

ILLAWARRA METALLURGICAL COAL:

**Appin – Longwalls 709, 710A, 710B, 711 and 905**

Subsidence Predictions and Impact Assessments for the Natural and Built Features due to the Extraction of the Proposed Longwalls 709, 710A, 710B, 711 and 905 at Appin Colliery

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MSEC448 (Rev. B) – *Appin Colliery – Longwalls 901 to 904 – Subsidence predictions and impact assessments for natural features and surface infrastructure in support of the Extraction Plan, dated 15 June 2012.*

Background reports available at [www.minesubsidence.com](http://www.minesubsidence.com)<sup>1</sup>:

Introduction to Longwall Mining and Subsidence (Revision A)

General Discussion of Mine Subsidence Ground Movements (Revision A)

Mine Subsidence Damage to Building Structures (Revision A)

<sup>1</sup> Direct link: [http://www.minesubsidence.com/index\\_files/page0004.htm](http://www.minesubsidence.com/index_files/page0004.htm)

Illawarra Metallurgical Coal (IMC), a wholly owned subsidiary of South32 Limited (South32), operates Appin Colliery, which is in the Southern Coalfield of New South Wales. The colliery is currently operating in two mining areas, referred to as Area 7 and Area 9.

IMC is preparing an Extraction Plan (EP) Application for Longwalls 709, 710A, 710B, 711 and 905 (LW709 to LW711 and LW905) in the Bulli Seam. This subsidence report provides information to support the EP Application which will be submitted to the Department of Planning, Industry and Environment.

The predicted subsidence effects for the existing, approved and proposed longwalls in Appin Areas 7 and 9 have been obtained using the Incremental Profile Method (IPM). The IPM has been calibrated for the local conditions using the available ground movement monitoring data from Appin Colliery and other nearby collieries.

The maximum predicted total subsidence effects are 1550 mm vertical subsidence, 7 mm/m tilt (i.e. 0.7 %, or 1 in 143), 0.08 km<sup>-1</sup> hogging curvature (i.e. minimum radius of curvature of 13 km) and 0.15 km<sup>-1</sup> sagging curvature (i.e. minimum radius of curvature of 7 km). The maximum predicted strains (excluding valley-related effects) are 1.0 mm/m tensile and 1.6 mm/m compressive based on the 95 % confidence levels.

The *Study Area* has been defined, as a minimum, as the surface area enclosed by the: 35° angle of draw line from the extents of the proposed LW709 to LW711 and LW905; and the predicted additional 20 mm subsidence contour due to the extraction of the proposed longwalls. The features that are located within but not limited to 600 m of the proposed longwalls and are predicted to experience far-field or valley-related effects and could be sensitive to these movements have also been included in the assessments provided in this report.

The assessments provided in this report should be read in conjunction with the assessments provided in the reports by other specialist consultants on the project. The main findings from this report are as follows:

- The Nepean River is located to the south and to the east of the proposed LW709 to LW711 and LW905. The thalweg (i.e. centreline) of the Nepean River is located at a minimum distance of 1.5 km from the proposed longwalls.

The river is predicted to experience vertical subsidence, upsidence and closure of less than 20 mm. It is considered unlikely, therefore, that the Nepean River would experience adverse physical impacts due to the mining-induced movements from LW709 to LW711 and LW905. Gas release zones have been observed along the river during the mining of longwalls in Areas 7 and 9. Further gas release zones could develop due to the mining of the proposed longwalls.

- Third order streams within the Study Area comprise sections of Foot Onslow Creek, Harris Creek, Navigation Creek and Navigation Creek Tributary 1. The total length of the third order sections of the creeks above the mining area is 4.2 km and the total length within the Study Area is 6.8 km. There are also first and second order creeks and tributaries located across the Study Area.

There are no predicted reversals of stream grade due to the proposed mining. It is considered unlikely, therefore, that there would be large-scale adverse changes in the levels of ponding or scouring of the banks along the creeks and tributaries within the Study Area due to the mining-induced tilt. It is possible that localised increased ponding could develop in some locations, where the natural grades are small, and upstream of the chain pillars and the edges of the mining area.

Fracturing of the uppermost bedrock can occur along the streams that are located directly above or adjacent to the proposed mining area. Surface water flow diversions could occur along the creeks and tributaries that are located directly above the mining area. It is unlikely, however, that there would be a net loss of water from the catchment.

Further assessments of the potential impacts on surface water are provided in the reports by the specialist surface water and groundwater consultants on the project.

- There are 7 cliffs that have been identified within the Study Area. The cliffs are located along Razorback Range in the western part of the Study Area and have formed in the sandstone members of the Wianamatta Group. The cliffs within the Study Area have lengths ranging between 25 m and 90 m and maximum heights ranging between 12 m and 20 m.

There are three cliffs that are located directly above the proposed LW710A and LW905. These cliffs could experience fracturing and, where the exposed rock face is marginally stable, could result in cliff instabilities. It has been estimated that between 3 % and 5 % of the total length, or between 1 % and 3 % of the total face area of cliffs located above the mining area would experience adverse impacts.

There are four other cliffs that are located within the Study Area but are outside the proposed mining area. It is possible that isolated rock falls could occur at these cliffs, but it is expected that on average this would represent less than 1 % of the total length or total face area of the cliffs.

- Rock outcrops are located across the Study Area, primarily along the Razorback Range and the incised creeks and tributaries. The extraction of the proposed longwalls is likely to result in fracturing of the rock outcrops and, where the rock is marginally stable, this could then result in instabilities. Previous experience in the Southern Coalfield indicates that the percentage of rock outcrops that are likely to be impacted by mining is very small, representing less than 3 % of the total surface area.
- The steep slopes within the Study Area have been identified along Razorback Range above LW710A, the western end of LW711 and above LW905. The surface soils for the steep slopes along Razorback Range have formed from the Wianamatta Group. The steep slopes along Razorback Range exhibit natural soil erosion.

Mining beneath the steep slopes is expected to result in increased horizontal movements in the downslope direction, causing tension cracks to appear at the tops and along the sides of the slopes and compression ridges to form at the bottoms of the slopes. The crack widths are expected to be typically between 25 mm and 50 mm; however, larger cracks in the order of 100 mm to 150 mm or greater could also develop.

There are built features located on and above the steep slopes along Razorback Range, including local roads, powerlines, telecommunications cables, houses, rural structures and survey control marks. It is recommended that monitoring and management strategies are developed for these built features, to identify localised and anomalous movements and to manage the potential impacts.

- The Main Southern Railway is located to the east of the proposed longwalls. The railway is situated immediately adjacent to the south-eastern corner of LW709 and it is 0.4 km from LW710B and 0.9 km from LW711, at its closest points. The infrastructure associated with the railway includes cuttings, embankments and culverts.

The predicted additional subsidence for the railway due to the mining of the proposed longwalls is 150 mm. Only low level additional subsidence effects are predicted since the railway is located outside the proposed mining area.

Monitoring and management plans have been developed for the mining of LW703 to LW708B, LW901 and LW902 directly beneath the railway. It is expected that the Main Southern Railway can be maintained in safe and serviceable conditions using the same strategies used for these existing longwalls.

- The M31 Hume Motorway is located 470 m from the south-eastern corner of LW709, at its closest point to the proposed longwalls. At this distance, the motorway is predicted to experience less than 20 mm vertical subsidence and it is not predicted to experience measurable tilts, curvatures or strains.

It is anticipated therefore that the M31 Hume Motorway would not experience adverse impacts due to the mining of the proposed longwalls. It is expected that the motorway can be maintained in safe and serviceable conditions during the mining period using the same management strategies adopted for the existing longwalls at Appin Colliery.

- The Nepean Twin Bridges are located 2.5 km south of LW905, at their closest point to the proposed longwalls. The bridges will experience additional low-level far-field horizontal movements due to the mining of the proposed longwalls.

Monitoring and management plans have been developed for the mining of LW701 to LW708B and LW901 to LW903. It is expected that the Nepean Twin Bridges can be maintained in safe and serviceable conditions using the same strategies used for these existing longwalls.

- Local roads are located across the Study Area and they are predicted to experience the full range of subsidence effects. It is expected that the local roads that are located directly above the proposed longwalls could experience cracking and heaving of the road surfaces. Previous experience of mining beneath local roads in the Southern Coalfield indicates that these impacts can be managed with the implementation of suitable management strategies.

Hawkey Road traverses a ridgeline of Razorback Range above the maingate of the proposed LW905. Top Ridge Road, Gibraltar Drive and other local roads are located at the top of Razorback Range above the proposed LW710A and the western end of the proposed LW711. It is recommended that ground movement and visual monitoring are carried out along these roads during active subsidence to identify increased or irregular movements and to manage potential impacts.

- Moreton Park Road Bridge (North) and Moreton Park Road Bridge (South) are located 1.5 km and 0.6 km, respectively, from the proposed longwalls. The bridges will experience additional low-level far-field horizontal movements due to the mining of the proposed longwalls. Monitoring and management plans have been developed for the mining of LW701 to LW708B. It is expected that the bridges can be maintained in safe and serviceable conditions using the same strategies used for these existing longwalls.

- There is no potable water, sewerage or gas infrastructure located within the Study Area. There is infrastructure located outside the Study Area; however, it is not predicted to experience measurable conventional subsidence effects. It is not anticipated, therefore, that adverse impacts would occur to this infrastructure.

A local sewer network is proposed to be constructed in the Menangle region in the next one to two years. The new infrastructure would need to be designed and constructed to accommodate mine subsidence effects prescribed by Subsidence Advisory NSW.

- There are 66 kV, 11 kV and low voltage powerlines located within the Study Area. Previous experience from the Southern Coalfield indicates that the powerlines could experience some minor impacts. It is expected that the remedial measures would include some adjustments of the cable catenaries, pole tilts and the consumer cables, as has been undertaken in the past, but any other impacts are expected to be relatively infrequent and easily repaired.
- The telecommunications infrastructure comprises direct buried optical fibre cables, underground copper cables and a telecommunications tower. Monitoring and management plans have been developed for the mining of LW703 to LW708B, LW901 and LW902 directly beneath the optical fibre and copper telecommunications cables. It is expected that these cables can be maintained in serviceable conditions using these same strategies.

The telecommunications tower is located outside and adjacent to the maingate of LW905. It is recommended that the predicted subsidence effects for the telecommunications tower is provided to the infrastructure owner so that a detailed structural analysis can be undertaken of the tower and its associated infrastructure.

- There are 581 rural structures identified within the Study Area, which includes sheds, garages, gazebos, pergolas, greenhouses, playhouses, shade structures and other non-residential building structures. There is extensive experience of mining directly beneath rural building structures in the Southern Coalfield which indicates that the incidence of impacts on these structures is very low. It is expected, therefore, that these structures would remain safe and serviceable and that any impacts could be remediated using well established building techniques.
- There are 239 farm dams identified within the Study Area. Based on previous experience from the Southern Coalfield, it is expected that the incidence of impacts on the farm dams would be low. Cracking or leakage of water in the farm dam walls could be identified from visual inspections and repaired, as required.
- There are nine Aboriginal heritage sites located within the Study Area or within 600 m of the proposed longwalls, all of which are open sites. Surface cracking could occur near the sites that are located directly above the proposed longwalls. It is unlikely that the artefacts and deposits themselves would be impacted by surface cracking. It is possible, however, that if remediation of the surface were required after mining, that these works could potentially impact the Aboriginal heritage sites.
- The survey marks located directly above the proposed mining area could experience the full range of predicted movements. The survey control marks in the vicinity of the proposed longwalls could also experience small vertical subsidence and far-field horizontal movements. It may be necessary on the completion of the proposed longwalls, when the ground has stabilised, to re-establish any state survey control marks that are required for future use.
- There are 177 houses identified within the Study Area, of which, 30 are located directly above the existing longwalls, 76 are located above the proposed longwalls and 71 are located outside the mining area. The potential impacts on the houses have been assessed using the method developed as part of ACARP Research Project C12015, which is described in Appendix B.

All houses within the Study Area are expected to remain safe and serviceable throughout the mining period, provided that they are in sound structural condition prior to mining. IMC has extensive experience of mining beneath houses at Appin and West Cliff Collieries. It has developed and acted in accordance with risk management plans to manage potential impacts to residential structures during the mining of LW701 to LW708 and LW901 to LW903.

