50	40	0.6	0.2	0.22	Along defect	L
INTERBEDDED SANDSTONE AND SILTSTONE	BH-F	16.53	50	40	1.3	0.53	Parallel to bedding	50	45	2.0	0.7	0.73	Along defect	M
INTERBEDDED SANDSTONE AND SILTSTONE	BHG	4.65	50	25	0.9	0.35	Parallel to bedding	50	35	2.9	1.3	1.27	Along defect	M / H
INTERBEDDED SANDSTONE AND SILTSTONE	BHG	5.32	50	40	0.2	0.08	Parallel to bedding	50	40	1.2	0.5	0.47	Along defect	VL / M
SHALE	BHG	6.80	50	40	0.8	0.32	Parallel to bedding	50	40	0.8	0.3	0.33	Along defect	M
SHALE	BHG	7.50	50	45	0.0	0.01	Parallel to bedding	50	33	0.8	0.4	0.38	Along defect	#N/A
SHALE	BHG	8.20	50	0.3	> 0.2	> 0.08	Bad break	50	33	0.8	0.4	0.38	Along defect	≥ VL
SHALE	BHG	9.25	50	35	0.1	0.04	Parallel to bedding	50	35	> 0.2	> 0.1	> 0.08	Bad break	VL
SHALE	BHG	10.60	50	20	0.2	0.06	Parallel to bedding	50	35	> 0.2	> 0.1	> 0.09	Bad break	VL
SHALE	BHG	10.98	50	48	1.4	0.4	Parallel to bedding	50	48	1.4	0.4	0.47	Along defect	M
SHALE	BHG	11.37	50	40	1.0	0.40	Parallel to bedding	50	40	1.3	0.5	0.53	Along defect	M
SHALE	BHG	12.05	50	48	0.7	0.30	Parallel to bedding	50	48	1.4	0.4	0.47	Along defect	L / M
SHALE	BHG	12.90	50	33	0.8	0.30	Parallel to bedding	50	33	1.1	0.5	0.52	Along defect	M
SHALE	BHG	13.28	50	38	1.0	0.39	Parallel to bedding	50	38	0.8	0.3	0.34	Along defect	M
SHALE	BHG	13.73	50	30	0.1	0.03	Parallel to bedding	50	38	0.8	0.3	0.34	Along defect	M
SHALE	BHG	13.80	50	30	> 0.4	> 0.2	Bad break	50	30	> 0.4	> 0.2	> 0.21	Bad break	≥ L
SHALE	BHG	14.05	50	38	0.8	0.30	Parallel to bedding	50	38	0.9	0.4	0.35	Along defect	M
SHALE	BHG	14.75	50	40	0.6	0.24	Parallel to bedding	50	40	1.3	0.5	0.50	Along defect	L / M

By: **TW**

Checked:

AS

Date: **11/3/2026**



POINT LOAD STRENGTH INDEX TEST RESULTS

Job No. **PSM5953**

Sheet **4** of **4**

Project **10 Roberts Road, Eastern Creek**

Test Method *AS 4133.4.1-2007 Methods of testing rocks for engineering purposes - Determination of point load strength index*

Sampling Technique *NLMC*
Storage History *North Ryde office storage*

Sampling Date *4/3/2026*
Testing Date *6/03/2026*

Test Machine *GSA 6510-0692*

Moisture Condition *Natural*

Tested By *TW*

Calibration Date *30/6/2025*

Loading Rate *< 30 seconds*

Rock Type	Location	Depth (m)	Diametral Tests					Axial Tests					AS 1726:2017 Strength Class	
			D (mm)	L (mm)	P (kN)	I _{s(50)} (MPa)	Failure Mode	W (mm)	D (mm)	P (kN)	I _s (MPa)	I _{s(50)} (MPa)		Failure Mode
SANDSTONE	BH-1	3.85	50	40	0.6	0.23	Parallel to bedding	50	40	1.2	0.5	0.48	Along defect	L / M
SANDSTONE	BH-1	4.35	50	35	0.8	0.31	Parallel to bedding	50	35	3.6	1.6	1.58	Along defect	M / H
SANDSTONE	BH-1	5.23						50	30	1.6	0.8	0.76	Along defect	M
SANDSTONE	BH-1	5.70	50	35	2.0	0.80	Parallel to bedding	50	30	1.2	0.6	0.60	Along defect	M
SANDSTONE	BH-1	3.09						50	30	0.6	0.3	0.27	Along defect	L
SANDSTONE	BH-1	6.40	50	40	10.2	4.09	Parallel to bedding	50	30	8.9	4.7	4.41	Along defect	VH
SANDSTONE	BH-1	7.58	50	45	1.9	0.77	Parallel to bedding	50	45	3.0	1.0	1.08	Along defect	M / H
SANDSTONE	BH-1	8.20	50	42	2.6	1.03	Parallel to bedding	50	42	3.7	1.4	1.39	Along defect	H
SANDSTONE	BH-1	9.87	50	40	0.3	0.12	Parallel to bedding	50	40	1.2	0.5	0.46	Along defect	L / M
SANDSTONE	BH-1	10.80	50	40	2.0	0.78	Parallel to bedding	50	40	2.9	1.1	1.15	Along defect	M / H
SANDSTONE	BH-1	11.60	50	40	10.6	4.23	Parallel to bedding	50	40	9.8	3.9	3.88	Along defect	VH
SANDSTONE	BH-1	12.33	50	40	2.6	1.03	Parallel to bedding	50	40	4.3	1.7	1.71	Along defect	H
SHALE	BH-1	12.72	50	45	0.9	0.34	Parallel to bedding	50	32	1.5	0.7	0.68	Along defect	M
SHALE	BH-1	13.15	50	32	1.2	0.46	Parallel to bedding	50	45	0.7	0.3	0.26	Along defect	L / M

