

## **GLENWOOD HIGH SCHOOL**

## CIVIL ENGINEERING FLOOD STUDY REPORT



Prepared for: School Infrastructure NSW

By: enstruct group pty ltd



# GLENWOOD HIGH SCHOOL CIVIL ENGINEERING FLOOD STUDY REPORT

#### **ISSUE AUTHORISATION**

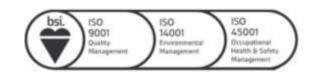
PROJECT: Glenwood High School

Project No: 6393

Rev	Date	Purpose of Issue / Nature of Revision	Prepared by	Reviewed by	Issue Authorise by
Α	18/10/21	ISSUE FOR REVIEW	NKK	KEH	KEH
В	10/11/21	ISSUE FOR APPROVAL	NKK	KEH	KEH
С	21/06/22	ISSUE FOR APPROVAL	NKK	KEH	KEH

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enstruct have been engaged by School Infrastructure NSW to provide civil engineering flooding advice for the proposed development at Glenwood High School (hereafter GHS). This report relates to the flooding characteristics relevant to the Main Works Building that is proposed in the northeast portion of the school

site and aims to satisfy the requirements of Section 16: Flooding of the Secretary's Environmental

Assessment Requirements (SEARs) for State Significant Development Application (SSD-23512960) for

Glenwood High School Upgrade.

This report identifies flood depths in and around the proposed Main Works Building area during the Probable Maximum Flood (PMF), 1% Annual Exceedance Probability (AEP), 0.5% AEP, and 0.2% AEP storm events, and outlines any flood risks associated with these depths. The flood heights, depths, and risks, have been identified through the preparation and analysis of a TUFLOW stormwater model.

The TUFLOW model indicates that the proposed new building location is not susceptible to flood inundation during the 1% AEP, 0.5% AEP, and 0.2% AEP storm events as the flood water is contained within the road reserve of Glenwood Park Drive and the drainage channel/water course. The new building location is affected by the flood depths caused by the very rare PMF storm event.

The north-eastern portion of the site is designated as a Low Hazard Risk area due to the extents of the PMF flood. Since the development site is in an elevated position in comparison to the flood heights contained in Glenwood Park Drive and the drainage channel during the 1%, 0.5% and 0.2% AEP storm events, no flood waters will be adversely impacted when the new building is constructed. Therefore, the overland flow paths and flood extents remain unaffected by the proposed development. In addition, neighbouring properties, existing assets, and infrastructure will not be impacted post construction of the new building works.

A freeboard of 0.5m above the 1% AEP flood level nearest to the site is required when setting Finished Floor Levels (FFL) and Flood Planning Levels (FPL) for building entrances. The current proposed FFL of the new building is RL60.90m AHD which is sufficiently above the 1% AEP flood height nearest to the building location.

There will be negligible building impacts due to climate change consequences such increased rainfall frequency and intensity. The design has sufficient controls and redundancy to cope with additional water without impact the proposed or existing buildings on the school campus.



This report specifically responds to the following Council comments:

46	The flood report is vague and does not provide critical information carried out for the flood modelling. Provide details of blockages, pipes, total catchment area, boundary conditions, flows, velocities etc.	All pipes are assumed and modelled blocked. All other information is included in report with the Tuflow model sent to Council. Refer section 2.2
47	Allow for a maximum isolated rise in flood level of 0.02 m external to the site in the 1% AEP event as a result of the development.	The 1% AEP flood extents does not inundate the school site. As the proposed development does not interact with the 1% AEP i.e. the school is flood free from the 1% AEP catchment flood flow. Refer Figure 13
48	All buildings in the floodplain (including both existing and proposed) are to be modelled as complete (i.e. 100%) blockages in the flood model. Simply applying Mannings 'n' value as suggested in Table 1 of the report is not accepted.	Flood model has been revised to include existing and proposed buildings as 100% blockages. Refer section 2.1
49	The post-development DEM is to be included to accurately represent any proposed ground level changes (i.e. cut and fill areas) in the post development model. This includes fill for areas such as driveway.	File provided
50	Adopt an impervious area of minimum 80% for the catchment. Amend the DRAINS model and flood report accordingly.	80% impervious has been adopted for DRAINS model
51	The maximum travel times for impervious catchment is 12 minutes and pervious catchment is 14 minutes. Amend the DRAINS model and catchment areas breakdown accordingly.	Travel times have been adopted in the DRAINS model
52	The flood report is to include flood maps for both the existing and post developed scenarios for the various storm events. This report is to include flood maps for the velocity, depths, hazard etc.	Refer Section 4.9 showing the flood hazard maps
53	Provide a flood difference map (developed - existing) for the 1% AEP. Include a 0 to 20 mm category with gradations below or above this figure.	As the proposed development does not interact with the 1% AEP i.e. the school is flood free from the 1% AEP catchment flood flow.
54	Ensure the inflow hydrograph in HECRAS matches the DRAINS model.	No HECRAS modelling is required to be undertaken
55	Provide a Flood Management Plan to address emergency flood management of the site include the use of appropriate warning signs, notices of procedures and depth gauges (if required).	Refer to Flood evacuation plan now produced
56	Provide the DRAINS hydrology and amended TUFLOW model electronically to Council.	TUFLOW and DRAINS Models provided for review

3

## Contents

1	Introduction	5
	1.1 The Proposal	
	1.2 The Site	6
2	Existing Site	7
	2.1 Site Topography	
	2.2 Existing Stormwater	
3	Flooding	8
4	Hydraulic Modelling	10
	4.1 TUFLOW Model Setup	10
	4.2 Model Extent	10
	4.3 Model Topography	10
	4.4 Surface Hydraulic Roughness	10
	4.5 Boundary Conditions	<b>1</b> 1
	4.6 Existing Stormwater	11
	4.7 Hydrological Modelling	11
	4.7.1 Hydrological Model Parameters	12
	4.7.2 Design Flood Events	12
	4.8 TUFLOW Model Flood Depths and Levels Results	12
	4.9 TUFLOW Model Hazard Results	14
	4.10 Proposed development implications	15
	4.10.1 Flood Planning Levels and Finished Floor levels	15
	4.10.2Impact on Neighbouring Properties	15
	4.10.3 Erosion, Siltation, and Destruction of Riparian Zones	15
	4.11 Climate Change	15
	4.12 Refuge	15
5	Conclusion	16

APPENDIX A	17
APPENDIX B	18
APPENDIX C	19
APPENDIX D	20
APPENDIX E	21
APPENDIX F	22

#### 1 Introduction

This Flood Study Report accompanies an Environmental Impact Statement (EIS) pursuant to Part 4 of the Environmental Planning and Assessment Act 1979 (EP&A Act) in support of a State Significant Development Application (SSD - 23512960).

The development is for upgrading works comprising alterations and additions to Glenwood High School at 85 Forman Avenue, Glenwood. The site is legally described as Lot 5227 DP 868693.

The site is roughly rectangular in shape, with a total area of 60,790m<sup>2</sup> and street frontages to Forman Avenue to the south and Glenwood Drive to the east. Glenwood Reserve adjoins the northern and western boundaries of the school.

This report addresses the relevant Secretary's Environmental Assessment Requirements (SEARs), specifically:

#### 16. Flooding

- Identify any flood risk on site in consultation with Council and having regard to the most recent flood studies for the development area and the potential effects of climate change, sea level rise and an increase in rainfall intensity (Section 4).
- Assess the impacts of the development, including any changes to flood risk onsite or off-site, and detail design solutions to mitigate flood risk where required (**Section 4**).
- Address Clause 5.21 of the Blacktown Local Environmental Plan 2015 (throughout report)
- Refer to relevant Policies and Guidelines: NSW Floodplain Development Manual (DIPNR, 2005).

This report also addresses the flooding requirements set out by Blacktown City Council and the Environment, Energy and Science Group (EES) within the NSW Department of Planning, Industry and Environment. Their submission to the SEARs flooding requirements can be referred to in **Appendix B and Appendix C**, respectively.

#### 1.1 The Proposal

The proposed development seeks to upgrade Glenwood High School. The upgrade consists of the following alterations and additions:

- Construction of a new three-storey building at the north-eastern portion of the site facing Glenwood Park Drive which will accommodate new learning spaces;
- Construction of one storey performance pavilion;
- Refurbishment of existing Building Block A (ground floor only) to provide one new support unit within the space of an existing general learning space;

- Refurbishment of Building Block D (ground floor only) to provide an additional office space and storeroom;
- Refurbishment of Building Block E to re-purpose it on the ground floor for computer learning spaces, staff and administration spaces as well as upgrades to the library on the first floor;
- Refurbishment of Building Block J to re-purpose it from visual arts and performing arts to learning spaces and workshops for food tech and woods/metal unit;
- Demolition of existing botany room and construction of a new single storey pavilion comprising of interview rooms and end-of trip facilities; and
- The proposed development will also involve ancillary works at the site associated with the proposed upgrades.



Figure 1: New Building Ground Floor Plan (Source: PTW Architects)

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Figure 2: New Building East Elevation (Source: PTW Architects)

#### 1.2 The Site

Glenwood High School is located at 85 Forman Avenue, Glenwood. An existing childcare centre is also located within the site. The site is located within the Blacktown City Council (BCC) Local Government Area (LGA) in the suburb of Glenwood. The school campus is situated within a well-established residential area approximately 4.7 km northeast of Blacktown Train Station.

The site is bound by residential development and Forman Street to the south, Glenwood Reserve to the north and west with residential development beyond, and Glenwood Park Road to the east, with a drainage channel/water course and residential development beyond. Refer to **Figure 3** and **Figure 4** below for the location of the school and surrounding neighbourhood.

Glenwood High School is located approximately 520m south of Parklea Public School, 1.5km to the southwest of Bella Vista Public School, and 3.5km to the east of Quakers Hill High School.

The built form and land use character surrounding the site is predominantly low scale, 1-2 storey dwelling houses. The site has a total area of 60,790m<sup>2</sup>. The site is legally described as Lot 5227 in Deposited Plan 868693.



Figure 3: Local Context Map (Source: Metromaps 2021 with Architectus overlay)



Figure 4: Site Context Map (Source: Metromaps 2021 with Architectus overlay)



#### 2 Existing Site

Glenwood High School is currently a co-educational high school and includes the following existing buildings and facilities:

- Ten (10) existing buildings comprising:
  - Building A and B: double storey classroom building;
  - o Building C: double storey classroom building with a fitness laboratory on the ground floor;
  - Building D: double storey building with administration and staff facilities on the ground floor and classrooms on the first floor;
  - Building E: double storey building with classrooms and science laboratories on the ground floor and main library on the first floor;
  - Building F: staff building;
  - Building G, H and J: single storey classroom buildings (Block J contains school canteen); Building
     K: single storey gymnasium; and
- There is an existing single storey childcare centre (known as Building L) at the southwestern corner of the site;
- Nineteen (19) single-storey demountable buildings, seventeen (17) of which comprise general learning spaces and two (2) which comprise of staff rooms;
- At-grade carpark providing for 93 car parking spaces accessed from two separate vehicular access points on Forman Avenue for high school staff;
- Three (3) support learning units.
- Existing on site bicycle parking racks are provided in the southeast corner of the school near the staff parking facility;
- Outdoor spaces comprising:
  - Quadrangle space between Buildings C, B and E;
  - Playing Field at the north-western corner of the site;
  - Games Court to the east of the playing field;
  - Covered outdoor space adjacent to the canteen, and
  - Grassed open play area in the centre of the site.

- There are three (3) pedestrian access points to the school, including:
  - One access point from Glenwood Park Drive; and
  - Two access points from Forman Avenue.
- The primary drop-off and pick-up area is located at Forman Avenue.

#### 2.1 Site Topography

The existing school site generally grades from the southwest from approximately RL69.00m AHD, to the northeast at RL59.00m AHD. The site is elevated above Glenwood Park Drive which bounds the site to the east and the road is lower than the school ground level along the entire eastern site boundary. Similarly, Foreman Avenue along the southern site frontage is below the ground level within the school site at all locations. Refer **Appendix A** for the site survey. The development location of the proposed Main Works Building on the site also grades from the southwest to the northeast corner of the school site. Therefore, the fall and height of the land in the location of the proposed building, and the school site in general, does not facilitate overland flow of stormwater. The site grades all runoff towards the road frontages of the site and collects any minor overland flow in existing in-ground drainage in paved areas and around buildings.

Existing buildings within the catchment have been modelled as blockages.

#### 2.2 Existing Stormwater

Existing in-ground stormwater throughout the school consists of stormwater pits and pipes collecting downpipes from the existing buildings, with water on surface areas and minor stormwater overland flow collected in pits. Council's stormwater infrastructure is located in Glenwood Park Drive and Forman Avenue. Refer to **Figure 6** for existing Council drainage infrastructure. In addition, there are five culverts allowing stormwater to flow within the drainage channel/water course beneath Glenwood Park Drive. This water course flows towards the north and into Glenwood Lake. Refer to **Figure 5** for Sydney Water culvert layout under Glenwood Park Drive.

The school site does not have any existing onsite stormwater attenuation or pollutant removal systems.

The TUFLOW model assumes the existing in-ground pit and pipe system is blocked.



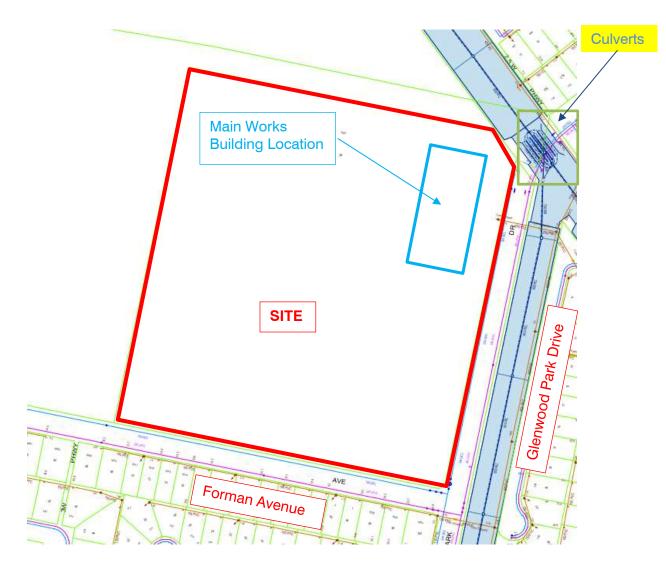


Figure 5: Existing Stormwater Infrastructure (Source: Sydney Water)

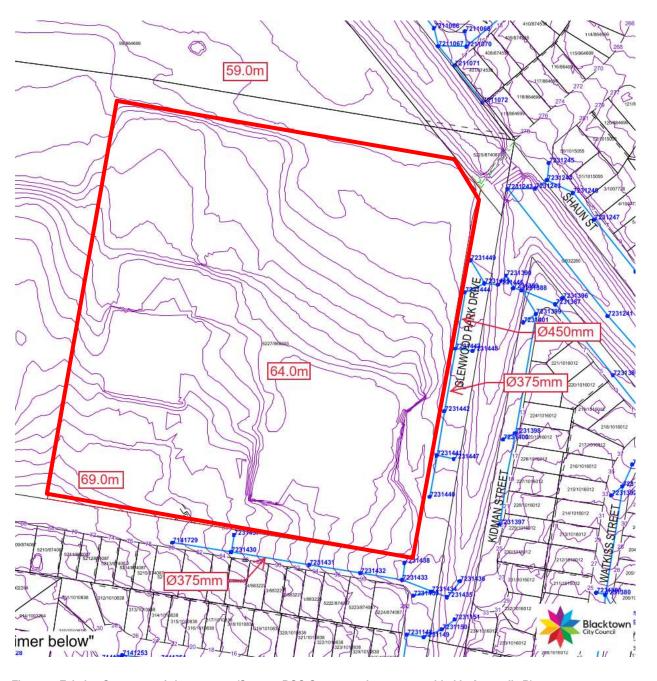


Figure 6: Existing Stormwater Infrastructure (Source: BCC Correspondence as provided in Appendix B)

### 3 Flooding

As per the BCC Engineering Guide for Development 2005 and Section 10.7 Planning Certificate for the site issued by Council, the site is not identified as flood prone, as shown in **Figure 7** and is identified as being in the Low Flood Risk Precinct of the Caddies Creek floodplain.

A few hundred metres north of the school site is Glenwood Lake. Glenwood Lake acts as a detention basin that holds water and discharges flows to Caddies Creek. Caddies Creek is a tributary of Cattai Creek. The watercourse adjacent the school site is a tributary of Caddies Creek and is a Class 1 stream order as classified by the NSW Department of Planning, Industry and Environment publication: *Determining stream order Fact sheet*. Refer to **Figure 8** for the location of Glenwood Lake with respect to the school campus.

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As noted in Woolcotts' Flood Report (16/06/20), Glenwood Lake was constructed as part of the Rouse Hill Development Area (RHDA) flooding and stormwater management strategy. This strategy consists of several detention basins constructed within the RHDA along the tributaries of Cattai Creek. The flood behaviour of these basins was modelled by WMA Water (engaged by Sydney Water) as part of the Rouse Hill Flood Study in 2014. An extract of this study showing Glenwood Lake acting as a detention basin during the 1% AEP event is shown in **Figure 9**.

Flood management of the site is to be in accordance with the NSW Floodplain Development Manual (DIPNR, 2005).

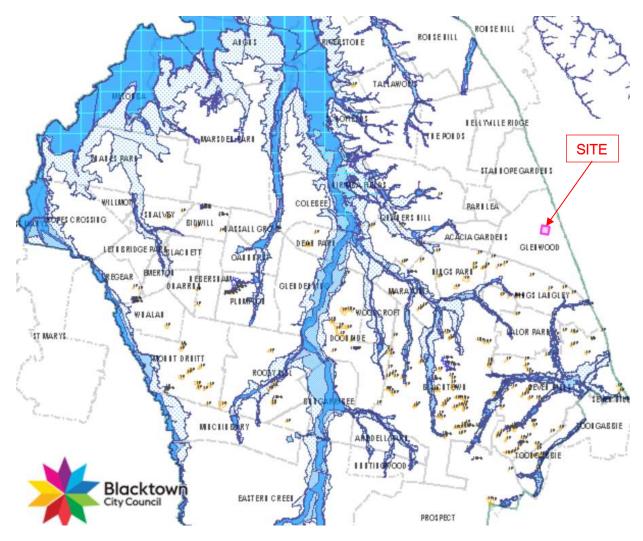


Figure 7: Blacktown City Council Flood Prone Land Extents



Main Works
Building Location

Figure 8: Glenwood Lake Location

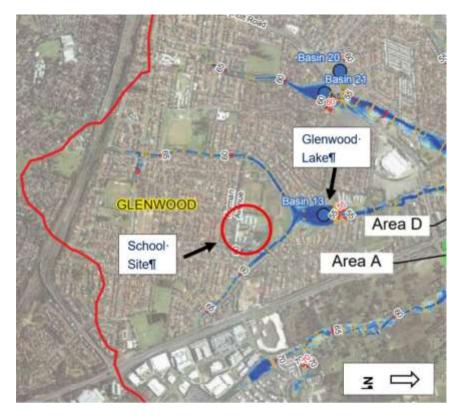


Figure 9: Glenwood Lake during a 1% AEP storm event (WMA Flood Report (as seen in Woolacotts' Flood Report 16/06/20))



#### 4 Hydraulic Modelling

To confirm the development site's susceptibility to flooding and assess the potential for flooding impacts affecting the site, a TUFLOW flood model was developed by enstruct using publicly available Digital Terrain Models (DTMs) from LIDAR survey data for the catchment area surrounding the site. The DTM represents the bare ground surface without any elevated objects, such as trees, cars, and items of a similar nature.

