



**Douglas Partners**

*Geotechnics • Environment • Groundwater*

*Integrated Practical Solutions*

**REPORT**

**on**

**MINE SUBSIDENCE ASSESSMENT  
OF POTHOLE RISK**

**PROPOSED PACIFIC NATIONAL DEPOT  
MANSFIELD STREET, GRETA**

**Prepared for**

**PACIFIC NATIONAL PTY LTD**

**Project 39129.03A**

**MAY 2010**



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**REPORT ON  
MINE SUBSIDENCE ASSESSMENT OF POTHOLE RISK  
PROPOSED PACIFIC NATIONAL DEPOT  
MANSFIELD STREET, GRETA**

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## **1. INTRODUCTION**

This report presents results of additional subsurface investigations to refine the assessment of pothole subsidence risk at the proposed Pacific National (PN) Depot, located off Mansfield Street, Greta, NSW. The assessment was carried out at the request of Pacific National Pty Ltd. It is understood that this report is to form part of PN's submission to the NSW Department of Planning seeking approval for the development.

The additional investigation was carried out in conjunction with a geotechnical investigation for bulk earthworks, the results of which are presented separately (Ref 1). A potential contamination assessment has also been undertaken at this site (Ref 4).

It is understood that the proposed development includes an access road and a series of rail tracks together with buildings and services.

Parts of the development site are underlain by shallow mine workings, as described in the report on Mine Subsidence Risk Assessment, Ref 3. The proposed developments affected by shallow workings include the access road, the three wagon storage roads and the turnout from the main line.

The purpose of the additional subsurface investigations was to supplement the data on shallow mine workings beneath the southern part of the PN site and to provide a basis for the re-assessment of the risk of pothole subsidence.

The results of the supplementary investigation are presented in this report, together with comments on subsidence risk and remedial works.

## **2. BACKGROUND**

Douglas Partners Pty Ltd (DP) completed a preliminary desk study in relation to Mine Subsidence Risk in December 2004 (Ref 2).

A subsurface investigation, comprising four cored bores, was undertaken in 2009 (Ref 3) to confirm the geotechnical model developed during the desk study. Reports from the Mine Subsidence Board (MSB) of a very large pothole opening on the site, together with the presence of coke in one of the cored bores, led to areas with less than 40 m cover being assessed as high pothole subsidence risk. Remedial grouting was suggested for the mine workings beneath the proposed footprint of structures, roads and tracks in areas with less than 40 m cover.

The client requested additional investigation to refine the risk and lateral extent of the mine subsidence hazard. A study comprising the use of seismic surveys and non core boring was undertaken.

## **3. SITE DESCRIPTION**

The site is located to the north of the Greta Railway Station, off Mansfield Street, Greta. The proposed development is bordered on the north-east by the Great Northern Railway and to the south-west by the proposed Hunter Expressway from the F3 to Branxton.

The proposed development is a railway siding off the main northern line for the routine maintenance of PN trains together with several structures housing maintenance facilities.

The site extends about 2.4 km north from the southern boundary of the lot, near Greta Railway Station, and is about 0.5 km wide at its widest. Site access for road vehicles is proposed to be from Mansfield Street, Greta.

#### **4. REGIONAL GEOLOGY**

Reference to the Newcastle Coalfields Regional Geology 1:100,000 Sheet indicates that the site is within the area of outcrop of the Maitland Group and underlain by the Greta Coal Measures. The Greta Coal Measures typically comprise sandstone, conglomerate, siltstone and coal. The Branxton Formation overlies the Greta Coal Measures and typically comprises conglomerate, sandstone and siltstone. The geology encountered in this investigation was consistent with this description.

Although the Greta Coal Measures consists of four seams, the site is underlain by shallow workings in the Greta Seam which is known to consist of two splits in the vicinity of Greta.

#### **5. SCOPE OF WORKS**

The scope of works for the investigation was required to address two issues:

- Confirming safe access for drilling equipment working in areas of low cover;
- Refining the risk of pothole subsidence and the lateral extent of that risk.

The investigation further developed the subsurface model developed in the previous phases of the investigation.

The current investigations comprised:

- Discussions with people familiar with the history of mining in the area;
- Ground probing radar (GPR) to search for potholes or open voids at shallow depth;
- Subsurface investigation using non cored bores to assess the presence of open voids or rock rubble at shallow depth.

For the purposes of this investigation, the client supplied a CAD file of the layout of the proposed development and the existing surface level of the site.

## **6. FIELD WORK**

### **6.1 Field Work Methods**

#### **6.1.1 Ground Probing Radar**

GPR profiling was undertaken over the footprint of the proposed development with areas of less than 20 m cover. The results of that investigation are given in DP report 39129.03-03 dated 28 January 2010. It was anticipated that the GPR would image the subsurface profile to a depth of about 10 m. However because of constraints such as high clay content in the soil, useful data was restricted to a depth of about 3 m to 4 m in the area south of Sawyers Creek and 6 m to 7 m north of the creek. While no evidence of voids or rubble was identified, the relatively shallow depth of penetration did not allow for a 'clearance' from future pothole subsidence risk.

The results of the GPR in the study area north of the creek were considered adequate to allow drilling operations to commence in areas of about 20 m cover and progress to areas with less cover.

### 6.1.2 Rotary Air Boreholes

As originally proposed, the drilling was to target potential voids and rubble chimneys identified by the GPR. In view of the limited penetration achieved by the GPR the methodology was varied to comprise line drilling at 4 m centres. The hole spacing was selected based on the dimensions of the bords shown on the record trace (RT). Three areas were selected for line drilling. These were:

- The proposed access road north of Sawyers Creek (Area 2);
- The proposed access road south of Sawyers Creek (Area 1);
- The proposed main siding (turnout) off the Great Northern Railway, which provides rail access to the site (Area 3).

Three lines of percussion bores were drilled, in the period 4 February to 24 February 2010, starting at approximately the 20 m cover depth to the Greta Seam and getting progressively shallower. The results of the drilling were monitored continuously by a geotechnical engineer and were used to assess the advisability of progressing to areas of shallower cover.

The locations of the bores are shown on Drawing 1, 1A and 1B, Appendix B.

The bores were drilled by a Bombadier (track mounted) rig. The bores were advanced by 100 mm solid flight augers to the level of rock below which rotary air drilling with a 95 mm button bit was used. The bores were continued to the depth of the base of the Greta Seam and were logged on the basis of the returned cuttings and penetration rate by experienced geotechnical engineers. Where voids were intersected, air return was lost and no cuttings were returned to the surface for logging.

### 6.1.3 Down Hole Assessment of Bores

Downhole axial CCTV inspections were undertaken in a number of bores believed to have intersected voids or rubble. The assessment comprised the following:

- Monitoring of gas using a confined spaces gas monitor prior to commencement to confirm the absence of an explosion risk;
- Assessment of the depth to the roof and floor of the mine and the presence of rubble and voids using a down-hole video camera in axial mode for 16 of the boreholes;
- Viewing of selected voids using a tilt up down-hole video camera. The orientation of the camera was obtained by viewing of a magnetic compass card with the camera in vertical orientation prior to tilting up. The camera was rotated at approximately 45° increments (e.g.: N, NE, E etc) prior to tilting up for viewing. The video was recorded to DVD.

#### **6.1.4 Survey**

Monteath & Powys set out the test locations prior to the investigation. Co-ordinates and surface levels were provided and are given on the Borehole Logs, Appendix A. As the work proceeded some additional bores were added to the investigation. These bores were in close proximity to the existing bores and were set out by DP using taped distances from the surveyed bores. The co-ordinates and RL's of such bores are approximate.

### **6.2 Field Work Results**

#### **6.2.1 General**

The subsurface conditions encountered at the test locations are presented in the borehole logs in Appendix A. These should be read in conjunction with the "Notes Relating to This Report" preceding them, which explain the descriptive terms and classification methods.

A summary of the subsurface conditions from this investigation and the terms used are presented below.

## 6.2.2 Air Rotary Boreholes

During drilling using the rotary air method the cuttings, air return and water return and drilling rate were used to infer the likely subsurface profile which was logged and assessments made on the presence of zones of rubble and rod fall. The term 'rod fall' was adopted to indicate zones where the drill string fell under its own weight without rotation or vertical pressure from the rig hydraulics. Such zones are either open voids or are infilled with very loose material.

The logging of the strata has been summarised into a number of interpreted categories for the purpose of this investigation as follows:

- *Sand/clay/weathered sandstone* - soil and extremely weathered rock near the surface;
- *Sandstone: highly weathered, orange-brown* – sandstone strata approximated highly weathered to moderately weathered, generally orange-brown, red-brown and pale grey/whitish in colour;
- *Sandstone: grey (fresh, medium to high strength)* – generally grey sandstone strata of medium to high strength and fresh with some conglomerate and pebbly bands;
- *Coal* – black coal with some dark grey siltstone/sandstone interbedded layers;
- *Rubble* – zones of irregular drilling indicating the absence of solid rock, often with sections of rod fall;
- *Rod Fall* – zones where the weight of the rods and air flow was sufficient to proceed.
- *Rock*: Areas where the strata drilled like intact rock but no cuttings were returned to the surface.

Comments on strength are indicative only as the cuttings returned by rotary air drilling are not suitable for visual/tactile assessment of rock strength and point load testing.

Interpretations of the subsurface conditions in each area are given as geological sections on Drawings 2 to 5 in Appendix B.

### 6.2.3 Down-Hole Assessment of Bores

Bores that were found to have significant sections of rod fall or rubble indicating voids were investigated using down-hole video camera in axial mode. This confirmed the presence of rubble and/or voids and allowed a more accurate assessment of the depth to the roof of the voids and the base of the open void/the top of rubble.

Tilt-up down-hole video camera assessment was undertaken in Bores 112, 216 and 302 as an aid to the assessment of the risk of pothole subsidence. The roof and floor of the open voids were identified, although the latter was masked by rubble. The walls of the voids could not always be identified because of limited penetration of the light and masked by turbidity. The materials comprising the observed walls ranged from coal or rock to rubble.

### 6.2.4 Groundwater

Free groundwater was observed in the open boreholes during the period they remained open. Free groundwater observations within the rotary air bores in Areas 1, 2 and 3 were found to have reduced levels of 42.8 to 42.9 AHD. It should be noted that groundwater levels are dependent on factors such as climatic conditions and soil permeability and therefore will vary with time.

### 6.2.5 Borehole Gas Monitoring

Borehole gas monitoring was undertaken during the drilling and down-hole camera inspections. The testing during drilling was undertaken for health and safety purposes, using a personal gas meter and generally indicated the absence of explosive gases.

Reduced levels of oxygen were recorded and carbon monoxide and hydrogen sulphide were detected in the bores shown below in Table 1. The readings were obtained within 0.5 m of the top of the borehole and generally dissipated within a couple of minutes.

**Table 1 – Results of Borehole Gas Monitoring**

<b>Bore</b>	<b>CH<sub>4</sub> (%)</b>	<b>O<sub>2</sub> (%)</b>	<b>CO ppm</b>	<b>H<sub>2</sub>S ppm</b>	<b>Comments</b>
104	0	18.4	7	5	Flowing
112	0	19.2	0	0	Flowing
203A	0	19	0	0	Flowing
301	0	20.9	35	0	Check
302	0	20.0	36	2	Flowing

## **7. PROPOSED DEVELOPMENT**

It is understood that the proposed development is a facility for servicing coal trains. A rail siding will leave the Great Northern Railway near the southern end of the site and will bifurcate to form five tracks running approximately parallel to the existing railway.

An internal access road will enter from the southern boundary of the site and extend across Sawyers Creek and continue the length of the development approximately parallel to the western site boundary.

Other facilities include an administration building, fuel storage area and buildings to accommodate maintenance and refuelling operations. It is understood that these will be towards the northern end of the site where the thickness of cover over the mine workings exceeds 40 m.

## 8. COMMENTS

### 8.1 Large Pothole in February 2009

As noted above, a large pothole developed in the PN site in February 2009. This was backfilled by the MSB. It is understood that the MSB measured neither the pothole dimensions nor its coordinates. Following informal inquiries from DP in August 2009 the District Officer indicated that he estimated the hole at 30 m long by 15 m wide by 3 m to 4 m deep at the shallow end sloping down to 10 m deep at the deep end. He also indicated that there were four other smaller holes running south away from the large hole. Based on the approximate location indicated by the District Officer, the pothole was about 8 m to 12 m east of Bores 101 to 106 in Area 1 (Access Road South of Sawyers Creek). Potholes of this size are unusual over areas of bord and pillar workings where potholes typically range up to about 5 m in diameter on initial collapse.

Two possible mechanisms could result in a pothole of this size:

The pothole could be formed by the collapse of a large mining void with dimensions of a least 15 m wide by 30 m long and 10 m high. This is an unusually large void. A mined void of these plan dimensions would only occur as part of a bord and pillar operation if pillars were extracted and the roof strata failed to collapse. Alternatively, a fire which destroyed several pillars could also result in a void of these plan dimensions;

**Or**

The pothole could be formed by piping erosion of the clay and clay-like extremely weathered rock. In this model water penetrating into the soils and extremely weathered rock (the soil) drains into subsidence cracks in the top of the bedrock and over time the soil erodes internally by piping until a large area of void develops within the soil under a desiccated clay crust. Eventually the void becomes too large for the clay to span the void and a collapse occurs producing a surface pothole.

The cause of the collapse was not investigated by the MSB who were primarily concerned with backfilling the pothole.

## 8.2 Meeting with Mr Arthur Beckett

Following an introduction from the Greta Historical Society, the senior engineering geologist met with Mr Arthur Beckett, a retired miner. Arthur had been a wheeler (a miner in charge of horses hauling coal wagons in the 1950s). Arthur had not worked in the Anvil Creek/Central Greta Collieries which predated his working life; however Arthur's father had been a miner also and had talked about these earlier mines.

In summary Arthur indicated that:

- The coal in the Central Greta colliery was loaded out using forks so the small coal was left in the mine. The practice was to heap the small coal against the pillars to keep the access clear up the centre of the bord. As the small coal was in a loose condition it had ready contact with the atmosphere and hence would have been at risk of spontaneous combustion. Arthur suggested this was the likely cause of the fires in this colliery;
- In the 1950s there was an open pothole on the north side of Sawyers Creek from which smoke continuously discharged and was thick enough to preclude observation of the floor of the pothole. The fire was extinguished by flooding, possibly in 1955, which also resulted in the abandoned mine workings in this area filling with water;
- Arthur did not know when the pothole had been backfilled but recalled that a number of collapses occurred in the Greta area following the flood and believed that the above pothole was filled as part of the general remediation of collapses that occurred following flooding;
- Arthur could not identify the precise location of the above pothole but said it was definitely near Area 2. Based on landform and variation of vegetation patterns, three possible locations were identified. One was about 14 m west of Bore 211 and 3 m north of Bore 218. The other two possible locations were about 26 m east of Bore 206 and 30 m east of Bore 209 and were under the proposed wagon storage lines.

### **8.3 Access Road South of Sawyers Creek (Area 1)**

#### **8.3.1 General**

The proposed access road traverses an area undermined by bord and pillar workings at a depth ranging from 5 m to 15 m. The line bores drilled to investigate this feature were drilled in an area with an estimated 10 m to 15 m cover over the top of the workings. Because of constraints of access and the presence of existing trees, the line bores followed the existing access track and hence are partly within and partly to the west of the proposed access road footprint. The large pothole backfilled in 2009 by the MSB was about 8 m to 12 m east of Bores 101 to 106 in Area 1.

#### **8.3.2 Subsurface Profile**

The subsurface profile revealed by the line bores generally indicate a thickness of about 9 m to 12 m of soil and highly weathered rock overlying generally medium strength sandstone, which forms the immediate roof of the coal seam/void/rubble areas. The sandstone ranges in thickness from 4 m to 1 m. Refer to Drawing 2, Appendix B.

Where rod falls (possible open voids) were encountered these ranged in height from about 1 m to 2.6 m but are typically about 2 m thick. The voids are generally associated with rubble thicknesses ranging from 1 m to 6 m in thickness but typically about 3 m to 4 m. This suggests that the void is already substantially filled with rubble either as a consequence of small coal left in the workings or because of the bulking of fallen material.

The total depth of cover over these features ranges from about 11 m to 15 m.

#### **8.3.3 Potential for Future Subsidence**

As discussed above there are two modes of surface pothole formation. One is upward migration of the roof of the void by progressive collapse; the other is erosion of clay and extremely low strength rock to produce cavities in the soil which subsequently collapse.

## **Progressive Roof Collapse**

Assuming a bulking factor of about 1.4 (i.e. 30% porosity of rubble) a 1 m high void would choke with rubble prior to reaching a height of 10 m and a 1.5 m high void prior to reaching the height of 15 m. Given the maximum observed void height of about 2.6 m the worse case future pothole subsidence from this mechanism would result in a pothole of up to about 1.6 m deep.