By: **TW**

Checked: **AS**

Date: **11/3/2026**

Appendix D CBR Results



FOUR DAY SOAKED CALIFORNIA BEARING RATIO TEST REPORT

Client: PSM Holdings Aust. Pty Limited
PSM Job No.: PSM5953

Report No.: L5160 - 1
Report Date: 4/11/2025
Page 1 of 1

BOREHOLE NUMBER	BH 10
DEPTH (m)	0.20 - 0.80
Surcharge (kg)	4.5
Maximum Dry Density (t/m ³)	1.68 STD
Optimum Moisture Content (%)	17.8
Moulded Dry Density (t/m ³)	1.65
Sample Density Ratio (%)	98
Sample Moisture Ratio (%)	97
Moisture Contents	
Insitu (%)	10.9
Moulded (%)	17.3
After soaking and	
After Test, Top 30mm(%)	32.4
Remaining Depth (%)	37.0
Material Retained on 19mm Sieve (%)	0
Swell (%)	2.0
C.B.R. value:	@2.5mm penetration 4.0

- NOTES:** Sampled and supplied by client. Samples tested as received.
- Refer to appropriate Borehole logs for soil descriptions
 - Test Methods : AS 1289 6.1.1, 5.1.1 & 2.1.1.
 - Date of receipt of sample and date tested: 23/10/20205 & 03/11/2025.



NATA Accredited Laboratory
Number:1327

Accredited for compliance with ISO/IEC 17025 - Testing.
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in full without approval of the laboratory. Results relate only to
the items tested or sampled.



04/11/2025
Authorised Signature / Date
(D. Treweek)

FOUR DAY SOAKED CALIFORNIA BEARING RATIO TEST REPORT

Client: PSM Holdings Aust. Pty Limited
PSM Job No.: PSM5953

Report No.: L5160 - 2
Report Date: 11/11/2025
Page 1 of 1

BOREHOLE NUMBER	BH 3	BH 12
DEPTH (m)	1.00 - 1.50	1.00 - 2.00
Surcharge (kg)	4.5	4.5
Maximum Dry Density (t/m ³)	1.69 STD	1.73 STD
Optimum Moisture Content (%)	19.5	18.6
Moulded Dry Density (t/m ³)	1.66	1.70
Sample Density Ratio (%)	98	98
Sample Moisture Ratio (%)	102	101
Moisture Contents		
Insitu (%)	22.4	19.1
Moulded (%)	19.8	18.9
After soaking and		
After Test, Top 30mm(%)	29.8	31.0
Remaining Depth (%)	22.4	21.3
Material Retained on 19mm Sieve (%)	0	0
Swell (%)	1.5	2.5
C.B.R. value:		1.5
	@2.5mm penetration	
	@5.0mm penetration	3.0

- NOTES:** Sampled and supplied by client. Samples tested as received.
- Refer to appropriate Borehole logs for soil descriptions
 - Test Methods : AS 1289 6.1.1, 5.1.1 & 2.1.1.
 - Date of receipt of sample and date tested: 03/11/2025 & 07/11/2025.



NATA Accredited Laboratory
Number:1327

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the items tested or sampled.



11/11/2025
Authorised Signature / Date
(D. Treweek)

Appendix E Aggressivity and Salinity Test Results





CERTIFICATE OF ANALYSIS

Work Order : **ES2533127**
Client : **PSM Holdings Aust Pty Limited**
Contact : HENRY ZHANG
Address : G3, 56 Delhi Road
North Ryde 2113
Telephone : ----
Project : PSM5953
Order number : PSM5953
C-O-C number : ----
Sampler : Tsz In Wong
Site : ----
Quote number : EN/333
No. of samples received : 2
No. of samples analysed : 2

Page : 1 of 3
Laboratory : Environmental Division Sydney
Contact : Customer Services ES
Address : 277-289 Woodpark Road Smithfield NSW Australia 2164
Telephone : +61-2-8784 8555
Date Samples Received : 23-Oct-2025 15:05
Date Analysis Commenced : 27-Oct-2025
Issue Date : 30-Oct-2025 11:46



Accreditation No. 825
Accredited for compliance with
ISO/IEC 17025 - Testing

This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted, unless the sampling was conducted by ALS. This document shall not be reproduced, except in full.

This Certificate of Analysis contains the following information:

- General Comments
- Analytical Results

Additional information pertinent to this report will be found in the following separate attachments: Quality Control Report, QA/QC Compliance Assessment to assist with Quality Review and Sample Receipt Notification.

