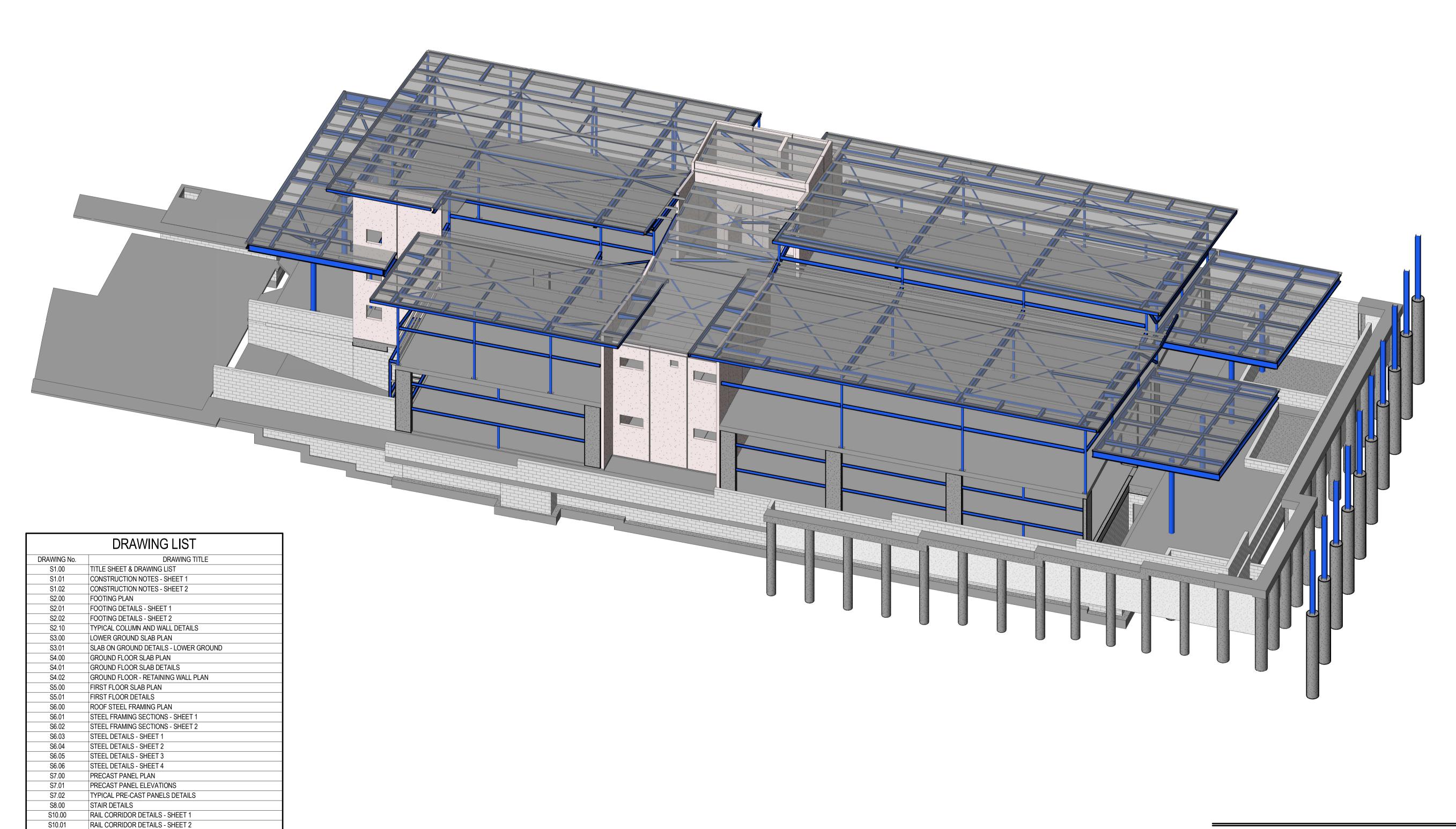
NEW LEARNING CENTRE 6A WATSFORD ROAD, CAMPBELLTOWN



FOR CONSTRUCTION

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Α	ISSUED FOR CONSTRUCTION	HW	D.M.	05-08-2021	Arch
4	ISSUED FOR CONSTRUCTION CERTIFICATE	S.W.	D.M.	08-01-2021	
3	ISSUED FOR TENDER	S.W.	D.M.	24-11-2020	K(
2	ISSUED FOR SSDA	HW	D.M.	05-03-2020	
1	ISSUED FOR TENDER	K.S.	N.V.	19-12-2019	
REVISION	AMENDMENT	DRAWN	DESIGNED	DATE	

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Project NEW LEARNING CENTRE	Drawn H.W.	Designed D.M.	Date DEC. 2	019
6A WATSFORD ROAD, CAMPBELLTOWN	Checked D.M.	Approved R.K.	Scale	
Title TITLE SHEET & DRAWING LIST	Drawing number	'	'	Revision
TITLE SHEET & DRAWING LIST	19712	-S1.00		A

NEW LEARNING CENTRE 6A WATSFORD ROAD, CAMPBELLTOWN

GENERAL NOTES:

- THESE DRAWINGS SHALL NOT BE USED FOR CONSTRUCTION UNTIL THEY ARE MARKED 'FOR CONSTRUCTION' AND APPROVED BY THE RELEVANT AUTHORITIES.
- THE WORD 'ENGINEER' AS USED IN THESE NOTES REFERS TO AN EMPLOYEE OR NOMINATED REPRESENTATIVE OF H & H CONSULTING ENGINEERS P/L (TRADING
- STRUCTURAL DRAWINGS AND NOTES SHALL BE READ IN CONJUNCTION WITH ALL ARCHITECTURAL AND OTHER CONSULTANT'S DRAWINGS REPORTS. SPECIFICATIONS AND ANY OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED. DURING THE COURSE OF CONSTRUCTION. ANY DISCREPANCIES SHALL BE REFERRED TO THE ENGINEER FOR CLARIFICATION OR DECISION BEFORE PROCEEDING WITH THE WORK.
- UNDIMENSIONED DISTANCES SHALL NOT BE OBTAINED BY SCALING OFF THE STRUCTURAL DRAWINGS OR MEASURING FROM THE ELECTRONIC DRAWINGS. DIMENSIONED SIZES OF ALL STRUCTURAL ELEMENTS AS SHOWN ON HENRY & HYMAS DRAWINGS SHALL TAKE PRECEDENCE OVER THOSE SHOWN ON OTHER CONSULTANT'S DRAWINGS. IT IS THE BUILDING CONTRACTOR'S RESPONSIBILITY TO VERIFY ALL DIMENSIONS PRIOR TO SETTING-OUT ON SITE.
- STRUCTURAL ELEMENTS INDICATED ON THESE DRAWINGS ARE SHOWN IN THEIR INTENDED COMPLETE STATE. UNLESS NOTED OTHERWISE ON THE DRAWINGS, THE BUILDING CONTRACTOR IS RESPONSIBLE FOR THE DESIGN AND ERECTION OF TEMPORARY WORKS INCLUDING PROPPING BRACING SHORING AND ANY OTHER REQUIREMENTS THAT ARE NECESSARY TO MAINTAIN THE STRUCTURE, OR ANY PART OF IT, IN A STABLE CONDITION DURING CONSTRUCTION. THE BUILDER SHALL OBTAIN ADVICE FROM APPROPRIATELY QUALIFIED AND EXPERIENCED PERSONNEL FOR THIS PURPOSE.
- NO PART OF THE STRUCTURE SHALL BE CONSTRUCTED ON OR ADJACENT TO ANY OF THE FOLLOWING UNLESS THE HAZARDS AND THE MITIGATION MEASURES IF REQUIRED, ARE INDICATED ON THE STRUCTURAL DRAWINGS;
 - EMBANKMENTS, BATTERS, WATER RETAINING STRUCTURES, RETAINING WALLS, PITS, SEWERS, SERVICE TRENCHES, DRAINAGE CANALS, CREEKS OR ANY OTHER POTENTIAL SOURCE OF DAMAGE TO THE STRUCTURE. IF ANY SUCH HAZARDS ARE ENCOUNTERED, THE ENGINEER SHALL BE NOTIFIED AND APPROVAL OBTAINED PRIOR TO PROCEEDING.
- THE BUILDING CONTRACTOR SHALL LOCATE ALL EXISTING AND PROPOSED SERVICES AND EASEMENTS, ON AND ADJACENT TO THE SITE. APPROVALS FROM THE RELEVANT STATUTORY AUTHORITIES AND THE ENGINEER SHALL BE OBTAINED PRIOR TO BUILDING ON OR OVER ANY SERVICES OR EASEMENTS.
- EXCAVATION WORK SHALL NOT BE CARRIED OUT BELOW THE LEVEL OF ANY ADJOINING BUILDING FOOTINGS WITHOUT EXCLUSIVE APPROVAL FROM THE ENGINEER. THE BUILDING CONTRACTOR MUST OBTAIN WRITTEN CONSENT FROM THE ADJOINING PROPERTY OWNERS PRIOR TO THE INSTALLATION OF UNDERPINNING, GROUND ANCHORS, DRAINAGE LINES OR ANY OTHER WORK BEYOND THE PROPERTY BOUNDARY.
- NO HOLES OR CHASES SHALL BE MADE IN ANY STRUCTURAL ELEMENT, UNLESS SHOWN ON THESE DRAWINGS OR WRITTEN APPROVAL IS OBTAINED FROM THE
- G10 A FULL DEPTH 'V' JOINT SHALL BE PROVIDED IN RENDER WHERE TWO DIFFERING STRUCTURAL MATERIALS MEET. EG. AT THE JUNCTION OF MASONRY WITH
- G11 ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH THE RELEVANT AUSTRALIAN STANDARDS, THE NATIONAL CONSTRUCTION CODE (FORMERLY BCA), AND THE REQUIREMENTS OF THE RELEVANT STATUTORY AUTHORITIES. ALL WORKMANSHIP SHALL CONFORM TO GOOD TRADE PRACTICE.
- G12 WATERPROOFING REQUIREMENTS SHALL BE AS SPECIFIED BY THE ARCHITECT AND ARE NOT NECESSARILY INDICATED ON STRUCTURAL DRAWINGS.
- G13 ONLY THE LATEST REVISIONS OF THE NOMINATED AUSTRALIAN STANDARDS SHALL APPLY WHERE REFERENCED ON THE DRAWINGS.
- G14 IT MUST BE NOTED THAT APPROVAL OF A SUBSTITUTION OR ALTERNATIVE FROM THE ENGINEER IS NOT. IN ITSELF AN AUTHORISATION FOR A VARIATION.
- G15 THE ENGINEER SHALL BE GIVEN AT LEAST 48 HOURS NOTICE FOR SITE

FLOOR SLAB DESIGN LOADS:

SUPERIMPOSED LOADS ARE GENERALLY IN ACCORDANCE WITH AS1170.1. AND AS NOTED BELOW UNLESS MORE STRINGENT REQUIREMENTS ARE SPECIFIED ELSEWHERE IN THE DOCUMENTATION.

LC	OCATION	SUPERIMPOSED DEAD LOAD (SDL) - kPa	LIVE LOAD (LL) - kPa
PUBLIC SCHOOLS	- GENERAL	1.5	3.0
	- STEEL ROOF	0.4	0.25
OTHER	- FIRE STAIRS	0.1	4.0
	- ACCESS STAIRS	1.5	4.0
	- AMENITIES	0.5	2.0
	- CORRIDORS & LOBBIES	1.5	4.0
	- TERRACES / BALCONIES	1.5	4.0

- L2 WIND LOADS ARE IN ACCORDANCE WITH AS1170.2 AND AS FOLLOWS:
 - WIND REGION

IMPORTANCE LEVEL

TERRAIN CATEGORY

• ANNUAL PROBABILITY OF EXCEEDANCE -1/1000

- REGIONAL WIND VELOCITY(VR) L3 EARTHQUAKE LOADS ARE IN ACCORDANCE WITH AS1170.4 AND AS FOLLOWS:
- IMPORTANCE LEVEL ANNUAL PROBABILITY OF EXCEEDANCE -1/1000
- SUBSOIL CLASS HAZARD FACTOR
- EARTHQUAKE DESIGN CATEGORY EDC II
- SNOW LOADS ARE IN ACCORDANCE WITH AS1170.3 AND AS FOLLOWS:
 - IMPORTANCE LEVEL
 - SNOW REGION
- ANNUAL PROBABILITY OF EXCEEDANCE 1/200 PROBABILITY FACTOR

FOUNDATIONS:

- FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT No. CES180704_ZDC-AB DATED 18/10/2018 PREPARED BY CONSULTING EARTH SCIENTISTS.
- FOOTINGS AND FOUNDATIONS HAVE BEEN DESIGNED FOR THE FOLLOWING BEARING PRESSURES:

PAD FOOTINGS - 150 kPa STRIP FOOTINGS - 150 kPa BORED PILES

FOUNDATION MATERIAL SHALL BE APPROVED BY THE CONSULTING GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF CONCRETE.

SAFETY IN DESIGN:

SUPERINTENDENT IMMEDIATELY.

DURING DEMOLITION AND SAFE DISPOSAL

/ RECYCLING OF MATERIALS ETC.)

- 1 SAFETY IN DESIGN IS THE INTEGRATION OF CONTROL MEASURES FARLY IN THE DESIGN PROCESS TO ELIMINATE OR, IF NOT REASONABLY PRACTICABLE, MINIMISE RISKS TO HEALTH AND SAFETY THROUGHOUT THE LIFE OF THE STRUCTURE BEING
- SAFETY IN DESIGN APPLIES TO THE DESIGN AND CONSTRUCTION PHASES, AND ALSO TO OPERATION, MAINTENANCE AND RENOVATION OF THE BUILDING OR FACILITY AS A WORKPLACE, AND FINALLY DEMOLITION OF THE STRUCTURE AT THE END OF ITS USEFUL LIFE.
- 2. ALL DESIGNERS INVOLVED IN THIS PROJECT MUST COMPLY WITH THE 'SAFE DESIGN OF STRUCTURES - CODE OF PRACTICE (2014)' PUBLISHED BY NSW WORKCOVER UNDER THE 'WORK HEALTH AND SAFETY ACT OF 2011' SOME OTHER DOCUMENTS RELEVANT TO SAFETY IN DESIGN ARE AS FOLLOWS;
- PRACTICAL GUIDE TO PLANNING THE SAFE ERECTION OF STEEL STRUCTURES (1ST EDITION OCTOBER 2016) - PUBLISHED BY STEEL AUSTRALIA
- EXCAVATION WORK CODE OF PRACTICE 2015 SAFEWORK AUSTRALIA DEMOLITION WORK CODE OF PRACTICE 2016 – SAFEWORK AUSTRALIA
- 3. IF ANY FURTHER ISSUES ARE IDENTIFIED BY ANY PARTY INVOLVED IN THIS PROJECT THAT ARE NOT INCLUDED IN THE SAFETY IN DESIGN REPORT, SUCH ISSUES SHALL BE BROUGHT TO THE NOTICE OF THE PROJECT MANAGER/
- 4. NORMAL HAZARDS AND RISKS DURING ALL LIFE CYCLE STAGES ARE THE RESPONSIBILITY OF THE RELEVANT PARTIES DURING THE STAGE CONCERNED, AS

LIFE CYCLE STAGE	RESPONSIBILITY
CONSTRUCTION	PRINCIPAL BUILDING CONTRACTOR
OCCUPATION	OWNER / OPERATOR
MAINTENANCE	OWNER / OPERATOR / MAINTENANCE CONTRACTOR
DECONSTRUCTION (DECOMMISSIONING OF SERVICES / UTILITIES. STABILITY OF THE STRUCTURE	DEMOLITION CONTRACTOR

REVISION

BORED OR DRIVEN PILES:

BP1 BORED PILES HAVE BEEN DESIGNED FOR THE SAFE WORKING LOADS INDICATED ON THE PLANS BASED ON THE RECOMMENDED ALLOWABLE BEARING PRESSURE AND SHAFT ADHESION VALUES. REFER TO THE GEOTECHNICAL REPORT FOR SITE SPECIFIC GEOTECHNICAL INFORMATION.

DRIVEN PILES SHALL BE INSTALLED ON A 'DESIGN & CONSTRUCT' BASIS.

WHERE PILES ARE INSTALLED ON A 'DESIGN & CONSTRUCT' BASIS, THE PILING CONTRACTOR SHALL DESIGN AND INSTALL THE PILES IN ACCORDANCE WITH AS2159 AND THE PROJECT SPECIFICATIONS. ULTIMATE LOADS MAY BE DETERMINED BY MILTIPLYING THE WORKING LOADS BY A FACTOR OF 1.4. IT IS THE PILING CONTRACTOR'S RESPONSIBILITY TO GATHER FURTHER INFORMATION IF REQUIRED, THAT MAY NOT BE INCLUDED IN THE GEOTECHNICAL REPORT.

FOR THE ENGINEER'S REVIEW PRIOR TO PILE INSTALLATION. BP3 BORED PILES MAY HAVE TO BE LINED IN WEAK SOILS TO PREVENT COLLAPSE CONTINUOUS FLIGHT AUGER (CFA) PILES OR 'TREMIE' METHOD OF CONCRETING

DESIGN CALCULATIONS AND THE INSTALLATION METHODS SHALL BE SUBMITTED

IT IS THE PILING CONTRACTOR'S RESPONSIBILITY TO USE APPROPRIATE PILING TECHNIQUES BASED ON THE RECOMMENDATIONS GIVEN IN THE GEOTECHNICAL

MAY BE USED IF GROUND WATER IS ENCOUNTERED.

RECTIFICATION SYSTEMS.

BP4 CENTRELINE OF THE PILES SHALL ALIGN WITH THE CENTRELINE OF THE COLUMNS

ABOVE OR SYMMETRICALLY LOCATED UNDER THE PILECAPS AS APPLICABLE, UNO.

- REFER TO THE ARCHITECTURAL DRAWINGS FOR THE COLUMN SETOUT. BP5 THE PILING CONTRACTOR SHALL VERIFY THE FOUNDING MATERIAL AND THE
- DEPTH OF PILES PRIOR TO PLACING REINFORCEMENT AND POURING CONCRETE BP6 ALL PILES HAVE BEEN DESIGNED TO CARRY THE DESIGN LOADS AT A MAXIMUM OUT-OF-POSITION TOLERANCE OF 75 mm IN ACCORDANCE WITH AS2159. UNLESS THE PILING CONTRACTOR CAN DEMONSTRATE OTHERWISE, ANY PILES WHICH ARE

ALL PILES SHALL BE INSTALLED USING A RIG CAPABLE OF MAINTAINING A MAXIMUM VERTICAL TOLERANCE OF ± 20 mm PER METRE LENGTH.

OUT OF POSITION BY MORE THAN 75 mm WOULD REQUIRE APPROPRIATE

- BP7 NOTIFY THE ENGINEER IMMEDIATELY IF ANY OBSTRUCTIONS ARE ENCOUNTERED OTHER THAN THOSE INDICATED IN THE GEOTECHNICAL REPORT.
- CONCRETE SHALL BE PLACED IN BORED PILES TO ENSURE A SOUND AND MONOLITHIC COMPACTED CONCRETE SHAFT UP TO THE CUT-OFF LEVEL. TAKE APPROPRIATE MEASURES TO AVOID SEGREGATION, BLEEDING AND GROUT DEFICIENCY OF THE PILE
- BP9 EACH PILE SHALL BE TRIMMED TO ± 25 mm OF THE CUT-OFF LEVEL, ANY DAMAGE CAUSED TO THE PILES DURING TRIMMING MUST BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.
- BP10 UPON COMPLETION OF PILING, THE PILING CONTRACTOR SHALL FURNISH THE FOLLOWING DOCUMENTS TO THE BUILDING CONTRACTOR AND THE ENGINEER.
 - i) A WORK-AS-EXECUTED SURVEY OF THE PLAN POSITIONS OF ALL PILES PREPARED BY A QUALIFIED SURVEYOR. ii) A CERTIFICATE FROM A QUALIFIED ENGINEER (IN THE CASE OF 'DESIGN & CONSTRUCT' PILES) THAT ALL PILES HAVE BEEN DESIGNED, INSTALLED AND THE LOADS AS INDICATED ON THE DRAWINGS.
- BP11 THE PILING RIG SHALL NOT BE DEMOBILISED FROM SITE UNTIL THE ENGINEER AND THE GEOTECHNICAL CONSULTANT HAVE ISSUED THE FINAL SIGN-OFF OF ALL

REINFORCEMENT

- REFER TO THE CONCRETE NOTES FOR SPECIFIED COVERS TO REINFORCEMENT. COVERS SHALL BE MAINTAINED AT ALL CHAMFERS, DRIP GROOVES AND RECESSES OR
- REINFORCEMENT IS SHOWN DIAGRAMATICALLY, IT IS NOT NECESSARILY SHOWN IN TRUE
- REINFORCEMENT SHALL NOT BE CUT OR WELDED ON SITE WITHOUT PRIOR APPROVAL FROM THE ENGINEER. AT SMALL PENETRATIONS LESS THAN 300 mm IN SIZE IN A WALL OR A SLAB, BARS SHALL BE DISPLACED TO EITHER SIDE.
- SITE BENDING OF REINFORCEMENT SHALL BE AVOIDED WHERE POSSIBLE. WHERE SITE BENDING IS SPECIFIED. OR UNAVOIDABLE. IT SHALL BE CARRIED OUT COLD. WITHOUT THE APPLICATION OF HEAT, AND IN ACCORDANCE WITH THE 'PRACTICE NOTE RPN1' OF THE STEEL REINFORCEMENT INSTITUTE OF AUSTRALIA (SRIA).
- SPLICES IN REINFORCEMENT SHALL BE MADE ONLY IN THE POSITIONS SHOWN ON THE DRAWINGS. WRITTEN APPROVAL OF THE ENGINEER SHALL BE OBTAINED FOR ANY OTHER SPLICES. WHERE LAP LENGTHS ARE NOT SHOWN THEY SHALL BE AS INDICATED

BAR SIZE	MINIMUM LAP LE	ENGTH (mm)
R10/N10	400	(500)
N12	500	(650)
N16	750	(950)
N20	1000	(1300)
N24	1250	(1600)
N28	1500	(1900)
N32	1750	(2200)
N36	2050	(2650)

NOTES: LENGTHS SHOWN IN BRACKETS APPLY TO HORIZONTAL BARS WITH MORE THAN 300 mm OF CONCRETE CAST BELOW THE BAR.

> THE ABOVE DEVELOPMENT LENGTHS APPLY ONLY FOR MAIN REINFORCEMENT IN CONCRETE 32 MPa OR HIGHER WITH A MINIMUM CLEAR COVER OF 20 mm FOR WALLS / SLABS AND 30 mm FOR COLUMNS / BEAMS WITH AT LEAST R10 FITMENTS. LAP LENGTHS FOR ANY OTHER COMBINATIONS SHALL BE CALCULATED IN ACCORDANCE WITH SECTION 13

- REINFORCEMENT SYMBOLS:
- DENOTES D500N DEFORMED BAR TO AS 4671
- DENOTES 250R HOT ROLLED PLAIN BAR TO AS 4671 SL/ RL - DENOTES HARD DRAWN WIRE REINFORCEMENT FABRIC TO AS4671 W - DENOTES R500L HARD DRAWN PLAIN WIRE TO AS4671
- FABRIC REINFORCEMENT SHALL BE LAPPED WITH TWO TRANSVERSE WIRES PLUS 50 mm. JOGGLES TO BARS SHALL CONSIST OF A LENGTH OF 12 BAR DIAMETERS BETWEEN THE BEGINNING AND THE END OF AN OFFSET OF ONE BAR DIAMETER.

HOOKS, BENDS AND COGS SHALL BE IN ACCORDANCE WITH AS3600, UNO ON THE

- R8 ALL REINFORCEMENT BARS SHALL BE CHAIRED AT MAXIMUM CENTRES AS FOLLOWS:
 - FABRIC 600 mm BOTH WAYS FOR MESH SL72 OR LOWER AND 800 mm FOR LARGER

EXTRA CHAIRS MAY BE REQUIRED ADJACENT TO THE SLAB EDGES AND JOINTS TO PREVENT UPWARD DEFLECTION OF THE FABRIC WHEN STOOD ON.

ALL REINFORCEMENT SHALL BE SECURELY SUPPORTED AND MAINTAINED IN CORRECT

- PLASTIC TIPPED STEEL CHAIRS SHALL ONLY BE USED FOR EXPOSURE CATEGORIES A1 AND A2. FULL PLASTIC CHAIRS SHALL BE USED AT ELEMENT FACES HAVING AN EXTERNAL EXPOSURE IN THE COMPLETED STRUCTURE. WHERE REINFORCEMENT IS SUPPORTED ON GROUND PROVIDE PLATES UNDER ALL BAR CHAIRS.
- POSITIONS DURING CONCRETING. AT THE END SUPPORT OF A SLAB ON A MASONRY WALL, ALL BOTTOM REINFORCEMENT SHALL EXTEND OVER THE MASONRY WALL BY 75 mm FOR N12 BARS OR 95 mm FOR N16

BARS. BARS SHALL BE COGGED IF COVER REQUIREMENTS PROHIBIT THIS.

CONCRETE

- ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS3600 AND OTHER RELEVANT AUSTRALIAN STANDARDS UNLESS VARIED BY THE ENGINEER.
- BEFORE THE COMMENCEMENT OF CONCRETING, THE BUILDING CONTRACTOR SHALL ENSURE THAT THE CONCRETOR IS FULLY AWARE OF ANY AREAS OF FORMWORK THAT HAVE BEEN PRE-CAMBERED OR PRE-SET. EXTREME CARE SHALL BE TAKEN TO ENSURE THAT THE SPECIFIED DEPTHS OF BEAMS AND SLABS ARE ACHIEVED OVER THE PRE-CAMBERED OR PRE-SET FORMWORK.
- C3 THE BUILDING CONTRACTOR SHALL PROVIDE CONSTANT SUPERVISION OF CONCRETE POURS AND ENSURE THAT;
 - ALL APPROVALS ARE OBTAINED FROM THE ENGINEER, PT CONTRACTOR, FORMWORK ENGINEER AND OTHER RELEVANT CONSULTANTS.
 - REINFORCEMENT IS INSTALLED ACCORDING TO THE DESIGN DRAWINGS AND SECURED TO PREVENT DISPLACEMENT DURING CONCRETING.
 - NO SITE WATER IS ADDED TO CONCRETE BEING POURED OR THE CONCRETE IN
 - · ALL CONCRETE INCLUDING SLABS ON GROUND AND FOOTINGS, SHALL BE FULLY VIBRATED USING A HIGH FREQUENCY MECHANICAL VIBRATOR TO ACHIEVE FULL
 - COMPACTION BY COMPLETELY FILLING THE FORMWORK, FREE OF STONE POCKETS AND THOROUGHLY EMBEDDING THE REINFORCEMENT, NO CONCRETE IS POURED WHEN THE AMBIENT TEMPERATURE EXCEEDS 35°C,
 - POURED CONCRETE IS PROTECTED FROM RAIN, WARM DRYING WINDS OR OTHER
 - EXTREME WEATHER EVENTS · COLUMNS AND WALLS SHALL NOT BE POURED TOGETHER WITH THE SLAB OVER.
 - A MINIMUM OF 6 HOURS GAP SHALL BE MAINTAINED BETWEEN THE POURS OF VERTICAL AND HORIZONTAL ELEMENTS FORMED CONCRETE SURFACES SHALL HAVE FORMWORK CLASS AND SURFACE FINISHES IN ACCORDANCE WITH AS3610, OR AS SPECIFIED BY THE PROJECT
- CONDUITS, PIPES AND THE LIKE SHALL BE PLACED WITHIN THE MIDDLE THIRD OF THE SLAB DEPTH AND AT A MINIMUM SPACING OF NOT LESS THAN 3 DIAMETERS. CONDUITS
- C5 NO HOLES, PENETRATIONS, CHASES AND CONSTRUCTION JOINTS, OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE MADE IN CONCRETE MEMBERS

AND PIPES SHALL NOT BE PLACED WITHIN THE CONCRETE COVER.

WITHOUT PRIOR APPROVAL OF THE ENGINEER.

- CONCRETE PLACEMENT SHALL BE PLANNED IN SUCH A WAY THAT SUFFICIENT TIME IS ALLOWED FOR THE FINISHING OPERATIONS TO BE COMPLETED WITHIN NORMAL WORKING HOURS. WHERE THE SLAB OR PAVEMENT IS CONSTRUCTED IN THE OPEN OF ON SITES EXPOSED TO WINDS. RAPID DRYING OF THE CONCRETE SURFACE RESULTING IN INCREASED RATE OF HARDENING MAY LEAVE INSUFFICIENT TIME TO TROWEL THE SURFACE. CONSTRUCTION OF THE BUILDING SHALL BE PROGRAMMED TO MINIMISE THESE PROBLEMS. FOR INTERIOR FLOORS, WHERE POSSIBLE, COMPLETE THE ROOF AND PREFERABLY THE WALLS BEFORE THE FLOOR SLAB IS PLACED.
- ALL CONCRETE INCLUDING SLABS ON GROUND AND FOOTINGS, SHALL BE FULLY VIBRATED USING A HIGH FREQUENCY MECHANICAL VIBRATOR TO ACHIEVE FULL COMPACTION BY COMPLETELY FILLING THE FORMWORK, FREE OF STONE POCKETS (HONEYCOMBS) AND THOROUGHLY EMBEDDING THE REINFORCEMENT
- THE FOLLOWING PRACTICES SHALL BE AVOIDED WHILE FINISHING AND TROWELLING
- EXCESSIVE WORKING OF THE CONCRETE SURFACE DURING COMPACTING, LEVELLING AND POWER-FLOATING OF A PAVEMENT. EXCESSIVE WORKING WOULD RESULT IN A LAYER OF CEMENT RICH MORTAR BEING BROUGHT TO THE
- FLOATING OR TROWELLING WHILE BLEED WATER CONTINUES TO RISE OR REMAINS ON THE SURFACE. RE-WORKING OF BLEED WATER IN TO THE SURFACE LAYER WOLLD SIGNIFICANTLY INCREASE THE WATER-CEMENT RATIO OF THE CONCRETE IN THE SURFACE LAYER RESULTING IN A WEAKENED SURFACE

BLEED WATER THAT WOULD PRODUCE A VERY POOR WEARING SURFACE.

- PRONE TO DUSTING AND DELAMINATION. USING A MIXTURE OF CEMENT AND STONE DUST (KNOWN AS DRIERS) TO ABSORB
- VERTICAL CONSTRUCTION JOINTS SHALL BE PROPERLY FORMED WITH AN EDGEBOARD. THOROUGHLY SCABBLE AND CLEAN THE FIRST POUR OF ALL LOOSE AND POORLY COMPACTED CONCRETE AND LAITANCE. SOAK AND APPLY 1 CEMENT: 2 SAND SLURRY OR AN APPROVED BONDING AGENT IMMEDIATELY PRIOR TO PLACING THE SECOND POUR. THE SECOND POUR SHALL BE THOROUGHLY COMPACTED AGAINST THE FIRST
- ALL CONCRETE SHALL BE PROPERLY CURED. CURING SHALL COMMENCE WITHIN 2 HOURS OF POURING AND SHALL CONTINUE FOR A MINIMUM OF 7 DAYS, USING AT LEAST ONE OF THE METHODS BELOW AND THEN FOLLOWED BY GRADUAL DRYING OUT. WHEN THE AMBIENT TEMPERATURE EXCEEDS 32°C CURING SHALL BE ACHIEVED USING METHODS a) OR b) ONLY.
 - a. PONDING OR CONTINUOUS SPRINKLING WITH POTABLE WATER.
 - h LISE AN ARSORRENT COVER KEPT CONSTANTLY WET c. USE AN IMPERMEABLE SHEET MEMBRANE OVER A MOISTENED SURFACE. THE MEMBRANE SHALL BE FIXED AND LAPPED SO THAT NO AIR CIRCULATION CAN OCCUR AT THE CONCRETE SURFACE
 - d. USE A CURING COMPOUND COMPLYING WITH AS3799, APPLIED UNIFORMLY IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. WHEN DRY THE COATING SHOULD BE CONTINUOUS, FLEXIBLE AND WITHOUT VISIBLE BREAKS OR PIN HOLES FOR AT LEAST SEVEN DAYS. THE COMPATIBILITY OF CURING COMPOUNDS WITH PROPOSED APPLIED FINISHES SHALL BE VERIFIED PRIOR TO APPLICATION.

FORMED SURFACES EXPOSED WITHIN 14 DAYS OF CASTING SHALL BE SPRAYED WITH AN APPROPRIATE CURING AGENT IMMEDIATELY UPON EXPOSURE

C10 NO MASONRY OR PARTITION WALLS SHALL BE CONSTRUCTED ON SUSPENDED FLOORS UNTIL 7 DAYS AFTER PROPPING HAS BEEN REMOVED AND ONLY WITH THE APPROVAL OF THE ENGINEER.

CONCRETE (CONTINUED)

C11 SPECIFICATIONS FOR CONCRETE:

THE FOLLOWING SPECIFICATIONS SHALL APPLY UNLESS MORE STRINGENT REQUIREMENTS ARE NOTED ELSEWHERE IN THE DOCUMENTATION.