## **Internal Erosion and Piping**

The material removed by such internal erosion is transported by water as fine particles and hence this mechanism is not self limited by bulking as is the case for progressive roof collapse. The potential depth of pothole formation by this method is dependent on the depth of clay and extremely low strength (clay like) weathered rock. The actual depth of such material could not be measured by rotary air drilling but judging by the previous pothole may in some areas be a significant proportion of the highly weathered sandstone material.

## **8.4 Access Road North of Sawyers Creek (Area 2)**

### **8.4.1 General**

The access road north of Sawyers Creek traverses an area of bord and pillar workings which ranges from 10 m to greater than 40 m depth below the surface. In the areas of line drilling the isopachs predict a depth of cover in the range of 10 m to 20 m over the top of the seam. As discussed above it is likely that there was previously an open pothole in the general area which discharged smoke from an underground fire.

### 8.4.2 Subsurface Profile

The longitudinal subsurface profile interpreted from the bores along the centre line of the proposed access road is given in Drawing 3, Appendix B. Two supplementary bores were drilled approximately 25 m to the west of, and 18 m to the east of Bore 212. This transverse profile is shown on Drawing 4, Appendix B.

The longitudinal section indicates a depth of about 1 m to 2 m of clay overlying highly weathered sandstone to a depth ranging from 4 m to 9 m but typically about 8 m to 9 m. This is generally underlain by grey, typically medium strength or stronger sandstone which ranges in thickness from about 3 m to 18 m. Rod falls indicating possible open voids were generally encountered at the base of the grey sandstone unit at depths ranging from 13 m to 18 m. The height of the voids ranged from about 1 m to 3.5 m.

Anomalous conditions were encountered in Bore 211 where a 1.7 m high void was encountered at a depth of about 8 m.

### 8.4.3 Potential for Pothole Failure

#### Progressive Roof Collapse

Voids in adjacent bores at similar elevations are only observed where the void is still within or near the seam and has not migrated upwards. Where voids have propagated upwards the voids in adjacent bores are at different elevations and in some cases the voids have already been choked off by the bulking of rubble. Given that a minimum of 3 m of grey (medium strength) sandstone generally overlies the voids (except for Bore 211) the rate of upward migration of the potholes in most cases will be slow and possibly be terminated by the sandstone. If upward migration is not stopped by the sandstone, the height which the bulking of the rock chimney would choke off the void depends on the diameter of the chimney and the height of the void. Choking heights are given in Table 2 for void heights of 1 m, 2 m and 3 m, and pothole (rock chimney) diameters of 2 m and 5 m.

The strength of the sandstone above the mine void was estimated from the chips and dust returned by air flushing from the drilling and is probably a conservative estimate. Bore 4, drilled in the vicinity of Area 2 in August 2009 (Ref 3) encountered about 14 m of high strength sandstone, with widely spaced joints, over the coal. Conditions in Bore 4 support the basis of the sandstone arresting the upward migration of the pothole.

**Table 2 - Height at Which Voids Will Choke Off**

Void Height (m)	Pothole (Rock Chimney) Diameter 2 m	Pothole (Rock Chimney) Diameter 5 m
1	10 m	4 m
2	11 m	4 m
3	12.6 m	4 m

Based on the above, any pothole formed by progressive collapse of the rock stratum would not be expected to exceed 2 m diameter and up to about 2 m deep.

### **Internal Erosion and Piping**

The material removed by such internal erosion is transported by water as fine particles and hence this mechanism is not self limited by bulking as is the case for progressive roof collapse. The potential depth of pothole formation by this method is dependent on the depth of clay and extremely low strength (clay like) weathered rock. The actual depth of such material could not be measured by rotary air drilling but, judging by the previous cored bores drilled to the west of this line, may be around 2 to 4 m.

### **8.5 Wagon Storage Roads**

No specific subsurface investigation was conducted along the storage roads, however given its proximity to Area 2, it is likely that conditions in this area are similar to the latter.

## 8.6 Turnout from Main Line (Area 3)

### 8.6.1 General

The Mine Plan (RT) indicates limited mining at the northern end of this area in the form of two tunnel headings crossing centrally beneath the section. Line drilling was restricted to the northern 20 m of the area where limited bord and pillar mining was undertaken.

### 8.6.2 Subsurface Profile

The longitudinal section, shown on Drawing 5, Appendix B, indicates a depth of about 1 m of clay/extremely weathered rock overlying highly weathered sandstone to a depth ranging from 10 m to 11 m. This is generally underlain by grey, typically medium strength or stronger sandstone which ranges in thickness from about 15 m to 21 m, but is typically in the range of 19 m to 21 m. With the exception of Bore 304, the medium strength sandstone forms the immediate roof of the seam/void and ranges from 9 m to 11 m thick. In Bore 304 the grey, medium strength sandstone is about 5 m thick and is underlain by about 5.5 m of weathered sandstone which forms the immediate roof of the coal seam at this location.

The strength of the sandstone above the mine void was estimated from the chips and dust returned by air flushing from the drilling and is probably a conservative estimate. Bore 4, drilled in the vicinity of Area 2 in August 2009 (Ref 3) encountered about 14 m of high strength sandstone, with widely spaced joints, over the coal. Conditions in Bore 4 support the basis of the sandstone arresting the upward migration of the pothole.

Rod falls indicating possible open voids were generally encountered at the base of the grey sandstone unit at depths ranging from 13 m to 18 m. Rod fall heights ranging from 1.5 m to 5.8 m were observed while drilling but subsequent CCTV inspection confirmed that the open void height ranged from 0.9 m to 1.9 m and the remainder of the rod fall was loose rubble. The top of the void ranged from a depth of 23 to 25 m and was at or below the top of the coal seam. Hence there appears to be no previously upward migration of mine voids in Area 3.

### **8.6.3 Potential for Pothole Subsidence**

#### **Progressive Roof Collapse**

Given the age of the workings and the present position of the voids it is probable that the rock roof in this location is strong enough to preclude progressive roof collapse and chimney formation. Comparison with Table 2 indicates that if chimneys were to develop in this area they would not be expected to reach the ground surface prior to choking off.

#### **Internal Erosion and Piping**

The material removed by such internal erosion is transported by water as fine particles and hence this mechanism is not self limited by bulking as is the case for progressive roof collapse. The potential depth of pothole formation by this method is dependent on the depth of clay and extremely low strength (clay like) weathered rock. The actual depth of such material could not be measured by rotary air drilling.

### **8.6.4 Tunnel Headings**

A pair of tunnel headings crosses the Turnout and Main Line in an area of shallow cover, as indicated on Drawing 1B, Appendix B. The condition of these tunnels and the roof over these tunnels is unknown. Development practice normally required by the MSB and DPI-Minerals is for such features to be located and backfilled during construction and hence these have not been considered during this investigation.

## 8.7 Remedial Works

### 8.7.1 General

The geotechnical investigation indicates that progressive roof collapse at the PN site would either choke off prior to reaching the surface or form small potholes with a maximum diameter of about 2 m and a maximum depth of about 2 m but typically less than 1 m. No surface expression of pothole subsidence from progressive roof collapse would be anticipated in areas exceeding 20 m cover.

There is however a risk of significantly larger subsidence potholes forming as a result of internal erosion and piping of clay and erodible weathered rock as a consequence of water inflow into the potholes and choked off debris chimneys, as is interpreted to be the cause of the large pothole observed in 2009.

Having regard to the ground conditions, including the strong sandstone roof and depth of cover, the measures for protection of the structures proposed for the areas of shallow mining and personnel who will use the development are described in the following sections.

### 8.7.2 Area 1, Access Road

The depth of cover in Area 1 is only about 10 m to 15 m. It is understood that the Access Road, including drainage and other related services, is the only development proposed for Area 1.

The mine voids beneath Area 1 should be fully grouted and any existing piping within the soils should be eliminated by grouting, by over-excavation and recompaction or by forced collapse by means of dynamic compaction.

Grouting and soil improvement works would need to extend at least 5 m each side of the Access Road.

### 8.7.3 Area 2, Access Road

The use of a continuously reinforced, possibly post-tensioned, concrete pavement designed to span a 5 m void is recommended for Area 2, the Access Road north of Sawyers Creek, where the depth of cover is less than 20 m.

Site improvements to minimise piping erosion, together with monitoring of the access road area for the incidence of pothole subsidence, should include the measures described below:

- Excavation of the footprint of the Access Road, and an additional 5 m each side beyond the footprint, to the top of very low strength, or better, rock. The anticipated depth of excavation in Area 2 is about 1 m to 3 m, based on the logging of the recent non-core bores. The depths would need to be confirmed during construction;
- Over-excavation of any cracks or rubble chimneys and backfilling with dental concrete to seal the rock surface against soil and water inflow;
- Placement of about 150 mm blinding layer of sand;
- Placement of a grid of coax cable for time-domain reflectometry to detect pothole formation at the top of the bedrock. The coax should be covered with a blinding layer of about 150 mm of sand;
- Place heavy duty geotextile filter cloth over the blinding layer prior to placing general filling;
- Three layers of soil reinforcing geogrids should be included in the lower 450 mm to 500 mm of general filling above the sand blinding layer;
- If the monitoring detects the onset of pothole subsidence then remedial grouting would be required to grout the rubble chimney/void.

The risk of piping erosion will be further controlled by drainage measures, including surface grades, to reduce or eliminate infiltration of stormwater into the soil and rock strata.

#### 8.7.4 Area 3, Turnout from Main Line

The abandoned tunnel headings under the central section of the turnout need to be located and backfilled. Depending on the depth of cover this could be undertaken by bulk earthworks or drilling and grouting.

There is little risk of future potholing over the limited bord and pillar workings with less than 20 m cover at the northern end of the siding. It is however noted that any pothole subsidence in this areas, if it occurred, would cause shutdown of the facility. Hence it is suggested that grouting be undertaken to backfill these workings under and within 5 m of the footprint of the siding, notwithstanding that future pothole subsidence is considered very unlikely in this area.

#### 8.7.5 Wagon Storage Roads

The wagon storage roads lie close to the Access Road in the shallow mining area north of Sawyers Creek. Potholes in this area are expected to be less than about 2 m diameter and 2 m deep, provided that drainage measures are in place to control surface runoff and reduce the risk of pothole enlargement by piping erosion.

The following works should be undertaken to prepare the wagon storage roads:

- Excavation of the footprint of the wagon storage roads (where the cover depth is less than 20 m), and an additional 5 m each side beyond the footprint, to the top of very low strength, or better, rock; the anticipated depth of excavation is about 1 m to 3 m, based on the logging of the recent non-core bores; the depths would need to be confirmed during construction;
- Over-excavation of any cracks or rubble chimneys and backfilling with dental concrete to seal the rock surface against soil and water inflow;
- Backfill the excavation up to subgrade level using the excavated soil and weathered rock, placed and compacted under engineering control.

Drainage measures should include lined table drains, sealed pits and adequate surface grades to prevent ponding of water.

## 9. LIMITATIONS

DP has prepared this report for this project at the proposed Pacific National Depot, Mansfield Street, Greta in accordance with DP's proposal NCL090449-4 dated 22 December 2009. The work was carried out under DP Conditions of Engagement. This report is provided for the exclusive use of Pacific National Pty Ltd for the specific project and purpose as described in the report. It should not be relied upon for other projects or purposes on the same or other site or by a third party.

The results provided in the report are considered to be indicative of the sub-surface conditions on the site only to the depths investigated at the specific sampling and/or testing locations, and only at the time the work was carried out. DP's advice may be based on observations, measurements, and tests or derived interpretations. The accuracy of the advice provided by DP in this report is limited by unobserved features and variations in ground conditions across the site in areas between test locations and beyond the site boundaries or by variations with time. The advice may be limited by restrictions in the sampling and testing which was able to be carried out, as well as by the amount of data that could be collected given the project and site constraints. Actual ground conditions observed or inferred at the test locations may differ from those which may be encountered elsewhere on the site. Should variations in subsurface conditions be encountered, then additional advice should be sought from DP and, if required, amendments made.

This report must be read in conjunction with the attached "Notes Relating to This Report" and any other attached explanatory notes and should be kept in its entirety without separation of individual pages or sections. DP cannot be held responsible for interpretations or conclusions from review by others of this report or test data, which are not otherwise supported by an expressed statement, interpretation, outcome or conclusion stated in this report. In preparing this report DP has necessarily relied upon information provided by the client and/or their agents.

**DOUGLAS PARTNERS PTY LTD**

Reviewed by:

**Greg Hawkins**  
Senior Associate**John Harvey**  
Principal**REFERENCES**

1. Douglas Partners “Geotechnical Investigation for Bulk Earthworks, Pacific National Depot”, Project 39129.03, dated March 2010.
2. Douglas Partners “Geotechnical Assessment of Mine Subsidence Risk. Proposed rail facility, Greta”, Project 39129, dated 14 December 2004.
3. Douglas Partners “Mine Subsidence Risk Assessment, Proposed Pacific National Depot”, Project 39129.01, dated October 2009.
4. Douglas Partners “Contamination Assessment, Proposed Pacific National Depot”, Project 39129.02, dated December 2009.

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**APPENDIX A**

***Notes Relating to this Report  
Borehole Logs (101 to 120, 111A, 200 to 219, 203A, 207A  
and 301 to 306)***

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## NOTES RELATING TO THIS REPORT

### Introduction

These notes have been provided to amplify the geotechnical report in regard to classification methods, specialist field procedures and certain matters relating to the Discussion and Comments section. Not all, of course, are necessarily relevant to all reports.

Geotechnical reports are based on information gained from limited subsurface test boring and sampling, supplemented by knowledge of local geology and experience. For this reason, they must be regarded as interpretive rather than factual documents, limited to some extent by the scope of information on which they rely.

### Description and Classification Methods

The methods of description and classification of soils and rocks used in this report are based on Australian Standard 1726, Geotechnical Site Investigations Code. In general, descriptions cover the following properties - strength or density, colour, structure, soil or rock type and inclusions.

Soil types are described according to the predominating particle size, qualified by the grading of other particles present (eg. sandy clay) on the following bases:

Soil Classification	Particle Size
Clay	less than 0.002 mm
Silt	0.002 to 0.06 mm
Sand	0.06 to 2.00 mm
Gravel	2.00 to 60.00 mm

Cohesive soils are classified on the basis of strength either by laboratory testing or engineering examination. The strength terms are defined as follows.

Classification	Undrained Shear Strength kPa
Very soft	less than 12
Soft	12—25
Firm	25—50
Stiff	50—100
Very stiff	100—200
Hard	Greater than 200

Non-cohesive soils are classified on the basis of relative density, generally from the results of standard penetration tests (SPT) or Dutch cone penetrometer tests (CPT) as below:

Relative Density	SPT "N" Value (blows/300 mm)	CPT Cone Value (q <sub>c</sub> — MPa)
Very loose	less than 5	less than 2
Loose	5—10	2—5
Medium dense	10—30	5—15
Dense	30—50	15—25
Very dense	greater than 50	greater than 25

Rock types are classified by their geological names. Where relevant, further information regarding rock classification is given on the following sheet.

### Sampling

Sampling is carried out during drilling to allow engineering examination (and laboratory testing where required) of the soil or rock.

Disturbed samples taken during drilling provide information on colour, type, inclusions and, depending upon the degree of disturbance, some information on strength and structure.

Undisturbed samples are taken by pushing a thin-walled sample tube into the soil and withdrawing with a sample of the soil in a relatively undisturbed state. Such samples yield information on structure and strength, and are necessary for laboratory determination of shear strength and compressibility. Undisturbed sampling is generally effective only in cohesive soils.

Details of the type and method of sampling are given in the report.

### Drilling Methods.

The following is a brief summary of drilling methods currently adopted by the Company and some comments on their use and application.

**Test Pits** — these are excavated with a backhoe or a tracked excavator, allowing close examination of the in-situ soils if it is safe to descent into the pit. The depth of penetration is limited to about 3 m for a backhoe and up to 6 m for an excavator. A potential disadvantage is the disturbance caused by the excavation.

**Large Diameter Auger (eg. Pengo)** — the hole is advanced by a rotating plate or short spiral auger, generally 300 mm or larger in diameter. The cuttings are returned to the surface at intervals (generally of not more than 0.5 m) and are disturbed but usually unchanged in moisture content. Identification of soil strata is generally much more reliable than with continuous spiral flight augers, and is usually supplemented by occasional undisturbed tube sampling.

**Continuous Sample Drilling** — the hole is advanced by pushing a 100 mm diameter socket into the ground and withdrawing it at intervals to extrude the sample. This is the most reliable method of drilling in soils, since moisture content is unchanged and soil structure, strength, etc. is only marginally affected.