Signatories

This document has been electronically signed by the authorized signatories below. Electronic signing is carried out in compliance with procedures specified in 21 CFR Part 11.

<i>Signatories</i>	<i>Position</i>	<i>Accreditation Category</i>
Ankit Joshi	Senior Chemist - Inorganics	Sydney Inorganics, Smithfield, NSW



General Comments

The analytical procedures used by ALS have been developed from established internationally recognised procedures such as those published by the USEPA, APHA, AS and NEPM. In house developed procedures are fully validated and are often at the client request.

Where moisture determination has been performed, results are reported on a dry weight basis.

Where a reported less than (<) result is higher than the LOR, this may be due to primary sample extract/digestate dilution and/or insufficient sample for analysis.

Where the LOR of a reported result differs from standard LOR, this may be due to high moisture content, insufficient sample (reduced weight employed) or matrix interference.

When sampling time information is not provided by the client, sampling dates are shown without a time component. In these instances, the time component has been assumed by the laboratory for processing purposes.

Where a result is required to meet compliance limits the associated uncertainty must be considered. Refer to the ALS Contract for details.

Key : CAS Number = CAS registry number from database maintained by Chemical Abstracts Services. The Chemical Abstracts Service is a division of the American Chemical Society.
LOR = Limit of reporting
^ = This result is computed from individual analyte detections at or above the level of reporting
ø = ALS is not NATA accredited for these tests.
~ = Indicates an estimated value.

- ED007 and ED008: When Exchangeable Al is reported from these methods, it should be noted that Rayment & Lyons (2011) suggests Exchange Acidity by 1M KCl - Method 15G1 (ED005) is a more suitable method for the determination of exchange acidity (H⁺ + Al³⁺).
- ED045G: The presence of Thiocyanate, Thiosulfate and Sulfite can positively contribute to the chloride result, thereby may bias results higher than expected. Results should be scrutinised accordingly.



Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)		Sample ID		BH07 (1-1.5m)	BH08 (0.5-1m)	----	----	----
Sampling date / time		23-Oct-2025 00:00		23-Oct-2025 00:00		----	----	----
Compound	CAS Number	LOR	Unit	ES2533127-001	ES2533127-002	-----	-----	-----
				Result	Result	----	----	----
EA002: pH 1:5 (Soils)								
pH Value	----	0.1	pH Unit	5.7	6.6	----	----	----
EA010: Conductivity (1:5)								
Electrical Conductivity @ 25°C	----	1	µS/cm	356	718	----	----	----
EA055: Moisture Content (Dried @ 105-110°C)								
Moisture Content	----	1.0	%	14.4	14.4	----	----	----
EA080: Resistivity								
Resistivity at 25°C	----	1	ohm cm	2810	1390	----	----	----
ED008: Exchangeable Cations								
Cation Exchange Capacity	----	0.1	meq/100g	4.0	8.2	----	----	----
Exchangeable Sodium Percent	----	0.1	%	19.5	23.2	----	----	----
ED040S : Soluble Sulfate by ICPAES								
Sulfate as SO4 2-	14808-79-8	10	mg/kg	170	110	----	----	----
ED045G: Chloride by Discrete Analyser								
Chloride	16887-00-6	10	mg/kg	400	1310	----	----	----



CERTIFICATE OF ANALYSIS

Work Order : **ES2534407**
Client : **PSM Holdings Aust Pty Limited**
Contact : HENRY ZHANG
Address : G3, 56 Delhi Road
North Ryde 2113
Telephone : ----
Project : PSM5953
Order number : PSM5953
C-O-C number : ----
Sampler : TSZIN WONG
Site : ----
Quote number : EN/333
No. of samples received : 4
No. of samples analysed : 4

Page : 1 of 3
Laboratory : Environmental Division Sydney
Contact : Customer Services ES
Address : 277-289 Woodpark Road Smithfield NSW Australia 2164
Telephone : +61-2-8784 8555
Date Samples Received : 03-Nov-2025 14:50
Date Analysis Commenced : 05-Nov-2025
Issue Date : 06-Nov-2025 16:42



Accreditation No. 825
Accredited for compliance with
ISO/IEC 17025 - Testing

This report supersedes any previous report(s) with this reference. Results apply to the sample(s) as submitted, unless the sampling was conducted by ALS. This document shall not be reproduced, except in full.

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- Analytical Results

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<i>Signatories</i>	<i>Position</i>	<i>Accreditation Category</i>
Ankit Joshi	Senior Chemist - Inorganics	Sydney Inorganics, Smithfield, NSW
Edwandy Fadjar	Organic Coordinator	Sydney Inorganics, Smithfield, NSW



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LOR = Limit of reporting
^ = This result is computed from individual analyte detections at or above the level of reporting
ø = ALS is not NATA accredited for these tests.
~ = Indicates an estimated value.