Element Slump Max. Cement Exposure Min. Conc. Conc. Comments

Element	Siump	Max. Agg. Size	Type	Classif'n.	Grade (f'c) MPa U.N.O	Conc. Cover (U.N.O.)	Comments
Mass Conc. Footings/Piers	80	20	GP	A2	25	-	
				A2 (Non- Agressive Soils)	32	45	
Reinforced Footings/Piers	80	20	GP	B1 (Low- Agressive Soils)	32	50	
				B2 (In High Sulphate or Saline Soils)	40	55	
				A2 - Internal	Refer	25	Greater Cove
Columns/Walls (Incl. Tilt- Up/Precast Wall Panels)	80	20	GP	B1 - External (1-50 km from Coast)	Column Details - 32 Min.	40	May Be Required for Fire. Refer C12.
				B2 - External (Up to 1 km from Coast)	Refer Column Details - 40 Min.	45	012.
Core Filling Grout	230±30	10	GP	-	20	-	
Internal Suspended Slabs/Beams	80	20	Reinf't. GP (U.N.O.) Post- Tensioned - SL	A1 - (Residential or > 50 km from Coast Only)	Refer Plans - 32 Min.	20	Greater Cove May Be Req' for Fire. Refe C12.
				A2		25	
External Suspended Slabs/Beams	80	20	SL	B1 - External (1-50 km from Coast)	Refer Column	30	Greater Cove May Be Req' for Fire. Refe
				B2 - External (Up to 1 km from Coast)	Details - 40 Min.	45	C12.
Internal Slab On Ground (Top Cover Only, Refer Reinf'd.	80	20	GP	A1 - (Residential or > 50 km from Coast Only)	32	25	With Wearing Allowance Of 5mm
Ftgs./Piers For Btm. Cover)				A2		30	
External Slab On Ground (Top Cover	80	20	SL	B1 - External (1-50 km from Coast)	32	45	With Wearing Allowance Of 5mm
Only, Refer Reinf'd. Ftgs./Piers For Btm. Cover)				B2 - External (Up to 1 km from Coast)	40	50	

NOTE.
NOTE:

ALL CONCRETE WITH SHRINKAGE LIMITED (SL) CEMENT SHALL HAVE A MAXIMUM SHRINKAGE STRAIN OF 650 MICROSTRAINS AS DETERMINED BY TESTS IN ACCORDANCE WITH AS 1012.13 AFTER 8 WEEKS OF DRYING.

WATER CEMENT RATIO OF CONCRETE SHALL NOT EXCEED 0.55 (EXCEPT

FOR CORE FILLING GROUT IN BLOCK WALLS). C12 UNLESS MORE STRINGENT CRITERIA IS SPECIFIED ELSEWHERE IN THE DOCUMENTATION. THE FOLLOWING CONCRETE COVERS SHALL BE USED FOR

REFER PT CONTRACTOR'S SHOP DRAWINGS

INCI CINT I C	ONTIVACT		orviviivoo.			
			Eleme	ent Type		
Fire Resistance	Flat Slab		Banded/Ribb	oed Slab	Column	Wall
Level (Minutes)		Simply S	upported	Continuous (One		
		One Way	Two Way	& Two Way)		
30					25	
60	25	25	25	25	30	25
90					35	
120	30	35			40	30
100	40		0.5			

CONVENTIONALLY REINFORCED ELEMENTS ONLY. FOR POST-TENSIONED SLABS,

D.M. 05-08-2021 ISSUED FOR CONSTRUCTION D.M. 06-05-2021 D.M. 08-01-2021 REVISED AS CLOUDED 4 ISSUED FOR CONSTRUCTION CERTIFICATE D.M. 24-11-2020 KOTURIC + CO ISSUED FOR TENDER D.M. 05-03-2020 ISSUED FOR SSDA ISSUED FOR TENDER K.S. N.V. 19-12-2019

DRAWN DESIGNED DATE

AMENDMENT

WARAKIRRI COLLEGE

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CONSTRUCTION NOTES - SHEET 1	19712-S	1.01		Α
Title	Drawing number			Revision
6A WATSFORD ROAD, CAMPBELLTOWN	Checked D.M.	Approved R.K.	Scale	
NEW LEARNING CENTRE	Drawn H.W.	Designed D.M.	Date DEC. 20	19

NEW LEARNING CENTRE 6A WATSFORD ROAD, CAMPBELLTOWN

- FORMWORK AND FALSEWORK SHALL BE DESIGNED, CONSTRUCTED AND STRIPPED IN ACCORDANCE WITH AS3610.
 - THE BUILDING CONTRACTOR SHALL ENGAGE A QUALIFIED PROFESSIONAL FOR THE DESIGN, CONSTRUCTION AND CERTIFICATION OF FORMWORK, FALSEWORK
 - DESIGN INFORMATION REGARDING THE GROUND SUPPORT FOR FORMWORK AND FALSEWORK SHALL BE DETERMINED FROM THE CONDITIONS EXISTING ON SITE AT THE TIME OF CONSTRUCTION.
 - THE FORMWORK SHALL NOT BE DESIGNED TO RELY ON RESTRAINT OR STABILITY FROM THE PERMANENT STRUCTURE WITHOUT PRIOR APPROVAL FROM THE
- WHERE APPLICABLE, FORMWORK SHALL BE DESIGNED TO ACCOMMODATE MOVEMENT AND LOAD REDISTRIBUTION FROM POST-TENSIONING. THE FORMWORK DESIGNER SHALL CONSULT WITH THE POST-TENSIONING CONTRACTOR ON THE DESIGN REQUIREMENTS.
- SIZES OF CONCRETE ELEMENTS DO NOT INCLUDE THE APPLIED FINISHES. BEAM DEPTHS ARE USUALLY NOTED FIRST AND INCLUDE THE SLAB THICKNESS. FOR CHAMFERS. DRIP GROOVES, REGLETS ETC., REFER TO THE ARCHITECT'S DRAWINGS AND/ OR SPECIFICATIONS
- PROVIDE UPWARD CAMBER OR PRESET IN FORMWORK TO SLABS AND BEAMS WHERE NOTED ON THE DRAWINGS
 - THE FORMWORKER SHALL MAKE THE BUILDING CONTRACTOR AND CONCRETOR FULLY AWARE OF THE LOCATIONS WHERE FORMWORK IS CAMBERED OR PRE-SET IN ORDER THAT THE FULL DEPTHS OF THE MEMBERS ARE ACHIEVED DURING CONCRETING
- FOR HORIZONTAL RC ELEMENTS, FORMWORK MAY BE STRIPPED WHEN THE CONCRETE HAS REACHED 80% OF ITS SPECIFIED 28 DAY STRENGTH UNO ON THE
- ALTERNATIVELY, FORMWORK MAY BE STRIPPED AND PROGRESSIVELY BACK-PROPPED AFTER 5 DAYS SUBJECT TO APPROVAL FROM THE ENGINEER. THE PROPPING SHALL REMAIN IN PLACE UNTIL THE CONCRETE HAS REACHED 80% OF THE SPECIFIED 28 DAY STRENGTH.
- ADDITIONAL CONDITIONS MAY APPLY IF THE SLAB IS TRANSFERRING A STRUCTURE ABOVE OR SUBJECT TO EXCESSIVE CONSTRUCTION LOADING.
- STRIPPING AND BACK PROPPING TO POST-TENSIONED SLABS SHALL BE AS DIRECTED BY THE POST-TENSION CONTRACTOR
- VERTICAL FORMS TO BEAM SIDES, COLUMNS AND WALLS MAY BE STRIPPED AFTER 3 DAYS AND WHEN THE FORMWORKER IS SATISFIED THAT STRIPPING WILL NOT DAMAGE THE GREEN CONCRETE.
- THE FORMWORKER SHALL PROVIDE CLEANOUTS TO ALL COLUMNS AND WALLS AND LEAVE THEM OPEN FOR THE ENGINEER'S INSPECTION, AND CLOSE OFF IMMEDIATELY PRIOR TO POURING.
- IN MULTI STOREY CONSTRUCTION, PROPPING SHALL BE PROVIDED FOR AT LEAST 3 LEVELS BELOW THE FLOOR BEING CAST. PROP REMOVAL SHALL BE SUBJECT TO THE APPROVAL OF THE ENGINEER TO AVOID OVER-STRESSING THE PREVIOUSLY CAST FLOORS.
- REFER TO THE ARCHITECT'S SPECIFICATIONS FOR THE REQUIRED CLASS OF SURFACE FINISH TO THE FORMED SURFACES.

MASONRY CONSTRUCTION:

- ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS3700 AND AS NOTED ON THE DRAWINGS.
- BRICK AND BLOCK COMPRESSIVE STRENGTH (fuc) SHALL BE 15 MPa MINIMUM UNO. STRENGTH GRADE SHALL BE CLEARLY INDICATED ON THE DELIVERY
- JOINT MORTAR SHALL BE OF CLASS M3 WITH 1:1:6 (CEMENT: LIME: SAND) PROPORTIONS BY VOLUME AND COMPLY WITH AS3700. MORTAR JOINTS SHALL BE 10 mm THICK AND HAVE A MAXIMUM TOOLED DEPTH OF 3 mm UNO.
- NON-LOAD BEARING WALLS SHALL BE SEPARATED FROM THE LOAD-BEARING ELEMENTS BY 15 mm THICK 'CANEITE' OR EXPANDED POLYSTYRENE UNO AT BOTH HORIZONTAL AND VERTICAL FACES.
 - NON-LOAD BEARING WALLS SHALL BE TIED TO THE SOFFITS OF BEAMS OR SLABS OVER BY USING 'MET 4-1' TIES (OR APPROVED FOLIVALENT). AT 800 mm MAX. CENTRES, UNO ON THE DRAWINGS, TO MANUFACTURER'S SPECIFICATIONS.
- WHERE CONCRETE SLABS BEAR ON UNREINFORCED MASONRY, INCLUDING CLAY BRICKS, RENDER THE BEARING SURFACE OF THE MASONRY WALL WITH 1:3 (CEMENT : SAND) MORTAR TO ACHIEVE A LEVEL SURFACE AND PLACE A PRE GREASED METAL SLIP JOINT PROTECTED BY 0.2 mm POLYETHYLENE SHEET TAPED TO THE FORMWORK BEFORE PLACING CONCRETE. SPECIAL DETAILS SUCH AS WATER-PROOFING MAY APPLY FOR ROOF SLABS OR SIMILARLY EXPOSED ELEMENTS.
- B6 <u>CONTROL JOINTS</u>
 - 1. CONTROL JOINTS SHALL BE PROVIDED IN MASONRY WALLS AS PER THE TABLE BELOW UNLESS CLOSER SPACINGS ARE SPECIFIED ELSEWHERE IN

MASONRY TYPE	LOCATION	JOINT SIZE (mm)	SPACING (m)
CONCRETE MASONRY	- EXTERNAL	10	7.0
	- EXTERNAL	10	5.0
	(WITH OPENINGS > 900mm IN HEIGHT)		
	- INTERNAL (FACE FINISHED)	10	6.0
	- INTERNAL (RENDERED)	10	5.0
LIGHT-WEIGHT MASONRY	- INTERNAL / EXTERNAL	10	6.0
CLAY MASONRY	- INTERNAL / EXTERNAL	15	6.0 *
	- PARAPET WALLS	15	4.0

- 2. CONTROL JOINTS SHALL BE PLACED AT HALF THE SPECIFIED SPACING FROM A CORNER, PROVIDE JOINTS TO MATCH JOINTS IN THE

* - FOR REACTIVE 'CLASS M' SITES ONLY. REFER TABLE 4.3 OF

AS3700 2011 FOR ARTICULATION JOINTS IN CLAY MASONRY

SUPPORTING STRUCTURE.

- 3. CONTROL JOINTS MUST BE KEPT FREE OF MORTAR AND SEALED WITH A POLYETHYLENE FOAM BACKING ROD SQUEEZED INTO THE GAP AND A GUNNED-IN MASTIC SEALANT, IF THE WALL IS TO BE FIRE-RATED, A FIRE-RATED SEALING SYSTEM WILL BE REQUIRED INSTEAD.
- B7 <u>BRICKWORK</u>
 - BRICKWORK SUPPORTED BY A SUSPENDED FLOOR SLAB SHALL NOT BE ERECTED UNTIL THE CONCRETE HAS GAINED FULL 28 DAY STRENGTH AND THE FORMWORK HAS BEEN REMOVED OR APPROVAL HAS BEEN GIVEN BY
 - 2. FOR CAVITY WALLS, WALL TIES SHALL BE PROVIDED AT 600 mm MAXIMUM CENTRES BOTH VERTICALLY AND HORIZONTALLY AND CONSIST OF 3 mm DIA. 316 GRADE STAINLESS STEEL WIRES UNO.
 - 3. WHERE AN EXTERNAL BRICK LEAF CONTINUES PAST THE SLAB EDGE, IT SHALL BE TIED TO THE SLAB EDGE BY USING 316 GRADE STAINLESS STEEL 'MET 6-1' TIES (OR APPROVED EQUIVALENT) AT 900 mm MAX. CENTRES
 - 4. IN MULTI-STOREY CONSTRUCTION, MASONRY WALLS SHALL BE VERTICALLY SUPPORTED AT EVERY SECOND FLOOR ON A SHELF ANGLE FIXED TO THE SLAB EDGE OR A CORBEL

MAX. SPAN mm	LINTEL SIZE	END BEARING
1000	90 x 10 BAR	110
1500	90 x 90 x 10 EA	110
2100	100 x 100 x 10 EA	110
2700	150 x 90 x 10 UA	150
3000	150 x 100 x 12 UA	150

- PROVIDE LINTELS. NOT SHOWN ON PLAN. AT ALL OPENINGS TO EACH 110 BRICK SKIN IN ACCORDANCE WITH THE SCHEDULE ABOVE. ERECT ANGLES WITH LONGEST LEG VERTICAL.
- HOT DIP GALVANISE ALL LINTELS. THERE MUST BE AT LEAST 3 COURSES OF BRICKWORK OVER THE CLEAR SPAN OPENING.
- ALL LINTELS MUST BE PROPPED DURING BRICKWORK CONSTRUCTION
- **BLOCKWORK**
 - 1. IN CORE-FILLED BLOCKWORK, EXCESS MORTAR PROTRUDING INTO THE CORES SHALL BE REMOVED BY RODDING AFTER EACH COURSE IS LAID. EVERY CORE FILLED WITH GROUT SHALL HAVE A CLEANOUT BLOCK IN THE BOTTOM COURSE.
 - 2. REINFORCEMENT SHALL BE PLACED AND SECURELY TIED IN POSITION AS SHOWN ON THE DRAWINGS. STARTER BARS SHALL BE HELD IN PLACE BY TYING TO HORIZONTAL BARS AT CLEANOUT BLOCKS. PROVIDE COVER TO REINFORCEMENT AS SHOWN IN THE DETAILS.
 - 3. CORE FILLING GROUT SHALL BE AS NOTED IN CONCRETE NOTES IN LIFTS NO MORE THAN 1200mm IN HEIGHT

A ISSUED FOR CONSTRUCTION

ISSUED FOR TENDER

ISSUED FOR SSDA

REVISION

ISSUED FOR TENDER

AMENDMENT

REVISED AS CLOUDED

- ALL STRUCTURAL STEEL, MATERIALS, FABRICATION AND ERECTION SHALL COMPLY WITH
- S2 STRUCTURAL STEEL SHALL BE GRADE 350 MINIMUM FOR HOLLOW SECTIONS AND GRADE 300 MINIMUM FOR ALL ROLLED SECTIONS UNO. STEEL FABRICATOR SHALL PROVIDE ALL CERTIFICATIONS FOR QUALITY AND GRADE OF STEEL MEMBERS AND STRUCTURAL BOLTS FOR THE ENGINEER'S REVIEW.
- BOLTS ARE DESIGNATED ON THE DRAWINGS BY THE NUMBER, DIAMETER, GRADE AND TIGHTENING PROCEDURE IN ACCORDANCE WITH AS4100 AND THE 'HANDBOOK 1: DESIGN OF STRUCTURAL STEEL CONNECTIONS' PUBLISHED BY ASI.
- BOLTS SHALL BE OF SIZE M20, GRADE 8.8/ S AND A MINIMUM OF 2 BOLTS PER CONNECTION UNO. CLEATS AND GUSSETS SHALL BE 10 mm THICK UNO.

ALL CLEATS AND DRILLINGS FOR FIXING OF TIMBER MEMBERS ETC. SHALL BE PROVIDED BY

- ALL PLATES INCLUDING BUT NOT LIMITED TO CAP, BASE AND GUSSET PLATES TO BE FULLY WELDED TO THE STEEL MEMBERS UNO.
- WELDING AND TESTING
 - UNLESS NOTED OTHERWISE, WELDS SHALL BE 6 mm CATEGORY 'SP' CONTINUOUS FILLET WELDS WITH APPROVED COVERED ELECTRODES.
 - WHERE STAINLESS STEEL IS WELDED TO MILD STEEL, USE A SUITABLE OVER ALLOYED
 - THE EXTENT OF NON-DESTRUCTIVE WELD EXAMINATION SHALL BE AS NOTED BELOW. RADIOGRAPHIC OR ULTRASONIC EXAMINATION SHALL BE TO AS1554.1, AS2177.1 AND AS2307.

TYPE OF WELD AND	EXAMINATION METHOD	EXTENT (% TOTAL
CATEGORY		LENGTH OF WELD)
FILLET WELDS, GP.SP	VISUAL INSPECTION	100%
BUTT WELDS, GP	VISUAL INSPECTION	100%
BUTT WELD, SP	VISUAL INSPECTION	100%
BUTT WELD SP	RADIOGRAPHIC OR	10%
	ULTRASONIC INSPECTION	

- FLASH WELDING AND TESTING OF ALL STUDS SHALL COMPLY WITH AS1554.2
- ALL CORNERS AND EDGES OF ALL EXTERNAL STEEL PLATES AND SECTIONS ARE TO BE ROUNDED TO A RADIUS OF NOT LESS THAN 2 mm PRIOR TO SURFACE PREPARATION.
- INTERNAL STEELWORK SHALL BE GRIT BLASTED TO CLASS 2.5 AND PAINTED WITH BLUE ZINC PHOSPHATE AND 75 mm DRY FILM THICKNESS UNLESS OTHERWISE NOTED IN ARCHITECTURAL SPECIFICATIONS.
- ALL EXTERNAL STEELWORK AND STEEL MEMBERS SPECIFIED ON THE DRAWINGS OR OTHER RELATED CONTRACT DOCUMENTS AS GALVANISED SHALL CONFORM TO THE REQUIREMENTS OF AS4680. THE MINIMUM APPLICATION RATE FOR GALVANISING SHALL BE 550 g/ sqm.
- PROVIDE 6 mm SEAL PLATES TO ALL HOLLOW SECTIONS, WITH "BREATHER" HOLES IF MEMBERS ARE TO BE HOT DIP GALVANISED.
- S9 CAMBER OR PRESET TO STRUCTURAL STEEL ROOF BEAMS, TRUSSES, PORTALS ETC., SHALL BE PROVIDED AS NOTED ON THE DRAWINGS.
- S10 ALL STRUCTURAL STEELWORK BELOW GROUND SHALL BE ENCASED IN CONCRETE WITH 75 mm COVER ALL AROUND OR PAINTED WITH 2 COATS OF APPROVED BITUMEN PAINT.
- S11 ALL PROPRIETARY CHEMICAL AND MECHANICAL ANCHORS ARE TO BE INSTALLED AT SPACINGS, EDGE DISTANCES AND DEPTHS AS INDICATED ON THE DRAWINGS. INSTALLATION SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS INCLUDING THE DRILLING METHOD, HOLE DIAMETER, CLEANING, CURING AND TIGHTENING.
- S12 USE NON-SHRINK GROUT WITH A MINIMUM COMPRESSIVE STRENGTH OF 40 MPa, TIGHTLY PACKED UNDER ALL BEARING AND BASE PLATES.
- S13 IF ANY TRANSLUCENT ROOF SHEETING IS SPECIFIED ON THE ARCHITECTURAL DRAWINGS, THEY SHALL BE OF A GAUGE COMPATIBLE WITH THE SPECIFIED PURLIN SPACING. ALTERNATIVELY, PROVIDE ADDITIONAL C10012 PURLIN TRIMMERS AS REQUIRED TO SUPPORT
 - SAFETY MESH UNDER TRANSLUCENT SHEETING, IF REQUIRED, SHALL CONFORM TO WORKCOVER REQUIREMENTS.
- S14 SUSPENDED CEILINGS AND BULKHEADS, WHERE SUPPORTED BY PURLINS, SHALL BE SUPPORTED BY WEB CONNECTIONS ONLY AND NOT HOOKED FROM THE BOTTOM LIP. THE BUILDING CONTRACTOR SHALL ENSURE THAT ALL SUB-CONTRACTORS COMPLY WITH THIS
- S15 ELECTRONIC OR HARD COPIES OF SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER AND APPROVAL OBTAINED REFORE COMMENCING FARRICATION
 - ENGINEER'S APPROVAL WILL ONLY COVER THE SECTION SIZES AND CONNECTIONS, NOT THE MEMBER LENGTHS OR DIMENSIONAL LAYOUT
- S16 STABILITY OF THE STRUCTURE DURING STEEL ERECTION IS THE STEEL ERECTOR'S RESPONSIBILITY. PROVIDE TEMPORARY BRACING AND/ OR GUY WIRES AS REQUIRED. REFER TO THE 'STEEL ERECTION GUIDE'

N.V. 19-12-2019

DRAWN DESIGNED DATE

STRUCTURAL STEEL (CONTINUED)

S17 STEELWORK ERECTION GUIDE

- S17.1 THIS GUIDE IS ONLY INTENDED TO PROVIDE THE STEEL ERECTOR WITH A RECOMMENDED PROCEDURE FOR ERECTING THE STEELWORK SAFELY AND EFFICIENTLY. THE FABRICATION AND ERECTION OF THE STRUCTURAL STEELWORK SHALL BE SUPERVISED BY A COMPETENT PERSON IN ORDER TO ENSURE THAT ALL REQUIREMENTS OF THE DESIGN ARE MET. HENRY & HYMAS WILL NOT BE LIABLE FOR THE QUALITY OF ERECTION NOR ASSUME ANY RESPONSIBILITY FOR ANY CONSTRUCTION DEFECTS RESULTING FROM IMPROPER ERECTION TECHNIQUES OR NEGLIGENCE OF OTHER PARTIES.
- S17.2 THE STEEL ERECTOR SHALL BE A COMPETENT PERSON FAMILIAR WITH THE FOLLOWING STANDARDS/ MANUALS AND OTHER INDUSTRY PRACTISES & GUIDELINES.
 - I) AS / NZS5131-2016 : STRUCTURAL STEELWORK FABRICATION AND ERECTION II) AS4100-1998 · STEEL STRUCTURES III) PRACTICAL GUIDE TO PLANNING THE SAFE ERECTION OF STEEL STRUCTURES (1ST EDITION) - AUSTRALIAN STEEL INSTITUTE (2016) IV) SAFE DESGN OF STRUCTURES CODE OF PRACTICE - SAFEWORK AUSTRALIA (2014)
- 17.3 THE CONSTRUCTION SITE SAFETY IS THE SOLE RESPONSIBILITY OF THE CONTRACTORS ON SITE. CONTRACTORS ARE RESPONSIBLE FOR FULL COMPLIANCE WITH ALL THE SAFETY REQUIREMENTS OF THE GOVERNING REGULATORY AUTHORITY AS WELL AS ANY ADDITIONAL
- REQUIREMENTS IMPOSED BY THE DEVELOPER. 17.4 THE STRUCTURE SHALL NOT BE SUBJECT TO EXCESSIVE CONSTRUCTION LOADING SUCH AS MATERIAL STACKING UNLESS EXPLICITLY NOTED ON THE DESIGN DRAWINGS.
- 17.5 ALL GUY ROPES AND PROPS SHALL BE DESIGNED BY A COMPETENT PERSON FOR AN OUT-OF-PLANE LOAD EQUAL TO 2.5% OF GRAVITY LOADS PLUS WIND LOADS ARISING FROM A 100 YEAR RETURN PERIOD. LONG SPAN RAFTERS/ TRUSSES SHALL BE BRACED AGAINST TWISTING AND BUCKLING.
- 17.6 OUTLINED BELOW IS HENRY & HYMAS' RECOMMENDED PROCEDURE FOR STEEL ERECTION. THE STEEL ERECTOR SHALL SUBMIT A DETAILED ERECTION SEQUENCE METHODOLOGY INCLUDING THE 'WITNESS AND HOLD POINTS' AND ANY DEVIATIONS FROM THE RECOMMENDED PROCEDURE FOR REVIEW BY HENRY & HYMAS PRIOR TO ERECTING ANY

STEELWORK ERECTION SEQUENCE – STEEL FRAMED STRUCTURE

- STEP 1 ERECT COLUMNS ALONG GRID 8 FROM GRID A TO D AND BRACE THEM WITH GUY ROPES OR PROPS TO RESTRAIN AGAINST POTENTIAL SWAY IN ANY DIRECTION. ERECT RAFTERS ALONG THE SAME GRID LINE STARTING FROM GRID A.
- ERECT COLUMNS AND RAFTERS ALONG GRID 7 AND PROGRESSIVELY ATTACH LEAD PURLINS/ STRUTS AND DIAGONAL BRACINGS BACK TO THE FRAME ALREADY ERECTED. SQUARE AND PLUMB BRACED BAYS BEFORE MOVING TO STEP 3.
- TEP 3 PROCEED WITH THE ERECTION OF THE REMAINING FRAMES ALONG GRIDS 6 O 1 INCLUDING ANY VERTICAL BRACING.
- STEP 4 TEMPORARY BRACING MAY BE REMOVED AFTER ALL THE PRIMARY MEMBERS SUCH AS COLUMNS, RAFTERS AND WALL/ ROOF BRACING ELEMENTS HAVE BEEN ERECTED AND SIGNED OFF BY HENRY & HYMAS, OR WHEN SUFFICIENT LATERAL STABILITY HAS BEEN ACHIEVED. INSTALLATION OF SECONDARY COMPONENTS SUCH AS PURLINS, GIRTS, FLY BRACING, FASCIA TRUSSES ETC. SHOULD FOLLOW.

PRECAST & TILT PANELS

- PRECAST OR TILT PANEL CONSTRUCTION SHALL COMPLY WITH AS3850 AND AS3600. ANY VARIATIONS TO THE DIMENSIONS, SPECIFIED PRODUCTS ETC., SHALL BE APPROVED BY THE ENGINEER.
- ALL REINFORCEMENT SHOWN ON THE STRUCTURAL DRAWINGS ARE FOR IN-SERVICE LOADINGS ONLY. THE PANEL CONTRACTOR SHALL DESIGN ANY ADDITIONAL REINFORCEMENT TO ENSURE THAT THE PANELS HAVE SUFFICIENT STRENGTH FOR LIFTING TRANSPORT FRECTION AND TEMPORARY SUPPORT CONDITIONS. ALL BRACINGS AND SUPPORTING STRUCTURES (DEADMAN OR FLOOR SLAB) SHALL BE STRUCTURALLY ADEQUATE TO SUPPORT THE WIND AND OTHER TEMPORARY LOADS.
- FLOOR BRACING INSERTS SHALL NOT BE LESS THAN 600 mm AWAY FROM ANY
- P4 THE PANEL CONTRACTOR SHALL PROVIDE AN ENGINEER'S CERTIFICATE TO THE BUILDING CONTRACTOR AND THE ENGINEER CONFIRMING THAT THE DESIGNS COMPLY WITH AS3850, AS3608 AND AS/NZS1170.2. THIS CERTIFICATE IS AN ESSENTIAL REQUIREMENT FOR THE ENGINEER'S SIGN-OFF OF THE PANEL SHOP
- P5 NO WELDING OR APPLICATION OF HEAT ARE PERMITTED TO ANY SPECIFIED INSERTS.
- REFER TO ARCHITECT'S DRAWINGS FOR SILL BEVEL, REBATE AND SPITTER DETAILS.
- ALL CAST-IN FERRULES SHALL BE OF 90 mm MINIMUM LENGTH INSTALLED WITH 300 mm LONG CROSS RODS. BASE OF ALL PANELS SHALL BE GROUTED TO PROVIDE A CONTINUOUS BEARING
- UNDER THE FULL LENGTH AND THICKNESS OF THE PANELS. WHERE PANELS SUPPORT SUSPENDED SLABS, GROUTING SHALL BE ADEQUATELY CURED PRIOR TO REMOVAL OF SLAB FORMWORK. PROVISION OF SHIMS IS PERMITTED ONLY AT THE POSITIONS SHOWN ON THE DRAWINGS.
- SURFACE QUALITY:
- i) DIMENSIONAL TOLERANCES OF PANELS SHALL COMPLY WITH TABLE 3.11(A) OF AS3850 AND SECTION 17.5 OF AS3600. CASTING BEDS (FLOOR SLABS, PAVEMENTS OR TEMPORARY CASTING BEDS) SHALL BE POURED TO THE TOLERANCES WITHIN THE REQUIRED SURFACE FINISHES OF THE PANELS. ii) ALL PANELS AND CASTING BEDS SHALL HAVE A STEEL TROWELLED FINISH
- WITHOUT TROWEL MARKS. CASTING - CONCRETE ELEMENTS MAY BE STACK-CAST IN THE REVERSE ORDER OF ERECTION. TILT PANELS SHALL BE CAST WITH THEIR EXTERNAL FACES DOWN TO

MINIMISE THE NEED FOR COSMETIC PATCHING AFTER ERECTION.

SUGGESTED CONSTRUCTION SEQUENCE

- SCS1. CONSTRUCT SHORING WALL 'SW2' ALONG RAIL CORRIDOR BOUNDARY PRIOR TO ANY ADJACENT EXCAVATION.
- EXCAVATE TO BASEMENT LEVEL. PROVIDE TEMPORARY BATTER TO PERIMETER OF EXCAVATION IN ACCORDANCE WITH THE GEOTECHNICAL REPORT
- CONSTRUCT STRIP & PAD FOOTINGS AT BASEMENT LEVEL
- CONSTRUCT COLUMNS, LIFT WALLS & RETAINING WALLS TO PERIMETER OF
- CONSTRUCT GROUND FLOOR SLAB.
- BACKFILL & COMPACT GROUND BEHIND BASEMENT RETAINING WALLS.
- COMPLETE SHORING WALL 'SW1', ACOUSTIC WALL & LANDSCAPE RETAINING WALLS. THESE ITEMS MAY ONLY BE COMPLETED ONCE (SCS1.) HAS BEEN COMPLETED.
- CONSTRUCT PERIMETER BLOCK RETAINING WALLS.
- CONSTRUCT COLUMNS & ERECT PRECAST PANELS FROM GROUND FLOOR TO FIRST FLOOR. PROVIDE TEMPORARY PROPPING TO PANELS.
- SCS10. CONSTRUCT FIRST FLOOR SLAB.
- ERECT PRECAST PANELS FROM FIRST FLOOR TO ROOF. PROVIDE TEMPORARY
- CONSTRUCT BASEMENT SLAB ON GROUND.

SCS12. ERECT STEEL FRAMING FOR ROOF & AWNINGS.

SCS14. POUR EXTERNAL PAVEMENTS / FOOTPATHS

ABBREVIATIONS

- ALTERNATELY - BUILDING JOINT
- BOTH SIDES - BOTTOM
- CONTINUOUS FILLET WELD - FULL PENETRATION BUTT WELD
- CONSTRUCTION JOINT C/S BRICK OR BLOCKWORK COURSE
- CTS - CENTRES - EACH FACE
- EW - EACH WAY - FAR FACE
- HORIZONTAL - ISOLATION JOINT
- LENGTH VARIES
- MOVEMENT JOINT - NEAR FACE
- NOMINAL
- NOT SHOWN ON ELEVATION NSOE
- NSOP - NOT SHOWN ON PLAN
- NOT TO SCALE
- POST-TENSIONED

- REINFORCED CONCRETE
- STAINLESS STEEL - TEMPORARY MOVEMENT JOINT
- TOP OF KERB
- TOW - TOP OF WALL U/S - UNDERSIDE - VERTICAL UNO

- OVER

- UNDER

- UNLESS NOTED OTHERWISE

- BEHIND

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D.M. 08-01-2021 4 ISSUED FOR CONSTRUCTION CERTIFICATE D.M. 24-11-2020 KOTURIC + CO. D.M. 05-03-2020

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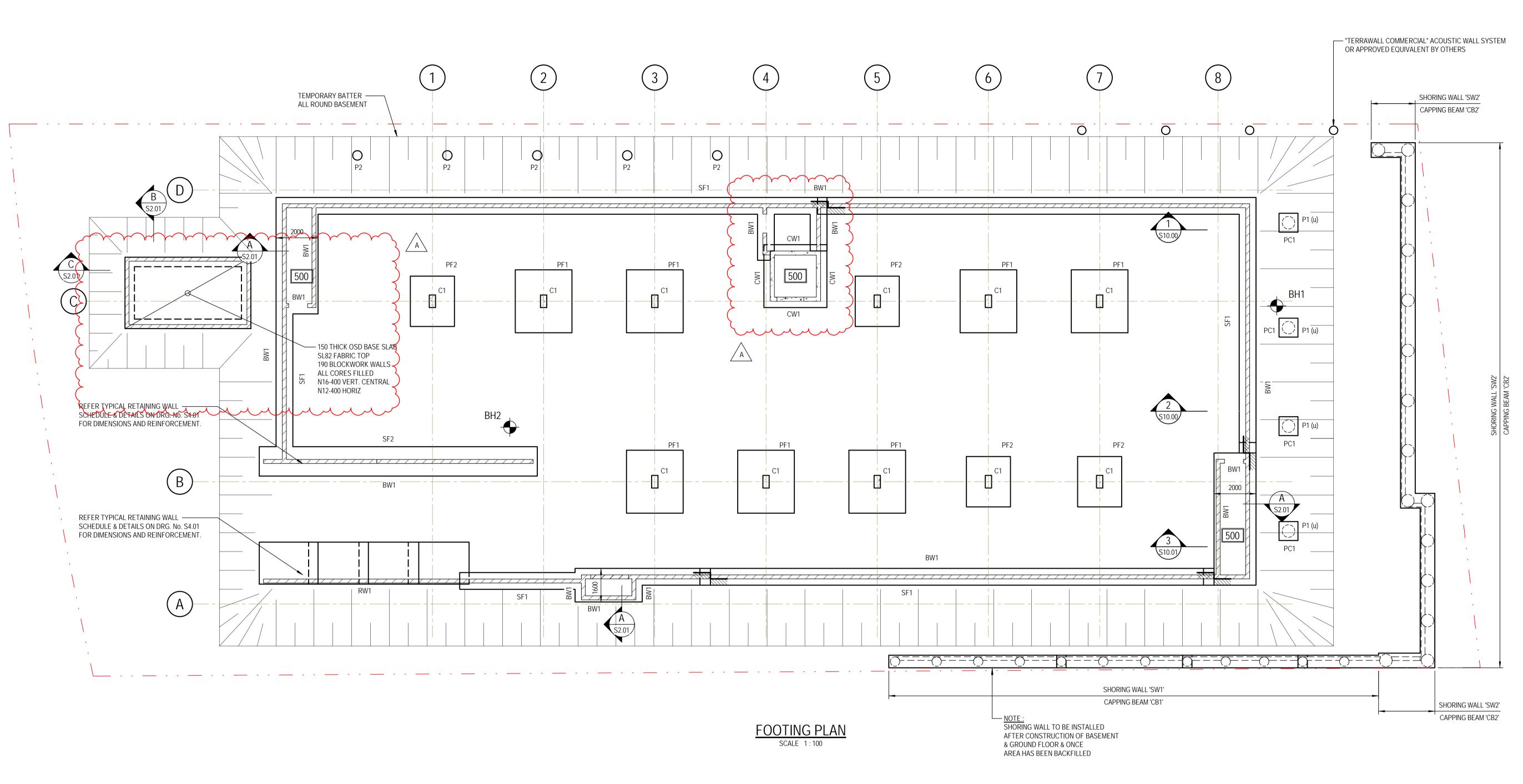
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H.W. D.M. DEC. 2019 NEW LEARNING CENTRE 6A WATSFORD ROAD. CAMPBELLTOWN D.M. Drawing number Revision **CONSTRUCTION NOTES - SHEET 2**



FOUNDATIONS:

- F1 FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT No. CES180704_ZDC-AB DATED 18/10/2018 PREPARED BY CONSULTING EARTH SCIENTISTS.
- F2 FOOTINGS AND FOUNDATIONS HAVE BEEN DESIGNED FOR THE FOLLOWING BEARING PRESSURES:

PAD FOOTINGS - 150 kPa STRIP FOOTINGS - 150 kPa

BORED PILES

FOUNDATION MATERIAL SHALL BE APPROVED BY THE CONSULTING GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF CONCRETE.