**Continuous Spiral Flight Augers** — the hole is advanced using 90—115 mm diameter continuous spiral flight augers which are withdrawn at intervals to allow sampling or in-situ testing. This is a relatively economical means of drilling in clays and in sands above the water

table. Samples are returned to the surface, or may be collected after withdrawal of the auger flights, but they are very disturbed and may be contaminated. Information from the drilling (as distinct from specific sampling by SPTs or undisturbed samples) is of relatively lower reliability, due to remoulding, contamination or softening of samples by ground water.

**Non-core Rotary Drilling** — the hole is advanced by a rotary bit, with water being pumped down the drill rods and returned up the annulus, carrying the drill cuttings. Only major changes in stratification can be determined from the cuttings, together with some information from 'feel' and rate of penetration.

**Rotary Mud Drilling** — similar to rotary drilling, but using drilling mud as a circulating fluid. The mud tends to mask the cuttings and reliable identification is again only possible from separate intact sampling (eg. from SPT).

**Continuous Core Drilling** — a continuous core sample is obtained using a diamond-tipped core barrel, usually 50 mm internal diameter. Provided full core recovery is achieved (which is not always possible in very weak rocks and granular soils), this technique provides a very reliable (but relatively expensive) method of investigation.

## Standard Penetration Tests

Standard penetration tests (abbreviated as SPT) are used mainly in non-cohesive soils, but occasionally also in cohesive soils as a means of determining density or strength and also of obtaining a relatively undisturbed sample. The test procedure is described in Australian Standard 1289, "Methods of Testing Soils for Engineering Purposes" — Test 6.3.1.

The test is carried out in a borehole by driving a 50 mm diameter split sample tube under the impact of a 63 kg hammer with a free fall of 760 mm. It is normal for the tube to be driven in three successive 150 mm increments and the 'N' value is taken as the number of blows for the last 300 mm. In dense sands, very hard clays or weak rock, the full 450 mm penetration may not be practicable and the test is discontinued.

The test results are reported in the following form.

- In the case where full penetration is obtained with successive blow counts for each 150 mm of say 4, 6 and 7

as      4, 6, 7  
          N = 13

- In the case where the test is discontinued short of full penetration, say after 15 blows for the first 150 mm and 30 blows for the next 40 mm

as      15, 30/40 mm.

The results of the tests can be related empirically to the engineering properties of the soil.

Occasionally, the test method is used to obtain samples in 50 mm diameter thin walled sample tubes in clays. In such circumstances, the test results are shown on the borelogs in brackets.

## Cone Penetrometer Testing and Interpretation

Cone penetrometer testing (sometimes referred to as Dutch cone — abbreviated as CPT) described in this report has been carried out using an electrical friction cone penetrometer. The test is described in Australian Standard 1289, Test 6.4.1.

In the tests, a 35 mm diameter rod with a cone-tipped end is pushed continuously into the soil, the reaction being provided by a specially designed truck or rig which is fitted with an hydraulic ram system. Measurements are made of the end bearing resistance on the cone and the friction resistance on a separate 130 mm long sleeve, immediately behind the cone. Transducers in the tip of the assembly are connected by electrical wires passing through the centre of the push rods to an amplifier and recorder unit mounted on the control truck.

As penetration occurs (at a rate of approximately 20 mm per second) the information is plotted on a computer screen and at the end of the test is stored on the computer for later plotting of the results.

The information provided on the plotted results comprises: —

- Cone resistance — the actual end bearing force divided by the cross sectional area of the cone — expressed in MPa.
- Sleeve friction — the frictional force on the sleeve divided by the surface area — expressed in kPa.
- Friction ratio — the ratio of sleeve friction to cone resistance, expressed in percent.

There are two scales available for measurement of cone resistance. The lower scale (0—5 MPa) is used in very soft soils where increased sensitivity is required and is shown in the graphs as a dotted line. The main scale (0—50 MPa) is less sensitive and is shown as a full line.

The ratios of the sleeve friction to cone resistance will vary with the type of soil encountered, with higher relative friction in clays than in sands. Friction ratios of 1%—2% are commonly encountered in sands and very soft clays rising to 4%—10% in stiff clays.

In sands, the relationship between cone resistance and SPT value is commonly in the range:—

$$q_c \text{ (MPa)} = (0.4 \text{ to } 0.6) N \text{ (blows per 300 mm)}$$

In clays, the relationship between undrained shear strength and cone resistance is commonly in the range:—

$$q_c = (12 \text{ to } 18) c_u$$

Interpretation of CPT values can also be made to allow estimation of modulus or compressibility values to allow calculation of foundation settlements.

Inferred stratification as shown on the attached reports is assessed from the cone and friction traces and from experience and information from nearby boreholes, etc. This information is presented for general guidance, but must be regarded as being to some extent interpretive. The test method provides a continuous profile of engineering properties, and where precise information on soil classification is required, direct drilling and sampling may be preferable.

## Hand Penetrometers

Hand penetrometer tests are carried out by driving a rod into the ground with a falling weight hammer and measuring the blows for successive 150 mm increments of penetration. Normally, there is a depth limitation of 1.2 m but this may be extended in certain conditions by the use of extension rods.

Two relatively similar tests are used.

- Perth sand penetrometer — a 16 mm diameter flat-ended rod is driven with a 9 kg hammer, dropping 600 mm (AS 1289, Test 6.3.3). This test was developed for testing the density of sands (originating in Perth) and is mainly used in granular soils and filling.
- Cone penetrometer (sometimes known as the Scala Penetrometer) — a 16 mm rod with a 20 mm diameter cone end is driven with a 9 kg hammer dropping 510 mm (AS 1289, Test 6.3.2). The test was developed initially for pavement subgrade investigations, and published correlations of the test results with California bearing ratio have been published by various Road Authorities.

## Laboratory Testing

Laboratory testing is carried out in accordance with Australian Standard 1289 "Methods of Testing Soil for Engineering Purposes". Details of the test procedure used are given on the individual report forms.

## Bore Logs

The bore logs presented herein are an engineering and/or geological interpretation of the subsurface conditions, and their reliability will depend to some extent on frequency of sampling and the method of drilling. Ideally, continuous undisturbed sampling or core drilling will provide the most reliable assessment, but this is not always practicable, or possible to justify on economic grounds. In any case, the boreholes represent only a very small sample of the total subsurface profile.

Interpretation of the information and its application to design and construction should therefore take into account the spacing of boreholes, the frequency of sampling and the possibility of other than 'straight line' variations between the boreholes.

## Ground Water

Where ground water levels are measured in boreholes, there are several potential problems;

- In low permeability soils, ground water although present, may enter the hole slowly or perhaps not at all during the time it is left open.
- A localised perched water table may lead to an erroneous indication of the true water table.
- Water table levels will vary from time to time with seasons or recent weather changes. They may not be

the same at the time of construction as are indicated in the report.

- The use of water or mud as a drilling fluid will mask any ground water inflow. Water has to be blown out of the hole and drilling mud must first be washed out of the hole if water observations are to be made.

More reliable measurements can be made by installing standpipes which are read at intervals over several days, or perhaps weeks for low permeability soils. Piezometers, sealed in a particular stratum, may be advisable in low permeability soils or where there may be interference from a perched water table.

## Engineering Reports

Engineering reports are prepared by qualified personnel and are based on the information obtained and on current engineering standards of interpretation and analysis. Where the report has been prepared for a specific design proposal (eg. a three storey building), the information and interpretation may not be relevant if the design proposal is changed (eg. to a twenty storey building). If this happens, the Company will be pleased to review the report and the sufficiency of the investigation work.

Every care is taken with the report as it relates to interpretation of subsurface condition, discussion of geotechnical aspects and recommendations or suggestions for design and construction. However, the Company cannot always anticipate or assume responsibility for:

- unexpected variations in ground conditions — the potential for this will depend partly on bore spacing and sampling frequency
- changes in policy or interpretation of policy by statutory authorities
- the actions of contractors responding to commercial pressures.

If these occur, the Company will be pleased to assist with investigation or advice to resolve the matter.

## Site Anomalies

In the event that conditions encountered on site during construction appear to vary from those which were expected from the information contained in the report, the Company requests that it immediately be notified. Most problems are much more readily resolved when conditions are exposed than at some later stage, well after the event.

## Reproduction of Information for Contractual Purposes

Attention is drawn to the document "Guidelines for the Provision of Geotechnical Information in Tender Documents", published by the Institution of Engineers, Australia. Where information obtained from this investigation is provided for tendering purposes, it is recommended that all information, including the written report and discussion, be made available. In circumstances where the discussion or comments section

is not relevant to the contractual situation, it may be appropriate to prepare a specially edited document. The Company would be pleased to assist in this regard and/or to make additional report copies available for contract purposes at a nominal charge.

### **Site Inspection**

The Company will always be pleased to provide engineering inspection services for geotechnical aspects of work to which this report is related. This could range from a site visit to confirm that conditions exposed are as expected, to full time engineering presence on site.

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## AN ENGINEERING CLASSIFICATION OF SEDIMENTARY ROCKS IN THE SYDNEY AREA

This classification system provides a standardized terminology for the engineering description of the sandstone and shales in the Sydney area, but the terms and definitions may be used elsewhere when applicable.

Under this system rocks are classified by Rock Type, Degree of Weathering, Strength, Stratification Spacing, and Degree of Fracturing. These terms do not cover the full range of engineering properties. Descriptions of rock may also need to refer to other properties (e.g. durability, abrasiveness, etc.) where these are relevant.

### ROCK TYPE DEFINITIONS

Rock Type	Definition
Conglomerate:	More than 50% of the rock consists of gravel sized (greater than 2mm) fragments
Sandstone:	More than 50% of the rock consists of sand sized (.06 to 2mm) fragments
Siltstone:	More than 50% of the rock consists of silt-sized (less than 0.06mm) granular particles and the rock is not laminated
Claystone:	More than 50% of the rock consists of clay or sericitic material and the rock is not laminated
Shale:	More than 50% of the rock consists of silt or clay sized particles and the rock is laminated

Rocks possessing characteristics of two groups are described by their predominant particle size with reference also to the minor constituents, e.g. clayey sandstone, sandy shale.

### DEGREE OF WEATHERING

Term	Symbol	Definition
Extremely Weathered	EW	Rock substance affected by weathering to the extent that the rock exhibits soil properties - i.e. it can be remoulded and can be classified according to the Unified Classification System, but the texture of the original rock is still evident.
Highly Weathered	HW	Rock substance affected by weathering to the extent that limonite staining or bleaching affects the whole of the rock substance and other signs of chemical or physical decomposition are evident. Porosity and strength may be increased or decreased compared to the fresh rock usually as a result of iron leaching or deposition. The colour and strength of the original fresh rock substance is no longer recognisable.
Moderately Weathered	MW	Rock substance affected by weathering to the extent that staining or discolouration of the rock substance usually by limonite has taken place. The colour and texture of the fresh rock is no longer recognisable.
Slightly Weathered	SW	Rock substance affected by weathering to the extent that partial staining or discolouration of the rock substance usually by limonite has taken place. The colour and texture of the fresh rock is recognisable.
Fresh	Fs	Rock substance unaffected by weathering, limonite staining along joints.
Fresh	Fr	Rock substance unaffected by weathering.

### STRATIFICATION SPACING

Term	Separation of Stratification Planes
Thinly laminated	<6 mm
Laminated	6 mm to 20 mm
Very thinly bedded	20 mm to 60 mm
Thinly bedded	60 mm to 0.2 m
Medium bedded	0.2 m to 0.6 m
Thickly bedded	0.6 m to 2 m
Very thickly bedded	>2 m

## ROCK STRENGTH

Rock strength is defined by the Point Load Strength Index (Is 50) and refers to the strength of the rock substance in the direction normal to the bedding. The test procedure is described by the International Society of Rock Mechanics (Reference).

Strength Term	Is(50) MPa	Field Guide	Approx. qu MPa*
Extremely Low:	0.03	Easily remoulded by hand to a material with soil properties	0.7
Very Low:	0.1	May be crumbled in the hand. Sandstone is "sugary" and friable.	2.4
Low:	0.3	A piece of core 150 mm long x 50 mm dia. may be broken by hand and easily scored with a knife. Sharp edges of core may be friable and break during handling.	7
Medium:	1	A piece of core 150 mm long x 50 mm dia. can be broken by hand with considerable difficulty. Readily scored with knife.	24
High:	3	A piece of core 150 mm long x 50 mm dia. cannot be broken by unaided hands, can be slightly scratched or scored with knife.	70
Very High:	10	A piece of core 150 mm long x 50 mm dia. may be broken readily with hand held hammer. Cannot be scratched with pen knife.	240
Extremely High:		A piece of core 150 mm long x 50 mm dia. is difficult to break with hand held hammer. Rings when struck with a hammer.	

\* The approximate unconfined compressive strength (qu) shown in the table is based on an assumed ratio to the point load index of 24:1. This ratio may vary widely.

## DEGREE OF FRACTURING

This classification applies to diamond drill cores and refers to the spacing of all types of natural fractures along which the core is discontinuous. These include bedding plane partings, joints and other rock defects, but exclude known artificial fractures such as drilling breaks






















Term	Description
Fragmented:	The core is comprised primarily of fragments of length less than 20 mm, and mostly of width less than the core diameter.
Highly Fractured:	Core lengths are generally less than 20 mm - 40 mm with occasional fragments.
Fractured:	Core lengths are mainly 30 mm - 100 mm with occasional shorter and longer sections.
Slightly Fractured:	Core lengths are generally 300 mm - 1000 mm with occasional longer sections and occasional sections of 100 mm - 300 mm.
Unbroken:	The core does not contain any fracture.

## REFERENCE











International Society of Rock Mechanics, Commission on Standardisation of Laboratory and Field Tests, Suggested Methods for Determining the Uniaxial Compressive Strength of Rock Materials and the Point Load Strength Index, Committee on Laboratory Tests Document No. 1 Final Draft October 1972

## GRAPHIC SYMBOLS FOR SOIL & ROCK




### SOIL

	BITUMINOUS CONCRETE
	CONCRETE
	TOPSOIL
	FILLING
	PEAT
	CLAY
	SILTY CLAY
	SANDY CLAY
	GRAVELLY CLAY
	SHALY CLAY
	SILT
	CLAYEY SILT
	SANDY SILT
	SAND
	CLAYEY SAND
	SILTY SAND
	GRAVEL
	SANDY GRAVEL
	CLAYEY GRAVEL
	COBBLES/BOULDERS
	TALUS





### SEDIMENTARY ROCK

	BOULDER CONGLOMERATE
	CONGLOMERATE
	CONGLOMERATIC SANDSTONE
	SANDSTONE FINE GRAINED
	SANDSTONE COARSE GRAINED
	SILTSTONE
	LAMINITE
	MUDSTONE, CLAYSTONE, SHALE
	COAL
	LIMESTONE

### METAMORPHIC ROCK

	SLATE, PHYLITTE, SCHIST
	GNEISS
	QUARTZITE

### IGNEOUS ROCK

	GRANITE
	DOLERITE, BASALT
	TUFF
	PORPHYRY

# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.11  
**EASTING:** 335644.8  
**NORTHING:** 1381995.3  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 101  
**PROJECT No:** 39129.03  
**DATE:** 12 Feb 10  
**SHEET** 1 OF 1

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54.0	1	SAND / CLAY / WEATHERED SANDSTONE								
53.0	3.0	SANDSTONE - Highly weathered, orange-brown								
52.0		From 8.25m, whitish bands								
51.0		From 10m, grey bands								
50.0	10.5	SANDSTONE - Grey (fresh, medium to high strength)					▼			
49.0		From 13.25m, possible rubble bands								
48.0	14.3	COAL - Black coal (fractured)								
47.0										
46.0										
45.0										
44.0										
43.0										
42.0										
41.0										
40.0										
39.0										
38.0										
37.0										
36.0										
35.0										
34.0										
33.0										
32.0										
31.0										
30.0										
29.0										
28.0										
27.0										
26.0										
25.0										
24.0										
23.0										
22.4	22.4	Bore discontinued at 22.4m, limit of investigation								

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-4m  
**TYPE OF BORING:** Solid flight auger to 4m, rotary air from 4m to 22.4m  
**WATER OBSERVATIONS:** Free groundwater observed at 11.2m, 16/3/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND			
A	Auger sample	pp	Pocket penetrometer (kPa)
D	Disturbed sample	PID	Photo ionisation detector
B	Bulk sample	S	Standard penetration test
U	Tube sample (x mm dia.)	PL	Point load strength Is(50) MPa
W	Water sample	V	Shear Vane (kPa)
C	Core drilling	▷	Water seep      ▽ Water level

CHECKED
Initials:
Date:



# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.25  
**EASTING:** 335642.5  
**NORTHING:** 1381992  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 102  
**PROJECT No:** 39129.03  
**DATE:** 12 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54.0	1	SAND / CLAY / WEATHERED SANDSTONE								
53.0	3.0	SANDSTONE - Highly weathered, orange-brown								
52.0	8	From 8m, grey-whitish bands								
51.0	10.0	SANDSTONE - Grey (fresh, medium to high strength)								
50.0	11.3	From 11.3m to 12m, possible rubble bands								
49.0	12.9	At 12.9m, coal band ~100mm								
48.0	14.8	COAL - Black coal								
47.0	20.4	SANDSTONE - Grey								
46.0	21.9	Bore discontinued at 21.9m, limit of investigation								

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-4m  
**TYPE OF BORING:** Solid flight auger to 4m, rotary air from 4m to 21.9m  
**WATER OBSERVATIONS:** Free groundwater observed at 11.4m, 16/3/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	▷ Water seep      ▽ Water level

CHECKED
Initials:
Date:







# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.72  
**EASTING:** 335636.1  
**NORTHING:** 1381981.9  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 105  
**PROJECT No:** 39129.03  
**DATE:** 15 Feb 10  
**SHEET** 1 OF 1

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54.0	1	SAND / CLAY / WEATHERED SANDSTONE								
53.0	3.0	SANDSTONE - Highly weathered, orange-brown								
52.0										
51.0										
50.0										
49.0										
48.0										
47.0										
46.0										
45.0										
44.0	10.4	At 10.3m, possible rubble band								
43.0		SANDSTONE - Grey (fresh, medium to high strength)								
42.0										
41.0		At 12.2m, possible rubble band ~100mm								
40.0	13.2	ROD FALL								
39.0										
38.0										
37.0	15.2	RUBBLE with some ROD FALL								
36.0										
35.0	17.2	ROCK								
34.0		From 17.5m, soft zones								
33.0	18.5	RUBBLE with some ROD FALL								
32.0										
31.0	21.3	ROCK								
30.0	22.3	Bore discontinued at 22.3m, limit of investigation								
29.0										
28.0										
27.0										
26.0										
25.0										

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-4m  
**TYPE OF BORING:** Solid flight auger to 4m, rotary air from 4m to 22.3m  
**WATER OBSERVATIONS:** Free groundwater observed at 11.8m, 16/3/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	▷ Water seep      ▽ Water level

CHECKED
Initials:
Date:



# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.72  
**EASTING:** 335635  
**NORTHING:** 1381978  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 106  
**PROJECT No:** 39129.03  
**DATE:** 15 Feb 10  
**SHEET** 1 OF 1

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54 53 52 51 50 49 48 47 46 45 44 43 42 41 40 39 38 37 36 35 34 33 32 31 30 29	1 2 3 4 5 6 7 8 9 10 10.5 11 12 13 13.4 14 15 16 16.2 17 17.2 18 19 19.4 20 21 21.0 22 22.0 23 24 25 26 27 28 29	SAND / CLAY / WEATHERED SANDSTONE  SANDSTONE - Highly weathered, orange-brown  SANDSTONE - Grey (fresh, medium to high strength) At 10.9m, possible rubble band, ~50mm-100mm  ROD FALL  RUBBLE  ROCK From 17.5m to 18.4m, soft band  RUBBLE  ROCK  Bore discontinued at 22.0m, limit of investigation						1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29		

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-4m  
**TYPE OF BORING:** Solid flight auger to 4m, rotary air from 4m to 22m  
**WATER OBSERVATIONS:** Free groundwater observed at 11.9m, 16/3/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	> Water seep      ▽ Water level

CHECKED
Initials:
Date:





# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.77  
**EASTING:** 335632.8  
**NORTHING:** 1381970.4  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 108  
**PROJECT No:** 39129.03  
**DATE:** 16 Feb 10  
**SHEET** 1 OF 1

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54.0	1	SAND / CLAY / WEATHERED SANDSTONE								
53.0	3.0	SANDSTONE - Highly weathered, orange-brown								
52.0	4									
51.0	5									
50.0	6									
49.0	7									
48.0	8									
47.0	9									
46.0	9.7	ROD FALL								
45.0	9.8	SANDSTONE - Grey (fresh, medium to high strength)								
44.0	11	From 11.9m to 12.8m, soft zone					▼			
43.0	12									
42.0	13.6	RUBBLE								
41.0	14									
40.0	14.9	ROD FALL / LOOSE RUBBLE								
39.0	15									
38.0	16									
37.0	17									
36.0	18									
35.0	19									
34.0	20									
33.0	20.75	RUBBLE								
32.0	21									
31.0	21.15	ROCK								
30.0	22									
29.0	22.2	Bore discontinued at 22.2m, limit of investigation								
28.0	23									
27.0	24									
26.0	25									
25.0	26									
24.0	27									
23.0	28									
22.0	29									

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-4m  
**TYPE OF BORING:** Solid flight auger to 4m, rotary air from 4m to 22.2m  
**WATER OBSERVATIONS:** Free groundwater observed at 11.9m, 16/3/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client, CCTV to 16.1m, blocked hole

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	▷ Water seep      ▽ Water level

CHECKED
Initials:
Date:





# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 55.02  
**EASTING:** 335627.3  
**NORTHING:** 1381964.9  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 110  
**PROJECT No:** 39129.03  
**DATE:** 16 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
55.0	1	SAND / CLAY / WEATHERED SANDSTONE								
53.0	2									
51.0	3									
49.0	4									
47.0	5.0	SANDSTONE - Highly weathered, orange-brown								
45.0	6									
43.0	7									
41.0	8									
39.0	9									
37.0	10									
35.0	11.0	SANDSTONE - Grey (fresh, medium to high strength)								
33.3	12						▼			
31.3	13.3	RUBBLE								
29.3	13.65									
27.3	14	SANDSTONE - Grey								
25.3	15									
23.3	16.3	At 15.8m, coal band, ~50mm-100mm								
21.3	17	COAL - Black coal								
19.3	18									
17.3	19									
15.3	20									
13.3	20.7	SANDSTONE - Grey								
11.3	21									
9.3	22									
7.3	22.4	Bore discontinued at 22.4m, limit of investigation								
5.3	23									
3.3	24									
1.3	25									
-0.7	26									
-2.7	27									
-4.7	28									
-6.7	29									

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-4m  
**TYPE OF BORING:** Solid flight auger to 4m, rotary air from 4m to 22.4m  
**WATER OBSERVATIONS:** Free groundwater observed at 12.2m, 16/3/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	▷ Water seep      ▽ Water level

CHECKED
Initials:
Date:



# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.68  
**EASTING:** 335625.4  
**NORTHING:** 1381960.5  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 111  
**PROJECT No:** 39129.03  
**DATE:** 16 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
64.0	1	SAND / CLAY / WEATHERED SANDSTONE								
53.0	4.0	SANDSTONE - Highly weathered, orange-brown								
44.0	7	From 7m, greyish bands								
43.0	10.7	SANDSTONE - Grey (fresh, medium to high strength)					▼			
41.0	13.1	RUBBLE								
40.0	13.6	SANDSTONE - Grey								
38.0	16.5	COAL - Black coal								
32.0	21.2	ROCK								
32.6	22.6	Bore discontinued at 22.6m, limit of investigation								

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-4m  
**TYPE OF BORING:** Solid flight auger to 4m, rotary air from 4m to 22.4m  
**WATER OBSERVATIONS:** Free groundwater observed at 11.8m, 16/3/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	▷ Water seep      ▽ Water level

CHECKED
Initials:
Date:


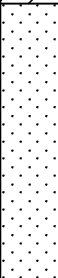












# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.76  
**EASTING:** 335625.9  
**NORTHING:** 1381955.3  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 111A  
**PROJECT No:** 39129.03  
**DATE:** 17 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54.0	1	SAND / CLAY / WEATHERED SANDSTONE								
53.0	2									
52.0	3									
51.0	4									
50.0	4.5	SANDSTONE - Highly weathered, orange-brown								
49.0	5									
48.0	6									
47.0	7	From 7m, whitish bands								
46.0	8									
45.0	9									
44.0	10.0	SANDSTONE - Grey (fresh, medium to high strength)								
43.0	11									
42.0	12						▼			
41.0	12.8	RUBBLE								
40.0	13.4	SANDSTONE - Grey								
39.0	14	At 14m, possible rubble band, ~100mm								
38.0	14.9	At 14.4m, possible rubble band, ~100mm								
37.0	15.5	RUBBLE								
36.0	16	SANDSTONE - Grey								
35.0	16.9	At 15.9m, possible rubble band, ~100mm								
34.0	17	At 16.3m, coal band								
33.0	18	At 16.6m, possible rubble band, ~100mm								
32.0	18	COAL - Black coal								
31.0	19									
30.0	19.7	ROD FALL								
29.0	20									
28.0	21.2	RUBBLE								
27.0	21.6	ROCK								
26.0	22									
25.0	23.4	Bore discontinued at 23.4m, limit of investigation								
24.0	24									
23.0	25									
22.0	26									
21.0	27									
20.0	28									
19.0	29									

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-4m  
**TYPE OF BORING:** Solid flight auger to 4m, rotary air from 4m to 22.4m  
**WATER OBSERVATIONS:** Free groundwater observed at 11.9m, 16/3/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND			
A	Auger sample	pp	Pocket penetrometer (kPa)
D	Disturbed sample	PID	Photo ionisation detector
B	Bulk sample	S	Standard penetration test
U	Tube sample (x mm dia.)	PL	Point load strength Is(50) MPa
W	Water sample	V	Shear Vane (kPa)
C	Core drilling	▷	Water seep
		▽	Water level

CHECKED
Initials:
Date:


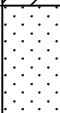

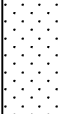








# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.7  
**EASTING:** 335631.2  
**NORTHING:** 1381955  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 112  
**PROJECT No:** 39129.03  
**DATE:** 17 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54.0	1	SAND / CLAY / WEATHERED SANDSTONE								
50.0	4.5	SANDSTONE - Highly weathered, orange-brown								
48.0	6.7	RUBBLE								
47.0	7.0	SANDSTONE - Highly weathered, orange-brown From 7.2m, whitish bands								
45.0	9.7	SANDSTONE - Grey (fresh, medium to high strength)								
43.0	12.1	RUBBLE								
42.0	12.6	SANDSTONE - Grey								
40.0	14.9	ROD FALL								
38.0	17.2	ROD FALL / LOOSE RUBBLE								
36.0	19.5	RUBBLE								
34.0	20.4	ROCK								
32.0	22.0	Bore discontinued at 22.0m, limit of investigation								

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-4m  
**TYPE OF BORING:** Solid flight auger to 4m, rotary air from 4m to 22m  
**WATER OBSERVATIONS:** Free groundwater observed at 11.8m, 16/3/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client, CCTV to 17.2m, top of rubble

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	> Water seep      ▽ Water level

CHECKED
Initials:
Date:






# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.72  
**EASTING:** 335634.1  
**NORTHING:** 1381947.5  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 114  
**PROJECT No:** 39129.03  
**DATE:** 17 Feb 10  
**SHEET** 1 OF 1

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54 53 52 51 50 49 48 47 46 45 44 43 42 41 40 39 38 37 36 35 34 33 32 31 30 29	1 2 3 4 4.5 5 6 7 8 9 9.7 10 11 12 13 14 14.5 14.9 15 16 17 18 19 19.8 20 21 21.5 22 23 24 25 26 27 28 29	<p>SAND / CLAY / WEATHERED SANDSTONE</p> <p>SANDSTONE - Highly weathered, orange-brown</p> <p>SANDSTONE - Grey (fresh, medium to high strength)</p> <p>At 12.5m to 12.8m, soft band</p> <p>At 13.3m, possible rubble band, ~100mm</p> <p>At 13.45m, possible rubble band, ~50mm</p> <p>RUBBLE</p> <p>COAL - Black coal</p> <p>SANDSTONE - Grey</p> <p>Bore discontinued at 21.5m, limit of investigation</p>						1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29		

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-4m

**TYPE OF BORING:** Solid flight auger to 4m, rotary air from 4m to 21.5m

**WATER OBSERVATIONS:**

**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND			
A	Auger sample	pp	Pocket penetrometer (kPa)
D	Disturbed sample	PID	Photo ionisation detector
B	Bulk sample	S	Standard penetration test
U	Tube sample (x mm dia.)	PL	Point load strength Is(50) MPa
W	Water sample	V	Shear Vane (kPa)
C	Core drilling	>	Water seep
		≡	Water level

CHECKED
Initials:
Date:



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# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.66  
**EASTING:** 335635.6  
**NORTHING:** 1381943.8  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 115  
**PROJECT No:** 39129.03  
**DATE:** 17 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54.66	1	FILLING / COAL CHITTER - Black / dark brown-grey	[Cross-hatch pattern]							
52.16	2.5	SAND / CLAY / WEATHERED SANDSTONE	[Diagonal lines]							
45.16	4.5	SANDSTONE - Highly weathered, orange-brown	[Dotted pattern]							
35.16	9.0	SANDSTONE - Grey (fresh, medium to high strength)	[Dotted pattern]							
33.16	13.1	RUBBLE	[Circular pattern]							
33.8	13.8	ROD FALL								
31.8	14.8	RUBBLE	[Circular pattern]							
35.16	19.2	ROCK	[Wavy pattern]							
20.4	20.4	Bore discontinued at 20.4m, limit of investigation								

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-4m  
**TYPE OF BORING:** Solid flight auger to 4m, rotary air from 4m to 20.4m  
**WATER OBSERVATIONS:** Free groundwater observed at 11.8m, 16/3/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	▷ Water seep      ▽ Water level

CHECKED
Initials:
Date:



# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.67  
**EASTING:** 335637  
**NORTHING:** 1381940.1  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 116  
**PROJECT No:** 39129.03  
**DATE:** 18 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54.67	1	FILLING / COAL CHITTER - Black / dark grey-brown								
52.4	2.4	SAND / CLAY / WEATHERED SANDSTONE								
4.5	4.5	SANDSTONE - Highly weathered, orange-brown								
9.0	9.0	SANDSTONE - Grey (fresh, medium to high strength)								
10.55	10.55	RUBBLE								
11.1	11.1	SANDSTONE - Grey								
12	12	At 11.6m, possible rubble band, ~100mm								
12.8	12.8	RUBBLE								
13.0	13.0	SANDSTONE - Grey								
14.2	14.2	At 13.3m, possible rubble band, ~100mm								
15	15	At 13.8m, coal band								
		COAL - Black coal								
16.9	16.9	ROD FALL								
18.6	18.6	RUBBLE								
19.1	19.1	ROCK								
20.4	20.4	Bore discontinued at 20.4m, limit of investigation								

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-6m  
**TYPE OF BORING:** Solid flight auger to 6m, rotary air from 6m to 20.4m  
**WATER OBSERVATIONS:** Free groundwater observed at 11.8m, 16/3/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	▷ Water seep      ▽ Water level

CHECKED
Initials:
Date:



# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.7  
**EASTING:** 335638.4  
**NORTHING:** 1381936.3  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 117  
**PROJECT No:** 39129.03  
**DATE:** 18 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
64.0	1	FILLING / COAL CHITTER - Black / dark grey								
53.0	2									
52.0	3									
51.0	4	SAND / CLAY / WEATHERED SANDSTONE								
50.0	5									
49.0	6									
48.0	6.3	SANDSTONE - Highly weathered, orange-brown								
47.0	7									
46.0	8	From 7.9m, whitish bands								
45.0	9									
44.0	10									
43.0	11									
42.0	12	From 12m, possible rubble band, ~50mm					▼			
41.0	13.1	ROD FALL								
40.0	14									
39.0	14.9	ROD FALL / LOOSE RUBBLE								
38.0	15									
37.0	16.1	RUBBLE								
36.0	16.3									
35.0	17									
34.0	18									
33.0	18.4	SANDSTONE - Grey								
32.0	19									
31.0	20.0	Bore discontinued at 20.0m, limit of investigation								
30.0	21									
29.0	22									
28.0	23									
27.0	24									
26.0	25									
25.0	26									
24.0	27									
23.0	28									
22.0	29									

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-6m  
**TYPE OF BORING:** Solid flight auger to 6m, rotary air from 6m to 20m  
**WATER OBSERVATIONS:** Free groundwater observed at 11.8m, 16/3/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client. CCTV to 14.9m, top of rubble

SAMPLING & IN SITU TESTING LEGEND			
A	Auger sample	pp	Pocket penetrometer (kPa)
D	Disturbed sample	PID	Photo ionisation detector
B	Bulk sample	S	Standard penetration test
U	Tube sample (x mm dia.)	PL	Point load strength Is(50) MPa
W	Water sample	V	Shear Vane (kPa)
C	Core drilling	▷	Water seep
		▽	Water level

CHECKED
Initials:
Date:



# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.75  
**EASTING:** 335639.7  
**NORTHING:** 1381932.5  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 118  
**PROJECT No:** 39129.03  
**DATE:** 18 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54.1	1	FILLING / COAL CHITTER - Black / dark grey-brown	[Cross-hatch pattern]							
52.5	2.5	SAND / CLAY / WEATHERED SANDSTONE	[Diagonal lines pattern]							
48.3	6.3	SANDSTONE - Highly weathered, orange-brown	[Dotted pattern]							
45.0	9.0	SANDSTONE - Grey (fresh, medium to high strength)	[Dotted pattern]							
42.1	12.1	RUBBLE	[Irregular shapes pattern]				▼			
41.7	13.7	COAL - Black coal	[Solid black pattern]							
37.2	17.2	ROCK	[Wavy lines pattern]							
18.5	18.5	SANDSTONE - Grey	[Dotted pattern]							
19.1	19.1	Bore discontinued at 19.1m, limit of investigation								
20	20									
21	21									
22	22									
23	23									
24	24									
25	25									
26	26									
27	27									
28	28									
29	29									

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-4.5m

**TYPE OF BORING:** Solid flight auger to 6m, rotary air from 6m to 19.1m

**WATER OBSERVATIONS:** Free groundwater observed at 11.9m, 16/3/10

**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND			
A	Auger sample	pp	Pocket penetrometer (kPa)
D	Disturbed sample	PID	Photo ionisation detector
B	Bulk sample	S	Standard penetration test
U	Tube sample (x mm dia.)	PL	Point load strength Is(50) MPa
W	Water sample	V	Shear Vane (kPa)
C	Core drilling	▷	Water seep
		≡	Water level

CHECKED
Initials:
Date:





# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.76  
**EASTING:** 335641.3  
**NORTHING:** 1381924.7  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 120  
**PROJECT No:** 39129.03  
**DATE:** 19 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54.76	1.0	FILLING / COAL CHITTER - Black / dark grey	[Cross-hatch pattern]							
53.76	2.0	SAND / CLAY / WEATHERED SANDSTONE	[Diagonal lines pattern]							
52.76	3.0		[Diagonal lines pattern]							
51.76	4.0		[Diagonal lines pattern]							
50.76	5.0	SANDSTONE - Highly weathered, orange-brown	[Dotted pattern]							
49.76	6.0		[Dotted pattern]							
48.76	7.0	from 7m, whitish bands	[Dotted pattern]							
47.76	8.0		[Dotted pattern]							
46.76	9.0		[Dotted pattern]							
45.76	10.0	SANDSTONE - Grey (fresh, medium to high strength)	[Dotted pattern]							
44.76	11.0		[Dotted pattern]							
43.76	12.0		[Dotted pattern]				▼			
42.76	13.1	At 12.5m, coal band, ~100mm	[Dotted pattern]							
41.76	13.9	ROD FALL	[Dotted pattern]							
40.76	14.0	RUBBLE - Loose	[Circular pattern]							
39.76	15.0		[Circular pattern]							
38.76	16.0		[Circular pattern]							
37.76	16.85	ROCK - Firm, possibly coal	[Wavy pattern]							
37.16	17.1	ROCK	[Wavy pattern]							
36.76	18.4	Bore discontinued at 18.4m, limit of investigation	[Wavy pattern]							
35.76	19.0									
34.76	20.0									
33.76	21.0									
32.76	22.0									
31.76	23.0									
30.76	24.0									
29.76	25.0									
28.76	26.0									
27.76	27.0									
26.76	28.0									
25.76	29.0									

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-3m  
**TYPE OF BORING:** Solid flight auger to 4m, rotary air from 4m to 18.4m  
**WATER OBSERVATIONS:** Free groundwater observed at 11.9m, 16/3/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	▷ Water seep      ▽ Water level

CHECKED
Initials:
Date:



# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 53.34  
**EASTING:** 335730.1  
**NORTHING:** 1382263  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 200  
**PROJECT No:** 39129.03  
**DATE:** 23 Feb 10  
**SHEET** 1 OF 1

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
53.00	1	SAND / CLAY / WEATHERED SANDSTONE								
52.00	2.0	SANDSTONE - Highly weathered, orange-brown								
51.00										
50.00										
49.00										
48.00										
47.00										
46.00										
45.00										
44.00										
43.00										
42.00										
41.00										
40.00										
39.00										
38.00										
37.00										
36.00										
35.00										
34.00										
33.00										
32.00										
31.00										
30.00										
29.00										
28.00										
27.00										
26.00										
25.00										
24.00										
23.00										
22.00										
22.1	22.1	ROD FALL								
22.6	22.6	RUBBLE - Loose								
25.2	25.2	ROCK								
29.45	29.45	Bore discontinued at 29.45m, limit of investigation								

**RIG:** Bombadier      **DRILLER:** Total (Curry)      **LOGGED:** Bear      **CASING:** 100mm diameter PVC-2.8m  
**TYPE OF BORING:** Solid flight auger to 3m, rotary air from 3m to 29.45m  
**WATER OBSERVATIONS:** Free groundwater observed at 10.5m, 24/2/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) interpolated from survey

SAMPLING & IN SITU TESTING LEGEND	
A	Auger sample
D	Disturbed sample
B	Bulk sample
U	Tube sample (x mm dia.)
W	Water sample
C	Core drilling
pp	Pocket penetrometer (kPa)
PID	Photo ionisation detector
S	Standard penetration test
PL	Point load strength Is(50) MPa
V	Shear Vane (kPa)
▷	Water seep
▽	Water level

CHECKED
Initials:
Date:







# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 53  
**EASTING:** 335733  
**NORTHING:** 1382250.9  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 203  
**PROJECT No:** 39129.03  
**DATE:** 05 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
53.0	1	SAND / CLAY / WEATHERED SANDSTONE								
52.0	2.0	SANDSTONE - Highly weathered, orange-brown								
51.0	3									
50.0	4									
49.0	5									
48.0	6									
47.0	7									
46.0	8.0	SANDSTONE - Grey (fresh, medium to high strength)								
45.0	9									
44.0	10						▼			
43.0	11									
42.0	12									
41.0	13									
40.0	14									
39.0	15									
38.0	16									
37.0	16.9	ROD FALL								
36.0	18									
35.0	18.4	RUBBLE								
34.0	19									
33.0	19.2	ROCK								
32.0	20									
31.0	20.4	Bore discontinued at 20.4m, limit of investigation								
30.0	21									
29.0	22									
28.0	23									
27.0	24									
26.0	25									
25.0	26									
24.0	27									
23.0	28									
22.0	29									

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-2m  
**TYPE OF BORING:** Solid flight auger to 2m, rotary air from 2m to 20.4m  
**WATER OBSERVATIONS:** Free groundwater observed at 10.1m, 24/2/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	▷ Water seep      ▽ Water level

CHECKED
Initials:
Date:



# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 52.92  
**EASTING:** 335733.3  
**NORTHING:** 1382248.9  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 203A  
**PROJECT No:** 39129.03  
**DATE:** 23 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
52.92	1	SAND / CLAY / WEATHERED SANDSTONE	[Diagonal Hatching]							
52.00	2.0	SANDSTONE - Highly weathered, orange-brown	[Dotted Pattern]							
48.00	4	From 4.0m, whitish bands	[Dotted Pattern]							
44.00	8.0	SANDSTONE - Grey (fresh, medium to high strength)	[Dotted Pattern]							
40.00	10		[Dotted Pattern]				▼			
37.00	15.6	RUBBLE	[Wavy Pattern]							
36.10	16.1	SANDSTONE - Grey (Fresh, medium to high strength)	[Dotted Pattern]							
35.00	17.0	ROD FALL	[Wavy Pattern]							
34.00	17.2	RUBBLE	[Wavy Pattern]							
33.00	18	ROCK	[Wavy Pattern]							
32.00	18.75	RUBBLE	[Wavy Pattern]							
31.00	19.0	SOLID ROCK	[Wavy Pattern]							
30.00	22.35	RUBBLE	[Wavy Pattern]							
29.00	22.9	ROD FALL	[Wavy Pattern]							
28.00	23.2	RUBBLE	[Wavy Pattern]							
27.00	24.4	ROCK	[Wavy Pattern]							
24.45	29.45	Bore discontinued at 29.45m, limit of investigation								

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-2.85m  
**TYPE OF BORING:** Solid flight auger to 3m, rotary air from 3m to 29.45m  
**WATER OBSERVATIONS:** Free groundwater observed at 10.0m, 24/2/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) interpolated from survey. CCTV to 22.65m, blocked hole

SAMPLING & IN SITU TESTING LEGEND			
A	Auger sample	pp	Pocket penetrometer (kPa)
D	Disturbed sample	PID	Photo ionisation detector
B	Bulk sample	S	Standard penetration test
U	Tube sample (x mm dia.)	PL	Point load strength Is(50) MPa
W	Water sample	V	Shear Vane (kPa)
C	Core drilling	▷	Water seep      ▽ Water level

CHECKED
Initials:
Date:



# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 52.85  
**EASTING:** 335733.7  
**NORTHING:** 1382246.8  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 204  
**PROJECT No:** 39129.03  
**DATE:** 04 Feb 10  
**SHEET** 1 OF 1

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
52.0	1	SAND / CLAY / WEATHERED SANDSTONE	[Diagonal hatching]							
50.0	2.0	SANDSTONE - Highly weathered, orange-brown	[Dotted pattern]							
48.0	3									
46.0	4									
44.0	5									
42.0	6									
40.0	7									
38.0	8									
36.0	8.5	SANDSTONE - Grey (fresh, medium to high strength)	[Dotted pattern]							
34.0	9									
32.0	10						▼			
30.0	11									
28.0	12									
26.0	13									
24.0	14									
22.0	15.3	ROD FALL								
20.0	16									
18.0	16.9	RUBBLE	[Wavy pattern]							
17.3	17.3	ROCK	[Wavy pattern]							
18.4	18.4	Bore discontinued at 18.4m, limit of investigation								
19	19									
20	20									
21	21									
22	22									
23	23									
24	24									
25	25									
26	26									
27	27									
28	28									
29	29									

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-2m  
**TYPE OF BORING:** Solid flight auger to 2m, rotary air from 2m to 18.4m  
**WATER OBSERVATIONS:** Free groundwater observed at 10.0m, 24/2/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	> Water seep      ▽ Water level

CHECKED
Initials:
Date:


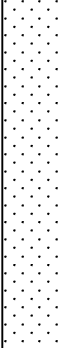









# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 52.7  
**EASTING:** 335734.5  
**NORTHING:** 1382243.3  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 205  
**PROJECT No:** 39129.03  
**DATE:** 05 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
62	1	SAND / CLAY / WEATHERED SANDSTONE								
54	1.5	SANDSTONE - Highly weathered, orange-brown								
50	2									
48	3									
46	4									
45	5									
44	6									
43	7									
42	8									
41	8.5	SANDSTONE - Grey (fresh, medium to high strength)								
40	9									
39	10									
38	11									
37	12									
36	13									
35	14									
34	15									
33	16									
32	16.2	COAL - Black coal								
31	16.5									
30	17	ROD FALL								
29	17.5									
28	17.6	RUBBLE								
27	18									
26	18.4	ROCK								
25	18.7									
24	18.7	RUBBLE								
23	19									
22	19.8	ROCK								
21	19.8	Bore discontinued at 19.8m, limit of investigation								
20										
19										
18										
17										
16										
15										
14										
13										
12										
11										
10										
9										
8										
7										
6										
5										
4										
3										
2										
1										

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-1.5m  
**TYPE OF BORING:** Solid flight auger to 1.5m, rotary air from 1.5m to 19.8m  
**WATER OBSERVATIONS:** Free groundwater observed at 9.8m, 24/2/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND			
A	Auger sample	pp	Pocket penetrometer (kPa)
D	Disturbed sample	PID	Photo ionisation detector
B	Bulk sample	S	Standard penetration test
U	Tube sample (x mm dia.)	PL	Point load strength Is(50) MPa
W	Water sample	V	Shear Vane (kPa)
C	Core drilling	▷	Water seep
		▽	Water level

CHECKED
Initials:
Date:





# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 52.5  
**EASTING:** 335736.1  
**NORTHING:** 1382235.1  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 207  
**PROJECT No:** 39129.03  
**DATE:** 24 Feb 10  
**SHEET** 1 OF 1

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
52.5	1	SAND / CLAY / WEATHERED SANDSTONE	[Diagonal hatching]							
50.0	2.0	SANDSTONE - Highly weathered, orange-brown	[Dotted pattern]							
43.5	9.0	SANDSTONE - Grey (fresh, medium to high strength)	[Dotted pattern]				▼			
36.5	16.5	RUBBLE / FRACTURED COAL	[Circular pattern]							
34.0	17.8	ROCK	[Wavy pattern]							
33.0	18.8	Bore discontinued at 18.8m, limit of investigation								
29.0	29									

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-2m  
**TYPE OF BORING:** Solid flight auger to 2m, rotary air from 2m to 18.8m  
**WATER OBSERVATIONS:** Free groundwater observed at 9.6m, 24/2/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND			
A Auger sample	pp Pocket penetrometer (kPa)	S Standard penetration test	
D Disturbed sample	PID Photo ionisation detector	PL Point load strength Is(50) MPa	
B Bulk sample	S Shear Vane (kPa)	Water seep	Water level
U Tube sample (x mm dia.)			
W Water sample			
C Core drilling			

CHECKED
Initials:
Date:







# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 52.59  
**EASTING:** 335737.4  
**NORTHING:** 1382227.2  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 209  
**PROJECT No:** 39129.03  
**DATE:** 09 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
52.59	1	SAND / CLAY / WEATHERED SANDSTONE								
60	2.1	SANDSTONE - Highly weathered, orange-brown								
60	3									
48	4									
47	5									
46	6									
45	7									
44	7.9	SANDSTONE - Highly weathered, orange-brown, soil-like cuttings								
44	8.6									
43	9.15	SANDSTONE - Light grey (slightly weathered to moderately weathered, medium strength)					▼			
43	10									
42	10.75	SANDSTONE - Highly weathered, orange-brown, soil-like cuttings								
42	11									
41	11.0	SANDSTONE - Light grey (slightly weathered to moderately weathered, medium strength)								
41	12									
40	13	SANDSTONE - Grey (fresh, medium to high strength)								
39	14									
38	15									
37	16									
36	16.2	RUBBLE								
36	17									
35	16.95	From 16.2, pebbly band								
35	17.5									
34	18	SANDSTONE - Grey								
34	19	ROD FALL								
33	18.9	ROD FALL / LOOSE RUBBLE								
33	20									
32	19.9	RUBBLE								
32	21									
31	21.5	ROCK								
31	22									
30	23	Bore discontinued at 22.85m, limit of investigation								
30	24									
29	25									
28	26									
27	27									
26	28									
25	29									

**RIG:** Bombadier                                      **DRILLER:** Total (Curry)                                      **LOGGED:** Bear                                      **CASING:** 100mm diameter PVC-2m  
**TYPE OF BORING:** Solid flight auger to 2m, rotary air from 2m to 22.85m  
**WATER OBSERVATIONS:** Free groundwater observed at 9.7m, 24/2/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client. CCTV to 18.9m, top of rubble

SAMPLING & IN SITU TESTING LEGEND			
A	Auger sample	pp	Pocket penetrometer (kPa)
D	Disturbed sample	PID	Photo ionisation detector
B	Bulk sample	S	Standard penetration test
U	Tube sample (x mm dia.)	PL	Point load strength Is(50) MPa
W	Water sample	V	Shear Vane (kPa)
C	Core drilling	▷	Water seep
		▽	Water level

CHECKED
Initials:
Date:





# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 52.54  
**EASTING:** 335738.9  
**NORTHING:** 1382219.4  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 211  
**PROJECT No:** 39129.03  
**DATE:** 10 Feb 10  
**SHEET** 1 OF 1

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
52.00	1	SAND / CLAY / WEATHERED SANDSTONE	[Diagonal Hatching]							
50.00	2.0	SANDSTONE - Highly weathered, orange-brown	[Dotted Pattern]							
48.00	4	From 4.0m, whitish bands	[Dotted Pattern]							
46.00	7.85	ROD FALL	[Dotted Pattern]							
44.00	9.6	RUBBLE: Loose rubble with solid rock up to 0.75m thick	[Circular Pattern]				▼			
42.00	21.25	ROCK	[Wavy Pattern]							
40.00	22.85	Bore discontinued at 22.85m, limit of investigation								

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-2m  
**TYPE OF BORING:** Solid flight auger to 2m, rotary air from 2m to 22.85m  
**WATER OBSERVATIONS:** Free groundwater observed at 9.6m, 16/3/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client. CCTV to 10.6m, blocked hole