- ALS is not NATA accredited for the analysis of Exchangeable Cations on Alkaline Soils when performed under ALS Method ED006.
- ED007 and ED008: When Exchangeable Al is reported from these methods, it should be noted that Rayment & Lyons (2011) suggests Exchange Acidity by 1M KCl - Method 15G1 (ED005) is a more suitable method for the determination of exchange acidity (H⁺ + Al³⁺).
- ED045G: The presence of Thiocyanate, Thiosulfate and Sulfite can positively contribute to the chloride result, thereby may bias results higher than expected. Results should be scrutinised accordingly.



Analytical Results

Sub-Matrix: SOIL (Matrix: SOIL)				Sample ID	BH03 (0.5-1m)	BH03 (1.5-1.95m)	BH05 (1-1.5m)	BH12 (0.5-0.95m)	----
Sampling date / time				31-Oct-2025 00:00	31-Oct-2025 00:00	31-Oct-2025 00:00	31-Oct-2025 00:00	----	
Compound	CAS Number	LOR	Unit	ES2534407-001	ES2534407-002	ES2534407-003	ES2534407-004	-----	
				Result	Result	Result	Result	----	
EA002: pH 1:5 (Soils)									
pH Value	----	0.1	pH Unit	9.4	5.1	6.3	8.0	----	
EA010: Conductivity (1:5)									
Electrical Conductivity @ 25°C	----	1	µS/cm	299	364	315	209	----	
EA055: Moisture Content (Dried @ 105-110°C)									
Moisture Content	----	1.0	%	15.5	16.3	16.2	17.0	----	
EA080: Resistivity									
Resistivity at 25°C	----	1	ohm cm	3340	2750	3170	4780	----	
ED006: Exchangeable Cations on Alkaline Soils									
∅ Cation Exchange Capacity	----	0.2	meq/100g	12.9	----	----	11.3	----	
∅ Exchangeable Sodium Percent	----	0.2	%	21.9	----	----	25.5	----	
ED008: Exchangeable Cations									
Cation Exchange Capacity	----	0.1	meq/100g	----	5.9	6.1	----	----	
Exchangeable Sodium Percent	----	0.1	%	----	23.4	15.2	----	----	
ED040S : Soluble Sulfate by ICPAES									
Sulfate as SO4 2-	14808-79-8	10	mg/kg	80	200	20	160	----	
ED045G: Chloride by Discrete Analyser									
Chloride	16887-00-6	10	mg/kg	110	440	440	150	----	

Appendix F Bulk Earthworks Specification (PSM5953-007S)



10 Roberts Road and Lenore Drive after Old Wallgrove Road, Eastern Creek

Bulk Earthworks Specification Filling, Cutting
and Testing

PSM5953-007S 17 November 2025

Goodman

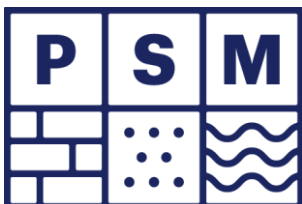


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Appendix A Subgrade Approval Report

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Appendix C Daily Report

Appendix D Certification Letter (Sample Only)



1. Scope

This specification details the requirements for the bulk earthworks to be undertaken at 10 Roberts Road and Lenore Drive after Old Wallgrove Road, Eastern Creek (the Site). The area where this specification is applicable is shown in Figure 1. This includes areas where material is filled to bulk earthworks level (BEL) within the Site.

Fill placed in accordance with this specification is denoted as Engineered Fill.

This specification does not address any environmental, contamination or erosion issues with respect to the fill material.

2. Site Preparation Works

The following tasks shall be undertaken as part of the Site Preparation Works:

1. To prepare the site for the earthworks:
 - a. Removal of stockpiles and mounds.
 - b. Clearing of the area including removal and disposal of all trees, stumps, roots, bush, other organic material, all vegetation both living and dead, all minor man-made structures (e.g. fences) and all rubbish.
 - c. Grubbing operations shall be carried out to a minimum depth of 0.5m below the surface, where grubbing is required.
 - d. Stripping of topsoil.
 - e. Demolition of structures as directed by the Principal. Extent of demolition works are not addressed by this Specification.
 - f. Decommissioning the services from the pre-existing infrastructure. This is to include backfilling any voids such that they do not collapse or undergo excessive settlement under the weight of the filling and building loads. Backfilling is to be undertaken with one of the following materials:
 - i. Cement stabilised sand (min. 3% cement) placed in accordance with the supplier requirements or
 - ii. Mass concrete or grout as approved by PSM.
 - iii. Engineered fill placed in accordance with Clauses 3.5 and 3.6 of the Specification.

Where any excavation is required to complete the above tasks, the surface exposed at completion of the excavation shall be treated in accordance with the Subgrade Preparation requirements in Clause 3.1.

3. Filling Works

3.1 Subgrade Preparation

The condition of the subgrade should be assessed immediately prior to the commencement of filling.

All Engineered Fill is to be placed on one of the following materials:

1. Bedrock.
2. Natural insitu material of at least stiff consistency.
3. Engineered compacted fill placed in accordance with this or other approved specifications for which the Geotechnical Inspection and Testing Authority (GITA) has a Level 1 certificate certifying compliance with that approved specification AND of at least stiff consistency.
4. Existing fill / pavement and other materials as approved by PSM.