FOOTING NOTES:

- 1. FOUNDATION MATERIAL SHALL BE INSPECTED BY A QUALIFIED GEOTECHNICAL ENGINEER TO VERIFY THAT THE DESIGN AND SOIL PARAMETERS SCHEDULED HAVE BEEN ACHIEVED ON SITE.
- 2. REFER FOUNDATION NOTES FOR ALLOWABLE BEARING CAPACITY
- 3. ALL PADS TO COLUMNS SHALL BE SET OUT ON COLUMN CENTERLINE (REFER ARCHITECT'S DRAWINGS FOR SET OUT)
- 5. PF ++++ SF ++++ (WHERE APPLICABLE)
 DENOTES TOP OF FOOTING LEVEL
 RELATIVE TO THE PROJECT DATUM LEVEL
 (REFER NOTE 5 ABOVE)
- 6. ALL TOP OF FOOTING LEVEL SHALL BE PF+ -400 SF+ -400 UNO.
- 7. PAD TAG DEFINITION:
 PF+ (PAD NUMBER / TYPE) WHERE APPLICABLE
 SF+ (STRIP FOOTING TYPE) WHERE APPLICABLE

PAD FOOTING SCHEDULE SIZE REINFORCEMENT SOIL / ROCK ALLOWABLE BEARING CAPACITY MARK No. DIM 'A' DIM 'B' DIM 'C' (DEPTH) 'X' BARS 'Y' BARS f'c (MPa) CAPACITY PF1 3000 2700 500 N20-200 N20-200 32.00 150 kPa PF2 2400 2100 500 N20-200 N20-200 32.00 150 kPa

	PIER SCHEDULE														
			REIN	FORCEMENT				ALLOWABLE	ALLOWABLE						
								END	SKIN						
					TIES /	SOCKET	FOUNDING	BEARING	FRICTION						
MARK No.	DIAMETER	f"c (MPa)	BARS	LENGTH	HELIX	LENGTH	STRATA	(kPa)	(kPa)						
P1	600	32	8-N16	6000 MIN.	R10-300 TIES	2000	STIFF CLAY	150	15						
P2	450	32	MASS CONCRETE	-	-	2000	-	-	-						
PC1					N20-200										

NOTE:
ALL SHALLOW FOOTINGS (PAD & STRIP FOOTINGS) TO BE SOCKETED
FOR MINIMUM 300mm INTO SOIL WITH ALLOWABLE BEARING CAPACITY
OF150 kPa. GEOTECHNICAL ENGINEER TO CONFIRM.

FOR CONSTRUCTION

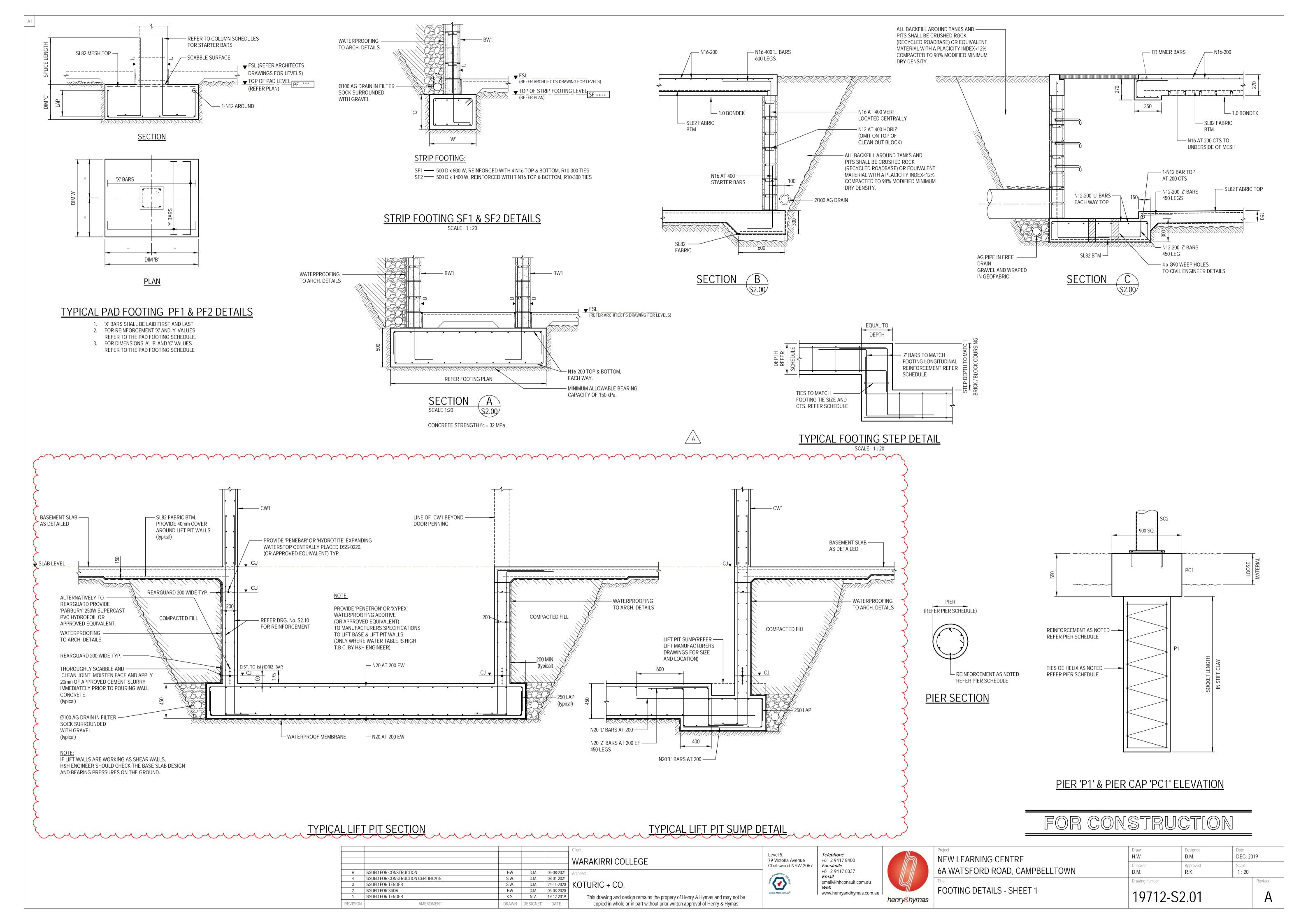


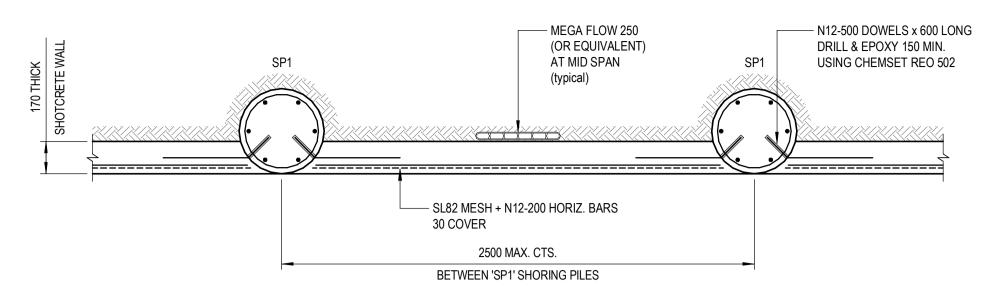
Α	ISSUED FOR CONSTRUCTION	HW	D.M.	05-08-202
4	ISSUED FOR CONSTRUCTION CERTIFICATE	S.W.	D.M.	08-01-2021
3	ISSUED FOR TENDER	S.W.	D.M.	24-11-2020
2	ISSUED FOR SSDA	HW	D.M.	05-03-2020
1	ISSUED FOR TENDER	K.S.	N.V.	19-12-2019
EVISION	AMENDMENT	DRAWN	DESIGNED	DATE
	4 3 2 1	4 ISSUED FOR CONSTRUCTION CERTIFICATE 3 ISSUED FOR TENDER 2 ISSUED FOR SSDA 1 ISSUED FOR TENDER	4 ISSUED FOR CONSTRUCTION CERTIFICATE S.W. 3 ISSUED FOR TENDER S.W. 2 ISSUED FOR SSDA HW 1 ISSUED FOR TENDER K.S.	4 ISSUED FOR CONSTRUCTION CERTIFICATE S.W. D.M. 3 ISSUED FOR TENDER S.W. D.M. 2 ISSUED FOR SSDA HW D.M. 1 ISSUED FOR TENDER K.S. N.V.



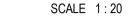
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henry&hymas	

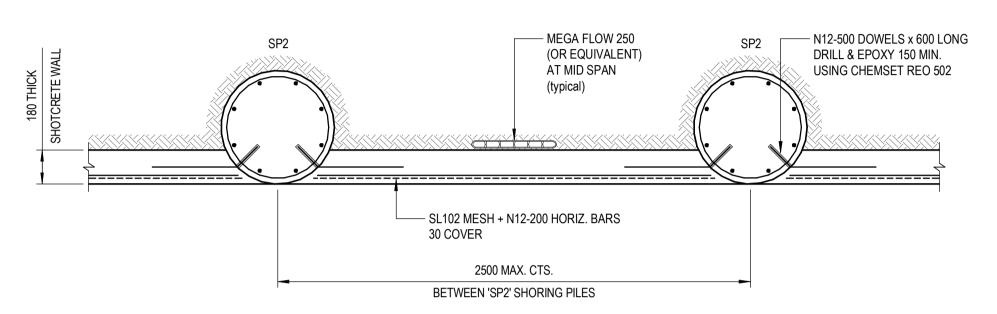
Project	Drawn	Designed	Date		
NEW LEARNING CENTRE	H.W.	D.M.	DEC. 20	19	
6A WATSFORD ROAD, CAMPBELLTOWN	Checked D.M.	Approved R.K.	Scale As indica	icated	
Title	Drawing number			Revision	
FOOTING PLAN	19712-S	2.00		A	





TYPICAL SHORING WALL 'SW1' DETAIL





TYPICAL SHORING WALL 'SW2' DETAIL

- 6-N20 VERT. BARS

R10-300 HELIX 200 PITCH

50 COVER TO HELIX

CAPPING BEAM 'CB1'

R10-300 TIES

- SHORING PILE 'SP1'

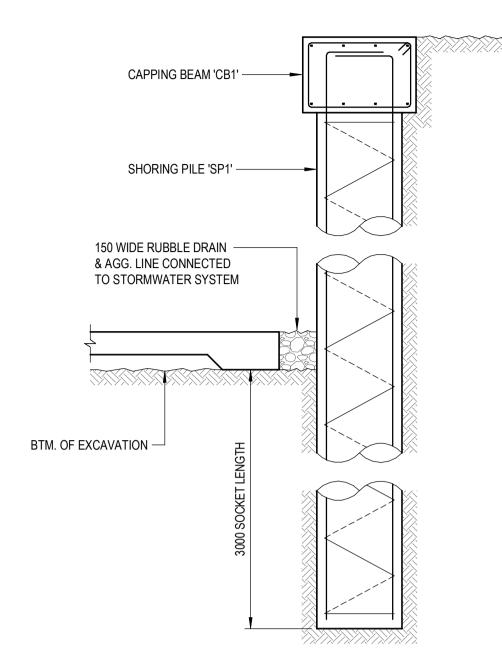
4-N16 BARS TOP & BTM.

CONCRETE STRENGTH f 'c = 32 MPa

FULL LENGTH

SHORING PILE 'SP1' SECTION

CAPPING BEAM 'CB1' DETAIL



TYPICAL SHORING WALL 'SW1' SECTION

800 MIN.

CAPPING BEAM

TIES TO MATCH

CAPPING BEAM

TIES & SPACING

REINFORCEMENT AS

PER TYPICAL DETAIL

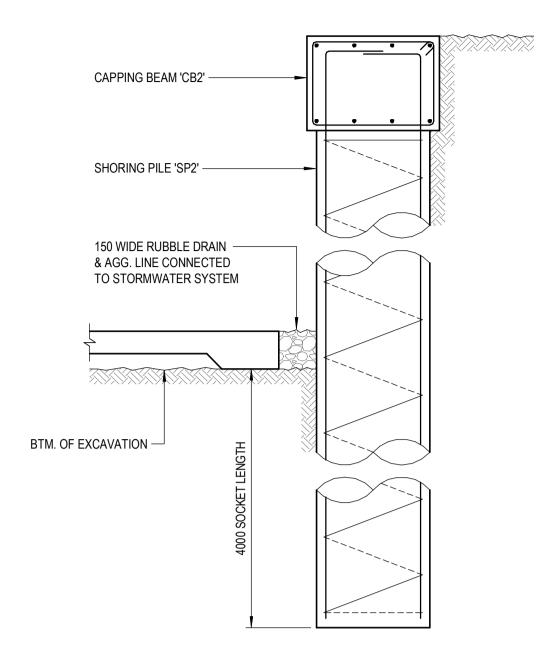
450

TYPICAL CAPPING BEAM STEP DETAIL

- 'Z' BARS TO MATCH CAPPING BEAM

REINFORCEMENT

800 MIN.



TYPICAL SHORING WALL 'SW2' SECTION

SHORING AND SHOTCRETE SHORING PILES

DESIGN OF RETENTION SYSTEMS.

- SH1 PRIOR TO COMMENCEMENT OF WORK, THE BUILDING CONTRACTOR SHALL OBTAIN ALL NECESSARY PERMITS AND APPROVALS FOR ANY ENCROACHMENT OF EXCAVATIONS AND GROUND ANCHORS IN TO THE ADJOINING PROPERTIES AND STREETS. LOCATE ALL SERVICES IN THE STREETS LIKELY TO BE AFFECTED BY THE ANCHORS.
- SH2 REFER TO THE ARCHITECTURAL DRAWINGS FOR THE FINISHED SURFACE LEVELS AND SET-OUT OF THE SHORING WALLS. FOOTING LEVELS OF THE ADJACENT BUILDINGS ARE TO BE CONFIRMED BY THE BUILDING CONTRACTOR
- BEFORE ANY WORK IS CARRIED OUT. SH3 SHORING WALLS HAVE BEEN DESIGNED IN ACCORDANCE WITH AS3600 AND AS4678, AND THE GEOTECHNICAL ENGINEER'S RECOMMENDATIONS FOR THE
- SH4 IN THE CASE OF A DESIGN AND CONSTRUCT (D&C) SHORING SYSTEM, THE CONTRACTOR SHALL SUBMIT THE PROPOSED DETAILS AND DESIGN CALCULATIONS FOR THE ENGINEER'S REVIEW PRIOR TO COMMENCING WORK ON
- SH5 INSTALLATION OF SHORING PILES SHALL BE IN ACCORDANCE WITH AS2159 WITH A MAXIMUM POSITIONAL TOLERANCE OF 50 mm AND A MAXIMUM VERTICAL STRAIGHTNESS OF 1% UNO.
- SH6 ALL EXCAVATIONS SHALL BE INSPECTED AT REGULAR INTERVALS BY A QUALIFIED GEOTECHNICAL ENGINEER. REFER TO THE RECOMMENDATIONS IN THE GEOTECHNICAL REPORT. DEWATERING PROPOSALS, IF APPLICABLE, SHALL BE SUBMITTED FOR THE ENGINEER'S REVIEW PRIOR TO COMMENCING ANY
- SH7 ROCK LEVELS SHALL BE VERIFIED ON SITE WITH THE INSTALLATION OF PILES. THE ENGINEER SHALL BE NOTIFIED IMMEDIATELY OF ANY SITE CONDITIONS DIFFERENT TO ASSUMPTIONS NOTED ON THE DRAWINGS.
- SH8 SURVEY POINTS SHALL BE ESTABLISED AT EVERY 5 m ALONG THE SHORING WALLS AND THE MOVEMENTS SHALL BE MONITORED DURING THE CONSTRUCTION PERIOD. THE MONITORING FREQUENCY SHALL BE AS FOLLOWS:
 - EVERY WEEK UNTIL EXCAVATION IS COMPLTE THEN MONTHLY UNTIL PRACTICAL COMPLETION.

SURVEY RECORDS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW,

- SH9 PILES IN CONTIGUOUS PILE WALLS SHALL BE LOCATED SO THE GAPS BETWEEN PILES ARE MINIMISED. INSTALLATION OF CONTIGUOUS PILES SHALL BE CARRIED OUT IN HIT 1 AND MISS 2 SEQUENCE.
- SH10 A COMPLETE DRILLING RECORD SHALL BE MAINTAINED FOR ALL PILES INDICATING THE DATE & TIME, DEPTHS, PLAN LOCATION, BEARING CAPACITY REACHED AND ANY PROBLEMS WITH SITE CONDITIONS.
- SH11 PILES SHALL BE CUT OFF AT THE LEVELS REQUIRED TO ACHIEVE THE CONSTRUCTION OF THE PILE CAPS / CAPPING BEAMS AS INDICATED ON THE
- SH12 PILING CONTRACTOR SHALL CERTIFY THAT THE PILES ARE STRUCTURALLY ADEQUATE TO SUSTAIN THE GIVEN WORKING LOADS. PILE TESTING REQUIREMENT SHALL BE CARRIED OUT BY THE PILING CONTRACTOR TO COMPLY

DRAWINGS. ALLOW PILE REINFORCEMENT TO PROTRUDE INTO PILE CAPS.

- SH13 UPON COMPLETION OF PILING, THE PILING CONTRACTOR SHALL FURNISH THE BUILDING CONTRACTOR AND THE ENGINEER, THE FOLLOWING DOCUMENTS.
 - i) A WORK-AS-EXECUTED SURVEY OF THE PLAN POSITIONS OF ALL PILES
- PREPARED BY A QUALIFIED SURVEYOR. ii) A CERTIFICATE FROM A QUALIFIED ENGINEER (IN THE CASE OF 'DESIGN & CONSTRUCT' PILES) THAT ALL PILES HAVE BEEN DESIGNED, TESTED AS NECESSARY AND INSTALLED IN ACCORDANCE WITH AS2159 TO SAFELY CARRY THE LOADS AS INDICATED ON THE DRAWINGS.
- SH14 THE PILING RIG SHALL NOT BE DEMOBILISED FROM SITE UNTIL THE ENGINEER AND THE GEOTECHNICAL CONSULTANT HAVE ISSUED THE FINAL SIGN-OFF OF ALL PILES.
- SH15 <u>SHOTCRETE:</u>
- ST15.1 THE CONTRACTOR SHALL SUBMIT A WORK METHOD STATEMENT FOR APPROVAL BY THE SUPERINTENDENT PRIOR TO COMMENCING THE WORKS. THE METHOD STATEMENT SHALL ADDRESS THE MIX DESIGN, QUALIFICATIONS OF THE OPERATORS, PLANT, SUBSTRATE PREPARATION AND SPRAYING PROCEDURE.
- ST15.2 ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS3600, CIA Z5, RMS QA SPECIFICATION B82 AND OTHER RELEVANT AUSTRALIAN

STANDARDS. ST15.3 SHOTCRETE COMPONENTS AND QUALITY SHALL BE AS FOLLOWS;

ELEMENT	STRENGTH GRADE	SLUMP (mm)	MAX. AGG. (mm)	MIN. BINDER	AVERAGE BASIC DRYING SHRINKAGE
SHOTCRETE WALL	S40	60	10	EUCOSHOT B. OR APPROVED EQUIVALENT REBOUND 10% MAX.	750 MICROSTRANS

ST15.4 SUBSTRATE PREPARATION

- SURFACES TO BE SPRAYED WITH SHOTCRETE SHALL BE FREE OF LOOSE AND FOREIGN MATERIAL. THE SURFACE SHALL BE COMPACTED, TRIMMED AND GRADED AS REQUIRED AND PRE-WETTED PRIOR TO THE
- APPLICATION OF SHOTCRETE. SURFACES TO WHICH SHOTCRETE IS APPLIED SHALL HAVE NO FREE OR
- RUNNING WATER. INSTALL DIVERSION DRAINS WHERE REQUIRED. ADDITIONAL STRIP DRAINS SHALL BE PROVIDED AT SEEPING JOINTS. ROCK DEFECTS OR WHERE INSTRUCTED BY THE GEOTECHNICAL ENGINEER.
- ANY UNSTABLE SUBSTRATE AREAS SHALL BE STABILISED BY INSERTING TIMBER SLEEPERS IN BETWEEN THE SHORING PILES OR AS DIRECTED BY THE GEOTECHNICAL ENGINEER.

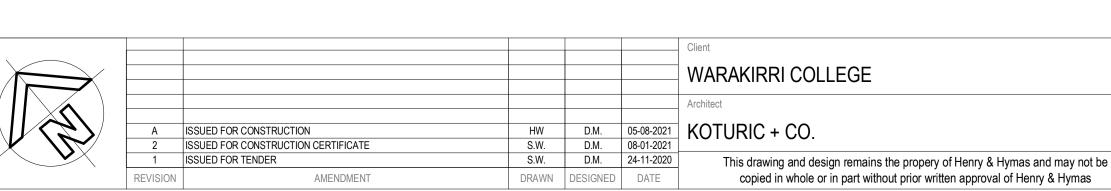
ST15.5 SPRAYING SHOTCRETE

- NO CONCRETE SHALL BE SPRAYED IN AIR TEMPERATURE LESS THAN 1 DEGREE CELSIUS (1° C), OR ABOVE 35° C.
- FRESHLY SPRAYED CONCRETE SHALL BE PROTECTED FROM RAIN OR WATER UNTIL THE SURFACE IS SUFFICIENTLY HARD TO PREVENT
- SPRAYING SHALL BE DISCONTINUED IF WIND OR AIR CURRENTS CAUSE
- SEPARATION OF THE NOZZLE STREAM DURING PLACEMENT. DURING STARTING AND STOPPING OF THE SPRAY OPERATION OR
- WHENEVER SPRAYING IS IRREGULAR THE NOZZLE SHALL BE DIRECTED AWAY FROM THE WORKS.
- ALL CORNERS AND ANY AREAS WHERE REBOUND CANNOT ESCAPE OR BE BLOWN FREE, SUCH AREAS SHALL BE FILLED PRIOR TO GENERAL
- REBOUND SHALL NOT BE WORKED INTO THE CONSTRUCTION OR RE-USED
- IN THE WORKS. • GUIDES SHALL BE SET UP TO ESTABLISH FINISHED SURFACES THESE GUIDES SHALL BE TO THE APPROVAL OF THE ENGINEER PRIOR TO
- SPRAYING. SHOTCRETE SHALL BE APPLIED IN SUCH A WAY THAT IT NEITHER SAGS
- NOR SLUMPS. • SHOTCRETE SHALL BE TROWELLED TO A SMOOTH SURFACE. REFER TO
- THE ARCHITECT'S SPECIFICATION FOR THE REQUIRED FINISH.
- FULL RECORDS OF ALL MATERIALS DELIVERED TO THE SPRAYED CONCRETE MIXER SHALL BE KEPT AND MADE AVAILABLE TO THE SUPERINTENDENT.
- ST15.6 INFILL SHOTCRETE PANELS SHALL BE APPLIED PROGRESSIVELY AS EXCAVATION PROGRESSES, AT VERTICAL LIFTS OF APPROXIMATELY 1.8 m. CONSTRUCTION JOINTS SHALL BE SCABBLED AND PRE-WETTED PRIOR TO THE APPLICATION OF SHOTCRETE.

ST15.7 QUALITY CONTROL

TESTING OF SHOTCRETE SHALL BE CARRIED OUT IN ACCORDANCE WITH

'RECOMMENDED PRACTICE FOR SPRAYED CONCRETE' PUBLISHED BY THE CONCRETE INSTITUTE OF AUSTRALIA.



- 8-N24 VERT. BARS

50 COVER TO HELIX

CAPPING BEAM 'CB2'

SHORING PILE 'SP2'

R10-300 TIES

4-N16 BARS TOP & BTM.

N12-300 HELIX 200 PITCH

CONCRETE STRENGTH f'c = 32 MPa

FULL LENGTH

SHORING PILE 'SP2' SECTION

CAPPING BEAM 'CB2' DETAIL

Level 5, 79 Victoria Avenue Chatswood NSW 2067 Facsimile





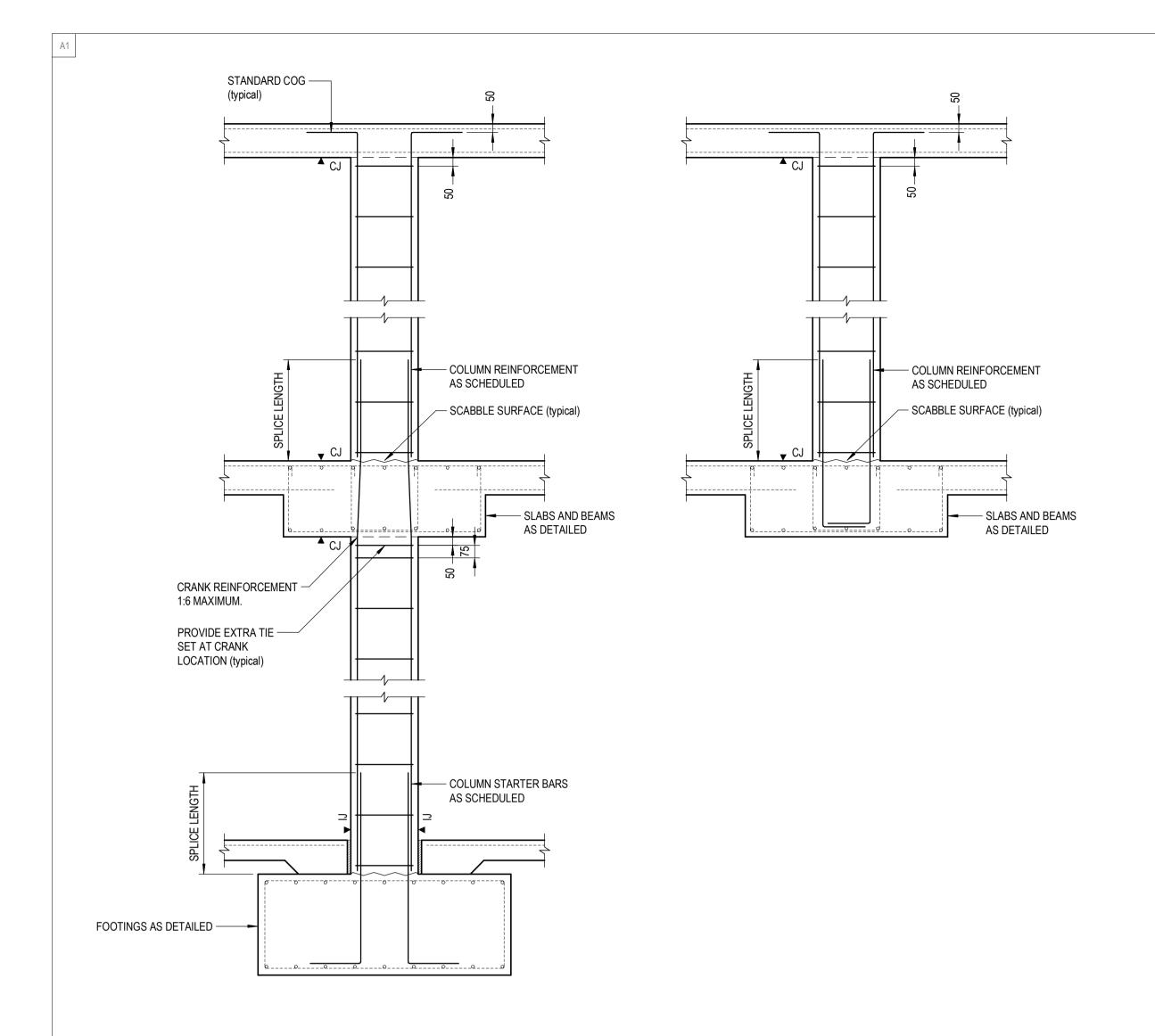
FOR CONSTRUCTION

NEW LEARNING CENTRE 6A WATSFORD ROAD, CAMPBELLTOW
FOOTING DETAILS - SHEET 2

D.M. 11/23/20 H.W. D.M. R.K. As indicated Revision Drawing number

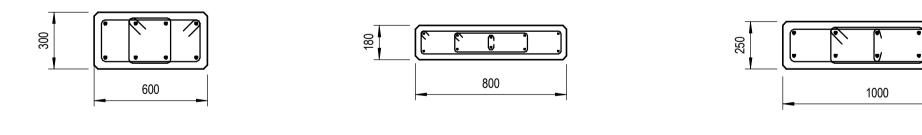
19712-S2.02

Α



TYPICAL COLUMN ELEVATIONS

COLUMN 'C1' DETAIL



COLUMN 'C2' DETAIL

8 N20

10 N16

10 N20

COLUMN 'C3' DETAIL

2R10-300

3R10-180

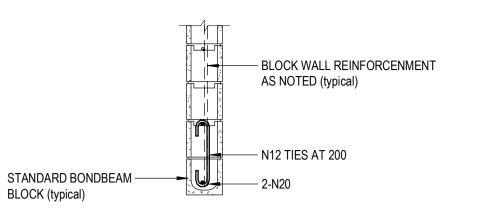
3R10-250

8 N20

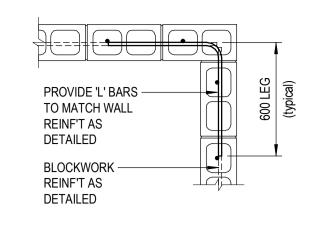
10 N16

10 N20

		COLUMN SCHEDULE		
COLUMN TYPE	fc (MPa)	COLUMN REINFORCEMENT	STARTER BARS	TIES

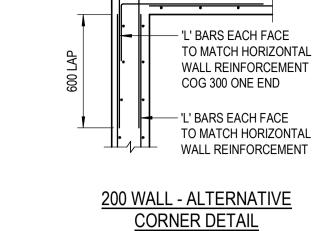






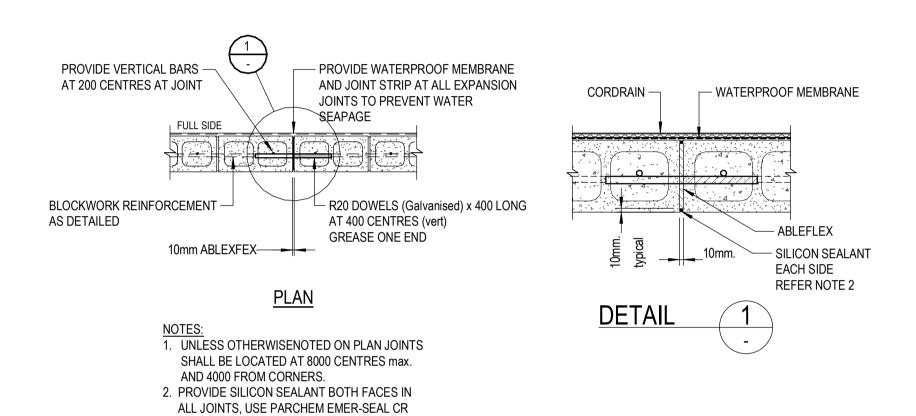
TYPICAL BLOCKWORK CORNER DETAIL

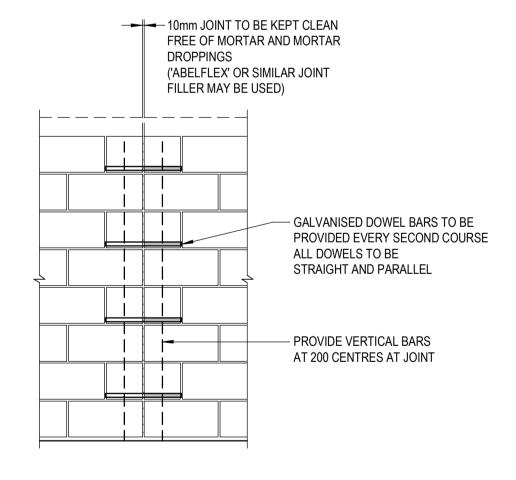
SCALE 1:20
(INTERNAL OR EXTERNAL)



TYPICAL WALL INTERSECTION PLAN DETAILS

WALL SCHEDULE											
TYPE MARK	WALL TYPE	f'c (MPa)	VERTICAL REINFORCEMENT	STARTER BARS	HORIZONTAL REINFORCEMENT						
BW1	200 SERIES BLOCKWORK	25	N16-200 CENTRAL	N16-200 CENTRAL	N16-400 CENTRAL						
CW1	200 RC WALL	40	N20-200 EACH FACE	N20-200 EACH FACE	N12-200 EACH FACE						





ELEVATION

TYPICAL BLOCKWORK VERTICAL EXPANSION JOINT

OR AN APPROVED EQUIVALENT.