SAMPLING & IN SITU TESTING LEGEND			
A	Auger sample	pp	Pocket penetrometer (kPa)
D	Disturbed sample	PID	Photo ionisation detector
B	Bulk sample	S	Standard penetration test
U	Tube sample (x mm dia.)	PL	Point load strength Is(50) MPa
W	Water sample	V	Shear Vane (kPa)
C	Core drilling	▷	Water seep
		≡	Water level

CHECKED
Initials:
Date:



# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 52.44  
**EASTING:** 335739.8  
**NORTHING:** 1382215.2  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 212  
**PROJECT No:** 39129.03  
**DATE:** 10 Feb 10  
**SHEET** 1 OF 1

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
	1	SAND / CLAY / WEATHERED SANDSTONE	[Diagonal lines pattern]							
	1.5	SANDSTONE - Highly weathered, orange-brown	[Dotted pattern]							
	2									
	3									
	4									
	5									
	6									
	7									
	8									
	9	SANDSTONE - Grey (fresh, medium to high strength)	[Dotted pattern]				▼			
	9.0									
	10									
	11									
	12									
	13	RUBBLE: Loose rubble with voids up to 0.35m high and solid rock up to 0.65m thick	[Circular void pattern]							
	13.1									
	14									
	15									
	16									
	17									
	18									
	19									
	20									
	20.25	ROCK	[Wavy pattern]							
	21									
	21.9	Bore discontinued at 21.9m, limit of investigation								
	22									
	23									
	24									
	25									
	26									
	27									
	28									
	29									

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-2m  
**TYPE OF BORING:** Solid flight auger to 2m, rotary air from 2m to 21.9m  
**WATER OBSERVATIONS:** Free groundwater observed at 9.6m, 24/2/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client. CCTV to 14.1m, camera jamming

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	▷ Water seep      ▽ Water level

CHECKED
Initials:
Date:







# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 51.56  
**EASTING:** 335739.3  
**NORTHING:** 1382202  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 215  
**PROJECT No:** 39129.03  
**DATE:** 11 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
51	1.0	SAND / CLAY / WEATHERED SANDSTONE								
50		SANDSTONE - Highly weathered, orange-brown								
49										
48										
47										
46										
45										
44										
43	7.85	RUBBLE								
42	8.0	SANDSTONE - Highly weathered, orange-brown								
41										
40										
39										
38										
37										
36										
35										
34										
33										
32										
31										
30										
29										
28										
27										
26										
25										
24										
23										
22										
21										
20	20.0	Bore discontinued at 20.0m, limit of investigation								

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-1.5m  
**TYPE OF BORING:** Solid flight auger to 1.5m, rotary air from 1.5m to 20m  
**WATER OBSERVATIONS:** Free groundwater observed at 8.7m, 24/2/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND			
A	Auger sample	pp	Pocket penetrometer (kPa)
D	Disturbed sample	PID	Photo ionisation detector
B	Bulk sample	S	Standard penetration test
U	Tube sample (x mm dia.)	PL	Point load strength Is(50) MPa
W	Water sample	V	Shear Vane (kPa)
C	Core drilling	▷	Water seep
		▽	Water level

CHECKED
Initials:
Date:





# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 51.11  
**EASTING:** 335738.9  
**NORTHING:** 1382195.7  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 217  
**PROJECT No:** 39129.03  
**DATE:** 11 Feb 10  
**SHEET** 1 OF 1

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
51.11	0.8	SAND / CLAY / WEATHERED SANDSTONE								
50.8	1	SANDSTONE - Highly weathered, orange-brown								
49.8	2									
48.8	3									
47.8	4									
46.8	5									
45.8	6									
44.8	7									
43.8	8						▼			
42.8	8.9	SANDSTONE - Grey (fresh, medium to high strength)								
41.8	10									
40.8	11									
39.8	12									
38.8	12.7	COAL - Black coal								
37.8	14	From 13.7m, fractured								
36.8	14.4	ROD FALL								
35.8	15									
34.8	16.5	RUBBLE								
33.8	17									
32.8	17.65	ROCK								
31.8	18									
30.8	19.5	Bore discontinued at 19.5m, limit of investigation								
29.8	20									
28.8	21									
27.8	22									
26.8	23									
25.8	24									
24.8	25									
23.8	26									
22.8	27									
21.8	28									
20.8	29									

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-1.4m  
**TYPE OF BORING:** Solid flight auger to 1.4m, rotary air from 1.4m to 19.5m  
**WATER OBSERVATIONS:** Free groundwater observed at 8.1m, 24/2/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	▷ Water seep      ▽ Water level

CHECKED
Initials:
Date:





# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 52.1  
**EASTING:** 335750  
**NORTHING:** 1382216  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 219  
**PROJECT No:** 39129.03  
**DATE:** 24 Feb 10  
**SHEET** 1 OF 1

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
52.0	1	SAND / CLAY / WEATHERED SANDSTONE	[Diagonal Hatching]							
50.0	2.0	SANDSTONE - Highly weathered, orange-brown	[Dotted Pattern]							
48.0	3									
46.0	4									
44.0	5									
42.0	6									
40.0	7									
38.0	8									
36.0	9						▼			
34.0	10.0	SANDSTONE - Grey (fresh, medium to high strength)	[Dotted Pattern]							
32.0	11									
30.0	12									
28.0	13									
26.0	13.8	ROD FALL								
24.0	15									
22.0	16									
20.0	17									
18.0	18.1	RUBBLE	[Circular Pattern]							
16.0	19									
14.0	19.6	ROCK	[Wavy Pattern]							
12.0	20									
10.0	21									
8.0	22									
6.0	23									
4.0	23.45	Bore discontinued at 23.45m, limit of investigation								
2.0	24									
0.0	25									
	26									
	27									
	28									
	29									

**RIG:** Bombadier                                      **DRILLER:** Total (Curry)                                      **LOGGED:** Bear                                      **CASING:** 100mm diameter PVC-1.5m

**TYPE OF BORING:** Solid flight auger to 3m, rotary air from 3m to 23.45m

**WATER OBSERVATIONS:** Free groundwater observed at 9.5m, 24/2/10

**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) determined off plans

SAMPLING & IN SITU TESTING LEGEND			
A Auger sample	pp Pocket penetrometer (kPa)		
D Disturbed sample	PID Photo ionisation detector		
B Bulk sample	S Standard penetration test		
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa		
W Water sample	V Shear Vane (kPa)		
C Core drilling	▷ Water seep      ▽ Water level		

CHECKED
Initials:
Date:





# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.63  
**EASTING:** 335787.9  
**NORTHING:** 1382354.4  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 301  
**PROJECT No:** 39129.03  
**DATE:** 11 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54.63 54.62 54.61 54.60 54.59 54.58 54.57 54.56 54.55 54.54 54.53 54.52 54.51 54.50 54.49 54.48 54.47 54.46 54.45 54.44 54.43 54.42 54.41 54.40 54.39 54.38 54.37 54.36 54.35 54.34 54.33 54.32 54.31 54.30 54.29 54.28 54.27 54.26 54.25 54.24 54.23 54.22 54.21 54.20 54.19 54.18 54.17 54.16 54.15 54.14 54.13 54.12 54.11 54.10 54.09 54.08 54.07 54.06 54.05 54.04 54.03 54.02 54.01 54.00 53.99 53.98 53.97 53.96 53.95 53.94 53.93 53.92 53.91 53.90 53.89 53.88 53.87 53.86 53.85 53.84 53.83 53.82 53.81 53.80 53.79 53.78 53.77 53.76 53.75 53.74 53.73 53.72 53.71 53.70 53.69 53.68 53.67 53.66 53.65 53.64 53.63 53.62 53.61 53.60 53.59 53.58 53.57 53.56 53.55 53.54 53.53 53.52 53.51 53.50 53.49 53.48 53.47 53.46 53.45 53.44 53.43 53.42 53.41 53.40 53.39 53.38 53.37 53.36 53.35 53.34 53.33 53.32 53.31 53.30 53.29 53.28 53.27 53.26 53.25 53.24 53.23 53.22 53.21 53.20 53.19 53.18 53.17 53.16 53.15 53.14 53.13 53.12 53.11 53.10 53.09 53.08 53.07 53.06 53.05 53.04 53.03 53.02 53.01 53.00 52.99 52.98 52.97 52.96 52.95 52.94 52.93 52.92 52.91 52.90 52.89 52.88 52.87 52.86 52.85 52.84 52.83 52.82 52.81 52.80 52.79 52.78 52.77 52.76 52.75 52.74 52.73 52.72 52.71 52.70 52.69 52.68 52.67 52.66 52.65 52.64 52.63 52.62 52.61 52.60 52.59 52.58 52.57 52.56 52.55 52.54 52.53 52.52 52.51 52.50 52.49 52.48 52.47 52.46 52.45 52.44 52.43 52.42 52.41 52.40 52.39 52.38 52.37 52.36 52.35 52.34 52.33 52.32 52.31 52.30 52.29 52.28 52.27 52.26 52.25 52.24 52.23 52.22 52.21 52.20 52.19 52.18 52.17 52.16 52.15 52.14 52.13 52.12 52.11 52.10 52.09 52.08 52.07 52.06 52.05 52.04 52.03 52.02 52.01 52.00 51.99 51.98 51.97 51.96 51.95 51.94 51.93 51.92 51.91 51.90 51.89 51.88 51.87 51.86 51.85 51.84 51.83 51.82 51.81 51.80 51.79 51.78 51.77 51.76 51.75 51.74 51.73 51.72 51.71 51.70 51.69 51.68 51.67 51.66 51.65 51.64 51.63 51.62 51.61 51.60 51.59 51.58 51.57 51.56 51.55 51.54 51.53 51.52 51.51 51.50 51.49 51.48 51.47 51.46 51.45 51.44 51.43 51.42 51.41 51.40 51.39 51.38 51.37 51.36 51.35 51.34 51.33 51.32 51.31 51.30 51.29 51.28 51.27 51.26 51.25 51.24 51.23 51.22 51.21 51.20 51.19 51.18 51.17 51.16 51.15 51.14 51.13 51.12 51.11 51.10 51.09 51.08 51.07 51.06 51.05 51.04 51.03 51.02 51.01 51.00 50.99 50.98 50.97 50.96 50.95 50.94 50.93 50.92 50.91 50.90 50.89 50.88 50.87 50.86 50.85 50.84 50.83 50.82 50.81 50.80 50.79 50.78 50.77 50.76 50.75 50.74 50.73 50.72 50.71 50.70 50.69 50.68 50.67 50.66 50.65 50.64 50.63 50.62 50.61 50.60 50.59 50.58 50.57 50.56 50.55 50.54 50.53 50.52 50.51 50.50 50.49 50.48 50.47 50.46 50.45 50.44 50.43 50.42 50.41 50.40 50.39 50.38 50.37 50.36 50.35 50.34 50.33 50.32 50.31 50.30 50.29 50.28 50.27 50.26 50.25 50.24 50.23 50.22 50.21 50.20 50.19 50.18 50.17 50.16 50.15 50.14 50.13 50.12 50.11 50.10 50.09 50.08 50.07 50.06 50.05 50.04 50.03 50.02 50.01 50.00 50.00	1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29	SAND / CLAY / WEATHERED SANDSTONE  SANDSTONE - Highly weathered, orange-brown  SANDSTONE - Grey (fresh, medium to high strength)  COAL - Black coal From 22m, fractured  From ~24m, cave-in jamming rods  Bore discontinued at 25.6m, cave-in jamming rods								

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-2m  
**TYPE OF BORING:** Solid flight auger to 2m, rotary air from 2m to 25.6m  
**WATER OBSERVATIONS:** Free groundwater observed at 11.7m, 24/2/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	> Water seep      ▽ Water level

CHECKED
Initials:
Date:



# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.55  
**EASTING:** 335789.2  
**NORTHING:** 1382350.6  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 302  
**PROJECT No:** 39129.03  
**DATE:** 19 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54.0	1.0	SAND / CLAY / WEATHERED SANDSTONE	[Diagonal Hatching]							
53.0		SANDSTONE - Highly weathered, orange-brown	[Dotted Pattern]							
52.0		From 2.5m, whitish bands	[Dotted Pattern]							
51.0			[Dotted Pattern]							
50.0			[Dotted Pattern]							
49.0			[Dotted Pattern]							
48.0			[Dotted Pattern]							
47.0			[Dotted Pattern]							
46.0			[Dotted Pattern]							
45.0			[Dotted Pattern]							
44.0	10.0	SANDSTONE - Grey (fresh, medium to high strength)	[Dotted Pattern]							
43.0			[Dotted Pattern]							
42.0			[Dotted Pattern]							
41.0			[Dotted Pattern]							
40.0			[Dotted Pattern]							
39.0			[Dotted Pattern]							
38.0			[Dotted Pattern]							
37.0			[Dotted Pattern]							
36.0			[Dotted Pattern]							
35.0			[Dotted Pattern]							
34.5	19.45	COAL - Black coal	[Solid Black]							
34.0	20.05	ROD FALL	[White]							
33.0			[White]							
32.0			[White]							
31.0			[White]							
30.0			[White]							
29.0			[White]							
28.6	22.6	ROD FALL / LOOSE RUBBLE	[Circular Pattern]							
28.0			[Circular Pattern]							
27.0			[Circular Pattern]							
26.1	25.9	RUBBLE	[Irregular Pattern]							
26.0	26.1	ROCK	[Wavy Pattern]							
25.0			[Wavy Pattern]							
24.0			[Wavy Pattern]							
23.0			[Wavy Pattern]							
22.0			[Wavy Pattern]							
21.0			[Wavy Pattern]							
20.0			[Wavy Pattern]							
19.0			[Wavy Pattern]							
18.0			[Wavy Pattern]							
17.0			[Wavy Pattern]							
16.0			[Wavy Pattern]							
15.0			[Wavy Pattern]							
14.0			[Wavy Pattern]							
13.0			[Wavy Pattern]							
12.0			[Wavy Pattern]							
11.7			[Wavy Pattern]				▼			
11.0			[Wavy Pattern]							
10.0			[Wavy Pattern]							
9.0			[Wavy Pattern]							
8.0			[Wavy Pattern]							
7.0			[Wavy Pattern]							
6.0			[Wavy Pattern]							
5.0			[Wavy Pattern]							
4.0			[Wavy Pattern]							
3.0			[Wavy Pattern]							
2.0			[Wavy Pattern]							
1.0			[Wavy Pattern]							
0.0			[Wavy Pattern]							
28.0	27.8	Bore discontinued at 27.8m, cave-in jamming rods	[White]							
27.0			[White]							
26.0			[White]							
25.0			[White]							
24.0			[White]							
23.0			[White]							
22.0			[White]							
21.0			[White]							
20.0			[White]							
19.0			[White]							
18.0			[White]							
17.0			[White]							
16.0			[White]							
15.0			[White]							
14.0			[White]							
13.0			[White]							
12.0			[White]							
11.0			[White]							
10.0			[White]							
9.0			[White]							
8.0			[White]							
7.0			[White]							
6.0			[White]							
5.0			[White]							
4.0			[White]							
3.0			[White]							
2.0			[White]							
1.0			[White]							
0.0			[White]							

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-2m  
**TYPE OF BORING:** Solid flight auger to 2m, rotary air from 2m to 27.8m  
**WATER OBSERVATIONS:** Free groundwater observed at 11.7m, 24/2/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client, CCTV to 22.6m, top of rubble

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	▷ Water seep      ▽ Water level

CHECKED
Initials:
Date:



# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.42  
**EASTING:** 335790.5  
**NORTHING:** 1382346.8  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 303  
**PROJECT No:** 39129.03  
**DATE:** 22 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54.42	1.0	SAND / CLAY / WEATHERED SANDSTONE	[Diagonal lines pattern]							
53.00		SANDSTONE - Highly weathered, orange-brown	[Dotted pattern]							
44.42	10.0	SANDSTONE - Grey (fresh, medium to high strength)	[Dotted pattern]					▼		
33.00	20.9	ROD FALL	[Blank]							
31.00	22.75	ROD FALL / LOOSE RUBBLE	[Circular pattern]							
25.70	25.4	RUBBLE	[Circular pattern]							
25.70	25.7	ROCK	[Wavy pattern]							
26.00	28.0	Bore discontinued at 28.0m, limit of investigation	[Blank]							

**RIG:** Bombadier                                      **DRILLER:** Total (Curry)                                      **LOGGED:** Bear                                      **CASING:** 100mm diameter PVC-2m  
**TYPE OF BORING:** Solid flight auger to 2m, rotary air from 2m to 28m  
**WATER OBSERVATIONS:** Free groundwater observed at 11.6m, 24/2/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client, CCTV to 22.75m, top of rubble

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	▷ Water seep      ▽ Water level

CHECKED
Initials:
Date:





# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.17  
**EASTING:** 335793.1  
**NORTHING:** 1382339.4  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 305  
**PROJECT No:** 39129.03  
**DATE:** 22 Feb 10  
**SHEET 1 OF 1**

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54.0	1.0	SAND / CLAY / WEATHERED SANDSTONE								
53.0		SANDSTONE - Highly weathered, orange-brown								
52.0										
51.0										
50.0										
49.0		From 5m, whitish bands								
48.0										
47.0										
46.0										
45.0										
44.0	10.0	SANDSTONE - Grey (fresh, medium to high strength)								
43.0										
42.0										
41.0										
40.0										
39.0										
38.0										
37.0										
36.0										
35.0	19.1	At 18.7m, coal band, ~100mm COAL - Black coal								
34.0										
33.0										
32.0										
31.0	23.1	ROD FALL								
30.0	24.0	ROD FALL / LOOSE RUBBLE								
29.0	24.6	RUBBLE								
28.0	25.1	ROCK								
27.0										
26.0										
25.0	28.5	Bore discontinued at 28.5m, limit of investigation								

**RIG:** Bombadier                      **DRILLER:** Total (Curry)                      **LOGGED:** Bear                      **CASING:** 100mm diameter PVC-2.6m  
**TYPE OF BORING:** Solid flight auger to 3m, rotary air from 3m to 28.5m  
**WATER OBSERVATIONS:** Free groundwater observed at 11.4m, 24/2/10  
**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client, CCTV to 24.3m, top of rubble

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	▷ Water seep      ▽ Water level

CHECKED
Initials:
Date:



# BOREHOLE LOG

**CLIENT:** Pacific National Pty Ltd  
**PROJECT:** Proposed Pacific National Depot  
**LOCATION:** Mansfield Street, Greta

**SURFACE LEVEL:** 54.2  
**EASTING:** 335792.6  
**NORTHING:** 1382335.4  
**DIP/AZIMUTH:** 90°/--

**BORE No:** 306  
**PROJECT No:** 39129.03  
**DATE:** 22 Feb 10  
**SHEET** 1 OF 1

RL	Depth (m)	Description of Strata	Graphic Log	Sampling & In Situ Testing				Water	Well Construction Details	
				Type	Depth	Sample	Results & Comments			
54	1.0	SAND / CLAY / WEATHERED SANDSTONE								
53		SANDSTONE - Highly weathered, orange-brown								
52										
51										
50										
49										
48										
47										
46		From 5.5m, whitish bands								
45										
44										
43	10.0	SANDSTONE - Grey (fresh, medium to high strength)								
42										
41										
40										
39										
38										
37										
36										
35										
34	20.0	COAL - Black coal								
33										
32										
31										
30										
29										
28	25.6	ROCK								
27										
26										
25										
24										
23										
22										
21										
20										
19										
18										
17										
16										
15										
14										
13										
12										
11										
10										
9										
8										
7										
6										
5										
4										
3										
2										
1										
0	28.0	Bore discontinued at 28.0m, limit of investigation								

**RIG:** Bombadier **DRILLER:** Total (Curry) **LOGGED:** Bear **CASING:** 100mm diameter PVC-2.6m

**TYPE OF BORING:** Solid flight auger to 3m, rotary air from 3m to 28m

**WATER OBSERVATIONS:**

**REMARKS:** Co-ordinates (ISG 56/1) and Surface Level (AHD) Surveyed by Client

SAMPLING & IN SITU TESTING LEGEND	
A Auger sample	pp Pocket penetrometer (kPa)
D Disturbed sample	PID Photo ionisation detector
B Bulk sample	S Standard penetration test
U Tube sample (x mm dia.)	PL Point load strength Is(50) MPa
W Water sample	V Shear Vane (kPa)
C Core drilling	▷ Water seep      ▽ Water level

CHECKED
Initials:
Date:



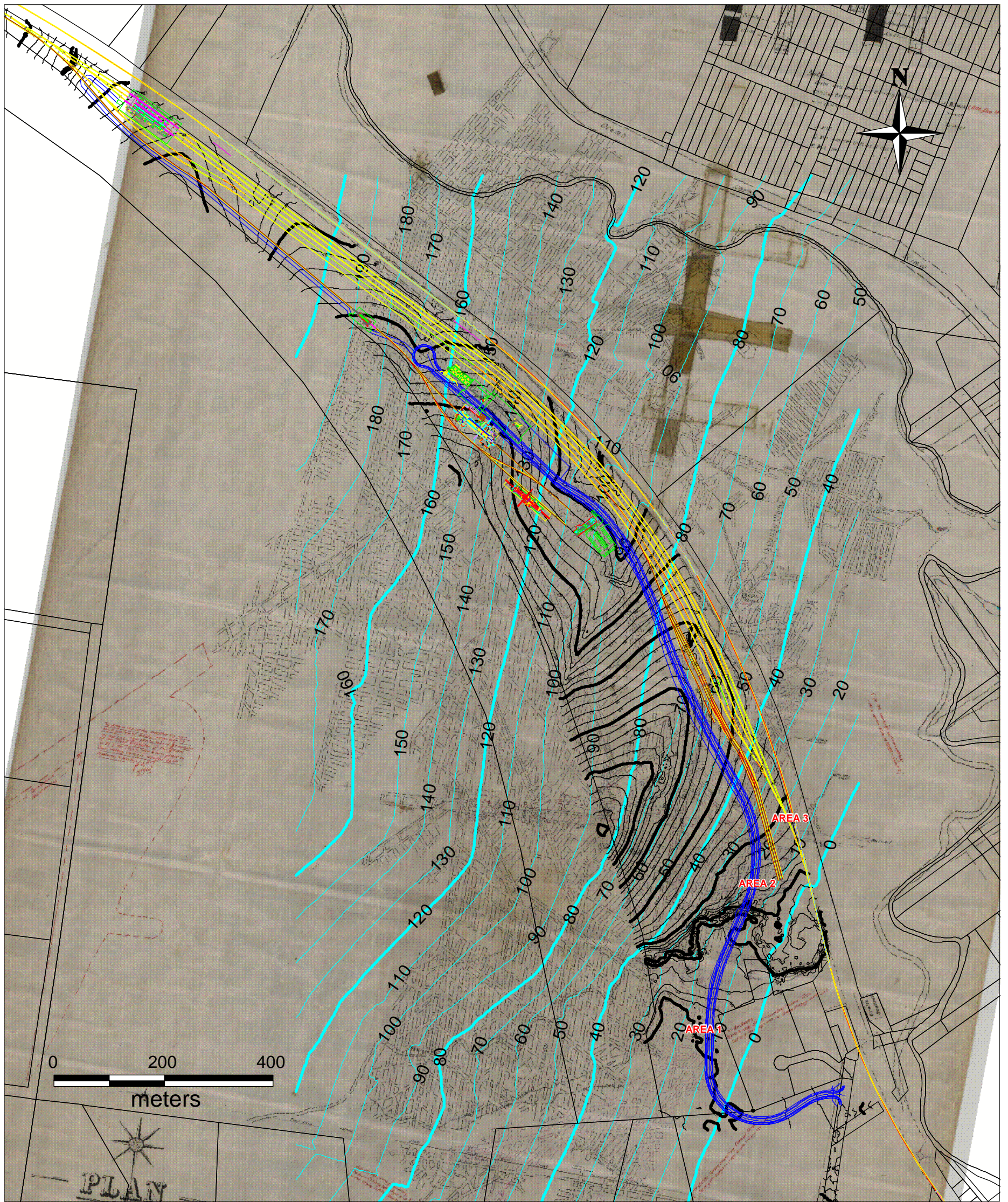
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**APPENDIX B**

***Drawing 1 – Site Plan***  
***Drawing 1A – Bore Locations Area 1***  
***Drawing 1B – Bore Locations Areas 2 and 3***  
***Drawings 2 to 5 – Sections Through Boreholes***

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TITLE: PACIFIC NATIONAL DEPOT  
 MANSFIELD STREET, GRETA  
 SITE PLAN

CLIENT: PACIFIC NATIONAL

DRAWN BY: JRB

SCALE: AS SHOWN

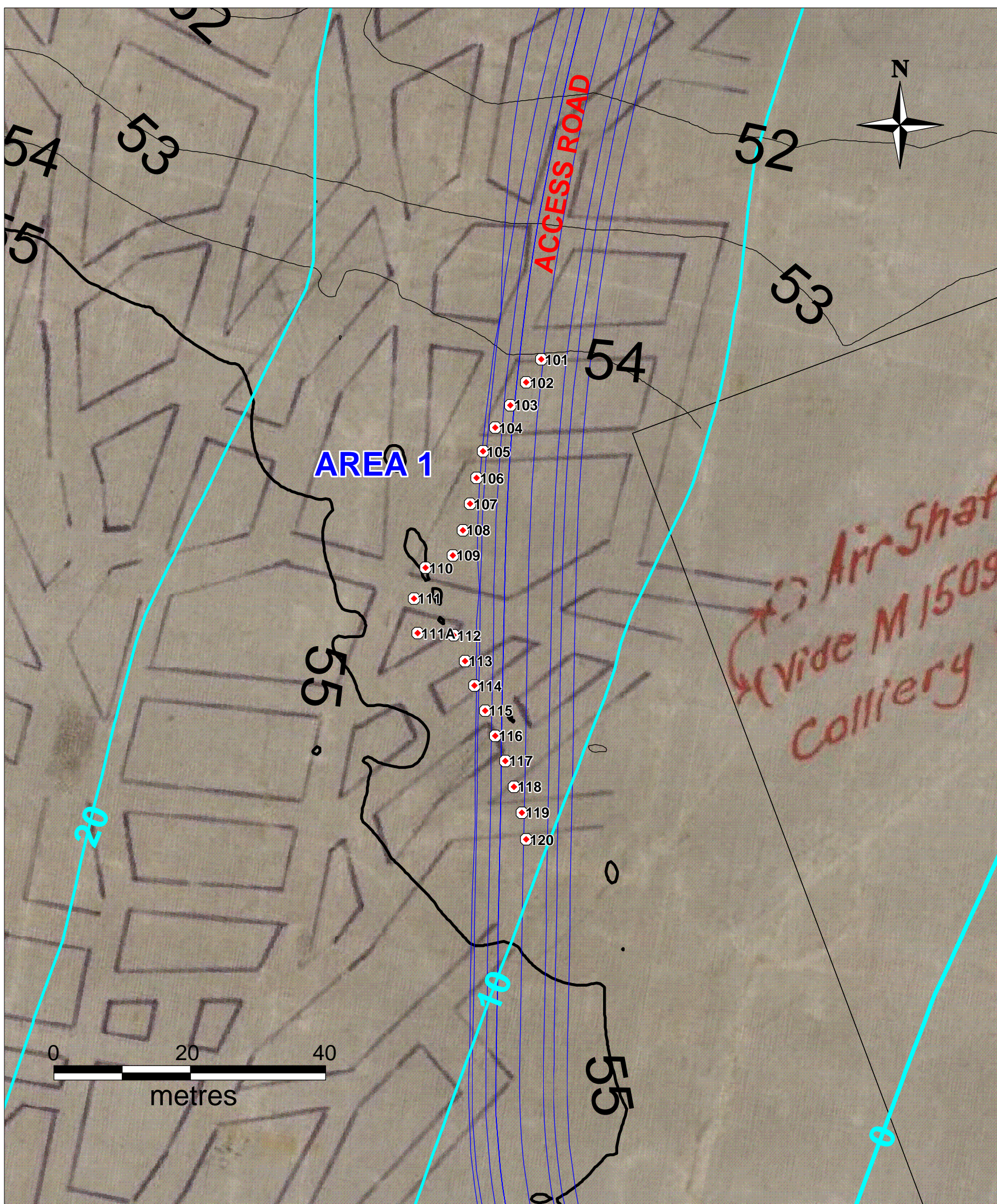
PROJECT No: 39129.03

OFFICE: NEWCASTLE

APPROVED BY:

DATE: 29 APRIL 2010

DRAWING No: 1



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TITLE: PACIFIC NATIONAL DEPOT  
 MANSFIELD STREET, GRETA  
 SITE PLAN - AREA 1

CLIENT: PACIFIC NATIONAL

DRAWN BY: JRB

SCALE: 1 : 600 (A3)

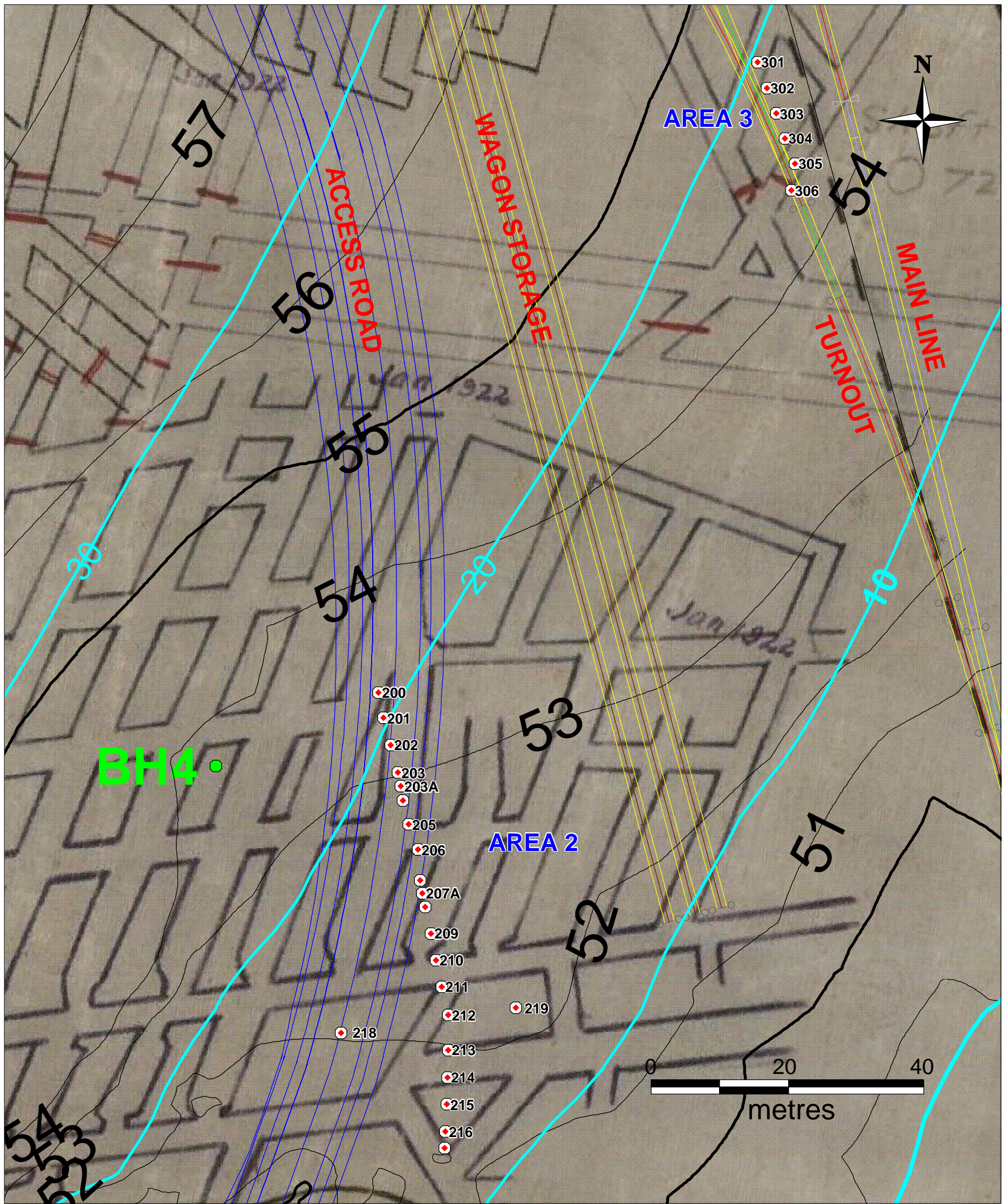
PROJECT No: 39129.03

OFFICE: NEWCASTLE

APPROVED BY:

