



Deicorp Projects (Tallawong Station) Pty Ltd

Traffic and Parking Impact Assessment Report

Proposed Mixed Use Development

Tallawong Station Precinct South

May 2021

ENGINEERING PLANNING SURVEYING CERTIFICATION



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Planning Secretary's Environmental Assessment Requirements

Section 4.12(8) of the *Environmental Planning and Assessment Act 1979*Schedule 2 of the Environmental Planning and Assessment Regulation 2000

Application Number	SSD-10425
Project Name	Stage 2 – Detailed Development Application - Tallawong Station Precinct South
Location	1-15 and 2-12 Conferta Avenue, Rouse Hill within Blacktown City Council
Applicant	DEICORP PROJECTS (TALLAWONG STATION) PTY LTD
Date of Issue	13/02/2020

Requirement	Relevant Report Section
Item 6 - Traffic, Parking and Access (operation)	
The EIS must include a traffic, parking and access assessment provide	ling:
Details of a car parking strategy which includes provision of car and bicycle parking for residential and non-residential uses, and consideration of sharing use of the car spaces between land uses.	Section 3.6 (bicycle parking) and Section 4 (car parking).
Details on the likely estimated future mode share for the various users (residents, visitors, etc) accessing the proposed development measures to encourage users of the development to make sustainable travel choices, including a green travel plan, walking, cycling, public transport and car sharing, adequate provision of bicycle parking and end of trip facilities and the minimisation of private car trips.	Green Travel Plan prepared by Barker Ryan Stewart.
Measures to include street tree planting.	Section 3.4
Impacts of the proposed development on the operation of existing and future transport networks, in particular bus corridors, including the public transport capacity and its ability to accommodate the forecast number of trips to and from the development.	Section 3.5 and the Green Travel Plan.
Modelling and analysis of pedestrian and cyclist access to the proposed development in consultation with TfNSW, together with an assessment of pedestrian and cyclist safety and consideration of the relationship with design and operation of the station.	Section 3.6 and the Green Travel Plan.

Detailed assessment of the existing and future performance of key intersections providing access to the site, supported by appropriate modelling and analysis to the satisfaction of TfNSW.	Section 5
Measures to mitigate impacts of the proposed development on the capacity and operation of existing and future traffic, public transport, pedestrian and bicycle networks, including any required upgrades.	Sections 3.4, 3.5, 3.6 and 5.
Details of existing and proposed vehicle access arrangements, including parking, pedestrian safety management, loading dock and servicing management with consideration of precinct wide shared loading docks and/or remote or off-site loading zone hub facilities, ensuring all servicing and loading occurs on-site and does not rely on kerbside controls.	Sections 3.2, 3.3, 4.7 and Appendix D.
An assessment of pedestrian and cyclist safety with consideration of the relationship with design, access and operation of the station.	Section 3.6



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List of Abbreviations

Abbreviations

DCP	Blacktown Development Control Plan 2015
	Blacktown Local Environmental Plan 2015
GFA	
AS/NZS2890.1	Australian Standards, 'AS/NZS 2890.1:2004 Off-Street Car Parking'
AS2890.2	
AS/NZS2890.6	Australian Standards, 'AS/NZS 2890.6:2002 Off-Street Parking for People with Disabilities'
rms	Roads and Maritime Services
RMS Guide	RMS Guide to Traffic Generating Developments, Version 2.2, October 2002
RMS Guide Update	eRMS Guide to Traffic Generating Developments, Updated Traffic Surveys

1 Introduction

Barker Ryan Stewart have been engaged by Deicorp Projects (Tallawong Station) Pty Ltd to prepare a Traffic and Parking Impact Assessment in accordance with the Planning Secretary's Environmental Assessment Requirements (SEARS), Blacktown Council's DCP and LEP and the Roads and Maritime Services (RMS) 'Guide to Traffic Generating Developments' to accompany a proposal for a mixed-use development comprised of residential units and retail and commercial uses.

The purpose of this report is to assess and address traffic, access, car parking and pedestrian and cycling impacts generated by the proposed development and recommend any mitigation measures where required. This can be briefly outlined as follows:

- The expected traffic generation to/from the proposed development.
- The impact of the proposed development on the road network.
- Intersection analysis based on traffic counts.
- Car parking strategy
- Vehicle parking provisions.
- Access design requirements.
- Vehicular requirements for delivery and waste collection.
- Safety of pedestrians and cyclists and the impact on existing pedestrian and cycling networks.
- Availability of public transport.

The assessment included a review of all known available traffic and transport documentation associated with the Tallawong South Precinct including but not limited to the following:

- Traffic and Transport Study prepared by SCT Consulting,
- The Sydney Metro Tallawong Station Precinct South Submissions Report, and
- The Technical Memorandum Tallawong Station SSDA Post Exhibition Responses to Traffic and Parking Comments prepared by SCT Consulting.

This Traffic and Parking Impact Assessment concludes that the subject site is suitable for the proposed development in relation to traffic impact, car parking provision, safety of and provision for pedestrians and cyclists.

2 Existing Conditions

2.1 Site Location

The site is located at 1-15 and 2-12 Conferta Avenue, Rouse Hill (Lots 293 and 294 DP 1213279). The two sites are currently unoccupied and have been cleared of vegetation. The sites are bisected by Conferta Avenue and bounded by Themeda Avenue and Tallawong Metro Station to the north, Cudgegong Road to the east, Schofields Road to the south, and the Tallawong Station commuter car park to the west.



Figure 1: Site Location (source: NearMap October 2019)

2.2 Existing Road Conditions

<u>Intersections</u>

The following four signalised intersections with turn bays are located at the corners of the Tallawong Station Precinct South.

- Cudgegong Road/ Schofields Road
- Tallawong Road / Schofields Road
- Themeda Avenue/ Cudgegong Road
- Themeda Avenue/ Tallawong Road

There are also two restricted left in left out intersections at Conferta Avenue and Tallawong Road and Conferta Avenue and Cudgegong Road.

Aristida Avenue has stop sign posted intersections with Conferta Avenue (Tintersection) and Themeda Avenue.

Schofields Road

Schofields Road is an urban arterial road that provides a major connection between Rouse Hill town centre to the east and Schofields suburb to the west. It generally consists of two lanes in each direction (3.1m wide) separated by a central concrete median with additional turning lanes at intersections to increase turning capacity. Shared pedestrian / cyclist paths are provided on each side of the road. The posted speed limit is 70km/hr. Intermittent bus lanes are included within intersections.

<u>Cudgegong Road</u>

Cudgegong Road is a local road that provides a north / south connection between Schofields Road at the southern end to Guntawong Road at the northern end. It generally has two northbound lanes (3.1m wide each) between the intersection with Schofields Road and the rail overpass and one southbound lane (3.1m wide) from Themeda Avenue to Conferta Avenue. South of Conferta Avenue there are two southbound lanes providing left and right turning lanes to Schofields Road at a major signalised T-intersection.

The Schofields Road / Cudgegong Road intersection has been upgraded to provide a left turn slip lane for eastbound vehicles turning left into Cudgegong Road. The required amount of road widening has been acquired from the south-eastern corner of the subject site to provide for this left turn slip lane. No further land acquisition from the subject site is required for this intersection.

A shared pedestrian / cyclist path is provided along the western side of the road. The posted speed limit on Cudgegong Road is 60km/hr.

Tallawong Road

Tallawong Road is a local road that provides a connection between Schofields Road at the south and Guntawong Road to the north. Generally, between Themda Avenue and Schofields Road, it has three 3.1m wide lane northbound lanes and two 3.1m wide Southbound lanes. Shared pedestrian / cyclist paths are provided on each side of the road. The posted speed limit is 60km/hr.

Conferta Avenue

Conferta Avenue is a local road running parallel with Schofields Road along the northern edge of Lot 293 and the southern edge of Lot 294. It connects Cudgegong Road to the east and Tallawong Road to the west and also provides access to the southern section of the commuter carpark. It has a single 3.8m wide lane of traffic in each direction. Each carriageway has a parking lane delineated by an edge line. The on-street parking has a 2-Hour time limit outside of morning and afternoon peak periods. The posted speed limit is 50km/hr.

Themeda Avenue

Themeda Avenue is a two-way local road consisting of 3.8m wide single lanes with 2.3m on-street parking on both sides of the road. The on-street parking has a 2-Hour time limit outside of morning and afternoon peak periods. It is adjacent to Tallawong metro Station and connects at signalised intersections at Cudgegong Road to the east and Tallawong Road to the west. The posted speed limit is 50km/hr.

Aristida Street

Aristida Street is a two-way local road consisting of 3.8m wide lanes. It connects Implexa Parade to the north and Conferta Avenue to the south. It is subject to a speed limit of 50km/hr.

2.3 **Traffic Flows and Volumes**

Traffic counts were undertaken during the morning and afternoon peak periods to gauge the performance of the current road system. The traffic counts were undertaken between the hours of 7am – 9am and 4pm – 6pm on Thursday 12th December 2019 at the following intersections:

- Cudgegong Road/ Schofields Road
- Tallawong Road / Schofields Road

Traffic counts were also undertaken between the hours of 7am – 9am and 4pm – 6pm on Thursday 12th March 2020 at the following intersections:

- Themeda Avenue/ Cudgegong Road; and
- Themeda Avenue/Tallawong Road.

(Note: These traffic counts were undertaken prior to the impact of the Covid-19 pandemic and represent typical traffic volumes in the neetwork).

The location of the intersections where the traffic counts were undertaken are highlighted in Figure 20 in Section 5 of this report.

The peak hour periods, traffic volumes and layouts for each of these intersections are summarised below in Figures 2 to 5, the full results of the traffic counts are available in Appendix E.

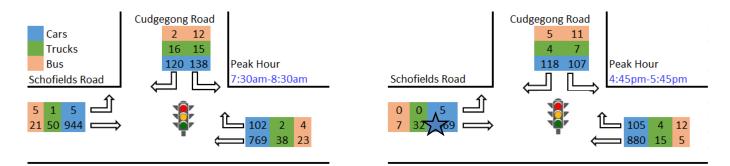


Figure 2: Cudgegong Road / Schofields Road Intersection Count

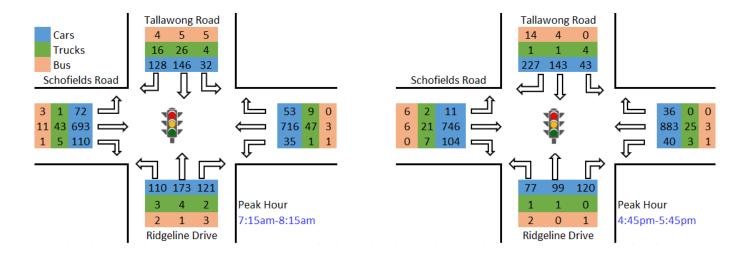


Figure 3: Tallawong Road / Schofields Road Intersection Count

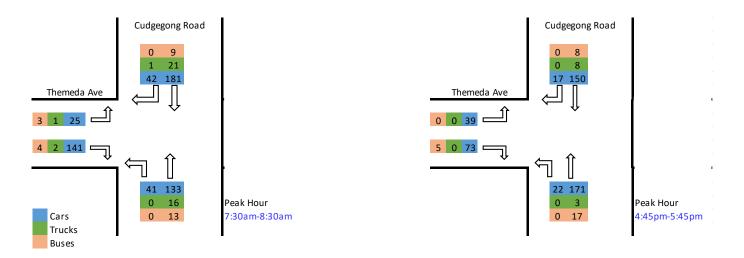


Figure 4: Themeda Avenue / Cudgegong Road Intersection Count

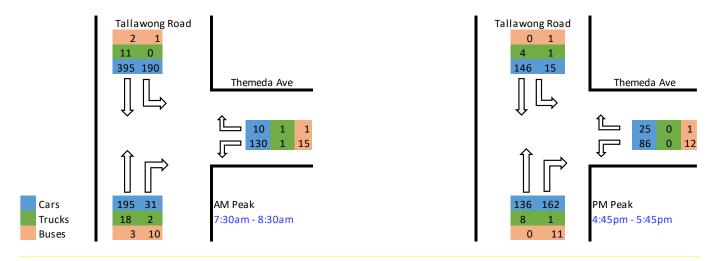


Figure 5: Themeda Avenue / Tallawong Road Intersection Count

2.4 Public Transport Facilities

The area is well serviced by public transport with four bus stops located within 400 metres of the site. These bus stops provide the following services:

- Route 607N Tallawong Station to City QVB via North West T-way and M2 Motorway
- Route 732 Rouse Hill to Blacktown via The Ponds
- Route 742 Marsden Park to Rouse Hill
- Route 747 Marsden Park to Rouse Hill via Riverstone
- Route 751 Rouse Hill Town Centre to Blacktown

The full bus network map is attached at **Appendix B**.

The Sydney Metro Northwest commenced services in June 2019. This rail link connects Sydney's north west region to the Sydney CBD with trains arriving every four minutes during peak hours. Accordingly, the Tallawong train station is located directly adjacent to the site (to the north) and provides connection to centres such as Castle Hill, Epping, Macquarie Park, and the Sydney CBD. **Appendix C** provides a context of the Sydney Metro Northwest with the greater Sydney Train Network.

As part of the Tallawong station facilities there are 1,000 commuter car parking spaces, 4 spaces for buses, 15 kiss-and-ride spaces, 9 taxi spaces and parking and storage for 35 bicycles.

Figure 6 is an extract from the Blacktown Growth Centres DCP and shows the public transport network map highlighting the proposed strategic bus corridors, bus routes, indicative bus stops and pedestrian catchments for bus stops (400m) and train stations (800m).

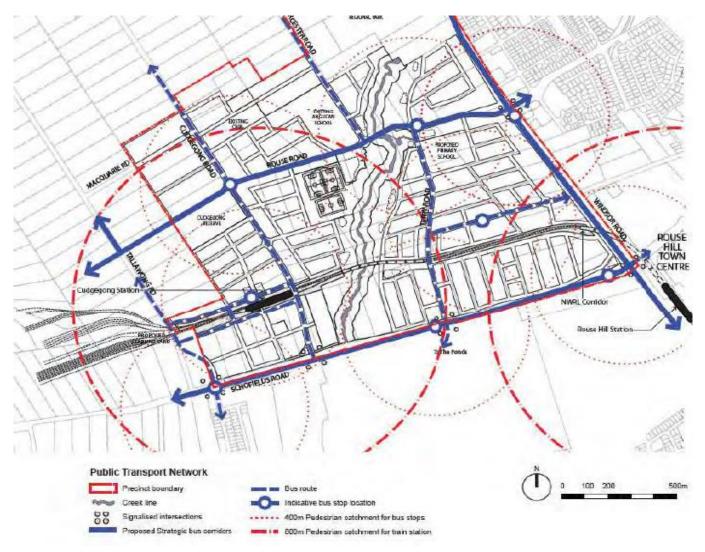


Figure 6: Blacktown Growth Centres DCP Schedule 4 Area 20 Precinct Public Transport Network Map 2011

Figure 7, below identifies existing public bus and transport options in close proximity to the site.

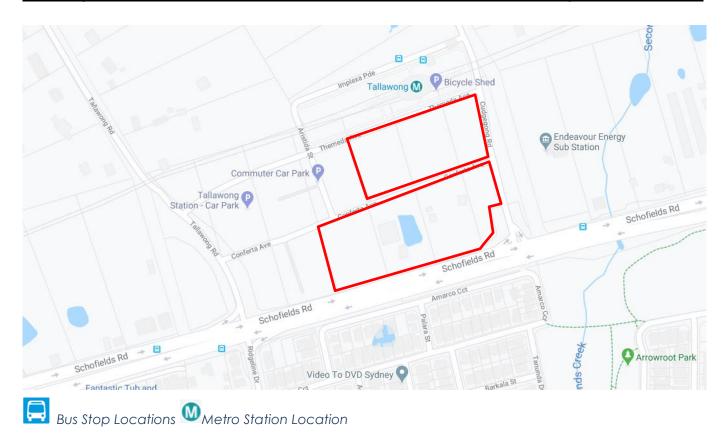


Figure 7: Location of bus stops and Sydney Metro Station to the site

2.5 Pedestrian and Bicycle Facilities

The pedestrian and cycling network extend within the road corridors of Schofields Road, Cudgegong Road and Tallawong Road.

In the immediate vicinity of the site, Themeda Avenue provides 3m wide pedestrian paths on both sides of the road. Conferta Avenue has 2.5m wide footpaths on the northern side of the road and a 1.9m footpaths on the southern side of the road. Aristida Street and Implexa Parade have footpaths on both sides of the road with a zebra crossing located on Themeda Avenue and Implexa Parade, directly in front of the Tallawong Metro Station.

Signalised pedestrian crossings are also available at the following signalised intersections:

- Cudgegong Road/ Schofields Road
- Tallawong Road / Schofields Road
- Themda Avenue/ Cudgegong Road
- Themeda Avenue/ Tallawong Road

As mentioned above, at Tallawong Station there are bike racks available and a bicycle shed with parking for 35 bicycles.

An extract from the DCP Pedestrian and Cycle Network Map shows the site pedestrian and bicycle links that link the Tallawong Station Precinct South to the surrounding area.

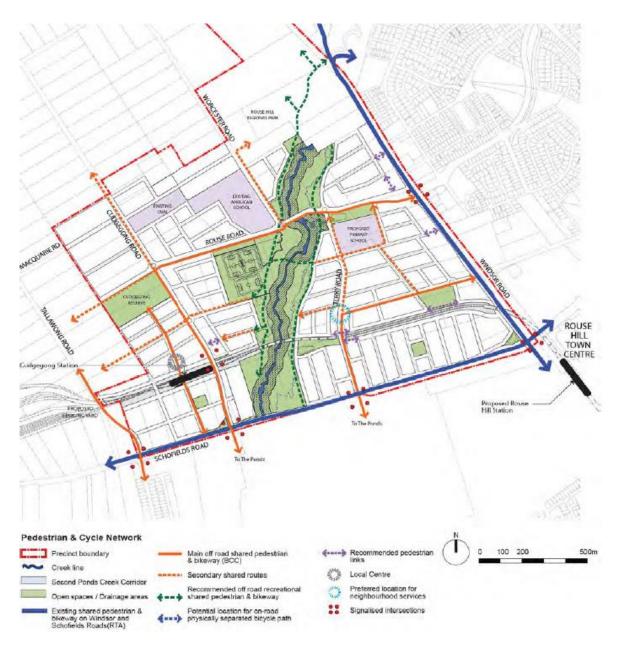


Figure 8: Blacktown Growth Centres DCP Schedule 4 Area 20 DCP Pedestrian and Cycle Network Map

In summary, the roads surrounding the site provide a high standard of pedestrian / cyclist facilities including shared paths and pedestrian and cyclist crossing facilities at each of the signalised intersections. The existing and future pedestrian and bicycle network surrounding the site is attached at **Appendix A**.

3 Proposed Development

3.1 Development Yield

The proposed development is comprised of residential and non-residential components as stipulated in the following table.

Table 1: Proposed Development Yield

Land Use		Yield
Residential	1 Bedroom	252 units
	2 Bedroom	682 units
	3 Bedroom	53 units
	Total	987 units
Retail		6,000m ²
Commercial		3,000m ²

3.2 Access

As shown in Figure 9 below, access to the site will be provided via the vehicle crossings arrangement as follows:

- Site 1A Combined car and loading dock vehicle access on the Northern side of Conferta Avenue;
- Site 1B Separated car and loading dock vehicle access on the Northern side of Conferta Avenue;
- Site 2A Separated car and loading dock vehicle access on the Northern side of the new proposed road;
- Site 2B, 2C & 2E vehicle access on the Southern side of Conferta Avenue;
- Site 2B, 2Cand 25E loading dock vehicle access on the Eastern side new proposed road; and
- Site 2D Separated car and loading dock vehicle access on the Southern side new proposed road.

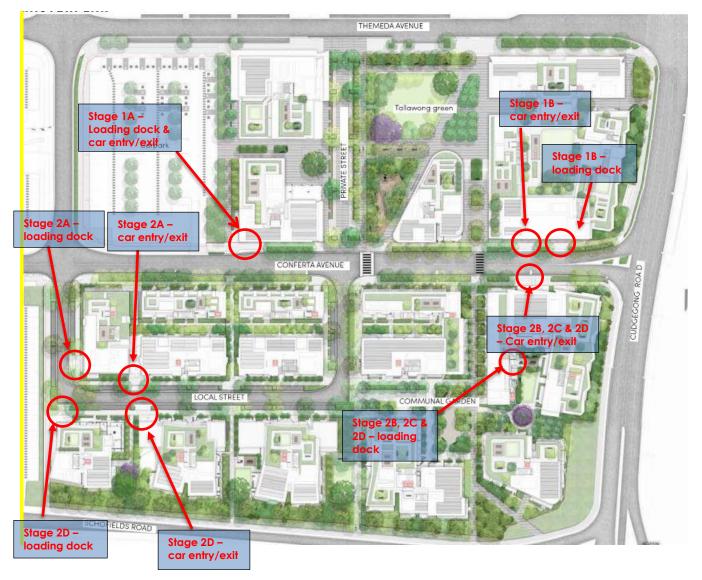


Figure 9: Extract from Landscape Plans showing the location of vehicle crossings within the development.

The entry/exit driveways comply with AS/NZS 2890.1-2004 Parking Facilities – Off-Street Car Parking, AS 2890.2-2002 Parking Facilities – Off Street Commercial Vehicle Facilities. More details are available in Section 4.7 of this report (Parking Compliance Check).

Car swept turning path plans of vehicles have been provided in **Appendix D** in accordance AS/NZS 2890.1-2004 Parking Facilities – Off-Street Car Parking.

3.3 Service Vehicles and Loading Requirements

Swept turning path plans of Blacktown Council's waste collection vehicle have been provided in **Appendix D** in accordance with AS 2890.2-2002 Parking Facilities – Off Street Commercial Vehicle Facilities and the waste vehicle dimensions outlined in Blacktown City Council's DCP Part G Site Waste Management and Minimisation.

The plans demonstrate forward ingress and egress of an 11 metre Medium Rigid Vehicle (MRV) between the loading docks and Conferta Avenue and between the loading docks and the proposed public road. The plans also show that a Council waste collection vehicle and a passenger vehicle can safely travel along the proposed public road in both directions, thereby maintaining two-way traffic flow.

Internally, the MRV is able to enter the loading docks in a forward direction, satisfactorily reverse into the waste collection area and loading bay and exit in a forward direction.

The swept turning paths demonstrate that the proposed loading and waste vehicle access provisions comply with Blacktown Council's DCP (minimum 25m) and the requirements of AS 2890.2-2018.