FOR CONSTRUCTION

H.W.

D.M.

Drawing number

Designed D.M.

Approved

R.K.

19712-S2.10

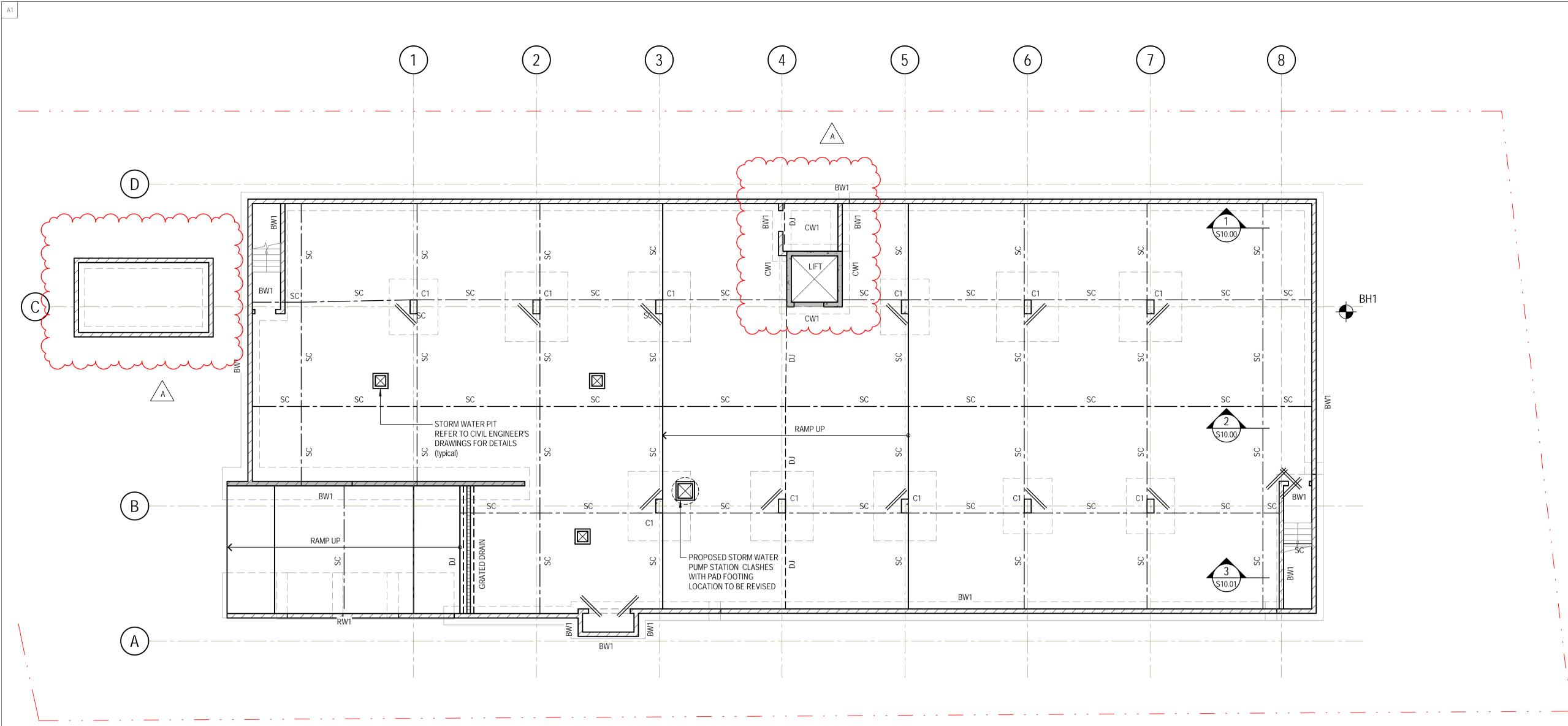
DEC. 2019

Revision

Α

1:20

					Client				Project
					WARAKIRRI COLLEGE	Level 5, 79 Victoria Avenue	Telephone +61 2 9417 8400		NEW LEARNING CENTRE
			_			Chatswood NSW 2067	Facsimile		6A WATSFORD ROAD, CAMPBELLTOWN
A	ISSUED FOR CONSTRUCTION	HW	D.M.	05-08-2021	Architect	anagement.	+61 2 9417 8337	iii iii	OA WATSFORD ROAD, CAIVIFDELLTOWN
4	ISSUED FOR CONSTRUCTION CERTIFICATE	S.W.	D.M.	08-01-2021		The state of the s	Email		Titla
3	ISSUED FOR TENDER	S.W.	D.M.	24-11-2020	KOTURIC + CO.	a	email@hhconsult.com.au		THE
2	ISSUED FOR SSDA	HW	D.M.	05-03-2020		Global-Mark.com.au®	Web www.henryandhymas.com.au		TYPICAL COLUMN AND WALL DETAILS
1	ISSUED FOR TENDER	K.S.	N.V.	19-12-2019	This drawing and design remains the propery of Henry & Hymas and may not be	Global-wark.com.au -	www.nemyandnymas.com.au	book (0 by moos	
REVISIO	N AMENDMENT	DRAWN	DESIGNED	DATE	copied in whole or in part without prior written approval of Henry & Hymas			henry&hymas	



LOWER GROUND SLAB PLAN

- SCALE 1:100

 120 THICK SLAB ON GROUND (fc = 32 MPa) WITH SL82 TOP MESH WITH 30 COVER (U.N.O.) ON 50 mm SAND & 100mm CRUSHED ROAD BASE OR DGB20 COMPACTED TO 98% SDD.
- ALL COLUMNS AND WALLS TO BE ISOLATED FROM SLAB ON GROUND BY ISOLATION JOINTS (IJ) ALL AROUND. REFER TO DRG. No. S3.01 FOR ISOLATION JOINT DETAIL.

SLAB ON GROUND NOTES:

- SG1 REFER 'CONCRETE NOTES' FOR SPECIFICATIONS ON CONCRETE SUPPLY, PLACING, FINISHING AND CURING.
- SG2 FOLLOWING THE COMPLETION OF EARTHWORKS, A SUBBASE OF 100 mm
 THICKNESS UNO, SHALL BE PLACED OVER THE SUBGRADE AND COMPACTED TO
 98% MODIFIED MAXIMUM DRY DENSITY.
- SG3 INSTALL A VAPOUR BARRIER IF SPECIFIED. REINFORCEMENT MESH SHALL BE PLACED AT THE SPECIFIED DEPTH SUPPORTED ON BAR CHAIRS SPACED ON A 0.8 1.0 m GRID FOR MESH SIZES SL82 OR LARGER AND 0.6 m SPACING FOR LIGHER MESH. INDEPENDENT SUPPORTS NOT RESTING ON THE REINFORCEMENT OR SIDE FORMS SHALL BE USED TO CARRY OTHER

CONSTRUCTION LOADINGS SUCH AS PLANT OR EQUIPMENT.

BAR CHAIRS SHALL BE FITTED WITH A PLATE SUPPORT UNDER THE LEGS TO PREVENT THEM PUNCTURING THE VAPOUR BARRIER AND SINKING INTO THE

THE PRACTICE OF LAYING REINFORCING MESH ON THE SUBBASE BEFORE CONCRETE IS PLACED AND LIFTING IT INTO POSITION AFTER PLACING, OR PLACING IT ON THE FINISHED SURFACE OF THE CONCRETE AND 'WALKING IT IN', ARE STRICTLY NOT PERMITTED.

SG4 WHERE A VAPOUR BARRIER IS SPECIFIED BENEATH A SLAB ON GROUND, PROVIDE A 0.2 mm POLYETHYLENE MEMBRANE OF MEDIUM IMPACT RESISTANCE IN ACCORDANCE WITH THE PROVISIONS OF AS2870. THE SHEETING SHALL BE CONTINUOUS UNDER THE SIDE FORMS AND LAPPED AT THE JOINTS BY A MINIMUM OF 200 mm.

THE VAPOUR BARRIER SHALL BE PLACED DIRECTLY ON THE SUBBASE, BUT IF THE SURFACE IS ROUGH AND LIKELY TO DAMAGE THE PLASTIC SHEETING, A BLINDING LAYER OF FINE MATERIAL SUCH AS QUARRY DUST SHALL BE PROVIDED.

SPECIAL CARE SHALL BE TAKEN TO AVOID DAMAGE TO THE VAPOUR BARRIER PRIOR TO AND DURING CONCRETING, AND ANY TEARS OR PERFORATIONS SHALL BE PATCHED IMMEDIATELY.

- FOR THE CONCRETE SUPPLY TRUCKS TO BE ABLE TO DISCHARGE THEIR LOADS CLOSER TO THE FINAL POSITION, THE SITE SHALL BE PLANNED WITHOUT ANY OBSTACLES SUCH AS EXCAVATED SOIL, BUILDING MATERIALS AND CONSTRUCTION SHEDS/ OFFICES. IF CONCRETE HAS TO BE MOVED BY MANUAL METHODS, IT SHALL BE CARRIED OUT WITH SHOVELS. POKER VIBRATORS SHALL NOT BE USED TO MOVE CONCRETE.
- SG6 SAW CUTTING CONCRETE PAVEMENTS SHALL BE COMPLETED WITHIN 4 TO 12 HOURS AFTER CONCRETE HAS SET ACCORDING TO THE FOLLOWING
 - MARK OUT SAW CUT LOCATIONS ACCURATELY USING A CHALK LINE
 - SAW CUT IN ONE PASS TO THE CORRECT DEPTH
- RECORD THE TIME OF SAW CUT TO BE AND LOG WITH THE BUILDER
 COMMENCE SAW CUTTING WITH THE 1ST CUT FROM THE OUTSIDE EDGE
 AND CONTINUE IN A POTATIONAL OPERATOWARDS THE MIDDLE OF SLAP.
- AND CONTINUE IN A ROTATIONAL ORDER TOWARDS THE MIDDLE OF SLAB THE PANEL, REFER SLAB ON GROUND DETAILS.

SLAB ON GROUND LEGEND:

+++ DENOTES SLAB THICKNESS

DENOTES SLAB STEPDOWN

DENOTES 2-N12 TRIMMERS TOP x 1200 LONG

SLAB JOINTS DENOTED THUS:

TKJ DENOTES TIED KEYED JOINT

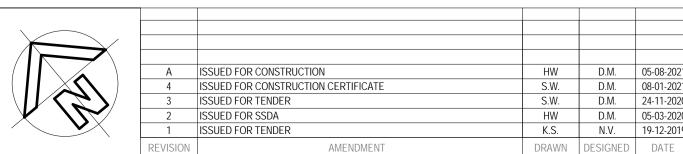
___DJ __ DENOTES KEYED JOINT

____SC___ DENOTED DOWELED JOINT

TG DENOTES SAW CUT JOINT

J DENOTES TOOLED GROOVE

FOR CONSTRUCTION



WARAKIRRI COLLEGE

HW D.M. 05-08-2021
S.W. D.M. 08-01-2021
S.W. D.M. 24-11-2020
HW D.M. 05-03-2020
K.S. N.V. 19-12-2019
DRAWN DESIGNED DATE

WARAKIRRI COLLEGE

Architect
KOTURIC + CO.

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Level 5,
79 Victoria Avenue
Chatswood NSW 2067

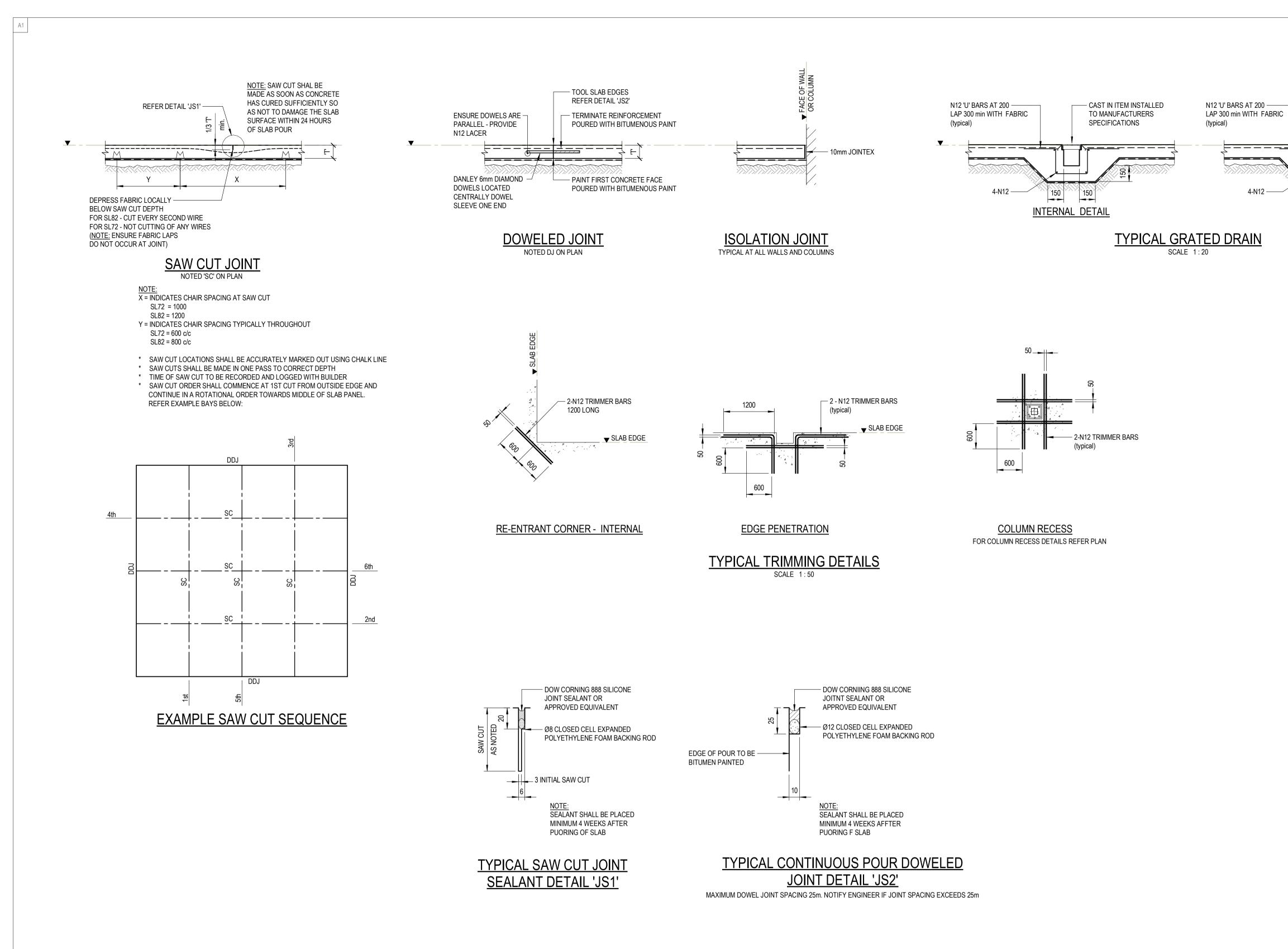
Chatswood NSW 2067

Facsimile
+61 2 9417 8337

Email
email@hhconsult.com.au
Web
www.henryandhymas.com.au



					
oject	Drawn H.W.	Designed D.M.	Date DEC, 2019		
EW LEARNING CENTRE					
A WATSFORD ROAD, CAMPBELLTOWN	Checked D.M.	Approved R.K.	Scale As indicated		
le	Drawing number		Revision		
OWER GROUND SLAB PLAN	19712	-S3.00	A		



- CAST IN ITEM INSTALLED

— 2-N12 TRIMMERS

TOP ALL ROUND

EACH WAY

(typical)

?x?

TYPICAL SUMP DETAIL

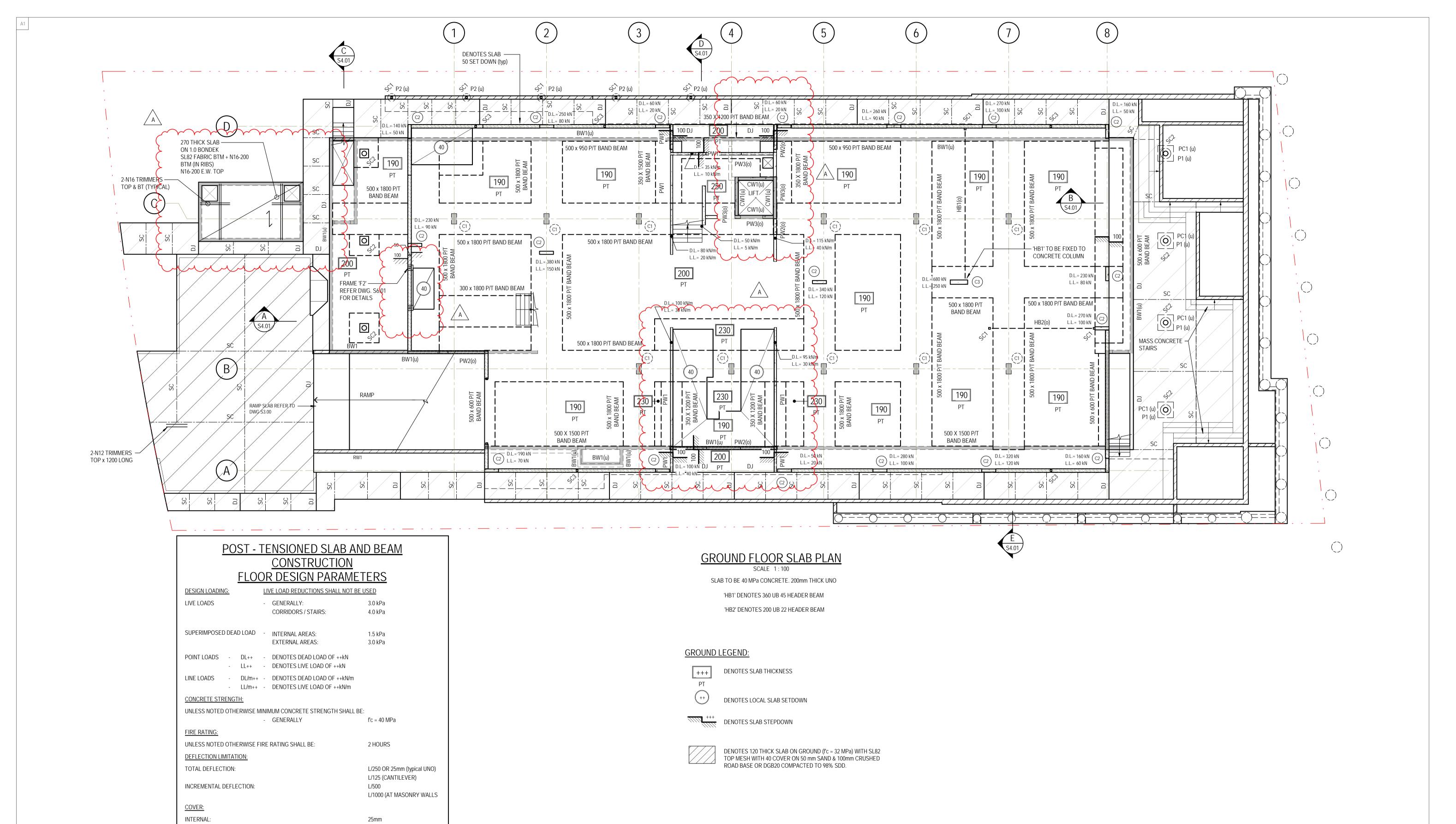
TO MANUFACTURERS

SPECIFICATIONS

150

EDGE DETAIL

					Client				Project	Drawn	Designed	Date
					WARAKIRRI COLLEGE	Level 5, 79 Victoria Avenue	Telephone +61 2 9417 8400		NEW LEARNING CENTRE	H.W.	D.M.	DEC. 2019
					WARANIRRI GOLLEGE	Chatswood NSW 2067				Checked	Approved	Scale
Α	ISSUED FOR CONSTRUCTION	HW	D.M.	05-08-2021	Architect	nagement.	+61 2 9417 8337	<i> </i>	6A WATSFORD ROAD, CAMPBELLTOWN	D.M.	R.K.	As indicated
4	ISSUED FOR CONSTRUCTION CERTIFICATE	S.W.	D.M.	08-01-2021		and the second	Email		Titlo	Drawing number		Revision
3	ISSUED FOR TENDER	S.W.	D.M.	24-11-2020	KOTURIC + CO.				Tiue	Drawing number		IVENISION
2	ISSUED FOR SSDA	HW	D.M.	05-03-2020		Global-Mark.com.au [®]	Web SLAB ON GROUND		SLAB ON GROUND DETAILS - LOWER GROUND	40740	00.04	
1	ISSUED FOR TENDER	K.S.	N.V.	19-12-2019	This drawing and design remains the propery of Henry & Hymas and may not be	Giobal-Mark.com.au-	www.henryandhymas.com.au			19712-	-S3 ()1	Ι Δ
REVISION	AMENDMENT	DRAWN	DESIGNED	DATE	copied in whole or in part without prior written approval of Henry & Hymas			henry&hymas		10112	00.01	/ \





1. THE MINIMUM CONCRETE STRENGTH AS NOTED MAY BE INCREASED BY THE POST TENSIONING

2. THE POST TENSIONING CONTRCTOR SHALL DESIGN AND DOCUMENTS ALL ELEMENTS CAST

3. STRUCTURAL SIZES AND FRAMING AS INDICATED ON PLAN ARE INDICATIVE ONLY, THE POST TENSIONING CONTRACTOR MAY VARY THE INDICATED SIZES AS REQUIRED TO COMPLY WITH

4. FOR CO-ORDINATION PURPOSES, ANY VARIATIONS TO THIS PLAN/DESIGN MADE BY THE POST

INTEGRALLY WITH THE POST TENSIONED FLOOR INCLUDING ALL EDGE BEAMS.

AUSTRALIAN STANDARDS OR TO PRODUCE A MORE ECONOMICAL DESIGN.

TENSIONING CONTRACTOR SHALL BE APPROVED BY HENRY AND HYMAS.

40mm

EXPOSED SURFACES:

PROVIDE PLASTIC OR CONCRETE BAR CHAIRS TO ALL EXPOSED SURFACES.

DESIGNER TO FACILITATE THE STRESSING PROGRAM.

| HW | D.M. | 05-08-2021 | S.W. | D.M. | 08-01-2021 | S.W. | D.M. | 24-11-2020 | S.J.K. | D.M. | 29-05-2020 | HW | D.M. | 05-03-2020 | K.S. | N.V. | 19-12-2019 | WARAKIRRI COL | Architect | KOTURIC + CO. | ISSUED FOR CONSTRUCTION 5 ISSUED FOR CONSTRUC 4 ISSUED FOR TENDER ISSUED FOR CONSTRUCTION CERTIFICATE 3 RE-ISSUED FOR SSDA 2 ISSUED FOR SSDA ISSUED FOR TENDER REVISION AMENDMENT DRAWN DESIGNED DATE

WARAKIRRI COLLEGE

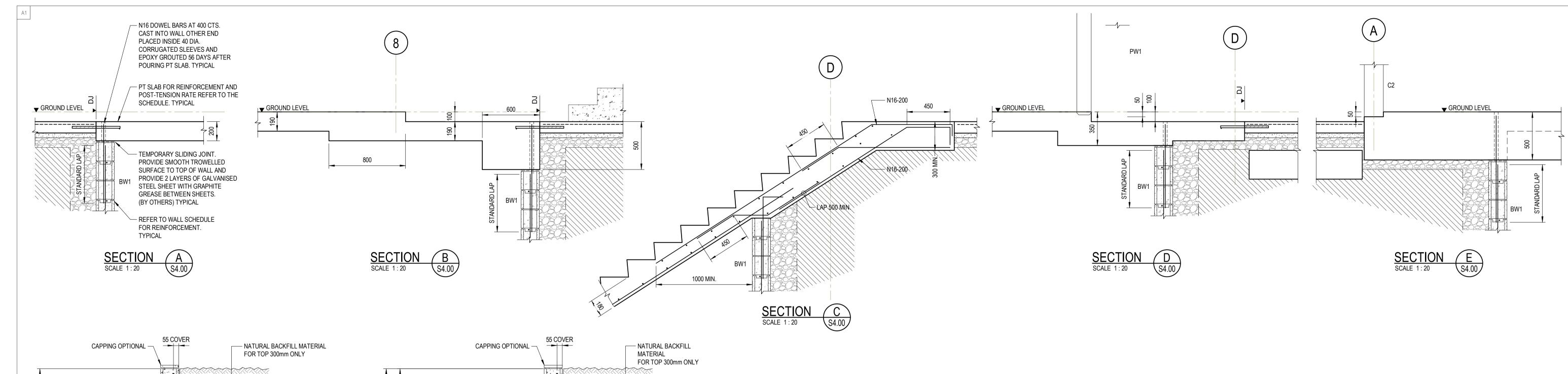
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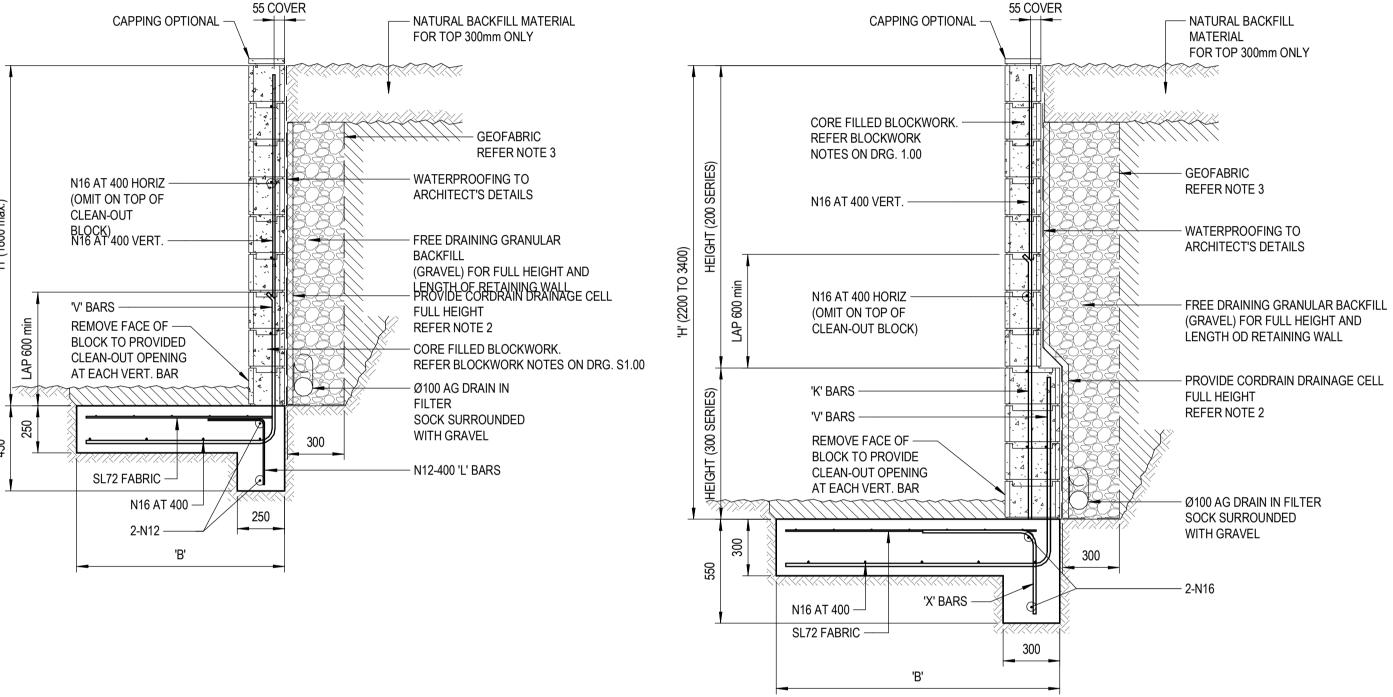


Telephone +61 2 9417 8400 Facsimile +61 2 9417 8337 Email email@hhconsult.com.au Web www.henryandhymas.com.au



Project	Drawn	Designed	Date	
NEW LEARNING CENTRE	H.W.	D.M.	DEC. 20	19
44 MATCEODD DOAD, CAMDDELL TOWN	Checked	Approved	Scale	
OA WATSFURD RUAD, CAMPBELLTUWN	D.M.	R.K.	As indica	ated
Title	Drawing number			Revision
GROUND FLOOR SLAB PLAN	19712-S	4.00		Α
	NEW LEARNING CENTRE 6A WATSFORD ROAD, CAMPBELLTOWN Title	NEW LEARNING CENTRE 6A WATSFORD ROAD, CAMPBELLTOWN Title CROUND ELOOP SLAP DLAN	NEW LEARNING CENTRE 6A WATSFORD ROAD, CAMPBELLTOWN Title H.W. Checked D.M. Approved R.K. Drawing number	NEW LEARNING CENTRE 6A WATSFORD ROAD, CAMPBELLTOWN Title CPOLIND ELOOP SLAR DLAN





150 OR 200 SERIES BLOCK WALL

200 OR 300 SERIES BLOCK WALL 5.0 kPa SURCHARGE

5.0 kPa SURCHARGE

RETAINING WALL SCHEDULE HEIGHT OF BLOCK TYPE TOTAL HEIGHT 'K' BARS 'H' (mm) 150 SERIES 200 SERIES 300 SERIES 800 800 800 N12 AT 400 1000 1000 900 N12 AT 400 1200 1200 1000 N12 AT 400 1400 1400 1100 N16 AT 400 1600 1600 N16 AT 400 1400 1800 1800 1600 N16 AT 400 2000 1400 N16 AT 200 N16 AT 200 600 1800 2200 1400 800 2000 N16 AT 200 N16 AT 200 2400 1600 2200 N16 AT 200 800 N16 AT 200 2600 1600 2400 N16 AT 200 N16 AT 200 1000 2800 2600 N16 AT 200 N16 AT 200 1800 1000 3000 2000 1000 3000 N16 AT 200 N16 AT 200 3200 2000 1200 3300 N20 AT 200 N16 AT 200 3400 2000 1400 3600 N20 AT 200 N16 AT 200

NOTES:

1. WATERPROOFING TO ARCHITECT'S DETAILS

2. INSTALL FULL HEIGHT DRAINAGE CELL TO REAR OF WALL, USE NYLEX CORDRAIN/18 OR APPROVED EQUIVALENTS.

3. PROVIDE GEOFABRIC MATERIAL AS SEPARATION BETWEEN GRANULAR BACKFILL (GRAVEL) AND NATURAL BACKFILL MATERIAL. USE BIDIM A24 OR APPROVED EQUIVALENT.