There are 8 houses located at the top of Razorback Range and directly above or adjacent to the proposed LW710A and the western end of the proposed LW711. These houses could experience increased effects due to the potential for non-conventional ground movements.

A Landslide Risk Assessment has been carried out and the key conclusion is that the *“results of the preliminary investigations indicate that the structures located on the identified steep slopes should be able to be effectively managed, using the techniques and management tools available and previously employed in similar geotechnical circumstances, to permit Appin Mine to extract the proposed longwalls”* (GHD, 2021d).

Geotechnical inspections have been carried out for three of the properties located at the top of Razorback Range (Refs. O02, O17 and O18) and they found that *“the [landslide] risk levels assessed for current conditions, upon which mining related impact are imposed, remain unchanged from the currently existing risk levels”* (GHD 2021a, 2021b and 2021c). The other *“residential sites that require appraisals can be conducted on a longwall-by-longwall basis”* so that *“the hazards can be managed through each subordinate longwall’s Structures Management Plan”* (GHD, 2021d).

Monitoring has been established for the houses located at the top of Razorback Range for the mining of LW904, which includes ground monitoring lines and Global Navigation Satellite System (GNSS) units located above the mining area and on the side of Razorback Range. Further monitoring is proposed on the private properties, subject to the approval of the individual property owners. Further ground monitoring lines and GNSS units will be established for LW709 to LW711 and LW905 as part of the Structures Management Plans for these proposed longwalls.

The assessments provided in this report indicate that the assessed levels of impact on the natural and built features can be managed by the preparation and implementation of appropriate management strategies. It should be noted, however, that more detailed assessments of some natural and built features have been undertaken by other specialist consultants, and the findings in this report should be read in conjunction with the findings in all other relevant reports.

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## Drawings

Drawings referred to in this report are included in Appendix E at the end of this report.

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MSEC1117-01	General layout and monitoring	A
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MSEC1117-03	Bulli Seam floor contours	A
MSEC1117-04	Bulli Seam thickness contours	A
MSEC1117-05	Bulli Seam depth of cover contours	A
MSEC1117-06	Geology	A
MSEC1117-07	Streams	A
MSEC1117-08	Cliffs and steep slopes	A
MSEC1117-09	Railways and roads	A
MSEC1117-10	Electrical infrastructure	A
MSEC1117-11	Telecommunications infrastructure	A
MSEC1117-12	Water and gas infrastructure	A
MSEC1117-13	Building structures	A
MSEC1117-14	Other built features	B
MSEC1117-15	Predicted additional subsidence contours due to LW709 to LW711 and LW905	A
MSEC1117-16	Predicted total subsidence contours due to LW702 to LW711 and LW901 to LW905	A

### 1.1. Background

Illawarra Metallurgical Coal (IMC), a wholly owned subsidiary of South32 Limited (South32), operates Appin Colliery, which is in the Southern Coalfield of New South Wales (NSW). The colliery is currently operating in two mining areas, referred to as Area 7 and Area 9.

IMC previously submitted a Part 3A Application for the extraction of future longwalls within the Bulli Seam including in Appin Areas 7 and 9. Report No. MSEC404 (Rev. D) was issued in August 2009 in support of that application. A Preferred Project Report (PPR) under the *Environmental Planning and Assessment Act 1979* (EP&A Act) was prepared following a request by the Director-General of the NSW Department of Planning, Infrastructure and Environment (DPIE). The key changes made via the PPR comprised the excision of mining domains known as North Cliff, Area 2 and Area 3 (part only). DPIE granted IMC approval under the EPA Act on the 22 December 2011 (08\_0150).

IMC has completed the mining of Longwalls 701 to 708A (LW701 to LW708A) in Area 7 and is currently mining Longwall 708B (LW708B). The colliery also has approval to mine Longwalls 709 and 710 (LW709 and LW710). The subsidence predictions and assessed impacts for these longwalls are provided in Report No. MSEC342 (Rev. C), which supported the Subsidence Management Plan Applications. Updated subsidence predictions and impact assessments are provided in Reports Nos. MSEC825 (Rev. A), MSEC878 (Rev. A) and MSE973 (Rev. A), which supported the Modification Applications.

IMC has also completed the mining of Longwalls 901 and 902 (LW901 and LW902) in Area 9 and is currently mining Longwall 903 (LW903). The mine also has Extraction Plan approval to mine Longwall 904 (LW904). The subsidence predictions and assessed impacts for these longwalls are provided in Report No. MSEC448 (Rev. B), which supported the Extraction Plan Application. Updated subsidence predictions and impact assessments are provided in Reports Nos. MSEC743 (Rev. A), MSEC829 (Rev. A) and MSEC1005 (Rev. A), which supported the Modification Applications.

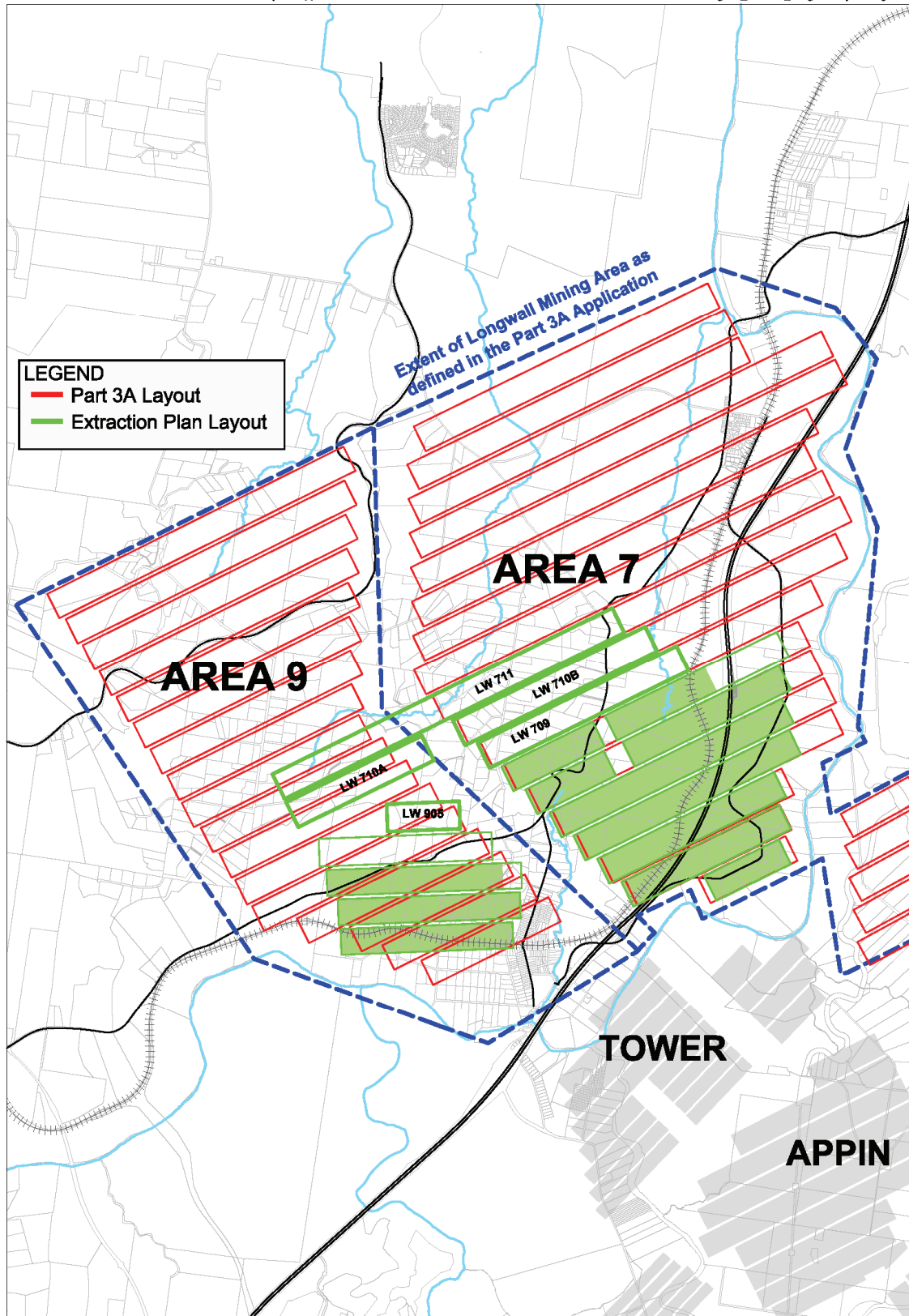
Mine Subsidence Engineering Consultants (MSEC) has now been engaged by IMC to prepare subsidence predictions and impact assessments for LW709 to LW711 and LW905. LW710 has been subdivided into two panels (LW710A and LW710B) and LW711 has been extended across both Areas 7 and 9.

The longwalls in Areas 7 and 9 have been modified from the layout of the *EA Base Plan Longwalls* which was indicated in the Part 3A Application. The longwall layout indicated in the Part 3A Application and in Report No. MSEC404 is referred to as the *Part 3A Layout* in this report. The currently proposed longwall layout in Areas 7 and 9 is referred to as the *Extraction Plan Layout* in this report.

The comparison between the longwalls based on the Part 3A Layout and the Extraction Plan Layout is provided in Fig. 1.1. The proposed LW709 to LW711 and LW905 are located within the *Extent of Longwall Mining Areas* as defined in the Part 3A Application and Report No. MSEC404.

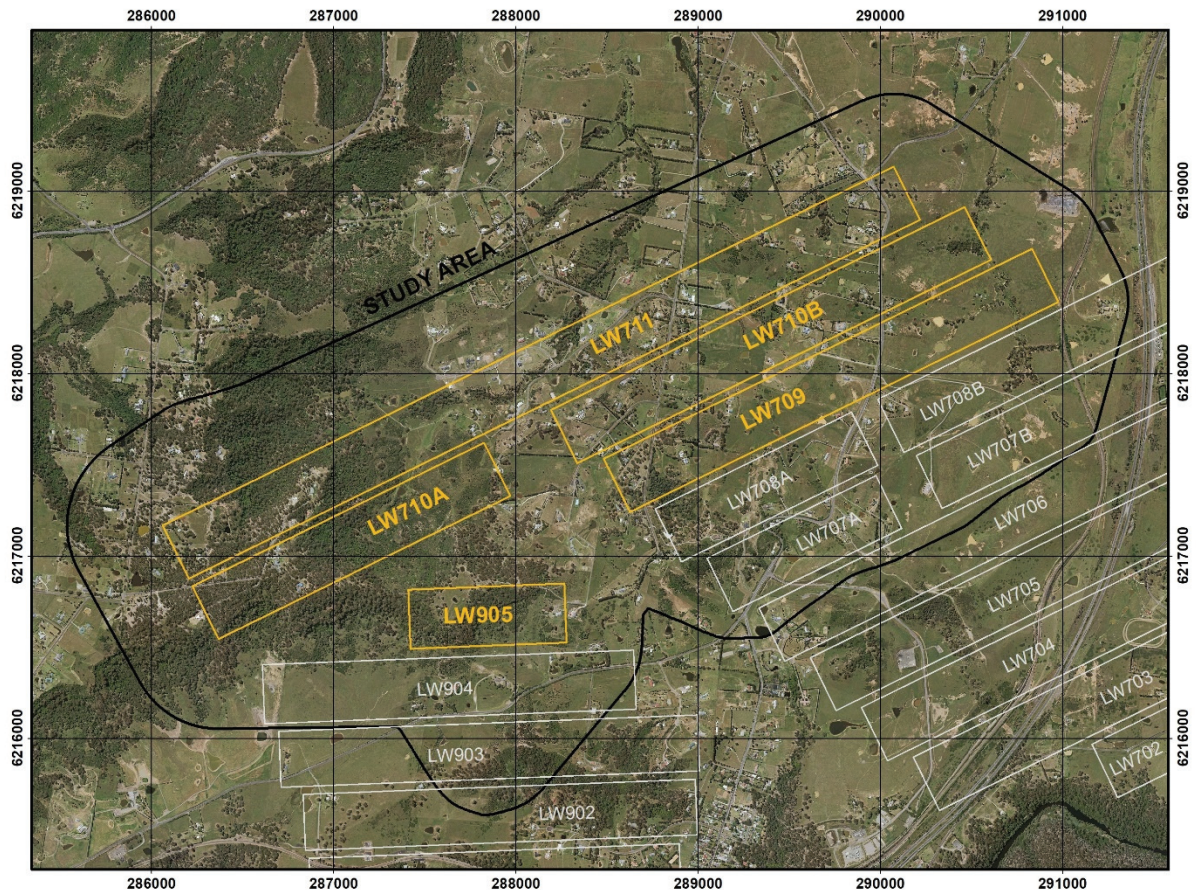
The eastern ends of LW709, LW710B and LW711 have been shortened from the extents indicated in the Part 3A Layout. LW710A and the western end of LW711 are located in Area 9, within its *Extent of Longwall Mining Area*.