#### 4.1 TUFLOW Model Setup

DTM surface levels and terrain data was retrieved from the ICSM Elvis Elevation and Depth – Foundation Spatial Data website for the area surrounding the site, ensuring the upstream stormwater catchment area that would drain towards the school site was included. The DEMs were provided in grid sizes of 2m. This contour data was then imported into QGIS, an open source Geographic Information System, whereby an accurate catchment plan could be produced for site. Refer to **Figure 10** for the development catchment extent.



Figure 10: Stormwater catchment plan surrounding school site

#### 4.2 Model Extent

The TUFLOW Model boundary is shown in **Figure 10**. The catchment area extends to Old Windsor Road in the east, the M7 Westlink to the south, Glenwood Park Drive in the west, and Glenwood Lake to the north. The model is comprised of a 2D domain of the study area ground surface reflecting the catchment topography, with varying roughness as dictated by land use.

#### 4.3 Model Topography

The model topography is based on a 2m DTM derived from LiDAR data. The model has a 2m regular grid which is considered adequate to represent flood behaviour for this flood assessment as the channels in the vicinity of the site are 20m and 30m wide. These are well defined by a 2m grid. As the LiDAR data has a 2m resolution, so there is no benefit in preparing a flood model with a grid finer than 2m, as required by Chapter 15 of the BCC WSUD Developer Handbook.

#### 4.4 Surface Hydraulic Roughness

Manning's 'n' values were applied to represent the roughness of the various land uses in the model domain. The aerial photographs were used to define spatial extent and type of land use in the study area. Adopted Manning's n values in the TUFLOW model are summarised in **Table 1**. Buildings within the floodplain have been modelled as full obstructions. A Manning's 'n' of 0.05 has been applied to residential lots to capture various flow obstructions such as fences, garden beds and the like, as required by Chapter 15 of the BCC WSUD Developer Handbook.

Table 1: Materials in catchment area

Material Number	Manning's Value 'n'	Surface Type
1	0.100	Heavy Vegetation
2	0.070	Medium Vegetation
3	0.050	Residential
4	0.025	Roadways
5	0.025	Pavement
6	0.04	Light Vegetation
7	0.070	Waterway/Channels
8	0.050	Residential High Density



#### 4.5 Boundary Conditions

Boundary conditions were set for locations of inflow to the catchment area alongside defining flows through the outlet structure from Glenwood Lake and the culverts located under Old Windsor Road. The flow calculation for the outlet structure from Glenwood Lake is shown in **Appendix D**. A site inspection undertaken by enstruct allowed an accurate flow profile to be developed for the basin outlet structure (**Figure 11 & 12**). One low flow outlet structure was noted, with two chambers being 1.5 m x 1.5 m with a total of  $4.5 \text{m}^2$  inlet area. The low flow structure connects to the downstream chamber with two 1.5 m wide x 0.9 m high box culverts. One high flow inlet structure with two chambers of 3 m x 3 m with a total inlet area of  $9 \text{m}^2$ . This structure connects to the downstream open channel with two 3 m wide x 1.2 m high box culverts (**Figure 12**).

There are five culverts below old Windsor Road, measuring at least 3m wide and 0.9m in height. Two 600mm diameter pipes are in the ground with one 600mm diameter pipe downstream of the bridge.



Figure 11: Looking upstream in the high flow culvert outlet, with low flow culvert seen beyond.

#### 4.6 Existing Stormwater

The in-ground drainage system (Council and Sydney Water) adjacent to the site was assumed to be blocked during the flood modelling in accordance with Chapter 15 of the BCC WSUD Developer Handbook.

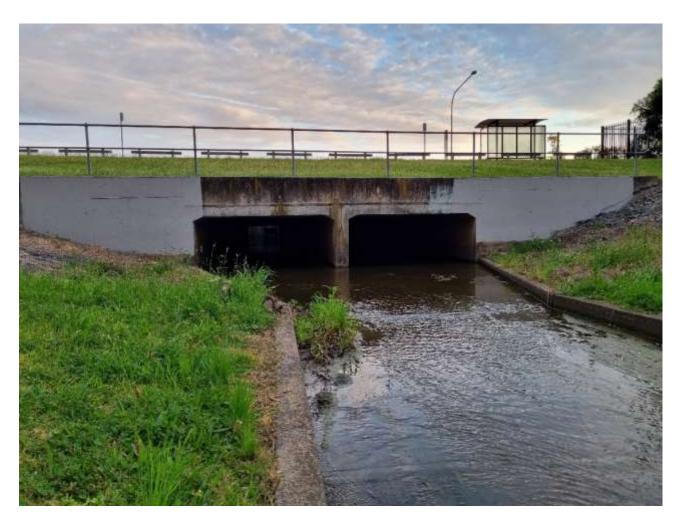


Figure 12: Photo downstream of Glenwood Lake showing two 3.0m wide x 1.2m high box culverts from outlet structure, discharging into an open culvert (adjacent Parklea Public School)

#### 4.7 Hydrological Modelling

A DRAINS model was set up to prepare the rainfall data specific to the site for the 1% AEP, 0.5% AEP, 0.2% AEP and Probable Maximum Flood (PMF) storm events. Design rainfall data for the catchment during the 1%, 0.5% and 0.2% AEP storm events was sourced from the Australia Rainfall and Runoff (ARR) 2016 Data Hub online.

The Probable Maximum Precipitation (PMP) rainfall profile for the site was estimated using the Commonwealth Bureau of Meteorology Estimation of Probable Maximum Precipitation in Australia: Generalised Short-Duration Method (GSDM). This allowed analysis of the PMF storm event to assess the sensitivity of the site to an increase in rainfall intensity and flood producing rainfall events due to climate change. This addresses the scope of works required in the SEARS Advice SSD-23512960. PMP rainfall calculations for the site can be seen in **Appendix E**.



#### 4.7.1 Hydrological Model Parameters

An initial and continuing loss model was utilised in the DRAINS model to represent rainfall losses during storm events such as depression storage and soil infiltration. Initial and continuing loss values are consistent with losses adopted when using ARR 1987. While ARR 2016 has proposed revised default losses, these are subject to uncertainty. Retaining the 1987 values provides appropriately conservative estimates for flood levels for design purposes. The following loss values were adopted:

#### 1% AEP storm event:

- Pervious areas: initial loss 17.4mm, continuing loss 1mm/hr
- Impervious areas: initial loss 1.5mm, continuing loss 0mm/hr.

#### 0.5% AEP storm event:

- Pervious areas: initial loss 17.4mm, continuing loss 1mm/hr
- Impervious areas: initial loss 1.5mm, continuing loss 0mm/hr.

#### 0.2% storm event:

- Pervious areas: initial loss 17.4mm, continuing loss 1mm/hr
- Impervious areas: initial loss 1.5mm, continuing loss 0mm/hr.

#### PMF storm event:

- Pervious areas: initial loss mm, continuing loss mm/hr
- Impervious areas: initial loss mm, continuing loss mm/hr.

#### 4.7.2 Design Flood Events

For the 1%, 0.5% and 0.2% AEP storm events, storm durations of 5, 10, 15, 20, 25, 20, 45, 60, 90, 120, 180, 270, 360, and 540 minutes were run. The PMF event was analysed at storm durations of 15, 30, 45, 60, 90, 120, 150, 180, 240, 300, and 360 minutes. Each duration was modelled with 10 temporal patterns and the median value for the inflow hydrograph for the 1%, 0.5%, and 0.2% AEP storm events was selected. The results were exported for use in the TUFLOW model. For the PMF event, the hydrological model was run with the temporal pattern outlined in the GSDM.

#### 4.8 TUFLOW Model Flood Depths and Levels Results

The TUFLOW model indicates that the new building location in the northeast corner of the school site is unaffected by flood water in the 1% AEP, 0.5% AEP, and 0.2% AEP storm events as per **Figures 13, 14 & 15**. During each of these storm events, the stormwater is contained within the roadway of Glenwood Park Drive and the grassed drainage channel/water course (Refer to **Figure 4** for the drainage channel location) to the northeast of the site, with the maximum flood height of RL 58.80m AHD, RL59.00m AHD, and RL59.20m AHD being the closest proximity to the site in each respective storm event whilst still being contained within the road reserve or within the drainage channel.



Figure 13: 1% AEP Storm Event Flood Levels





Figure 14: 0.5% AEP Storm Event Flood Levels



Figure 15: 0.2% AEP Storm Event Flood Levels

The PMF storm event is defined as 'the greatest depth of precipitation for a given duration meteorologically possible for a given size storm area at a particular location at a particular time of year' by the World Meteorological Organisation. The risk of it happening in any 1 year is extremely rare and is considered to be between a 1 in 10,000 to 1 in 10,000,000 chance. The TUFLOW model indicates that the new building location is susceptible to PMF flood heights of up to RL 60.30m AHD (**Figure 16**).



Figure 16: PMF Flood Levels

#### 4.9 TUFLOW Model Hazard Results

The Flood Risk Precincts at the site have been analysed in accordance with BCC and ARR 2019 classifications. As shown in **Figure 17**, the northeast portion of the site where the Main Works Building is proposed is within in a Low Risk precinct due to the extent of flood water in a PMF storm event (PMF extent as shown in **Figure 16**). There are High Risk areas in Glenwood Park Drive as well as within the drainage channel which corresponds to the high hazard caused in the 1% AEP storm event, but no high risk hazard areas exist within the school campus.

Medium High Risk zones are also present in Glenwood Park Drive and the drainage channel which represent the non-high hazard areas during the 1% AEP storm event. As the maximum flood depth traversing the site is 0.5m during the PMF storm event, this corresponds to a H2 level site in accordance with Figure 6.7.9 Combined Flood Hazard Curves in the Australian Rainfall and Runoff (2019) A Guide to Flood Estimation. No areas of higher hazard risk are located within the school campus.



Figure 17: Hazard Map



#### 4.10 Proposed development implications

#### 4.10.1 Flood Planning Levels and Finished Floor levels

Since the new proposed building location is not susceptible to flooding in the 1%, 0.5%, and 0.2% AEP storm events, there is no impact on the proposed building design. BCC requires a freeboard for Finished Floor Levels (FFL) of 500mm above the 1% AEP flood level in the trunk drainage area (Blacktown City Council Engineering Guide for Development 2005). It is therefore recommended that the FFL for the new classroom building is a minimum of RL58.80m AHD to maximise safety against flooding. This is 500mm above the 1% AEP flood height in Glenwood Park Drive of RL58.80m AHD. The current proposed FFL of the new building at RL60.90m AHD is sufficiently above the 1% AEP flood depth in the nearby roadway and drainage channel as well as the flood height in the PMF event. This meets the minimum freeboard requirement.

#### 4.10.2 Impact on Neighbouring Properties

It is evident from the TUFLOW model that the proposed new building development will not cause detrimental increases in the potential flood affectation of other neighbouring properties, assets, and infrastructure, as the floodwater remains contained within the roadway and drainage channel. Since the location of the proposed building is not within an overland flow path or flooding extents during the 1%, 0.5%, and 0.2% AEP events, no flood waters will be adversely diverted when the new building is constructed. Likewise, no increase in flow velocity will be caused as the stormwater flow is not being diverted, compensating a downstream system. Therefore, the overland flow paths and flood extents remain unaffected by the proposed development, and hazard category does not increase.

#### 4.10.3 Erosion, Siltation, and Destruction of Riparian Zones

As the stormwater flow paths in the 1%, 0.5% and 0.2% AEP storm events are not obstructed or altered by the proposed development, by an increase in stormwater volume or velocity causing erosion or siltation within a flow path is not anticipated to occur.

The stormwater runoff pollutant profile may change slightly due to the change from impervious grassed land to pervious building roof post development. This alteration will be very minimal as the proposed building structure largely matches the extent of the existing demountable buildings on the site. Water filtration devices and methods are also to be incorporated into the civil design for the new building to manage any increase in stormwater pollution. The water balance including water quantity and quality of Glenwood Lake will therefore not be impacted by the development. Refer to the **Civil Engineering** report for WSUD and stormwater pollutant removal and treatment proposed.

#### 4.11 Climate Change

As shown in the TUFLOW model results, the flood levels during a 1%, 0.5%, and 0.2% AEP storm event are located outside the school site boundaries and are contained within the roadway of Glenwood Park Drive and the drainage channel/water course. As these major storm events are only expected to occur once in every 100, 200 and 500 years and are an approximation of the future 1% AEP storm events due to climate change, it is suggested that the site is at minimal flood risk due to climate change. The school campus is in an elevated position in relation to the flood heights in major storm events. With the proposed building FFL of RL60.90m AHD is also 0.6m above the very rare PMF storm event flood levels (RL60.30m AHD). Therefore, the effects of climate change on the proposed development design has been taken into consideration and it is believed that the Main Works Building will remain above flood heights even with an increase in flood depths due to climate change by having FFLs significantly above the anticipated flood heights for future 1% AEP storm events.

#### 4.12 Refuge

In case of an unprecedented flood emergency or occurrence of a PMF flood event, personnel on the site are to seek refuge within the site and shelter-in place at a level above the PMF flood height (Section 15.3.1 of the BCC WSUD Developer Handbook). As the Glenwood Park Drive road reserve contains flood water up to 1m in depth in the 1%, 0.5%, and 0.2% AEP storm events, leaving the site and access to Glenwood Park Drive bridge of the drainage channel will be unsafe and as such, remaining in the building above the flood levels is the desired option of refuge in this event. Alternatively, leaving the school site on roads away from the bridge is possible as roads to the south are still flood free.

As required by the SEARs, consultation with the NSW State Emergency Service (SES) has been made. Correspondence with the SES is provided in **Appendix F.** 

#### 5 Conclusion

The TUFLOW model indicates that the new building location in the northeast corner of the school site is unaffected by flood water in the 1% AEP, 0.5% AEP, and 0.2% AEP storm events. The new building location is susceptible to PMF flood heights of up to RL 60.30m AHD.

The site is nominated as a Low Risk area in its north-eastern portion, and H2 hazard level in accordance with Figure 6.7.9 Combined Flood Hazard Curves in the Australian Rainfall and Runoff (2019) A Guide to Flood Estimation.

Since the location of the proposed building is not within an overland flow path or flooding extents during the 1%, 0.5%, and 0.2% AEP events, no flood waters will be adversely diverted when the new building is constructed. Likewise, no increase in flow velocity will be caused as the stormwater flow is not being diverted, compensating a downstream system.

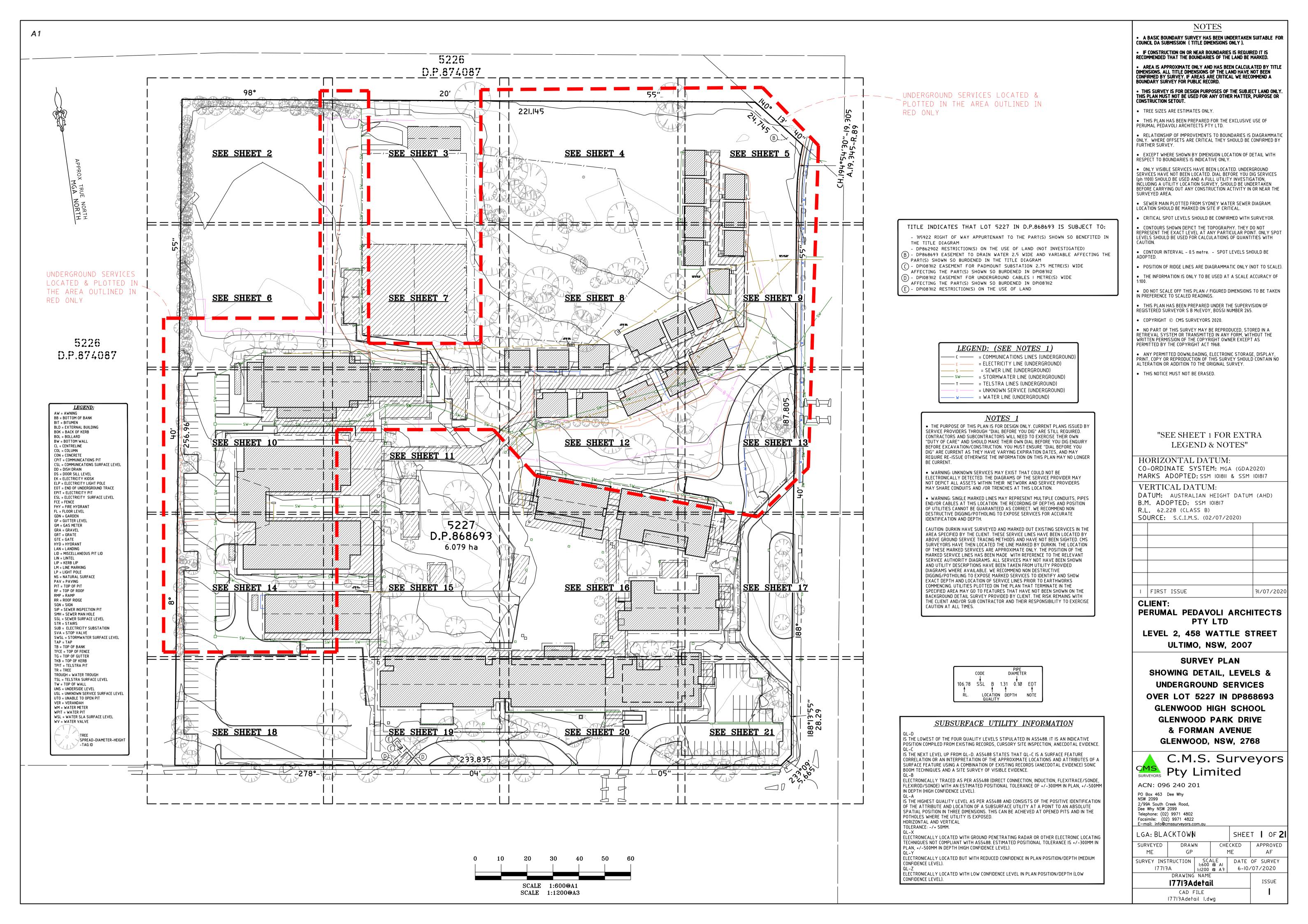
The current proposed FFL of the new building at RL60.90m AHD is sufficiently above the 1% AEP flood depth in the nearby roadway and drainage channel as well as the flood height in the PMF event. This meets the minimum freeboard requirement.