3.4 Public Domain Improvements

Public domain improvements are proposed to the extent outlined in the extracts from the Civil Plans in Figure 10 below not only to accommodate vehicle, pedestrian and bicycle movements from the development to the surrounding road, pedestrian and cycle networks but to also improve the landscaping and overall amenity of the precinct for pedestrians.

Apart from upgrading Conferta Avenue to accommodate a safe movement of vehicles, pedestrians and cyclists, a new public road is also proposed to allow vehicle access to the basement parking and loading bay area and to provide for safe pedestrian movements.

To facilitate pedestrian movement from the precinct to Tallawong Station and the bus stops adjacent to the station, pedestrian and cycling links are provided through the site as shown in Figure 12 and two pedestrian crossings are proposed in Conferta Avenue as indicated in Figure 10 below.

To address concerns about pedestrian and cyclist safety while crossing Conferta Avenue it would be possible to provide a limited amount of on-street parking along both sides of the road as shown in Figure 10 below. To further enhance pedestrian / cyclist safety, it is recommended that kerb extensions be provided on each side of the proposed rossings.

The presence of on-street parking has been shown to reduce traffic speeds as drivers are more aware of vehicle parking movements and adjust their speed accordingly. This on-street parking could most effectively be provided by line marking a parking lane on each side of Conferta Avenue and providing a combination of time-limited parking signs, No Stopping and No Parking signs as shown in Figure 10. The installation of the line marking and signposting can only be implemented with the approval of Blacktown City Council and, if approved, would be installed as part of the development proposal.

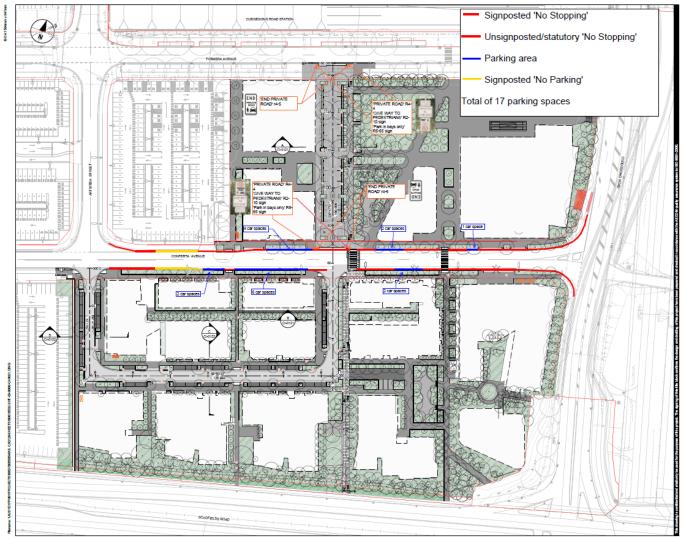


Figure 10: Extract from Civil Plans showing the proposed on-street parking in Conferta Avenue

According to the Landscape Plans, "The proposed development will deliver a new standard in public domain along the North West Priority Growth Area. Delivering high end public realm comprising of green open space with high end playground". And, "The streetscape is designed to be to create a vibrant, diverse and cool character".

With regards to accessibility the Landscape Plans state, "All residents and local, community may access and enjoy the public domain universal access to AS1428 has been incorporated into where possible connected walkways, the public spaces and building entries without compromising design quality."

Figure 11 below is an extract from Landscape Plans showing an overview of the proposed tree planting through the site, including street tree planting within the existing Conferta Avenue, Themeda Avenue and the proposed new public road.



Figure 11: Extract from Landscape Plans showing the proposed landscape works.

3.5 Public Transport network

Note that the proposed 987 residential units is less than the original 1,100 residential units approved in the concept proposal. Therefore, the Sydney Metro Northwest line and bus network would have been designed to accommodate a higher density on the site than what is currently approved.

From the Green Travel Plan prepared by Barker Ryan Stewart it was estimated that the precinct once fully completed and occupied would generate approximately 100 people per hour traveling by train in the morning peak between 7am and 9am and the evening peak between 4pm and 6pm. It is also estimated that approximately 50 people per hour would leave the precinct to travel by bus in the morning 7am to 9am peak and the evening peak between 4pm and 6pm.

The Sydney Metro Northwest can have trains departing in each direction every four minutes (15 trains per hour) and has the capacity of moving approximately 20,000 people in each direction every hour. Therefore, the additional commuters estimated to be generated by the development would be easily catered for by Sydney Metro Northwest in fact this is the reason the metro line was built in order to accommodate residents in the Northwest Growth area.

The number of people from the precinct likely to commute by bus would be able to be catered for by the buses leaving from Tallawong Station. Figure 6 shows the bus public transport map in the vicinity of the site.

3.6 Pedestrian and Bicycle Facilities

Pedestrian and cyclist access modelling to the proposed development was not considered necessary as the off road pedestrian and cycle paths in Schofields Road and Cudgewong Road and the wider network would have been modelled on a regional scale and the network would be able to cater for the number of pedestrians and cyclists that would be generated by the development, particularly that the development yield proposed is a smaller residential unit yield than what was previously approved by the concept SSDA.

Site Pedestrian Access

Extensive public pedestrian paths exist in the precinct area to support the recently constructed Tallawong Station. The site is located immediately adjacent to the train station with a number of key public facilities within walking distance to the site.

The site is well connected with pedestrian footpaths which allow easy access from Tallawong station to and from the main pedestrian entries to the precinct.

In addition to the public domain works outlined in Section 3.4 above and the proposed through site cycle and pedestrian links shown in Figure 12 below, the existing pedestrian network of footpaths on surrounding public roads would encourage residents, staff and users of the precinct to walk from destination to destination or use a combination of public transport and walking. This would represent a significant mode shift toward active transport based on promotion of site permeability and north-south shared pedestrian links through the site.

The pedestrian site links proposed within the precinct together with the proposed upgrades to the public domain within Conferta Avenue including the two pedestrian crossings will provide safe pedestrian access to Tallawong Station and the surrounding pedestrian network.

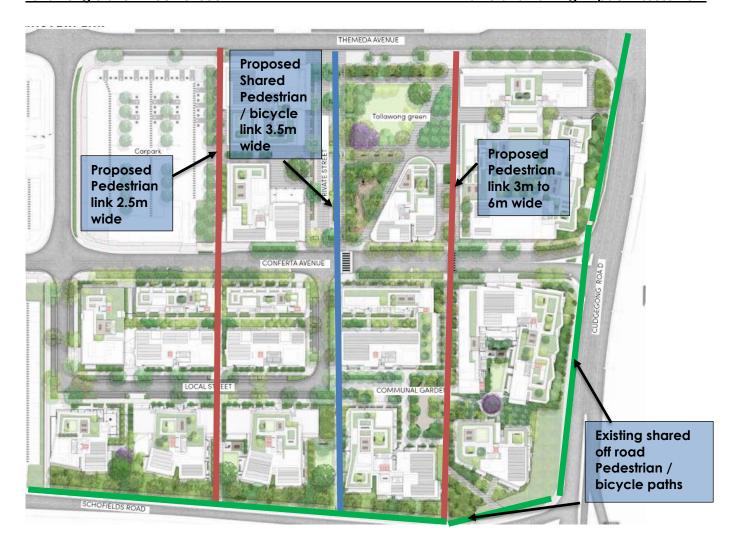


Figure 12: Extract from Landscape Plans with pedestrian and bicycle links added

Bicycle Facilities

In regard to cycling, the 2016 census showed that only 0.2% of the population cycled to and from work. However, assuming a more conservative estimate of 1% of the total residents cycling each day will give a total of 150 cyclists per day.

It is proposed to provide 1,086 bicycle spaces to the precinct which is considered more than sufficient to cater for the storage and facility needs of the residents and their visitors. Note that there is no requirement in the Concept Plan SSD 9063 to provide the non-residential uses with bicycle storage.

The bicycle storage spaces are proposed to be provided at a rate of one space for each residential unit for residents and one space for each 10 units for residential visitors in accordance with the Concept Plan SSD 9063 requirements. This equates to $987 \times 1 + 987/10 = 1,086$ bicycle parking spaces.

Bicycle parking is proposed to be provided in the following locations adjacent to lifts:

- Site 1A Basement 1 55.2m² storage area for 24 bicycles and 76.2m² for 31 bicycles.
- Site 1A Basement 2 144.5m² storage area for 69 bicycles.
- Site 1B Basement 3 179.8m² storage area for 111 bicycles, 148.2m² for 38 bicycles, 114.7m² for 50 bicycles, 43.6m² for 24 bicycles and 43.1m² for 19 bicycles.
- Site 2A Mezzanine 359m² storage area and 182 bicycles.
- Site 2B, 2C & 2E Mezzanine 167.6m² and 74 bicycles.

- Site 2B, 2C & 2E Basement 1 storage area 51m² for 20 bicycles, 186.3m² for 80 bicycles, 164.7m² for 80 bicycles and 187.1m² for 95 bicycles.
- Site 2D Mezzanine 127m² storage area for 60 bicycles and 258m² storage area for 129 bicycles.

The precinct will also incorporate dedicated cycle and pedestrian links as shown in Figure 12 above. This includes off road bicycle links in Schofields Road and Cudgegong Road and on road link in Themda Avenue and Conferta Avenue.

Access will be enhanced from Schofields Road through the precinct to Tallawong Station and proposed Cudgegong Town Centre in accordance with Blacktown Growth Centre Precincts DCP mapping. As the North West Growth Centre begins to support greater number of residents, it is anticipated that the provision of additional safe and connected bicycle infrastructure will encourage alternative modes of transport to be utilised by residents and visitors to the site.

The development will connect to Rouse Hill Town Centre via the existing shared path along Schofields Rd. **Appendix A** shows the cycleways along Cudgegong and Tallawong Roads that have been partially constructed by Sydney Metro.

Overall, the precinct will have safe and excellent access for cyclists to public transport and bicycle network and facilities which can accommodate the bicycle movements generated by the proposal.

4 Car Parking Assessment and Strategy

4.1 Parking Requirements

The basement level parking includes 1,361 spaces, comprising of 1,111 residential spaces, 87 commercial spaces and 167 retail spaces. The applicable parking rates for the development are provided below:

Concept SSD 9063 Approval Rates

- 0.6 space per 1-bedroom unit
- 0.9 spaces per 2-bedroom unit
- 1.4 spaces per 3-bedroom unit
- 1 visitor space per 10 units
- 1 space per 60m² retail space
- 1 space per 70m² commercial space

*Blacktown Council (minimum) DCP Rates (Growth Centre Precincts)

- 1 space per 1-bedroom unit
- 1 space per 2-bedroom unit
- 1.5 spaces per 3-bedroom unit
- 1 visitor space per 5 units
- 1 space per 22m² retail space (over 200m²)

1 space per 40m² office space

RMS Guide Rates (Metropolitan Sub-Regional)

- 0.6 space per 1-bedroom unit
- 0.9 spaces per 2-bedroom unit
- 1.4 spaces per 3-bedroom unit
- 1 visitor space per 5 units
- 4.5 spaces per 100m² retail space
- 1 space per 40m² commercial space

Proposed Rates

- 1 space per 1-bedroom unit
- 1 space per 2-bedroom unit
- 1.4 spaces per 3-bedroom unit
- 1 visitor space per 10 units
- 1 space per 36m² retail space
- 1 space per 36m² commercial space

*Note: The Blacktown DCP minimum parking requirement can be reduced to the RMS rate due to the site being located within 800 metres of a railway station. A summary of the applicable parking requirements in relation to the development yield is included in Table 2 below.

Table 2: Minimum car parking requirements

Land Use		SSD	9063 nits	Conce SSD 906 Approv	53	Propos Units		SSD 9063 rates	RMS Guide	Council DCP	Proposed Parking	
	1-Be	d	220	units	132		252 un	its	151	151	252	252
	2-Be	đ	660	units	594		682 un	its	614	614	682	682
Residential	3-Be	ġ	220	units	308		53 uni	ts	74	74	80	74
Residential	Visito	Visitors		otal nits: 100	110		Total units: 987		99	198	198	99
	Car Was	h										4
Residential sub total		al			1,144				938	1,037	1,212	1,111
Land Use		Conc SSD 9 GF	063	Concept SSD 9063 Approval Requirement		Pr	oposed GFA		0 9063 ates	RMS Guide	Council DCP	Proposed Parking
Commercial		3,000)m ²		43	3	,000m ²		43	75	75	83
Retail		6,000)m ²	1	00	6	,000m ²		100	270	273	167
Non-Residen total	tial	Tota 9,000		1	43		Total = ,000m²		143	345	348	250

Land Use	Concept SSD 9063 Approval requirement	SSD 9063 rates	RMS Guide Requirement (based on proposal)	Council DCP Requirement (based on proposal)	Proposed Parking
Residential sub total	1,144	919	1,037	1,212	1,111
Non-Residential total	143	143	345	348	250
Total	1,287	1,062	1,382	1,560	1,361

4.2 Transit orientated development parking guiding principal

'As a guiding principle, Sydney Metro and Landcom consider that a reduced car parking provision for Tallawong Station Precinct South is fundamental to achieving:

- An exemplar transit orientated development (maximising the benefits of fast frequent metro connections with services every 4 minutes in the peak and 10 minutes in off peak.
- A town centre where cars do not dominate
- Activation and life on the street
- Less congestion of precinct roads.

Providing high levels of car parking does not align with the key principles of transit orientated development'

The parking provision included in the concept approval for the development was determined by applying the car parking requirements of State Environmental Planning Policy No 65 - Design Quality of Residential Apartment Development (SEPP 65) and the Apartment Design Guide which states that:

"Car parking is provided based on proximity to public transport in metropolitan Sydney and centres in regional areas. For development on sites that are within 800m of a railway station or light rail stop in the Sydney Metropolitan Area the **minimum** car parking requirement for residents and visitors is set out in the Guide to Traffic Generating Developments, or the car parking requirement prescribed by the relevant council, whichever is less".

Given the location of the development close to Tallawong Station, the parking requirements in the RMS Guidelines for Metro Sub-Regional CBD Centres were adopted in the concept approval when determining the required number of parking spaces for the residential components of the site.

The parking rates applicable to the non-residential (commercial / retail) component of the proposed development adopted for the concept approval were based on the rates in the Parramatta DCP for Epping Town Centre.

4.3 Tallawong Station existing commuter car parks

As part of the Tallawong station facilities there are 1,000 car parking spaces (820 regular spaces, 30 accessible spaces and 150 compact car spaces) in the three commuter carparking areas. There are also 4 spaces for buses, 15 kiss-and-ride spaces and 9 taxi spaces provided in Themeda Avenue and Implexa Parade.

The results of the of the AM peak period on Thursday 12 March 2020 are indicated in Figure 13 and the PM peak period on the same day are indicated in Figure 14.

Time	Car Park 1 (147 spaces)			Car Park	2 (454 sp	aces)		Car Park 3 (397 spaces)				
Time	In	Out	Total parked	Available spaces	In	Out	Total parked	Available spaces	ln	Out	Total parked	Available space
6:45			147	0			239	215	Ť		5	39
7:00			147	0	69)	308	146			134	26
7:15			147	0	98	3	406	48			196	20
7:30			147	0	39)	445	9			243	15
7:45			147	0	9)	454	0			317	8
8:00			147	0			454	0			354	4
8:15			147	0			454	0			377	2
8:30			147	0			454	0			396	
8:45			147	0			454	0			397	
tor bikes	also enter	ed Car Park	2 between 7am-	7:30am								

Figure 13: The results of the commuter car park survey AM

Time	Car Park 1 (147 spaces)				Car Park 2	(454 space	s)		Car Pa	rk 3 (397 spa	aces)	
rime	In	Out	Total parked	Available spaces	In	Out	Total parked	Available spaces	In	Out	Total parked	Available spaces
4:00												
4:15											354	4
4:30	3	6	119	28	3	17	380	74			350	4
4:45	2	16	105	42		13	367	87			342	5
5:00	4	8	101	46	1	40	328	126			326	7
5:15	1	. 10	92	55	4	39	293	161			302	9
5:30	3	10	85	62		38	255	199			269	12
5:45	4	7	82	65	1	31	225	229			239	15
6:00	3	4	81	66	1	45	181	273			200	19
formal	car parking	on opposite	e side of Cudgeg	gong Road to deve	lopment sit	e - 108 cars	parked at 4:15	pm				
n street	parking in (Cudgegong	Road stretching	from bridge over 5	Sydney Met	ro west to	Rouse Road (40	0m) - 64 cars park	ed at 4	:15pm		
n Street	parking in (Grassland C	ourt - 58 cars pa	arked at 4:15pm								

Figure 14: The results of the commuter car park survey PM

In summary we undertook a car park survey on Thursday 12 March 2020 with the following results:

- P1 (147 spaces) Full by 6:15am
- P2 (454 spaces) Full by 7:45am
- P3 (397 spaces) Full by 8:30am
- P1 had 28 spaces available by 4:30pm and 66 available at 6pm
- P2 had 74 spaces available by 4:30pm and 273 available by 6pm
- P3 had 43 spaces available at 4:15pm and 197 available by 6pm.
- Note that the 30 accessible spaces in P1 remained available throughout the survey.
- Informal car parking on the opposite side of Cudgegong Road to the development site 108 vehicles parked at 4:15pm
- On street parking in Cudgegong Road stretching from Sydney Metro to Rouse Road (400m) 64 cars parked at 4:15pm.
- On Street parking in Grassland Court 58 cars parked at 4:15pm.

The car parking survey reveals that the 1,000 existing car parking spaces located at the Tallawong Station are already at capacity (with the exception of the accessible spaces in P1 which remained vacant) with a substantial overflow of all-day parking currently occurring onto the surrounding street network and adjoining properties during the working week.

Note that this car park survey was also undertaken at a time when the coronavirus was already impacting the use of public transport.

Furthermore, NearMap satellite images taken once per month show that on consecutive months on the dates indicated below the car parks were full.

- Tuesday 29th Oct 2019
- Thursday 12th of Sep 2019
- Friday 16th Aug 2019
- Friday 19th of July 2019



Figure 15: NearMap image 29 October 2019 showing the car parks full.

This indicates that the moment that the Tallawong Station car parks were completed and opened they were at capacity.

4.4 Basis of Proposed Parking Provision

As outlined in Section 4.1 the proposed parking provision is summarised in the following table.

Table 3: Proposed parking

Land Use	Concept SSD 9063 Approval requirement	SSD 9063 rates	RMS Guide Requirement (based on proposal)	equirement (based on (based on	
Residential sub total	1,144	919	1,037	1,212	1,111
Non-Residential total	143	143	345	348	250
Total	1,287	1,062	1,382	1,560	1,361

It should be noted that the primary purpose of the Sydney Metro Northwest is to connect the North-West area with major employment and education centres such Epping, Macquarie University, Macquarie Park, North Ryde, Chatswood and the Sydney CBD primarily for those commuting to work and university.

It is acknowledged that planning for new developments in this area needs to consider the principles of transit-oriented development and the implementation of targeted travel demand management measures and initiatives to reduce the need and reliance on private vehicle travel.

However, the guiding principles do not take into consideration the unique location of Tallawong Precinct South being located at the end of the Sydney Metro Norwest Line and the extensive catchment area it serves. The 1,000 existing commuter car parking spaces located at the Tallawong Station are already at capacity with a substantial overflow of all-day parking currently occurring onto the surrounding street network during every workday. By 8:30am the existing car parks are full by commuters travelling on the Sydney Metro Norwest line, once full drivers of vehicles then proceed to park in the surrounding street network.

The catchment area beyond the North West Growth centre is predominantly rural in nature with poor or no public transport options available to the residents.

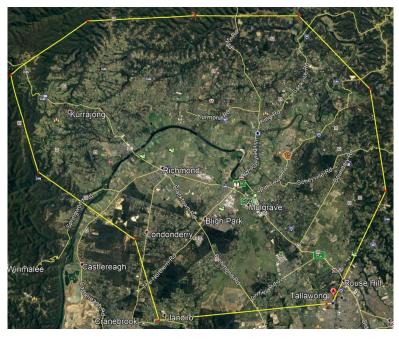


Figure 16: Estimated Catchment area of car users that currently park at Tallawong Station

Even once the Sydney Metro Norwest extends to the Western Sydney airport and connects with Schofields Train Station, this will only partially reduce this catchment area. Also, the surrounding North West Growth sector in the immediate vicinity of Tallawong Station and to the west and east of the station is only partially developed. Once this is fully developed this would put even more pressure on the at capacity Tallawong Station car parks.

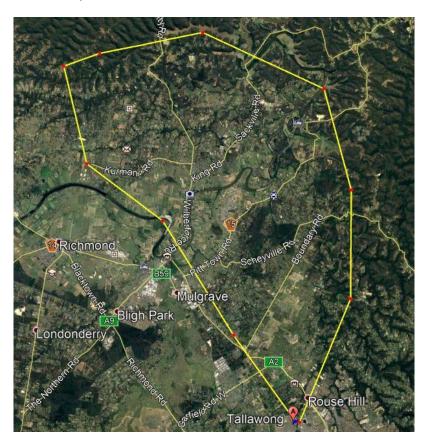


Figure 17: Estimated Catchment area of car users once Sydney Metro Northwest connects with Schofields Station

Furthermore, in the developing area around Tallawong Station there will also be a high demand for a range of local trips by residents on week days such as school / childcare centre pick up and drop off and on weekends for accessing family sporting activities, shopping and visiting family and friends at destinations that are not readily served by public transport. Consequently, there will continue to be a high demand for parking spaces associated with new developments in this area.

Evidence of this high demand for parking can be seen in a review of six residential developments recently approved by Blacktown City Council in the vicinity of Tallawong Station. This review has revealed that, in almost all cases, the parking rates adopted were 1 space per 1 and 2-bedroom units and 2 spaces per 3 bedroom units, with visitor parking at a rate of 1 space per 5 units. The parking rates proposed for the residential component of this development are less than the rates that have been adopted in these recently approved developments.

CBRE provided advise to the saleability of residential units in the area and they stated that units without a car parking space are difficult to sell compared to those that do. (See letter at **Appendix G**)

Strictly complying with the car parking rates outlined by the Concept Approval would result in a development that would have insufficient car parking to cater for residential component of the development and would put pressure on the at capacity Tallawong Station car parks and on-street parking in the surrounding road network.

The proposed parking rates are considered better able to meet the travel demands associated with the characteristics of the Tallawong area; are consistent with the parking provision of other approved developments in the vicinity of the Tallawong station and will prevent a detrimental impact on the already at capacity Tallawong Station commuter car parking areas and the surrounding road network.