The existing, approved and proposed LW901 to LW905 have been rotated 24° clockwise compared with the longwalls indicated in the Part 3A Layout and Report No. MSEC404. The western end of LW905 has been shortened to accommodate the proposed LW710A and the extended LW711.



**Fig. 1.1 Existing, approved and proposed longwalls in Areas 7 and 9**

The existing, approved and proposed longwalls in Areas 7 and 9 are also shown in Drawing No. MSEC1117-01, in Appendix E. The longwalls and the Study Area, as defined in Section 2.1, have been overlaid on an aerial photograph of the area, and are shown in Fig. 1.2.



Courtesy of IMC

**Fig. 1.2 Aerial showing LW709 to LW711 and LW905 and the Study Area**

IMC is preparing an Extraction Plan Application for the proposed LW709 to LW711 and LW905 in Areas 7 and 9. MSEC has been commissioned by IMC to:

- prepare subsidence predictions for the proposed longwalls, including the cumulative movements due to the previously extracted and approved longwalls in Areas 7 and 9;
- identify the natural and built features in the vicinity of the proposed longwalls;
- provide subsidence predictions for each of these features;
- prepare impact assessments, in conjunction with other specialist consultants, for each of the natural and built features; and
- recommend management strategies and monitoring.

This report has been prepared to support the Extraction Plan Application for the proposed LW709 to LW711 and LW905 which will be submitted to the DPIE. In some cases, this report will refer to other sources of information on specific natural and built features. This report, therefore, should be read in conjunction with the other relevant reports associated with this application.

Chapter 1 provides background information on the study, including the mining geometry, surface and seam and overburden lithology.

Chapter 2 defines the Study Area and provides a summary of the natural and built features identified within this area.

Chapter 3 provides an overview of the methods that have been used to predict the mine subsidence effects resulting from the extraction of the existing, approved and proposed longwalls.

Chapter 4 provides the maximum predicted subsidence effects resulting from the extraction of the existing, approved and proposed longwalls.

Chapters 5 and 6 provide the descriptions, predictions and impact assessments for each of the natural and built features that have been identified within the Study Area. Recommendations for each of these features are also provided, which have been based on the predictions and impact assessments.

## 1.2. Mining geometry

The layout of the proposed LW709 to LW711 and LW905 are shown in Drawing No. MSEC1117-01, in Appendix E. A summary of the dimensions for these longwalls is provided in Table 1.1. The longwalls are proposed to be extracted from the Bulli Seam.

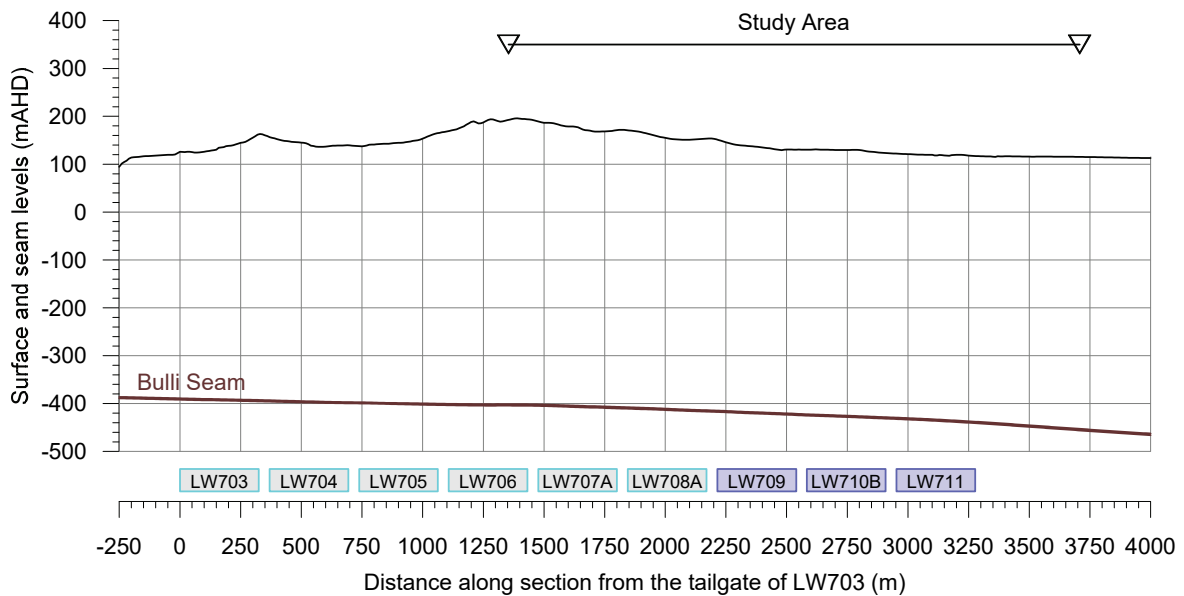
**Table 1.1 Geometry of the proposed LW709 to LW711 and LW905**

Longwall	Overall void length including installation heading (m)	Overall void width including first workings (m)	Overall tailgate chain pillar width (m)
LW709	2615	324	45
LW710A	1787	324	-
LW710B	2529	324	45
LW711	4469	324	45
LW905	858	324	54

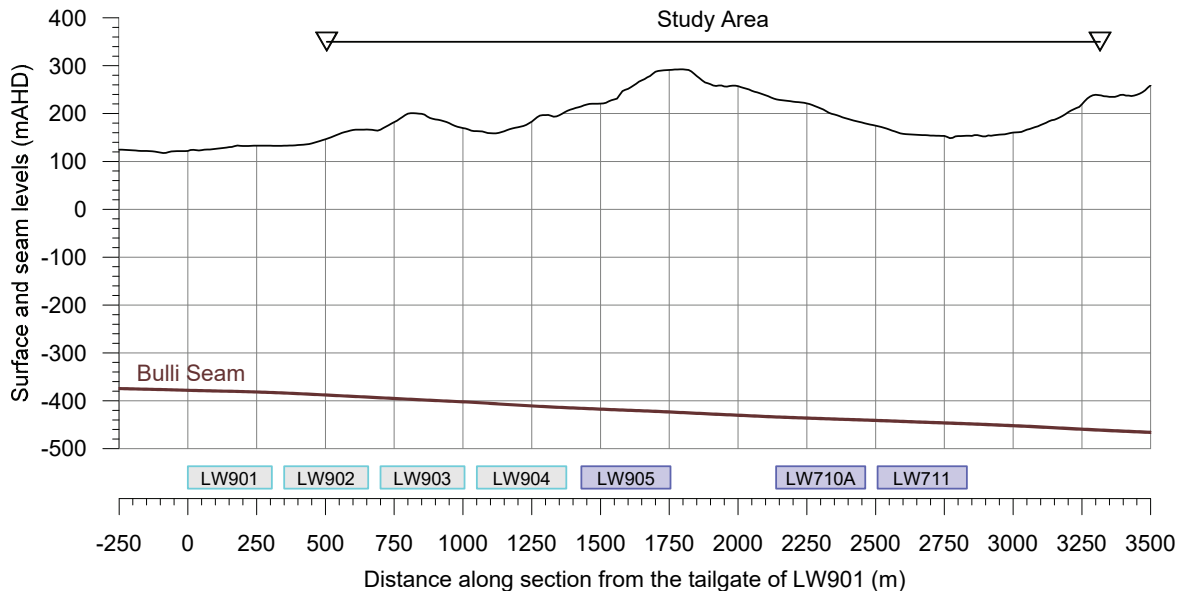
The lengths of longwall extraction excluding the installation headings are approximately 9 m less than the overall void length provided in the above table. The longwall face widths excluding the first workings are 315 m. The longwalls will be extracted from west to east within the Bulli Seam.

## 1.3. Surface and seam levels

The levels of the natural surface and the Bulli Seam are illustrated along Section 1 and Section 2 in Fig. 1.3 and Fig. 1.4, respectively. The locations of these sections are shown in Drawings Nos. MSEC1117-02 to MSEC1117-04. The definition of the Study Area is provided in Section 2.1.



**Fig. 1.3 Surface and seam levels along Section 1**



**Fig. 1.4 Surface and seam levels along Section 2**

The surface level contours are shown in Drawing No. MSEC1117-02. The proposed longwalls are located to the north and west of the Nepean River. The western ends of LW710A, LW711 and LW905 mine beneath Razorback Range. The streams above the proposed mining area generally flow in north and north-easterly directions and drain into the Nepean River several kilometres to the north. The streams above the existing and approved longwalls generally flow in a southerly direction and also drain into the Nepean River.

The surface levels directly above the proposed longwalls vary between 100 metres above Australian Height Datum (mAHD) and 320 mAHD. The lowest surface level occurs along Foot Onslow Creek where it crosses the finishing (i.e. eastern) end of LW710B. The highest surface level occurs at the top of Razorback Range above the western end of LW710A.

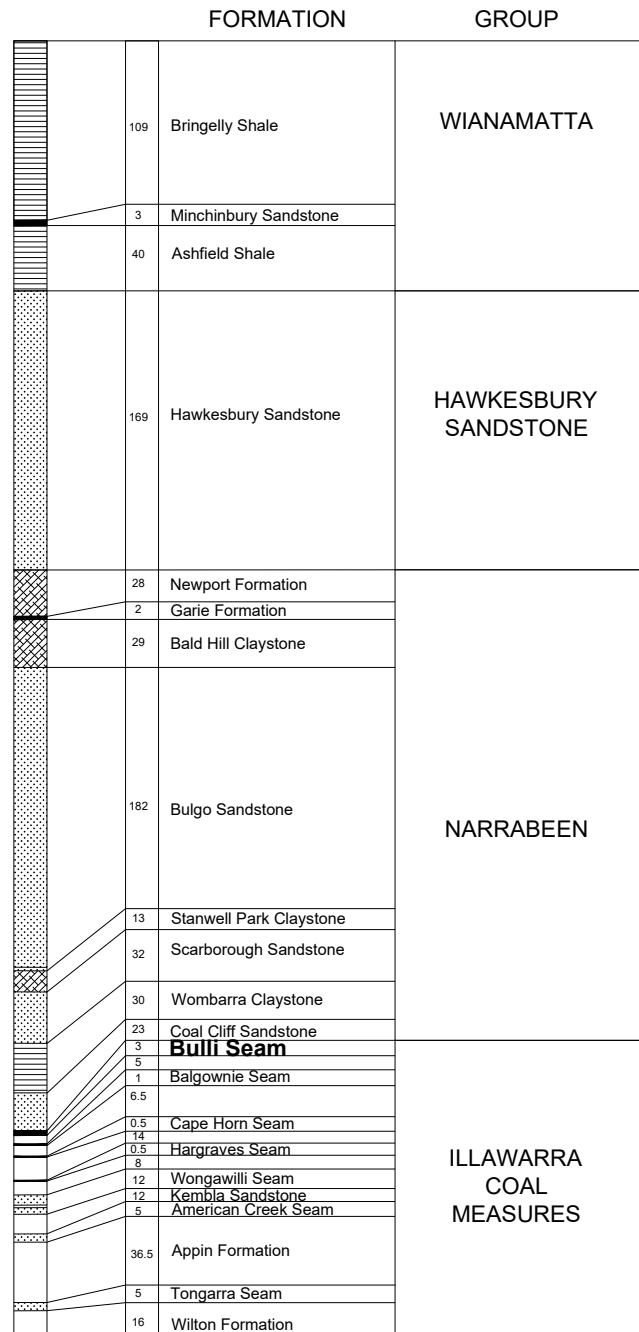
The seam floor contours, seam thickness contours and depth of cover contours for the Bulli Seam are shown in Drawings Nos. MSEC1117-03, MSEC1117-04 and MSEC1117-05, respectively. The seam generally dips from the south towards the north, with an average grade across the mining area of approximately 2.0 %, or 1 in 50.