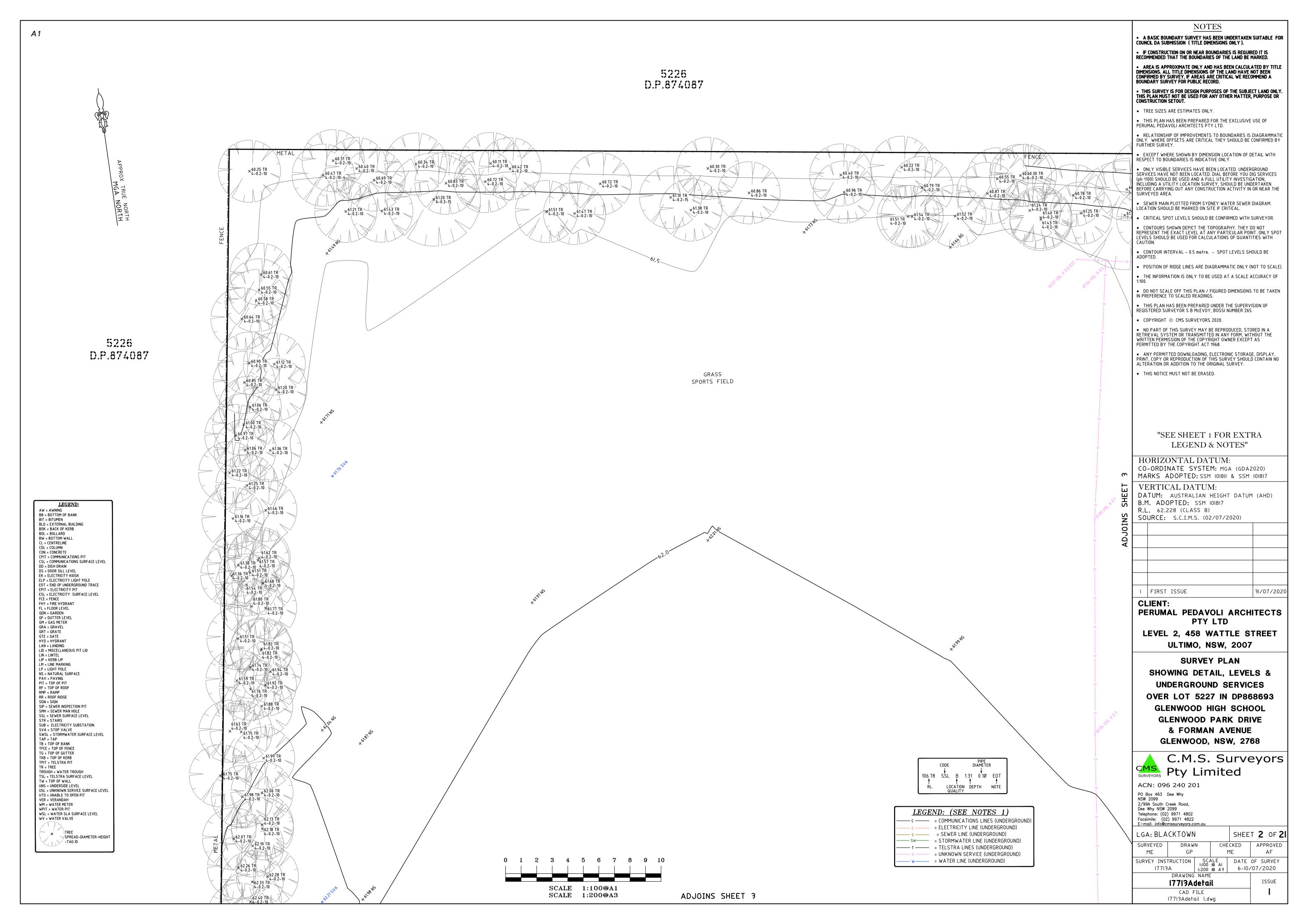
The school site is in an elevated position in relation to the flood heights in major storm events. The proposed FFL of RL60.90m AHD is also 0.6m above the very rare PMF storm event flood levels (RL60.30m AHD). It is believed that the Main Works Building will remain above flood heights even with an increase in flood depths due to climate change.

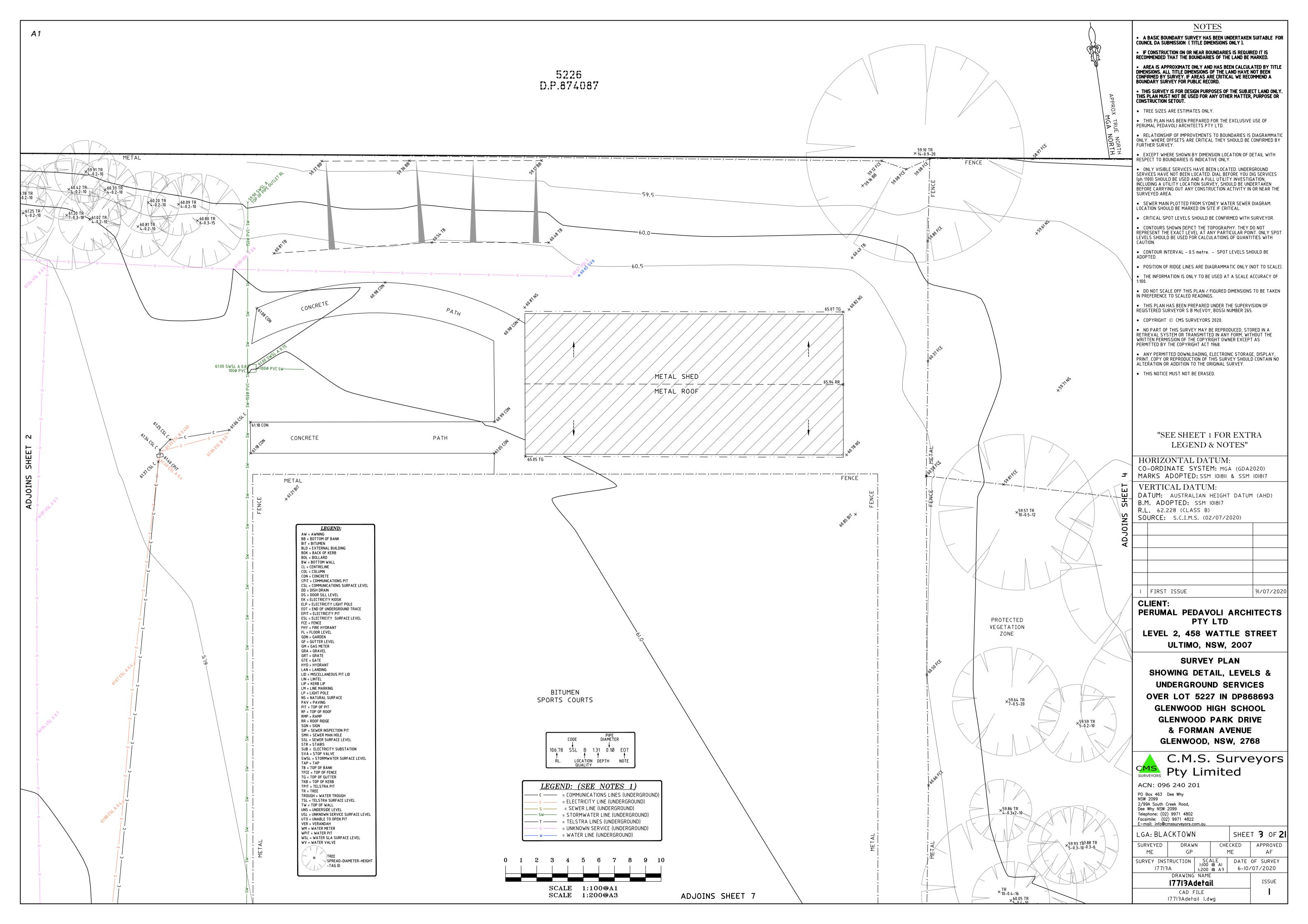
A shelter-in-place refuge approach is recommended for the site in a rare/large storm events where departure from the site onto Glenwood Park Drive can be unsafe, especially towards to the watercourse/bridge. Departure away from the watercourse, Glenmore Park Drive, may still be possible, noting that the risk of flash flooding and overland flow still exists, but with limited depth of water.