Although not required to support the proposed parking rates, there is potential for four GoGet shared parking spaces to be provided in the new private road in the vicinity of the Tallawong Station.

In summary, it is proposed that the proposed parking rates of one space per one and two bedroom units, 1.4 spaces for each three bedroom unit, no dedicated visitor parking and one space per 30m² of GFA for the retail and commercial component should be adopted.

These rates are considered to be better able to meet the travel demands associated with the characteristics of the Tallawong area and consistent with the parking provision of other approved developments in the Blacktown City Council and The Hills Council LGA within vicinity of the Tallawong Metro station and to reduce the detrimental impact at the already at capacity Tallawong Station car parks and the surrounding road network.

The proposed parking provision of 1,361 spaces is only 74 spaces more than what was approved in the concept plan and is less than the minimum requirements of the RMS Guide, Blacktown DCP and the maximum rates of transit orientated development guiding principles.

4.5 Residential Parking Provision

The proposed provision of 1,111 residential spaces for the 987 units proposed equates to just over one space (1.12 spaces) per unit and is considered satisfactory to cater for the car parking requirements generated by the development.

The residential parking provision is based on the rates of one space per one and two-bedroom units, 1.4 spaces for each three bedroom unit and one visitor parking space per 10 units.

The proposed 1,111 residential spaces are considered to be warranted and acceptable for the following reasons:

- The proposed 1,111 residential spaces are 74 spaces more than the SEPP65 minimum parking requirement of 1,037 spaces based on the car parking rates required by the RMS Guide.
- The residential component of 1,111 spaces proposed is 101 spaces less than the 1,212 spaces based on the minimum car parking rates required by the Blacktown DCP.
- The original concept approval for the site was for 1,100 residential units and the provision of 1,144 car parking spaces for those units. The current proposal is for 987 residential units and the provision of 1,111 car parking spaces. This is a net reduction of 33 total residential car parking spaces compared to that which was approved in the concept proposal.
- Adequate secure on-site parking is a necessity for all residents to enable them to have a car
 parking space available 24/7 so they can drive to destinations such as work, schools, sports
 grounds, recreational areas, visiting friends and relatives etc where there are inadequate public
 transport modes available from the site.
- The provision of car parking for visitors to the residential apartments is outlined in a report prepared by Interpark Australia dated March 2021. The proposal is to implement a system whereby residential visitors will have after-hours access to the parking spaces allocated to the

commercial developments within the site. The provision of parking for residential visitors is based on the following principles:

- Minimum visitor parking would address, to some extent, the transit orientated development guiding principles to minimise the parking provision.
- o Minimising visitor parking would encourage visitors to use the public transport during peak periods.
- o There would be an opportunity for visitors to utilise the 23 car parking spaces to be provided in the new private road and 34 on street parking in the new road (see Figure 10).
- Residential visitors tend to visit outside of peak working week hours when the commercial parking spaces will be unoccupied.

4.6 Non - Residential Parking Provision

The non-residential parking rates adopted for the concept approval were from the Parramatta DCP for Epping Town Centre. However, the travel demand, socio-economic and environmental characteristics of Tallawong and surrounding areas are vastly different to those at Epping being a well-established location close to employment and educational facilities with highly-developed road and public transport networks. The non-residential parking rates for the Epping Town Centre are therefore not considered to be appropriate for use at Tallawong.

In addition, the types of commercial / retail developments proposed have now been identified in more detail since the planning for the concept approval was conducted. Consequently, the parking rates required by RMS and Blacktown City Council are considered to be more appropriate as they relate to specific land use types rather than the generic commercial / retail land uses referred to in the concept approval.

The RMS rates in their Guide to Traffic Generating Development for retail and commercial uses are derived from undertaking numerous car parking surveys of similar retail and commercial developments and are considered appropriate in determining the demand for parking for these types of uses.

However, with a view to follow the guiding principles of transit oriented development with respect to the minimising the provision of parking adjacent Tallawong Station Precinct South, Deicorp has sought advice from an expert that negotiates leases/sale of properties to all the supermarket chains and other retail and commercial operators. They sought to determine the minimum amount of parking that could be provided for retail and commercial uses on site that would still make that component of the development viable and usable.

The expert Shiprock Advisory have stated that one space per 30m² would be the minimum rate at which parking should be provided for the retail and commercial uses. (letter is attached at **Appendix G**).

Strictly complying with the car parking rates outlined by the Concept approval would result in a development that would have insufficient car parking to cater for non-residential component of the development and would put pressure on the existing at capacity Sydney Metro commuter car park and on-street parking in the surrounding road network.

On this basis the proposed provision of 250 non-residential spaces based on the rate of one space per 36m² is considered satisfactory to cater for the car parking requirements generated by the retail and commercial component of the development.

The proposed provision of 250 spaces is 98 spaces less than the 348 spaces required by the Blacktown DCP car parking rates and 95 spaces less than the 345 spaces required by the RMS Guide to Traffic Generating Developments car parking rates.

4.7 Parking Strategy

Condition A19 of the approved Concept Plan SSD 9063 nominates the following minimum car parking rates:

Use	Minimum Rate			
Residential dwellings	0.6 car space per 1-bedroom			
	0.9 car space per 2-bedroom			
	1.4 car space per 3-bedroom			
Residential visitor	0.1 car space per dwelling			
Affordable Housing	As required by State Environmental Planning Police			
	(Affordable Rental Housing) 2009, or the residential			
	dwelling rates above, whichever is the lesser			
Retail floor area	1 car space/60sqm GLFA			
Commercial floor area	1 car space/70sqm GFA			
Bicycle space for residents	1 bicycle space/dwelling			
Bicycle spaces for visitors	1 bicycle space/10 dwellings			

In addition, Condition B12 of the Concept Plan SSD 9063 requires that the application is to include a parking strategy to maximise efficiency of car parking spaces including the consideration of sharing use of car spaces between land uses.

The proposed development provides car parking and bicycle parking based on the following rates:

Use	Proposed car parking rate
Residential dwellings	1 car space per 1-bedroom
	1 car space per 2-bedroom
	1.4 car spaces per 3-bedroom
Residential visitor	1 space per 10 units
Retail floor area	1 car space/36sqm GLFA
Commercial floor area	1 car space/36sqm GFA
Bicycle space for residents	1 bicycle space/dwelling
Bicycle spaces for visitors	1 bicycle space/10 dwellings

The proposed residential and the commercial car parking rates exceed the minimum requirement and are therefore compliant with Condition A19 car parking rates. Notwithstanding this, the Design Review Panel requested further explanation to support the proposed car parking rates. Accordingly, the proposed car parking rates are justified as follows:

- The car parking rates comply with the minimum control expressed by Condition A19.
- Whilst the proposed development is characterised as transit-oriented development, it is still necessary to acknowledge the unique location of Tallawong Precinct South at the end of the Sydney Metro Norwest line and the extensive catchment area it serves. In particular, the North West Growth sector has only been partially developed and the catchment area beyond the North West Growth centre is predominantly rural in nature with poor or no public transport options available to the residents. Many of the services and facilities in the local area still need to be accessed by car including schools, childcare centres, sporting facilities, parks, and visiting family and friends at destinations that are not readily served by public transport. Accordingly, whilst the close proximity of the metro line is an excellent asset for the future residents and businesses and will reduce some private car trips, particularly during the week to access primary employment centres, it does not cater for all trips which will be required by residents in the local area.
- The proposed parking rates have been arrived at in order to meet the travel demands associated with the characteristics of the Tallawong area; and are consistent with the parking provision of other approved developments in the vicinity of the Tallawong station. In particular, the residential parking

- provision in other nearby recently approved developments demonstrates the demand for parking by residents, which indicates that it is necessary to provide 1 space per 1 and 2-bedroom units and 1.4 spaces per 3-bedroom units in order to address this demand.
- If the demand for residential parking is not met by the proposed development because residential parking provision is reduced to the minimum rates in Condition A19, this would result in a significant detrimental impact to the demand for on-street parking within the already at-capacity Tallawong Station public car parking areas and the surrounding streets including those in the adjacent The Ponds development where there is currently un-timed on-street parking. This reduction in parking is the equivalent of 173 car spaces (1,111 938) and therefore the lack of provision of these parking spaces within the development will likely result in this demand needing to be satisfied in the surrounding streets. This is a concern which has already been raised by nearby residents during the consultation for the approved Concept Plan SSD 9063.
- The non-residential parking rates adopted for the concept approval were adopted from the Parramatta DCP for Epping Town Centre. However, the travel demand, socio-economic and environmental characteristics of Tallawong and surrounding areas are vastly different to those at Epping being a well-established location close to employment and educational facilities with highly developed public transport networks. The non-residential parking rates for the Epping Town Centre are therefore not considered to be appropriate for use at Tallawong. Furthermore, an insufficient amount of non-residential car parking will undermine the ability to attract retailers and especially an anchor tenant for the supermarket which will undermine the ability to achieve the critical mass of retailers and shoppers necessary to achieve a properly activated centre.
- Notwithstanding that the car parking rates are higher than the minimums identified in Condition A19, the proposed development will still result in a reduced traffic impact when compared to that which was anticipated by the approved Concept Plan SSD 9063 as the proposal has 113 or 10.2% less apartments.

Having regard to the above, it has been demonstrated that the proposed car parking rates are both compliant and justified having regard to the context of the site and there is no lawful or reasonable basis for an alternative provision of parking.

Finally, the proposal is accompanied by a Green Travel Plan prepared by Barker Ryan Stewart which identifies a range of sustainable travel initiatives to complement the existing transport options and provide a holistic strategy to positively influence occupant behaviour.

4.8 Parking Compliance Check

Barker Ryan Stewart has reviewed the Architectural plans prepared by Turner. This review included the layout of car parking and internal roadways / ramps and overall, we are satisfied that the design is consistent with the requirements of Standards AS/NZS 2890.1, AS 2890.2-2002, AS 2890.3-2015 and AS/NZS 2890.6 and Blacktown Council DCP. It is anticipated that the car park will function in a satisfactory manner and in accordance with the original design intent. A summary of critical parameters assessed regarding the Australian Standards is included below.

Table 4: Compliance Table

Control	Proposed	Compliance			
AS2890.1-2004 (Off-street Car Parking), AS2890.2-2002 (Off-street commercial vehicle facilities) and Council DCP					
2.4.1 Car Space Dimensions:					
Class 1A (residential)	2.4m x 5.4m with 5.8m aisle width	Yes			
Class 3 (commercial and retail)	2.6m x 5.4m with 5.8m aisle width				
2.4.2 Blind aisle Extension & Clearance	1 aisle extension provided for end of aisle parking spaces	Yes			

2.5.2 Layout Roadways/Ramps	Passenger vehicle: Minimum 5.5m wide for two-way flow	Yes	
2.5.2 Layout Roadways/Ramps P.C. 1:: 2.5.3 Roadway/Ramp Grades Sit C.G. 3r Sit	Heavy vehicle: minimum 6.5m wide for two-way flow		
2.5.2 Pagdway/Pamp Crades	Passenger vehicle: Max 1:4 (25%) with 2m 1:8 (12.5%) transition ramp	Yes	
2.5.5 Roddwdy/Rdmp Grades	Heavy vehicle: Max 1:6.5 (15.4%) with 1:16 (6.25%) in 7m of travel	res	
	Site 1A Category 3 (6m entry, 4-6m exit and a 1-3m median)		
3.2.2 Driveway Width	Site 1B Category 3 (6m entry, 4-6m exit and a 1-3m median)	Yes	
	Site 2A Category 2 (6-9m combined entry / exit)		
	Site 2B, 2C and 2E Category 3 (6m entry, 4-6m exit and a 1-3m median)		
	Site 2D Category 2 (6-9m combined entry / exit)		
	Loading dock for all sites Confirmed via swept path analysis		
3.3 Gradients of Driveways	Maximum 1:20 for the first 6m within the property boundary.	Yes	
	property been dary.		
5.2 Column Location/Spacing	Columns and other obstructions kept clear of parking envelope (as depicted in Figure 5.2 of AS2890.1)	Yes	
	Columns and other obstructions kept clear of parking envelope (as depicted in Figure		
5.3 Headroom	Columns and other obstructions kept clear of parking envelope (as depicted in Figure 5.2 of AS2890.1) Passenger vehicles: Minimum 2.2m Heavy vehicles: 4.5m	Yes	
5.3 Headroom Control	Columns and other obstructions kept clear of parking envelope (as depicted in Figure 5.2 of AS2890.1) Passenger vehicles: Minimum 2.2m		
5.3 Headroom	Columns and other obstructions kept clear of parking envelope (as depicted in Figure 5.2 of AS2890.1) Passenger vehicles: Minimum 2.2m Heavy vehicles: 4.5m	Yes	
5.3 Headroom Control	Columns and other obstructions kept clear of parking envelope (as depicted in Figure 5.2 of AS2890.1) Passenger vehicles: Minimum 2.2m Heavy vehicles: 4.5m	Yes	

As shown in the table above, the development car park and access design comply with the relevant Australian Standards.

5 Traffic Assessment

The impact of the proposed development on the surrounding road network was assessed using SIDRA Intersection modelling software. The traffic counts outlined in Section 2.3 and traffic generation estimated below in Section 5.1 were used to determine an overall traffic level for the area post-development. Section 5.2 describes how these additional trips were distributed amongst the critical intersections chosen for study.

Ultimately Section 5.3 outlines the SIDRA analysis undertaken which found that the increased traffic resulting from the proposed development will not have a significant impact on the efficiency of the surrounding road network.

5.1 Trip Generation

Currently the site is vacant and as such has no traffic generation associated with it. Trip generation rates for the proposal were determined using the *Guide to Traffic Generating Developments*, *Updated Traffic Surveys (TDT 2013/04a)* and the rates previously used by SCT Consulting in their Traffic and Transport Study for the Concept Approval for the site.

The *Updated Traffic Surveys* document states that the recommended trip generation rates were based on ten surveys conducted in 2012, eight within Sydney, and one each in the Hunter and Illawarra. All developments were close to public transport, greater than six storeys and almost exclusively residential in nature. Appendix B1 of the document also provides details of the locations where the surveys were conducted and, although located within Sydney, locations such as Strathfield and Pyrmont were classed as "Regional Areas" for the purposes of the trip rate assessment. Consequently, Tallawong would also be regarded as a regional area and the appropriate trip generation rates have been adopted for this assessment.

Due to the proximity of the site to rail and bus services, the primarily residential nature and scale of the development and the high standard and connectivity of pedestrian and cyclist facilities, it was determined that the average Regional residential trip rates were appropriate for this assessment.

The proposed estimated traffic volumes generated by the proposed development are outlined in the Table 6 using the following trip rate shown in Table 5:

Table 5: Trip generation rates

Use	AM trip rates	PM trip rates	
Residential (Regional Average)	0.53 trips per unit	0.32 trips per unit	
Retail	1.94 per 100m ²	2.7 trips per 100m ²	
Commercial	1.6 trips per 100m ²	1.2 trips per 100m ²	

Table 6: Proposed development – traffic generation

Land Use	Yield	AM Peak Hour Trips	ln	Out	PM Peak Hour Trips	In	Out
Residential	987 units	523	37	150	316	118	30
Retail	6,000 m ²	117	58	59	162	81	81
Commercial	3,000 m ²	48	29	19	36	14	22
Total	-	688	124	228	514	213	133

The retail trip rates used in Table 5 are considered satisfactory given the proximity of the site to Tallawong Station and the reduction in parking to be provided in the retail component of the site compared to the RMS 'Guide to Traffic Generating Development' rates. They were previously accepted/endorsed by the Department of Planning, Blacktown City Council, TfNSW and RMS as outlined in the Sydney Metro Northwest Places Program Tallawong Station Precinct South Submissions Report dated October 2018.

Note that when applying the retail area of 4,500 m² used by SCT Consulting in their analysis and the subject retail and commercial component of the proposal together with the reduction in the proposed number of residential units the overall trips generated by the development is less than that assessed as part of the Concept Approval.

The table above shows the proposed development generates approximately 688 trips during the AM peak hour and 514 trips during the PM peak hour.

5.2 Trip Distribution

This section outlines the methodology used to distribute the trips generated by the proposed development. The in / out distribution for the proposal was as follows:

Residential
 Retail
 Commercial
 Residential
 20% in, 80% out (AM), 80% in, 20% out (PM)
 50% in, 50% out (AM and PM)
 60% in, 40 %out (AM), 40%in, 60% out (PM)

Trips were assigned based on a distribution of 10% each to the north and west, and 40% each to the east and south, based on the employment location of resident workers from the 2016 census which indicated that employment locations were largely located in the greater Sydney area.

Access to and from all basement carparks will be via Conferta Avenue, however, as traffic movements at the Conferta Avenue / Cudgegong Road and the Conferta Avenue / Tallawong Road intersections are restricted to left in / left out only several trips will need to be made via Themeda Avenue.

The following diagrams provide summaries of the routes that vehicles will use to enter and exit between the site and the surrounding road network.

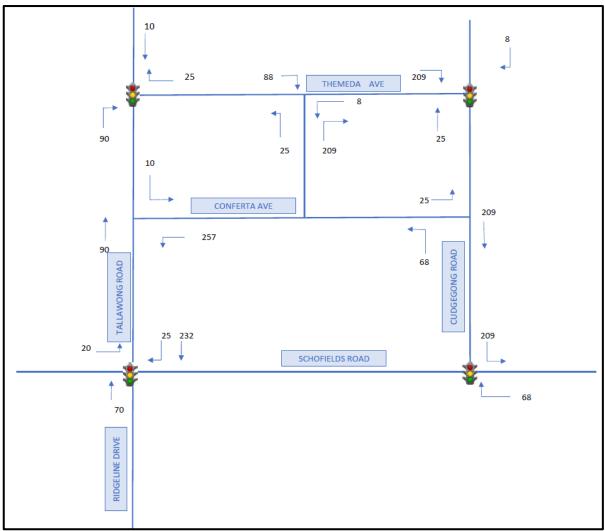


Figure 18: AM Trip Assignment

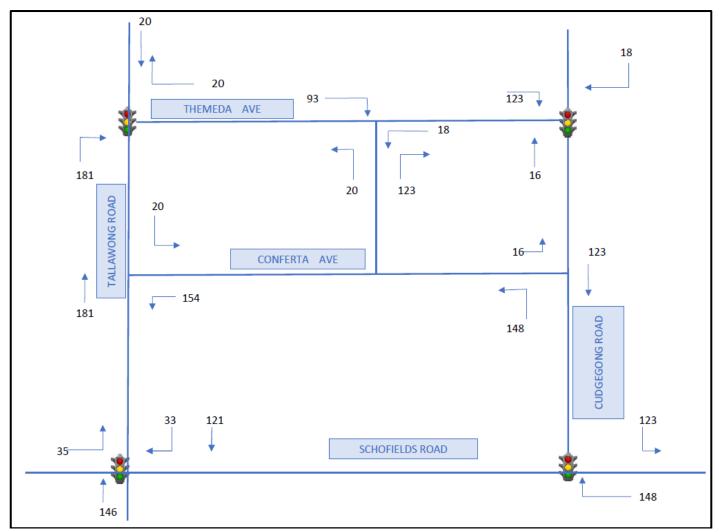


Figure 19: PM Trip Assignment

5.3 SIDRA Analysis and Impact of Generated Traffic

Intersection performance has been assessed using the SIDRA modelling software which uses the level of service (delay) model adopted by Transport for NSW to assess intersection performance.

Average delay is used to determine the level of service (LOS), which ranges from 'A' (excellent level of service) to 'F', with a LOS of 'D' being the minimum acceptable performance. The intersections outlined at the start of Section 2.3 have been assessed as a network for the existing and 10-year growth traffic volumes for AM and PM peak periods.

To assess the expected growth rate over the next 10 years, reference was made to the Transport for NSW vehicle counting station (ID No. 71024) located on Windsor Road between Merriville Road and Sanctuary Drive, Rouse Hill. The traffic volumes recorded at this station increased from an average daily volume of 47,589 vehicles per day to 53,958 vehicles per day, an increase of 6,369 or 13.4% over 8 years (1.67% per annum).

In order to ensure a robust and conservative assessment of the impact of the development on the road network, a growth rate of 2% per annum was applied to the traffic volumes at the surveyed intersections for the projected 10-year growth scenarios.



Figure 20: Intersections analysed using SIDRA

The differences in performance between these two scenarios are summarised in the tables below, with the full movement summary attached at **Appendix F.**

Table 7: Cudgegong Road / Schofields Road SIDRA Modelling Summary

	dgegong Road/	Existing S	Scenario	10-year growth scenario						
Sc	chofields Road	Existing Conditions	Post Development Condition	Existing Conditions	Post Development Condition					
AM	Delay (s)	17.3	17.7	26.3	23.1 *(25.6)					
AM	LOS	В	В	В	B *(B)					
PM	Delay (s)	16.0	19.6	16.8	23.5 *(26.0)					
F/V	LOS	В	В	В	B *(B)					

Table 8: Tallawong Road / Schofields Road / Ridgeline Drive SIDRA Modelling Summary

	illawong Road/ chofields	Existing :	Scenario	10-year growth scenario					
	Road / lgeline Dr	Existing Conditions	Post Development Condition	Existing Conditions	Post Development Condition				
AM	Delay (s)	27.7	36.8	31.7	47.0*(53.4)				
AM	LOS	В	С	С	D *(D)				
PM	Delay (s)	28.2	32.2	36.4	36.7 *(46.5)				
r/VI	LOS	В	С	С	C *(D)				

Table 9: Themeda Avenue/ Cudgegong Road SIDRA Modelling Summary

	meda Ave/	Existing S	Scenario	10-year growth scenario						
Cu	dgegong Road	Existing Conditions	Post Development Condition	Existing Conditions	Post Development Condition					
	Delay (s)	14.1	20.9	15.4	21.2*(34.1)					
AM	LOS	Α	В	В	B *(C)					
PM	Delay (s)	12.8	17.5	13.4	17.5 *(29.5)					
P/M	LOS	Α	В	Α	B *(C)					

Table 10: Themeda Avenue / Tallawong Road SIDRA Modelling Summary

	neda Ave/	Existing S	Scenario	10-year grov	wth scenario
10	ıllawong Road	Existing Conditions	Post Development Condition	Existing Conditions	Post Development Condition
A A A	Delay (s)	17.5	23.0	18.7	24.5 *(37.6)
AM	LOS	В	В	В	B *(C)
DAA	Delay (s)	21.5	25.9	24.5	26.6 *(39.0)
PM	LOS	В	В	В	B *(C)

^{*} The Sidra model for this assessment adopted a network cycle time of 80 seconds for the future network scenarios based on this being the optimum cycle time to minimise delays at each intersection. However, feedback from Transport for NSW indicated that a cycle time of 140 seconds should be adopted in the modelling. Consequently, additional modelling was conducted for the critical "Post-development / 10-year Growth Scenario" based on a user-given cycle time of 140 seconds. The results are shown in brackets in the tables above indicating that the network will operate within acceptable parameters on a cycle time of 140 seconds, however, average delays will increase at some of the intersections, thereby reducing levels from B to C and C to D which is still well within accepted levels.