The depths of cover to the Bulli Seam directly above the proposed longwalls vary between 530 m and 750 m. The minimum depth of cover occurs along Foot Onslow Creek where it crosses the finishing (i.e. eastern) end of LW709. The maximum depth of cover occurs along the Razorback Range above the western end of LW711.

The thickness of the Bulli Seam within the extents of the proposed LW709 to LW711 and LW905 vary between 2.8 m and 3.3 m. The proposed longwalls will extract the full seam thickness.

## 1.4. Geological details

Appin Colliery lies in the southern part of the Permo-Triassic Sydney Basin, within which the main coal bearing sequence is the Illawarra Coal Measures, of Late Permian age. The Illawarra Coal Measures contain several seams, the uppermost of which is the Bulli Seam. A typical stratigraphic section for Appin Area 9, through the Razorback Range, is shown in Fig. 1.5 (Source: IMC).



**Fig. 1.5 Typical stratigraphic section for Appin Colliery through Razorback Range (Source: IMC)**

All of the sediments that form the overburden to the Bulli Seam belong to the Hawkesbury Tectonic Stage, which comprises three stratigraphic divisions. The lowest division is the Narrabeen Group, which is subdivided into a series of interbedded sandstone and claystone units. It ranges in age from Lower to Middle Triassic and varies in thickness with a median of 310 m.

Overlying the Narrabeen Group is the Hawkesbury Sandstone Group, which is a series of bedded sandstone units which dates from the Middle Triassic and has a median thickness of 170 m. Above the Hawkesbury Sandstone is the Wianamatta Group, which consists of shales and siltstones with a variable thickness within the Study Area, ranging from less than 10 m to 200 m.

The major sandstone units are interbedded with shale and claystone units. The major sandstone units are the Scarborough, the Bulgo and the Hawkesbury Sandstones. The rocks exposed in the Nepean River valley belong to the Hawkesbury Sandstone Group. The creeks and drainage lines within the Study Area traverse the Wianamatta Group Shale to where they enter the Nepean River valley. Within the Narrabeen Group, the claystones and shales generally exist in discrete but thinner beds of less than 15 m thickness, or are interbedded as thin bands within the sandstone.

The major claystone units are the Bald Hill and Stanwell Park Claystones, which lie above and below the Bulgo Sandstone at the base of the Hawkesbury Sandstone. The claystones vary in thickness and, in some places, are more than 25 m thick.

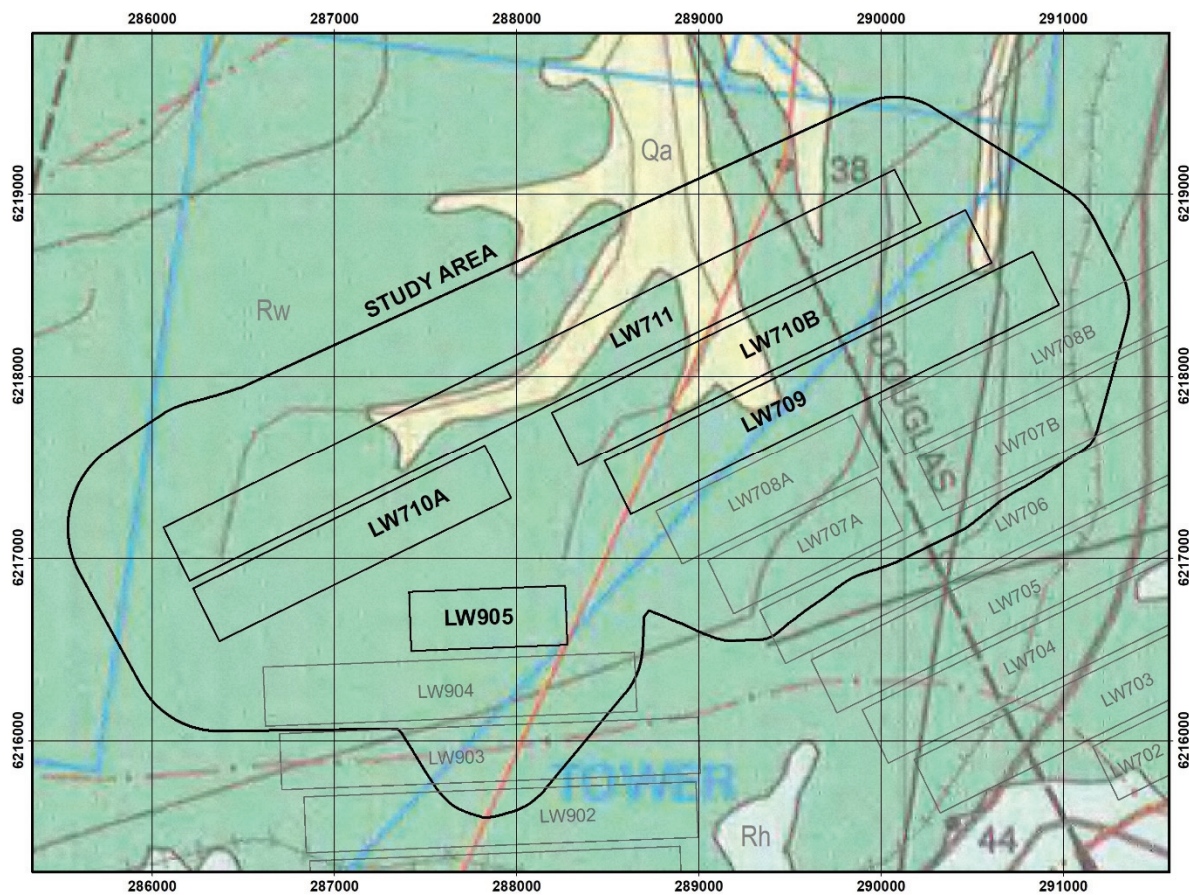
The geological structures which have been identified at seam level are shown in Drawing No. MSE1117-06.

There are to NNW-SSE orientated *zones of geological structure* that cross Appin Areas 7 and 9. These zones comprise series of dykes with thicknesses up to approximately 3.5 m and minor faults with displacements up to approximately 3 m. Longwalls 707 and 708 have been split into four shorter panels (i.e. LW707A, LW707B, LW708A and LW708B) to avoid this zone.

The proposed LW709, LW710B and LW711 cross one of the *zones of geological structure* and LW905 is located on the eastern side of the other zone. Increased vertical subsidence is not anticipated in these locations, as it was not observed when LW703 to LW706 and LW901 to LW903 previously mined through these zones. Localised irregular surface movements (i.e. compressive strain and heaving) were observed near these zones; however, these appear to be due to surface topography (i.e. drainage lines and cuttings) rather than the geological structures. In any case, the surface features located above and near the *zones of geological structure* have been assessed for potential localised irregular ground movements.

Elsewhere, minor faults with displacements up to approximately 1 m cross the proposed LW709 to LW711 and LW905. Increased subsidence or localised irregular movements are not anticipated in these locations, as it was when longwalls previously mined through similar geological structures in Appin Areas 7 and 9. Surface features have been assessed for potential anomalous irregular movements, as the predicted strains are based on statistical analyses of monitoring data that comprises these effects.

The surface lithology within the Study Area is illustrated in Fig. 1.6, which shows the proposed longwalls overlaid on Geological Series Sheets 9029, published in 1999 by the NSW Department of Planning and Environment Division of Resources and Geoscience, formerly the Department of Primary Industries (DPI).



**Fig. 1.6 Surface lithology within the Study Area (DPI Geological Series Sheets 9029)**

The surface lithology above the proposed longwalls generally comprises the Wianamatta Group (Rw), with Quaternary Alluvium (Qa) exposed along Navigation Creek and its tributaries in the northern part of the Study Area.

### 2.1. Definition of the Study Area

The *Study Area* is defined as the surface area that could be affected by the extraction of the proposed LW709 to LW711 and LW905. Two areas have been considered in this report, being the *Study Area based on the 35° angle of draw and predicted 20 mm subsidence contour* and the *Study Area based on the 600 m boundary*.

The Study Area based on the 35° angle of draw and predicted 20 mm subsidence contour represents the minimum extent for the assessments for the conventional ground movements, i.e. vertical subsidence and its associated effects. Low level conventional ground movements can extend beyond this area. The natural and built features located outside this area, which could experience these low level movements and could be sensitive to these movements, have also been included in the assessments provided in this report.

The Study Area based on the 600 m boundary represents the minimum extent of the assessments for the valley-related effects. This distance is based on the recommendations from the Southern Coalfield Inquiry (DPIE, 2008) for the risk management zones. The natural and built features located outside the 600 m boundary, which could experience valley-related effects and could be sensitive to these movements, have also been included in the assessments provided in this report.

The extent of the Study Area based on the 35° angle of draw and predicted 20 mm subsidence contour has been calculated by combining the areas bounded by the following limits:

- The 35° angle of draw line from the extent of the proposed LW709 to LW711 and LW905; and
- The predicted limit of vertical subsidence, taken as the additional 20 mm subsidence contour, due to the mining of LW709 to LW711 and LW905 only.

The depths of cover contours for the Bulli Seam are shown in Drawing No. MSEC1117-05. The depths of cover directly above the proposed LW709 to LW711 and LW905 vary 530 m and 750 m. The 35° angle of draw line, therefore, has been determined by drawing a line that is a horizontal distance varying between 370 m and 525 m around the extents of the proposed longwall voids.

The predicted limit of vertical subsidence, taken as the predicted additional 20 mm subsidence contour due to the mining of LW709 to LW711 and LW905, has been determined using the calibrated Incremental Profile Method (IPM), which is described in Chapter 3. The predicted additional subsidence contours, including the 20 mm subsidence contour, are shown in Drawing No. MSEC1117-15, in Appendix E.

The predicted additional 20 mm subsidence contour extends beyond the 35° angle of draw above the previous longwalls in Areas 7 and 9. Elsewhere, the contour is located inside the angle of draw. The Study Area based on the 35° angle of draw line and predicted 20 mm subsidence contour is shown in Drawing No. MSEC1117-01, in Appendix E.

The features that are located within 600 m of the proposed LW709 to LW711 and LW905 (referred to as the 600 m boundary) and are predicted to experience valley-related effects and could be sensitive to these movements have been included in the assessments provided in this report. These features include streams, cliffs and Aboriginal heritage sites.

There are additional features that are located outside the 600 m boundary that could experience either far-field horizontal or valley-related effects. The surface features that could be sensitive to such movements have been identified and have also been included in the assessments provided in this report. These features include the road bridges and survey control marks.

### 2.2. Natural and built features within the Study Area

A summary of the natural and built features located within the Study Area is provided in Table 2.1. The locations of these features are shown in Drawings Nos. MSEC1117-07 to MSEC1117-14, in Appendix E. The descriptions, predictions and impact assessments for the natural and built features are provided in Chapters 5 and 6. The section number references are provided in Table 2.1.