TYPICAL RETAINING WALL RW1 DETAIL

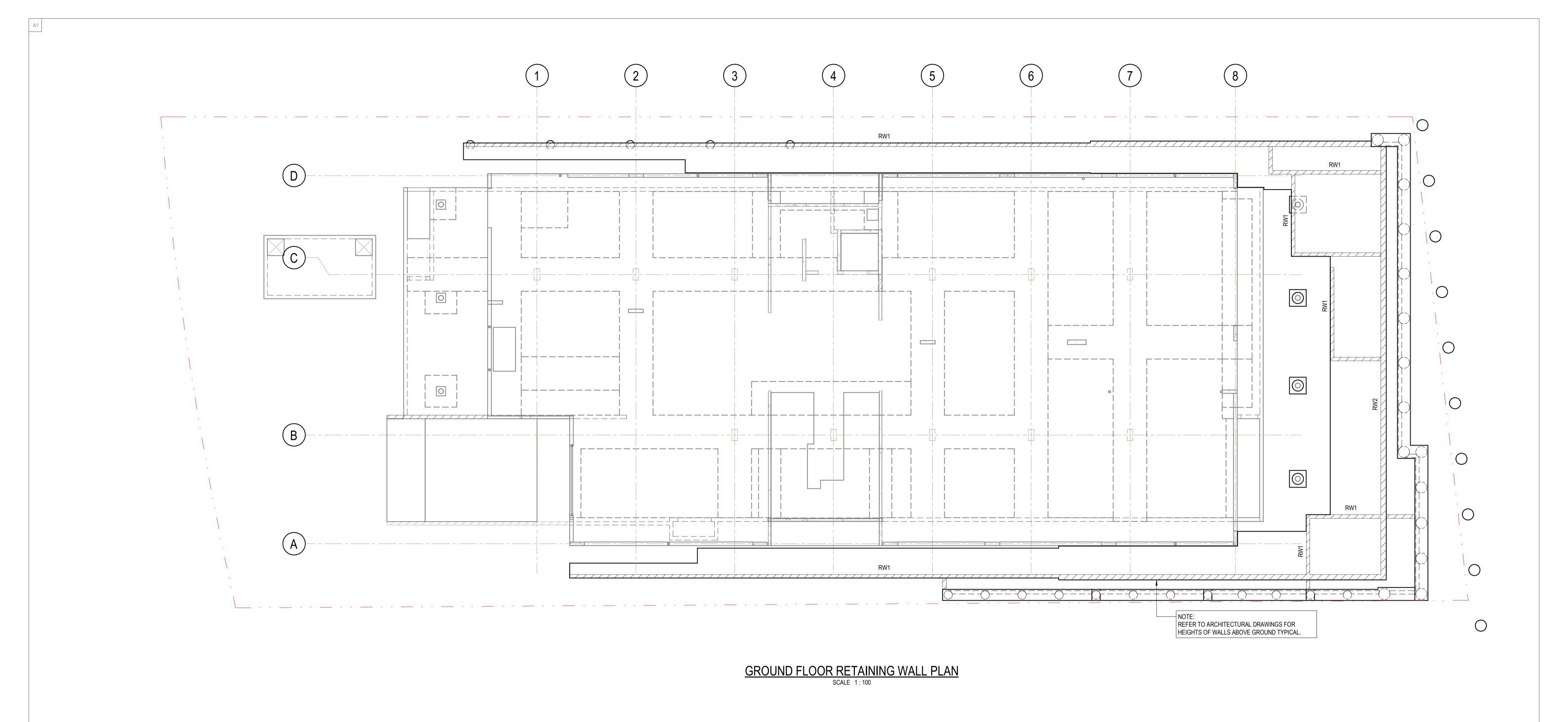
Α	ISSUED FOR CONSTRUCTION	HW	D.M.	05-08-2021
5	ISSUED FOR CONSTRUCTION CERTIFICATE	S.W.	D.M.	08-01-2021
4	ISSUED FOR TENDER	S.W.	D.M.	24-11-2020
3	RE-ISSUED FOR SSDA	S.J.K.	D.M.	29-05-2020
2	ISSUED FOR SSDA	HW	D.M.	05-03-2020
1	ISSUED FOR TENDER	K.S.	N.V.	19-12-2019
REVISION	AMENDMENT	DRAWN	DESIGNED	DATE

	Client	١.
	WARAKIRRI COLLEGE	
021		(
021	Architect	ĺ
020		
020	KOTURIC + CO.	
020		
019	This drawing and design remains the propery of Henry & Hymas and may not be	
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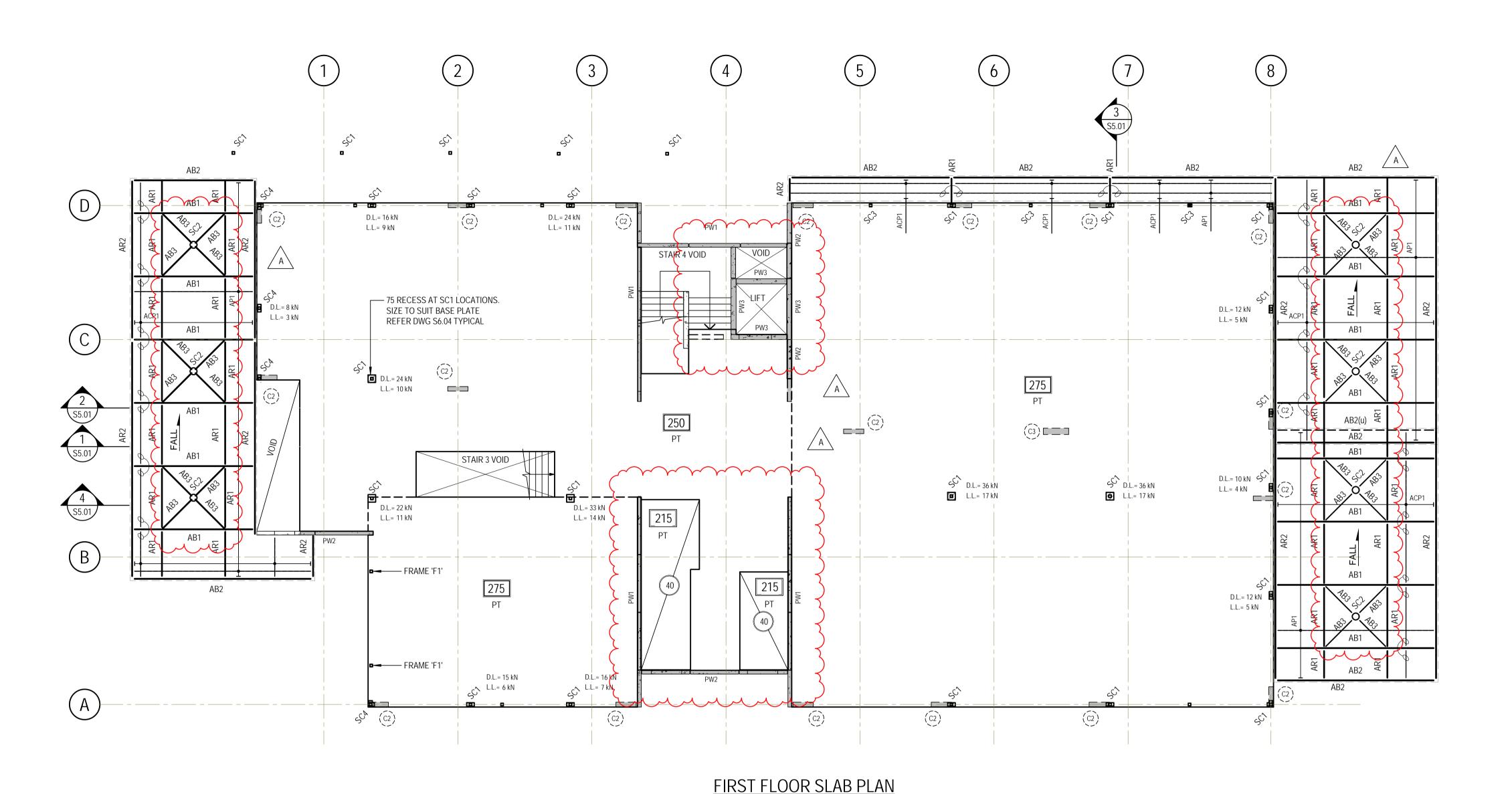
Level 5, 79 Victoria Avenue **Telephone** +61 2 9417 8400 Chatswood NSW 2067 +61 2 9417 8337 email@hhconsult.com.au www.henryandhymas.com.au



TOJECT JEW LEARNING CENTRE	Drawn H.W.	Designed D.M.	Date DEC. 20)19
SA WATSFORD ROAD, CAMPBELLTOWN	Checked D.M.	Approved R.K.	Scale 1:20	
BROUND FLOOR SLAB DETAILS	19712-S	1 ∩1		Revision
	19/12-3	4.01		\Box



,					WARAKIRRI COLLEGE	Level 5, 79 Victoria Avenue	Telephone +61 2 9417 8400		Project NEW LEARNING CENTRE	Drawn S.K.	Designed D.M.	Date 05/28/20
					Architect	Chatswood NSW 2067	Facsimile +61 2 9417 8337		6A WATSFORD ROAD, CAMPBELLTOWN	Checked D.M.	Approved R.K.	Scale 1:100
A ISSUED FOR COI 3 ISSUED FOR COI 2 ISSUED FOR TEN	NSTRUCTION NSTRUCTION CERTIFICATE IDER	HW S.W. S.W.	D.M. D.M.	05-08-2021 08-01-2021 24-11-2020	KOTURIC + CO.	13 (SO) 9001	Email email@hhconsult.com.au Web		GROUND FLOOR - RETAINING WALL PLAN	Drawing number		Revision
1 ISSUED FOR SSE	AMENDMENT	S.J.K. DRAWN	D.M. DESIGNED	29-05-2020 DATE	This drawing and design remains the propery of Henry & Hymas and may not be copied in whole or in part without prior written approval of Henry & Hymas	Global-Mark.com.au [®]	www.henryandhymas.com.au	henry&hymas	SINGUID FEOUR TREIT MINITO WITE FEITH	19712-	S4.02	1



ALL SLABS TO BE 275mm THICK U.N.O.

CONCRETE GRADE f'c = 40MPa

POST - TENSIONED SLAB AND BEAM CONSTRUCTION

FLOOR DESIGN PARAMETERS

CORRIDORS / STAIRS:

DESIGN LOADING:

LIVE LOAD REDUCTIONS SHALL NOT BE USED

LIVE LOADS - GENERALLY: 3.0 ki

SUPERIMPOSED DEAD LOAD - INTERNAL AREAS: 1.5 kPa EXTERNAL AREAS: 3.0 kPa

4.0 kPa

L/1000 (AT MASONRY WALLS

25mm

40mm

POINT LOADS - DL++ - DENOTES DEAD LOAD OF ++kN
- LL++ - DENOTES LIVE LOAD OF ++kN/m
LINE LOADS - DL/m++ - DENOTES DEAD LOAD OF ++kN/m

CONCRETE STRENGTH:

UNLESS NOTED OTHERWISE MINIMUM CONCRETE STRENGTH SHALL BE:

- GENERALLY f'c = 40 MPa

FIRE RATING:

UNLESS NOTED OTHERWISE FIRE RATING SHALL BE: 2 HOURS

- LL/m++ - DENOTES LIVE LOAD OF ++kN/m

DEFLECTION LIMITATION:

TOTAL DEFLECTION: L/250 OR 25mm (typical UNO)
L/125 (CANTILEVER)
INCREMENTAL DEFLECTION: L/500

<u>COVER:</u> INTERNAL:

EXPOSED SURFACES:
PROVIDE PLASTIC OR CONCRETE BAR

CHAIRS TO ALL EXPOSED SURFACES.

NOTE:

1. THE MINIMUM CONCRETE STRENGTH AS NOTED MAY BE INCREASED BY THE POST TENSIONING

DESIGNER TO FACILITATE THE STRESSING PROGRAM.

2. THE POST TENSIONING CONTRCTOR SHALL DESIGN AND DOCUMENTS ALL ELEMENTS CAST

INTEGRALLY WITH THE POST TENSIONED FLOOR INCLUDING ALL EDGE BEAMS.

3. STRUCTURAL SIZES AND FRAMING AS INDICATED ON PLAN ARE INDICATIVE ONLY, THE POST

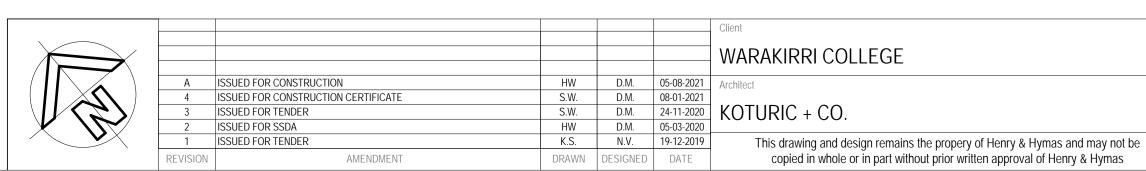
TENSIONING CONTRACTOR MAY VARY THE INDICATED SIZES AS REQUIRED TO COMPLY WITH AUSTRALIAN STANDARDS OR TO PRODUCE A MORE ECONOMICAL DESIGN.

4. FOR CO-ORDINATION PURPOSES, ANY VARIATIONS TO THIS PLAN/DESIGN MADE BY THE POST TENSIONING CONTRACTOR SHALL BE APPROVED BY HENRY AND HYMAS.

AWNING S	AWNING STEEL MEMBER SCHEDULE									
MEMBER TAG	AWNING MEMBER SIZE									
AB1	200 UB 22									
AB2	380 PFC									
AB3	101.6 x 6.4 CHS									
AR1	200 UB 22									
AR2	380 PFC									
ACP1	C15019 AT 1200CTS MAX. 2 ROWS OF BRIDGING, LAP AS NOTED									

AP1 Z15019 AT 1200CTS MAX. 3 ROWS OF BRIDGING, LAP AS NOTED

FOR CONSTRUCTION

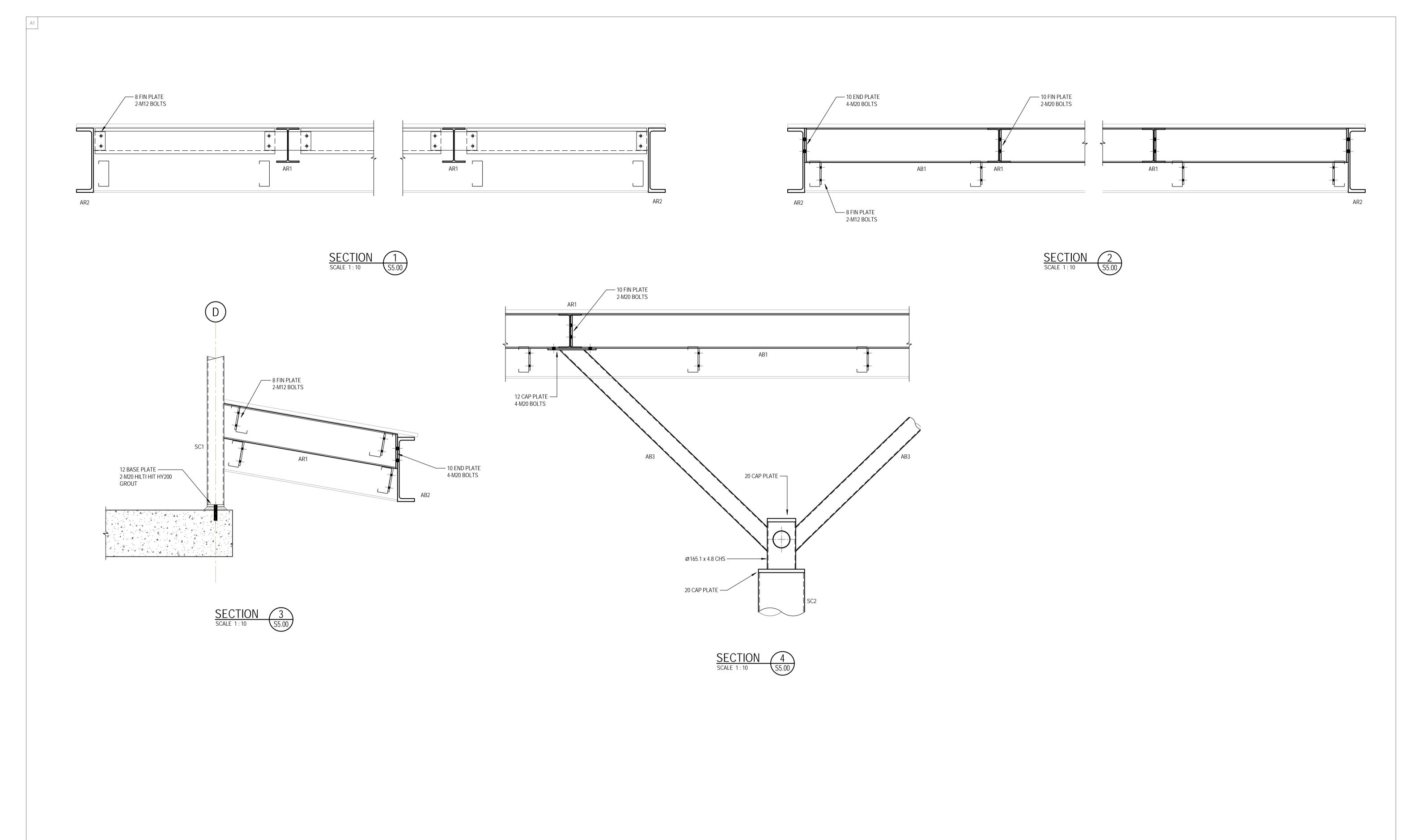


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79 Victoria Avenue
Chatswood NSW 2067

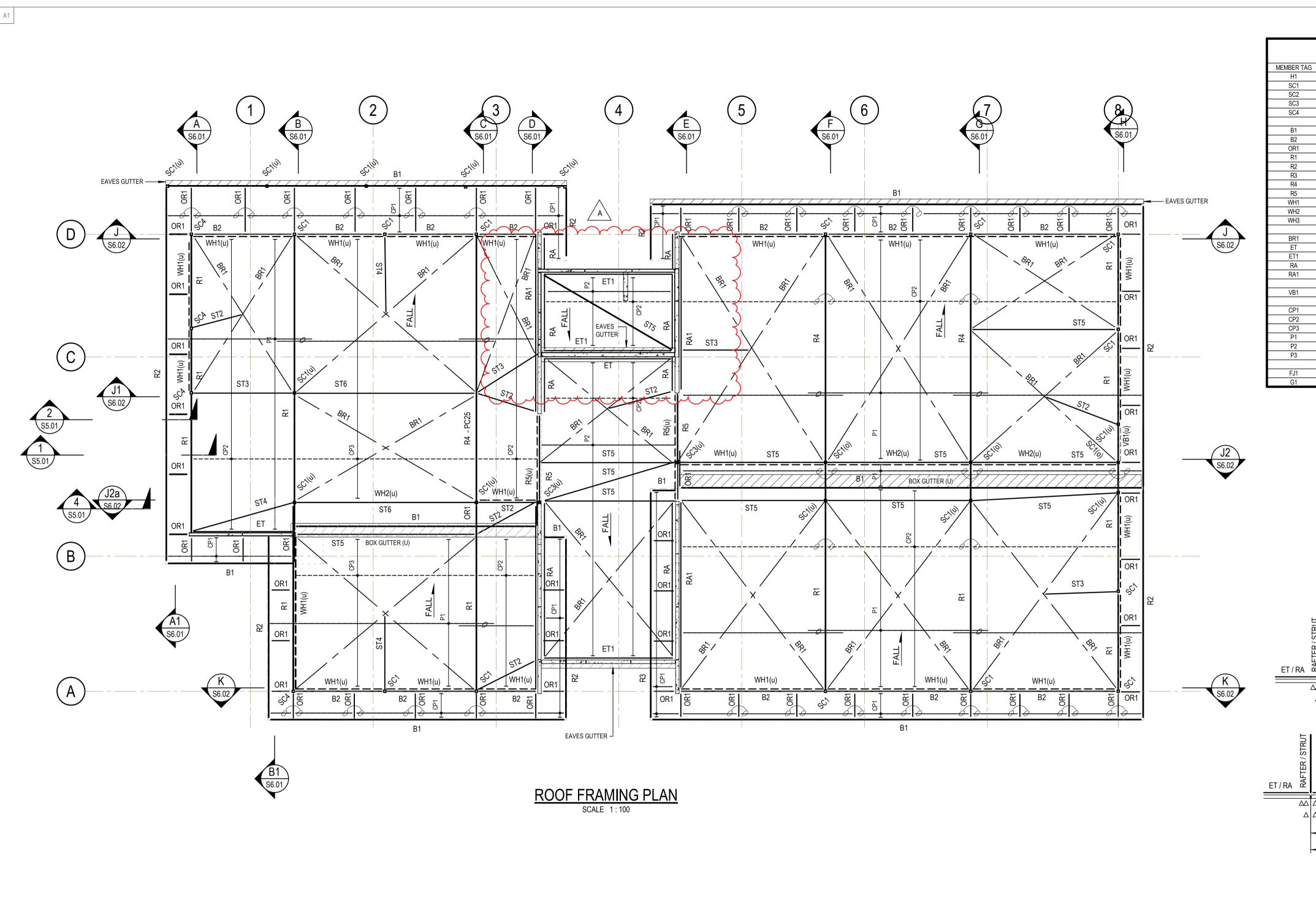
Facsimile
+61 2 9417 8337
Email
email@hhconsult.com.au
Web
www.henryandhymas.com.au

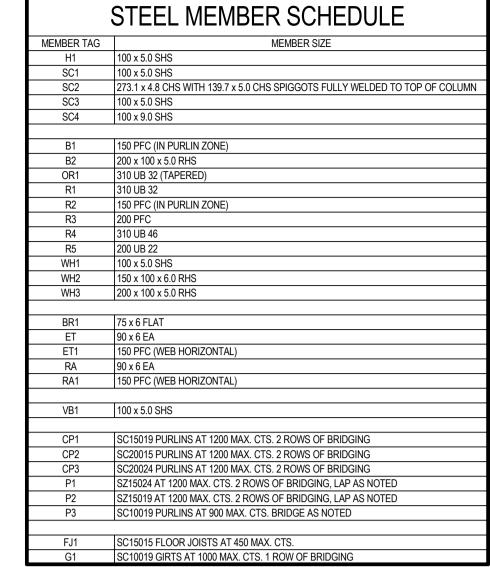


Project	Drawn	Designed	Date	210
NEW LEARNING CENTRE	H.W.	D.M.	DEC. 20)19
6A WATSFORD ROAD, CAMPBELLTOWN	Checked D.M.	Approved R.K.	As indic	ated
Title	Drawing number			Revision
FIRST FLOOR SLAB PLAN	19712-S	5.00		Α



	Client	ARAKIRRI COLLEGE	79 Victoria Avenue	Telephone +61 2 9417 8400	Project NEW LEARNING CENTRE	Drawn H.W.	Designed D.M.	Date 07/29/21
	Archit		Management 10	Facsimile +61 2 9417 8337 Email	6A WATSFORD ROAD, CAMPBELLTOWN	D.M.	R.K.	1:10
A ISSUED FOR CONSTRUCTION	HW D.M. 05-08-2021	TURIC + CO. This drawing and design remains the propery of Henry & Hymas and may not be	Global-Mark.com.au®	email@hhconsult.com.au Web www.henryandhymas.com.au henry&hymas	FIRST FLOOR DETAILS		-S5.01	∆





ET / RA

L ≤ 7000

Equal | Equal | Equal

7000 < L < 12000

TYPICAL ET / RA FIXING DETAIL

ET / RA

△△ △△ − RAFTER

 $\triangle \triangle - STRUT$

STRUTS		
ST1	ø76.1 x 2.3 CHS	(EQUIVALENT SHS = 75 x 2.5)
ST2	ø88.9 x 2.6 CHS	(EQUIVALENT SHS = 75 x 2.5)
ST3	ø101.6 x 2.6 CHS	(EQUIVALENT SHS = 100 x 3.0
ST4	ø114.3 x 3.2 CHS	(EQUIVALENT SHS = 100 x 3.0)
ST5	ø139.7 x 3.0 CHS	(EQUIVALENT SHS = 125 x 4.0
ST6	ø165.1 x 3.0 CHS	(EQUIVALENT SHS = 125 x 4.0)
ST7	ø165.1 x 3.5 CHS	(EQUIVALENT SHS = 125 x 4.0)
ST8	ø168.3 x 4.8 CHS	(EQUIVALENT SHS = 150 x 5.0)
ST9	ø168.3 x 6.4 CHS	(EQUIVALENT SHS = 150 x 5.0)
ST10	ø219.1 x 4.8 CHS	(EQUIVALENT SHS = 200 x 5.0)
ST11	ø219.1 x 6.4 CHS	(EQUIVALENT SHS = 200 x 6.0)
ST12	ø219.1 x 8.2 CHS	(EQUIVALENT SHS = 200 x 6.0)

PURLIN NOTES: 1. UNLESS NOTED OTHERWISE ALL PURLINS SHALL BE LAPPED AS FOLLOWS:-SPAN MINIMUM LAP ≤ 6000 900 > 6000 ≤ 8000 1200 > 8000 ≤ 12000 1500 > 12000 1800 2. PURLINS, GIRTS AND BRIDGING TO BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS SPECIFICATION. 3. UNLESS NOTED OTHERWISE PROVIDE PURLIN / GIRT BRIDGING AS FOLLOWS: PURLIN SIZE MAXIMUM PURLIN SPAN PURLIN SIZE 0 BRIDGES 1 BRIDGES 2 BRIDGES 100 2000 4000 (3000) 6000 (5000) 150 3000 6000 (5000) 9000 (7000) 200 4000 8000 (7000) 12000 (9000) 250 5000 10000 (8000) 15000 (12000) NOTED: FIGURES IN BRACKETS APPLY TO SIMPLY SUPPORTED PURLINS

NOTES:

- FOR ALL GENERAL STEELWORK NOTES REFER CONSTRUCTION NOTES ON DRAWING \$1.00
 REFER PANEL ELEVATION FOR FERRULE LOCATIONS.
- ALL BOLTS FOR FERRULE CONNECTION SHALL BE GRADE 8.8/S WITH 30mm MINIMUM ENGAGEMENT TO FERRULES.
- 4. ALL BOLTS FOR STEEL TO STEEL CONNECTIONS SHALL BE M20 GRADE 8.8/S U.N.O
 5. ALL BRACE MEMBERS SHALL BE TIED TO EVERY SECOND PURLIN CROSSED WITH
- UNISTRUT STRAP TO PREVENR SAG.
- 6. PRECAMBER TO ALL STRUCTURE STEEL RAFTERS, TRUSSES AND PORTALS SHALL BE 5mm FOR EVERY 2000mm OF SPAN UNLESS NOTED OTHERWISE ON PLAN.
- (REFER LEGEND BELOW FOR DESIGNATION)
- 7. UNLESS NOTED OTHERWISE ALL EXTERNAL STRUCTURAL STEEL SHALL BE HOT DIPPED GALVANISED.
- 8. UNLESS NOTED OTHERWISE FLY BRACE RAFTERS EVERY 3rd PURLIN.
- PROVIDE FIRE PROTECTION TO ALL STRUCTURAL STEEL ELEMENTS AS REQUIRED
 (REFER ARCHITECT'S DRAWING FOR FIRE RATING REQUIREMENTS)

<u>LEGEND:</u>

- DENOTES M20 FERRULE INSERTS PLUS PLATE TO ET AND RA MEMBERS (SITE WELDED AFTER STEEL ERECTION - REFER DETAILS)
- DENOTES M20 FERRULE INSERTS MIN. 95mm LONG INSTALLED WITH 400mm LONG CROSS BAR
- ▼ DENOTES M24 FERRULE INSERTS PLUS PLATE TO ET AND RA MEMBERS (SITE WELDED AFTER STEEL
- ERECTION REFER DETAILS)

 ▼ DENOTES M24 FERRULE INSERTS
- DENOTES MOST TRUBOLT

RAFTER - PC**

- DENOTES M20 TRUBOLT
- WELD PLATE (refer detail for location)
- RAFTER PS** DENOTES UPWARD PRESENT OF **mm AT SPLICE

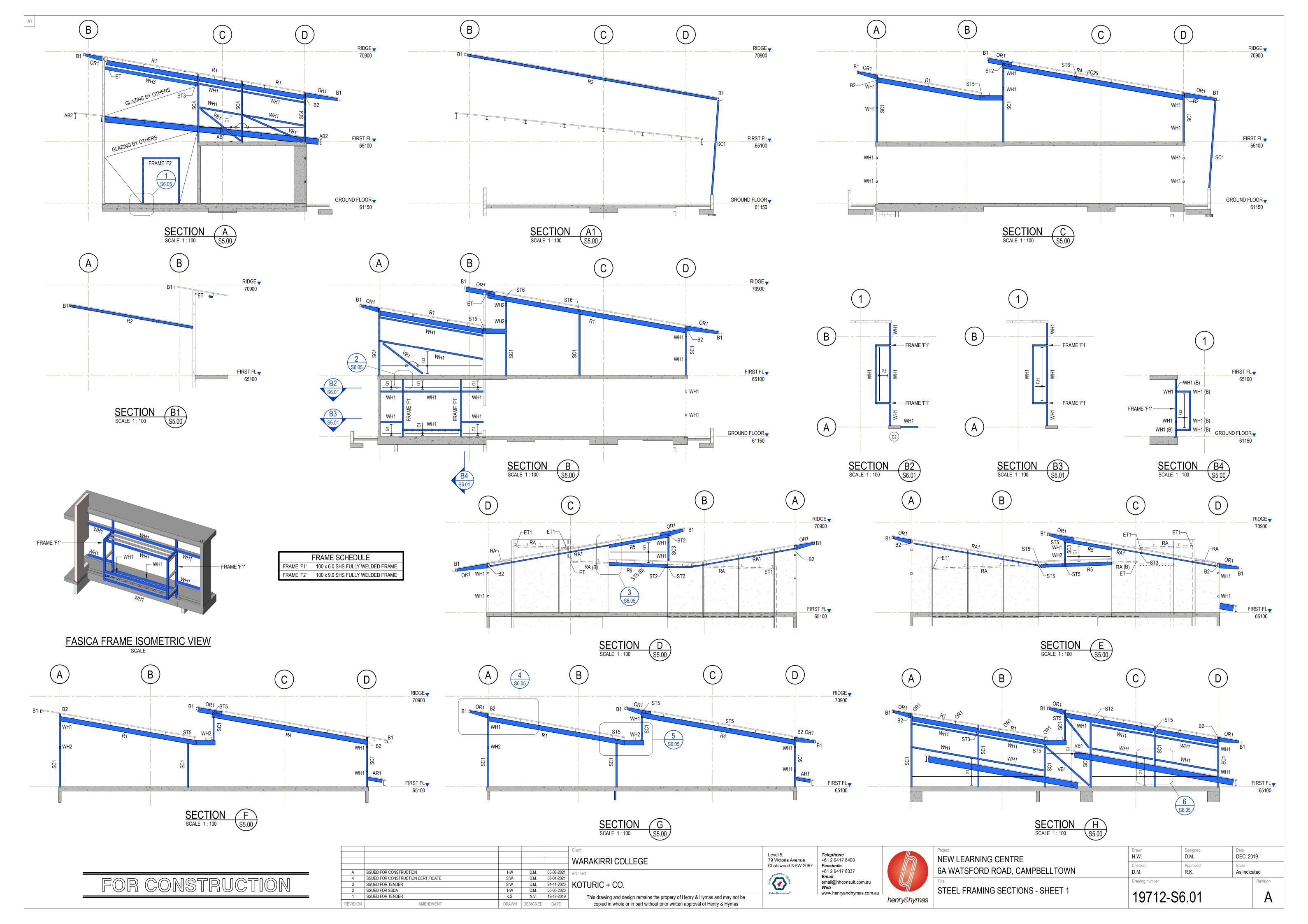
DENOTES UPWARD PRECAMBRE OF **mm AT MIDSPAN