These results therefore indicate that the optimum cycle time for this network is 80 seconds as adopted by the original modelling.

In summary, the intersections surrounding the site currently operate at a high level of service with acceptable delays and will continue to operate satisfactorily with spare capacity based on the expected background growth in traffic volumes over the next 10 years and the additional traffic that will be generated by the proposed development. The development traffic will not have any significant impacts on level of service and delays currently experienced within the road network. Consequently, the development can be supported based on traffic grounds without the warrant to upgrade the existing local road network.

The SIDRA analysis and our conclusions are in line with the analysis undertaken in the Traffic and Transport Study prepared for the concept proposal for this development by SCT Consulting, dated 18 May 2018.

5.4 Impact of Construction Traffic

The construction phase of the development will require the delivery of machinery, equipment and materials to the site by a range of heavy vehicles up to a 19 metre articulated vehicle. Access to and from the site will be restricted primarily to the major roads in the area, Schofields Road which has links to the wider state road network at Windsor Road in the east and Richmond Road in the west. The only local roads that will be used for access to and from the site will be Cudegong Road and Tallawong Road which have direct connections with the Schofields Road via signalized intersections.

The management of construction traffic on the surrounding road network is detailed in a Construction Pedestrian and Traffic Management Plan prepared for this development by Barker Ryan Stewart.

5.5 Impact on Road Safety

The additional traffic volumes that will be generated by the proposed development is not expected to have any impact on road safety on the surrounding road network.

The main intersections providing access to and from the site are all controlled by traffic signals with pedestrian crossing facilities on all approaches. The streets directly surrounding and through the site have been designed for low speed environments with raised crossings for pedestrians and cyclists.

A review of all existing pedestrian and cyclist infrastructure has identified a high level of safety and connectivity in all streets surrounding the site and the proposed internal pedestrian / cyclist infrastructure has been designed to connect seamlessly with this existing infrastructure. All shared paths will be identified with the required signage and line marking to minimise any potential pedestrian / cyclist conflicts.

Driveway widths have been designed to minimise crossing distances for pedestrians, while adequately providing for vehicle swept paths. Driveways will also be designed to provide appropriate sight lines for pedestrian safety and to provide the required sight distances to approaching traffic in accordance with AS/NZS 2890.1.

A Road Safety Audit for the Construction phase has been prepared by Barker Ryan Stewart.

6 Conclusion

This Traffic and Parking Impact Assessment has been prepared in accordance with the Planning Secretary's Environmental Assessment Requirements (SEARS), Blacktown Council's DCP and LEP, the Roads and Maritime Services (RMS) 'Guide to Traffic Generating Developments' to accompany a Development Application to the Blacktown City Council for the development of a mixed-use development.

Parking requirements have been assessed in accordance with the approved concept parking rates, Blacktown City Council DCP, the RMS Guide, and proposed rates.

The proposed parking provision provides a balance between the minimum requirements of the RMS Guide, Blacktown DCP, the transit orientated development maximum rates and to alleviate the potential detrimental impact on the existing Sydney Metro commuter car park and existing road network which are already at capacity.

The proposed provision of 1,111 residential spaces for the 987 units is considered satisfactory to cater for the car parking requirements generated by the development.

The proposed provision of 250 spaces is considered satisfactory to cater for the retail and commercial car parking requirements generated by the development.

It is proposed to provide 1,086 bicycle spaces to the precinct which is considered more than sufficient to cater for the storage and facility needs of the residents, visitors, employees and customers.

Traffic surveys and modelling were undertaken on the relevant intersections as follows:

- Cudgegong Road/ Schofields Road
- Tallawong Road / Schofields Road
- Themeda Avenue/ Cudgegong Road
- Themeda Avenue/ Tallawong Road

The SIDRA analysis and our conclusions are in line with the analysis undertaken in the Traffic and Transport Study prepared by SCT Consulting, dated 18 May 2018. The development traffic would have only a minor impact on existing, delays and Level of Service along this traffic network. Accordingly, any network upgrades will be required due solely to the existing conditions and the general growth in traffic that will be expected to occur over future years and not as a result of the additional traffic that will be generated by the proposed development.

The proposed parking and loading facilities have been designed in accordance with the requirements of AS/NZS 2890.1 – Off Street Car Parking, AS 2890.2 – Off-Street Commercial Vehicle Facilities and AS/NZS 2890.6 - Off-street Parking for People with Disabilities. These facilities are also considered practical and safe ensuring that all traffic generated by the development can enter and exit the site in a forward direction.

The Traffic and Parking Impact Assessment concludes that the subject site is suitable for the proposed mixed-use development in relation to traffic impact, car parking provision and the provision for pedestrian and bicycle facilities. The development is considered to have negligible effect on the safety and operating outcome of the surrounding transport network.

7 References

Australian Standards, 'AS/NZS 2890.1:2004 Off-Street Car Parking'.

Australian Standards, 'AS 2890.2:2018 Off-Street Commercial Vehicle Facilities'.

Australian Standards, 'AS/NZS 2890.6:2002 Off-Street Parking for People with Disabilities'.

Blacktown city Council DCP.

NSW Department of Planning, 'SEPP (Infrastructure) 2007'.

Roads and Maritime Services, 'Guide to Traffic Generating Developments' Version 2.2 dated October 2002.

Roads and Maritime Services, 'Guide to Traffic Modelling' Version 1.0 dated February 2013.

Traffic and Transport Study prepared by SCT Consulting, dated 18 May 2018.

Sydney Metro - Tallawong Station Precinct South Submissions Report, dated October 2018.

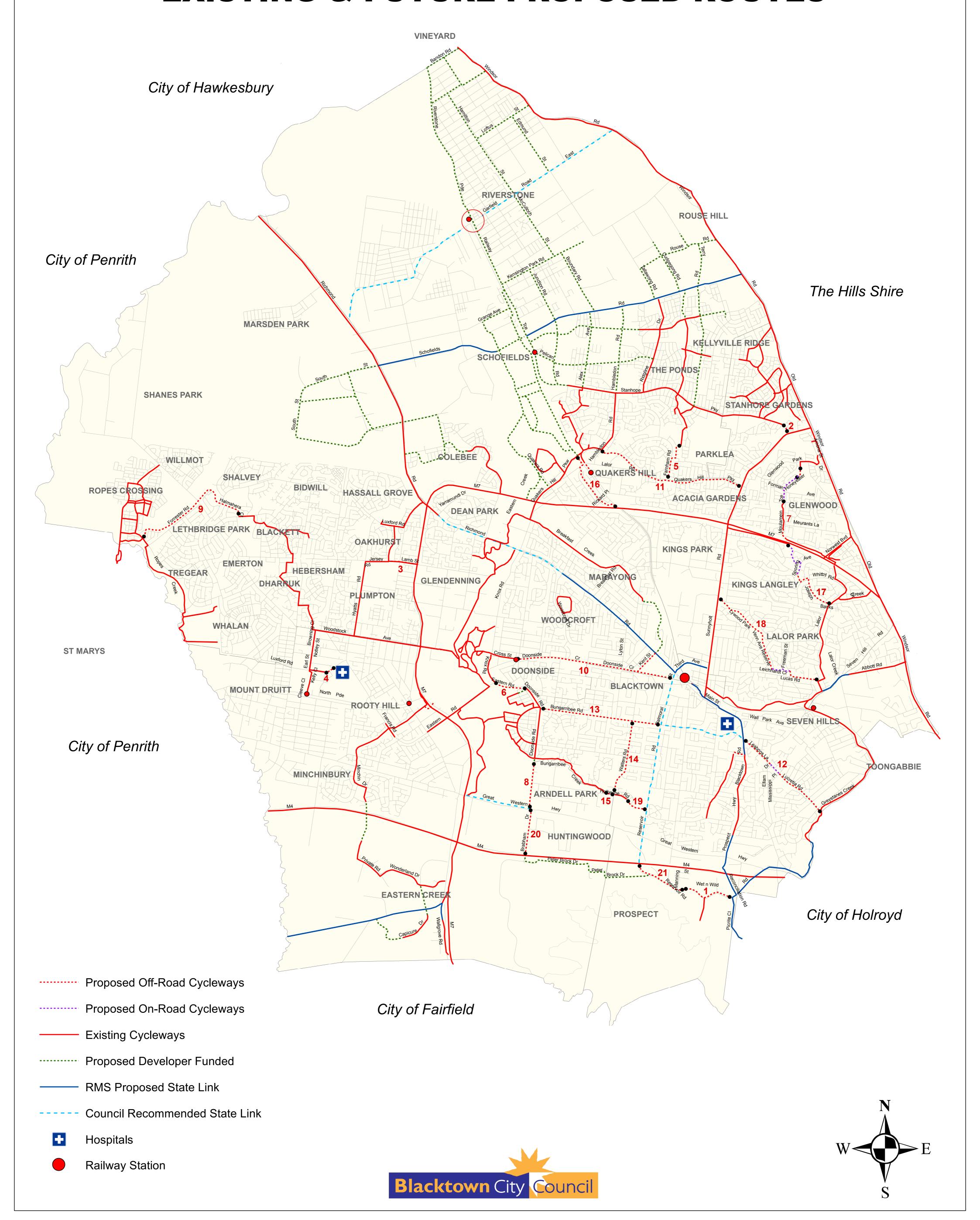
Technical Memorandum – Tallawong Station SSDA – Post Exhibition Responses to Traffic and Parking Comments prepared by SCT Consulting.

Appendix A

Pedestrian and bicycle network

BLACKTOWN CITY COUNCIL

2016 BIKE PLAN EXISTING & FUTURE PROPOSED ROUTES



Appendix B

Bus network

Appendix C
Sydney Train Map

Sydney rail network











Sydney metro and train lines



Metro North West Line Chatswood



North Shore North Shore Western Richmond



Inner West & Leppington Line Inner West Leppington City



Bankstown Line Liverpool Lidcombe City



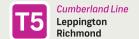
Eastern Suburbs & Illawarra Line **Eastern Suburbs** Illawarra Cronulla



Line under construction

Check timetables and trip planners for train services and connections







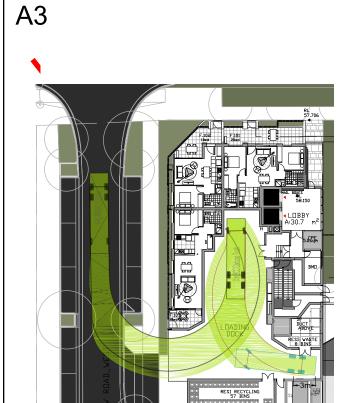


Olympic Park Line Olympic Park

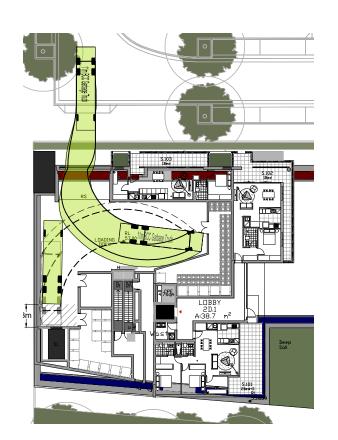


Appendix D

Swept Path Analysis



11m WASTE VEHICLE TURNING PATH



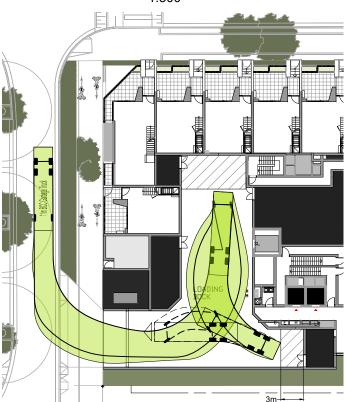
11m WASTE VEHICLE
TURNING PATH WITH 3m REAR CLEARANCE
1:500



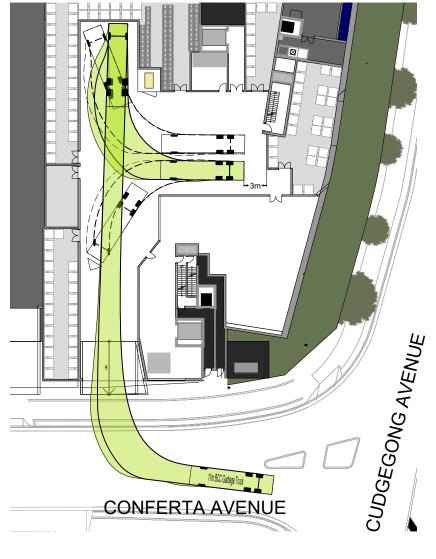
11m WASTE VEHICLE
TURNING PATH WITH 3m REAR CLEARANCE
1:500



8.8m MRV TURNING PATH



11m WASTE VEHICLE TURNING PATH WITH 3m REAR CLEARANCE 1:500



11m WASTE VEHICLE
TURNING PATH WITH 3m REAR CLEARANCE
1:500

A.H.D.

Datum:

 REV
 AMENDMENT
 ISSUED
 DATE

 A
 CONCEPT DESIGN
 JB
 20/04/2020

 B
 CONSTRUCTION TURNING CIRCLES
 JB
 3/09/2020

 C
 WASTE AREA TURNING PATHS
 JB
 10/09/2020

 D
 NEW LOADING DOCK ADDED
 JB
 16/11/2020

BARKER RYAN STEWART P: 02 9699 0005 P: 02 4968 8388
CENTRAL COAST P: 02 4926 8555
P: 02 4925 5255 P: 07 5682 6565

DEICORP Pty Ltd

TALLAWONG STATION PRECINCT SOUTH PROPOSED DEVELOPMENT

WASTE AREA TURNING PATHS

Designed: BM Scales: Plan
Drawn: JB Horiz.
Checked: GB X-Sect

SY190226-01-103
File Ref. REV.
SY190226 D







5 0 5 10 15 20 25 METRES 1:500

REV	AMENDMENT	ISSUED	DATE	1
В	CONSTRUCTION TURNING CIRCLES	JB	3/09/2020	H
С	WASTE AREA TURNING PATHS	JB	10/09/2020	
D	NEW LOADING DOCK ADDED	JB	16/11/2020	
Е	B99 PATHS TO BASEMENT PARKING	JB	20/11/2020	
F	R85 & R00 PATHS TO RASEMENT PARKING	ID.	24/11/2020	B

0:0		BARKER RYAN STEWART	Si P: Ci P:
0	TOTAL PROJEC	OT SOLUTIONS	

SYDNEY HUNTER
P. 02 9659 0005 P. 02 4966 8388
CENTRAL COAST S.E. QLD
P. 02 4325 5255 P. 07 5582 6555

www.brs.com.au

DEICORP Pty Ltd

TALLAWONG STATION PRECINCT SOUTH PROPOSED DEVELOPMENT

BASEMENT ACCESS TURNING PATHS

 Designed:
 BM
 Scales:
 Plan
 1:500

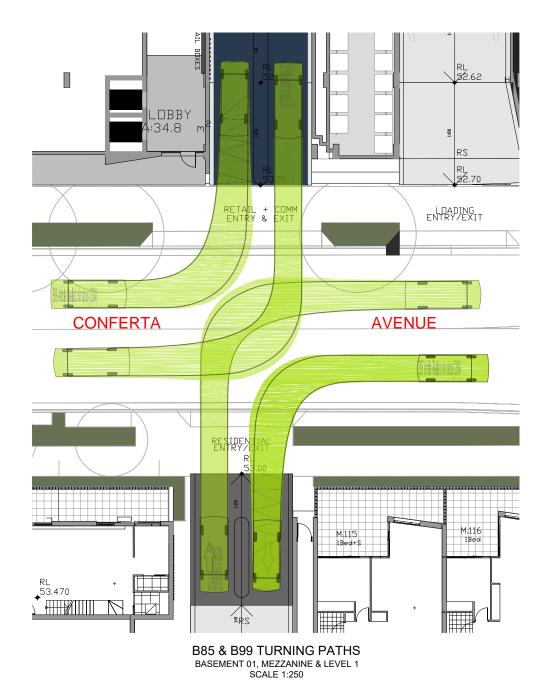
 Drawn:
 JB
 Horiz.

 Vert.
 Vert.

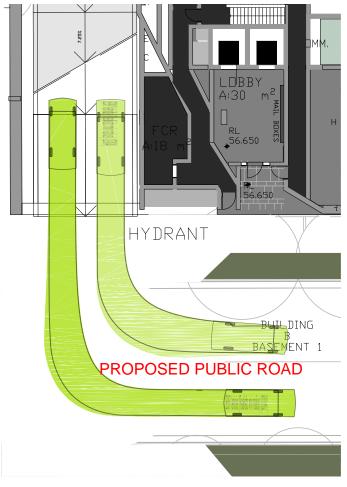
 Checked:
 GB
 X-Sect.

 Datum:
 A.H.D.

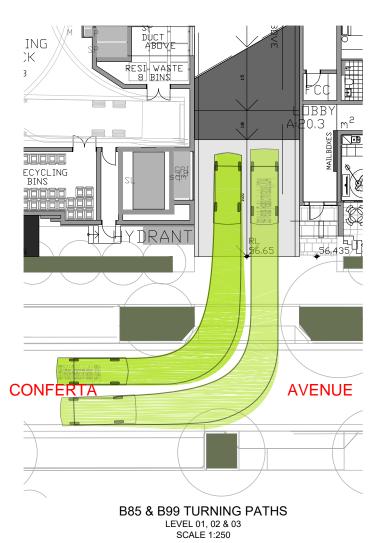
SY190226-01-104
File Ref. REV.
SY190226 D







B85 & B99 TURNING PATHS MEZZANINES, LEVEL 01 & 02 SCALE 1:250



2.5	0	2.5	5	7.5	10	12.5	1.250
			METRES			_	1:250

REV B	AMENDMENT CONSTRUCTION TURNING CIRCLES	ISSUED DA	020	BARKER	SYDNEY P: 02 9659 0005	HUNTER P: 02 4966 8388	Client:	TALLAWONG STATION PRECINCT SOUTH	Designed:		Scales: Plan 1:250	Plan No.	04.404
С	WASTE AREA TURNING PATHS	JB 10/09	2020	RYAN	CENTRAL COA: P: 02 4325 5255	ST S.E. QLD P: 07 5582 6555	DELOOPD DELLA	PROPOSED DEVELOPMENT	Drawn:	JB	Horiz Vert	SY190226-	-01-104
D	NEW LOADING DOCK ADDED	JB 16/11	2020	STEWART			DEICORP Pty Ltd		Checked:	RD	X-Sect	File Ref.	REV.
Е	B99 PATHS TO BASEMENT PARKING	JB 20/11	2020	AL PROJECT SOLUTIONS		www.brs.com.au mail@brs.com.au		DAGENENT AGGEGG TURNING BATUR				SY190226	ח
F	B85 & B99 PATHS TO BASEMENT PARKING	JB 24/11	2020 ENGINE	ERING PLANNING PROJECT MANAGEMENT SURV	VEYING CERTIFICATION	ABN: 26 134 067 842		BASEMENT ACCESS TURNING PATHS			Datum: A.H.D.		U

Appendix E

Traffic Counts



Date #####

Project Site Cudgegong Road/Themeda Avenue

Project Number SY190226

Client Combined Projects (tallawong) Pty. Ltd.

TOTAL PROJECT SOLUTIONS

Cars

Time	Western Approach (EB	3)	Southern Approach (NB) Northern Approach (SB)	1!	5 min Total	1hr Total
	Left Straight	Right U-turn	Left Straight Right U-turn Left Straight Right	U-turn	S IIIII TOLAI	IIII TOtal
7:00	10	5	4 41 13	20	93	
7:15	9	19	11 41 24	24	128	
7:30	6	23	5 29 26	10	99	
7:45	4	39	10 38 51	15	157	477
8:00	6	42	12 25 54	12	151	535
8:15	9	37	14 41 50	5	156	563
8:30	7	45	4 58 44	5	163	627
8:45	14	38	72 72	2	198	668

Time	Eastern	Approach			Western	Approach			Norther	n Approach			Souther	n Approach	15 min Total	1hr Total		
Tille	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	13 mm Total	IIII TOtal
7:00				1						5	5		Ĭ		1		7	
7:15										10)						10	
7:30				1											3		4	
7:45		1								10)				5		16	37
8:00										2	2				7	1	10	40
8:15				1						4	1				6		11	41
8:30										2	2				3		5	42
8:45										2	2				4		6	32



Date 12-Mar-20

Project Site Cudgegong Road/Themeda Avenue

Project Number SY190226

Client Combined Projects (tallawong) Pty. Ltd.

Cars

	Time	Western Approa	ich (EB)						Souther	n Appro	oach (I	NB)		Northern	n Approach (SB)		15 min Total	1hr Total
L	Tillle	Left Strai	ght Right	U-turn	Left	Straight	Right	U-turn	Left	Stra	ight	Right	U-turn	Left	Straight	Right	U-turn	13 IIIII Totai	IIII TOLAI
	4:00	6		13							52				90)	6	167	
	4:15	14		20						7	32				69	9	2	144	
	4:30	6		16						11	31				45	5	3	112	
	4:45	13		13						5	42				30)	1	104	527
	5:00	12		24						6	49				43	3	4	138	498
	5:15	4		9						5	39				43	3	3	103	457
	5:30	10		27						6	41				34	1	9	127	472
	5:45	18		25						6	34				46	5	5	134	502
		39		73						22	171				150)	17		

Time	Eastern A	pproach			Western App	roach			Northern	Approach			Southerr	Approach			15 min Total	1hr Total
Tille	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	13 mm rotar	IIII TOLAI
4:00																	0	
4:15														:	3		3	
4:30										4	1			:	3		7	
4:45										1	L			:	3		4	14
5:00														:	1		1	15
5:15										1	L			:	2		3	15
5:30										1	L			:	2		3	11
5:45																	0	7



Project Site Cudgegong Road/Schofields Road

Project Number SY190226

Client Combined Projects (tallawong) Pty. Ltd.