**Table 2.1 Natural and built features within the Study Area**

Item	Within Study Area	Section number reference	Item	Within Study Area	Section number reference
<b>NATURAL FEATURES</b>			<b>FARM LAND AND FACILITIES</b>		
Catchment Areas or Declared Special Areas	✓	5.1	Agricultural Utilisation or Agricultural Suitability of Farm Land	✓	6.12
Rivers or Creeks	✓	5.2 & 5.3	Farm Buildings or Sheds	✓	6.13
Aquifers or Known Groundwater Resources	✓	5.4	Tanks	✓	6.14
Springs	x		Gas or Fuel Storages	x	
Sea or Lake	x		Poultry Sheds	x	
Shorelines	x		Glass Houses	x	
Natural Dams	x		Hydroponic Systems	x	
Cliffs or Pagodas	✓	5.5 & 5.6	Irrigation Systems	x	
Steep Slopes	✓	5.7	Fences	✓	6.15
Escarpments	✓	5.8	Farm Dams	✓	6.16
Land Prone to Flooding or Inundation	x		Wells or Bores	✓	6.17
Swamps, Wetlands or Water Related Ecosystems	✓	5.10	Any Other Farm Features	x	
Threatened or Protected Species	✓	5.11	<b>INDUSTRIAL, COMMERCIAL AND BUSINESS ESTABLISHMENTS</b>		
National Parks	x		Factories	x	
State Forests	x		Workshops	x	
State Conservation Areas	x		Business or Commercial Establishments or Improvements	✓	6.18
Natural Vegetation	✓	5.11	Gas or Fuel Storages or Associated Plants	x	
Areas of Significant Geological Interest	x		Waste Storages or Associated Plants	x	
Any Other Natural Features Considered Significant	x		Buildings, Equipment or Operations that are Sensitive to Surface Movements	x	
<b>PUBLIC UTILITIES</b>			Surface Mining (Open Cut) Voids or Rehabilitated Areas	x	
Railways	✓	6.1	Mine Infrastructure Including Tailings Dams or Emplacement Areas	x	
Roads (All Types)	✓	6.2 & 6.3	Any Other Industrial, Commercial or Business Features	x	
Bridges	✓	6.4 & 6.5	<b>AREAS OF ARCHAEOLOGICAL OR HERITAGE SIGNIFICANCE</b>		
Tunnels	x			✓	6.20
Culverts	✓	6.6	<b>ITEMS OF ARCHITECTURAL SIGNIFICANCE</b>		
Water, Gas or Sewerage Infrastructure	✓	6.7 to 6.9		x	
Liquid Fuel Pipelines	x		<b>PERMANENT SURVEY CONTROL MARKS</b>		
Electricity Transmission Lines or Associated Plants	✓	6.10		✓	6.21
Telecommunication Lines or Associated Plants	✓	6.11	<b>RESIDENTIAL ESTABLISHMENTS</b>		
Water Tanks, Water or Sewage Treatment Works	x		Houses	✓	6.22
Dams, Reservoirs or Associated Works	x		Flats or Units	x	
Air Strips	x		Caravan Parks	x	
Any Other Public Utilities	x		Retirement or Aged Care Villages	x	
<b>PUBLIC AMENITIES</b>			Associated Structures such as Workshops, Garages, On-Site Waste Water Systems, Water or Gas Tanks, Swimming Pools or Tennis Courts	✓	6.23
Hospitals	x		Any Other Residential Features	x	
Places of Worship	x		<b>ANY OTHER ITEM OF SIGNIFICANCE</b>		
Schools	x			x	
Shopping Centres	x		<b>ANY KNOWN FUTURE DEVELOPMENTS</b>		
Community Centres	x			x	
Office Buildings	x				
Swimming Pools	x				
Bowling Greens	x				
Ovals or Cricket Grounds	x				
Race Courses	x				
Golf Courses	x				
Tennis Courts	x				
Any Other Public Amenities	x				

### 3.1. Introduction

The following sections provide overviews of conventional and non-conventional mine subsidence effects and the methods that have been used to predict these movements. Further information is also provided in the background reports entitled *Introduction to Longwall Mining and Subsidence* and *General Discussion on Mine Subsidence Ground Movements* which can be obtained from [www.minesubsidence.com](http://www.minesubsidence.com).

### 3.2. Overview of conventional subsidence effects

The normal ground movements resulting from the extraction of longwalls are referred to as conventional or systematic subsidence movements. These movements are described by the following parameters:

- **Subsidence** usually refers to vertical displacement of a point, but subsidence of the ground actually includes both vertical and horizontal displacements. These horizontal displacements in some cases, where the subsidence is small beyond the longwall goaf edges, can be greater than the vertical subsidence. Subsidence is usually expressed in units of *millimetres (mm)*.
- **Tilt** is the change in the slope of the ground as a result of differential subsidence and is calculated as the change in subsidence between two points divided by the distance between those points. Tilt is, therefore, the first derivative of the subsidence profile. Tilt is usually expressed in units of *millimetres per metre (mm/m)*. A tilt of 1 mm/m is equivalent to a change in grade of 0.1 %, or 1 in 1000.
- **Curvature** is the second derivative of subsidence, or the rate of change of tilt, and is calculated as the change in tilt between two adjacent sections of the tilt profile divided by the average length of those sections. Curvature is usually expressed as the inverse of the **Radius of Curvature** with the units of *1/kilometres (km<sup>-1</sup>)*, but the values of curvature can be inverted, if required, to obtain the radius of curvature, which is usually expressed in *kilometres (km)*.
- **Strain** is the relative differential horizontal movements of the ground. **Normal strain** is calculated as the change in horizontal distance between two points on the ground, divided by the original horizontal distance between them. Strain is typically expressed in units of *millimetres per metre (mm/m)*. **Tensile Strains** occur where the distances between two points increase and **Compressive Strains** occur when the distances between two points decrease. So that ground strains can be compared between different locations, they are typically measured over bay lengths that are equal to the depth of cover between the surface and seam divided by 20.

Whilst mining-induced normal strains are measured along monitoring lines, ground shearing can also occur both vertically and horizontally across the directions of monitoring lines. Most of the published mine subsidence literature discusses the differential ground movements that are measured along subsidence monitoring lines; however, differential ground movements can also be measured across monitoring lines using 3D survey monitoring techniques.

- **Horizontal shear deformation** across monitoring lines can be described by various parameters including horizontal tilt, horizontal curvature, mid-ordinate deviation, angular distortion and shear index. It is not possible, however, to determine the horizontal shear strain across a monitoring line using 2D or 3D monitoring techniques. High deformations along monitoring lines (i.e. normal strains) are generally measured where high deformations have been measured across the monitoring line (i.e. shear deformations), and vice versa.

The **incremental** subsidence, tilts, curvatures and strains are the additional effects which result from the extraction of each longwall. The **cumulative** subsidence, tilts, curvatures and strains are the accumulated effects which result from the extraction of a series of longwalls. The **total** subsidence, tilts, curvatures and strains are the final effects at the completion of a series of longwalls. The **travelling** tilts, curvatures and strains are the transient movements as the longwall extraction face mines directly beneath a given point.

### 3.3. Far-field movements

The measured horizontal movements at survey marks which are located beyond the longwall goaf edges and over solid unmined coal areas are often much greater than the observed vertical movements at those marks. These movements are often referred to as *far-field movements*.

Far-field horizontal movements tend to be bodily movements towards the extracted goaf area and are accompanied by very low levels of strain. These movements generally do not result in impacts on natural features or built environments, except where they are experienced by large structures which are very sensitive to differential horizontal movements.

In some cases, higher levels of far-field horizontal movements have been observed where steep slopes or surface incisions exist nearby, as these features influence both the magnitude and the direction of ground movement patterns. Similarly, increased horizontal movements are often observed around sudden changes in geology or where blocks of coal are left between longwalls or near other previously extracted series of longwalls. In these cases, the levels of observed subsidence can be slightly higher than normally predicted, but these increased movements are generally accompanied by very low levels of tilt and strain.

Far-field horizontal movements and the method used to predict such movements are described further in Section 4.5.

### **3.4. Overview of non-conventional subsidence movements**

Conventional subsidence profiles are typically smooth in shape and can be explained by the expected caving mechanisms associated with overlying strata spanning the extracted void. Normal conventional subsidence movements due to longwall extraction are easy to identify where longwalls are regular in shape, the extracted coal seams are relatively uniform in thickness, the geological conditions are consistent and surface topography is relatively flat.

As a general rule, the smoothness of the profile is governed by the depth of cover and lithology of the overburden, particularly the near-surface strata layers. Where the depth of cover is high, say greater than 400 m, the observed subsidence profiles along monitoring lines are generally smooth. Where the depth of cover is shallow, say less than 100 m, the observed subsidence profiles along monitoring lines are generally irregular. Very irregular subsidence movements are observed with much higher tilts, curvatures and strains at very shallow depths of cover where the collapsed zone above the extracted longwalls extends up to or near to the surface.

Irregular subsidence movements are occasionally observed at the deeper depths of cover along an otherwise smooth subsidence profile. The cause of these irregular subsidence movements can be associated with:

- sudden or abrupt changes in geological conditions;
- steep topography; and
- valley-related mechanisms.

Non-conventional movements due to geological conditions, steep topography and valley-related effects are discussed in the following sections.

#### **3.4.1. Non-conventional subsidence movements due to changes in geological conditions**

Most non-conventional ground movements are a result of the reaction of near-surface strata to increased horizontal compressive stresses due to mining operations. Some of the geological conditions that influence these irregular subsidence movements are the blocky nature of near-surface sedimentary strata layers and the presence of unknown faults, dykes or other geological structures, cross bedded strata, thin and brittle near-surface strata layers and pre-existing natural joints. The presence of these geological features near the surface can result in a bump in an otherwise smooth subsidence profile and these bumps are usually accompanied by locally increased tilts, curvatures and strains.

Even though it may be possible to explain most observed non-conventional ground movements, there remain some observed irregular ground movements that cannot be explained with available information. The term “anomaly” is therefore reserved for those non-conventional ground movement cases that were not expected to occur and cannot be explained by any of the above possible causes.

It is not possible to predict the locations and magnitudes of non-conventional anomalous movements. In some cases, approximate predictions for the non-conventional ground movements can be made where the underlying geological or topographic conditions are known in advance. It is expected that these methods will improve as further knowledge is gained through ongoing research and investigation.

In this report, non-conventional ground movements are being included statistically in the predictions and impact assessments, by basing these on the frequency of past observed occurrence of both the conventional and non-conventional ground movements and impacts. The analysis of strains provided in Section 4.3 includes those resulting from both conventional and non-conventional anomalous movements. The impact assessments for the natural and built features, which are provided in Chapters 5 and 6, include historical impacts resulting from previous longwall mining which have occurred as the result of both conventional and non-conventional subsidence movements.

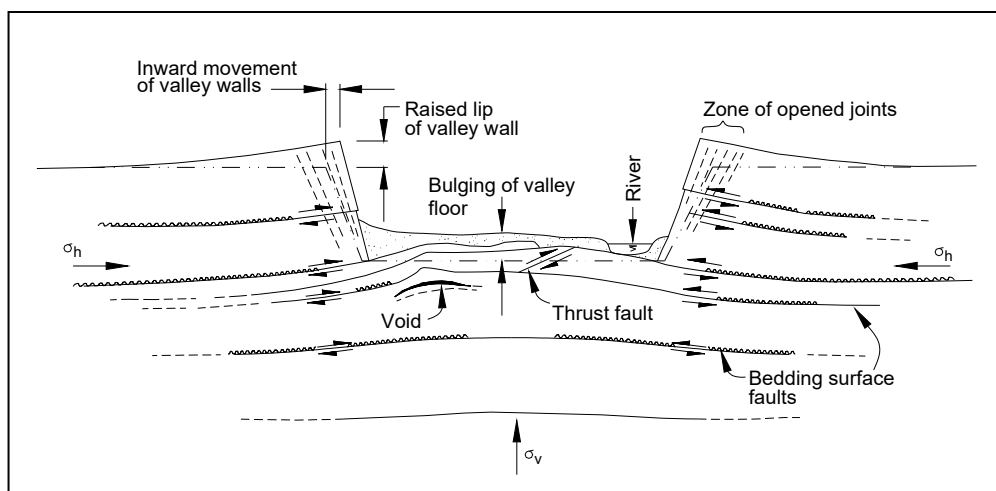
### 3.4.2. Non-conventional subsidence movements due to steep topography

Non-conventional movements can also result from increased horizontal movements in the downslope direction where longwalls are extracted beneath steep slopes. In these cases, elevated tensile strains develop near the tops and on the sides of the steep slopes and elevated compressive strains develop near the bases of the steep slopes. The potential impacts resulting from the increased horizontal movements include the development of tension cracks at the tops and on the sides of the steep slopes and compression ridges at the bottoms of the steep slopes.

Further discussions on the potential for downslope movements for the steep slopes within the Study Area are provided in Section 5.7.

### 3.4.3. Valley-related effects

The streams within the Study Area will be affected by valley-related effects, which are commonly observed in the Southern Coalfield. Valley bulging movements are a natural phenomenon, resulting from the formation and ongoing development of the valley, as illustrated in Fig. 3.1. The potential for these natural movements is influenced by the geomorphology of the valley.