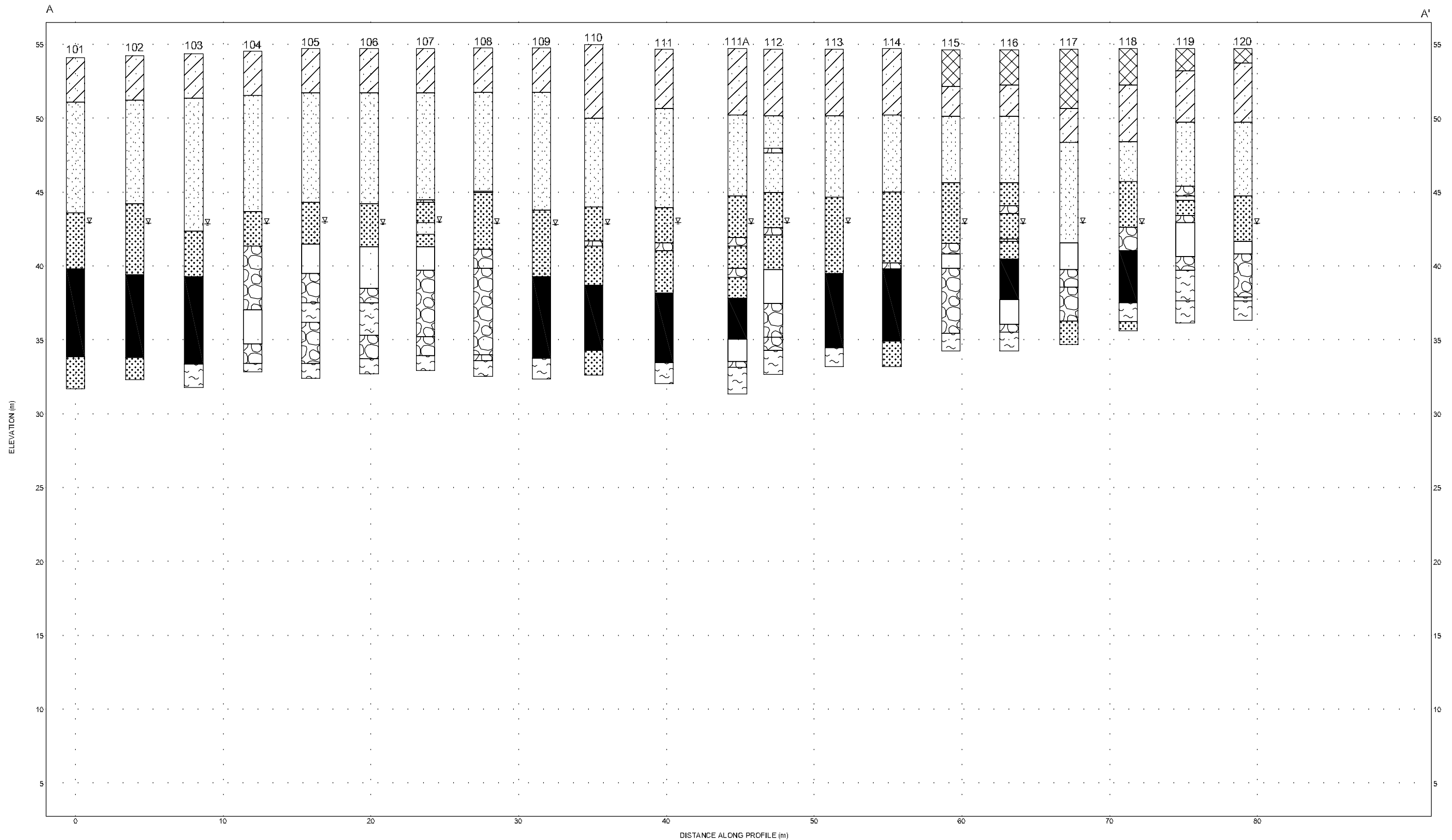
DATE: 29 APRIL 2010

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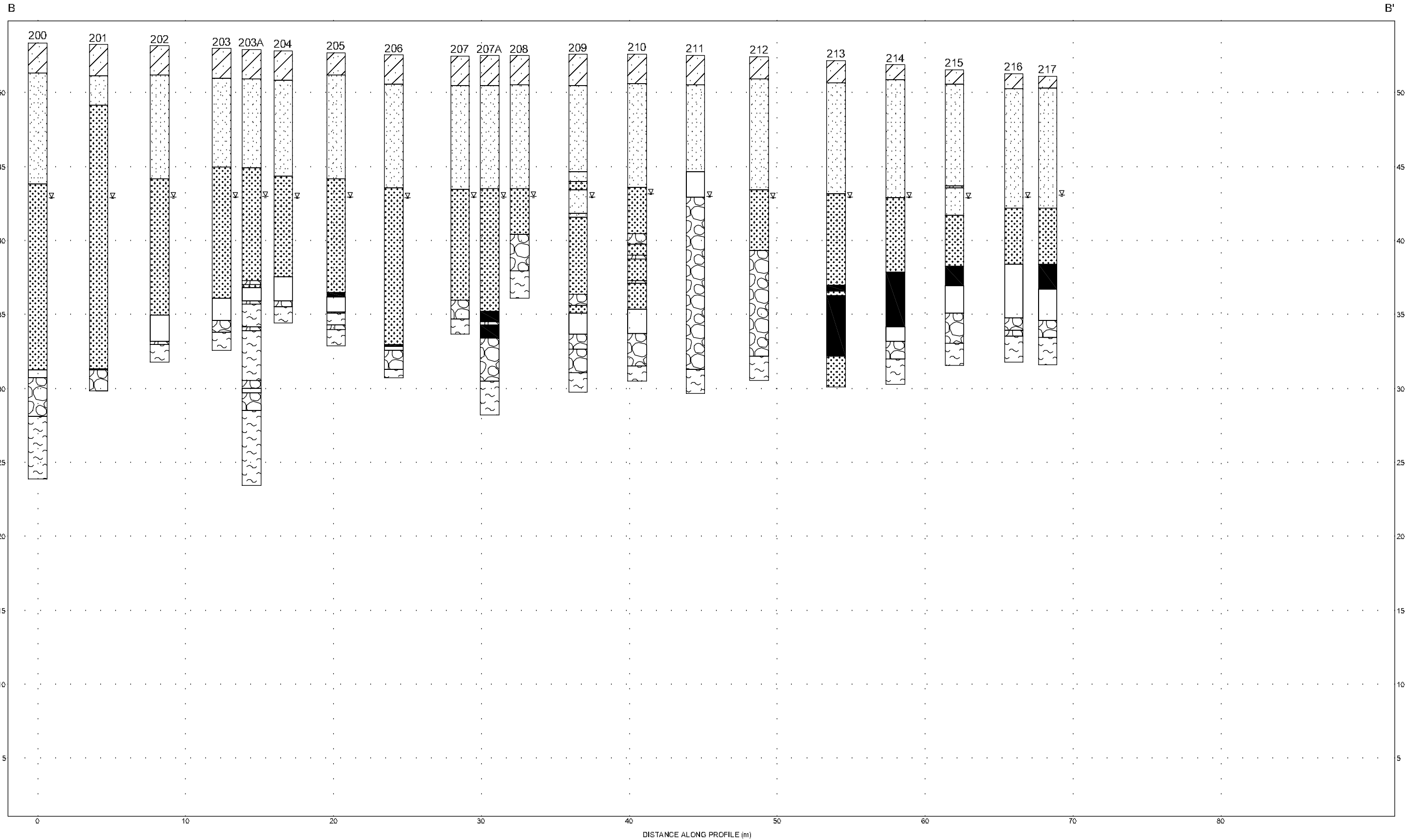
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TITLE: PACIFIC NATIONAL DEPOT MANSFIELD STREET, GRETA SITE PLAN - AREAS 2 AND 3			
CLIENT: PACIFIC NATIONAL			
DRAWN BY: JRB	SCALE: 1: 600 (A3)	PROJECT No: 39129.03	OFFICE: NEWCASTLE
APPROVED BY:	DATE: 29 APRIL 2010	DRAWING No: 1B	

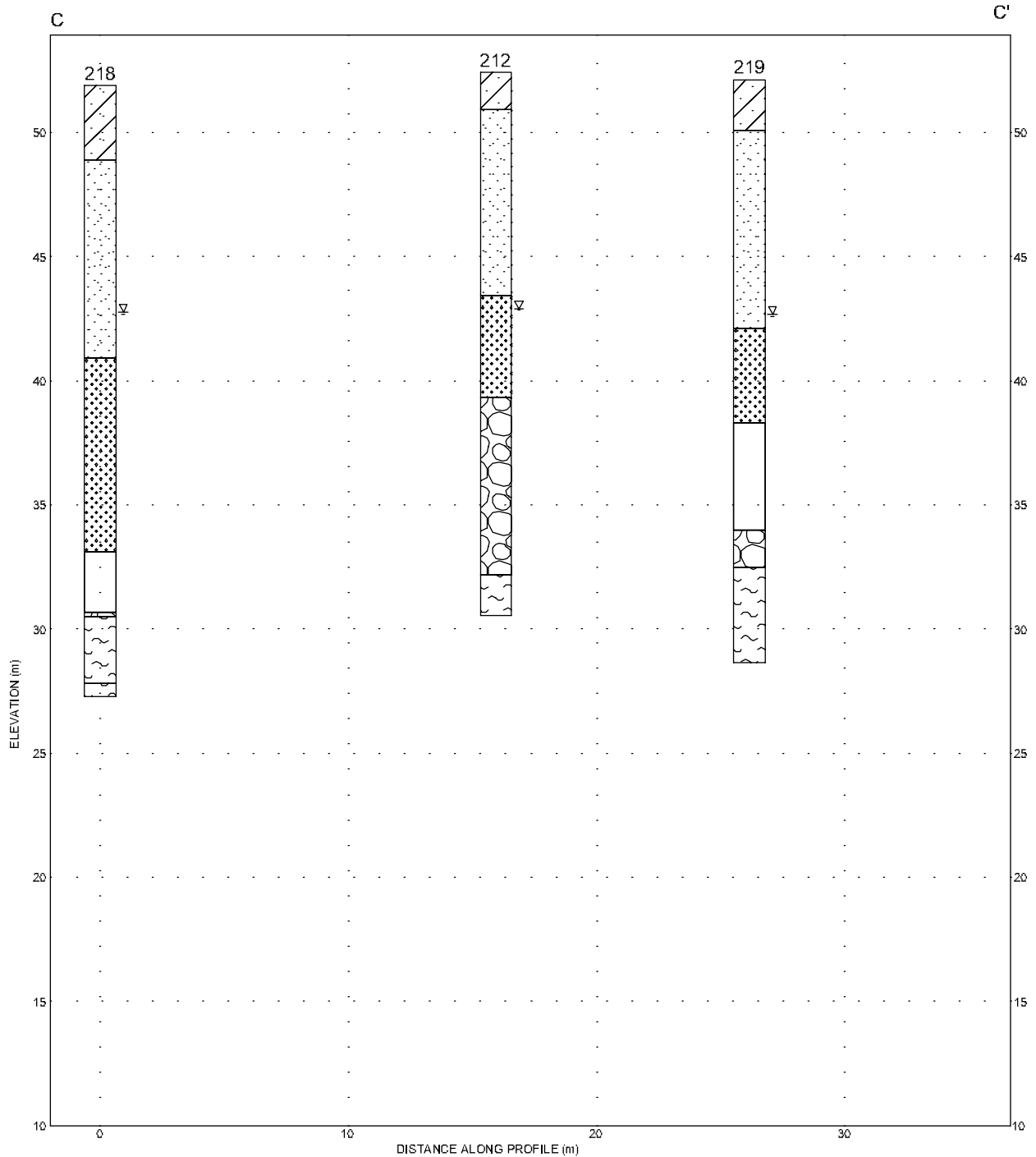


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






Sand/Clay/Weathered Sandstone	Rubble
Sandstone, highly weathered	Rod Fall
Sandstone, grey	Filling
Coal	Water Level
Rock	



- LEGEND**
- Sand/Clay/Weathered Sandstone
  - Sandstone, highly weathered
  - Sandstone, grey
  - Coal
  - Rock
  - Rubble
  - Rod Fall
  - Water Level



**LEGEND**

-  Sand/Clay/Weathered Sandstone
-  Sandstone, highly weathered
-  Sandstone, grey
-  Rock
-  Rubble
-  Rod Fall
-  Water Level



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**TITLE:** Section Through Area 2 Bores to Investigate Pothole Subsidence Risk  
 Pacific National Depot  
 Mansfield Street, Greta

**CLIENT:** PACIFIC NATIONAL PTY LTD

Ref: P:\39129.03\Drawings\Afr Cross Sections\39129.03 Section C

**OFFICE:** NEWCASTLE

**DRAWN BY:** PLH

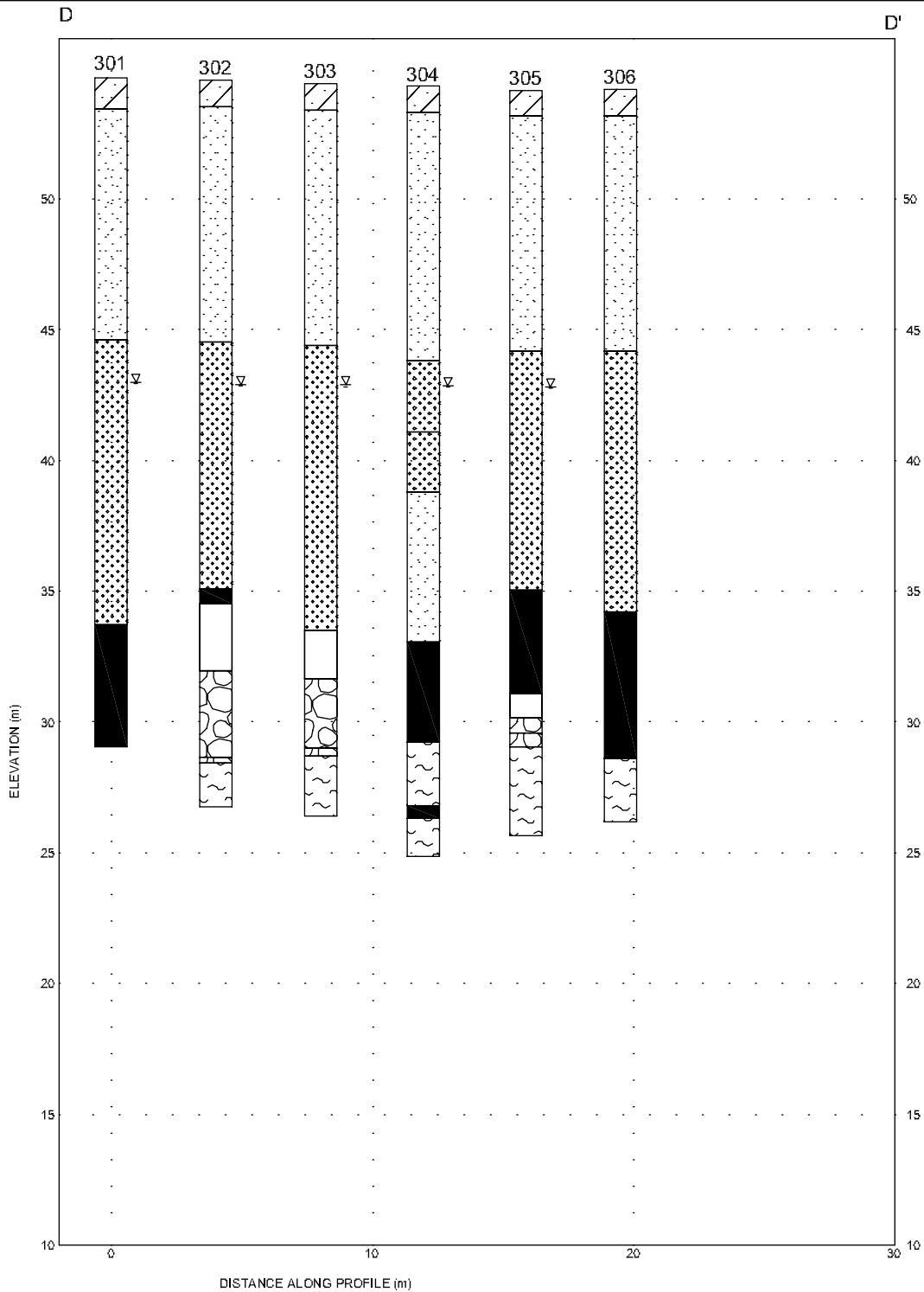
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





**PROJECT No:** 39129.03

**APPROVED BY:**

**DATE:**

**DRAWING No:** 4



- LEGEND**
-  Sand/Clay/Weathered Sandstone
  -  Rubble
  -  Sandstone, highly weathered
  -  Rod Fall
  -  Sandstone, grey
  -  Coal
  -  Rock
  -  Water Level



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 Perth, Wyong,

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 Townsville, Cairns

**TITLE:** Section Through Area 3 Bores to Investigate Pothole Subsidence Risk  
 Pacific National Depot  
 Mansfield Street, Greta

**CLIENT:** PACIFIC NATIONAL PTY LTD

Ref: P\139129.03\Drawings\Air Cross Sections\139129.03 Section D

**OFFICE:** NEWCASTLE

**DRAWN BY:** PLH

**SCALE:** 1:250

**PROJECT No:** 39129.03

**APPROVED BY:**

**DATE:**

**DRAWING No:** 5