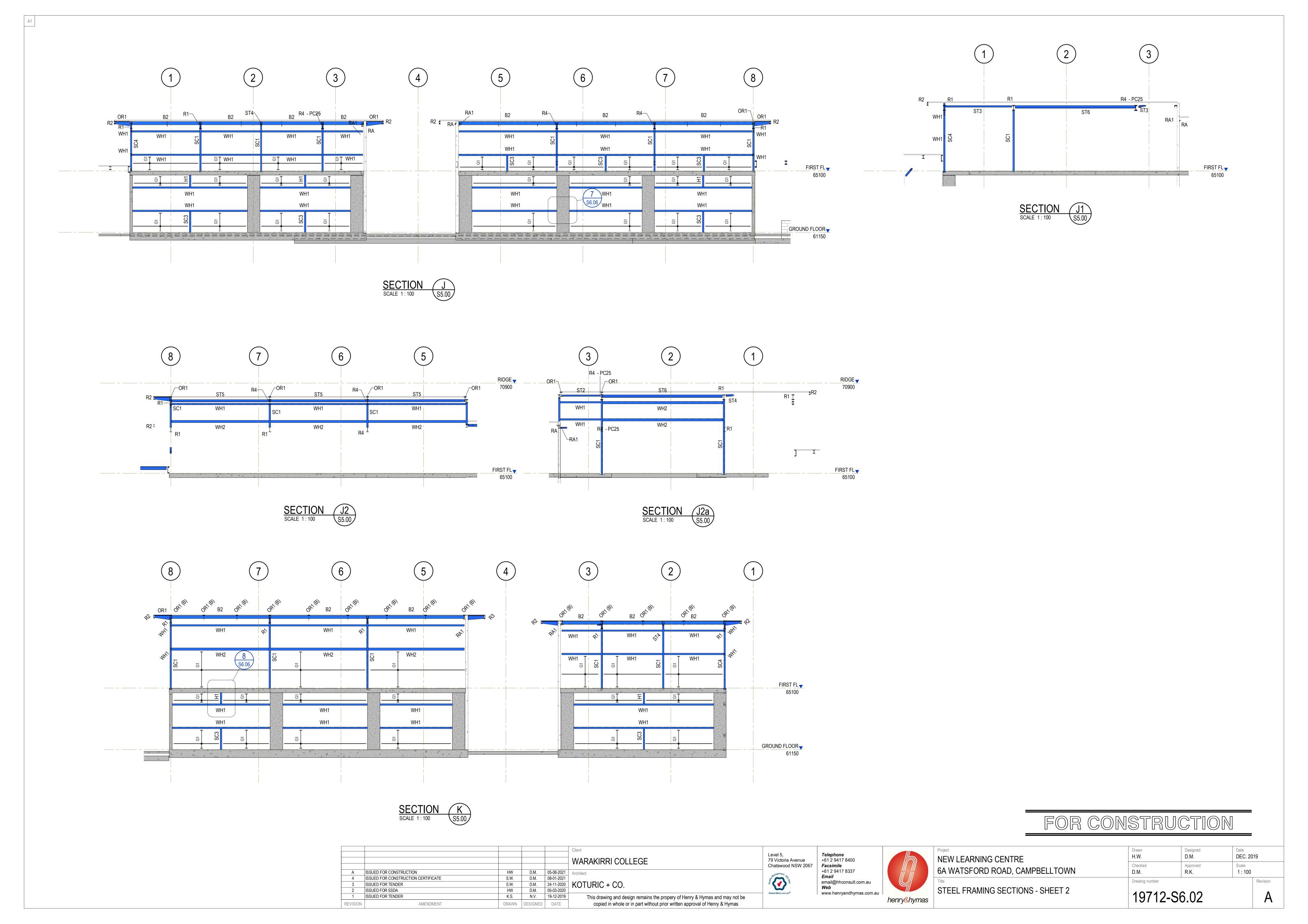
						Client
						WARAKIRRI COLLEGE
	Α	ISSUED FOR CONSTRUCTION	HW	D.M.	05-08-2021	Architect
	4	ISSUED FOR CONSTRUCTION CERTIFICATE	S.W.	D.M.	08-01-2021	
	3	ISSUED FOR TENDER	S.W.	D.M.	24-11-2020	KOTURIC + CO.
\mathcal{X}	2	ISSUED FOR SSDA	HW	D.M.	05-03-2020	
	1	ISSUED FOR TENDER	K.S.	N.V.	19-12-2019	This drawing and design remains the propery of Henry & Hymas and may not be
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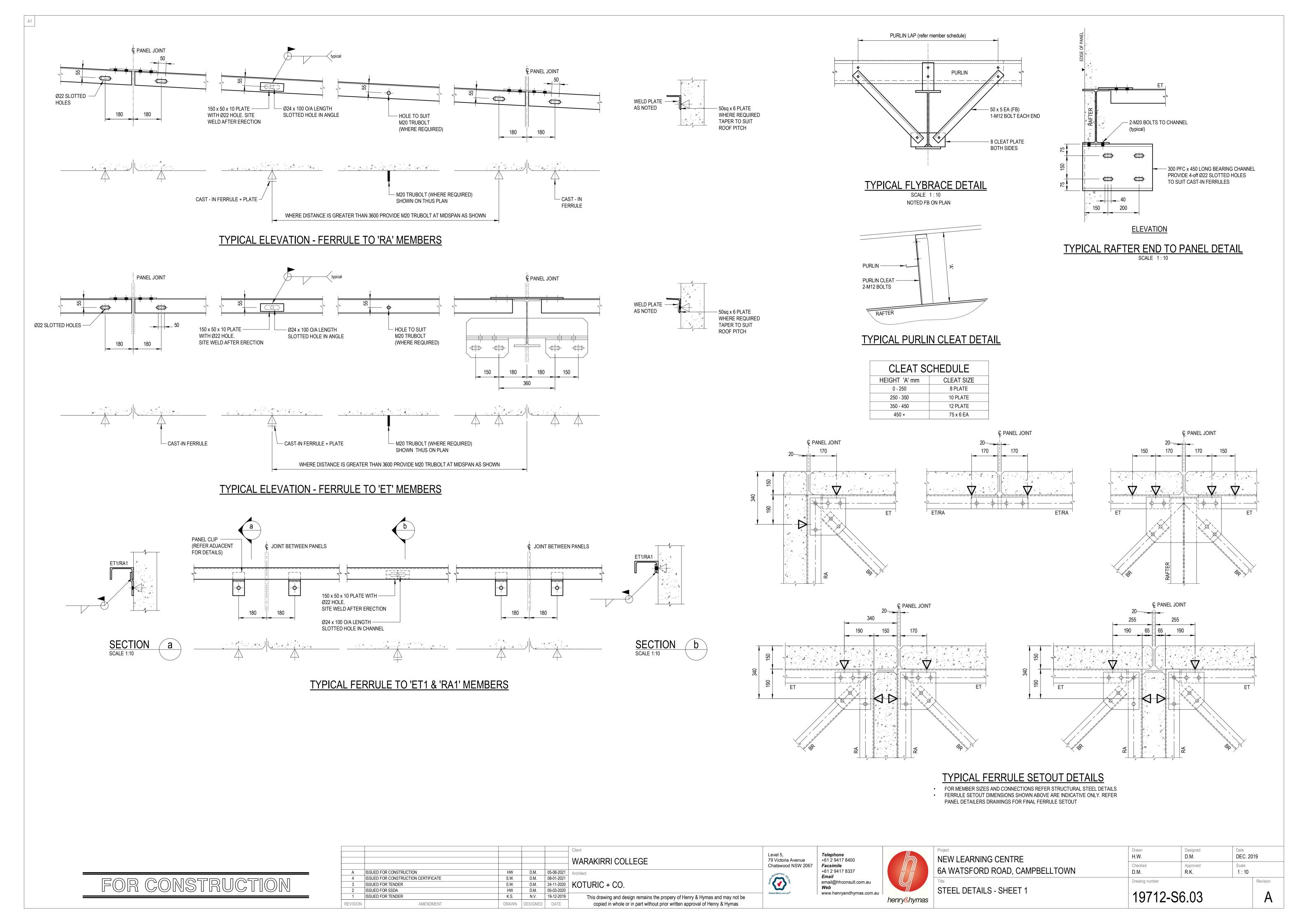


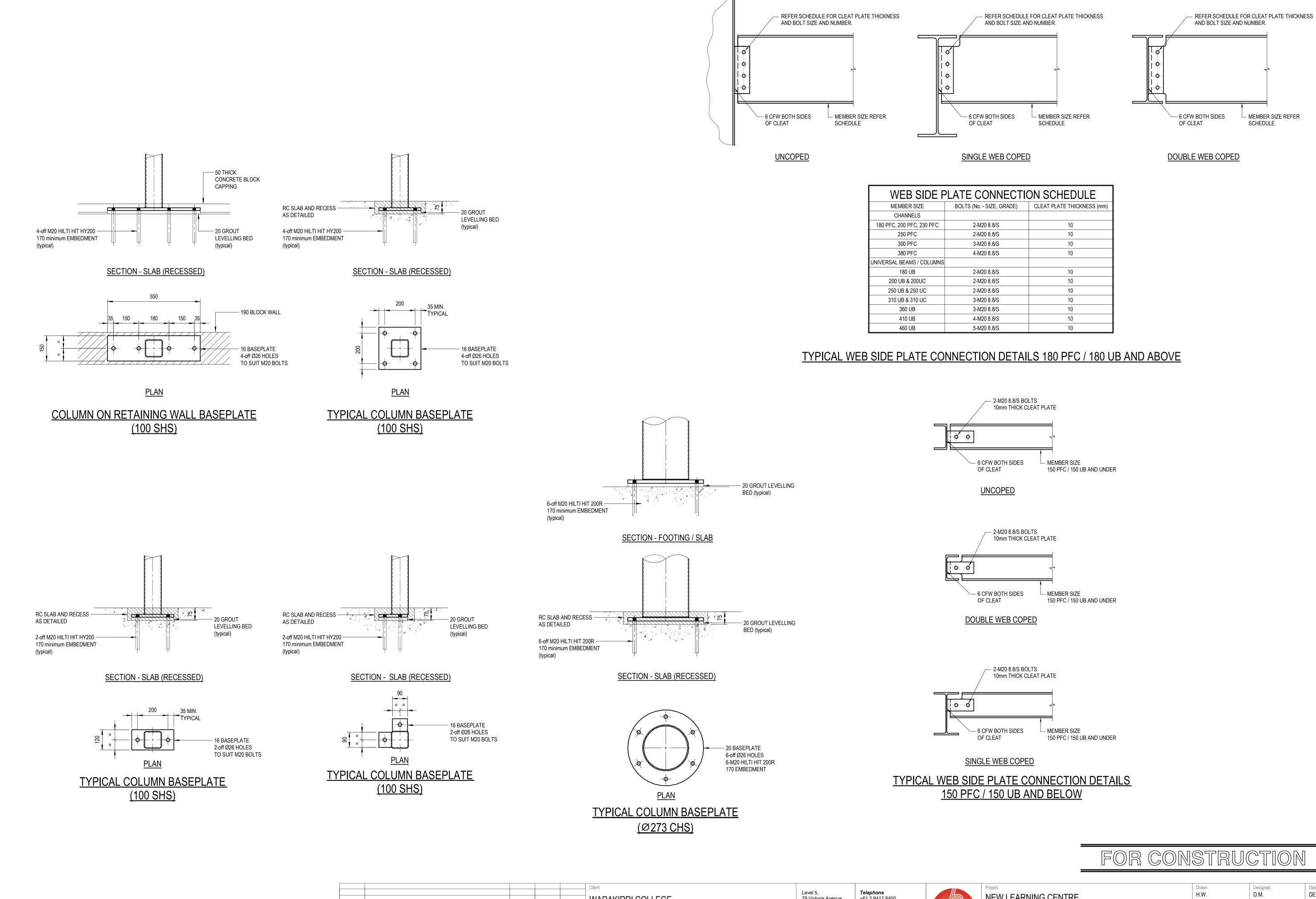


	Project NEW LEARNING CENTRE	Drawn H.W.	Designed D.M.	Date DEC. 20	19
	6A WATSFORD ROAD, CAMPBELLTOWN	Checked D.M.	Approved R.K.	Scale As indica	ated
	Title	Drawing number			Revision
s	ROOF STEEL FRAMING PLAN	19712-S	6.00		Α





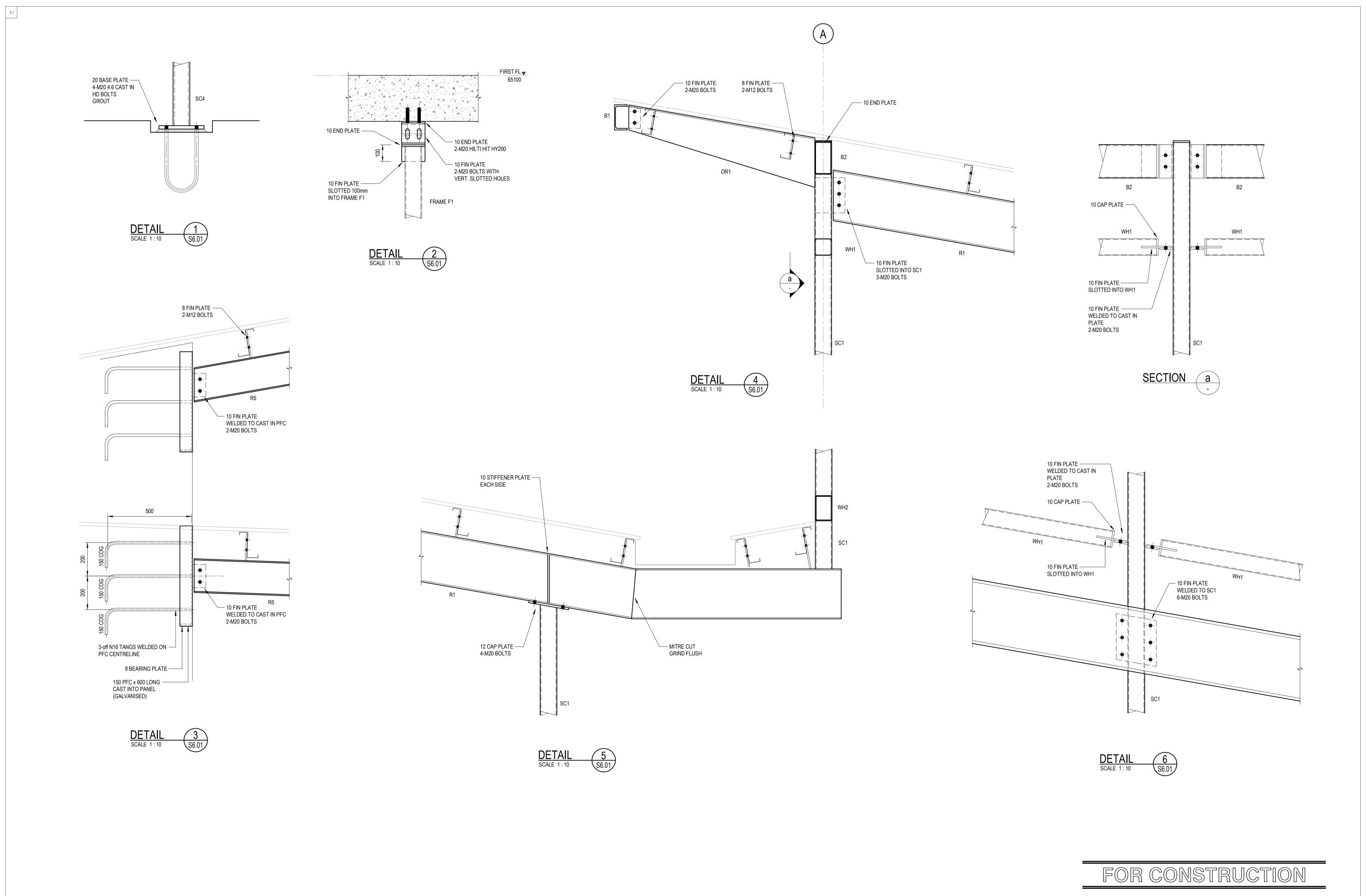




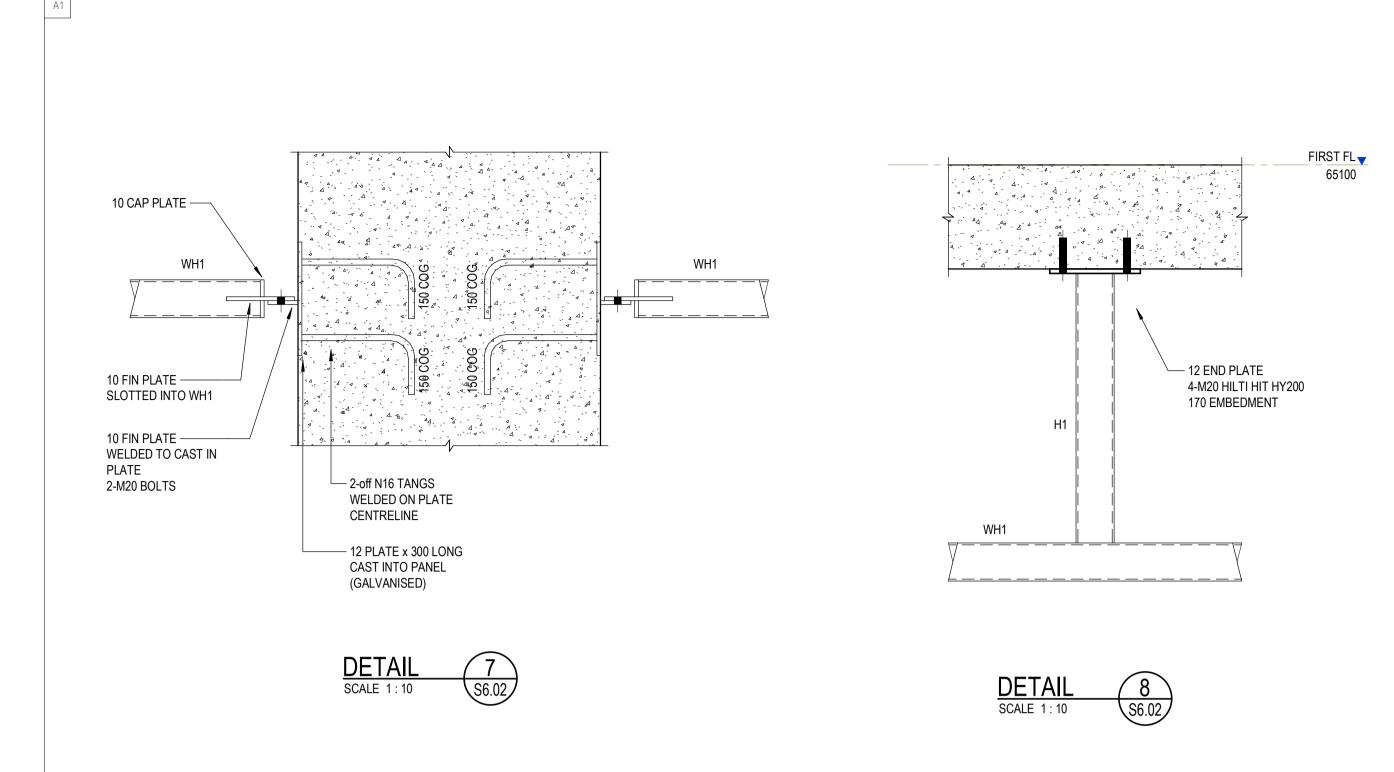
- MEMBER SIZE REFER

SCHEDULE

				WARAKIRRI COLLEGE	Level 5, 79 Victoria Avenue	Telephone +61 2 9417 8400		NEW LEARNING CENTRE	Drawn H.W.	Designed D.M.	Date DEC. 2019
A	ISSUED FOR CONSTRUCTION HW	D.M.	05-08-2021	Architect	Chatswood NSW 2067			6A WATSFORD ROAD, CAMPBELLTOWN	Checked D.M.	Approved R.K.	Scale 1:10
3	ISSUED FOR CONSTRUCTION CERTIFICATE S.W. ISSUED FOR TENDER S.W.	D.M.	08-01-2021 24-11-2020	KOTURIC + CO.	0 9001	email@hhconsult.com.au		Title	Drawing number		Revision
1	ISSUED FOR SSDA HW ISSUED FOR TENDER K.S.	D.M. N.V.	05-03-2020 19-12-2019	This drawing and design remains the propery of Henry & Hymas and may not be	Global-Mark.com.au [⊚]	www.henryandhymas.com.au	hone of hymno	STEEL DETAILS - SHEET 2	19712-	S6 04	Δ
REVISION	AMENDMENT DRAWN	DESIGNED	DATE	copied in whole or in part without prior written approval of Henry & Hymas			henry&hymas		10112	00.0 1	/ \



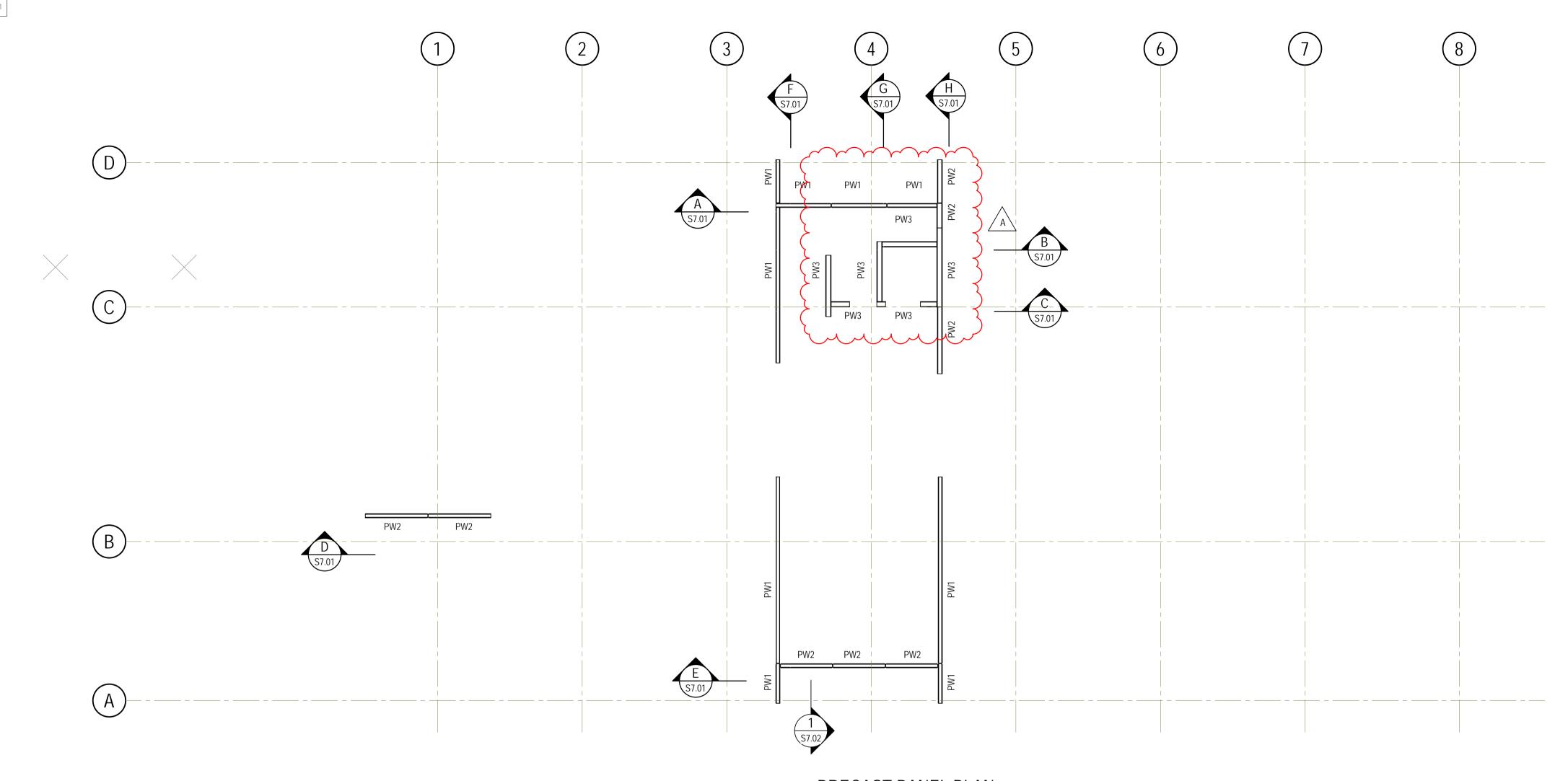
		WARAKIRRI COLLEGE	Level 5, 79 Victoria Avenue	Telephone +61 2 9417 8400		Project NEW LEARNING CENTRE	Drawn H.W.	Designed D.M.	Date 08/01/21
		Architect	Chatswood NSW 2067			6A WATSFORD ROAD, CAMPBELLTOWN	Checked D.M.	Approved R.K.	Scale 1:10
		KOTURIC + CO.	Global-Mark.com.au®	email@hhconsult.com.au Web www.henryandhymas.com.au	W W	STEEL DETAILS - SHEET 3	Drawing number	0.05	Revision
A ISSUED FOR CONSTRUCTION HW REVISION AMENDMENT DRAWN	D.M. 05-08-2021 DESIGNED DATE	This drawing and design remains the propery of Henry & Hymas and may not be copied in whole or in part without prior written approval of Henry & Hymas		www.nomyananymao.oom.aa	henry&hymas		19712-S	6.05	A



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					Architect	Management. 16	+61 2 9417 8337 Email
					KOTURIC + CO.	9001	email@hhconsult.com.au
					1.010110 - 00.	Global-Mark.com.au®	Web www.henryandhymas.com.au
A	ISSUED FOR CONSTRUCTION	HW	D.M.	05-08-2021	This drawing and design remains the propery of Henry & Hymas and may not be		· · · · · · · · · · · · · · · · · · ·
REVISION	AMENDMENT	DRAWN	DESIGNED	DATE	copied in whole or in part without prior written approval of Henry & Hymas		

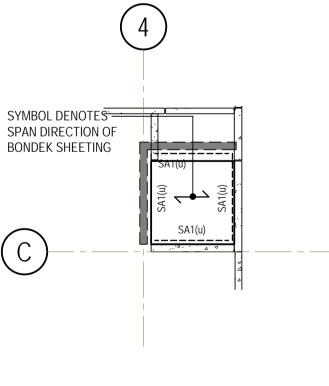


	Project NEW LEARNING CENTRE	Drawn H.W.	Designed D.M.	Date 08/01/21	
	6A WATSFORD ROAD, CAMPBELLTOWN	Checked D.M.	Approved R.K.	Scale 1:10	
	Title	Drawing number			Revision
es.	STEEL DETAILS - SHEET 4	19712-S	6.06		Α



PRECAST PANEL PLAN

PW1 150 PRECAST PANEL, SL82 MESH CENTRAL (fc 32 MPa) PW2 180 PRECAST PANEL, SL82 MESH EACH FACE (fc 32 MPa) PW3 200 PRECAST PANEL, SL92 MESH EACH FACE (fc 32 MPa)



LIFT FALSE ROOF NOTES:

- 1. 180 THICK SLAB ON 1.0 BMT BONDEK SHEETING REINFORCED WITH SL82 MESH TOP WITH 30 COVER
- 2. CONCRETE STRENGTH f 'c = 32 MPa
- 3. PROVIDE 1 ROW OF PROPPING AT MIDSPAN
- 4. SLAB IS SUITABLE TO SUPPORT MAX. S.W.L. OF 4000 KG FOR LIFT INSTALLATION
- 5. LIFTING EYE INSTALLATION & DESIGN BY OTHERS
- 6. ENSURE LIFTING EYES ARE LOCATED IN PAN OF SHEETING ENSURE NO RIBS ARE CUT
- 7. 'SA1' DENOTES 90 x 8 EA. DRILL & EPOXY M16-400 MAX. CTS. HILTI HIT-V ANCHORS WITH HILTI HIT-HY 200. 150 EMBEDMENT

LIFT FALSE ROOF PLAN

FOR CONSTRUCTION

WARAKIRRI COLLEGE
 HW
 D.M.
 05-08-2021
 Architect

 S.W.
 D.M.
 08-01-2021

 S.W.
 D.M.
 24-11-2020

 HW
 D.M.
 05-03-2020

 K.S.
 N.V.
 19-12-2019

 This drawing and des
 ISSUED FOR CONSTRUCTION ISSUED FOR CONSTRUCTION CERTIFICATE ISSUED FOR TENDER 2 ISSUED FOR SSDA This drawing and design remains the propery of Henry & Hymas and may not be copied in whole or in part without prior written approval of Henry & Hymas ISSUED FOR TENDER DRAWN DESIGNED DATE REVISION AMENDMENT

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	Project NEW LEARNING CENTRE	Drawn K.S.	Designed D.M.	Date DEC. 20	19
	6A WATSFORD ROAD, CAMPBELLTOWN	Checked D.M.	Approved R.K.	Scale As indica	ated
	Title	Drawing number			Revision
S	PRECAST PANEL PLAN	19712-S	7.00		Α

PANEL NOTES:

1. INSIDE FACE SHAL BE THE FACE INTERNAL TO THE STRUCTURAL

2. FOR FERRULE AND RESTRAINT DETAILS REFER DRAWING ++++++++

1. THESE ELEVATIONS ARE PROVIDED FOR THE PURPOSES OF PANEL DETAILING ONLY AND SHOW THE MINIMUM STRUCTURAL (INSERVICE) REINFORCEMENT TO BE INCLUDED IN THE PANEL DESIGN. THE REINFORCEMENT SHOWN IS NOT NECESSARILY SUFFICIENT FOR LIFTING. PANELS SHALL BE CONSTRUCTED TO THE PANEL ARRANGEMENTS AND DETAIL SHEET PROVIDED SEPARATELY.

2. UNLESS OTHERWISE NOTED ALL PANELS SHALL BE 150 THICK

3. TWO HARD COPIES OF THE PANEL SHOP DRAWINGS SHALL BE SUBMITTED FOR APPROVAL. THIS APPROVAL SHALL COVER THE

4. THE PRECAST PANEL CONTRACTOR IS RESPONSIBLE FOR THE DESIGN OF ANY ADDITIONAL REINFORCEMENT WHICH MAY BE REQUIRED TO ENSURE LIFTING STRESSES ARE WITHIN THE RELEVENT CODES AND ALL BRACING AND SUPPORTING STRUCTURES (DEADMAN OR FLOOR SLAB) ARE STRUCTURALLY ADEQUATE TO SUPPORT ALL RELEVANT WIND LOADINGS. THE SUB CONTRACTOR SHALL PROVIDE AN ENGINEERS CERTIFICATION (WITH THEIR REGISTRATION NUMBER) FOR ALL THESE ITEMS TO THE BUILDER & FORWARD A COPY TO H & H CERTIFYING THAT THEY COMPLY WITH AS3850, AS3608 AND AS/NZS1170.2

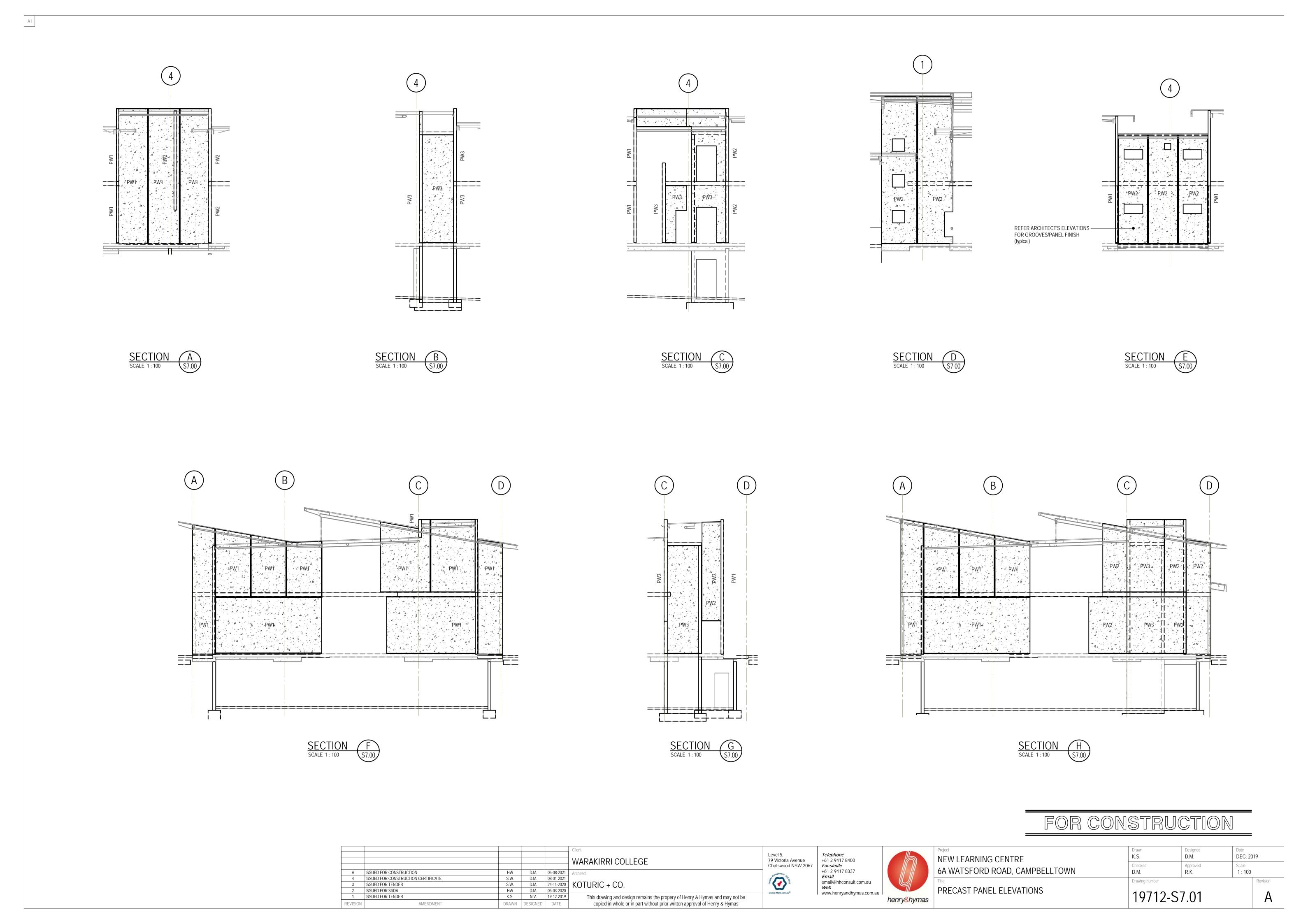
H & H ARE UNABLE TO REVIEW OR SIGN OFF PANEL SHOP DRAWINGS

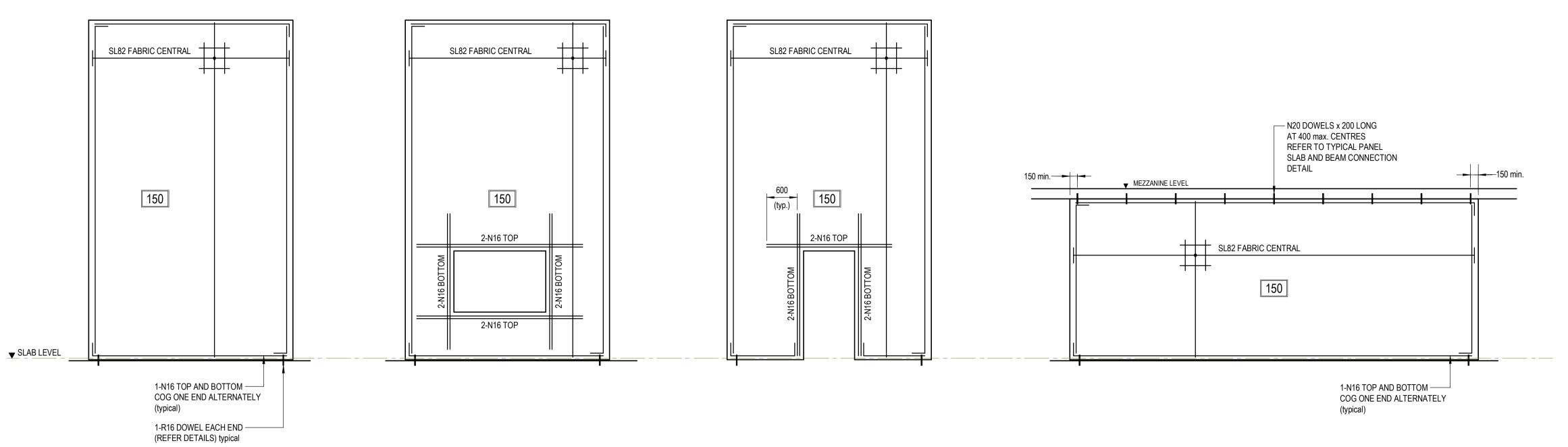
WITH SL82 FABRIC REINFORCEMENT CENTRAL.