Proof rolling shall only be undertaken under the direction of PSM. PSM may also direct a bridging layer of Engineered Fill be placed and compacted to a Dry or Hilf Density Ratio (Standard Compaction) of between 98% and 102%. Any such layer shall be a Lot under Clause 6.3.



The GITA should satisfy itself that the subgrade has not been desiccated, affected by rain or disturbed. If the GITA cannot so satisfy itself, then the subgrade should be compacted and moisture conditioned to be in accordance with Clauses 3.5 and 3.6 of this specification.

Engineered Fill shall be placed only on subgrade approved by the GITA as being in accordance with this specification.

3.2 Base Geometry

The slope of any buried batter shall be less (flatter) than 1H:1V unless otherwise directed by PSM.

The contractor shall remove or flatten any geometrical obstructions (e.g. protrusions or holes) such that subsequent Engineered Fill can be placed to achieve the requirements of this specification.

Engineered Fill shall be placed only on areas where the base geometry has been approved by the GITA.

3.3 Material

3.3.1 Imported Fill

Imported Engineered Fill is to conform to one of the following definitions:

1. "Virgin excavated natural material" (VENM) as defined by the Protection of the Environment Operations Act 1997 No 156, Schedule 1, on Page 209:
"Virgin excavated natural material (eg clay, gravel, sand, soil and rock) that is not mixed with any other waste and that:
 - iv. has been excavated from areas that are not contaminated, as a result of industrial, commercial, mining or agricultural activities, with manufactured chemicals and that does not contain sulphide ores or soils, or
 - v. consists of excavated natural materials that meet such criteria as may be approved by the EPA".
2. "Excavated natural material" (ENM) as defined under Clause 93 of the Protection of the Environment Operations (Waste) Regulation 2014:
"Excavated natural material is naturally occurring rock and soil (including but not limited to materials such as sandstone, shale, clay and soil) that has:
 - i. been excavated from the ground, and
 - ii. contains at least 98% (by weight) natural material, and
 - iii. does not meet the definition of Virgin Excavated Natural Material in the Act.
 - iv. Excavated Natural Material does not include material that has been located in a hotspot; that has been processed; or that contains asbestos, Acid Sulphate Soils (ASS), Potential Acid Sulphate soils (PASS) or sulfidic ores."

3.3.2 Site Won Material

Site Won material shall comprise material won from excavations on site including natural and existing fill, including crushed concrete pavement on site. They need to satisfy Clause 3.3.3.

3.3.3 All Fill

The Engineered Fill shall be approved by the GITA as suitable for use in a structural fill.

Engineered Fill shall not comprise unsuitable material that includes:

- organic soils, such as many topsoils, severely root-affected subsoils and peat
- silts, or materials that have the deleterious engineering properties of silt
- other materials with properties that are unsuitable for the forming of structural fill; unless it is approved by PSM,

The GITA shall assess that the proportion of deleterious material in each Lot is not greater than 1% by weight. Deleterious material is defined by Table 3051.6 of the RMS QA Specification 3051 (Edition 7 June 2020) as:

"Rubber, Plastic, Paper, Cloth, Paint, Wood and Other Vegetable Matter"



If the GITA is not able to visually assess the above criterion, the GITA shall arrange appropriate testing.

All Engineered Fill particles shall be able to be incorporated within a single layer. Further, less than 30% of particles shall be retained on the 37.5 mm sieve.

Engineered Fill shall be able to be tested in accordance with the Standard Compaction method (AS1289.5.4.1) or Hilf test method (AS1289.5.7.1). These methods require less than 20% retained on the 37.5 mm sieve. Where between 20% and 30% of particles are retained on the 37.5 mm sieve the above test methods shall still be adopted and test reports annotated appropriately.

These requirements should be met by the material after placement and compaction.

Only material approved by the GITA shall be placed as Engineered Fill.

3.4 Fill Zonation and Placement

HOLD POINT

Process Held	Placing of Fill
Submission detail	The Contractor / GITA submit to PSM a Weekly Certificate as defined in Section 7.2.1 of this specification for the earthworks completed to the previous Saturday no later than 5 pm of the subsequent Wednesday.
Release of Hold Point	PSM to confirm receipt of Weekly Certificate and recommend release of Hold Point if initial assessment of the Weekly Certificate indicates it complies with requirements of this specification. The contract superintendent should then release the Hold Point if it considers appropriate.

Engineered Fill shall be placed in accordance with the following requirements:

1. In near horizontal, laterally extensive layers of uniform material and thickness, deposited systematically across the work area as determined by the GITA.
2. The compacted thickness of each layer shall be equal to or less than 300 mm.

Engineered Fill shall only be placed on subgrade in accordance with this specification and approved by the GITA.

3.5 Compaction

Engineered Fill shall be placed and compacted to a Dry or Hilf Density Ratios (Standard Compaction) of between 98% and 102%.

The insitu density shall be measured over the full depth of each layer placed.

3.6 Moisture Control

The placement moisture variation or Hilf moisture variation shall be controlled to be between 2% dry of optimum and 2% wet of optimum.