**Fig. 3.1 Valley formation in flat-lying sedimentary rocks (after Patton and Hendren 1972)**

Valley-related effects can be caused by or accelerated by mine subsidence as the result of a number of factors, including the redistribution of horizontal in situ stresses and downslope movements. Valley-related effects are normally described by the following parameters:

- **Upsidence** is the reduced subsidence, or the relative uplift within a valley which results from the dilation or buckling of near-surface strata at or near the base of the valley. The magnitude of upsidence, which is typically expressed in the units of *millimetres (mm)*, is the difference between the observed subsidence profile within the valley and the conventional subsidence profile which would have otherwise been expected in flat terrain.
- **Closure** is the reduction in the horizontal distance between the valley sides. The magnitude of closure, which is typically expressed in the units of *millimetres (mm)*, is the greatest reduction in horizontal distance between any two points on the opposing valley sides.
- **Compressive Strains** occur within the bases of valleys as a result of valley closure and upsidence movements. **Tensile Strains** also occur in the sides and near the tops of the valleys as a result of valley closure movements. The magnitudes of these strains, which are typically expressed in the units of *millimetres per metre (mm/m)*, are calculated as the changes in horizontal distance over a standard bay length, divided by the original bay length.

The predicted valley-related effects for the streams above and near the existing longwalls in Appin Areas 7 and 9 were determined using the empirical method outlined in ACARP Research Project No. C9067 (Waddington and Kay, 2002), referred to as the 2002 ACARP method.

More recently, the empirical prediction method has been refined based on further research undertaken as part of ACARP Research Project No. C18015 (Kay and Waddington, 2014), referred to as the 2014 ACARP method. This method only provides predictions for valley closure and not for upsidence.

The predictions based on the 2002 ACARP method can be directly compared with the predictions provided in previous MSEC subsidence reports for the approved and completed longwalls in Appin Areas 7 and 9 and with other case studies. The assessments provided in this report, therefore, have been based on the predictions obtained using the 2002 ACARP method.

The predicted strains resulting from valley-related effects for the streams in the Study Area have been determined using the ground monitoring data for longwalls that have previously mined beneath or near to streams in the Southern Coalfield, including at Appin Colliery. Refer to the impact assessments for the streams in Chapter 5 for further details.

Further details can be obtained from the background report entitled *General Discussion on Mine Subsidence Ground Movements* which can be obtained at [www.minesubsidence.com](http://www.minesubsidence.com).

### 3.5. The Incremental Profile Method

The predicted conventional subsidence effects for the existing and proposed longwalls have been determined using the Incremental Profile Method (IPM), which has been developed by MSEC. The method is an empirical model based on a large database of observed monitoring data from previous mining within the Southern, Newcastle, Hunter and Western Coalfields of NSW.

The database consists of detailed subsidence monitoring data from collieries in NSW including: Angus Place, Appin, Baal Bone, Bellambi, Beltana, Blakefield South, Bulli, Chain Valley, Clarence, Coalcliff, Cooranbong, Cordeaux, Corrimal, Cumnock, Dartbrook, Delta, Dendrobium, Eastern Main, Ellalong, Fernbrook, Glennies Creek, Gretley, Invincible, John Darling, Kemira, Lambton, Liddell, Mandalong, Metropolitan, Mt. Kembla, Munmorah, Nardell, Newpac, Newstan, Newvale, Newvale 2, South Bulga, South Bulli, Springvale, Stockton Borehole, Teralba, Tahmoor, Tower, Wambo, Wallarah, Western Main, Ulan, United, West Cliff, West Wallsend, and Wyee.

The database consists of the measured incremental subsidence profiles, which are the additional subsidence profiles resulting from the extraction of each longwall within a series of longwalls. It can be seen from the normalised incremental subsidence profiles within the database, that the observed shapes and magnitudes are reasonably consistent where the mining geometry and local geology are similar.

Subsidence predictions made using the IPM use the database of measured incremental subsidence profiles, the longwall geometries, local surface and seam information and geology. The method tends to over-predict the conventional subsidence effects (i.e. is slightly conservative) where the mining geometry and geology are within the range of the empirical database. The predictions can be further tailored to local conditions where observed monitoring data is available close to the mining area.

Further details on the IPM are provided in the background report entitled *General Discussion on Mine Subsidence Ground Movements* which can be obtained from [www.minesubsidence.com](http://www.minesubsidence.com).

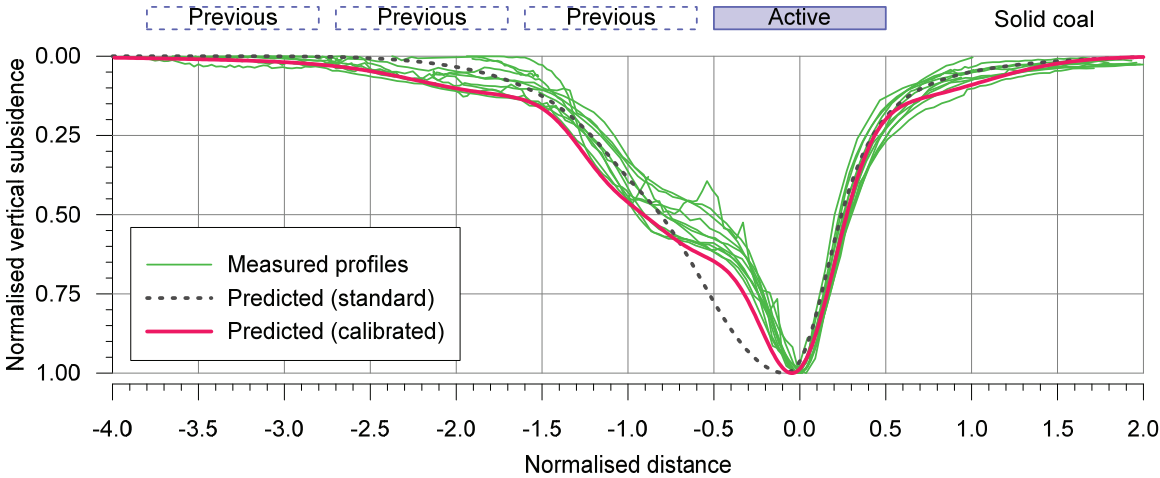
### 3.6. Calibration of the IPM

The use of the IPM at the Appin Colliery has been continually reviewed and refined based on the latest available ground movement monitoring data. The subsidence model has been reviewed after the completion of each longwall as part of the End of Panel reports.

The subsidence predictions for LW701 to LW704, LW705 to LW710 and LW901 to LW904 are provided in Reports Nos. MSEC209 (Rev. E), MSEC342 (Rev. C) and MSEC448 (Rev. B), respectively. The predictions for these approved longwalls were obtained using the standard IPM for the Southern Coalfield based on monitoring data from the Bulli Seam, including from Appin, Tahmoor, Tower and West Cliff Collieries.

The standard IPM has been reviewed based on the latest monitoring data from Appin Colliery. The data from Area 7 include the ARTC monitoring line, Early Warning monitoring line, HW2 East monitoring line, HW2 West monitoring line, Menangle Road monitoring line, Moreton Park Road monitoring line and Telstra monitoring line. The data from Area 9 include the ARTC monitoring line and Menangle Road monitoring line.

The measured normalised incremental profiles for the monitoring lines are shown as the green lines in Fig. 3.2. The incremental profiles represent the additional movements due to the mining of each longwall projected onto a line transverse to the active longwall. The magnitudes have been normalised by dividing by the maximum value so that the profile varies between zero and 1.0. The distances have been normalised by dividing by the width of the active longwall, with zero representing its centreline. The longitudinal and oblique monitoring lines have not been included in this figure as the distances cannot be normalised.



**Fig. 3.2 Normalised vertical subsidence versus normalised distance**

The predicted normalised incremental profile obtained using the standard IPM is shown by the dashed grey line in Fig. 3.2. The standard model over-predicts the vertical subsidence above the active longwall tailgate and under-predicts the vertical subsidence above the previous longwalls.

The measured profiles of vertical subsidence comprise a number of components. These include sagging of the overburden strata above the longwall void, compression of the chain pillars, compression of the solid coal abutments, compression of the immediate floor and roof and reactivation of previously mined areas.

A review of the predicted incremental vertical subsidence profile obtained from the standard IPM has found that it over-predicts the component of pillar compression and under-predicts the components of sag subsidence, reactivation of previously mined areas and abutment compression.

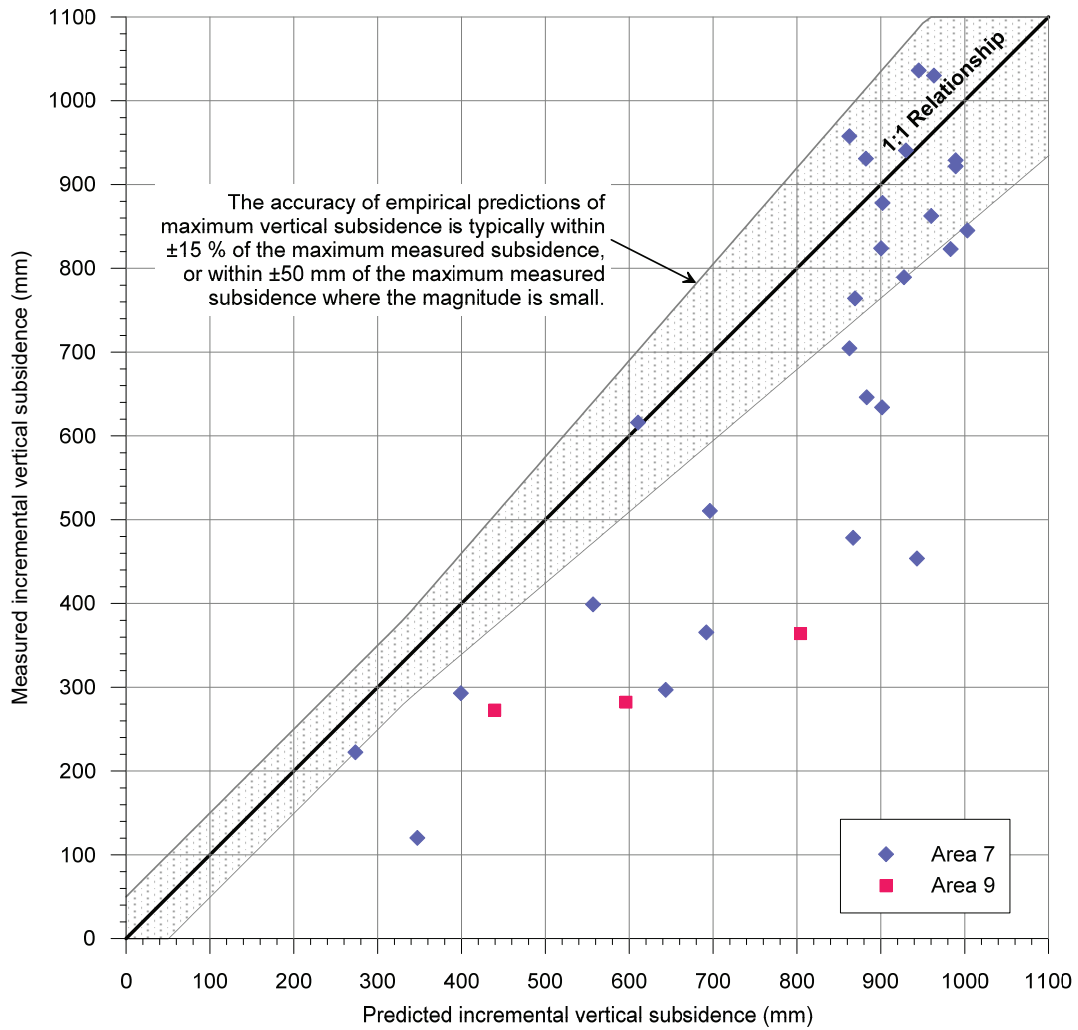
The model has been calibrated using the latest monitoring data from Appin Colliery to improve the predictions of each of the components and, hence, the incremental profile of vertical subsidence. The predicted normalised incremental profile obtained from the calibrated IPM is shown by the solid red line in Fig. 3.2. It is considered that the calibrated model more closely matches the measured movements when compared with the standard IPM.

A comparison between the maximum measured and maximum predicted incremental vertical subsidence for the monitoring lines at Appin Colliery is provided in Fig. 3.3. The predictions are based on the standard IPM. The measured and predicted values represent the additional movements due to the mining of each longwall.

The measured and predicted values are reasonably similar (i.e. close to the 1:1 relationship) at the upper end of the range (i.e. top-right corner of Fig. 3.3). The standard IPM provides more conservative predictions where the measured values are towards the lower end of the range (i.e. bottom of Fig. 3.3). The measured vertical subsidence in Area 7 is also generally greater than the measured vertical subsidence in Area 9. This could be partly due to the less extensive monitoring above Area 9.