UNTIL WE RECEIVE THIS DESIGN CERTIFICATION

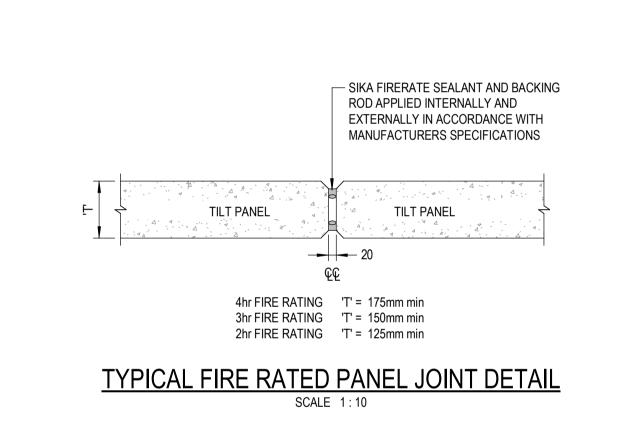
IN-SERVICE LOADING CONDITIONS ONLY.

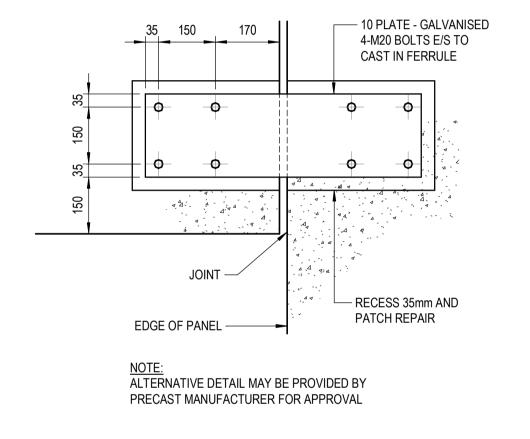
PANEL STRUCTURAL REINFORCEMENT:

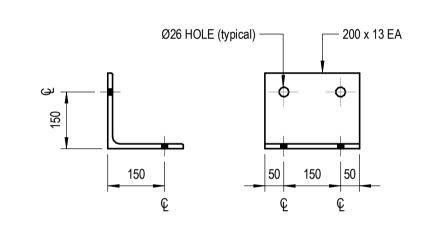




TYPICAL PRECAST PANEL ELEVATIONS







PANEL NOTES:

1. INSIDE FACE SHAL BE THE FACE INTERNAL TO THE STRUCTURAL

1. THESE ELEVATIONS ARE PROVIDED FOR THE PURPOSES OF

PANEL STRUCTURAL REINFORCEMENT:

2. FOR FERRULE AND RESTRAINT DETAILS REFER DRAWING +++++-++

PANEL DETAILING ONLY AND SHOW THE MINIMUM STRUCTURAL

THE REINFORCEMENT SHOWN IS NOT NECESSARILY SUFFICIENT

FOR LIFTING. PANELS SHALL BE CONSTRUCTED TO THE PANEL

ARRANGEMENTS AND DETAIL SHEET PROVIDED SEPARATELY.

2. UNLESS OTHERWISE NOTED ALL PANELS SHALL BE 150 THICK

3. TWO HARD COPIES OF THE PANEL SHOP DRAWINGS SHALL BE

SUBMITTED FOR APPROVAL. THIS APPROVAL SHALL COVER THE

4. THE TILT-UP PANEL CONTRACTOR IS RESPONSIBLE FOR THE DESIGN

OF ANY ADDITIONAL REINFORCEMENT WHICH MAY BE REQUIRED TO ENSURE LIFTING STRESSES ARE WITHIN THE RELEVENT CODES AND ALL BRACING AND SUPPORTING STRUCTURES (DEADMAN OR FLOOR SLAB) ARE STRUCTURALLY ADEQUATE TO SUPPORT ALL RELEVANT

WIND LOADINGS. THE SUB CONTRACTOR SHALL PROVIDE AN ENGINEERS CERTIFICATION (WITH THEIR REGISTRATION NUMBER) FOR ALL THESE

ITEMS TO THE BUILDER & FORWARD A COPY TO H & H CERTIFYING THAT THEY COMPLY WITH AS3850, AS3608 AND AS/NZS1170.2

UNTIL WE RECEIVE THIS DESIGN CERTIFICATION

H & H ARE UNABLE TO REVIEW OR SIGN OFF PANEL SHOP DRAWINGS

WITH SL82 FABRIC REINFORCEMENT CENTRAL.

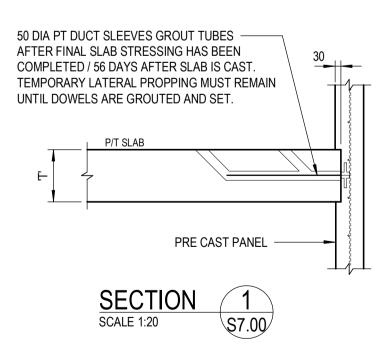
IN-SERVICE LOADING CONDITIONS ONLY.

(INSERVICE) REINFORCEMENT TO BE INCLUDED IN THE PANEL DESIGN.

TYPICAL TIED PLATE DETAIL

SCALE 1:10

TYPICAL ERECTION ANGLE DETAIL



TYPICAL PANEL DOWEL DETAIL

PANEL REINFORCEMENT

- GROUT INJECTION TUBE

EMBEDON 636 PREMIXED

DRILL AND EPOXY 100mm

NON-SHRINK GROUT OR

EQUIVALENT

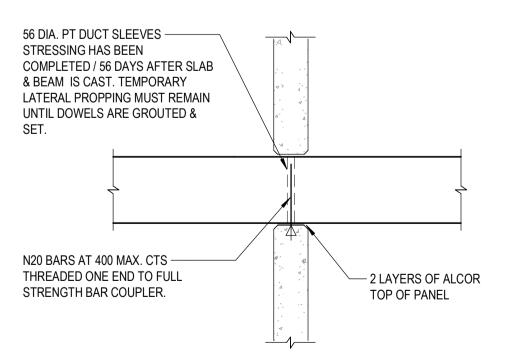
- GROUT FILL BLOCKOUT USING

- Ø16 MS ROUND DOWEL x 200 LONG

INTO PANEL BEARING MATERIAL

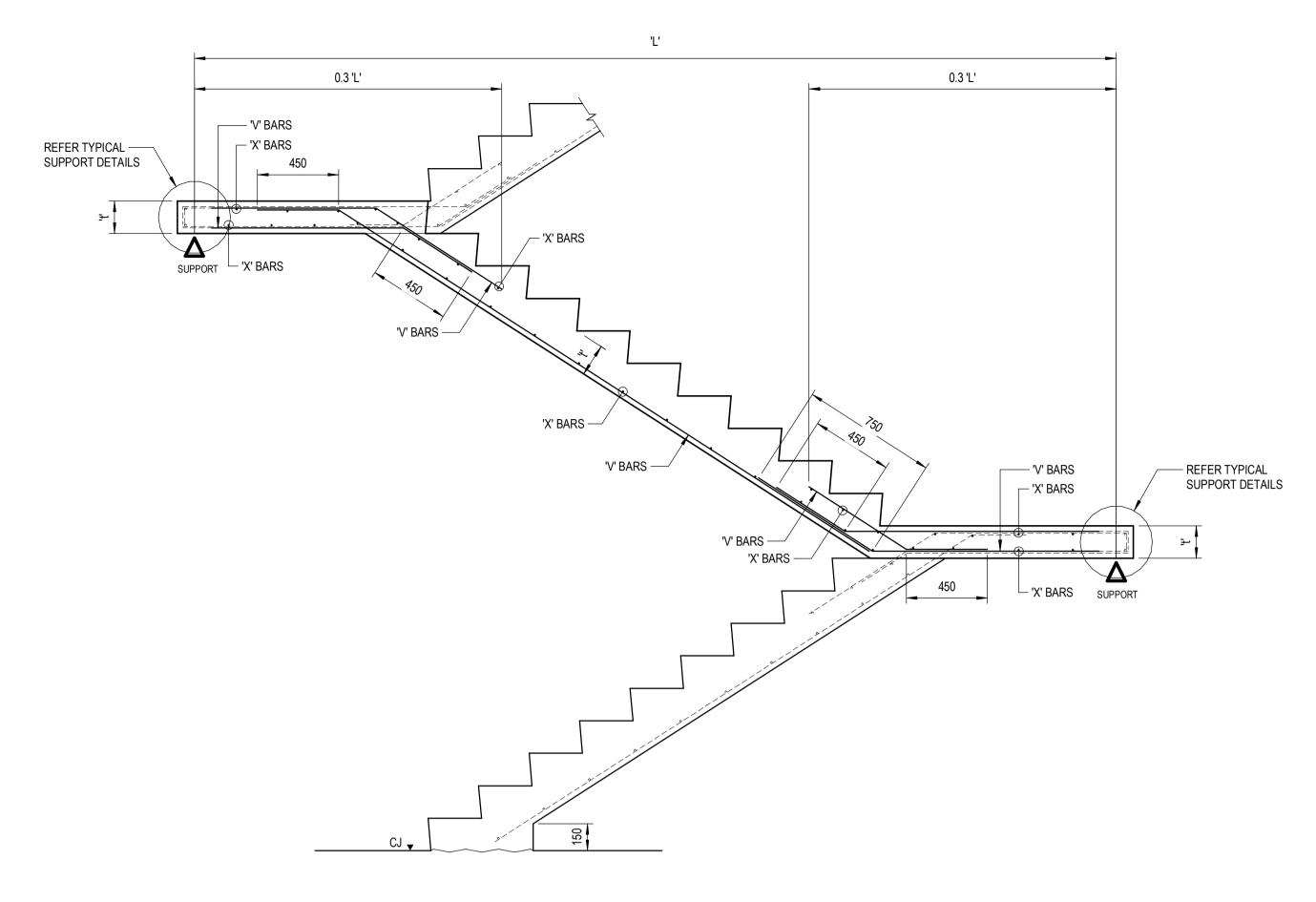
AS DETAILED

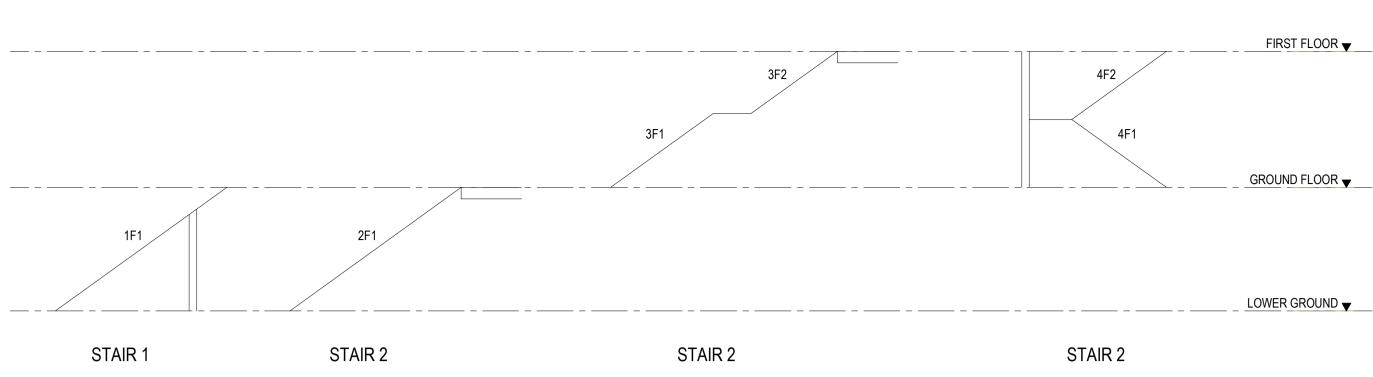
DOWEL BLOCKOUT -



TYPICAL PANEL / SLAB & BEAM CONNECTION DETAIL

				Client	Level 5,	Telephone		NEW LEARNING CENTRE	Drawn H.W.	Designed D.M.	Date DEC. 2019
				WARAKIRRI COLLEGE	79 Victoria Avenue Chatswood NSW 2067	+61 2 9417 8400 Facsimile			Checked	Approved	Scale
A ISSUED FOR CONSTRUCTION	HW	D.M.	05-08-2021	Architect	anagement	+61 2 9417 8337	<i> </i>	6A WATSFORD ROAD, CAMPBELLTOWN	D.M.	R.K.	As indicated
4 ISSUED FOR CONSTRUCTION CERTIFICATE	S.W.	D.M.	08-01-2021	1/07/17/0	The so	Email email@hhconsult.com.au		Title	Drawing number		Revision
3 ISSUED FOR TENDER	S.W.	D.M.	24-11-2020	KOTURIC + CO.	ā (V) 2	Web			2.49		
2 ISSUED FOR SSDA	HW	D.M.	05-03-2020		Global-Mark.com.au®	www.henrvandhvmas.com.au		TYPICAL PRE-CAST PANELS DETAILS	40740	07.00	
1 ISSUED FOR TENDER	K.S.	N.V.	19-12-2019	This drawing and design remains the propery of Henry & Hymas and may not be	Siobal-mark.com.au	www.nemyandnymas.com.au	book to by the		19712.	-S7 02	<i>[</i>
REVISION AMENDMENT	DRAWN	DESIGNED	DATE	copied in whole or in part without prior written approval of Henry & Hymas			henry&hymas		10112	-57.02	



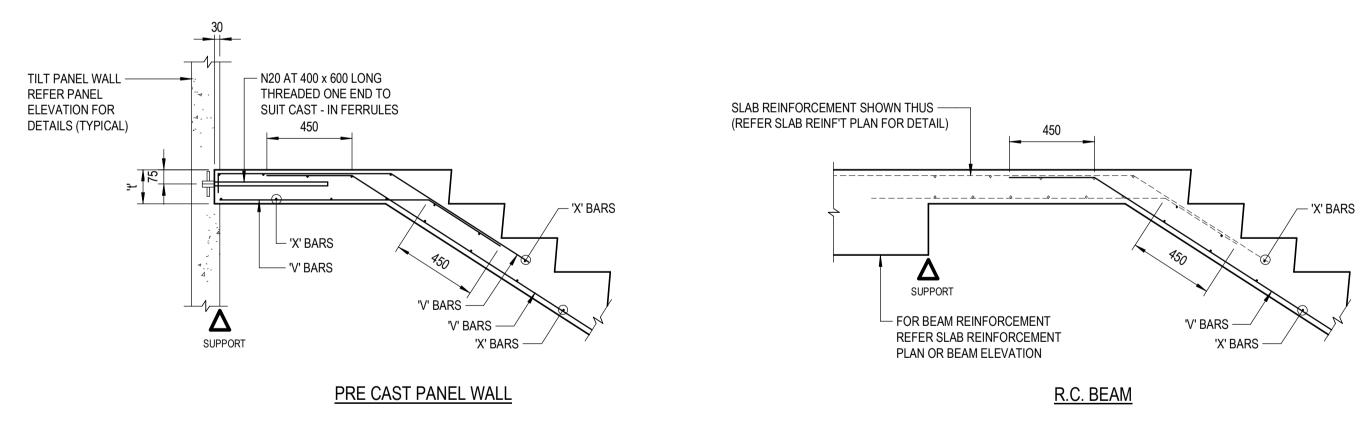


STAIR KEY ELEVATION SCALE 1:100

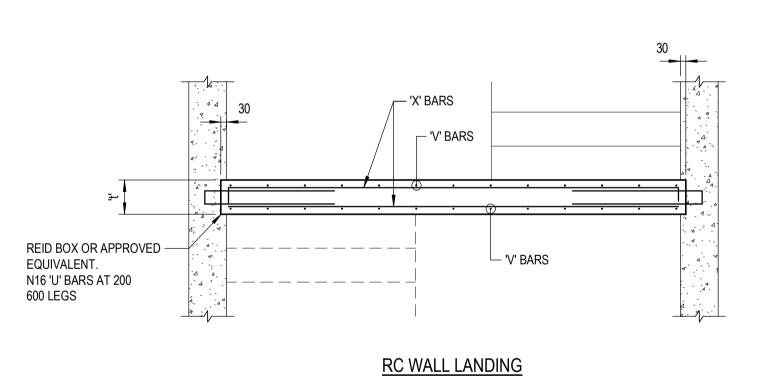
STAIR No. SCHEDULE						
FLIGHT No.	THROAT THICKNESS	V BARS	X BARS			
1F1	180	N16-150	N12-300			
2F1	180	N16- 200	N12-300			
3F1	200	N20-175	N12-300			
3F2	200	N20-175	N12-300			
4F1	180	N16-150	N12-300			
4F2	200	N20-170	N12-300			

TYPICAL SECTION THROUGH STAIR FLIGHT

FOR REINFORCEMENT DESIGNATIONS i.e. 'X' or 'V' BARS REFER STAIR SCHEDULES (TYPICAL)



/--- 'V' BARS 'V' BARS ----'X' BARS -└─ 'V' BARS L---- 'X' BARS 'V' BARS — 'V' BARS — 'X' BARS —



TYPICAL STAIR SUPPORT DETAILS

TYPICAL SECTION THROUGH MID-LANDING FOR REINFORCEMENT DESIGNATIONS i.e. 'X' or 'V' BARS REFER STAIR SCHEDULES (TYPICAL)

STAIR LANDING TYP DETAIL

SCALE 1:20

FOR CONSTRUCTION

					Client	Laural 5	Τ
					WARAKIRRI COLLEGE	Level 5, 79 Victoria Avenue Chatswood NSW 2067	
	A ISSUED FOR CONSTRUCTION	HW	D.M.	05-08-2021	Architect	agemen	
,	4 ISSUED FOR CONSTRUCTION CERTIFICATE	S.W.	D.M.	08-01-2021	, as most	In Manager To	
;	3 ISSUED FOR TENDER	S.W.	D.M.	24-11-2020	KOTURIC + CO.	001	
	2 ISSUED FOR SSDA	HW	D.M.	05-03-2020	TO FORM FOR	Global-Mark.com.au®	
	1 ISSUED FOR TENDER	K.S.	N.V.	19-12-2019	This drawing and design remains the propery of Henry & Hymas and may not be	Giodai-Mark.com.au®	'
REVI	ISION AMENDMENT	DRAWN	DESIGNED	DATE	copied in whole or in part without prior written approval of Henry & Hymas		

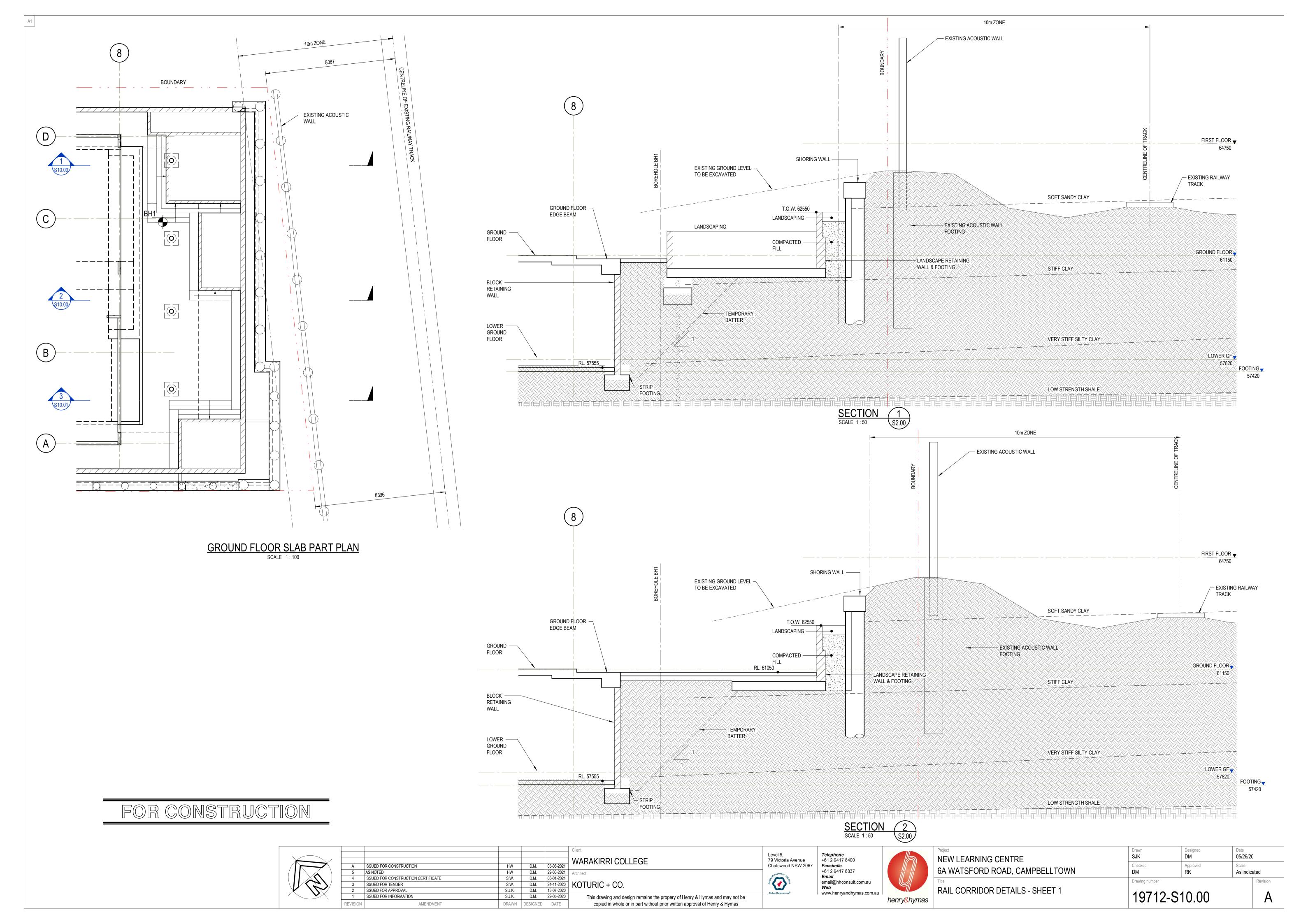
email@hhconsult.com.au www.henryandhymas.com.au

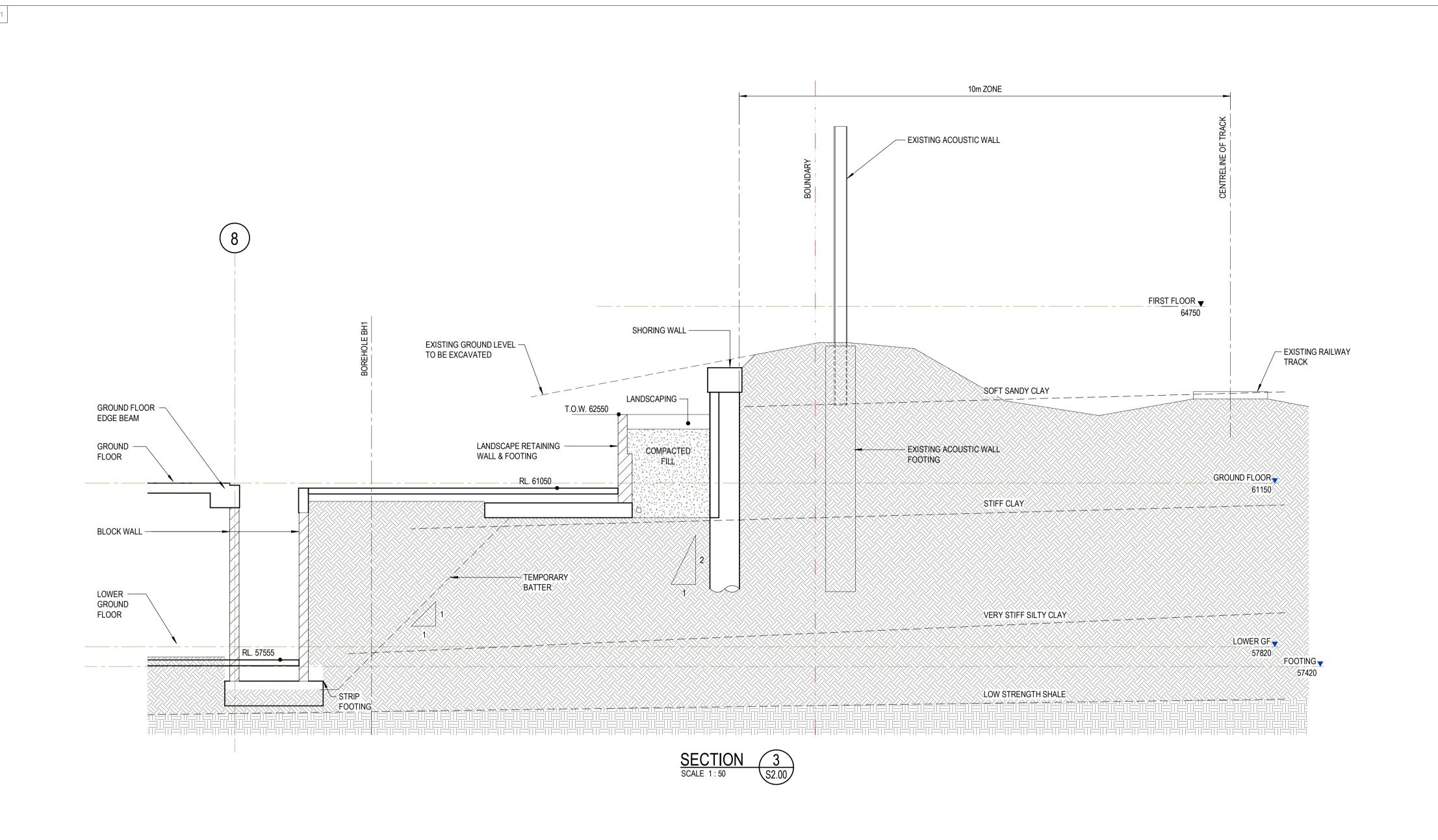
Telephone +61 2 9417 8400

+61 2 9417 8337



Project	Drawn H.W.	Designed D.M.	Date DEC, 20	10
NEW LEARNING CENTRE 6A WATSFORD ROAD, CAMPBELLTOWN	Checked D.M.	Approved R.K.	Scale As indica	
Title	Drawing number			Revision
STAIR DETAILS	19712-S	8.00		Α





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А	ISSUED FOR CONSTRUCTION	HW	D.M.	05-08-2021	ĺ
5	AS NOTED	HW	D.M.	29-03-2021	/
4	ISSUED FOR CONSTRUCTION CERTIFICATE	S.W.	D.M.	08-01-2021	ĺ
3	ISSUED FOR TENDER	S.W.	D.M.	24-11-2020	
2	ISSUED FOR APPROVAL	S.J.K.	D.M.	13-07-2020	L.
1	ISSUED FOR INFORMATION	S.J.K.	D.M.	29-05-2020	ĺ
REVISION	AMENDMENT	DRAWN	DESIGNED	DATE	ı

WARAKIRRI COLLEGE

Architect

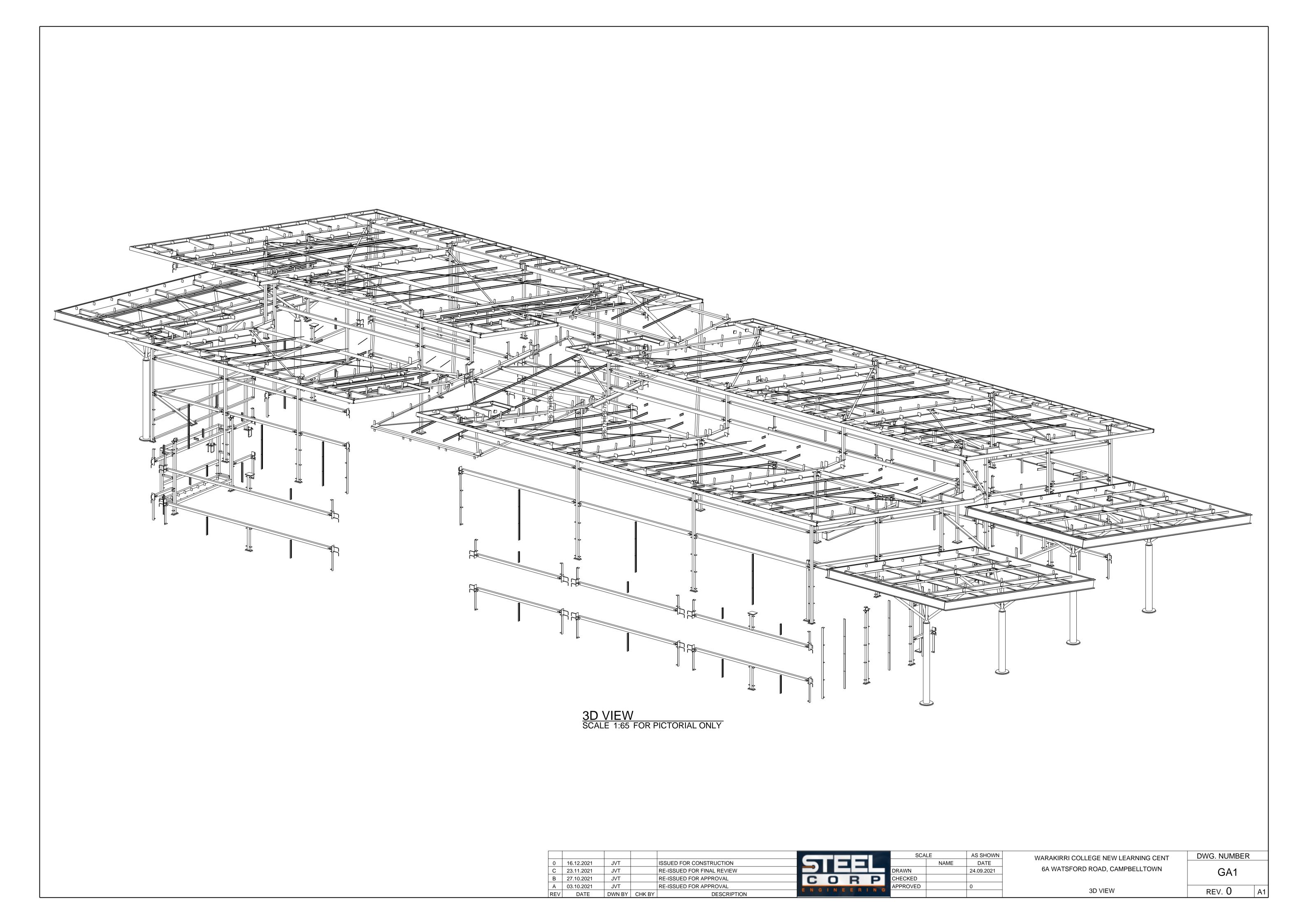
WOTURIC + CO.