Cars

Time	Eastern A	Approach (WB)			Western A	pproach (EB)			Northerr	n Approach (SB)		Southerr	n Approach (NB)		15 min Total	1hr Total
Time	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	13 mm rotar	IIII TOLAI
7:00	2	232				170	38						13		9		464	
7:15	1	213				156	30						15		28		443	
7:30		227				211	29						37		33		537	
7:45		228				178	29						26		24		485	1929
8:00	2	256				192	23						41		37		551	2016
8:15	3	233				188	21						34		26		505	2078
8:30	4	249				147	11						23		25		459	2000
8:45	1	230				174	16						24		21		466	1981

TTUCKS																		
Time	Eastern .	Approach			Western /	Approach			Norther	n Approach			Souther	n Approach			15 min Total	1hr Total
Title	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	13 IIIII Totai	IIII TOLAI
7:00		20				16	5						2		3		46	
7:15		24				11	5						5		1		46	
7:30		14				13	2						5		2		36	
7:45		26				13							6		4		49	177
8:00	1	23				15	1						9		8		57	188
8:15		18				13	1						7		4		43	185
8:30	1	19				10	3						7		4		44	193
8:45	1	16				17	5						5		3		47	191



Project Site Cudgegong Road/Rouse Road

Project Number SY190226

Client Combined Projects (tallawong) Pty. Ltd.

Cars

Time	Eastern /	Approach (W	√B)		Western Ap	proach (EB)			Northerr	ո Approach (SB)		Southern	Approach (NB)		15 min Total	1hr Total
	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	13 IIIII Total	IIII TOLAI
4:00	4	204				172	26						16		18		440	
4:15	3	224				176	18						34		26		481	
4:30	3	256				192	15						41		37		544	
4:45	1	271				236	22						27		25		582	2047
5:00	2	263				167	16						31		41		520	2127
5:15	1	236				212	31						26		28		534	2180
5:30	1	299				265	36						23		24		648	2284
5:45	4	200				162	11						13		17		407	2109

Time	Eastern	Approach			Western Ap	proach			Norther	n Approach			Southern	n Approach			15 min Total	1hr Total
Tille	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	13 mm rotar	IIII TOtal
4:00	1	8				10	3						2		1		25	
4:15		8				3	1						2		4		18	
4:30	1	23				15	1						9		8		57	
4:45		7				15	3						4		4		33	133
5:00		9				9	1						1		2		22	130
5:15		14				15	7						3				39	151
5:30		11				13	4						4		1		33	127
5:45	1	7				8	1						1				18	112



Project Site Schofields Road/Tallawong Road

Project Number SY190226

Client Combined Projects (tallawong) Pty. Ltd.

Cars

Time		Eastern	Approach (WE	3)		Western Appr	oach (EB)			Northern Ap	proach (SE	3)		Southern Ap	oroach (NB)	15 min Total	1hr Total
Tille	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	13 IIIII Totai	IIII TOLAI
7:00	3	152	4		29	163	12		8	24	10		9	53	14		481	
7:15	14	171	2		33	159	9		10	28	24		24	62	18		554	
7:30	13	188	14		26	195	31		9	42	23		22	42	33		638	
7:45	7	206	15		7	195	28		4	52	67		45	41	38		705	2378
8:00	1	151	22		6	144	42		9	24	14		19	28	32		492	2389
8:15	2	143	13		7	184	15		5	12	7		30	22	41		481	2316
8:30	7	114	19		3	127	20		2	17	11		14	9	21		364	2042
8:45	5	117	7		17	148	14		17	25	18		25	35	28		456	1793

Time		Easte	ern Approach			Western Ap	proach			Northern A	Approach			Southern	Approach		15 min Total	1hr Total
Tille	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	13 mm Total	IIII TOLAI
7:00	1	7	0		2	15			1	0	3		1	3	3		36	
7:15	3	14	3		1	20	5		1	3	6		3	1			60	
7:30		15	1		5	22			1	4	1		2	3	1		55	
7:45		17	4		1	15	4		1	2	6			3	3		56	207
8:00	1	17	3		1	14	3		1	17	3		1	1	1		63	234
8:15		6	2		2	19	6			1	3		2	1	3		45	219
8:30	1	19	3		1	12	2			3	4		3	1	1		50	214
8:45	1	9	3		3	4			2	4	4		1	1	1		33	191



Project Site Tallawong Road/Cudgegong Road

Project Number SY190226

Client Combined Projects (tallawong) Pty. Ltd.

Cars

Time		Easterr	n Approach ((WB)		Western App	roach (EB)			Northern Ap	proach (SI	B)		Southern App	oroach (NE	В)	15 min Total	1hr Total
Tillle	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	15 IIIII 10tai	IIII TOLAI
4:00		130	7		5	138	21		2	28	19		13	25	23		411	
4:15	2	209	4			195	20		9	22	41		16	21	33		572	
4:30	2	197	11		4	197	31		10	27	40		21	18	30		588	
4:45	9	215	7		4	175	27		12	33	59		19	36	26		622	2193
5:00	12	233	11		5	201	36		12	29	41		22	19	28		649	2431
5:15	6	211	10		1	178	16		11	38	76		15	17	33		612	2471
5:30	13	224	8		1	192	25		8	43	51		21	27	33		646	2529
5:45	7	184	9			149	33		10	37	73		22	25	39		588	2495
6:00																	0	1846

Time		East	ern Approa	ch		Western A	pproach			Northern A	Approach			Southern	Approach		15 min Total	1hr Total
Tille	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	13 IIIII TOtai	IIII TOLAI
4:00		13	1			8	3		1	3	2		6	1	1		39	
4:15		21	2			10	2			1					1		37	
4:30	1	8			1	13			2	2	9		2		2		40	
4:45	1	13				14	4		2	1	8		1				44	160
5:00	1	6				9	2		1	1	3						23	144
5:15		6			1	5	2			1	4			1			20	127
5:30		7				14				2	4		1		1		29	116
5:45	1	8				6			2	3	2			1			23	95
6:00									1								0	72



Date 12-Mar-20

Project Site Tallawong Road/Themeda Av

Project Number SY190226

Client Combined Projects (tallawong) Pty. Ltd.

Time	Eastern A	Approach (WB)			Western Ap	oproach (EB)			Northern	Approach (SB)		Southern	n Approach	(NB)		15 min Total	1hr Total
Tille	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	13 IIIII Total	IIII TOLAI
7:00					2	23		2		38	1	L40	1	.9 :	36		258	
7:15					2	25		3		42	1	.67	1	.2	17		296	
7:30					4	13		2		41	. 1	.24		7 !	59		276	
7:45					2	19		5		43		94	1	.4 !	56		241	1071
8:00					3	18		1		44		92		4	10		219	1032
8:15					2	20		2		62		85		6	10		215	951
8:30					1	.0		2		51		43		5	10		151	826
8:45					1	.5		2		27		32		3 3	36		115	700
_					13	80		10		190		195	3	1 10	95			

Trucks

Cars

TTUCKS																		
Time	Eastern	Approach			Western A	Approach			Northern	n Approach			Souther	n Approach			15 min Total	1hr Total
Tille	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	13 mm Total	IIII TOtal
7:00					Ĭ					6	5				5		11	
7:15										4	ļ				6		10	
7:30								1		4	ļ				3		8	
7:45						1				2	2	1			3		7	36
8:00										4	ļ				3		7	32
8:15										4	1	1			2		7	29
8:30										4	ļ				5		9	30
8:45										5	5						5	28



Project Site Tallawong Road/Themeda Av

Project Number SY190226

Client Combined Projects (tallawong) Pty. Ltd.

Cars

	Time	Eastern A	Approach (W	/B)		Western A	Approach (EB)			Northern	Approach (SB)		Southern	n Approach (NB)		15 min Total	1hr Total
	Tille	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	13 IIIII Totai	IIII TOtal
I	4:00						19		5		3:	1	25		5 2	9		114	
	4:15						7		2		34	4	38		4 3	1		116	
	4:30						15		6		33	2	30		1 3	9		123	
	4:45						17		4		3:	1	35		3 4	1		131	484
	5:00						27		7		3:	1	42		3 4	4		154	524
	5:15						15		4		3:	1	41		6 2	6		123	531
	5:30						27		10		43	3	44		3 3	5		162	570
	5:45						28		9		3	3	58		2 3	0		160	599
	6:00																	0	445
_		-				-	86		25	-	130	6	162	1	.5 14	6			-

Time	Eastern A	pproach			Western Ap	proach			Northern	Approach			Souther	n Approach			15 min Total	1hr Total
Tille	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	Left	Straight	Right	U-turn	13 IIIII TOtal	IIII TOtal
4:00															2		2	
4:15											3	1			3		7	
4:30								1			4			1			6	
4:45															1		1	16
5:00											2	1					3	17
5:15											3				1		4	14
5:30											3		_		2		5	13
5:45											1						1	13
6:00																	0	10

Appendix F1 SIDRA Movement Summary

Existing



Site: 101 [Cudgegong / Themeda Existing AM]

♦ Network: N101 [Tallawong **Existing AM]**

Cudgegong / Conferta Existing AM Site Category: (None)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV		Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance		Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Cudg	egong Roa	ad											
1	L2	43	0.0	43	0.0	0.151	19.1	LOS B	1.4	11.2	0.77	0.67	0.77	28.7
2	T1	171	17.9	171	17.9	0.151	16.7	LOS B	1.8	13.7	0.88	0.73	0.88	31.3
Appro	ach	214	14.3	214	14.3	0.151	17.2	LOS B	1.8	13.7	0.86	0.72	0.86	30.8
North:	: Cudge	egong Roa	ıd											
8	T1	222	14.2	222	14.2	0.208	13.1	LOS A	1.8	13.9	0.70	0.59	0.70	16.4
9	R2	45	2.3	45	2.3	0.208	18.4	LOS B	1.5	11.1	0.71	0.64	0.71	14.9
Appro	ach	267	12.2	267	12.2	0.208	14.0	LOS A	1.8	13.9	0.70	0.60	0.70	16.1
West:	Theme	eda Ave												
10	L2	31	13.8	31	13.8	0.045	11.3	LOS A	0.2	1.5	0.36	0.60	0.36	37.7
12	R2	155	4.1	155	4.1	0.214	10.3	LOS A	0.9	6.5	0.33	0.62	0.33	38.6
Appro	ach	185	5.7	185	5.7	0.214	10.5	LOS A	0.9	6.5	0.34	0.62	0.34	38.4
All Ve	hicles	666	11.1	666	11.1	0.214	14.1	LOS A	1.8	13.9	0.65	0.64	0.65	30.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pede	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	11	24.3	LOS C	0.0	0.0	0.90	0.90
P3	North Full Crossing	13	24.3	LOS C	0.0	0.0	0.90	0.90
P4	West Full Crossing	21	24.3	LOS C	0.0	0.0	0.90	0.90
All Pe	edestrians	44	24.3	LOS C			0.90	0.90

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Thursday, 16 April 2020 4:51:13 PM



Site: CT [Cudgegong / Themeda Existing PM]

♦♦ Network: N101 [Tallawong **Existing PM**]

Cudgegong / Conferta Existing PM Site Category: (None)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Cudg	egong Roa	ad											
1	L2	23	0.0	23	0.0	0.141	17.8	LOS B	1.3	10.9	0.74	0.63	0.74	31.4
2	T1	201	10.5	201	10.5	0.141	15.2	LOS B	1.9	13.5	0.86	0.70	0.86	33.0
Appro	ach	224	9.4	224	9.4	0.141	15.4	LOS B	1.9	13.5	0.85	0.69	0.85	32.9
North	: Cudge	egong Roa	d											
8	T1	175	9.6	175	9.6	0.129	11.1	LOS A	1.1	8.4	0.63	0.52	0.63	18.7
9	R2	18	0.0	18	0.0	0.129	15.8	LOS B	1.0	7.3	0.63	0.54	0.63	18.0
Appro	ach	193	8.7	193	8.7	0.129	11.5	LOS A	1.1	8.4	0.63	0.52	0.63	18.6
West:	Theme	eda Ave												
10	L2	41	0.0	41	0.0	0.060	9.5	LOS A	0.2	1.3	0.26	0.59	0.26	39.0
12	R2	82	6.4	82	6.4	0.126	10.1	LOS A	0.4	3.1	0.29	0.60	0.29	38.8
Appro	ach	123	4.3	123	4.3	0.126	9.9	LOSA	0.4	3.1	0.28	0.60	0.28	38.9
All Ve	hicles	540	8.0	540	8.0	0.141	12.8	LOS A	1.9	13.5	0.64	0.61	0.64	32.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pede	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	14	24.3	LOS C	0.0	0.0	0.90	0.90
P3	North Full Crossing	9	24.3	LOS C	0.0	0.0	0.90	0.90
P4	West Full Crossing	16	24.3	LOS C	0.0	0.0	0.90	0.90
All Pe	edestrians	39	24.3	LOS C			0.90	0.90

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Site: SC [SCHOFIELDS / CUDGEGONG - AM]

♦ Network: N101 [Tallawong **Existing AM]**

SCHOFIELDS ROAD / CUDGEGONG ROAD

Site Category: (None)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance		Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
East:	SCHO	FIELDS R	DAC											
5	T1	874	7.3	874	7.3	0.484	11.9	LOS A	5.3	38.9	0.73	0.63	0.73	43.3
6	R2	114	5.6	114	5.6	0.322	34.9	LOS C	1.0	7.5	0.97	0.74	0.97	28.3
Appro	ach	987	7.1	987	7.1	0.484	14.6	LOS B	5.3	38.9	0.76	0.65	0.76	40.8
North	: CUDO	SEGONG I	ROAD											
7	L2	174	16.4	174	16.4	0.313	26.8	LOS B	2.9	22.8	0.94	0.81	0.94	37.0
9	R2	145	13.0	145	13.0	0.638	35.0	LOS C	2.7	21.2	1.00	0.84	1.11	18.3
Appro	ach	319	14.9	319	14.9	0.638	30.5	LOSC	2.9	22.8	0.97	0.82	1.02	30.2
West:	SCHO	FIELDS R	OAD											
10	L2	12	54.5	12	54.5	0.018	15.6	LOS B	0.1	1.3	0.61	0.63	0.61	38.0
11	T1	1068	7.0	1068	7.0	0.614	15.9	LOS B	8.5	63.3	0.92	0.81	0.92	46.6
Appro	ach	1080	7.5	1080	7.5	0.614	15.9	LOS B	8.5	63.3	0.91	0.80	0.91	46.5
All Ve	hicles	2386	8.3	2386	8.3	0.638	17.3	LOS B	8.5	63.3	0.86	0.74	0.86	42.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Ped	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P2	East Full Crossing	8	24.3	LOS C	0.0	0.0	0.90	0.90
P3	North Full Crossing	16	24.3	LOS C	0.0	0.0	0.90	0.90
P4	West Full Crossing	13	24.3	LOS C	0.0	0.0	0.90	0.90
All Pe	edestrians	37	24.3	LOS C			0.90	0.90

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.



Site: SC [SCHOFIELDS / CUDGEGONG - PM]

♦♦ Network: N101 [Tallawong **Existing PM**]

SCHOFIELDS ROAD / CUDGEGONG ROAD

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 60 seconds (Network Practical Cycle Time)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Bacl Vehicles	of Queue Distance	Prop. Queued	Effective A	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m		Rate		km/h
East:	SCHO	FIELDS R	DAC											
5	T1	947	2.2	947	2.2	0.508	11.5	LOS A	5.9	41.8	0.73	0.64	0.73	43.7
6	R2	127	13.2	127	13.2	0.379	35.3	LOS C	1.2	9.1	0.97	0.75	0.97	28.1
Appro	oach	1075	3.5	1075	3.5	0.508	14.4	LOS A	5.9	41.8	0.76	0.65	0.76	41.0
North	: CUDO	GEGONG I	ROAD											
7	L2	132	14.4	132	14.4	0.247	26.6	LOS B	2.1	16.8	0.93	0.79	0.93	37.1
9	R2	134	7.1	134	7.1	0.645	35.7	LOS C	2.5	18.5	0.99	0.83	1.09	18.0
Appro	oach	265	10.7	265	10.7	0.645	31.2	LOS C	2.5	18.5	0.96	0.81	1.01	29.2
West	: SCHO	FIELDS R	OAD											
10	L2	5	0.0	5	0.0	0.006	14.3	LOS A	0.1	0.4	0.59	0.61	0.59	38.6
11	T1	1166	3.5	1166	3.5	0.633	14.2	LOS A	8.9	64.4	0.88	0.78	0.88	47.7
Appro	oach	1172	3.5	1172	3.5	0.633	14.2	LOS A	8.9	64.4	0.88	0.78	0.88	47.7
All Ve	hicles	2512	4.3	2512	4.3	0.645	16.0	LOS B	8.9	64.4	0.84	0.73	0.84	43.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Ped	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P2	East Full Crossing	18	24.3	LOS C	0.0	0.0	0.90	0.90
P3	North Full Crossing	21	24.3	LOS C	0.0	0.0	0.90	0.90
P4	West Full Crossing	13	24.3	LOS C	0.0	0.0	0.90	0.90
All Pe	edestrians	52	24.3	LOS C			0.90	0.90

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Thursday, 16 April 2020 4:55:39 PM

Site: 101 [SCHOFIELDS / TALLAWONG / RIDGELINE - AM]

♦♦ Network: N101 [Tallawong Existing AM]

SCHOFIELDS / TALLAWONG / RIDGELINE

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 60 seconds (Network Practical Cycle Time)

Mov	ement	t Perform	ance	- Vehi	cles									
Mov ID	Turn	Demand	Flows	Arrival	Flows	Deg. Satn	Average Delay	Level of Service	Aver. Bad Queue		Prop. Queued	Effective Stop	Aver. A No.	Averag e
		Total		Total	HV				Vehicles Di	stance		Rate	Cycles S	Speed
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
		SELINE DE												
1	L2	126	3.3	126	3.3	0.226	22.9	LOS B	1.7	12.5	0.79	0.75	0.79	42.7
2	T1	143	2.2	143	2.2	0.737	32.3	LOS C	2.8	20.1	1.00	0.88	1.25	29.8
3	R2	158	4.0	158	4.0	0.742	37.4	LOS C	3.1	22.4	1.00	0.90	1.25	27.5
Appr	oach	427	3.2	427	3.2	0.742	31.4	LOS C	3.1	22.4	0.94	0.85	1.12	33.5
East	SCHO	FIELDS R	ROAD											
4	L2	24	0.0	24	0.0	0.025	14.0	LOS A	0.2	1.7	0.59	0.66	0.59	47.3
5	T1	772	6.1	772	6.1	0.722	26.2	LOS B	7.2	52.9	0.99	0.87	1.03	40.8
6	R2	78	13.5	78	13.5	0.462	35.7	LOS C	1.5	11.4	1.00	0.76	1.00	25.1
Appr	oach	874	6.6	874	6.6	0.722	26.7	LOS B	7.2	52.9	0.98	0.85	1.02	39.8
North	n: TALL	AWONG F	ROAD											
7	L2	35	18.2	35	18.2	0.070	27.1	LOS B	0.6	4.7	0.95	0.74	0.95	18.2
8	T1	146	6.5	146	6.5	0.774	34.8	LOS C	2.9	21.6	1.00	0.85	1.17	33.0
9	R2	128	9.0	128	9.0	0.312	33.8	LOS C	1.1	8.5	0.95	0.75	0.95	33.0
Appr	oach	309	8.8	309	8.8	0.774	33.5	LOS C	2.9	21.6	0.98	0.80	1.06	32.1
West	t: SCH	OFIELDS F	ROAD											
10	L2	49	2.1	49	2.1	0.051	13.9	LOS A	0.5	3.2	0.54	0.66	0.54	41.7
11	T1	808	6.5	808	6.5	0.752	23.9	LOS B	7.2	52.8	0.97	0.90	1.09	34.1
12	R2	125	2.5	125	2.5	0.697	37.5	LOS C	2.4	17.5	1.00	0.86	1.21	36.8
Appr	oach	983	5.8	983	5.8	0.752	25.2	LOS B	7.2	52.8	0.95	0.88	1.08	34.9
All V	ehicles	2594	6.0	2594	6.0	0.774	27.7	LOS B	7.2	52.9	0.96	0.86	1.06	36.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Ave Service Pe		of Queue Distance m	Prop. E Queued St	ffective op Rate					
P1	South Full Crossing	11	24.3	LOS C	0.0	0.0	0.90	0.90					
P2	East Full Crossing	13	24.3	LOS C	0.0	0.0	0.90	0.90					
P3	North Full Crossing	14	24.3	LOS C	0.0	0.0	0.90	0.90					
P4	West Full Crossing	16	24.3	LOS C	0.0	0.0	0.90	0.90					
All Pe	edestrians	53	24.3	LOS C			0.90	0.90					

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay) Pedestrian movement LOS values are based on average delay per pedestrian movement. Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: 101 [SCHOFIELDS / TALLAWONG / RIDGELINE - PM]

♦ Network: N101 [Tallawong **Existing PM**]

SCHOFIELDS / TALLAWONG / RIDGELINE

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 60 seconds (Network Practical Cycle Time)

Мо	vement	Performa	ance -	Vehic	les									
Mov	/ Turn	Demand				Deg.	Average	Level of	Aver. Back			Effective A		
ID		Total	HV	Total	HV	Satn	Delay	Service	venicies	Distance	Queuea	Stop Rate	Cycles S	speed
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
Sou		SELINE DR												
1	L2	84	3.8	84	3.8	0.151	22.5	LOS B	1.1	8.2	0.77	0.73	0.77	42.9
2	T1	105	1.0	105	1.0	0.538	30.0	LOS C	2.0	13.8	0.99	0.78	1.02	30.9
3	R2	127	0.8	127	0.8	0.683	37.2	LOS C	2.5	17.4	1.00	0.85	1.18	27.5
App	roach	317	1.7	317	1.7	0.683	30.9	LOS C	2.5	17.4	0.94	0.80	1.02	33.4
Eas	t: SCHO	FIELDS R	DAC											
4	L2	46	9.1	46	9.1	0.038	8.2	LOS A	0.3	1.9	0.44	0.64	0.44	51.1
5	T1	959	3.1	959	3.1	0.836	29.3	LOS C	9.5	67.8	1.00	0.95	1.13	39.2
6	R2	38	0.0	38	0.0	0.205	34.3	LOS C	0.7	4.8	0.97	0.72	0.97	25.6
App	roach	1043	3.2	1043	3.2	0.836	28.6	LOS C	9.5	67.8	0.97	0.92	1.10	39.3
Nor	th: TALL	AWONG R	OAD											
7	L2	49	8.5	49	8.5	0.093	26.3	LOS B	8.0	6.0	0.92	0.74	0.92	18.6
8	T1	156	3.4	156	3.4	0.808	35.4	LOS C	3.2	22.8	1.00	0.91	1.31	32.8
9	R2	255	6.2	255	6.2	0.709	37.4	LOS C	2.5	18.6	1.00	0.85	1.17	31.5
App	roach	460	5.5	460	5.5	0.808	35.6	LOS C	3.2	22.8	0.99	0.86	1.19	31.3
We	st: SCHC	FIELDS R	OAD											
10	L2	20	42.1	20	42.1	0.018	7.3	LOS A	0.1	0.6	0.28	0.59	0.28	49.0
11	T1	814	3.5	814	3.5	0.706	21.7	LOS B	6.9	49.2	0.94	0.85	1.01	35.5
12	R2	117	6.3	117	6.3	0.668	37.2	LOS C	2.3	16.7	1.00	0.85	1.17	36.9
App	roach	951	4.7	951	4.7	0.706	23.3	LOS B	6.9	49.2	0.94	0.84	1.01	36.0
All '	/ehicles	2771	3.9	2771	3.9	0.836	28.2	LOS B	9.5	67.8	0.96	0.87	1.07	36.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians												
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate					
P1	South Full Crossing	13	24.3	LOS C	0.0	0.0	0.90	0.90					
P2	East Full Crossing	18	24.3	LOS C	0.0	0.0	0.90	0.90					
P3	North Full Crossing	21	24.3	LOS C	0.0	0.0	0.90	0.90					
P4	West Full Crossing	15	24.3	LOS C	0.0	0.0	0.90	0.90					
All Pe	destrians	66	24.3	LOS C			0.90	0.90					