Placement moisture content of the Engineered Fill shall be measured.

4. Cutting

4.1 Subgrade Condition

The subgrade is to comprise one of the following materials:

1. Bedrock.
2. Natural insitu material of at least stiff consistency.
3. Existing fill and other materials as approved by PSM.

Proof rolling shall only be undertaken under the direction of PSM.

The GITA should satisfy itself that the subgrade has not been desiccated, affected by rain or disturbed. If the GITA cannot so satisfy itself, then the subgrade should be excavated and filled to the BEL in accordance with this specification.



5. Survey

5.1 Filling Areas

The survey requirements are as follows:

1. Any approved subgrade shall be surveyed prior to first filling such that subgrade levels are established to within ± 0.1 m. The area subject to approval shall be assessed and shown on a plan drawing to an accuracy of at least ± 5 m in plan.
2. The Lot boundaries shall be assessed and shown on a plan drawing to an accuracy of at least ± 5 m in plan.
3. The location of the field density tests shall be assessed and shown on the Lot boundary plan drawing to an accuracy of at least ± 5 m in plan.
4. The elevation of the field density tests shall be surveyed to an accuracy of ± 0.05 m.

The plan drawing shall show at the boundaries of the site and other identifiable site features, so as to allow the location of the lots and the test to be recoverable.

5.2 Cutting Areas

Any approved subgrade for cut areas shall be surveyed such that subgrade levels are established to within ± 0.1 m.

6. Inspection and Testing

6.1 Role of the GITA

A NATA accredited Geotechnical Inspection and Testing Authority (GITA) shall be contracted to document and certify that the works undertaken by the contractor has been completed in accordance with the relevant design and specifications.

6.2 Level 1 Control

The GITA shall adopt Level 1 responsibility as described in Section 8.2 of AS 3798-2007 "Guidelines on earthworks for commercial and residential developments":

"The primary objective of Level 1 Inspection and Testing is for the geotechnical inspection and testing authority (GITA) to be able to express an opinion on the compliance of the work. The GITA is responsible for ensuring that the inspection and testing are sufficient for this purpose.

The geotechnical inspection and testing authority needs to have competent personnel on site at all times while earthwork operations are undertaken. Such operations include:

- *Completion of removal of top soil*
- *Placing of imported or cut material*
- *Compaction and adding/removal of moisture*
- *Trenching and backfilling*
- *Test rolling*
- *Testing*

The superintendent should agree a suitable inspection and testing plan prior to commencement of the works.

On completion of the earthworks, the GITA will usually be required to provide a report setting out the inspections, sampling and testing it has carried out, and the locations and results thereof. Unless very unusual conditions apply, the GITA should also be able to express an opinion that the works (as far as it has been able to determine) comply with the requirements of the specification and drawings."

For this particular contract, Level 1 responsibility includes:

1. Lot testing as per Clause 6.3 of this specification.

2. A frequency of compaction testing not less than that specified in Clause 6.4 of this specification.
3. The GITA documenting and reporting its activity in the terms required by Clause 7 of this specification.
4. The GITA undertaking adequate inspections and testing to comply with the above requirements and to be able to certify the fill in the terms required by Clause 7 of this specification.

6.3 Lot Testing

This specification requires lot testing to be undertaken.

A Lot is defined as a single layer of Engineered Fill consisting of uniform material which has undergone similar treatment (both moisture conditioning and compaction) and that represents no more than one day's work.

Lot testing comprises the following:

1. A Lot shall be identified by the Contractor or the GITA with a Lot Number and presented for testing.
2. A Lot shall be deemed to be in accordance with the specification if all the tests undertaken within the Lot are in accordance with the specification, i.e. "a none to fail basis".
3. If any one test undertaken within a Lot fails, the whole of the Lot shall be reworked and retested.

Any portion of the placed Engineered Fill must be part of a single lot and all Lots will require approval by the GITA.

6.4 Testing Frequency (Compaction Testing)

The frequency of compaction testing for each lot shall not be less than the greater of:

1. For lot less than 50 m³:
 - a. 1 test per lot.
2. For lot between 50 m³ and 100 m³:
 - a. 2 tests per lot.
3. For lot greater than 100 m³:
 - a. 1 test per 500 m³ of material placed
 - b. 3 tests per lot.

A laboratory moisture content test shall be undertaken for each field density test. Tests shall be located randomly across the Lot.

6.5 Proof Rolling and Plate Load Testing

Proof rolling, together with minor boxing out and refilling, of the upper surface of the bulk earthworks will be undertaken as directed by PSM. The plant to be adopted depends upon the design loads adopted by the structural engineers for each section of the site.

Plate load testing shall be undertaken at direction of PSM at final bulk earthworks level (BEL). Expected test frequency is approximately a day of testing for each building pad.

6.6 Inspection, Testing and Survey

The GITA shall at least undertake the following tasks:

Cut areas

1. Identify the subgrade as one of the subgrade types listed in Clause 4.1 of this specification and assess that the subgrade condition of cut areas is in accordance with the subgrade condition requirements of Clause 4.1 of this specification.
2. Should Engineered Fill be required to fill overcut areas, assess that filling has been placed in accordance with this specification.