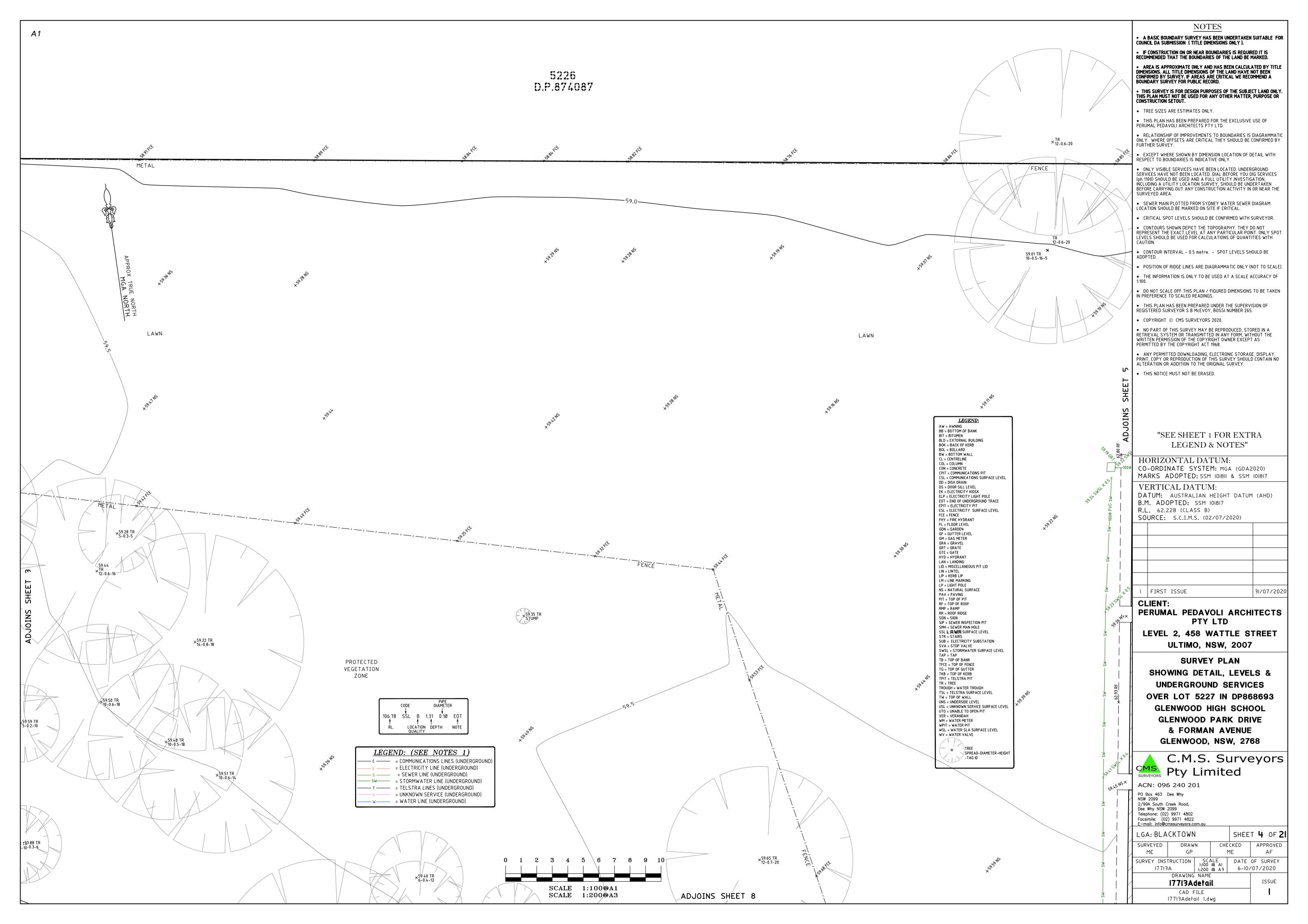
## APPENDIX A

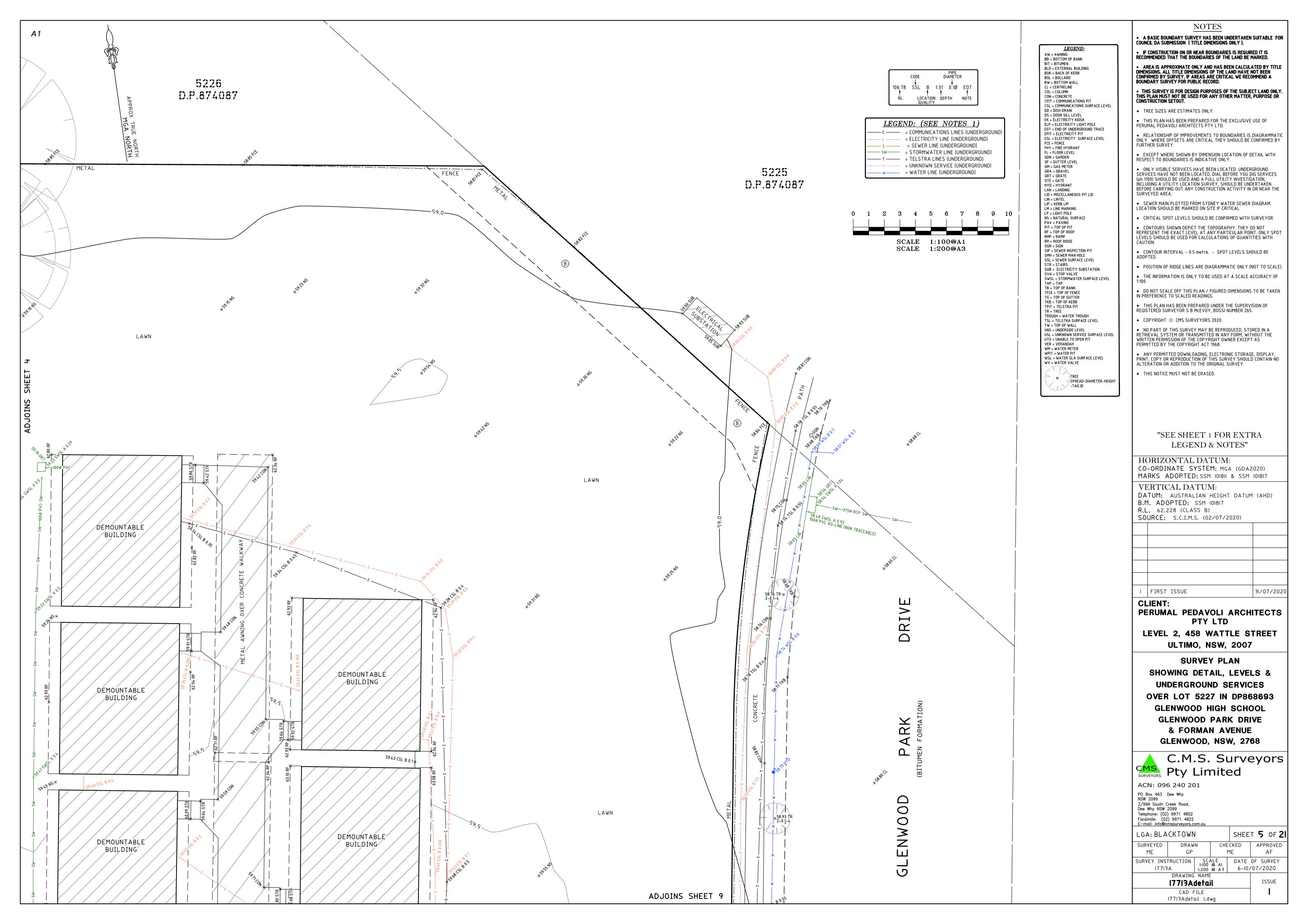
## SITE SURVEY

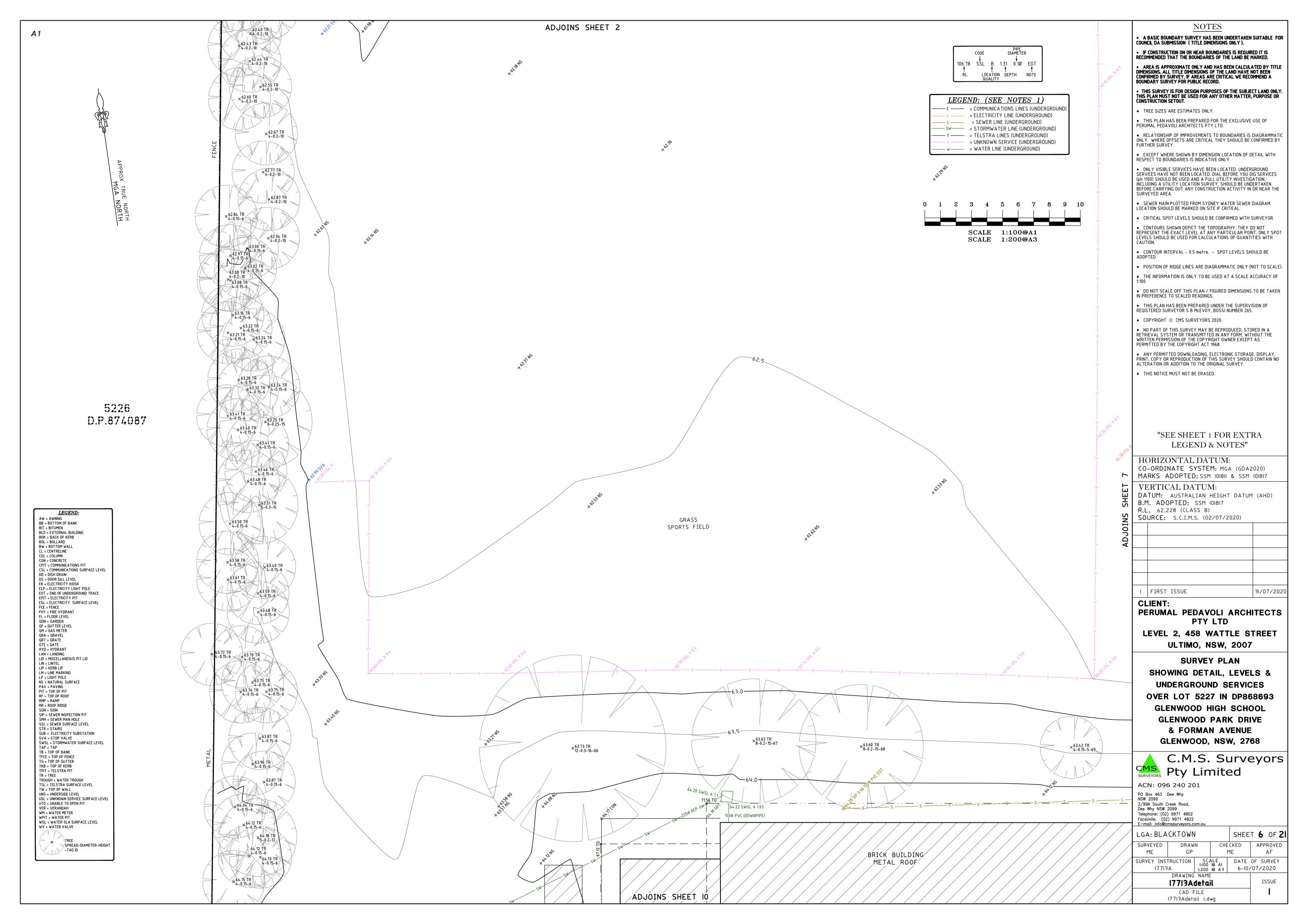


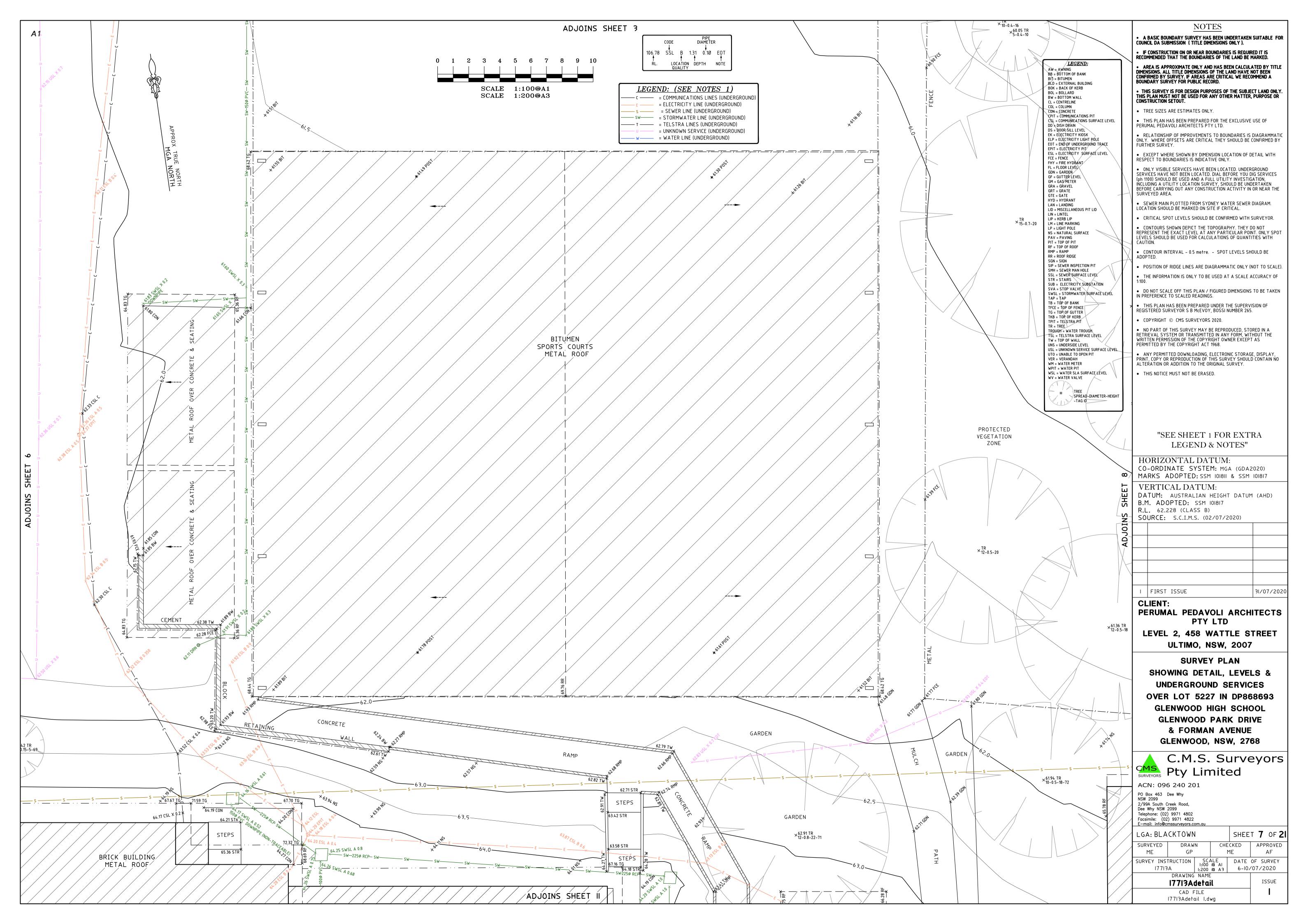


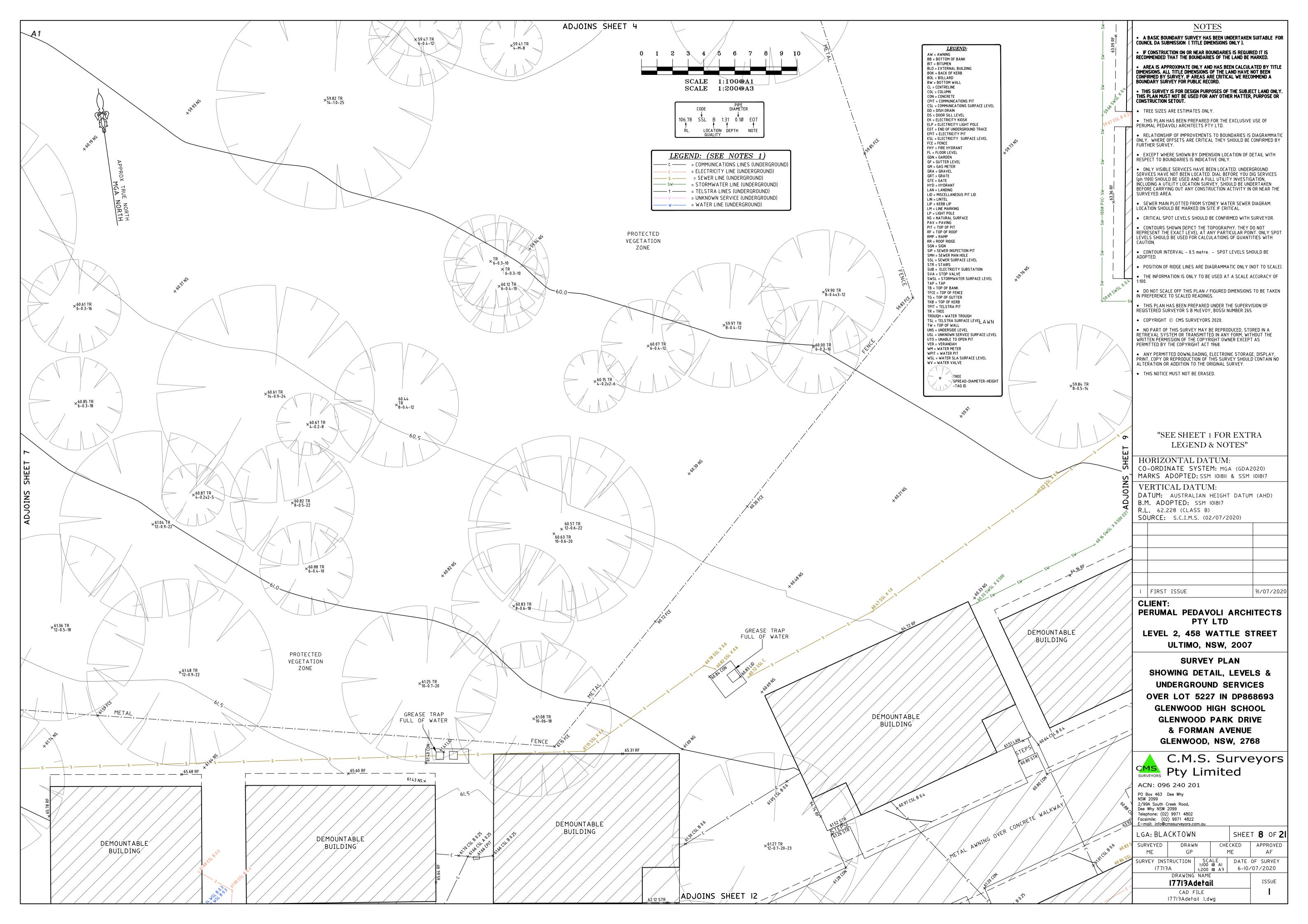


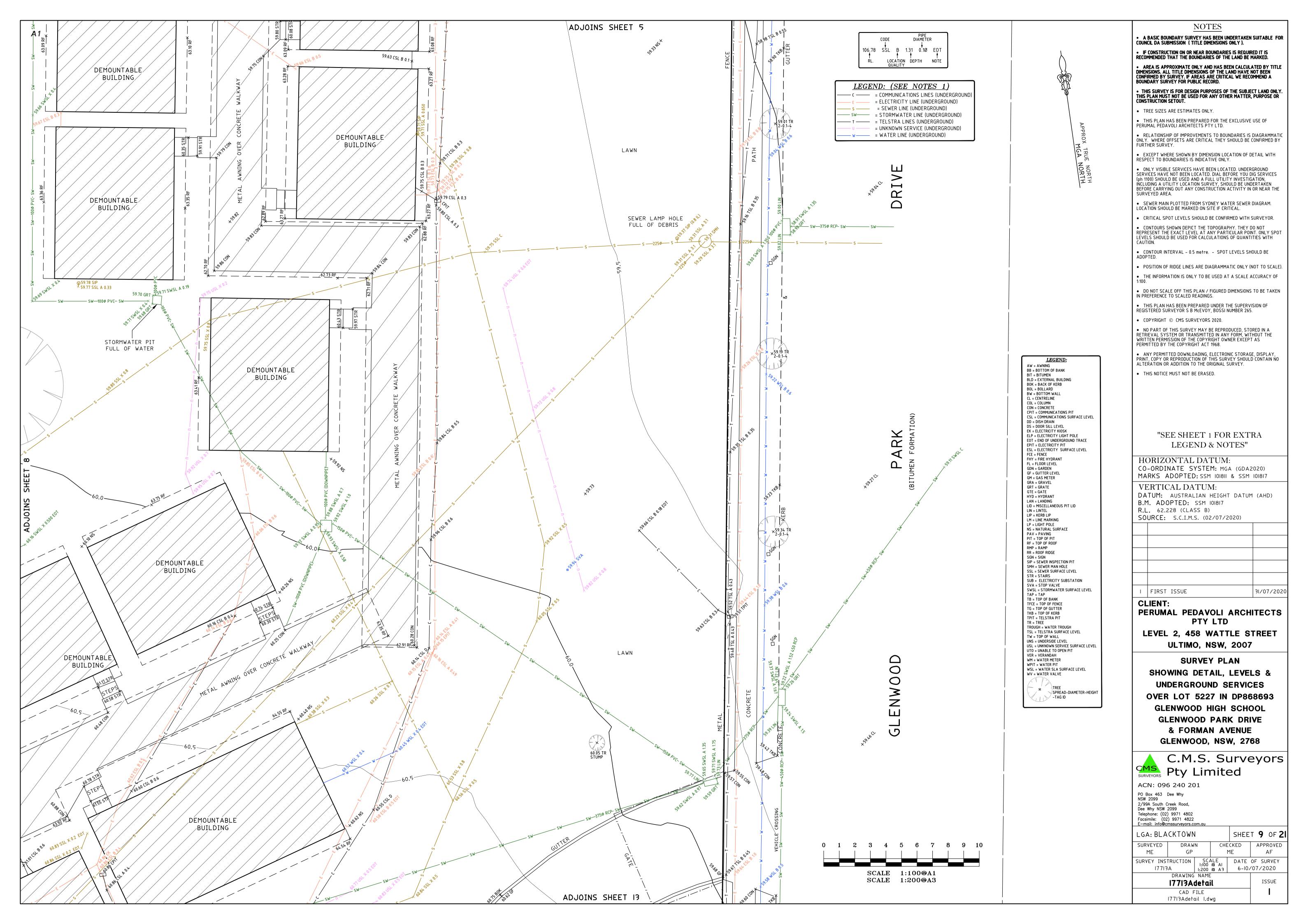


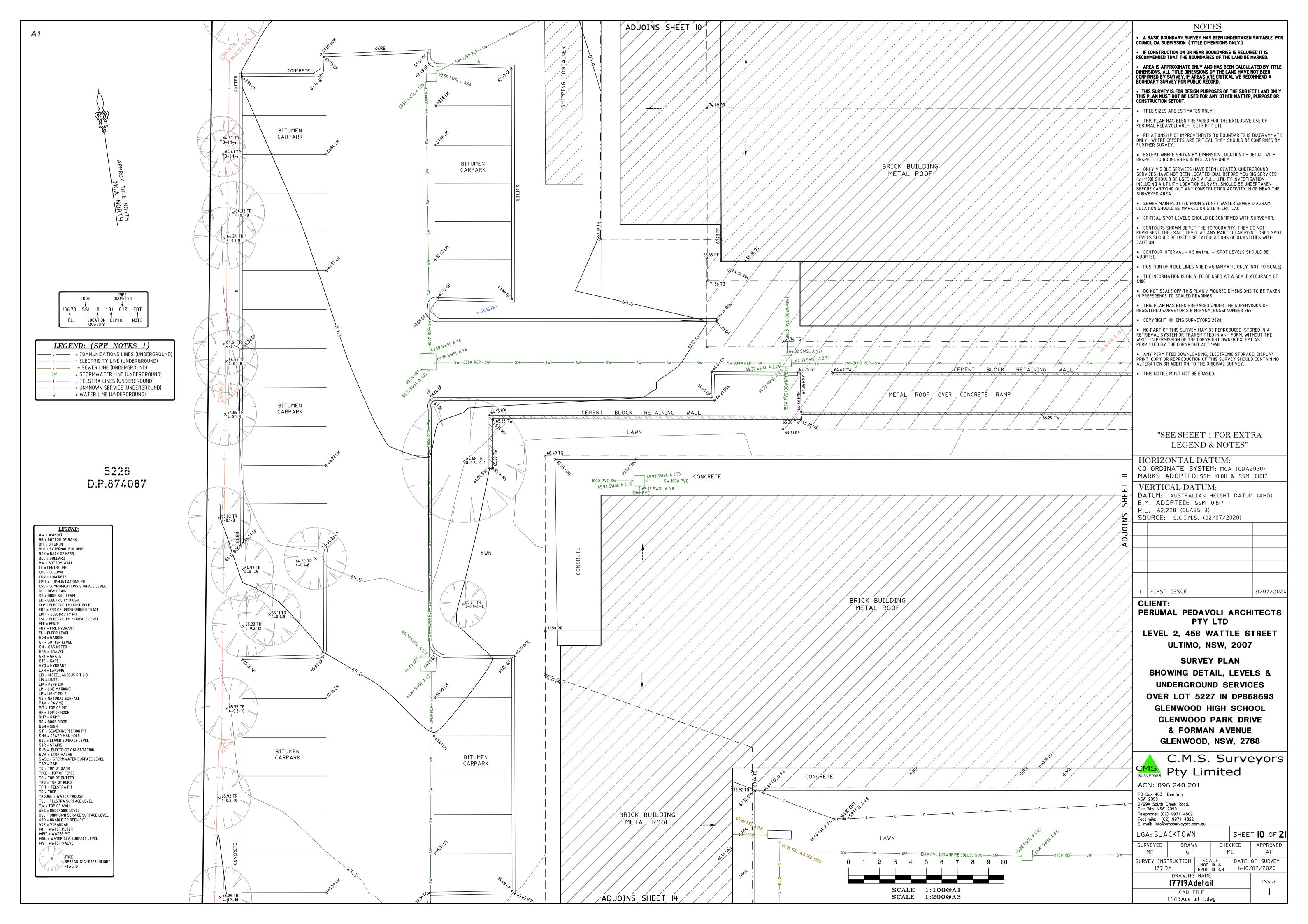


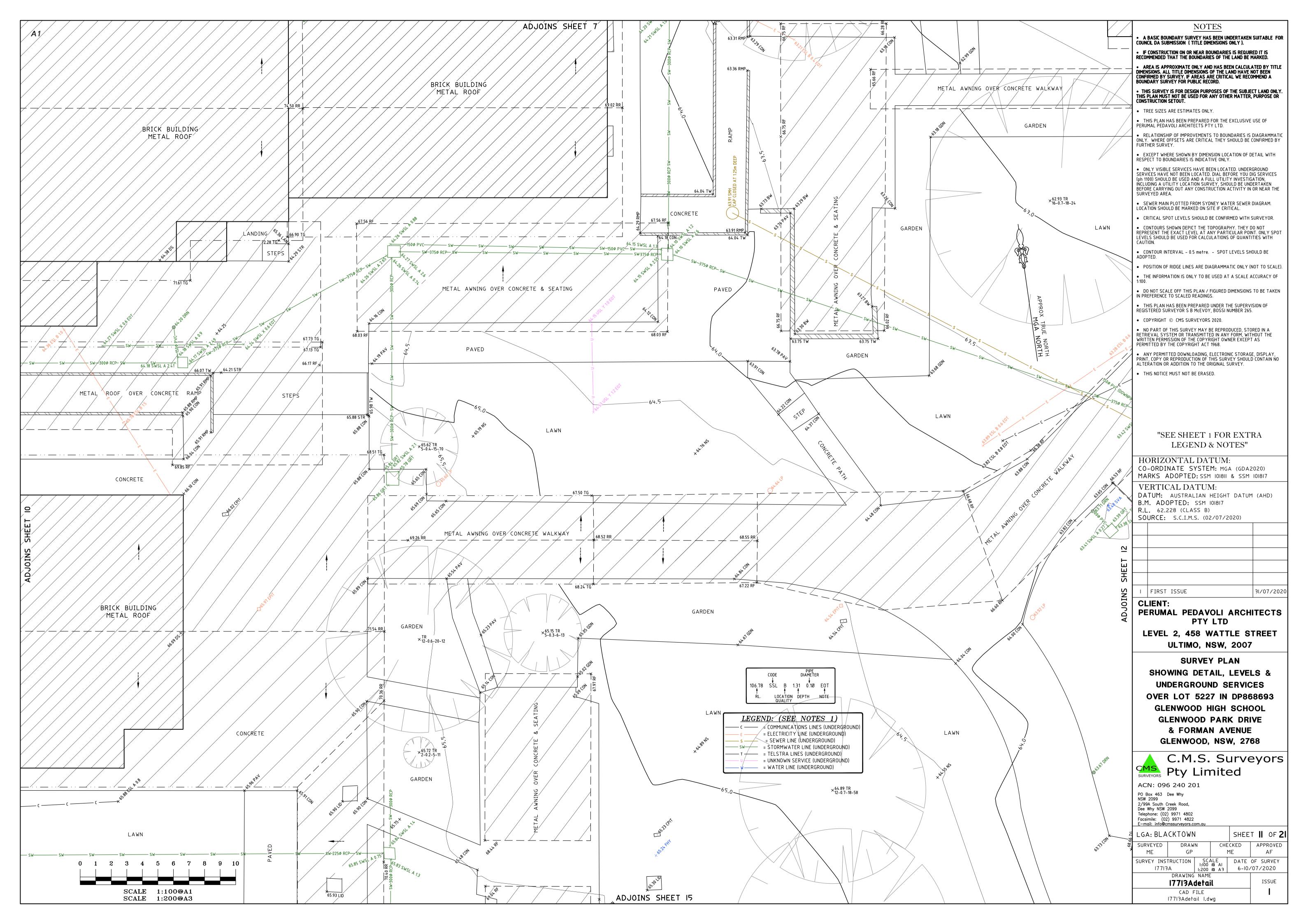


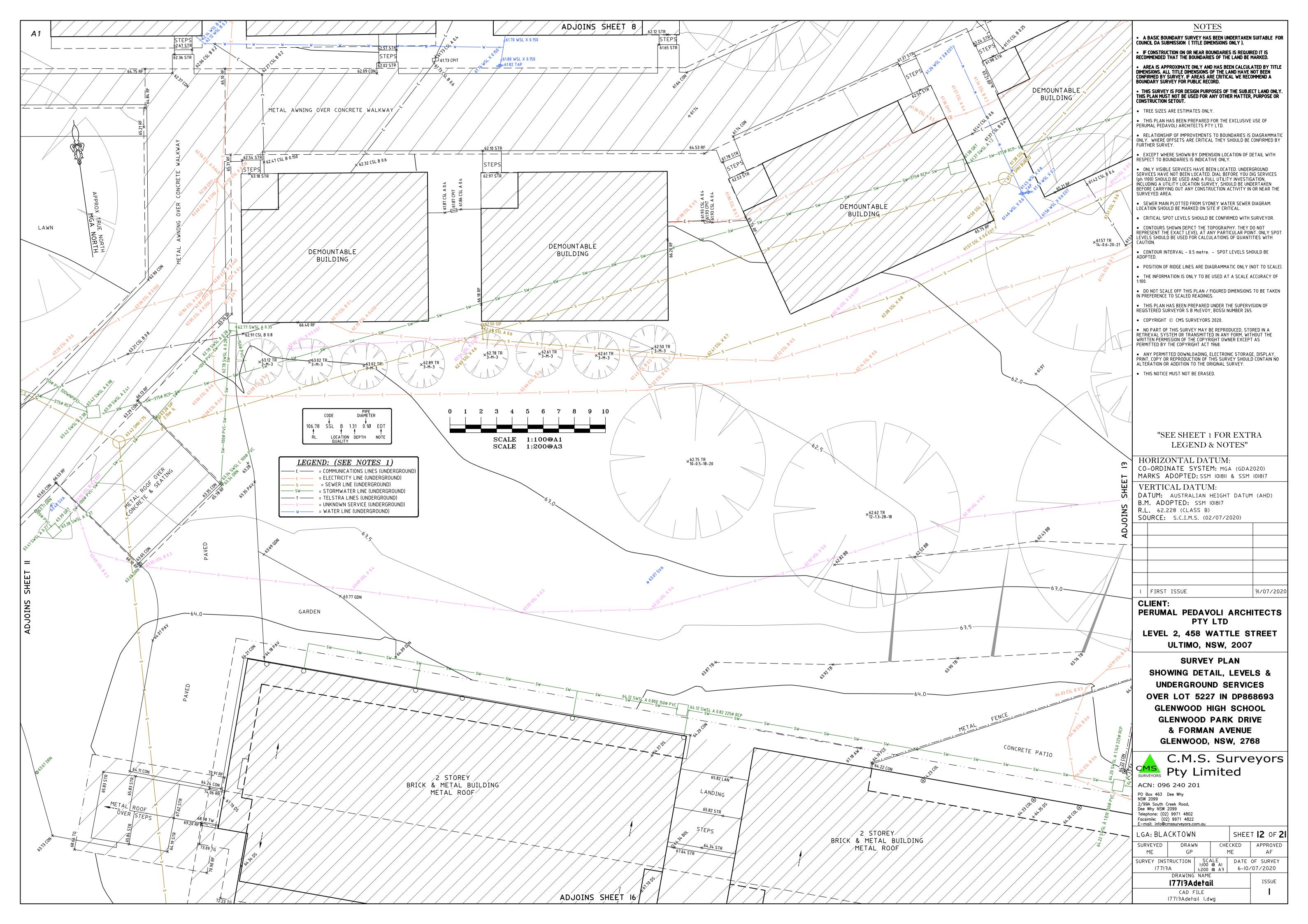


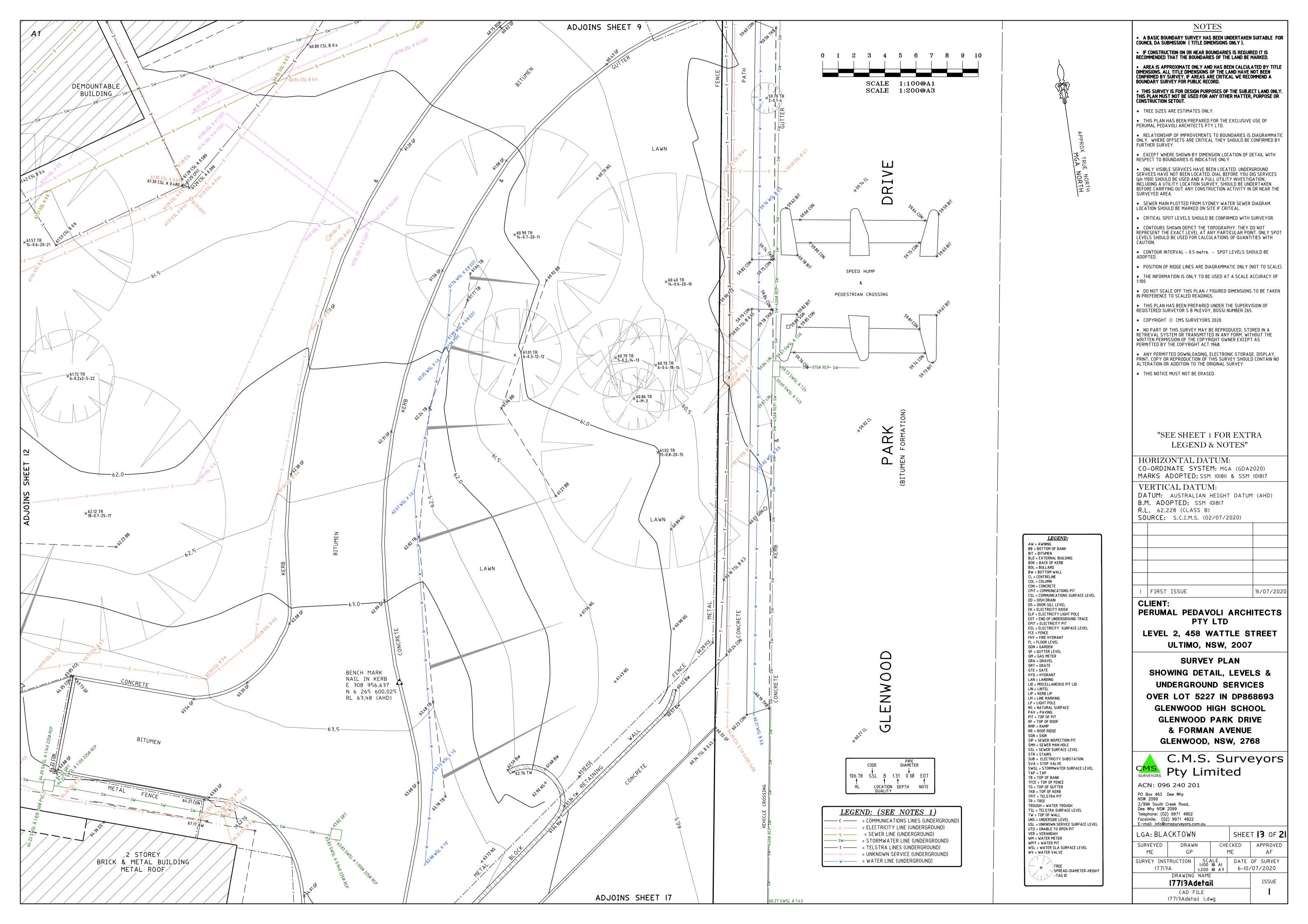


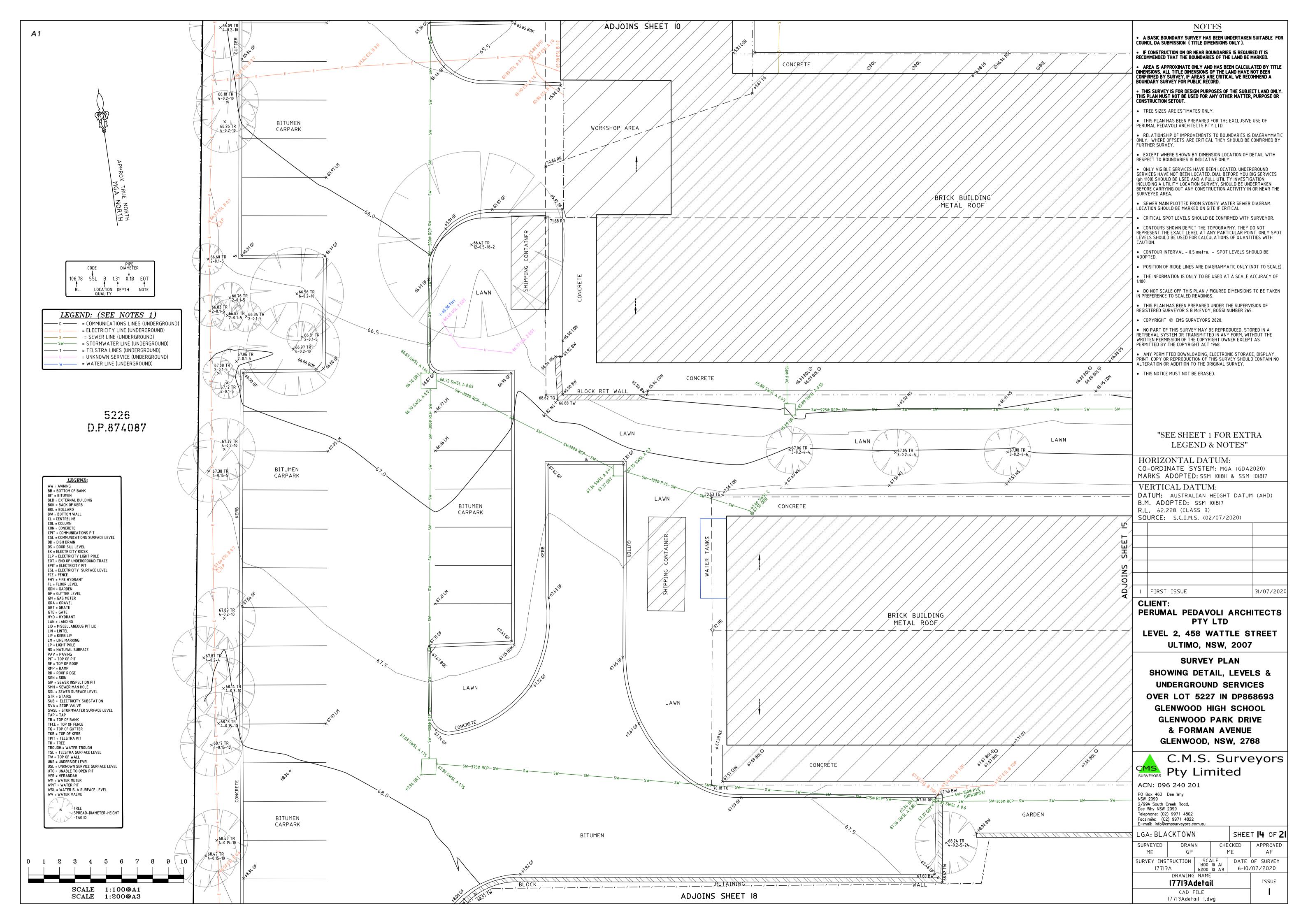


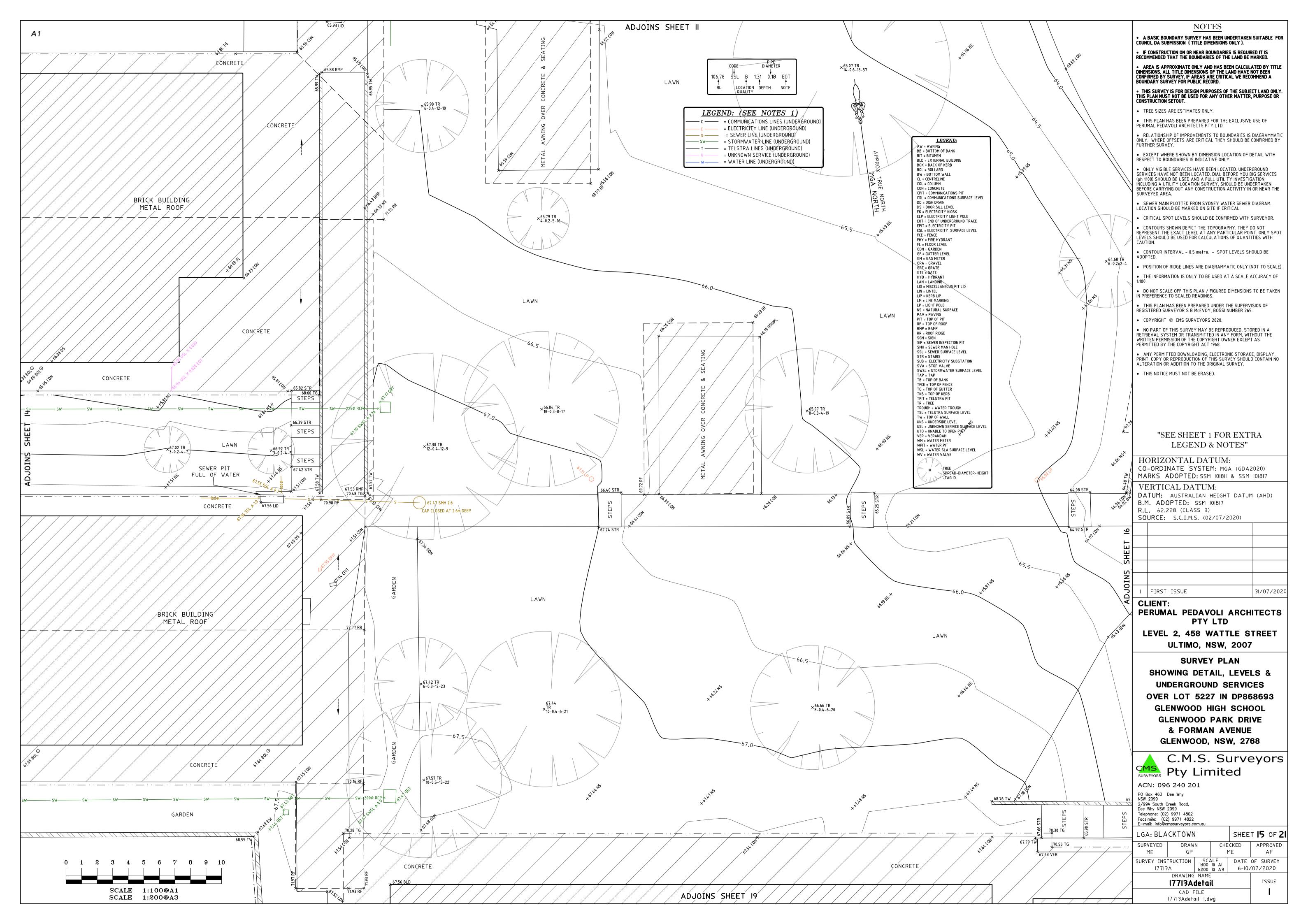




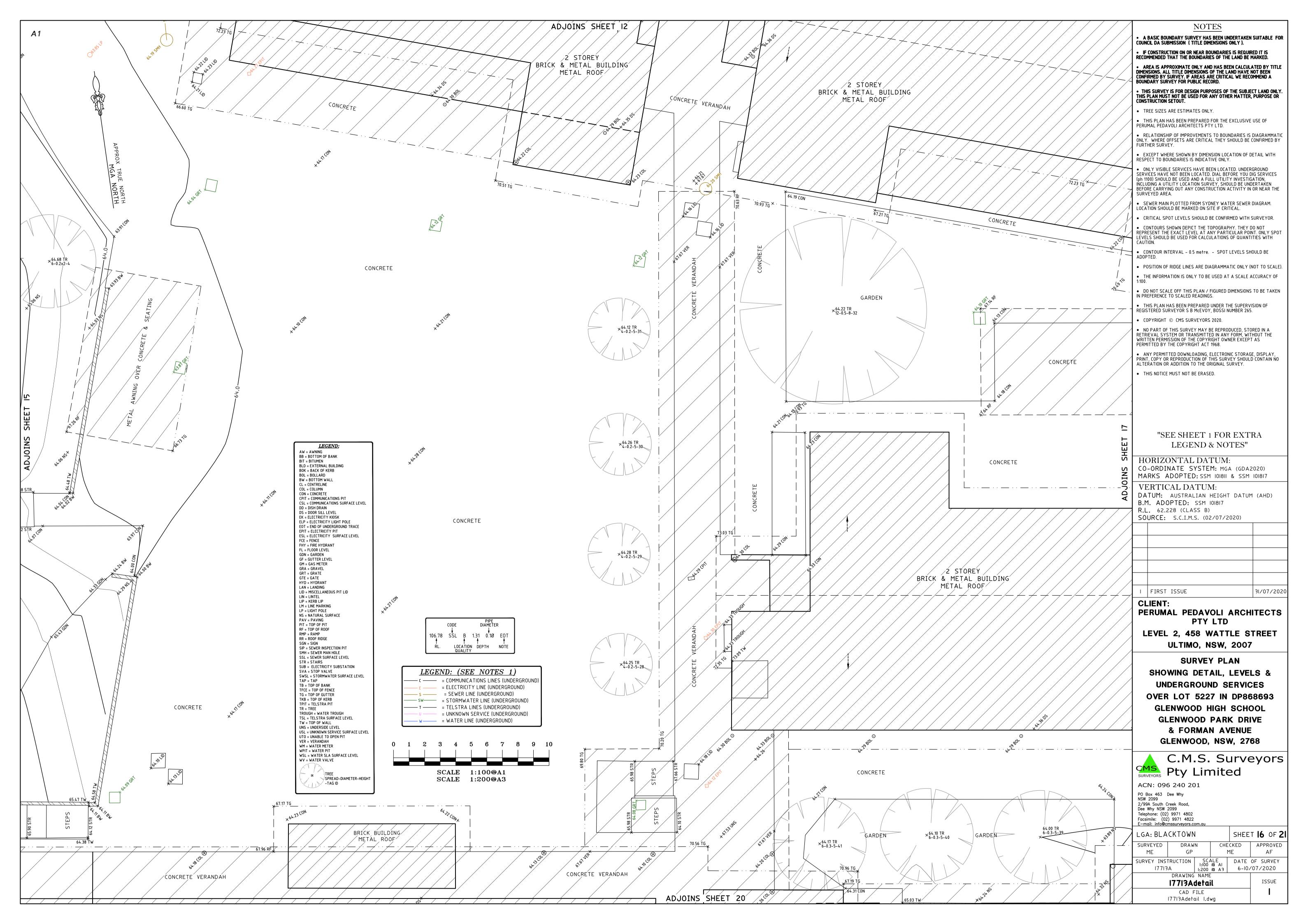


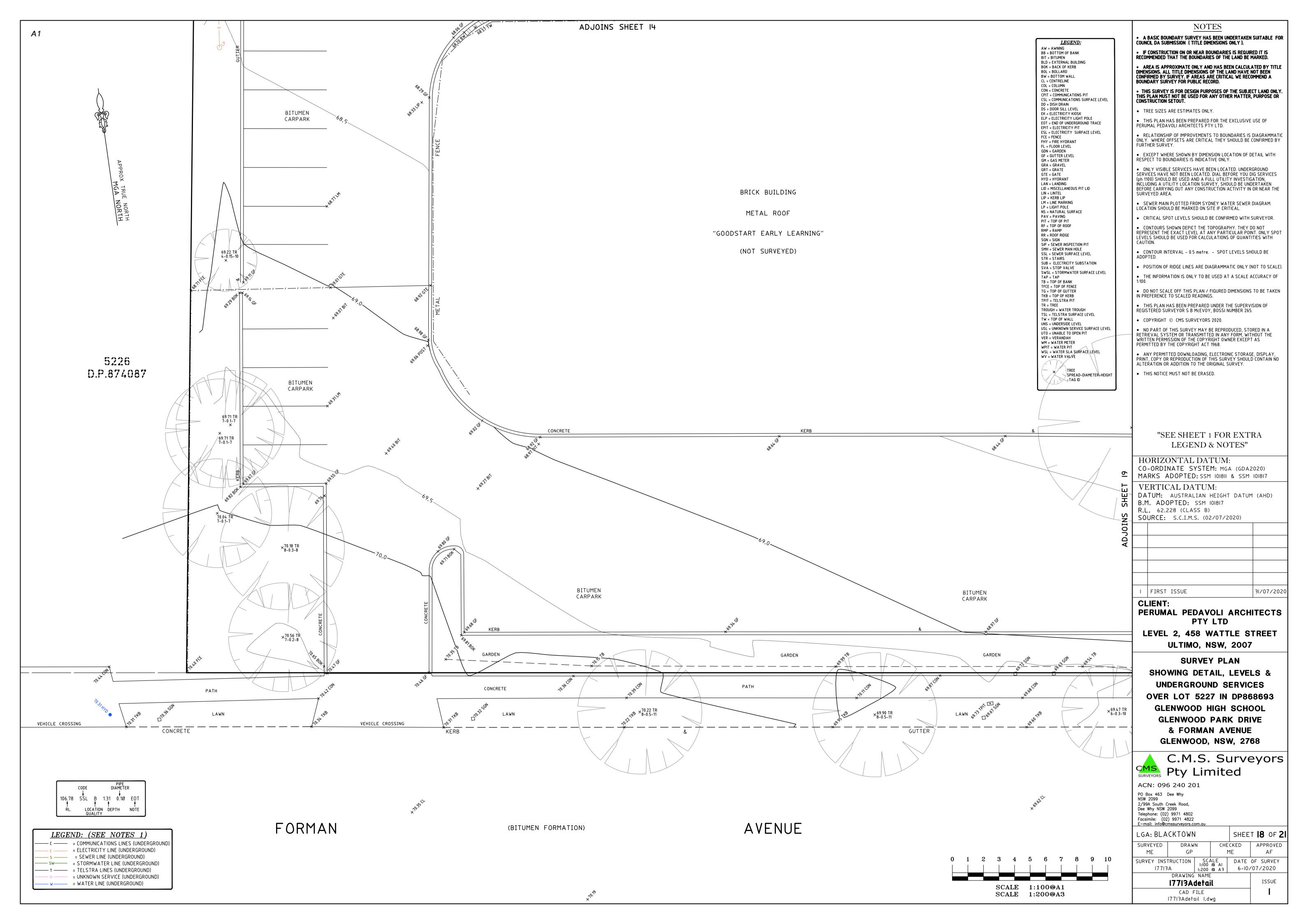


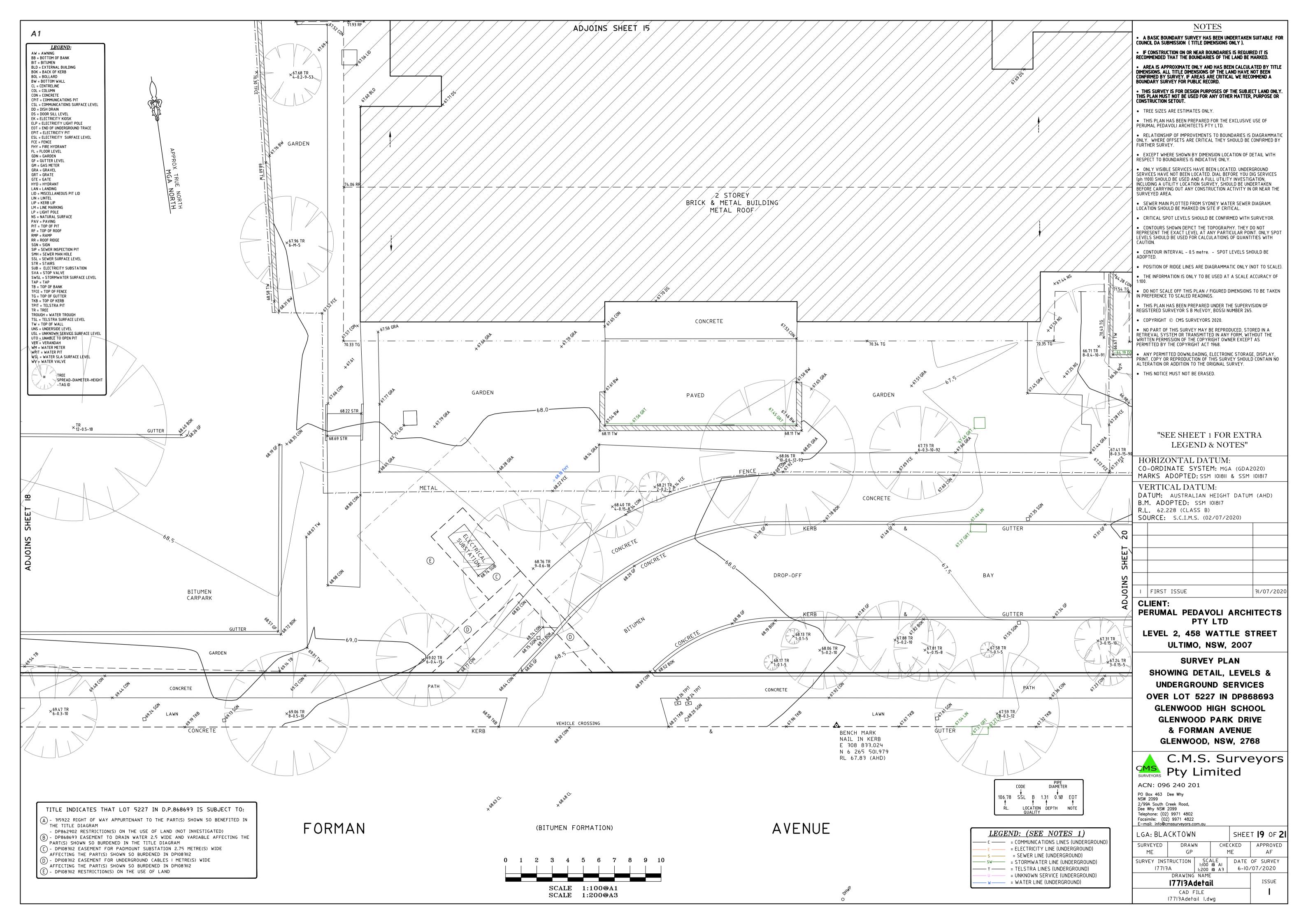


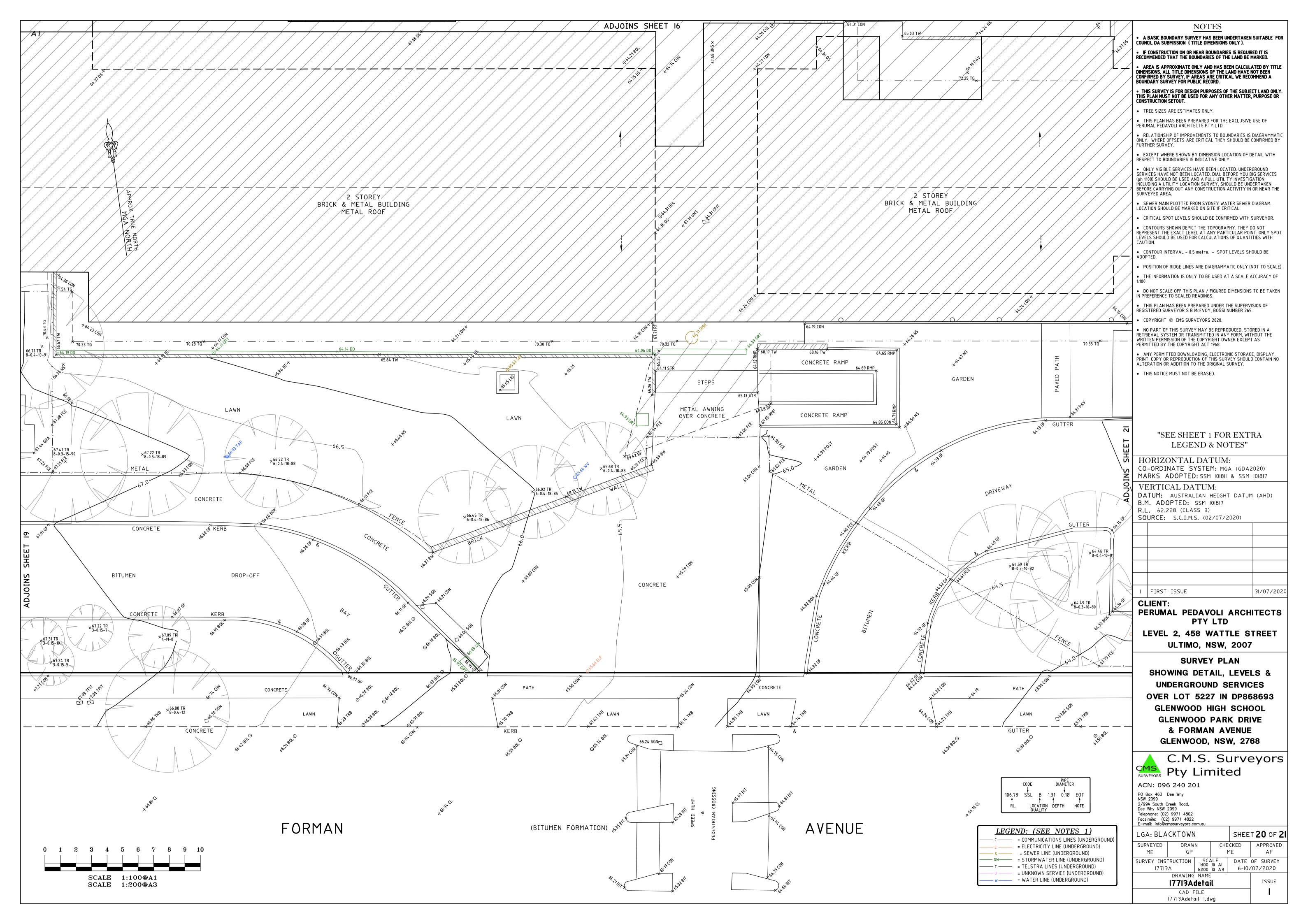


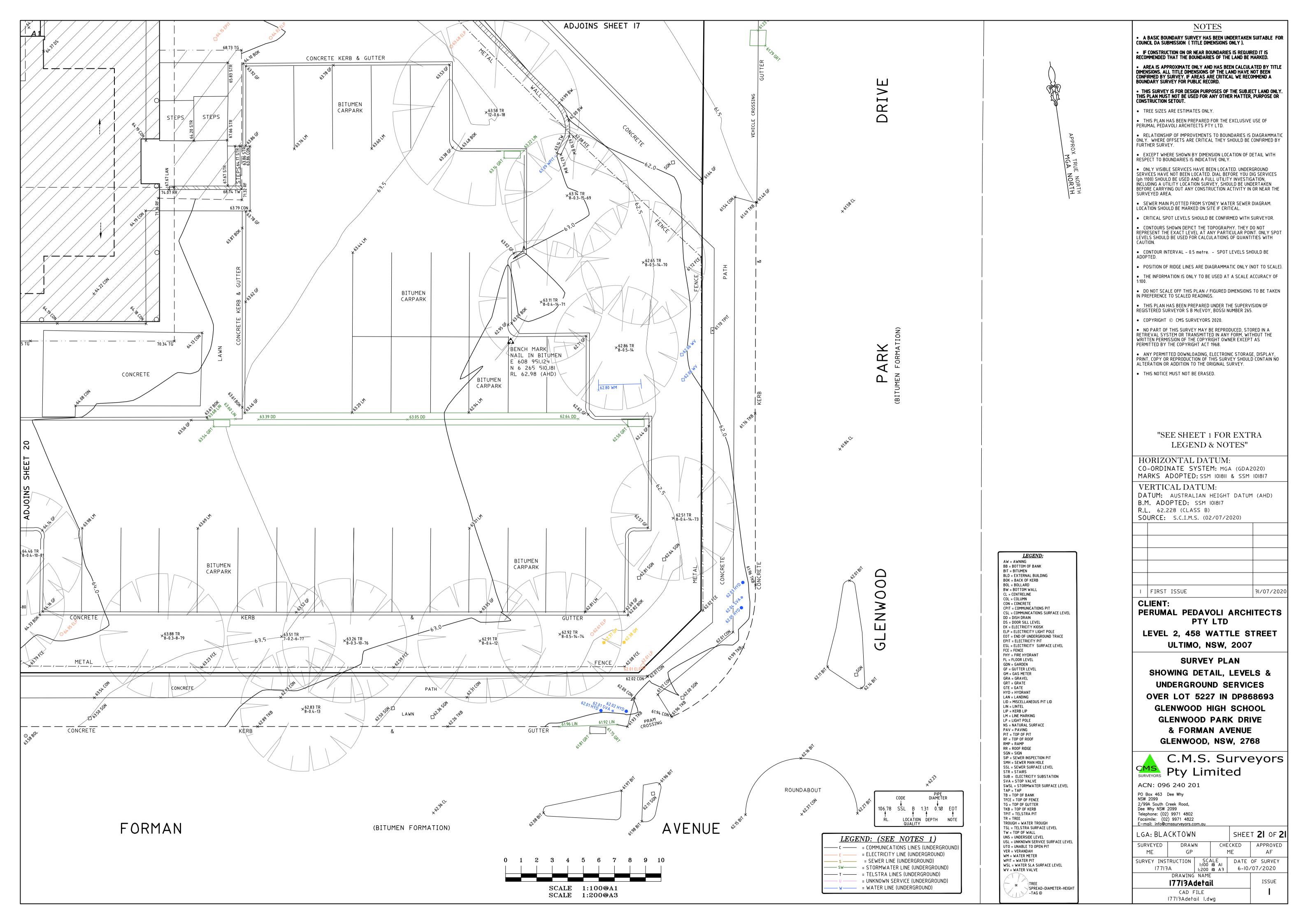














## APPENDIX B

# BCC FLOOD ADVICE LETTER



File no: 326021

1 September 2021

**Boris Santana** 

**Dear Boris** 

#### 85 Forman Avenue Glenwood being Lot 5227 in DP 868693

I refer to your email on 6th August 2021 regarding flood information for the above property.

#### Flood Assessment

Based on the latest information available to us from consultants and/or statutory authorities, there are no flood planning controls that apply to this property.

Council's position is that a Complying Development Certificate (CDC) is not available for this site.

#### Recommendations

Council's Asset Design section's only requirement relating to flooding or drainage would be for the habitable floor level to be a minimum of 225 mm above finished ground levels.

However, given the contour information available for the site, there is the possibility that the lot is subject to currently un-mapped overland flooding. To determine if it is, the following should be assessed by the applicant:

- 1. A development application must provide a detail survey to Australian Height Datum (AHD) and certified (signed) by a registered surveyor. The survey is to include:
  - Sufficient spot levels with contours
  - Any existing floor levels
  - The origin and level of the benchmark used and a local benchmark on top of kerb installed for use during construction.
- 2. The applicant needs to review site topography that directly concentrates flows to the site including trapped drainage low points.

- 3. Any future development within the overland flow area, if identified, would have to prove that it does not increase the flood risk to life or the surrounding area and it must maintain an appropriate overland flow path. This will require a Flood Study Report at your cost unless the future development is contained within the existing building footprint. Should you wish to proceed with a flood study please contact Council for specific flood modelling requirements.
- 4. Any future development must comply with general requirements for Flood Modelling as outlined in Blacktown City Council's Water Sensitive Urban Design (WSUD)

  Developer Handbook. Chapter 15.3: Design Standards outlines a number of different developments, and states minimum requirements with regards to flooding.