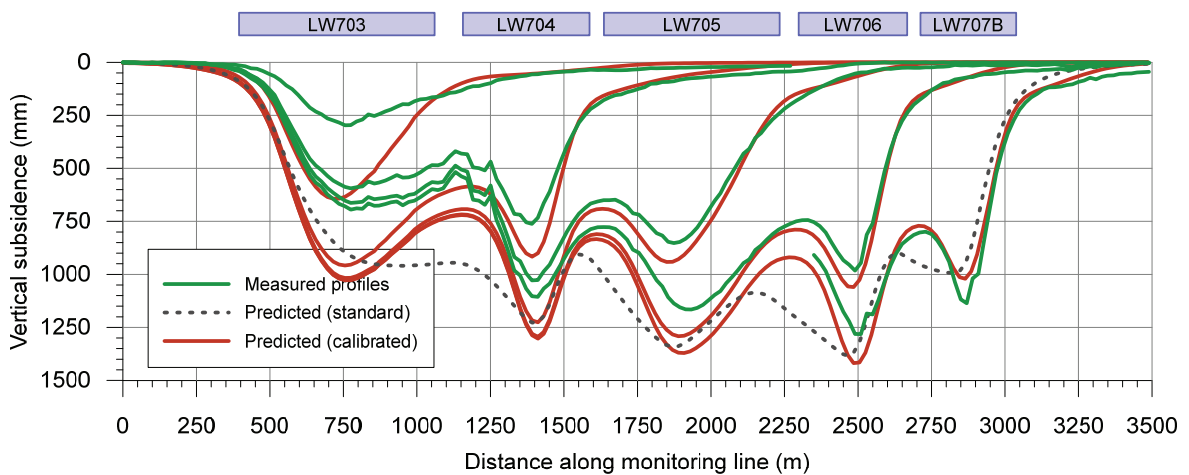
The maximum measured incremental vertical subsidence values are less than the maximum predicted incremental vertical subsidence or are within +15 % or +50 mm of the maximum predicted values. This is considered to be in the accuracy of subsidence prediction methodologies (DeBono *et al.*, 2014).

The calibrated IPM therefore has not been modified for the prediction of maximum incremental vertical subsidence. That is, the prediction curves for maximum incremental vertical subsidence for the calibrated IPM are the same as the standard model.

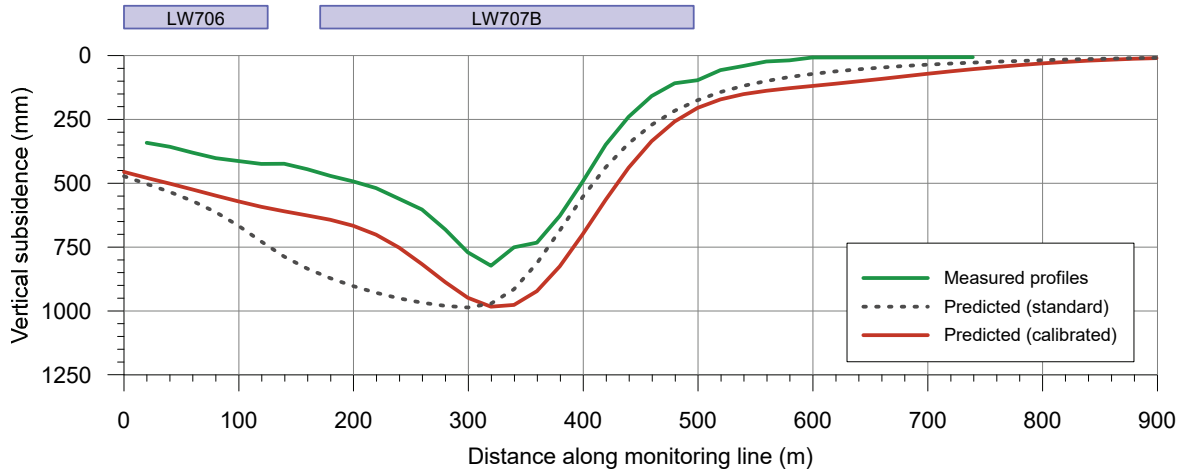


**Fig. 3.3 Comparison of maximum measured and maximum predicted (standard IPM) incremental vertical subsidence for the monitoring lines at Appin Colliery**

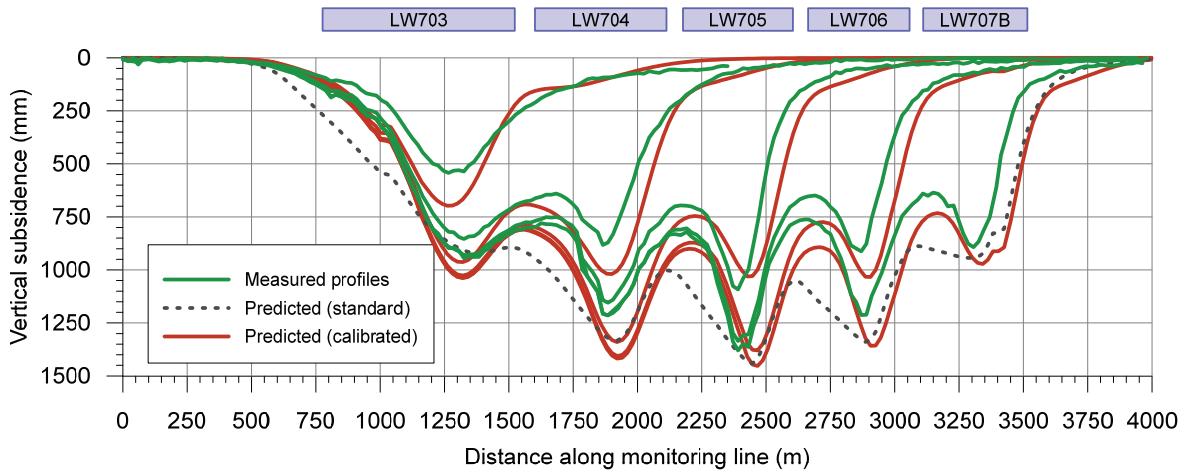
Comparisons of the measured and predicted profiles of total vertical subsidence are provided along the: ARTC monitoring line in Fig. 3.4; Early Warning monitoring line in Fig. 3.5; HW2 East monitoring line in Fig. 3.6; HW2 West monitoring line in Fig. 3.7; Moreton Park Road monitoring line in Fig. 3.8; and the Telstra monitoring line in Fig. 3.9.



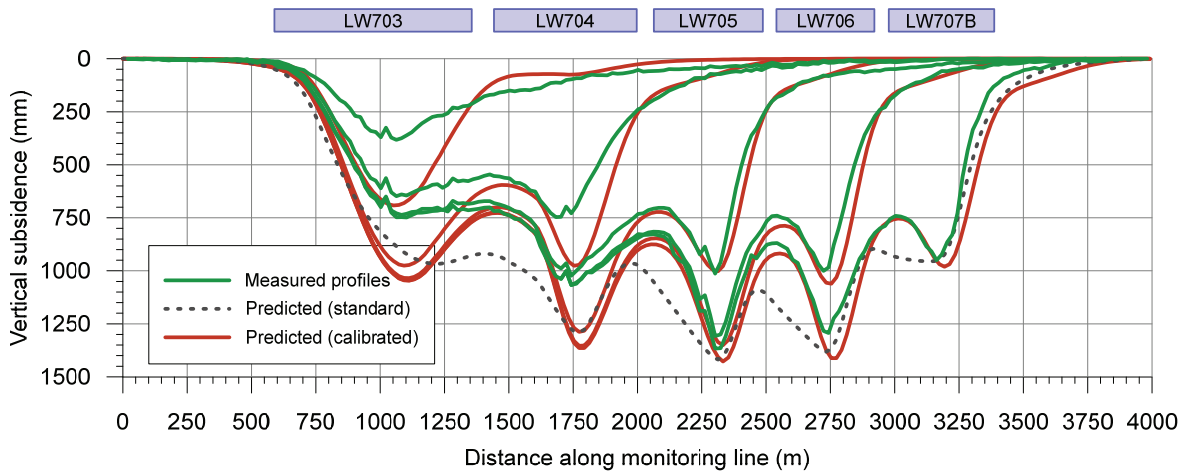
**Fig. 3.4 Measured and predicted total vertical subsidence for the ARTC monitoring line**



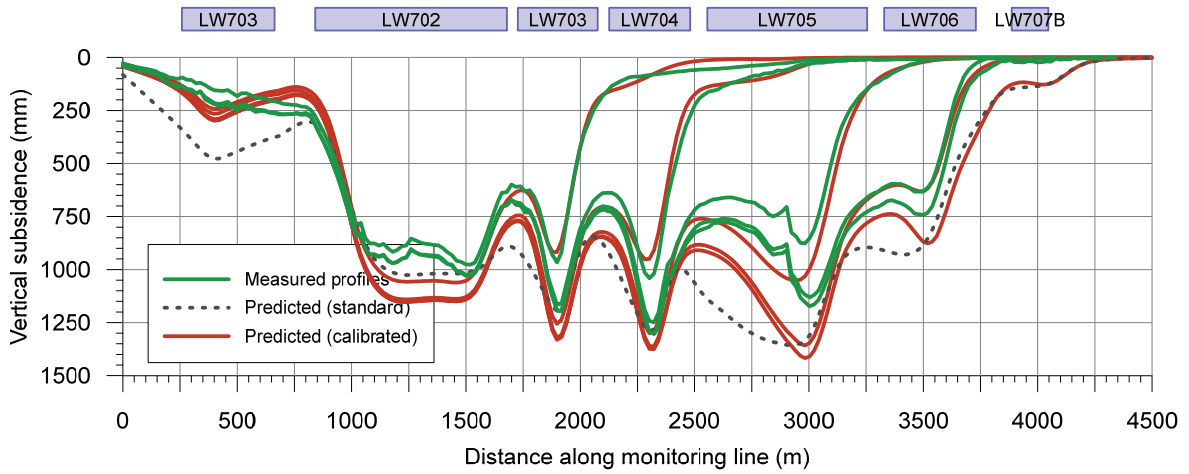
**Fig. 3.5 Measured and predicted total vertical subsidence for the Early Warning monitoring line**



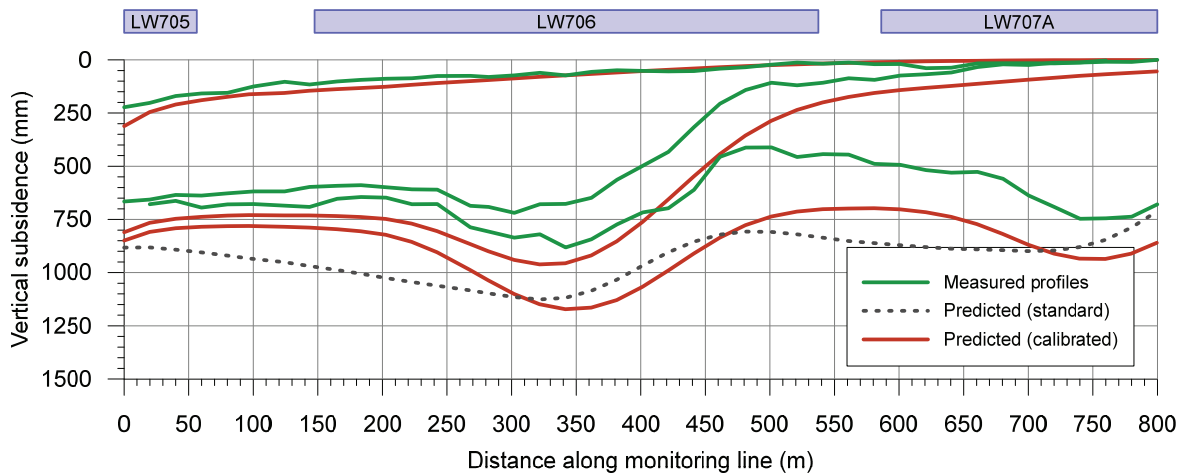
**Fig. 3.6 Measured and predicted total vertical subsidence for the HW2 East monitoring line**