Site: 101 [Tallawong / Themeda Existing AM]

Tallawong / Themeda Existing AM Site Category: (None)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Tallav	vong Road												
2	T1	227	9.7	227	9.7	0.106	8.2	LOS A	1.6	11.9	0.63	0.51	0.63	39.7
3	R2	45	27.9	45	27.9	0.292	37.4	LOS C	0.9	7.5	1.00	0.74	1.00	14.4
Appro	ach	273	12.7	273	12.7	0.292	13.0	LOS A	1.6	11.9	0.69	0.55	0.69	32.4
East:	Theme	da Ave												
4	L2	154	11.0	154	11.0	0.412	28.4	LOS B	2.5	19.0	0.90	0.79	0.90	30.4
6	R2	13	16.7	13	16.7	0.035	26.1	LOS B	0.2	1.5	0.81	0.67	0.81	32.2
Appro	ach	166	11.4	166	11.4	0.412	28.2	LOS B	2.5	19.0	0.90	0.78	0.90	30.6
North	: Tallaw	ong Road												
7	L2	201	0.5	201	0.5	0.435	19.8	LOS B	4.2	29.9	0.79	0.75	0.79	13.3
8	T1	429	3.2	429	3.2	0.435	15.0	LOS B	4.3	31.0	0.79	0.69	0.79	14.8
Appro	ach	631	2.3	631	2.3	0.435	16.5	LOS B	4.3	31.0	0.79	0.71	0.79	14.3
All Ve	hicles	1069	6.4	1069	6.4	0.435	17.5	LOS B	4.3	31.0	0.78	0.68	0.78	23.9

♦ Network: N101 [Tallawong

Existing AM]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate						
P1	South Full Crossing	18	24.3	LOS C	0.0	0.0	0.90	0.90						
P2	East Full Crossing	13	24.3	LOS C	0.0	0.0	0.90	0.90						
P3	North Full Crossing	6	24.3	LOS C	0.0	0.0	0.90	0.90						
All Pe	edestrians	37	24.3	LOS C			0.90	0.90						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Thursday, 16 April 2020 4:51:13 PM



Site: TT [Tallawong / Themeda Existing PM]

♦♦ Network: N101 [Tallawong **Existing PM**]

Tallawong / Themeda Existing PM Site Category: (None)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Tallav	vong Road												
2	T1	152	5.6	152	5.6	0.067	7.8	LOS A	1.0	7.1	0.65	0.51	0.65	40.3
3	R2	183	6.9	183	6.9	0.296	23.2	LOS B	2.8	21.0	0.89	0.80	0.89	20.2
Appro	ach	335	6.3	335	6.3	0.296	16.2	LOS B	2.8	21.0	0.78	0.67	0.78	27.5
East:	Theme	da Ave												
4	L2	103	12.2	103	12.2	0.302	28.7	LOS C	1.6	12.8	0.89	0.76	0.89	30.3
6	R2	27	3.8	27	3.8	0.076	27.2	LOS B	0.4	3.0	0.84	0.70	0.84	31.7
Appro	ach	131	10.5	131	10.5	0.302	28.4	LOS B	1.6	12.8	0.88	0.75	0.88	30.6
North	: Tallaw	ong Road												
7	L2	18	11.8	18	11.8	0.309	30.5	LOS C	1.5	10.7	0.93	0.73	0.93	9.7
8	T1	158	2.7	158	2.7	0.309	25.8	LOS B	1.5	10.7	0.93	0.72	0.93	9.9
Appro	ach	176	3.6	176	3.6	0.309	26.3	LOS B	1.5	10.7	0.93	0.72	0.93	9.9
All Ve	hicles	641	6.4	641	6.4	0.309	21.5	LOS B	2.8	21.0	0.84	0.70	0.84	24.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate						
P1	South Full Crossing	21	24.3	LOS C	0.0	0.0	0.90	0.90						
P2	East Full Crossing	16	24.3	LOS C	0.0	0.0	0.90	0.90						
P3	North Full Crossing	18	24.3	LOS C	0.0	0.0	0.90	0.90						
All Pe	edestrians	55	24.3	LOS C			0.90	0.90						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Appendix F2 SIDRA Movement Summary

Existing + Development



Site: CT [Cudgegong / Themeda Existing AM + Devt]

♦ Network: N101 [Tallawong Existing AM + Devt]

Cudgegong / Conferta Existing AM + Devt

Site Category: (None)

Movement Performance - Vehicles														
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles			Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Cudg	egong Roa	ad											
1	L2	43	0.0	43	0.0	0.261	33.5	LOS C	2.3	18.6	0.98	0.79	0.98	19.9
2	T1	197	15.5	197	15.5	0.261	29.2	LOS C	2.6	19.6	0.99	0.80	0.99	23.4
Appro	ach	240	12.7	240	12.7	0.261	30.0	LOS C	2.6	19.6	0.99	0.79	0.99	22.8
North:	: Cudge	egong Roa	ıd											
8	T1	222	14.2	222	14.2	0.347	24.0	LOS B	2.8	21.9	0.87	0.71	0.87	10.4
9	R2	54	2.0	54	2.0	0.347	30.1	LOS C	2.1	16.0	0.88	0.74	0.88	9.4
Appro	ach	276	11.8	276	11.8	0.347	25.2	LOS B	2.8	21.9	0.87	0.72	0.87	10.2
West:	Theme	eda Ave												
10	L2	31	13.8	31	13.8	0.032	11.7	LOS A	0.3	2.3	0.49	0.64	0.49	37.4
12	R2	375	1.7	375	1.7	0.357	12.8	LOS A	4.0	28.6	0.55	0.72	0.55	36.6
Appro	ach	405	2.6	405	2.6	0.357	12.7	LOS A	4.0	28.6	0.54	0.71	0.54	36.7
All Ve	hicles	921	8.0	921	8.0	0.357	20.9	LOS B	4.0	28.6	0.76	0.73	0.76	27.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate						
P1	South Full Crossing	13	29.3	LOS C	0.0	0.0	0.91	0.91						
P3	North Full Crossing	15	29.3	LOS C	0.0	0.0	0.91	0.91						
P4	West Full Crossing	18	29.3	LOS C	0.0	0.0	0.91	0.91						
All Pe	edestrians	45	29.3	LOS C			0.91	0.91						

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Wednesday, 3 June 2020 4:55:36 PM



Site: CT [Cudgegong / Themeda Existing PM + DEVT]

♦ Network: N101 [Tallawong **Existing PM + Devt]**

Cudgegong / Conferta Existing PM + DEVT

Site Category: (None)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV		l Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles I			Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Cudg	egong Roa	ad											
1	L2	23	0.0	23	0.0	0.219	25.6	LOS B	1.7	13.9	0.75	0.63	0.75	24.8
2	T1	218	9.7	218	9.7	0.219	24.7	LOS B	2.6	18.8	0.89	0.73	0.89	26.0
Appro	ach	241	8.7	241	8.7	0.219	24.8	LOS B	2.6	18.8	0.88	0.72	0.88	25.9
North	: Cudge	egong Roa	d											
8	T1	175	9.6	175	9.6	0.220	20.6	LOS B	1.9	14.6	0.80	0.65	0.80	11.8
9	R2	37	0.0	37	0.0	0.220	26.6	LOS B	1.5	11.1	0.81	0.69	0.81	10.7
Appro	ach	212	8.0	212	8.0	0.220	21.7	LOS B	1.9	14.6	0.80	0.66	0.80	11.6
West:	Theme	eda Ave												
10	L2	41	0.0	41	0.0	0.042	6.7	LOS A	0.1	0.7	0.13	0.56	0.13	41.3
12	R2	212	2.5	212	2.5	0.219	7.1	LOS A	0.7	4.9	0.17	0.58	0.17	41.6
Appro	ach	253	2.1	253	2.1	0.219	7.1	LOSA	0.7	4.9	0.16	0.58	0.16	41.5
All Ve	hicles	705	6.1	705	6.1	0.220	17.5	LOS B	2.6	18.8	0.60	0.65	0.60	29.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pede	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	14	29.3	LOS C	0.0	0.0	0.91	0.91
P3	North Full Crossing	24	29.3	LOS C	0.0	0.0	0.92	0.92
P4	West Full Crossing	19	29.3	LOS C	0.0	0.0	0.92	0.92
All Pe	edestrians	57	29.3	LOS C			0.92	0.92

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Wednesday, 3 June 2020 4:55:58 PM

Site: SC [SCHOFIELDS / CUDGEGONG - EXISTING AM + DEVT]

♦♦ Network: N101 [Tallawong Existing AM + Devt]

SCHOFIELDS ROAD / CUDGEGONG ROAD EXISTING AM +DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
East:	SCHO	FIELDS R	DAC											
5	T1	874	7.3	874	7.3	0.439	11.4	LOS A	5.6	40.9	0.67	0.58	0.67	43.8
6	R2	185	3.4	185	3.4	0.602	42.1	LOS C	2.1	14.8	1.00	0.80	1.09	25.4
Appro	ach	1059	6.7	1059	6.7	0.602	16.8	LOS B	5.6	40.9	0.72	0.62	0.74	38.9
North	: CUDO	SEGONG I	ROAD											
7	L2	394	7.2	394	7.2	0.709	29.9	LOS C	7.3	54.5	0.90	0.85	0.94	35.7
9	R2	145	13.0	145	13.0	0.595	32.4	LOS C	2.8	21.6	0.90	0.78	0.91	19.3
Appro	ach	539	8.8	539	8.8	0.709	30.6	LOS C	7.3	54.5	0.90	0.83	0.93	32.6
West:	SCHO	FIELDS R	OAD											
10	L2	12	54.5	12	54.5	0.016	15.0	LOS B	0.1	1.1	0.46	0.60	0.46	38.5
11	T1	1068	7.0	1068	7.0	0.557	12.1	LOS A	7.8	58.2	0.70	0.62	0.70	49.2
Appro	ach	1080	7.5	1080	7.5	0.557	12.2	LOS A	7.8	58.2	0.70	0.62	0.70	49.1
All Ve	hicles	2678	7.4	2678	7.4	0.709	17.7	LOS B	7.8	58.2	0.75	0.66	0.76	42.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pede	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P2	East Full Crossing	13	29.3	LOS C	0.0	0.0	0.91	0.91
P3	North Full Crossing	21	29.3	LOS C	0.0	0.0	0.92	0.92
P4	West Full Crossing	19	29.3	LOS C	0.0	0.0	0.92	0.92
All Pe	destrians	53	29.3	LOS C			0.91	0.91

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Wednesday, 3 June 2020 4:55:36 PM Project: C:\Users\robert\Documents\Tallawong\Sidra\[SY190226] EXISTING + Devt.sip8

Site: SC [SCHOFIELDS / CUDGEGONG - EXISTING PM + DEVT]

♦♦ Network: N101 [Tallawong Existing PM + Devt]

SCHOFIELDS ROAD / CUDGEGONG ROAD EXISTING PM +DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
East:	SCHO	FIELDS R	DAC											
5	T1	947	2.2	947	2.2	0.505	13.3	LOS A	6.8	48.2	0.73	0.64	0.73	42.0
6	R2	283	5.9	283	5.9	0.625	39.3	LOS C	3.0	22.3	0.99	0.83	1.07	26.5
Appro	oach	1231	3.1	1231	3.1	0.625	19.2	LOS B	6.8	48.2	0.79	0.68	0.81	37.0
North	: CUDO	GEGONG I	ROAD											
7	L2	261	7.3	261	7.3	0.431	32.7	LOS C	5.3	39.2	1.00	0.84	1.00	34.4
9	R2	134	7.1	134	7.1	0.585	42.0	LOS C	2.9	21.8	1.00	0.80	1.02	16.0
Appro	oach	395	7.2	395	7.2	0.585	35.8	LOS C	5.3	39.2	1.00	0.83	1.01	29.5
West	: SCHO	FIELDS R	OAD											
10	L2	5	0.0	5	0.0	0.006	15.1	LOS B	0.1	0.4	0.51	0.60	0.51	37.9
11	T1	1166	3.5	1166	3.5	0.630	14.4	LOS A	9.4	67.9	0.81	0.72	0.81	47.6
Appro	oach	1172	3.5	1172	3.5	0.630	14.4	LOS A	9.4	67.9	0.81	0.72	0.81	47.5
All Ve	hicles	2797	3.8	2797	3.8	0.630	19.6	LOS B	9.4	67.9	0.83	0.72	0.84	40.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pede	strians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P2	East Full Crossing	19	29.3	LOS C	0.0	0.0	0.92	0.92
P3	North Full Crossing	24	29.3	LOS C	0.0	0.0	0.92	0.92
P4	West Full Crossing	21	29.3	LOS C	0.0	0.0	0.92	0.92
All Pe	edestrians	64	29.3	LOS C			0.92	0.92

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Wednesday, 3 June 2020 4:55:58 PM Project: C:\Users\robert\Documents\Tallawong\Sidra\[SY190226] EXISTING + Devt.sip8

Site: STR [SCHOFIELDS / TALLAWONG / RIDGELINE - AM + Property Prop

SCHOFIELDS / TALLAWONG / RIDGELINE Existing AM + Devt

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

Мо	vement	Performa	ance -	Vehic	les									
Mo\ ID	/ Turn	Demand Total		Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
Sou		ELINE DR												
1	L2	126	3.3	126	3.3	0.170	20.2	LOS B	1.7	12.3	0.68	0.73	0.68	44.1
2	T1	217	1.5	217	1.5	0.486	26.0	LOS B	4.1	28.8	0.92	0.75	0.92	33.0
3	R2	158	4.0	158	4.0	0.865	47.9	LOS D	3.9	28.2	1.00	1.01	1.53	23.8
App	roach	501	2.7	501	2.7	0.865	31.4	LOS C	4.1	28.8	0.88	0.83	1.05	33.0
Eas	t: SCHO	FIELDS R	DAC											
4	L2	24	0.0	24	0.0	0.030	17.9	LOS B	0.3	2.1	0.63	0.67	0.63	44.8
5	T1	772	6.1	772	6.1	0.842	35.4	LOS C	8.8	64.7	1.00	0.94	1.14	36.6
6	R2	78	13.5	78	13.5	0.539	41.9	LOS C	1.7	13.6	1.00	0.78	1.03	22.7
App	roach	874	6.6	874	6.6	0.842	35.5	LOS C	8.8	64.7	0.99	0.92	1.12	35.9
Nor	th: TALL	AWONG R	OAD											
7	L2	35	18.2	35	18.2	0.052	25.3	LOS B	0.6	5.0	0.89	0.73	0.89	19.1
8	T1	408	2.3	408	2.3	0.921	44.9	LOS D	10.2	72.7	1.00	1.03	1.30	29.2
9	R2	155	7.5	155	7.5	0.434	40.0	LOS C	1.6	12.2	0.98	0.76	0.98	30.6
Арр	roach	598	4.6	598	4.6	0.921	42.5	LOS C	10.2	72.7	0.99	0.94	1.20	29.3
Wes	st: SCHC	FIELDS R	OAD											
10	L2	71	1.5	71	1.5	0.085	18.3	LOS B	0.9	6.2	0.62	0.69	0.62	37.9
11	T1	808	6.5	808	6.5	0.878	37.6	LOS C	9.9	72.7	1.00	1.07	1.36	27.4
12	R2	125	2.5	125	2.5	0.814	46.0	LOS D	3.0	21.4	1.00	0.94	1.41	33.9
App	roach	1004	5.7	1004	5.7	0.878	37.3	LOS C	9.9	72.7	0.97	1.03	1.32	29.1
All \	/ehicles	2977	5.2	2977	5.2	0.921	36.8	LOSC	10.2	72.7	0.97	0.94	1.19	32.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

Move	Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate						
P1	South Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91						
P2	East Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91						
P3	North Full Crossing	26	29.3	LOS C	0.0	0.0	0.92	0.92						
P4	West Full Crossing	26	29.3	LOS C	0.0	0.0	0.92	0.92						
All Pe	destrians	84	29.3	LOSC			0.92	0.92						

Site: 101 [SCHOFIELDS / TALLAWONG / RIDGELINE - EXISTING PM + Devt]

SCHOFIELDS / TALLAWONG / RIDGELINE EXIST PM + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

Mov	/ement	Perform	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV		Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
		ELINE DF												
1	L2	84	3.8	84	3.8	0.132	22.7	LOS B	1.2	8.8	0.72	0.73	0.72	42.7
2	T1	259	0.4	259	0.4	0.769	33.7	LOS C	5.7	40.2	1.00	0.92	1.19	29.1
3	R2	127	0.8	127	8.0	0.683	41.9	LOS C	2.8	20.0	1.00	0.85	1.16	25.8
App	roach	471	1.1	471	1.1	0.769	34.0	LOS C	5.7	40.2	0.95	0.87	1.10	31.1
East	:: SCHO	FIELDS R	OAD											
4	L2	46	9.1	46	9.1	0.041	9.3	LOS A	0.3	2.2	0.40	0.63	0.40	50.4
5	T1	959	3.1	959	3.1	0.836	33.6	LOS C	10.9	78.0	1.00	0.94	1.11	37.4
6	R2	38	0.0	38	0.0	0.239	42.5	LOS C	0.8	5.9	1.00	0.73	1.00	22.5
App	roach	1043	3.2	1043	3.2	0.836	32.8	LOS C	10.9	78.0	0.97	0.92	1.08	37.4
Nort	h: TALL/	AWONG R	ROAD											
7	L2	49	8.5	49	8.5	0.082	25.1	LOS B	0.9	6.5	0.87	0.74	0.87	19.2
8	T1	283	1.9	283	1.9	0.848	37.1	LOS C	6.7	47.5	1.00	0.97	1.27	32.1
9	R2	289	5.5	289	5.5	0.801	44.0	LOS D	3.4	24.8	1.00	0.90	1.27	29.2
App	roach	622	4.1	622	4.1	0.848	39.3	LOS C	6.7	47.5	0.99	0.92	1.24	30.1
Wes	t: SCHC	FIELDS F	ROAD											
10	L2	57	14.8	57	14.8	0.047	8.1	LOS A	0.3	2.3	0.34	0.62	0.34	47.7
11	T1	814	3.5	814	3.5	0.706	24.8	LOS B	7.9	56.5	0.94	0.84	0.99	33.5
12	R2	117	6.3	117	6.3	0.779	45.1	LOS D	2.7	20.3	1.00	0.91	1.34	34.2
Арр	roach	987	4.5	987	4.5	0.779	26.3	LOS B	7.9	56.5	0.92	0.84	1.00	34.2
All V	'ehicles	3123	3.5	3123	3.5	0.848	32.2	LOSC	10.9	78.0	0.95	0.89	1.09	34.1

♦ Network: N101 [Tallawong

Existing PM + Devt]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

Move	ement Performance - Po	edestrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	24	29.3	LOS C	0.0	0.0	0.92	0.92
P2	East Full Crossing	15	29.3	LOS C	0.0	0.0	0.91	0.91
P3	North Full Crossing	32	29.3	LOS C	0.1	0.1	0.92	0.92
P4	West Full Crossing	18	29.3	LOS C	0.0	0.0	0.91	0.91
All Pe	destrians	88	29.3	LOS C			0.92	0.92



Site: 101 [Tallawong / Themeda Existing AM + Devt]

♦ Network: N101 [Tallawong Existing AM + Devt]

Tallawong / Themeda Existing AM + Devt

Site Category: (None)

Move	ement	Performa	ınce -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV		Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles			Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Tallav	vong Road												
2	T1	165	13.4	165	13.4	0.073	8.5	LOS A	1.2	9.7	0.66	0.52	0.66	39.1
3	R2	140	9.0	140	9.0	0.468	39.7	LOS C	3.0	22.6	1.00	0.80	1.00	13.7
Appro	ach	305	11.4	305	11.4	0.468	22.9	LOS B	3.0	22.6	0.81	0.65	0.81	22.9
East:	Theme	da Ave												
4	L2	154	11.0	154	11.0	0.446	34.5	LOS C	3.2	24.2	0.99	0.81	0.99	27.5
6	R2	39	5.4	39	5.4	0.109	34.2	LOS C	0.8	5.6	0.95	0.74	0.95	28.5
Appro	ach	193	9.8	193	9.8	0.446	34.4	LOS C	3.2	24.2	0.98	0.79	0.98	27.7
North:	: Tallaw	ong Road												
7	L2	201	0.5	201	0.5	0.456	22.9	LOS B	5.1	36.1	0.80	0.76	0.80	11.7
8	T1	440	3.1	440	3.1	0.456	18.2	LOS B	5.2	37.3	0.81	0.71	0.81	12.9
Appro	ach	641	2.3	641	2.3	0.456	19.6	LOS B	5.2	37.3	0.81	0.72	0.81	12.5
All Ve	hicles	1139	6.0	1139	6.0	0.468	23.0	LOS B	5.2	37.3	0.84	0.71	0.84	20.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Ped	lestrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	26	29.3	LOS C	0.0	0.0	0.92	0.92
P2	East Full Crossing	31	29.3	LOS C	0.1	0.1	0.92	0.92
P3	North Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91
All Pe	edestrians	73	29.3	LOS C			0.92	0.92

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Wednesday, 3 June 2020 4:55:36 PM