  This document is published on Blacktown City Council's website: https://www.blacktown.nsw.gov.au/Plan-build/Stage-2-plans-and-guidelines/Developers-toolkit-for-water-sensitive-urban-design-WSUD/MUSIC-modelling
- 5. Allow for the pipes adjacent to the site to be blocked in accordance with Chapter 15 of the WSUD Developer Handbook and include in a Flood Model of the property.
- 6. Submit to Council a copy of this Flood Advice Letter, the Flood Study Report and electronic files of the Flood Model with the Development Application (DA).
- Council can supply additional information, such as catchment plans and/or ALS/Lidar data for a fee. Contact floodadvice@blacktown.nsw.gov.au for this information.

#### **Background**

Please read this letter in conjunction with Attachment A, General Flood Information, and B Complying Development Information.

Please find attached a copy of an extract from Council's Geographical Information System (GIS) showing the approximate extent of Blacktown City's flood information in this area. Please note the disclaimer and the contour interval of 0.5 metres.

Council's flood mapping is available on Council's web site. To start go to <a href="http://maps.blacktown.nsw.gov.au">http://maps.blacktown.nsw.gov.au</a>. Council's flood mapping only covers the areas where it has information. A property that is not identified does not mean that there are no flood issues. It is the responsibility of the person enquiring to check the natural fall of the land and to ensure that the subject property is not affected by local stormwater overland flows that might affect existing or future development on this land.

The above comments are preliminary and not complete. A more detailed assessment at development application stage may result in modifications and/or additions to these comments (This advice is not a guarantee of approval).

However, please also be advised that there may be other non-flood related issues that might impact on the use of the land.

If you would like to discuss this matter further, please contact me on 9839 6384.

Yours faithfully

Philip Mcateer

Senior Stormwater Engineer

Encl. Attachment A & B, Flood map

#### Disclaimer:

The information contained in this letter is only valid on the date of issue. This letter has been prepared with all due care and in good faith using the best information available to us.

We provide no warranties in relation to the completeness or accuracy of the information contained in this letter, and do not accept liability for any loss or damage resulting from, or in connection with, its contents or its use.

#### Attachment A

#### **General Flood Information**

The flood levels supplied are for the pre-developed existing conditions

The flood levels supplied do not take climate change into consideration. These flood levels should not be used to set floor levels or to identify the extent of flooding over the property as our current flood models may not have included blockage factors nor changes in land-use and landform since the date of the study.