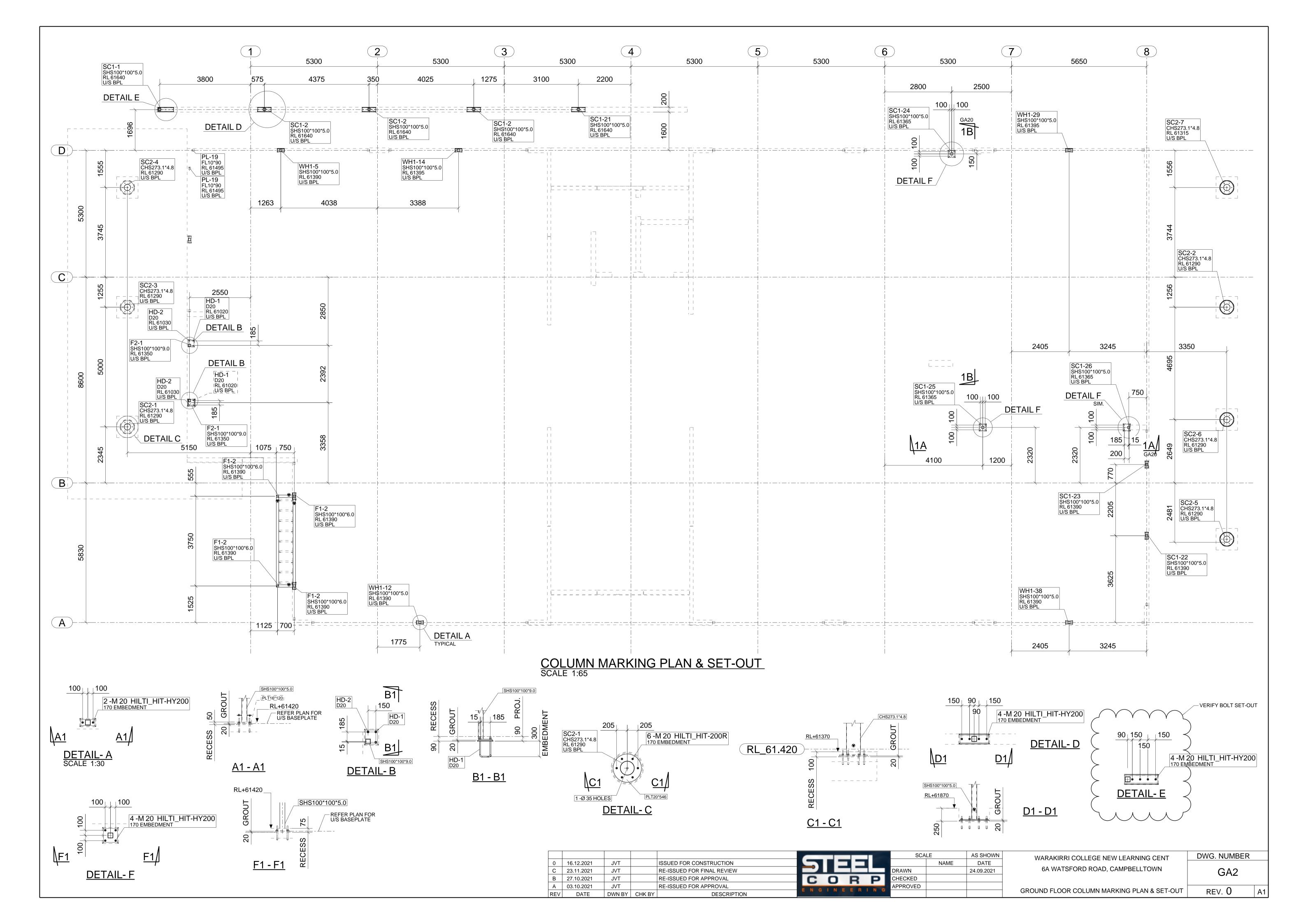
This drawing and design remains the propery of Henry & Hymas and may not be copied in whole or in part without prior written approval of Henry & Hymas

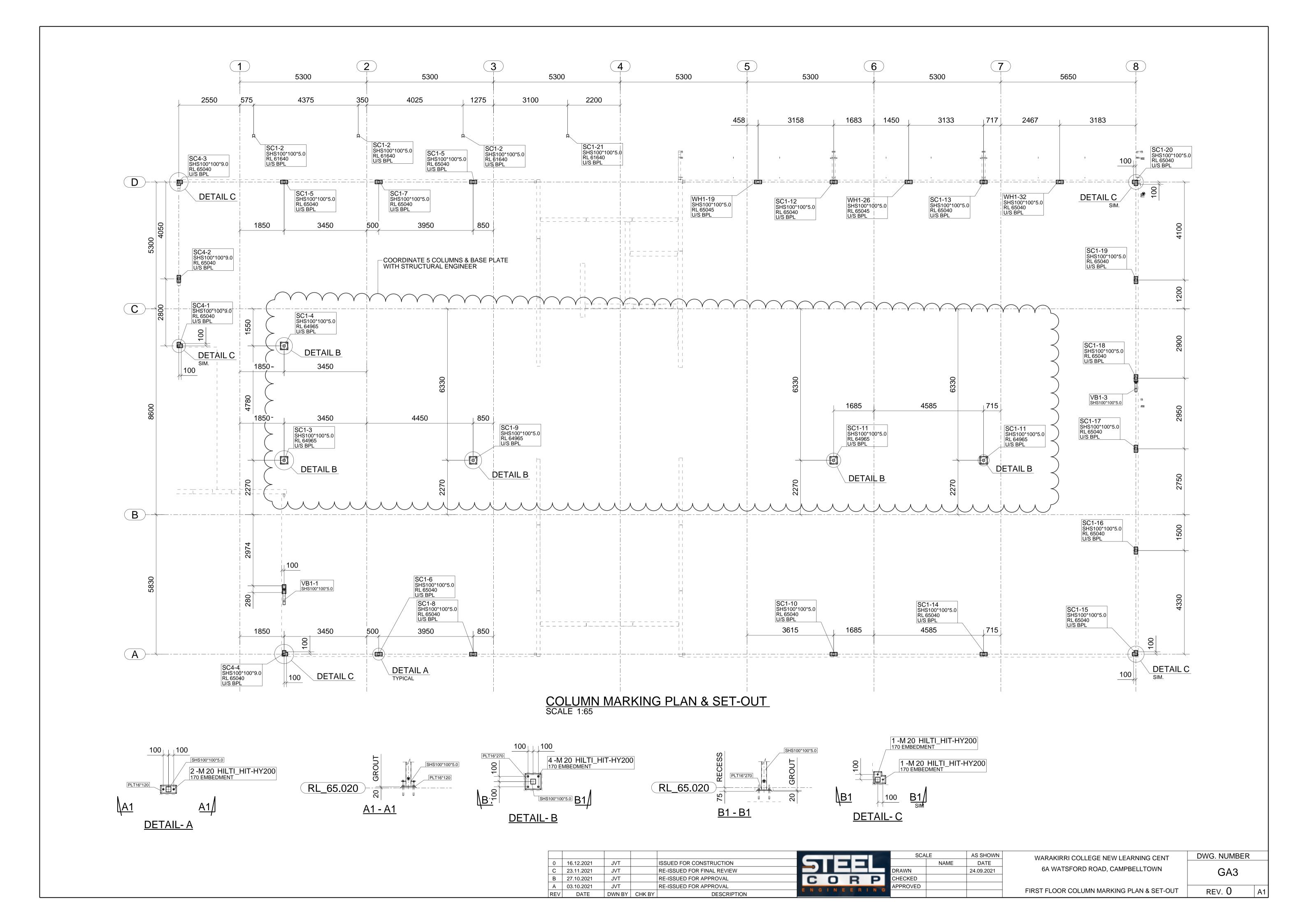


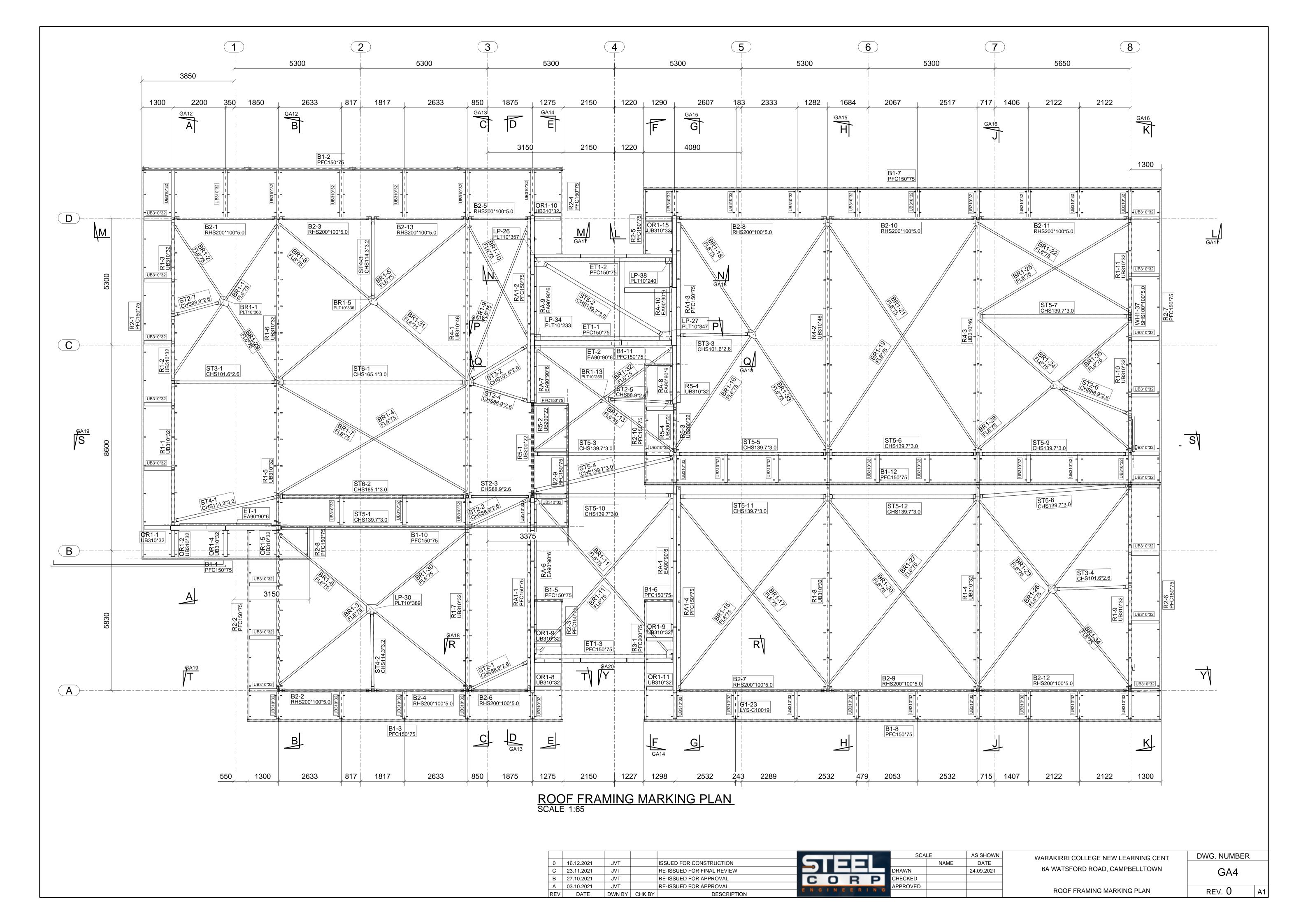
henry&hymas	

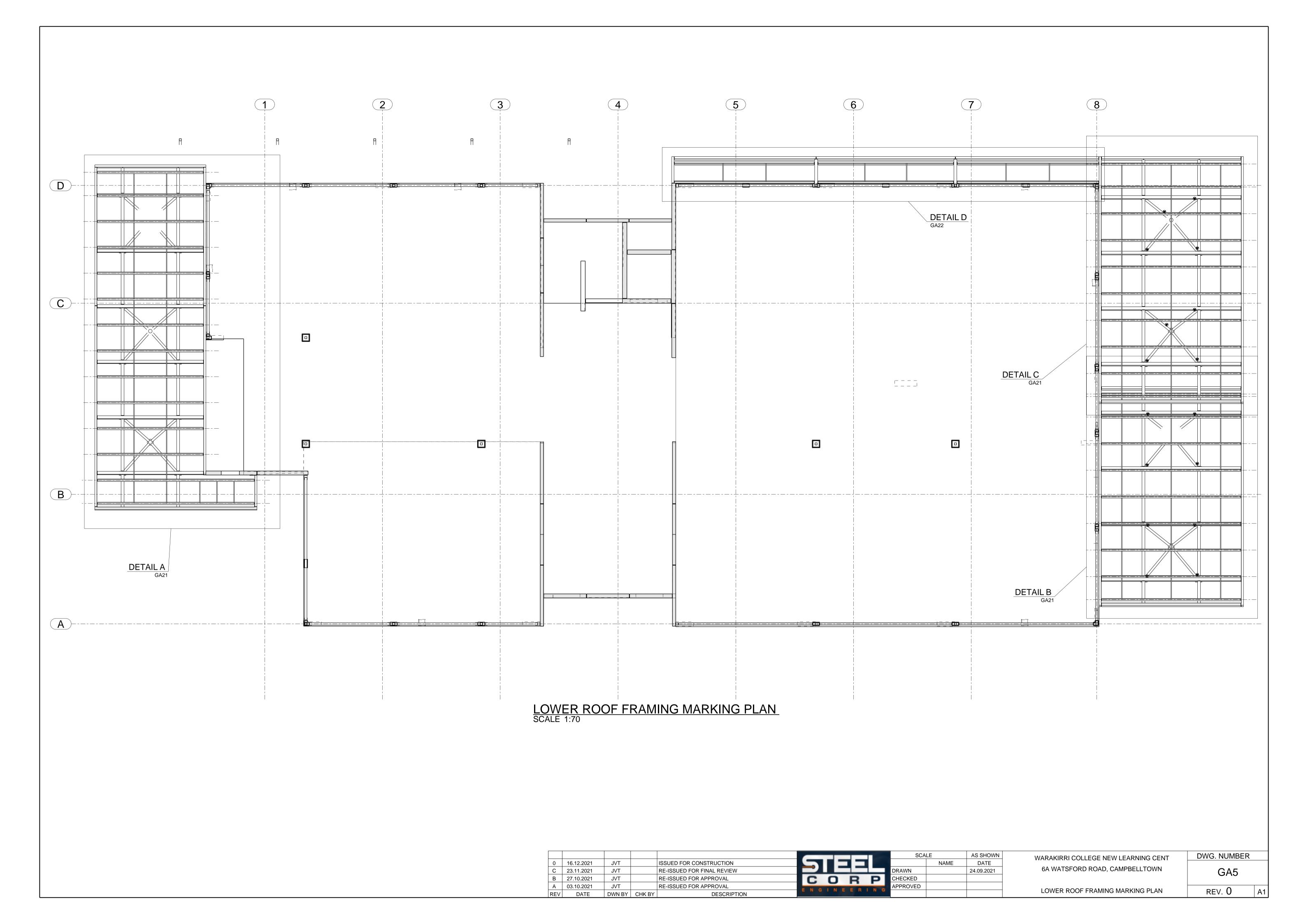
5	RAIL CORRIDOR DETAILS - SHEET 2	19712-S	10.01		Α
	Title	Drawing number			Revision
	6A WATSFORD ROAD, CAMPBELLTOWN	Checked DM	Approved RK	Scale 1:50	
	NEW LEARNING CENTRE	Drawn SJK	Designed DM	Date 05/27/20)

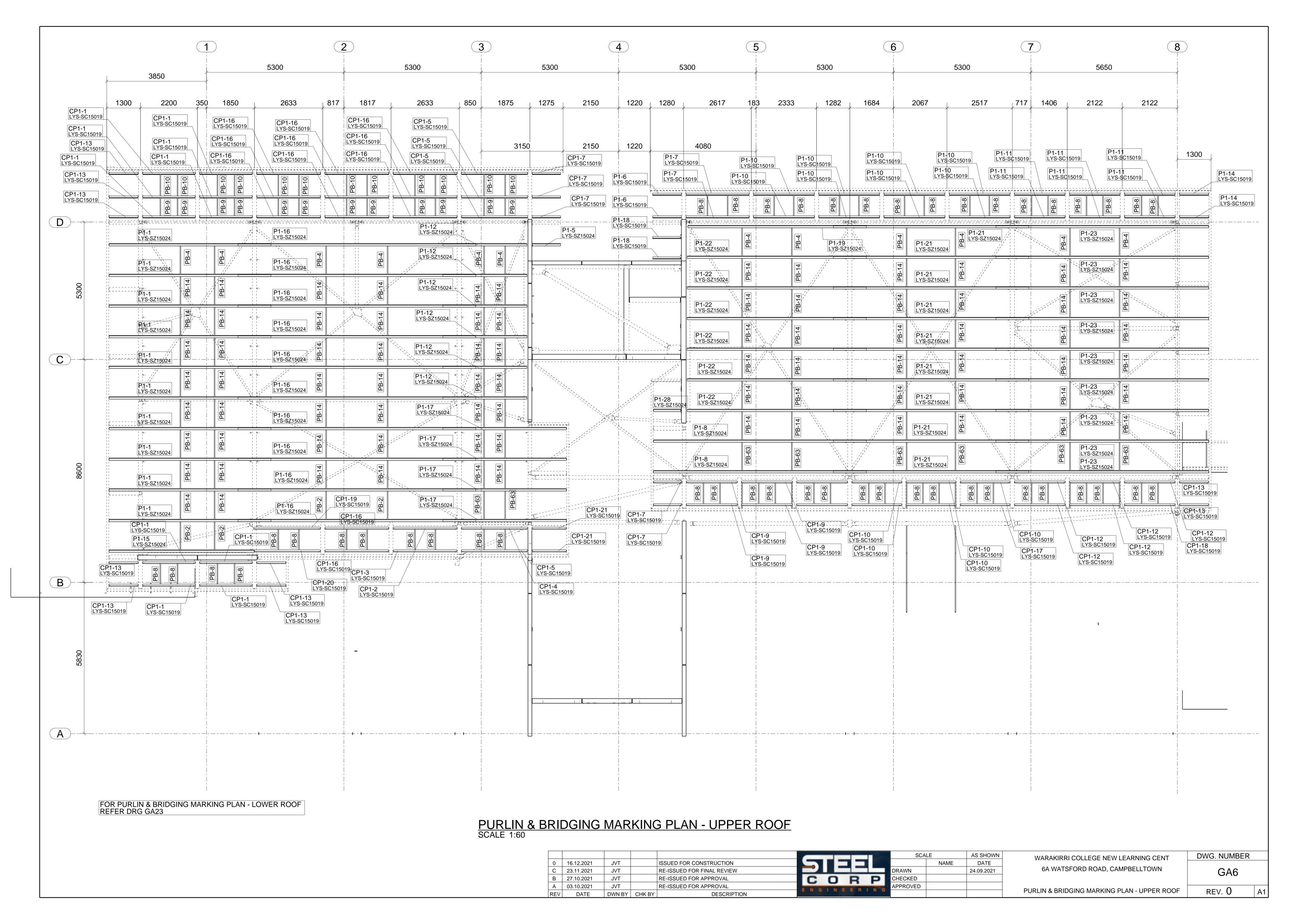


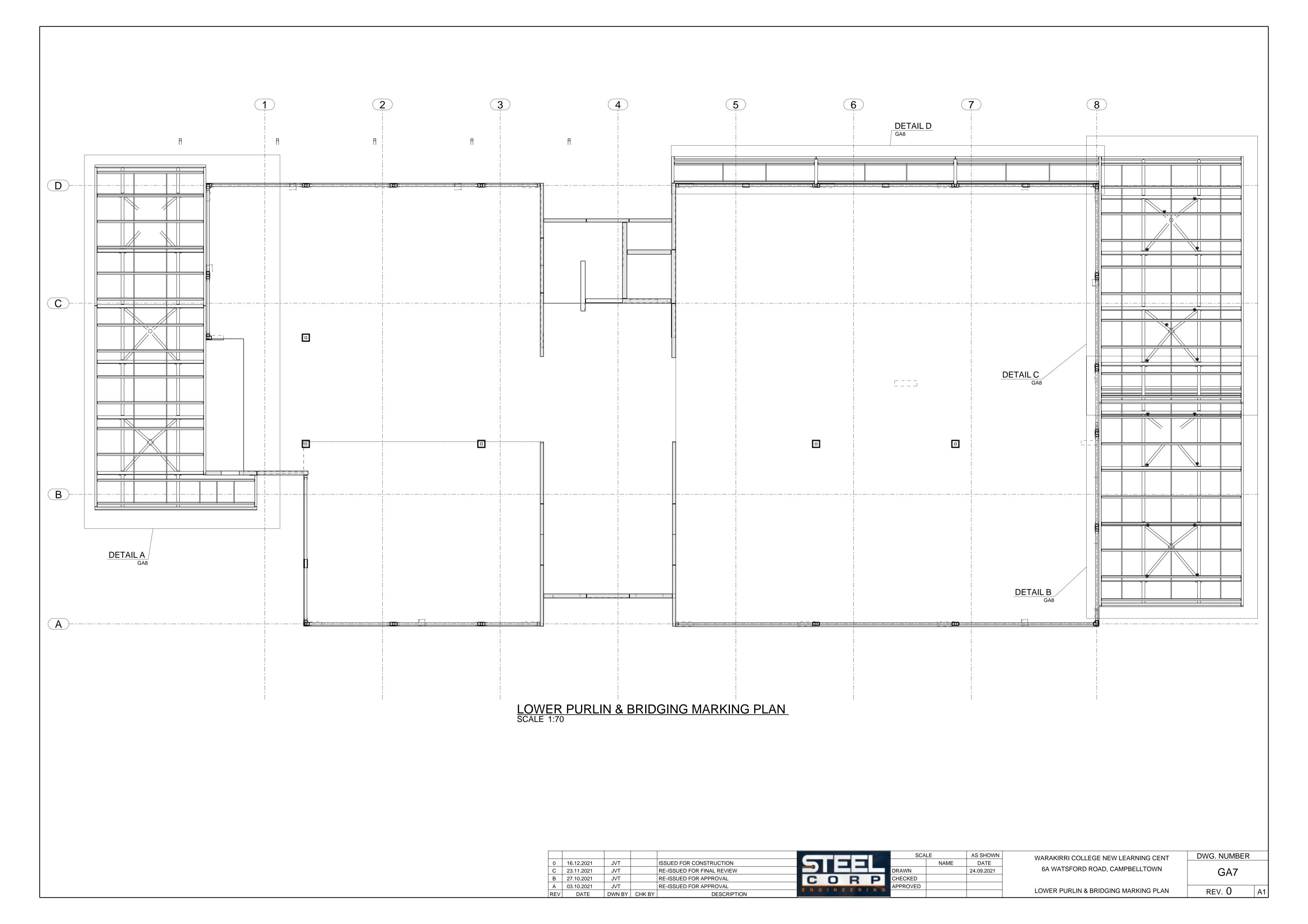


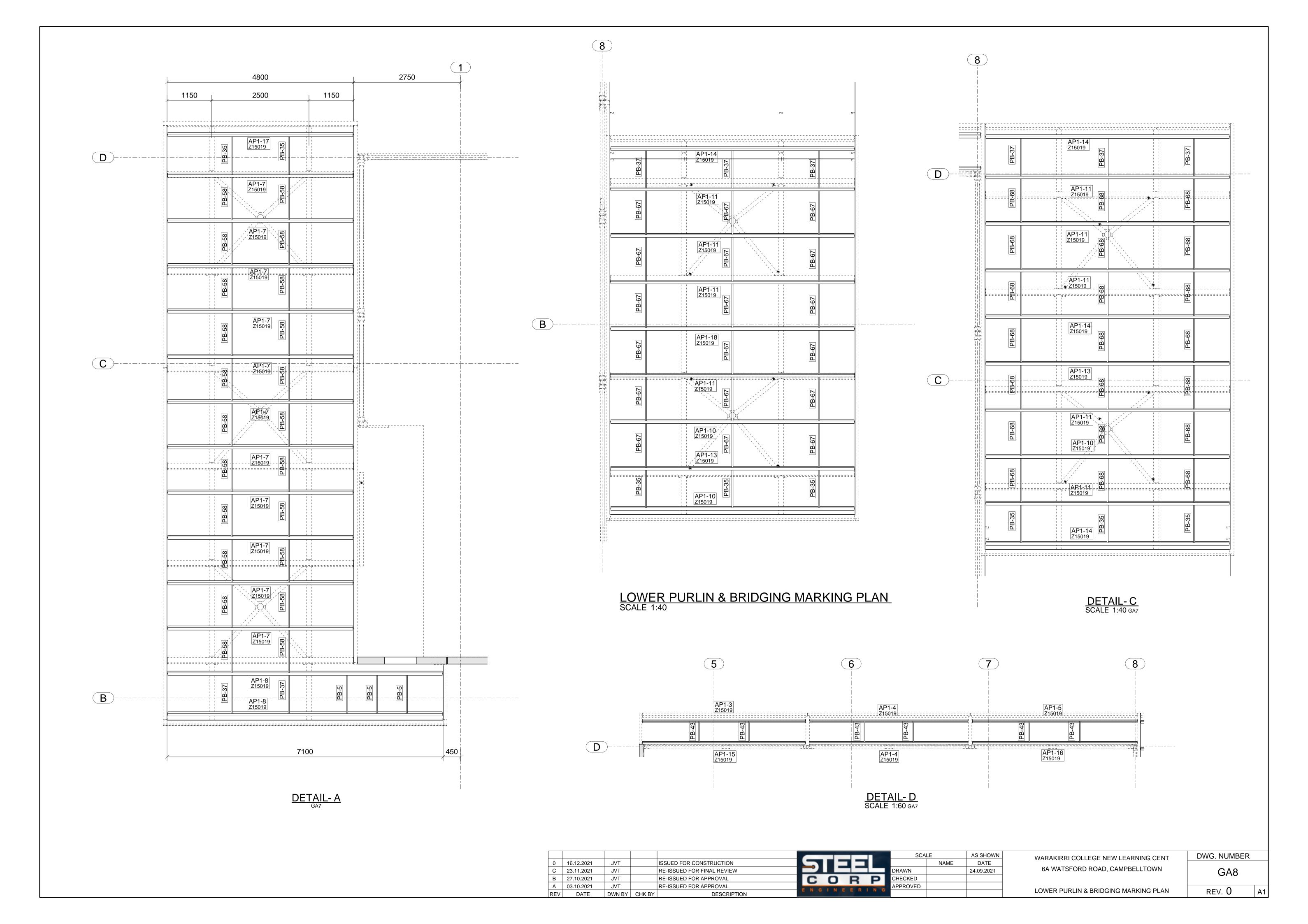


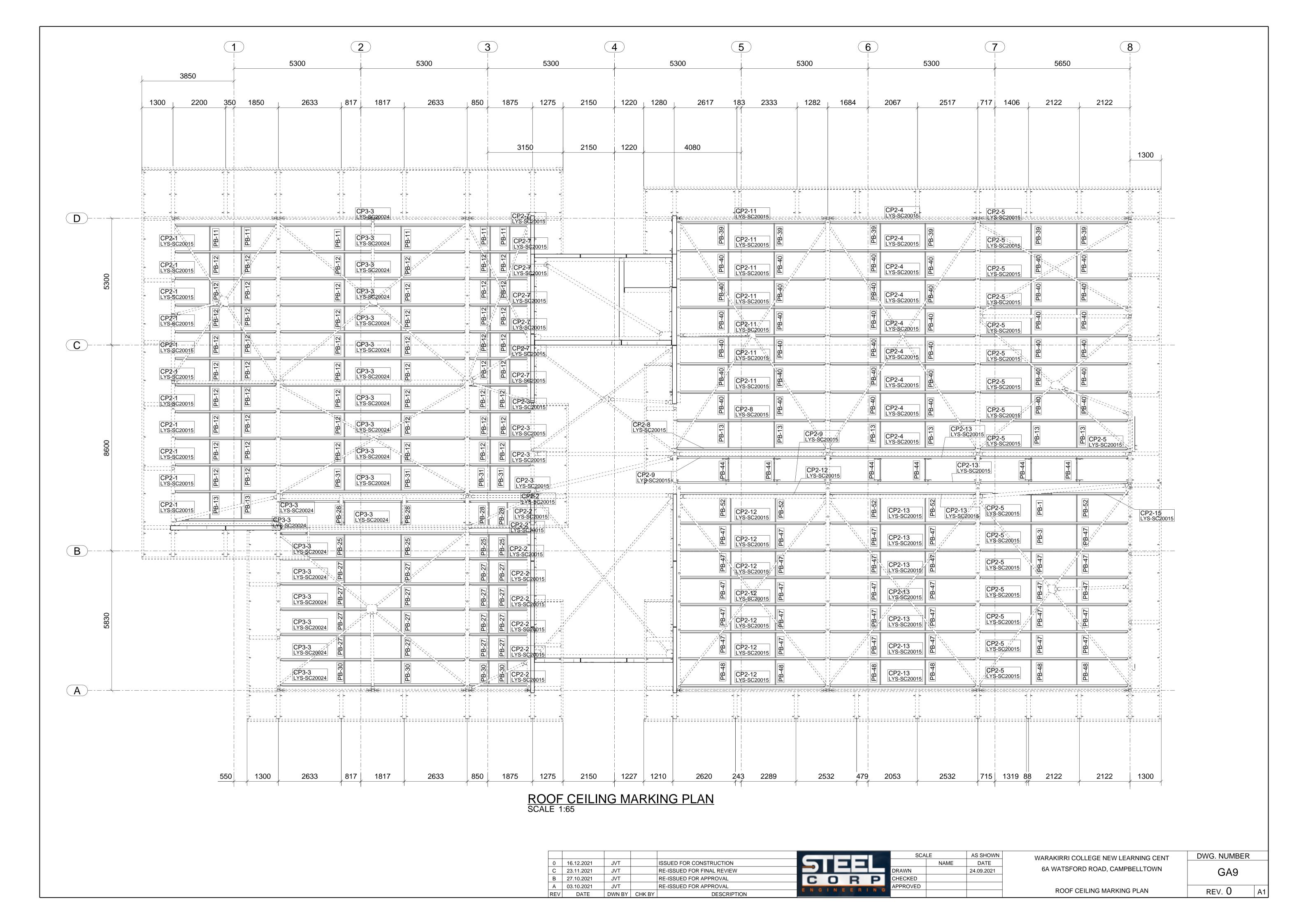


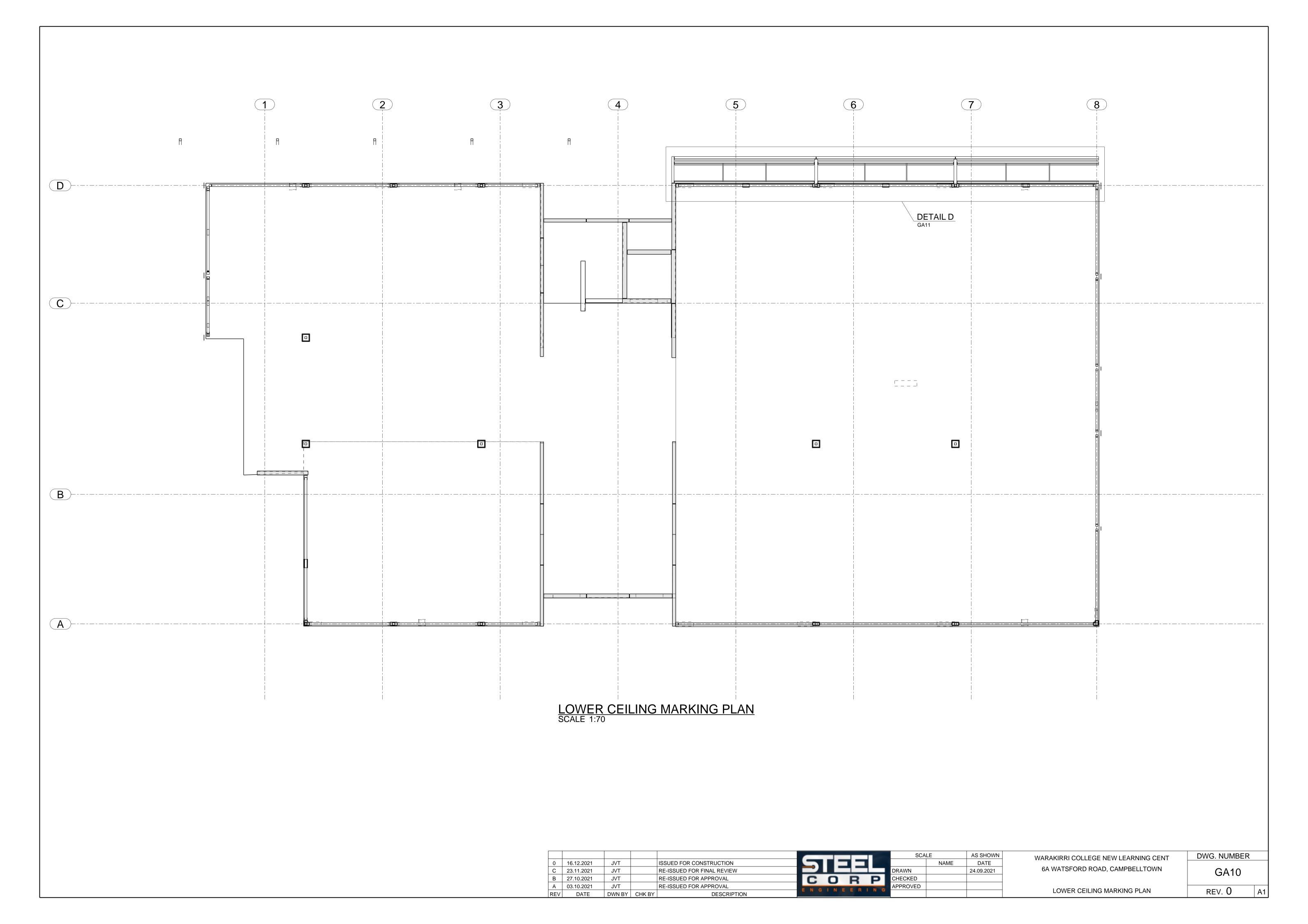


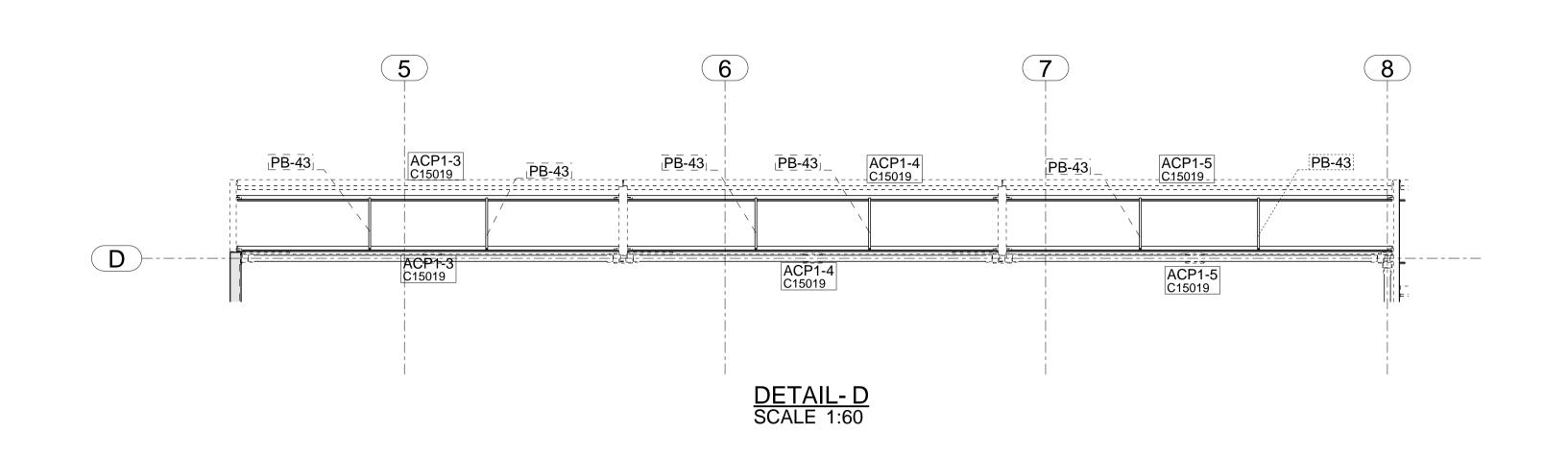












 0
 16.12.2021
 JVT
 ISSUED FOR CONSTRUCTION

 C
 23.11.2021
 JVT
 RE-ISSUED FOR FINAL REVIEW

 B
 27.10.2021
 JVT
 RE-ISSUED FOR APPROVAL

 A
 03.10.2021
 JVT
 RE-ISSUED FOR APPROVAL

 REV
 DATE
 DWN BY
 CHK BY
 DESCRIPTION



SCALE AS SHOWN
NAME DATE
24.09.2021

ED
/ED
LOWER CEILING MARKING PLAN

DWG. NUMBER

GA11

REV. 0 A1