Site: TT [Tallawong / Themeda Existing PM + DEVT]

♦ Network: N101 [Tallawong **Existing PM + Devt]**

Tallawong / Themeda Existing PM + DEVT

Site Category: (None)

Move	ement	Performa	ınce -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles [Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Tallav	vong Road												
2	T1	152	5.6	152	5.6	0.059	8.2	LOS A	1.2	8.9	0.74	0.59	0.74	39.7
3	R2	346	3.6	346	3.6	0.406	25.5	LOS B	6.4	46.0	0.93	0.84	0.93	18.9
Appro	ach	498	4.2	498	4.2	0.406	20.2	LOS B	6.4	46.0	0.87	0.77	0.87	23.5
East:	East: Themeda Ave													
4	L2	103	12.2	103	12.2	0.423	36.8	LOS C	2.2	16.7	0.99	0.78	0.99	26.6
6	R2	48	2.2	48	2.2	0.185	35.2	LOS C	1.0	7.0	0.97	0.74	0.97	28.1
Appro	ach	152	9.0	152	9.0	0.423	36.3	LOS C	2.2	16.7	0.98	0.77	0.98	27.1
North	: Tallaw	ong Road												
7	L2	18	11.8	18	11.8	0.403	36.6	LOS C	2.0	14.3	0.96	0.75	0.96	8.2
8	T1	179	2.4	179	2.4	0.403	31.9	LOS C	2.0	14.4	0.96	0.75	0.96	8.3
Appro	ach	197	3.2	197	3.2	0.403	32.3	LOS C	2.0	14.4	0.96	0.75	0.96	8.3
All Ve	hicles	846	4.9	846	4.9	0.423	25.9	LOS B	6.4	46.0	0.91	0.76	0.91	21.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Ped	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	25	29.3	LOS C	0.0	0.0	0.92	0.92
P2	East Full Crossing	29	29.3	LOS C	0.1	0.1	0.92	0.92
P3	North Full Crossing	13	29.3	LOS C	0.0	0.0	0.91	0.91
All Pe	edestrians	67	29.3	LOS C			0.92	0.92

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Wednesday, 3 June 2020 4:55:58 PM

Appendix F3 SIDRA Movement Summary

Existing + Growth



Site: CT [Cudgegong / Themeda Existing AM + GROWTH]

♦ Network: N101 [Tallawong Existing AM + GROWTH]

Cudgegong / Conferta Existing AM + GROWTH

Site Category: (None)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back of Vehicles			Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Cudg	egong Roa	ad											
1	L2	52	0.0	52	0.0	0.175	20.8	LOS B	1.9	15.3	0.76	0.68	0.76	27.3
2	T1	205	17.9	205	17.9	0.175	18.9	LOS B	2.5	19.1	0.88	0.74	0.88	29.5
Appro	ach	257	14.3	257	14.3	0.175	19.3	LOS B	2.5	19.1	0.86	0.72	0.86	29.1
North	: Cudge	egong Roa	d											
8	T1	266	14.2	266	14.2	0.245	15.0	LOS B	2.5	19.5	0.70	0.60	0.70	15.0
9	R2	54	2.0	54	2.0	0.245	20.7	LOS B	2.0	15.2	0.72	0.65	0.72	13.4
Appro	ach	320	12.2	320	12.2	0.245	15.9	LOS B	2.5	19.5	0.70	0.61	0.70	14.7
West:	Theme	eda Ave												
10	L2	37	14.3	37	14.3	0.053	11.3	LOS A	0.2	1.8	0.32	0.60	0.32	37.7
12	R2	185	4.0	185	4.0	0.248	10.0	LOS A	1.1	7.6	0.29	0.61	0.29	38.9
Appro	ach	222	5.7	222	5.7	0.248	10.2	LOS A	1.1	7.6	0.29	0.61	0.29	38.7
All Ve	hicles	799	11.1	799	11.1	0.248	15.4	LOS B	2.5	19.5	0.64	0.64	0.64	29.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Ped	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91
P3	North Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91
P4	West Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91
All Pe	edestrians	47	29.3	LOS C			0.91	0.91

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Wednesday, 3 June 2020 5:35:18 PM Project: C:\Users\robert\Documents\Tallawong\Sidra\[SY190226] EXISTING + GROWTH.sip8



Site: CT [Cudgegong / Themeda Existing PM + GROWTH]

♦ Network: N101 [Tallawong **Existing PM + GROWTH]**

Cudgegong / Conferta Existing PM + GROWTH

Site Category: (None)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV		Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back of Vehicles			Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Cudg	egong Roa	ad											
1	L2	27	0.0	27	0.0	0.161	19.2	LOS B	1.9	15.1	0.74	0.64	0.74	30.0
2	T1	241	10.5	241	10.5	0.161	16.9	LOS B	2.6	18.7	0.86	0.71	0.86	31.4
Appro	ach	268	9.4	268	9.4	0.161	17.2	LOS B	2.6	18.7	0.85	0.70	0.85	31.3
North	North: Cudgegong Road													
8	T1	211	10.0	211	10.0	0.150	12.2	LOS A	1.5	11.6	0.62	0.52	0.62	17.5
9	R2	22	0.0	22	0.0	0.150	17.3	LOS B	1.3	10.1	0.63	0.55	0.63	16.5
Appro	ach	233	9.0	233	9.0	0.150	12.7	LOS A	1.5	11.6	0.62	0.52	0.62	17.4
West:	Theme	eda Ave												
10	L2	49	0.0	49	0.0	0.072	7.2	LOS A	0.1	0.9	0.12	0.56	0.12	40.8
12	R2	99	6.4	99	6.4	0.150	7.9	LOS A	0.3	2.3	0.16	0.57	0.16	40.9
Appro	ach	148	4.3	148	4.3	0.150	7.6	LOS A	0.3	2.3	0.14	0.56	0.14	40.9
All Ve	hicles	649	8.1	649	8.1	0.161	13.4	LOSA	2.6	18.7	0.61	0.61	0.61	31.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pede	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91
P3	North Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91
P4	West Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91
All Pe	edestrians	47	29.3	LOS C			0.91	0.91

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Wednesday, 3 June 2020 5:22:18 PM Project: C:\Users\robert\Documents\Tallawong\Sidra\[SY190226] EXISTING + GROWTH.sip8

Site: SC [SCHOFIELDS / CUDGEGONG - AM EXIST + GROWTH]

♦♦ Network: N101 [Tallawong Existing AM + GROWTH]

SCHOFIELDS ROAD / CUDGEGONG ROAD AM EXIST + GROWTH

Site Category: (None)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
East:	SCHO	FIELDS R	DAC											
5	T1	1049	7.4	1049	7.4	0.678	18.7	LOS B	9.0	65.2	0.87	0.77	0.87	37.4
6	R2	136	5.4	136	5.4	0.224	33.6	LOS C	1.3	9.3	0.90	0.75	0.90	28.8
Appro	ach	1185	7.2	1185	7.2	0.678	20.4	LOS B	9.0	65.2	0.88	0.76	0.88	36.2
North	: CUDG	SEGONG I	ROAD											
7	L2	208	16.2	208	16.2	0.292	22.4	LOS B	3.5	27.6	0.83	0.79	0.83	39.3
9	R2	175	13.3	175	13.3	0.598	28.0	LOS B	3.1	23.9	0.84	0.77	0.84	21.2
Appro	ach	383	14.8	383	14.8	0.598	25.0	LOS B	3.5	27.6	0.83	0.78	0.84	33.1
West	SCHO	FIELDS R	OAD											
10	L2	14	53.8	14	53.8	0.024	19.9	LOS B	0.2	1.9	0.68	0.65	0.68	34.3
11	T1	1282	7.0	1282	7.0	0.863	32.1	LOS C	15.0	111.4	1.00	1.02	1.18	37.9
Appro	ach	1296	7.5	1296	7.5	0.863	31.9	LOS C	15.0	111.4	1.00	1.01	1.17	37.9
All Ve	hicles	2864	8.3	2864	8.3	0.863	26.3	LOS B	15.0	111.4	0.93	0.88	1.00	36.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pede	strians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P2	East Full Crossing	37	29.3	LOS C	0.1	0.1	0.92	0.92
P3	North Full Crossing	37	29.3	LOS C	0.1	0.1	0.92	0.92
P4	West Full Crossing	37	29.3	LOS C	0.1	0.1	0.92	0.92
All Pe	edestrians	111	29.3	LOS C			0.92	0.92

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Wednesday, 3 June 2020 5:35:18 PM Project: C:\Users\robert\Documents\Tallawong\Sidra\[SY190226] EXISTING + GROWTH.sip8

Site: SC [SCHOFIELDS / CUDGEGONG - PM EXIST + GROWTH]

SCHOFIELDS ROAD / CUDGEGONG ROAD PM EXIST + GROWTH

Site Category: (None)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
East:	SCHO	FIELDS R	DAC											
5	T1	1137	2.2	1137	2.2	0.557	11.9	LOS A	8.0	56.6	0.72	0.64	0.72	43.4
6	R2	153	13.1	153	13.1	0.530	41.8	LOS C	1.7	13.0	1.00	0.77	1.03	25.6
Appro	oach	1289	3.5	1289	3.5	0.557	15.4	LOS B	8.0	56.6	0.75	0.65	0.75	40.1
North	: CUDO	GEGONG I	ROAD											
7	L2	158	14.7	158	14.7	0.313	32.4	LOS C	3.1	24.6	0.98	0.81	0.98	34.4
9	R2	161	7.2	161	7.2	0.705	43.4	LOS D	3.6	26.8	1.00	0.84	1.08	15.6
Appro	oach	319	10.9	319	10.9	0.705	38.0	LOS C	3.6	26.8	0.99	0.82	1.03	26.4
West	: SCHO	FIELDS R	OAD											
10	L2	6	0.0	6	0.0	0.006	13.8	LOS A	0.1	0.4	0.46	0.60	0.46	39.1
11	T1	1399	3.5	1399	3.5	0.695	13.2	LOS A	11.4	82.0	0.81	0.73	0.81	48.4
Appro	oach	1405	3.5	1405	3.5	0.695	13.2	LOS A	11.4	82.0	0.81	0.73	0.81	48.4
All Ve	hicles	3014	4.3	3014	4.3	0.705	16.8	LOS B	11.4	82.0	0.80	0.71	0.81	42.9

♦♦ Network: N101 [Tallawong Existing PM + GROWTH]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pede	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P2	East Full Crossing	24	29.3	LOS C	0.0	0.0	0.92	0.92
P3	North Full Crossing	15	29.3	LOS C	0.0	0.0	0.91	0.91
P4	West Full Crossing	19	29.3	LOS C	0.0	0.0	0.92	0.92
All Pe	destrians	58	29.3	LOS C			0.92	0.92

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Wednesday, 3 June 2020 5:22:18 PM Project: C:\Users\robert\Documents\Tallawong\Sidra\[SY190226] EXISTING + GROWTH.sip8

Site: STR [SCHOFIELDS / TALLAWONG / RIDGELINE AM EXIST + GROWTH]

◆◆ Network: N101 [Tallawong
Existing AM + GROWTH]

SCHOFIELDS / TALLAWONG / RIDGELINE AM EXIST + GROWTH

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

Мо	vement	Performa	ance -	Vehic	les									
Mo ID	v Turn	Demand Total		Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
Sou		ELINE DR												
1	L2	152	2.8	152	2.8	0.270	26.0	LOS B	2.5	17.6	0.80	0.77	0.80	41.2
2	T1	172	1.8	172	1.8	0.771	37.0	LOS C	3.9	27.8	1.00	0.91	1.26	27.7
3	R2	188	3.4	188	3.4	0.800	43.1	LOS D	4.4	31.5	1.00	0.94	1.30	25.4
App	oroach	512	2.7	512	2.7	0.800	36.0	LOS C	4.4	31.5	0.94	0.88	1.14	31.5
Eas	st: SCHO	FIELDS R	DAC											
4	L2	24	0.0	24	0.0	0.024	13.6	LOS A	0.2	1.4	0.41	0.63	0.41	47.6
5	T1	772	6.1	772	6.1	0.651	27.8	LOS B	8.1	59.6	0.98	0.84	0.98	40.0
6	R2	78	13.5	78	13.5	0.462	43.4	LOS D	1.7	13.6	1.00	0.77	1.00	22.3
App	oroach	874	6.6	874	6.6	0.651	28.8	LOS C	8.1	59.6	0.96	0.83	0.96	38.8
Noi	th: TALL	AWONG R	OAD											
7	L2	41	17.9	41	17.9	0.082	26.6	LOS B	0.7	5.8	0.87	0.73	0.87	18.5
8	T1	176	6.6	176	6.6	0.814	40.5	LOS C	4.1	30.1	1.00	0.88	1.18	30.8
9	R2	154	8.9	154	8.9	0.339	37.4	LOS C	1.6	11.7	0.95	0.76	0.95	31.5
App	oroach	371	8.8	371	8.8	0.814	37.7	LOS C	4.1	30.1	0.96	0.81	1.05	30.4
We	st: SCHC	FIELDS R	OAD											
10	L2	59	1.8	59	1.8	0.058	14.2	LOS A	0.6	4.2	0.51	0.66	0.51	41.4
11	T1	971	6.5	971	6.5	0.814	28.8	LOS C	10.5	77.2	0.98	0.97	1.15	31.3
12	R2	149	2.1	149	2.1	0.830	45.9	LOS D	3.6	25.5	1.00	0.96	1.43	34.0
Арр	oroach	1179	5.7	1179	5.7	0.830	30.3	LOS C	10.5	77.2	0.96	0.95	1.16	32.2
All	Vehicles	2935	5.8	2935	5.8	0.830	31.7	LOSC	10.5	77.2	0.96	0.88	1.08	34.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

Move	ement Performance - Pede	strians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91
P2	East Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91
P3	North Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91
P4	West Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91
All Pe	destrians	63	29.3	LOS C			0.91	0.91

Site: 101 [SCHOFIELDS / TALLAWONG / RIDGELINE PM EXIST + GROWTH]

SCHOFIELDS / TALLAWONG / RIDGELINE PM EXIST + GROWTH

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

Mov	/ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV		Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
		ELINE DF												
1	L2	101	4.2	101	4.2	0.153	22.2	LOS B	1.4	10.5	0.71	0.73	0.71	43.0
2	T1	126	0.8	126	8.0	0.376	28.7	LOS C	2.4	17.2	0.93	0.74	0.93	31.5
3	R2	153	0.7	153	0.7	0.817	45.2	LOS D	3.6	25.4	1.00	0.95	1.38	24.7
App	roach	380	1.7	380	1.7	0.817	33.6	LOS C	3.6	25.4	0.90	0.82	1.05	32.2
East	: SCHO	FIELDS R	OAD											
4	L2	35	15.2	35	15.2	0.033	9.8	LOS A	0.3	2.1	0.46	0.63	0.46	49.7
5	T1	905	4.0	905	4.0	0.831	33.7	LOS C	10.3	74.0	1.00	0.94	1.11	37.3
6	R2	81	0.0	81	0.0	0.439	39.8	LOS C	1.7	12.2	1.00	0.77	1.00	23.4
App	roach	1021	4.0	1021	4.0	0.831	33.3	LOS C	10.3	74.0	0.98	0.91	1.08	36.8
Nort	h: TALL/	AWONG R	OAD											
7	L2	60	8.8	60	8.8	0.095	22.2	LOS B	0.9	6.9	0.76	0.73	0.76	20.8
8	T1	251	27.7	251	27.7	0.875	39.5	LOS C	6.2	53.4	1.00	1.02	1.37	31.1
9	R2	305	6.2	305	6.2	0.849	45.7	LOS D	3.7	27.0	1.00	0.95	1.38	28.6
App	roach	616	15.2	616	15.2	0.875	40.9	LOS C	6.2	53.4	0.98	0.96	1.31	29.3
Wes	t: SCHC	FIELDS R	ROAD											
10	L2	24	43.5	24	43.5	0.022	7.5	LOS A	0.1	0.9	0.27	0.59	0.27	48.8
11	T1	976	3.5	976	3.5	0.889	37.3	LOS C	12.3	87.8	1.00	1.09	1.36	27.5
12	R2	140	6.0	140	6.0	0.799	44.8	LOS D	3.3	24.3	1.00	0.93	1.36	34.3
Арр	roach	1140	4.6	1140	4.6	0.889	37.6	LOS C	12.3	87.8	0.98	1.06	1.34	28.9
All V	ehicles	3157	6.1	3157	6.1	0.889	36.4	LOSC	12.3	87.8	0.97	0.96	1.22	32.2

♦♦ Network: N101 [Tallawong Existing PM + GROWTH]

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

Movement Performance - Pedestrians													
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate					
P1	South Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91					
P2	East Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91					
P3	North Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91					
P4	West Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91					
All Pe	destrians	63	29.3	LOS C			0.91	0.91					



Site: TT [Tallawong / Themeda Existing AM + GROWTH]

♦ Network: N101 [Tallawong Existing AM + GROWTH]

Tallawong / Themeda Existing AM + GROWTH

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Practical Cycle Time)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV		l Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Tallav	vong Road												
2	T1	274	10.0	274	10.0	0.125	9.3	LOS A	2.2	16.6	0.65	0.54	0.65	38.0
3	R2	54	27.5	54	27.5	0.403	43.6	LOS D	1.2	10.4	1.00	0.75	1.00	12.8
Appro	ach	327	12.9	327	12.9	0.403	14.9	LOS B	2.2	16.6	0.71	0.57	0.71	30.4
East:	Theme	da Ave												
4	L2	184	10.9	184	10.9	0.468	31.7	LOS C	3.5	26.9	0.93	0.80	0.93	28.8
6	R2	15	14.3	15	14.3	0.038	28.6	LOS C	0.3	2.0	0.83	0.68	0.83	31.0
Appro	ach	199	11.1	199	11.1	0.468	31.4	LOS C	3.5	26.9	0.92	0.79	0.92	29.0
North	: Tallaw	ong Road												
7	L2	241	0.4	241	0.4	0.466	20.2	LOS B	5.6	39.9	0.76	0.75	0.76	13.0
8	T1	515	3.1	515	3.1	0.466	15.5	LOS B	5.8	41.3	0.76	0.68	0.76	14.5
Appro	ach	756	2.2	756	2.2	0.466	17.0	LOS B	5.8	41.3	0.76	0.70	0.76	14.0
All Ve	hicles	1282	6.3	1282	6.3	0.468	18.7	LOS B	5.8	41.3	0.77	0.68	0.77	22.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Ped	lestrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec		Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91
P2	East Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91
P3	North Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91
All Pe	edestrians	47	29.3	LOS C			0.91	0.91

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Wednesday, 3 June 2020 5:35:18 PM Project: C:\Users\robert\Documents\Tallawong\Sidra\[SY190226] EXISTING + GROWTH.sip8



Site: TT [Tallawong / Themeda Existing PM + growth]

♦ Network: N101 [Tallawong **Existing PM + GROWTH]**

Tallawong / Themeda Existing PM + growth

Site Category: (None)

Move	ment	Performa	ınce -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles			Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Tallav	vong Road												
2	T1	182	5.8	182	5.8	0.079	8.5	LOS A	1.4	10.2	0.63	0.50	0.63	39.1
3	R2	219	6.7	219	6.7	0.346	28.7	LOS C	4.1	30.2	0.93	0.82	0.93	17.4
Appro	ach	401	6.3	401	6.3	0.346	19.5	LOS B	4.1	30.2	0.79	0.67	0.79	24.7
East:	Theme	da Ave												
4	L2	123	12.0	123	12.0	0.336	31.6	LOS C	2.3	17.4	0.89	0.77	0.89	28.8
6	R2	33	3.2	33	3.2	0.084	29.7	LOS C	0.6	4.0	0.83	0.71	0.83	30.5
Appro	ach	156	10.1	156	10.1	0.336	31.2	LOS C	2.3	17.4	0.88	0.76	0.88	29.2
North	: Tallaw	ong Road												
7	L2	21	10.0	21	10.0	0.324	33.2	LOS C	2.0	14.4	0.92	0.73	0.92	9.0
8	T1	189	2.8	189	2.8	0.324	28.5	LOS B	2.0	14.5	0.92	0.73	0.92	9.1
Appro	ach	211	3.5	211	3.5	0.324	28.9	LOS C	2.0	14.5	0.92	0.73	0.92	9.1
All Ve	hicles	767	6.3	767	6.3	0.346	24.5	LOS B	4.1	30.2	0.84	0.71	0.84	22.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Ped	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	21	29.3	LOS C	0.0	0.0	0.92	0.92
P2	East Full Crossing	16	29.3	LOS C	0.0	0.0	0.91	0.91
P3	North Full Crossing	13	29.3	LOS C	0.0	0.0	0.91	0.91
All Pe	edestrians	49	29.3	LOS C			0.91	0.91

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Wednesday, 3 June 2020 5:22:18 PM Project: C:\Users\robert\Documents\Tallawong\Sidra\[SY190226] EXISTING + GROWTH.sip8

Appendix F4 SIDRA Movement Summary

Existing + Growth + Development

Site: CT [Cudgegong / Themeda Existing AM + GROWTH + DEVT]

Cudgegong / Conferta Existing AM + GROWTH + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 80 seconds (Network Optimum Cycle Time - Minimum Delay)

Move	ement	Performa	ince -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
South	i: Cudg	egong Roa	ad											
1	L2	52	0.0	52	0.0	0.267	34.6	LOS C	2.9	23.0	0.94	0.78	0.94	19.4
2	T1	225	13.6	225	13.6	0.267	31.1	LOS C	3.3	25.0	0.98	0.80	0.98	22.6
Appro	ach	277	11.0	277	11.0	0.267	31.7	LOS C	3.3	25.0	0.97	0.79	0.97	22.0
North	: Cudge	egong Roa	d											
8	T1	266	14.2	266	14.2	0.376	25.7	LOS B	3.7	29.4	0.85	0.71	0.85	9.9
9	R2	62	1.7	62	1.7	0.376	32.3	LOS C	2.8	20.8	0.87	0.74	0.87	8.8
Appro	ach	328	11.9	328	11.9	0.376	26.9	LOS B	3.7	29.4	0.86	0.72	0.86	9.7
West	Theme	eda Ave												
10	L2	37	14.3	37	14.3	0.039	9.7	LOS A	0.2	1.6	0.25	0.58	0.25	38.8
12	R2	405	1.8	405	1.8	0.393	10.2	LOS A	2.9	20.3	0.32	0.64	0.32	38.7
Appro	ach	442	2.9	442	2.9	0.393	10.2	LOS A	2.9	20.3	0.31	0.63	0.31	38.7
All Ve	hicles	1047	7.8	1047	7.8	0.393	21.1	LOS B	3.7	29.4	0.66	0.70	0.66	26.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pede	strians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	34.3	LOS D	0.1	0.1	0.93	0.93
P3	North Full Crossing	53	34.3	LOS D	0.1	0.1	0.93	0.93
P4	West Full Crossing	53	34.3	LOS D	0.1	0.1	0.93	0.93
All Pe	destrians	158	34.3	LOS D			0.93	0.93