Fill areas

3. Identify the subgrade as one of the subgrade types listed in Clause 3.1 of this specification and assess that the subgrade condition of any area prior to placement of fill material is in accordance with the subgrade preparation requirements of Clause 3.1 of this specification. Should the subgrade material comprise "Other



materials as approved by PSM, eg. existing fill intended to be left in place.”, PSM should be requested to inspect and provide approval prior to filling.

The GITA needs to include / refer to PSM approval in its weekly report for subgrade comprising existing fill and other materials as approved by PSM

4. Assess that the base geometry of any area prior to placement of fill material is in accordance with the base geometry requirements of Clause 3.2 of this specification.
5. Assess that the material placed is in accordance with the fill material requirements of Clause 3.3 of this specification.
6. Assess the proportion of deleterious material is in accordance with the requirements of Clause 3.3.3.
7. Assess that the Engineered Fill has been placed in accordance with the requirements for fill zonation and placement of Clause 3.4 of this specification.
8. Assess that each Lot as presented for approval by the contractor is in accordance with the requirements for Lot definition of Clause 6.3 of this specification.
9. Ensure that the survey requirements in Clause 5 of this specification have been completed.
10. Estimate the approximate volume of Engineered Fill placed in each Lot presented for approval.
11. Conduct Lot testing in accordance with the construction control testing requirements of Clauses 6.3 and 6.4 of this specification.
12. Assess that the compaction of each Lot is in accordance with the requirements of Clause 3.5 of this specification. The GITA shall select a depth of insitu density tests that allows the density of the full layer to be assessed.
13. Assess that the moisture variation of each Lot is in accordance with the requirements for moisture control in Clause 3.6 of this specification.
14. Conduct material property testing in accordance with the material testing requirements in this specification.

7. Reporting and Certification

7.1 Reporting

The GITA shall produce at least the following reports:

1. *VENM / ENM Validation Reports*. Such a report shall transmit the VENM or ENM validation certificates for the fill imported to site. Such validation reports could be provided by an environmental consultant engaged by the Contractor.
2. *Subgrade Approval Reports* (a sample is attached). Such a report shall:
 - a. Document assessments undertaken for tasks 1 and task 3 of Clause 6.6 including reporting the subgrade type.
 - b. Document the subgrade survey that has been undertaken.
 - c. Approve or reject the subgrade condition and base geometry for filling, based on tasks 3 and 4 of Clause 6.6.
 - d. Approve or reject the subgrade condition for cut areas based on task 1 and 3.
3. *Lot Approval Reports* (a sample is attached). Such a report shall:
 - a. Document assessments, testing and survey undertaken for tasks 3 to 14 of Clause 6.6.
 - b. Report the results of testing undertaken for task 11 of Clause 6.6.
 - c. Approve or reject lots based on tasks 12 and 13 of Clause 6.6.
4. *Material Testing Reports*. Such a report shall:
 - a. Report the results of material property testing undertaken for task 14 of Clause 6.6.
5. *Daily Reports* (a sample is attached). Such a report shall be completed daily and shall:
 - a. Document time spent on site by the GITA personnel.
 - b. List subgrade assessments and approvals undertaken each day with reference to relevant Subgrade Approval Report(s).



- c. List Lots presented, accepted and approved or rejected each day, with reference to relevant Lot Approval Report(s).
- d. List survey undertaken each day as for task 9 of Clause 6.6 and not already documented in the Subgrade or Lot Approval Reports.
- e. Document other relevant activities undertaken on site that day (site instructions, breakdowns, compaction equipment used, etc.).

7.2 Certification

7.2.1 Weekly Certificates

The GITA shall produce a Weekly Certificate for any week in which earthworks are undertaken in accordance with this specification. The Weekly Certificate will cover all works from the previous Weekly Certificate until the end of work on a Saturday.

The Weekly Certificate shall transmit the following:

- Copy or reference to the complete specification document(s).
- Subgrade Approval Reports.
- Lot Approval Reports.
- Material property testing reports.
- Daily Reports.
- Survey of subgrade geometry prior to filling or in cut areas.
- Plan survey drawing showing lot boundaries and location of density tests.
- Survey documenting filling undertaken to date and showing location of testing.
- Provide an Excel spreadsheet presenting the results of the week's acceptance testing completed by the GITA.

And certify that:

“All the earthworks undertaken and the subgrade condition in the cut areas [in the stated period] are documented in the above reports and have been undertaken in accordance with the Specification (Ref. PSM5953-007S)”.