#### **Probable Maximum Flood**

This is the largest flood that could conceivably occur at a particular location. The PMF defines the extent of flood prone land that is, the floodplain. The risk of it happening in any one year is extremely rare and is considered to be between a 1 in 100,000 to a 1 in 10,000,000 chance.

#### 1% AEP Flood

This is a flood that has a probability of 1% (or a 1 in 100 chance) of occurring in any given year.

#### Flood risk precincts

Precincts have been defined based on hydraulic and survey information available to Council. In many cases a more definitive indication of flood risk precinct extents can be determined by relating surveyed ground levels at AHD to the relevant hydraulic and/or flood level criteria.

The Low Flood Risk Precinct is all land within the floodplain, i.e. within the extent of the Probable Maximum Flood (PMF) but not identified as either a high flood risk or medium flood risk precinct. Therefore, the Low Flood Risk Precinct is all the land between the 1% AEP and PMF flood extents.

The Medium Flood Risk Precinct is the land below the 1% AEP flood level subject to a low hydraulic hazard (in accordance with the provisional criteria outlined in the N.S.W. Government Floodplain Development Manual 2005).

The High Flood Risk Precinct is the land subject to a high hydraulic hazard (in accordance with the provisional criteria outlined in the N.S.W. Government Floodplain Development Manual 2005) in a 1% AEP flood event and/or subject to potential evacuation difficulties during a flood.

# "Development on Flood Prone Land" guidelines

Our guidelines can be found in "Blacktown Development Control Plan (DCP) 2015 Part A" publication. This publication is available on Council's Website "www.blacktown.nsw.gov.au". To find this document click on Planning and Development tab on the home page and then select Blacktown Development Control Plan 2015. This publication is currently under review in respect of floodplain planning issues.

# Council's flood mapping is available on Council's web site

To start click Discover Blacktown tab on the home page and then select Maps Online and follow the instructions. Council's flood mapping only covers the areas where it has information. A property that is not identified does not mean that there are no flood issues. It is the responsibility of the person enquiring to check the natural fall of the land and to ensure that the subject property is not affected by local stormwater overland flows that might affect existing or future development on this land.

#### Flood Planning Level

The Flood Planning Level is a special area of the Low Flood Risk Precinct that is within the 0.5m freeboard of the 1% AEP flood level.

# We are currently reviewing the flood modelling in this area

Finalisation of this modelling work may result in a variation to the quoted flood levels and amendments to our current flood mapping extents. The flood levels shown are under review and therefore are preliminary information only.