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Thursday, 4 June 2020 8:46:04 AM

Site: CT [Cudgegong / Themeda Existing PM + GROWTH + DEVT]

♦ Network: N101 [Tallawong Existing PM + GROWTH + DEVT]

Cudgegong / Conferta Existing PM + GROWTH + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Optimum Cycle Time - Minimum Delay)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance		Effective A Stop Rate	ver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	: Cudg	egong Roa	ad											
1	L2	27	0.0	27	0.0	0.236	24.2	LOS B	1.9	15.4	0.70	0.61	0.70	25.8
2	T1	258	9.8	258	9.8	0.236	23.6	LOS B	3.1	22.2	0.88	0.73	0.88	26.6
Appro	ach	285	8.9	285	8.9	0.236	23.7	LOS B	3.1	22.2	0.86	0.71	0.86	26.6
North	: Cudge	egong Roa	ıd											
8	T1	211	10.0	211	10.0	0.242	19.2	LOS B	2.2	17.1	0.78	0.64	0.78	12.4
9	R2	41	0.0	41	0.0	0.242	25.2	LOS B	1.7	12.7	0.79	0.68	0.79	11.3
Appro	ach	252	8.4	252	8.4	0.242	20.2	LOS B	2.2	17.1	0.78	0.65	0.78	12.2
West:	Theme	eda Ave												
10	L2	49	0.0	49	0.0	0.053	8.1	LOS A	0.2	1.4	0.20	0.58	0.20	40.2
12	R2	227	2.3	227	2.3	0.249	8.7	LOS A	1.1	8.1	0.25	0.61	0.25	40.1
Appro	ach	277	1.9	277	1.9	0.249	8.5	LOSA	1.1	8.1	0.24	0.60	0.24	40.1
All Ve	hicles	814	6.3	814	6.3	0.249	17.5	LOS B	3.1	22.2	0.62	0.66	0.62	29.0

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pede	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	29.3	LOS C	0.1	0.1	0.92	0.92
P3	North Full Crossing	53	29.3	LOS C	0.1	0.1	0.92	0.92
P4	West Full Crossing	53	29.3	LOS C	0.1	0.1	0.92	0.92
All Pe	edestrians	158	29.3	LOS C			0.92	0.92

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: SC [SCHOFIELDS / CUDGEGONG - AM EXIST + GROWTH + DEVT]

SCHOFIELDS ROAD / CUDGEGONG ROAD AM EXIST + GROWTH + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 80 seconds (Network Optimum Cycle Time - Minimum Delay)

Mov	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service		of Queue Distance	Prop. Queued	Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
East:	SCHO	FIELDS R	DAC											
5	T1	1049	7.4	1049	7.4	0.612	17.8	LOS B	9.4	68.4	0.80	0.71	0.80	38.1
6	R2	206	3.1	206	3.1	0.353	38.9	LOS C	2.3	16.4	0.93	0.77	0.93	26.6
Appro	oach	1256	6.7	1256	6.7	0.612	21.3	LOS B	9.4	68.4	0.82	0.72	0.82	35.6
North	: CUDO	GEGONG I	ROAD											
7	L2	428	7.9	428	7.9	0.609	26.4	LOS B	8.4	62.9	0.87	0.83	0.87	37.4
9	R2	175	13.3	175	13.3	0.630	37.4	LOS C	3.9	30.5	0.94	0.81	0.96	17.5
Appro	oach	603	9.4	603	9.4	0.630	29.6	LOS C	8.4	62.9	0.89	0.82	0.89	32.9
West	: SCHC	FIELDS R	OAD											
10	L2	14	53.8	14	53.8	0.022	19.2	LOS B	0.2	1.8	0.58	0.63	0.58	34.9
11	T1	1282	7.0	1282	7.0	0.767	21.8	LOS B	14.0	103.8	0.92	0.85	0.96	43.0
Appro	oach	1296	7.5	1296	7.5	0.767	21.8	LOS B	14.0	103.8	0.91	0.85	0.95	42.9
All Ve	ehicles	3155	7.5	3155	7.5	0.767	23.1	LOS B	14.0	103.8	0.87	0.79	0.89	38.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pede	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P2	East Full Crossing	37	34.3	LOS D	0.1	0.1	0.93	0.93
P3	North Full Crossing	37	34.3	LOS D	0.1	0.1	0.93	0.93
P4	West Full Crossing	37	34.3	LOS D	0.1	0.1	0.93	0.93
All Pe	destrians	111	34.3	LOS D			0.93	0.93

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Thursday, 4 June 2020 8:46:04 AM

Site: SC [SCHOFIELDS / CUDGEGONG - PM EXIST + GROWTH + DEVT]

♦ Network: N101 [Tallawong Existing PM + GROWTH + DEVT]

SCHOFIELDS ROAD / CUDGEGONG ROAD PM EXIST + GROWTH + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Optimum Cycle Time - Minimum Delay)

Move	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
East:	SCHO	FIELDS R	DAC											
5	T1	1137	2.2	1137	2.2	0.648	15.8	LOS B	9.2	65.4	0.82	0.73	0.82	39.7
6	R2	308	6.5	308	6.5	0.768	43.0	LOS D	3.5	26.2	1.00	0.91	1.27	25.2
Appro	oach	1445	3.1	1445	3.1	0.768	21.6	LOS B	9.2	65.4	0.86	0.77	0.92	35.3
North	: CUDO	GEGONG I	ROAD											
7	L2	287	8.1	287	8.1	0.441	31.1	LOS C	5.7	42.6	0.99	0.85	0.99	35.1
9	R2	161	7.2	161	7.2	0.529	40.1	LOS C	3.5	25.7	1.00	0.81	1.00	16.6
Appro	oach	448	7.7	448	7.7	0.529	34.3	LOS C	5.7	42.6	0.99	0.83	0.99	29.8
West	SCHO	FIELDS R	OAD											
10	L2	6	0.0	6	0.0	0.007	17.1	LOS B	0.1	0.6	0.63	0.62	0.63	36.2
11	T1	1399	3.5	1399	3.5	0.804	22.0	LOS B	14.3	103.0	0.97	0.92	1.04	42.9
Appro	oach	1405	3.5	1405	3.5	0.804	22.0	LOS B	14.3	103.0	0.97	0.92	1.04	42.8
All Ve	hicles	3299	3.9	3299	3.9	0.804	23.5	LOS B	14.3	103.0	0.92	0.84	0.98	38.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pede	strians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P2	East Full Crossing	37	29.3	LOS C	0.1	0.1	0.92	0.92
P3	North Full Crossing	21	29.3	LOS C	0.0	0.0	0.92	0.92
P4	West Full Crossing	21	29.3	LOS C	0.0	0.0	0.92	0.92
All Pe	destrians	79	29.3	LOS C			0.92	0.92

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Site: STR [SCHOFIELDS / TALLAWONG / RIDGELINE AM EXIST + GROWTH + DEVT]

SCHOFIELDS / TALLAWONG / RIDGELINE AM EXIST + GROWTH + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 80 seconds (Network Optimum Cycle Time - Minimum Delay)

Mov	ement	Performa	ance -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV		Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
Sout		ELINE DR	RIVE											
1	L2	154	4.1	154	4.1	0.214	23.2	LOS B	2.5	17.8	0.71	0.75	0.71	42.5
2	T1	246	1.7	246	1.7	0.562	30.4	LOS C	5.4	38.1	0.94	0.78	0.94	30.7
3	R2	189	3.9	189	3.9	0.922	59.0	LOS E	5.7	41.1	1.00	1.10	1.65	20.9
Appr	oach	589	3.0	589	3.0	0.922	37.7	LOS C	5.7	41.1	0.90	0.87	1.11	30.4
East	: SCHO	FIELDS R	OAD											
4	L2	29	0.0	29	0.0	0.033	18.0	LOS B	0.3	2.3	0.49	0.65	0.49	44.8
5	T1	926	6.1	926	6.1	0.893	45.1	LOS D	13.1	95.7	1.00	1.04	1.27	33.1
6	R2	94	13.5	94	13.5	0.635	50.2	LOS D	2.4	18.9	1.00	0.80	1.06	20.3
Appr	oach	1049	6.6	1049	6.6	0.893	44.8	LOS D	13.1	95.7	0.99	1.01	1.23	32.4
Nort	h: TALL	AWONG R	OAD											
7	L2	41	17.9	41	17.9	0.064	28.1	LOS B	0.8	6.6	0.87	0.74	0.87	17.8
8	T1	420	2.8	420	2.8	0.969	61.1	LOS E	13.1	94.0	1.00	1.13	1.45	24.7
9	R2	180	7.6	180	7.6	0.449	43.5	LOS D	2.1	15.9	0.98	0.77	0.98	29.3
Appr	oach	641	5.1	641	5.1	0.969	54.1	LOS D	13.1	94.0	0.99	1.00	1.28	25.7
Wes	t: SCHC	FIELDS R	OAD											
10	L2	80	1.3	80	1.3	0.089	18.5	LOS B	1.1	7.5	0.59	0.69	0.59	37.8
11	T1	971	6.5	971	6.5	0.930	49.7	LOS D	15.1	110.2	1.00	1.17	1.47	23.3
12	R2	151	2.8	151	2.8	0.960	67.6	LOS E	4.9	34.9	1.00	1.15	1.89	28.3
Appr	oach	1201	5.7	1201	5.7	0.960	49.8	LOS D	15.1	110.2	0.97	1.14	1.46	24.8
All V	ehicles	3481	5.4	3481	5.4	0.969	47.0	LOS D	15.1	110.2	0.97	1.03	1.30	28.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

Move	Movement Performance - Pedestrians									
Mov		Demand	Average	Level of	Average Back	of Queue	Prop.	Effective		
ID	Description	Flow	Delay	Service	Pedestrian	Distance	Queued	Stop Rate		
		ped/h	sec		ped	m				
P1	South Full Crossing	21	34.3	LOS D	0.0	0.0	0.93	0.93		
P2	East Full Crossing	21	34.3	LOS D	0.0	0.0	0.93	0.93		
P3	North Full Crossing	21	34.3	LOS D	0.0	0.0	0.93	0.93		
P4	West Full Crossing	21	34.3	LOS D	0.0	0.0	0.93	0.93		

Site: STR [SCHOFIELDS / TALLAWONG / RIDGELINE PM EXIST + GROWTH + DEVT]

♦ Network: N101 [Tallawong Existing PM + GROWTH + DEVT]

SCHOFIELDS / TALLAWONG / RIDGELINE PM EXIST + GROWTH + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Optimum Cycle Time - Minimum Delay)

Мо	vement	Performa	ance -	Vehic	les									
Mov	/ Turn	Demand Total		Arrival Total	l Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h		veh/h	%	v/c	sec		veh	m				km/h
Sou		ELINE DR												
1	L2	101	4.2	101	4.2	0.159	22.9	LOS B	1.5	10.7	0.73	0.74	0.73	42.6
2	T1	280	0.4	280	0.4	0.907	44.0	LOS D	7.3	51.1	1.00	1.10	1.55	25.2
3	R2	153	0.7	153	0.7	0.715	41.5	LOS C	3.4	24.0	1.00	0.87	1.18	25.9
App	roach	534	1.2	534	1.2	0.907	39.3	LOS C	7.3	51.1	0.95	0.97	1.29	29.1
Eas	t: SCHO	FIELDS R	DAC											
4	L2	35	15.2	35	15.2	0.031	9.6	LOS A	0.3	2.1	0.49	0.64	0.49	49.9
5	T1	905	4.0	905	4.0	0.831	33.7	LOS C	10.3	74.0	1.00	0.94	1.11	37.3
6	R2	81	0.0	81	0.0	0.439	42.9	LOS D	1.8	12.6	1.00	0.77	1.00	22.4
App	roach	1021	4.0	1021	4.0	0.831	33.6	LOS C	10.3	74.0	0.98	0.91	1.08	36.7
Nor	th: TALL	AWONG R	OAD											
7	L2	60	8.8	60	8.8	0.099	27.5	LOS B	1.1	8.2	0.90	0.75	0.90	18.0
8	T1	222	2.8	222	2.8	0.731	36.5	LOS C	4.9	35.4	1.00	0.88	1.11	32.3
9	R2	337	4.7	337	4.7	0.812	43.5	LOS D	3.9	28.7	1.00	0.92	1.27	29.4
App	roach	619	4.4	619	4.4	0.812	39.4	LOS C	4.9	35.4	0.99	0.89	1.18	29.9
Wes	st: SCHC	FIELDS R	OAD											
10	L2	61	17.2	61	17.2	0.053	8.8	LOS A	0.4	2.8	0.37	0.63	0.37	47.0
11	T1	977	3.6	977	3.6	0.889	37.3	LOS C	12.3	87.8	1.00	1.09	1.36	27.5
12	R2	140	6.0	140	6.0	0.799	44.8	LOS D	3.3	24.3	1.00	0.93	1.36	34.3
App	roach	1178	4.6	1178	4.6	0.889	36.7	LOS C	12.3	87.8	0.97	1.05	1.31	29.2
All V	/ehicles	3352	3.8	3352	3.8	0.907	36.7	LOSC	12.3	87.8	0.97	0.96	1.21	31.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

Move	Novement Performance - Pedestrians								
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate	
P1	South Full Crossing	21	29.3	LOS C	0.0	0.0	0.92	0.92	
P2	East Full Crossing	21	29.3	LOS C	0.0	0.0	0.92	0.92	
P3	North Full Crossing	21	29.3	LOS C	0.0	0.0	0.92	0.92	
P4	West Full Crossing	21	29.3	LOS C	0.0	0.0	0.92	0.92	
All Pe	destrians	84	29.3	LOSC			0.92	0.92	

Site: TT [Tallawong / Themeda Existing AM + GROWTH + DEVT]

Tallawong / Themeda Existing AM + GROWTH + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 80 seconds (Network Optimum Cycle Time - Minimum Delay)

Move	ement	Performa	ince -	Vehic	les									
Mov ID	Turn	Demand Total	Flows HV	Arrival Total	Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	ı: Tallav	ong Road												
2	T1	274	10.0	274	10.0	0.117	9.8	LOS A	2.4	18.4	0.67	0.55	0.67	37.2
3	R2	148	9.9	148	9.9	0.489	44.4	LOS D	3.6	27.4	1.00	0.81	1.00	12.6
Appro	ach	422	10.0	422	10.0	0.489	22.0	LOS B	3.6	27.4	0.79	0.64	0.79	23.9
East:	Theme	da Ave												
4	L2	184	10.9	184	10.9	0.503	38.2	LOS C	4.3	32.9	1.00	0.82	1.00	26.0
6	R2	41	5.1	41	5.1	0.108	37.4	LOS C	0.9	6.7	0.95	0.74	0.95	27.3
Appro	ach	225	9.8	225	9.8	0.503	38.1	LOS C	4.3	32.9	0.99	0.81	0.99	26.2
North	: Tallaw	ong Road												
7	L2	241	0.4	241	0.4	0.525	25.2	LOS B	7.1	50.0	0.82	0.78	0.82	10.8
8	T1	527	3.4	527	3.4	0.525	20.4	LOS B	7.2	51.8	0.82	0.73	0.82	11.8
Appro	ach	768	2.5	768	2.5	0.525	21.9	LOS B	7.2	51.8	0.82	0.74	0.82	11.5
All Ve	hicles	1416	5.9	1416	5.9	0.525	24.5	LOS B	7.2	51.8	0.84	0.72	0.84	19.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pede	strians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	34.3	LOS D	0.1	0.1	0.93	0.93
P2	East Full Crossing	53	34.3	LOS D	0.1	0.1	0.93	0.93
P3	North Full Crossing	53	34.3	LOS D	0.1	0.1	0.93	0.93
All Pe	destrians	158	34.3	LOS D			0.93	0.93

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Organisation: BARKER RYAN STEWART | Processed: Thursday, 4 June 2020 8:46:04 AM

Tallawong / Themeda Existing PM + growth + DEVT

Site Category: (None)

Signals - Fixed Time Coordinated Cycle Time = 70 seconds (Network Optimum Cycle Time - Minimum Delay)

Move	Novement Performance - Vehicles													
Mov ID	Turn	Demand Total	Flows HV		Flows HV	Deg. Satn	Average Delay	Level of Service	Aver. Back Vehicles	of Queue Distance		Effective A Stop Rate	Aver. No.A Cycles S	
		veh/h	%	veh/h	%	v/c	sec		veh	m				km/h
South	ո։ Tallav	vong Road												
2	T1	182	5.8	182	5.8	0.072	8.3	LOS A	1.5	10.7	0.72	0.58	0.72	39.5
3	R2	409	3.6	409	3.6	0.495	27.4	LOS B	7.8	56.1	0.96	0.86	0.96	18.0
Appro	oach	592	4.3	592	4.3	0.495	21.5	LOS B	7.8	56.1	0.89	0.77	0.89	22.7
East:	Theme	eda Ave												
4	L2	124	12.7	124	12.7	0.464	36.1	LOS C	2.6	20.0	0.98	0.79	0.98	26.8
6	R2	54	2.0	54	2.0	0.187	34.2	LOS C	1.1	7.6	0.95	0.75	0.95	28.5
Appro	oach	178	9.5	178	9.5	0.464	35.5	LOS C	2.6	20.0	0.97	0.78	0.97	27.4
North	: Tallav	vong Road												
7	L2	21	10.0	21	10.0	0.474	37.0	LOS C	2.4	17.1	0.97	0.77	0.97	8.1
8	T1	211	2.5	211	2.5	0.474	32.2	LOS C	2.4	17.2	0.97	0.76	0.97	8.2
Appro	oach	232	3.2	232	3.2	0.474	32.7	LOS C	2.4	17.2	0.97	0.76	0.97	8.2
All Ve	hicles	1001	4.9	1001	4.9	0.495	26.6	LOS B	7.8	56.1	0.92	0.77	0.92	20.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Network Data dialog (Network tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

Move	ement Performance - Pede	estrians						
Mov ID	Description	Demand Flow ped/h	Average Delay sec	Level of Service	Average Back Pedestrian ped	of Queue Distance m	Prop. Queued	Effective Stop Rate
P1	South Full Crossing	53	29.3	LOS C	0.1	0.1	0.92	0.92
P2	East Full Crossing	53	29.3	LOS C	0.1	0.1	0.92	0.92
P3	North Full Crossing	53	29.3	LOS C	0.1	0.1	0.92	0.92
All Pe	edestrians	158	29.3	LOSC			0.92	0.92

Level of Service (LOS) Method: SIDRA Pedestrian LOS Method (Based on Average Delay)

Pedestrian movement LOS values are based on average delay per pedestrian movement.

Intersection LOS value for Pedestrians is based on average delay for all pedestrian movements.

Appendix G

Car Park Scheme Advice

SHIPROCK ADVISORY

PO Box 1020 Caringbah NSW 1495 · Phone: 0439 646 168

Email: peter@shirprockadvisory.com.au

15 April 2020

Greg Colbran

Development and Planning Executive
Deicorp
Level 3,
161 Redfern Street
Redfern NSW 2016

Dear Greg

Proposed Mixed Use Development incorporating Retail and Commercial Tallawong Station Precinct South- Parking allocation for Retail/Commercial

We refer to the proposed mixed-use site at the above location.

We commenced a retail/commercial marketing campaign in November 2019 to seek interest to lease out the retail and commercial at the above site.

This has involved presentations and meetings with Coles, Woolworths and other Supermarket operators (Aldi, Metcash, Independents) and associated support specialty shops such as; Fresh Food, Food and Beverage, Restaurants, Services, Gym, Child Care, medical, pharmacy and office tenants.

As these conversations commenced last year, we have found some major concerns being raised from our prospective tenants that the proposed parking allocation of 147 retail/commercial car spaces.

The comments from tenants and from my experience is that the parking allocation as proposed is inadequate to meet the customer needs of the Retail/Commercial precinct which will impact the viability of the Centre.

In my experience and if we are unable to achieve adequate parking this will impact on being able to secure retailers and or the appropriate retailers for the site to be able to have a viable business in this location.

Tallawong is an emerging greenfield location which will be a densely populated location in the future however in the meantime the community currently drives to all the retail centers within the catchment.

Whilst I note we have an adjoining commuter car park for the metro station which has been found to be at capacity by 6am in the morning.

Therefore, the proposed amended retail/commercial of 300 car spaces would be adequate to accommodate the parking requirements for a 9000 sqm Retail/Commercial Centre as proposed at Tallawong.

I would welcome the opportunity to discuss this further as to concerns raised on the parking allocation.

Any questions please contact me.

Yours Sincerely

Peter Mokas



14th April 2020

CBRE (RP) Pty Ltd ABN 92 127 174 207

Level 29, 177 Pacific Hwy North Sydney NSW 2060

> T 61 2 8969 8500 F 61 2 8969 8599

> > www.cbre.com.au

Att: John Vamvakaris
Chief Operations Officer-Development
DEICORP
Level 4, 161 Redfern Street
Redfern NSW 2016

Dear John,

Re: Tallawong Car Park Scheme

As one of Australia's largest property services companies, CBRE has a deep understanding of the drivers of demand for residential properties across the country.

Our extensive experience in the sales and marketing of residential properties in new release areas like Sydney's North West enable us to accurately advise on the expectations and preferences of purchasers.

This experience confirms that purchasers of apartments in new development areas across the North West, including purchasers of 1-bedroom apartments, have a strong preference for car parking linked to their apartment. Whilst the major appeal of these areas is affordability and greatly improved transport as a result of the new train line, buyers rely on personal transport i.e. their own car for access to schools sporting amenities visiting friends and family etc. They rely on transport by train mainly for access to and from work, also other city-based events.

Sales of 1-bedroom apartments that do not include parking is significantly slower and results in a disproportionately lower price than comparable apartments with parking provided.

Our strong advice to developers of projects in the North West of Sydney is that a decision to not provide parking for <u>all 1-bedroom</u> apartments will dramatically reduce the saleability and demand for these dwellings.

We believe the correct scheme for Tallawong apartments needs to be as tabled below:

1 bed	1 car space
2 bed	1 car space
3 bed	2 car space

If you have any questions, please do not hesitate to contact me on 0418200236.

Yours sincerely,





David Milton CBRE Managing Director Residential Projects