7.2.2 Interim or Final Filling Certificate

At the completion of the bulk earthworks, or as requested by the Client, the GITA shall provide an Interim or Final Filling Certificate which shall:

1. Transmit a reference list of the Weekly Certificates.
2. Provide an Excel spreadsheet presenting the results of all the acceptance testing completed by the GITA.
3. Certify that *“All the earthworks undertaken and the subgrade condition in the cut areas [in the stated period] are documented in the above reports and have been undertaken in accordance with the Specification (Ref. PSM5953-007S Rev 0).”*



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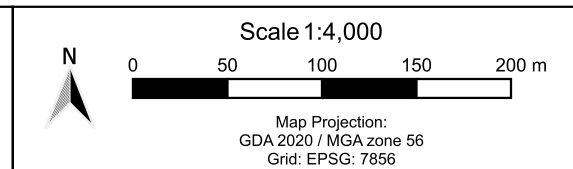
Level 3 22 Delhi Street
West Perth WA 6005
+61 8 9462 8400





- Legend**
- Warehouse Site - 10 Roberts Road
 - Transgrid Site - Lenore Drive after Wallgrove Road

Notes:
 1. Aerial imagery sourced from nearmap.com dated 10 October 2025.
 2. Site boundaries are approximate.



Goodman
 Project Atlas 10 Roberts Road
 Eastern Creek NSW

SITE LOCALITY PLAN

PSM	Created By:	TW	Revision:	A
	Date:	10 Nov 2025	Paper Size:	A3

PSM5953-007S Figure 1

Appendix A

Subgrade Approval Report



GEOTECHNICAL INSPECTION AND TESTING AUTHORITY

NATA accreditation number



SUBGRADE APPROVAL REPORT

Client:	Contractor:
Job number:	Report number:
Project:	Technician:

Subgrade areas assessed:

Area ID	Date	Approximate extent	Subgrade description	Geometry summary	Specification reference	Compliance (Pass/Fail)	Survey reference	Approved (Yes/No)

COMMENTS:

Signed: _____ Date: _____

Appendix B

Lot Approval Report





GEOTECHNICAL INSPECTION AND TESTING AUTHORITY
NATA accreditation number

LOT APPROVAL REPORT

Client: _____	Report number: _____
Job number: _____	Report date: _____
Project: _____	Technician: _____
Contractor: _____	Test methods: _____

LOT ID: _____	Sheet _____	of _____
Retest (Yes/No) _____	Original test report number: _____	
Specification reference _____	_____	
Location: _____	_____	
Lot boundary survey reference/location: _____	_____	
Materials description: _____	<i>(MATERIAL TYPE, colour, minor components, maximum particle size)</i>	
Material identification: _____	<i>(Identify the material as defined in Clause 2.3.1, Clause 2.3.2 or Clause 2.3.3 of the Specification)</i>	
Deleterious material assessment: _____	<i>(Report proportion of deleterious material)</i>	
Layer thickness: _____	_____	
Accepted as Lot: (Yes/No) _____	Date: _____	_____
Approximate volume (m3) _____	Number of tests required: _____	

Test ID No.				
Test soil description				
Date tested:				
Grid reference				
Surveyed test locations (RL,E,N)				
Test depth (mm)				
Max size (mm)				
% Oversize material (wet)				
Field wet density (t/m ³)				
Field moisture content (%)				
PWCD (t/m ³)				
Compactive effort				
Moisture variation (%)				
HILF density ratio (%)				
TEST (Pass/Fail)				

LOT APPROVAL	(Pass/Fail)	Signed: _____	Date: _____
---------------------	-------------	---------------	-------------

Appendix C

Daily Report





GEOTECHNICAL INSPECTION AND TESTING AUTHORITY

NATA accreditation number

DAILY REPORT

Client:	Report number:
Job number:	Report date:
Project:	Level of testing: Level 1
Location:	Technician:
Contractor	

Time on site:
Time off site:

1. Subgrade Approval

Areas ID	Subgrade Approval Report No:	Comments

2. Lot Approval

Lot ID	Lot Approval Report No:	Comments

3. Survey

Type of survey	Survey undertaken by:	Reference

4. Instructions received on site

--

5. Instructions given on site

--

COMMENTS:

--

Signed:

Date:

Appendix D

Certification Letter (Sample Only)



Our Ref:

Date:

Addressed to: Earthwork Contractor

Attention: Earthwork Contractor Representative

Dear

**RE: SAMPLE INTERIM (OR FINAL) FILLING CERTIFICATE
INDUSTRIAL DEVELOPMENT, BULK EARTHWORKS
CERTIFICATION OF EARTHWORKS
BETWEEN [DATE OF COMMENCEMENT] AND [DATE OF COMPLETION]**

In the period between [date start] and [date finish] the contractor has undertaken earthworks in areas XXX and XXX.

During the above period:

- The GITA has prepared the following Subgrade Approval Reports:

1. Subgrade Approval Report No 1
2.

- The GITA has prepared the following Lot Approval Reports:

1. Lot Approval Report No 1
2.

- The GITA has prepared the following Daily Reports:

1. Daily Report No 1.....
2.

- The following subgrade survey was undertaken:

1. Subgrade Survey reference.....
2.

- The following weekly survey was undertaken:

1. Weekly survey of week endingreference.....
2.

Copies of all the above documents are attached.

The GITA certifies that all the earthworks undertaken in the above stated period are documented in the above reports and have been undertaken in accordance with the Specifications (ref. PSM3821-006S, dated XXX) a copy of which is attached, with the exception of:

1. List outstanding issues (not approved subgrade, lots, unsuitable material, failed tests etc.)
2.

Signed

GITA