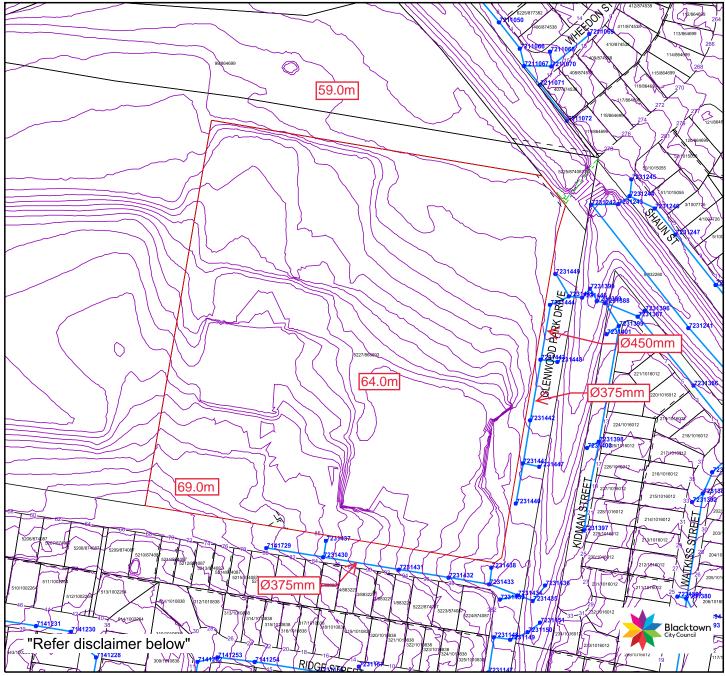
#### Attachment B Complying Development Information

The information you seek may vary between "complying development" i.e.: routine development which has minimal environmental impact, providing the design complies with each relevant development standard of the pertinent State Environmental Planning Policy (SEPP) or "local development" i.e.: development, including State significant development, which may have significant environmental impact and for which a Development Consent and Construction Certificate are required under the Environmental Planning and Assessment Act 1979.

The property is identified on Council's records as affected by "Local Runoff" according to the latest information available to Council. Local Runoff is considered to be the equivalent of a "flow path" for the purposes of State Environmental Planning Policy (Exempt and Complying Development Codes) 2008.

Where your enquiry relates to a proposed Complying Development Certificate (CDC), any flood control lot provisions of the relevant SEPP must be investigated.

The SEPP may provide that a CDC cannot be issued for any flood control lot until either Council or a professional engineer who specialises in hydraulic engineering (e.g.: a person holding the relevant accreditation under the NSW Building Professionals Board accreditation scheme) certifies that the land is not a flood storage area, a floodway area, a flow path, a high hazard area or a high-risk area.



Copyright NSW Land Information Centre 1996

Plot Date: 30/08/2021

Copyright Blacktown City Council

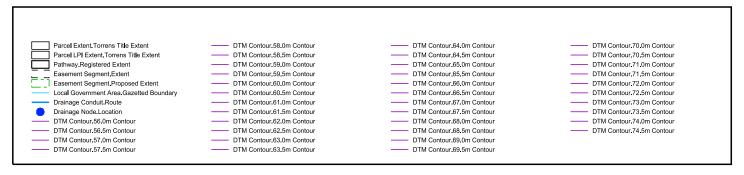
# BLACKTOWN CITY COUNCIL Flood Risk Map

This is Council's current flood map. However Council is currently reviewing the flood modelling in this area. Finalisation of this modelling work may result in a variation to the quoted flood levels and amendments to Council's current flood mapping extents.

DISCLAIMER: The flood risk precincts shown are based on information available to Council and should be regarded as an indicative guide only. A more accurate indication of the extent of the respective flood risk precincts can be determined by relating surveyed ground levels at Australian Height Datum (AHD) to the hydraulic and/or flood level criteria determining flood risk precinct boundaries. This information may be obtained by a written request to Council accompanied by a ground level survey to AHD prepared by a Registered Surveyor. Should flood risk precinct extents be required for the purpose of a financial transaction of any nature, then the parties to that transaction should apply to Council for formal certification and/or seek independent legal or professional advice.



Scale 1:3000



Plotted by: swadmin a4 bcic flood portait external.xml

# enstruct

# APPENDIX C

# EES ADVICE



DOC21/591115 SSD-23512960

Jenny Chu
Infrastructure Assessments
Planning and Assessment Group
Department of Planning, Industry and Environment
4 Parramatta Square, 12 Darcy Street
Parramatta NSW 2150

Dear Ms Chu,

#### Glenwood High School Upgrade (SSD-23512960)

I refer to the e-mail of 5 July 2021, requesting advice from Environment, Energy and Science Group (EES) in the Department of Planning, Industry and Environment on the Secretary's Environmental Assessment Requirements (SEARs) for the above.

EES has reviewed the 1 July 2021 request for SEARs and the draft SEARs and provides the following comments and attached recommended Environmental Assessment Requirements.

#### **Biodiversity**

EES recommends that key issue No.11 be amended to include the attached biodiversity requirements within the SEARs. Please note in relation to the 4<sup>th</sup> dot point, the minimum information and spatial data requirements are in Tables 24 and 25 of the Biodiversity Assessment Method (BAM), and as required more broadly by the revised BAM 2020. Other requirements, such as those relating to the BAM Calculator and Biodiversity Offsets and Agreements Management System (BOAMS), are detailed in various guidelines, practice notes, updates and other advices issued by EES to BAM accredited assessors (see: www.environment.nsw.gov.au/topics/animals-and-plants/biodiversity/accredited-assessors/assessor-resources).

#### Flooding

EES recommends that key issue No.16 be amended to include the attached flooding requirements within the SEARs.

Soil and Water

EES recommends that key issue No.17 be amended to include the attached soil and water requirements within the SEARs.

Should you have any queries on this advice please contact Richard Bonner, Senior Conservation Planning Officer, on 9995 6917 or richard.bonner@environment.nsw.gov.au

Yours sincerely

S. Harrison

16/07/21

Susan Harrison

Senior Team Leader Planning Greater Sydney Branch Biodiversity and Conservation Division

## Environment, Energy and Science Group recommended Environmental Assessment Requirements – Glenwood High School Upgrade (SSD-23512960)

#### **Biodiversity**

- Biodiversity impacts related to the proposed development are to be assessed in accordance with Section 7.9 of the *Biodiversity Conservation Act 2016*, the Biodiversity Assessment Method 2020 and documented in a Development Assessment Report (BDAR). The BDAR must include information in the form detailed in the *Biodiversity Conservation Act 2016* (s.6.12), *Biodiversity Conservation Regulation 2017* (s.6.8) and Biodiversity Assessment Method.
- The BDAR must document the application of the avoid, minimise and offset framework including assessing all direct, indirect and prescribed impacts in accordance with the Biodiversity Assessment Method 2020.
- The BDAR must include details of the measures proposed to address the offset obligation as follows:
  - a. The total number and classes of biodiversity credits required to be retired for the development;
  - b. The number and classes of like-for-like biodiversity credits proposed to be retired;
  - c. The number and classes of biodiversity credits proposed to be retired in accordance with the variation rules:
  - d. Any proposal to fund a biodiversity conservation action;
  - e. Any proposal to make a payment to the Biodiversity Conservation Fund.

If seeking approval to use the variation rules, the BDAR must contain details of the reasonable steps that have been taken to obtain requisite like-for-like biodiversity credits.

- The BDAR must be submitted with all spatial data associated with the survey and assessment as per Appendix 11 of the BAM.
- The BDAR must be prepared by a person accredited in accordance with the *Accreditation* Scheme for the Application of the Biodiversity Assessment Method Order 2017 under s.6.10 of the Biodiversity Conservation Act 2016.

#### **Flooding**

- The EIS must map the following features relevant to flooding as described in the *Floodplain Development Manual 2005* (NSW Government 2005) including:
  - a. Flood prone land.
  - b. Flood planning area, the area below the flood planning level.
  - c. Hydraulic categorisation (floodways and flood storage areas).
  - d. Flood Hazard.
- The EIS must describe flood assessment and modelling undertaken in determining the design flood levels for events, including a minimum of the 5% Annual Exceedance Probability (AEP), 1% AEP, flood levels and the probable maximum flood, or an equivalent extreme event.
- The EIS must model the effect of the proposed development (including fill) on the flood behaviour under current flood behaviour for a range of design events as identified above. This includes the 0.5% and 0.2% AEP year flood events as proxies for assessing sensitivity to an increase in rainfall intensity of flood producing rainfall events due to climate change.

#### **Flooding**

- Modelling in the EIS must consider and document:
  - a. Existing council flood studies in the area and examine consistency to the flood behaviour documented in these studies.
  - b. The impact on existing flood behaviour for a full range of flood events including up to the probable maximum flood, or an equivalent extreme flood.
  - c. Impacts of the development on flood behaviour resulting in detrimental changes in potential flood affection of other developments or land. This may include redirection of flow, flow velocities, flood levels, hazard categories and hydraulic categories
  - d. Relevant provisions of the NSW Floodplain Development Manual 2005.
- The EIS must assess the impacts on the proposed development on flood behaviour, including:
  - a. Whether there will be detrimental increases in the potential flood affectation of other properties, assets and infrastructure.
  - b. Consistency with Council floodplain risk management plans.
  - c. Consistency with any Rural Floodplain Management Plans.
  - d. Compatibility with the flood hazard of the land.
  - e. Compatibility with the hydraulic functions of flow conveyance in floodways and storage in flood storage areas of the land.
  - f. Whether there will be adverse effect to beneficial inundation of the floodplain environment, on, adjacent to or downstream of the site.
  - g. Whether there will be direct or indirect increase in erosion, siltation, destruction of riparian vegetation or a reduction in the stability of river banks or watercourses.
  - h. Any impacts the development may have upon existing community emergency management arrangements for flooding. These matters are to be discussed with the NSW SES and Council.
  - i. Whether the proposal incorporates specific measures to manage risk to life from flood. These matters are to be discussed with the NSW SES and Council.
  - j. Emergency management, evacuation and access, and contingency measures for the development considering the full range or flood risk (based upon the probable maximum flood or an equivalent extreme flood event). These matters are to be discussed with and have the support of Council and the NSW SES.
  - k. Any impacts the development may have on the social and economic costs to the community as consequence of flooding.

#### Soil and Water

- The EIS must map the following features relevant to water and soils including:
  - a. Acid sulfate soils (Class 1, 2, 3 or 4 on the Acid Sulfate Soil Planning Map).
  - b. Rivers, streams, wetlands, estuaries (as described in s.4.2 of the Biodiversity Assessment Method).
  - c. Wetlands as described in s.4.2 of the Biodiversity Assessment Method.
  - d. Groundwater.
  - e. Groundwater dependent ecosystems.
  - f. Proposed intake and discharge locations.
- The EIS must describe background conditions for any water resource likely to be affected by the development, including:
  - a. Existing surface and groundwater.
  - b. Hydrology, including volume, frequency and quality of discharges at proposed intake and discharge locations.
  - c. Water Quality Objectives (as endorsed by the NSW Government <a href="http://www.environment.nsw.gov.au/ieo/index.htm">http://www.environment.nsw.gov.au/ieo/index.htm</a>) including groundwater as appropriate that represent the community's uses and values for the receiving waters.
  - d. Indicators and trigger values/criteria for the environmental values identified at (c) in accordance with the ANZECC (2000) Guidelines for Fresh and Marine Water Quality and/or local objectives, criteria or targets endorsed by the NSW Government.
  - e. The Risk-based Framework for Considering Waterway Health Outcomes in Strategic Land-use Planning Decisions (OEH, 2017).
- The EIS must assess the impacts of the development on water quality, including:
  - a. The nature and degree of impact on receiving waters for both surface and groundwater, demonstrating how the development protects the Water Quality Objectives where they are currently being achieved, and contributes towards achievement of the Water Quality Objectives over time where they are currently not being achieved. This should include an assessment of the mitigating effects of proposed stormwater and wastewater management during and after construction.
  - b. Identification of proposed monitoring of water quality.
  - c. Consistency with any relevant certified Coastal Management Program (or Coastal Zone Management Plan).
- The EIS must assess the impact of the development on hydrology, including:
  - a. Water balance including quantity, quality and source.
  - b. Effects to downstream rivers, wetlands, estuaries, marine waters and floodplain areas.
  - c. Effects to downstream water-dependent fauna and flora including groundwater dependent ecosystems.
  - d. Impacts to natural processes and functions within rivers, wetlands, estuaries and floodplains that affect river system and landscape health such as nutrient flow, aquatic connectivity and access to habitat for spawning and refuge (e.g. river benches).
  - e. Changes to environmental water availability, both regulated/licensed and unregulated/rules-based sources of such water.
  - f. Mitigating effects of proposed stormwater and wastewater management during and after construction on hydrological attributes such as volumes, flow rates, management methods and re-use options.
  - g. Identification of proposed monitoring of hydrological attributes.

#### (END OF SUBMISSION)

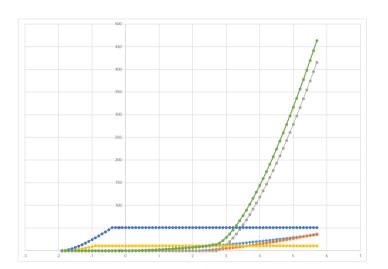


### APPENDIX D

GLENWOOD LAKE OUTLET STRUCTURE FLOW CALCULATIONS

Low Flow Weir			н	igh Flow Weir			Road				Throug	ah the Culvert 1.5x0.9	9					Through the culvert	3x1.5			
RL (m) H (m)	Q	(m3/s)		L (m) H (	m) Q (m	13/s)	RL (m) H	Q	(m3/s)	RL	н	,	Q	2 x culverts (L/s)	20	(m3/s)	RL	Culvert Depth (m)		(I/s) 2 x culverts	(L/s) 2C	2 (m3/s)
54	0			56	0	0	56.7	0	0		52.2		0	0	0	0		52.1	0	0	0	0
54.1	0.1	0.082535447		56.1	0.1	0.165070894	56.8	0.1	2.529822128		52.3		0.1	198.1	396.2	0.3962		52.2	0.1	404.4	808.8	0.8088
54.2	0.2	0.233445497		56.2	0.2	0.466890994	56.9	0.2	7.155417528		52.4		0.2	590.8	1181.6	1.1816		52.3	0.2	1227.2	2454.4	2.4544
54.3	0.3	0.428866763		56.3	0.3	0.857733525	57	0.3	13.14534138		52.5		0.3	1100.4	2200.8	2.2008		52.4	0.3	2322.2	4644.4	4.6444
54.4	0.4	0.660283575		56.4	0.4	1.320567151	57.1	0.4	20.23857703		52.6		0.4	1694.5	3389	3.389		52.5	0.4	3628.4	7256.8	7.2568
54.5	0.5	0.922774349		56.5	0.5	1.845548699	57.2	0.5	28.28427125		52.7		0.5	2353.5	4707	4.707		52.6	0.5	5107	10214	10.214
54.6	0.6	1.213018384		56.6	0.6	2.426036768	57.3	0.6	37.18064012		52.8		0.6	3064.2	6128.4	6.1284		52.7	0.6	6731.5	13463	13.463
54.7	0.7	1.528577868		56.7	0.7	3.057155737	57.4	0.7	46.85296149		52.9		0.7	3817.2	7634.4	7.6344		52.8	0.7	8481.3	16962.6	16.9626
54.8	0.8	1.867563975		56.8	0.8	3.73512795	57.5	0.8	57.24334022		53		8.0	4605.2	9210.4	9.2104		52.9	0.8	10340.7	20681.4	20.6814
54.9	0.9	2.228457067		56.9	0.9	4.456914134	57.6	0.9	68.30519746		53.1		0.9	5422.7	10845.4	10.8454		53	0.9	12296.6	24593.2	24.5932
55	1	2.61		57	1	5.22	57.7	1	80		53.2		1	5422.7	10845.4	10.8454		53.1	1	14338.4	28676.8	28.6768
55.1	1.1	3.011130203		57.1	1.1	6.022260406	57.8	1.1	92.29517864				Usi	ing Mannings of 0.2				53.2	1.1	16457.2	32914.4	32.9144
55.2	1.2	3.4309341		57.2	1.2	6.8618682	57.9	1.2	105.162731									53.3	1.2	18645.4	37290.8	37.2908
55.3	1.3	3.868615217		57.3	1.3	7.737230435	58	1.3	118.5782442									53.4	1.3	20896.5	41793	41.793
55.4	1.4	4.323471105		57.4	1.4	8.646942211		1.4	132.5201871									53.5		23204.7	46409.4	46.4094
55.5	1.5	4.794876171		57.5	1.5	9.589752343		1.5	146.9693846									53.6	1.5	25565.1	51130.2	51.1302
55.6	1.6	5.282268604		57.6	1.6	10.56453721		1.6	161.9086162										U	sing Mannings of 0.2		
55.7	1.7	5.785140214		57.7	1.7	11.57028043		1.7	177.3223054													
55.8	1.8	6.303028415		57.8	1.8	12.60605683		1.8	193.1962733													
55.9	1.9	6.835509776 H	FW	57.9	1.9	13.67101955		1.9	209.517541													
56	2	7.382194796	0	58	2	14.76438959		2	226.27417													
56.1	2.1	7.942723595	0.165070894		2.1	15.88544719		2.1	243.4551293													
56.2	2.2	8.516762343	0.466890994		2.2	17.03352469		2.2	261.0501867													
56.3		9.104000258	0.857733525		2.3	18.20800052		2.3	279.0498163													
56.4	2.4	9.704147072	1.320567151		2.4	19.40829414		2.4	297.445121													
56.5	2.5	10.31693087	1.845548699		2.5	20.63386173		2.5	316.227766													
56.6	2.6	10.94209622	2.426036768 R		2.6	21.88419243		2.6	335.3899223													
56.7	2.7	11.57940259	3.057155737	0	2.7	23.15880518		2.7	354.9242173													
56.8	2.8	12.22862295	3.73512795	3.399448485	2.8	24.4572459		2.8	374.8236919													
56.9	2.9	12.88954254	4.456914134	9.615092303	2.9	25.77908508		2.9	395.0817637													
57	3	13.56195782	5.22	17.66405248	3	27.12391565		3	415.6921938													
57.1	3.1	14.24567552	6.022260406	27.19558788	3.1	28.49135105																
57.2	3.2	14.9405118	6.8618682	38.00698949	3.2	29.8810236																
57.3	3.3	15.6462915	7.737230435	49.96148517	3.3	31.292583																
57.4	3.4	16.3628475	8.646942211	62.958667	3.4	32.72569501																
57.5	3.5	17.09002011	9.589752343	76.92073843	3.5	34.18004023																
57.6	3.6	17.82765654	10.56453721	91.78510909	3.6	35.65531307																
57.7	3.7	18.57561039	11.57028043	107.5	3.7	37.15122078																
57.8	3.8	19.33374126	12.60605683	124.0216463	3.8	38.66748252																
57.9	3.9	20.10191433	13.67101955	141.3124198	3.9	40.20382867																
58	4	20.88	14.76438959	159.3395157	4	41.76																

RL (m)	H (m)			☐ High Flow	Q Roa		9 culverts	Q 3x1.5 culverts	TOTAL
	52.1	-1.9	0		0	0		0 0	0
	52.2	-1.8	0		0	0		0.8088	0
	52.3	-1.7	0		0	0	0.396		0
	52.4	-1.6	0		0	0	1.181		0
	52.5	-1.5	0		0	0	2.200		0
	52.6	-1.4	0		0	0	3.38		0
	52.7	-1.3	0		0	0	4.70		0
	52.8	-1.2	0		0	0	6.128		0
	52.9	-1.1	0		0	0	7.634		0
	53	-1	0		0	0	9.210		0
	53.1	-0.9	0		0	0	10.845		0
	53.2	-0.8	0		0	0	10.845		0
	53.3	-0.7	0		0	0	10.845		0
	53.4	-0.6	0		0	0	10.845		0
	53.5 53.6	-0.5 -0.4	0		0	0	10.845		0
			0		0		10.845		0
	53.7	-0.3				0	10.845		
	53.8	-0.2	0		0	0	10.845		0
	53.9	-0.1	0		0	0	10.845		0
	54	0			0	0	10.845		0.000505447
	54.1	0.1	0.082535447		0	0	10.845		0.082535447
	54.2	0.2	0.233445497		0	0	10.845		0.233445497
	54.3	0.3	0.428866763		0	0	10.845		0.428866763
	54.4	0.4	0.660283575		0	0	10.845		0.660283575
	54.5	0.5	0.922774349		0	0	10.845		0.922774349
	54.6	0.6	1.213018384		0	0	10.845		1.213018384
	54.7	0.7	1.528577868		0	0	10.845		1.528577868
	54.8	0.8	1.867563975		0	0	10.845		1.867563975
	54.9	0.9	2.228457067		0	0	10.845		2.228457067
	55	. 1	2.61		0	0	10.845		2.61
	55.1	1.1	3.011130203		0	0	10.845		3.011130203
	55.2	1.2	3.4309341		0	0	10.845		3.4309341
	55.3	1.3	3.868615217		0	0	10.845		3.868615217
	55.4	1.4	4.323471105		0	0	10.845		4.323471105
	55.5	1.5	4.794876171		0	0	10.845		4.794876171
	55.6	1.6	5.282268604		0	0	10.845		5.282268604
	55.7	1.7	5.785140214		0	0	10.845		5.785140214
	55.8	1.8	6.303028415		0	0	10.845		6.303028415
	55.9	1.9	6.835509776		0	0	10.845		6.835509776
	56	2	7.382194796		0	0	10.845		7.382194796
	56.1	2.1	7.942723595	0.1650708		0	10.845		8.107794488
	56.2	2.2	8.516762343	0.4668909		0	10.845		8.983653336
	56.3	2.3	9.104000258	0.857733		0	10.845		9.961733783
	56.4	2.4	9.704147072	1.320567		0	10.845		11.02471422
	56.5	2.5	10.31693087	1.8455486		0	10.845		12.16247957
	56.6	2.6	10.94209622	2.426036		0	10.845		13.27143677
	56.7	2.7	11.57940259	3.057155		0	10.845		13.90255574
	56.8	2.8	12.22862295	3.73512		529822128	10.845		17.11035008
	56.9	2.9	12.88954254	4.456914		155417528	10.845		22.45773166
	57	3	13.56195782			3.14534138	10.845		29.2107413
	57.1	3.1	14.24567552	6.0222604		0.23857703	10.845		37.10623743
	57.2	3.2	14.9405118	6.86186		3.28427125	10.845		45.99153945
	57.3	3.3	15.6462915	7.737230		7.18064012	10.845		55.76327056
	57.4	3.4	16.3628475	8.6469422		5.85296149	10.845		66.345303
	57.5	3.5	17.09002011	9.5897523		.24334022	10.845		77.6784925
	57.6	3.6	17.82765654	10.56453		3.30519746	10.845		89.7151346
	57.7	3.7	18.57561039	11.570280		80	10.845		102.415680
	57.8	3.8	19.33374126	12.606056		2.29517864	10.845		115.746635
	57.9	3.9	20.10191433	13.671019		05.162731	10.845		129.679150
	58	4	20.88	14.764389		18.5782442	10.845		144.188033
	58.1	4.1	21.66787355	15.88544		32.5201871	10.845		159.251034
	58.2	4.2	22.46541486	17.033524	469 1	16.9693846	10.845		174.848309
	58.3	4.3	23.27250813	18.208000		1.9086162	10.845		190.962016
	58.4	4.4	24.08904162	19.408294	414 1	77.3223054	10.845	4 51.1302	207.575999
	58.5	4.5	24.91490744	20.63386		3.1962733	10.845		224.67553
	58.6	4.6	25.75000127	21.884192		209.517541	10.845		242.247133
	58.7	4.7	26.59422227	23.15880		226.27417	10.845		260.278375
	58.8	4.8	27.4474728	24.4572		13.4551293	10.845		278.757775
	58.9	4.9	28.3096583	25.77908!		1.0501867	10.845		297.674671
	59	5	29.18068711	27.123915		79.0498163	10.845		317.01913
	59.1	5.1	30.06047034	28.49135		97.445121	10.845	4 51.1302	336.78187
	59.2	5.2	30.94892174	29.88102	236	316.227766	10.845	4 51.1302	356.954189
	59.3	5.3	31.84595754	31.292		35.3899223	10.845		377.527905
	59.4	5.4	32.75149637	32.725695		54.9242173	10.845		398.4953123
	59.5	5.5	33.66545912	34.180040	023 3	4.8236919	10.845		419.849132
	59.6	5.6	34.58776884	35.655313		5.0817637	10.845		441.582476
	59.7	5.7	35.51835068	37.151220		15.6921938	10.845		463.6888146
	3711								



# enstruct

## APPENDIX E

# PMP CALCULATION

#### PMF ESTIMATION CALCULATION WORKSHEET

### Refer to Commonwealth Buraeu of Meteorology The Estimation of Probable Maximum Precipitation in Australia: Generalised Short-Duration Method Report for complete method.

PROJECT: MODELLER: DATE:	6393 NKK 27/09/2021		,,
Site Area:	1 km2	m2	
Type of generalised method:	GSAM Coastal Zone	Refer Figure 1	
Duration Limit:	6	Refer Figure 2	

Terrain Category: Rough

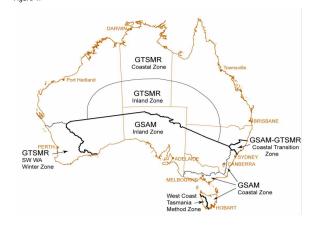
Elevation Adjustment Factor: Elevation <= 1500m Moisture Adjustment Factor: 0.7 Refer Figure 3

PMP Value:

Where;

S = smooth % of catchment area
Ds = initial rainfal depth for the smooth terrain categories
R = rough % of catchment area
Dr = initial rainfal depth for the rough terrain categories
MAF = moistuer adjustment factor
EAF = Elevation adjustment factor

Storm Event 0.25hr 0.5hr 0.75hr 1hr 1.5hr 2hr 2.5hr 3hr 4hr 5hr PMP 171.5 245 308 357 458.5 539 595 656.6 745.5 823.9 870.1



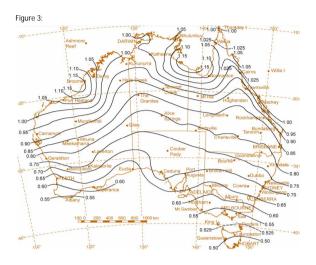


Figure 2: INTERMEDIATE ZONE

Limits 1000 km² 3 hours 1000 km² 6 hours

Depth Duration Area Curve												RAINFALL PER	STORM EVENT	
Refer t	o Figure 4 in BON	A PMF Report	15 MIN STOR	M EVENT				30 MIN STORM	1 EVENT				45 MIN STOR	RM EVEN
Storm Event		units			0.	25hr				0.	50hr			
0.25hr	245	245 mm			m	ım	mm/hr			m	m	mm/hr		
0.5hr	350	350 mm	0	0	0	0	0	0	0	0	0	0	0	0
0.75hr	440	440 mm	0.75	4	4	6.8	544	1.5	4	4	10	400	2.25	4
1hr	510	510 mm	1.5	10	6	10.2	816	3	10	6	15	600	4.5	10
.5hr	655	570 mm	2.25	18	8	13.6		4.5	18	8	20	800	6.75	18
thr .	770	647 mm	3	25	7	11.9	952	6	25	7	17.5	700	9	25
.5hr	850	690 mm	3.75	32	7	11.9		7.5	32	7	17.5	700	11.25	32
3hr	938	727 mm	4.5	39	7	11.9		9	39	7	17.5	700	13.5	39
4hr	1065	793 mm	5.25	46	7	11.9	952	10.5	46	7	17.5	700	15.75	46
ihr	1177	855 mm	6	52	6	10.2	816	12	52	6	15	600	18	52
Shr	1243	900 mm	6.75	59	7	11.9	952	13.5	59	7	17.5	700	20.25	59
			7.5	64	5	8.5	680	15	64	5	12.5	500	22.5	64
	PMP Value	e	8.25	70	6	10.2	816	16.5	70	6	15	600	24.75	70
Storm Event	PMP Neare	est 10mm	9	75	5	8.5	680	18	75	5	12.5	500	27	75
25hr	171.5	170	9.75	80	5	8.5	680	19.5	80	5	12.5	500	29.25	80
5hr	245	250	10.5	85	5	8.5	680	21	85	5	12.5	500	31.5	85
75hr	308	310	11.25	89	4	6.8	544	22.5	89	4	10	400	33.75	89
nr	357	360	12	92	3	5.1	408	24	92	3	7.5	300	36	92
ihr	458.5	460	12.75	95	3	5.1	408	25.5	95	3	7.5	300	38.25	95
thr .	539	540	13.5	97	2	3.4	272	27	97	2	5	200	40.5	97
.5hr	595	600	14.25	99	2	3.4	272	28.5	99	2	5	200	42.75	99
hr	656.6	660	15	100	1	1.7	136	30	100	1	2.5	100	45	100
hr	745.5	750				170					250			
Shr	823.9	820												
hr	870.1	870	1.5HR STORN	I EVENT				2HR STORM EV	/ENT				2.5HR STOR	M EVENT
					1.	5hr				2h	nr			
					m	ım	mm/hr			m	m	mm/hr		
			0	0	0	0	0	0	0	0	0	0	0	0
			4.5	4	4		245.3333	6	4	4	21.6	216	7.5	4
			9	10	6	27.6	368	12	10	6	32.4	324	15	10
			13.5	18	8	36.8	490.6667	18	18	8	43.2	432	22.5	18
			18	25	7		429.3333	24	25	7	37.8	378	30	25
			22.5	32	7	32.2	420 3333	30	32	7	37.8	378	37.5	32

4E MAIN CTORNA DUENT		20 MIN STODM FUENT	RAINFALL PER			ALID STORM DUENT		15 MIN	20 MIN	4E MAIN	40 MIN
15 MIN STORM EVENT  0 0 0 0.75 4 1.5 10 8 2.5 12 8 3 3 25 4.5 39 4.5 99 7.5 90 7.5 90 7.5 90 7.5 85 11.25 85 11.25 85 11.25 87 12.5 99 12.9 92 12.75 97	0.25hr mm mm/hr 0 0 0 4 6.8 544 4 6 102.8 18 6 1058 8 13.6 1058 8 13.6 1058 8 13.6 1058 8 13.6 1058 8 13.6 1058 8 13.6 1058 8 13.6 1058 8 13.6 1058 8 158 6 10.2 8 16 7 11.9 952 6 10.2 8 16 7 11.9 952 8 15 8 5 680 5 8 5 680 5 8 5 680 4 6.8 544 3 5.1 408 3 5.1 408 3 5.1 408 2 3.4 272	30 MIN STORM EVENT  0 0 0 1.5 4 3 10 4.5 18 6 25 7.5 32 9 9 99 10.5 46 10.5 59 11.5 64 10.5 70 10.5 80 21 85 22.5 89 24 92 25.5 95 27 97	0.50hr mm/hr 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	45 MIN STORM EVENT  0 0 0 2.25 4 4.5 10 6.75 10 8.7 25 11.25 32 13.5 32 15.75 66 20.25 59 20.25 59 20.25 59 20.25 59 33.75 89 36.92 38.25 95	0.75hr mm /hr 0 0 4 12.4 330.6667 18.6 49.6 13333 5 15.5 413.3333 4 12.4 330.6667 3 9.3 248 3 9.3 248 2 6.2 16.5 333	12 25 15 32 18 39 21 46 24 52 27 59 30 64 33 70 36 75 39 80 42 85 45 89 48 92 51 95	0.75hr mm mm/hr 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	15 MIN 0 0 1.5 816 3 952 4.5 952 6 816 7.5 680 9 9 9 6 680 12.5 680 13.5 272 15 136	30 MIN 0 0 0 1.5 400 3 600 4.5 900 6.7 700 1.5	45 MIN 0 0 0 0 0 225 330.6667 44.5 496 46.5 566 1333 9 9 578.6667 1135 578.6667 1135 578.6667 46.5 578.6667 47.5 48.5 496 49.5 49.5 49.5 49.5 49.5 49.5 49.5 49.5	60 MIN 0 0 3 288 6 432 2 56 443 2 56 4 56 4 57 2 77 56 6 54 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6 6
14.25 99 15 100 1.5HR STORM EVENT  0 0 0 4.5 4 9 10 13.5 18 18 25 22.5 32 27 39 31.5 46 40.5 59 45 64 49.5 70 58.5 80 67.5 88 67.5 88 67.5 88 67.5 88 67.5 88 67.5 89 76.5 99 90 100	2 3.4 272 1 1.77 136 170 1.5hr mm mm/hr 0 0 0 0 4 18.4 245,333 6 27.6 368 8 36.8 490.6667 7 32.2 429.333 7 32.2 429.333 7 32.2 429.333 7 32.2 429.333 7 32.2 429.333 6 27.6 368 7 32.2 429.333 6 27.6 368 7 32.2 429.333 80.6667 5 23.306.6667 5 23.306.6667 5 23.306.6667 1 36.8 184 1 3.8 184 1 4.8 184 1 4.8 183 1 5.8 184 1 6.8 184 1 6.8 183 1 6.8 667	28.5 99 30 100  2HR STORM EVENT  0 0 0 6 4 12 10 18 18 24 25 30 32 36 39 42 46 48 52 54 59 60 64 66 77 77 77 78 80 81 85 99 90 99 10 99 114 99 110 100	2 5 200 1 2.5 100 250 1 2.5 100 250 250 260 270 280 281 21.6 21.6 21.6 24.4 324 8 43.2 432 7 37.8 378 7 37.8 378 7 37.8 378 7 37.8 378 7 37.8 378 7 37.8 378 7 37.8 378 7 37.8 378 7 37.8 378 7 37.8 378 7 37.8 378 7 3	45. 99 45 100  2.5HR STORM EVENT  0 0 0 7.5 4 15 10 22.5 18 30 25 37.5 32 45 39 52.5 46 60 52 67.5 59 67.5 64 82.5 70 97.5 64 82.5 70 97.5 89 1103 89 1103 89 1123 97 125 99 1150 100	2.5hr mm mm/hr  2.5hr mm/hr  0 0 0  4 24 192  6 36 288  8 48 384  7 42 336  7 42 336  7 42 336  7 42 336  7 42 336  5 30 240  6 36 288  7 42 336  5 30 240  6 36 288  1 31 444  3 18 1444  3 18 1444  3 18 1444  3 18 1444  3 18 1444  3 18 1444  3 18 1444  4 19 12 96  6 48	57 99 60 100  3HR STORM EVENT  0 0 9 4 18 10 27 18 36 25 45 32 54 39 63 46 72 52 81 59 90 64 99 70 108 75 117 80 126 85 135 89 144 92 153 95	2 7.2 144 1 3.6 72 360 360 372 387  387  0 0 0 0 4 26.4 176 6 39.6 264 8 52.8 352 7 46.2 308 7 46.2 308 7 46.2 308 7 46.2 308 6 39.6 264 7 46.2 308 7 46.2 308 7 5 5 308 7 5 5 308 7 5 5 308 7 5 5 308 7 5 5 308 7 5 5 308 7 5 5 308 7 5 5 308 7 5 5 5 308 7 5 5 5 308 7 5 5 5 5 308 7 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	1.5HR 0 0 3 0 4.5 245.3333 9 368 13.5 490.6667 18 429.3333 22.5 429.3333 31.5 429.3333 36 368 40.5 429.3333 45 306.6667 63 306.6667 63 306.6667 63 506.6667 67.5 248.3333 7.2 184 81 122.6666 7 90 61.3333 4HR 0 0 0 1.3333	2HR  0 0 0 126 12 324 18 432 24 378 30 378 36 378 36 378 42 378 48 324 72 270 84 270 84 270 84 270 84 270 86 162 102 162 102 162 103 108 108 114 108 114 108 115 131 2 30 1668 45 262 4	2.55HR 0 0 0 7.5 192 15 288 22.5 384 30 336 45 336 60 288 67.5 336 60.7 240 105 240 105 240 112.5 192 112.5 1144 13.5 96 142.5 96 145.6 6HR 0 0 0 4 86 6HR 0 18 116 36 174 54 232	3HR 0 0 9 176 18 264 27 352 36 308 45 308 54 308 72 264 81 308 90 220 108 220 117 220 126 220 116 220 117 82 163 33 163 44
4HR STORM EVENT  0 0 12 4 24 10 36 18 40 25 72 39 100 59 1100 64 66 52 100 99 1100 68 1100 85	4hr mm mm/hr 0 0 0 0 4 30 150 6 45 225 262.5 3.25 262.5 5 37.5 187.5 5 37.5 187.5 125 2 15 2 15 750	5HR STORM EVENT  0 0 0 15 4 3 30 10 45 18 60 25 75 32 90 39 105 46 120 59 135 59 150 64 120 75 125 89 200 85 225 89 240 92 245 99 300 100	Shr mm/hr mm/hr mm mm/hr mm mm/hr mm mm/hr mm mm/hr mm mm/hr mm mm/hr mm	6HR STORM EVENT  0 0 0 18 4 4 36 10 54 18 72 25 108 39 1124 452 1162 99 118 97 129 85 1214 70	6hr mm/hr 0 0 0 4 34.8 116 6 52.2 118 8 69.6 232 7 60.9 203 7 60.9 203 6 69.9 203 6 69.9 203 6 54.9 203 6 54.9 174 55 43.5 145 5 43.		000	38 262.5 60 262.5 72 262.5 84 262.5 96 22.8 100 262.5 120 187.5 132 27.5 144 187.5 168 187.5 180 180 180 180 180 180 180 180 180 180	15 22-4 16 22-9 6 17 22-9 6 10 22-9 6 10 18-8 120 18-8 150 18-8 150 18-8 160 18-4 161 18-4 162 18-2 164 210 18-4 164 225 18-2 240 98-4 255 98-4 255 98-4 270 65-6 285 65-6 300 32-8	293 203 203 203 203 203 203 203 203 203 20	



### APPENDIX F

NSW STATEMENT EMERGENCY SERVICE CONSULTATION

#### Kelvin Holey

From: Kelvin Holey

Sent: Tuesday, 26 October 2021 12:57 PM

To: rra@ses.nsw.gov.au

Cc: Boris Santana; Sidoti, Marisa; Morrison, Peter (AU)

Subject: Glenwood High School - Redevelopment: SEARS Flood requirements [Filed 26 Oct 2021 12:56]
Attachments: 6393-SEARs Flood Study Report-211018-verA-NKK.pdf; Appendix C - EES Group advice.pdf

#### To whom it may concern:

As per the requirements of consent and the need to liaise with the SES for the redevelopment of Glenwood High School, please find attached draft flood study which will be issued as part of the approval submission. This report indicates that the redevelopment is above the 0.2% AEP flood height and will not have an impact on flooding or on neighbouring properties.

Any feedback or comments on the report to satisfy development consent is appreciated. I have been given Maria Frazer's number should I need to get touch: 0458 737 188

I have included the correspondence from EES for information and background. Should there be any questions, do not hesitate to contact me on 0421 421 847

Regards, Kelvin