**Fig. 3.7 Measured and predicted total vertical subsidence for the HW2 West monitoring line**



**Fig. 3.8 Measured and predicted total vertical subsidence for the Moreton Park Road monitoring line**



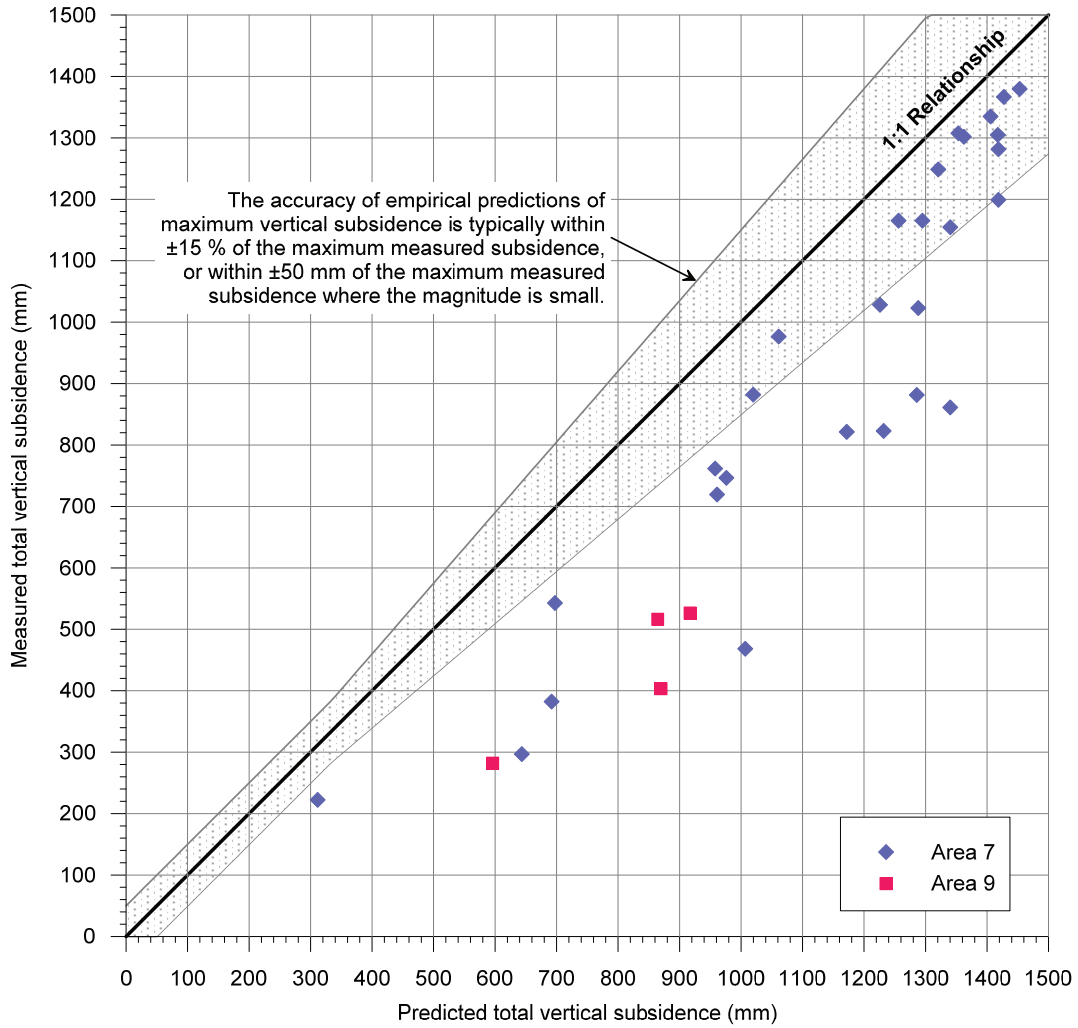
**Fig. 3.9 Measured and predicted total vertical subsidence for the Telstra monitoring line**

It is considered that the calibrated IPM provides reasonable predictions for the monitoring lines at Appin Colliery. The profiles more closely match the maximum measured vertical subsidence above each of the longwalls and the lesser vertical subsidence above each of the chain pillars.

### 3.7. Reliability of the calibrated IPM

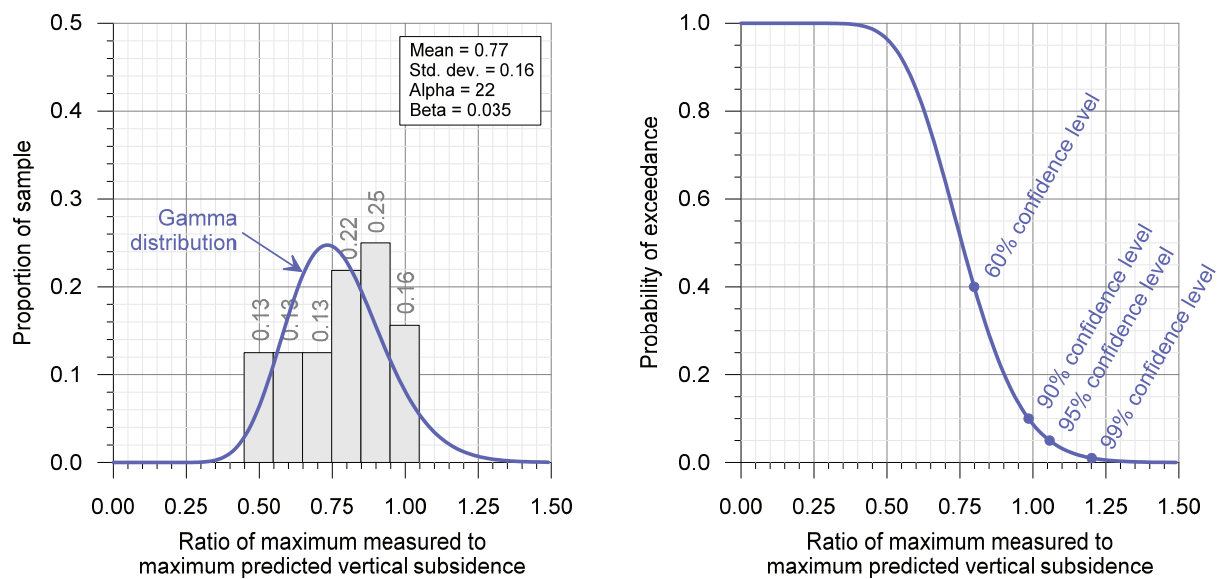
A comparison between the maximum measured and maximum predicted total vertical subsidence for the monitoring lines at Appin Colliery is provided in Fig. 3.10. The predictions are based on the calibrated IPM. The measured and predicted values represent the total movements after the mining of each longwall.

The maximum measured total vertical subsidence values are less than the maximum predicted total values.



**Fig. 3.10 Comparison of maximum measured and maximum predicted (calibrated IPM) total vertical subsidence for the monitoring lines at Appin Colliery**

The distribution of the ratio of the maximum measured to maximum predicted total vertical subsidence for the monitoring lines is illustrated on the left-side of Fig. 3.11. A gamma distribution has been fitted to the data and this is also shown on the left-side of this figure. The probabilities of exceedance based on the fitted gamma distribution are shown in the right-side of Fig. 3.11.



**Fig. 3.11 Distribution of the ratio of maximum measured to maximum predicted total vertical subsidence for the monitoring lines at Appin Colliery**

The ratios of maximum measured to maximum predicted total vertical subsidence vary between 0.46 and 0.97. That is, the maximum measured total vertical subsidence varies between 46 % and 97 % of the maximum predicted total values based on the calibrated IPM.

The mean ratio of the maximum measured to maximum predicted total vertical subsidence for the monitoring lines is 0.77. The maximum measured vertical subsidence therefore is on average 77 % of the maximum predicted values based on the calibrated IPM.

The 90 % confidence level represents a ratio of maximum measured to maximum predicted total vertical subsidence of approximately 1.0. That is, there is a probability of approximately 10 % that the maximum measured total vertical subsidence will exceed the maximum predicted value based on the calibrated IPM.

The 95 % and 99 % confidence levels represent ratios of maximum measured to maximum predicted total vertical subsidence of 1.06 and 1.20, respectively. That is, there are probabilities of 5 % and 1 % that the maximum measured total vertical subsidence will exceed the maximum predicted values by 6 % and 20 %, respectively.

A comparison of the mining geometry for the proposed LW709 to LW711 and LW905 with that for the existing and approved longwalls in Appin Areas 7 and 9 is provided in Table 3.1.

**Table 3.1 Comparison of the mine geometry for the longwalls at Appin Colliery**

Parameter	Proposed LW709 to LW711 and LW905		LW702 to LW708 and LW901 to LW904	
	Range	Average	Range	Average
Longwall width	325	325	305 / 325	320
Depth of cover	530 ~ 750	615	495 ~ 610	545
W/H ratio	0.44 ~ 0.61	0.53	0.50 ~ 0.64	0.59
Extraction height	2.8 ~ 3.3	3.1	2.4 ~ 3.4	3.0

The void width for each of the proposed LW709 to LW711 and LW905 of 325 m is the same as the void width for each of the existing and approved LW702 to LW708B and LW904. However, LW901 to LW903 have a narrower void width of 305 m. The comparisons of measured and predicted vertical subsidence found that the calibrated IPM is more conservative for the narrower longwalls in Area 9.

The average depth of cover for the proposed LW709 to LW711 and LW905 of 615 m is greater than the average value for LW702 to LW708B and LW901 to LW904 of 545 m. The depths of cover above the western ends of the proposed longwalls are greater than the existing and approved longwalls, as they mine further beneath Razorback Range.

The average width-to-depth ratio for the proposed LW709 to LW711 and LW905 of 0.53 is therefore less than the average ratio for LW702 to LW708B and LW901 to LW904 of 0.59. The predicted vertical subsidence for the proposed longwalls is therefore less than that for the existing and approved longwalls. However, this difference is small since the longwalls are all in a similar part of the subcritical range.

The average mining heights for the proposed LW709 to LW711 and LW905 of 3.1 m is similar to but slightly greater than the average mining height for LW702 to LW708B and LW901 to LW904 of 3.0 m. However, there is a narrow range of mining heights for the proposed longwalls when compared with the existing and approved longwalls.

It is considered appropriate, therefore, to adopt the calibrated IPM for the proposed LW709 to LW711 and LW905. It is anticipated that the calibrated model will provide reasonable, if not conservative predictions for the proposed longwalls.

#### 4.1. Introduction

The following sections provide the maximum predicted conventional subsidence effects due to the mining of the proposed LW709 to LW711 and LW905. The predicted subsidence effects and the impact assessments for the natural and built features are provided in Chapters 5 and 6.

The predicted vertical subsidence, tilt and curvature have been obtained using the IPM, which has been calibrated based on the latest monitoring data from Appin Colliery, as described in Section 3.6. The predicted strains have been determined by analysing the strains measured at Appin Colliery and other nearby collieries, where the longwall width-to-depth ratios and extraction heights are similar to those for the existing and proposed longwalls.

The maximum predicted subsidence effects and the predicted subsidence contours provided in this report describe the conventional movements and do not include the valley-related upsidence and closure movements, nor the effects of faults and other geological structures. Such effects have been addressed separately in the impact assessments for each feature provided in Chapters 5 and 6.

#### 4.2. Maximum predicted conventional subsidence, tilt and curvature

A summary of the maximum predicted values of incremental conventional vertical subsidence, tilt and curvature due to the mining of LW709 to LW711 and LW905 is provided in Table 4.1. The incremental values represent the additional movements due to the mining of each longwall only.

**Table 4.1 Maximum predicted incremental vertical subsidence, tilt and curvature due to the mining of LW709 to LW711 and LW905**

Due to longwall	Maximum predicted incremental vertical subsidence (mm)	Maximum predicted incremental tilt (mm/m)	Maximum predicted incremental hogging curvature (km <sup>-1</sup> )	Maximum predicted incremental sagging curvature (km <sup>-1</sup> )
LW709	950	6.5	0.07	0.14
LW710A	425	2.5	0.02	0.05
LW710B	925	6.5	0.06	0.12
LW711	950	6.5	0.07	0.14
LW905	650	4.5	0.04	0.08

The predicted additional vertical subsidence contours due to the mining of LW709 to LW711 and LW905 are shown in Drawing No. MSEC1117-15, in Appendix E. These contours represent the additional movements due to the mining of the proposed longwalls only, i.e. after the completion of LW708B and LW904.

The predicted total vertical subsidence contours due to the mining in Appin Areas 7 and 9 are shown in Drawing No. MSEC1117-16, in Appendix E. These contours represent the total movements due to the mining of the existing, approved and proposed longwalls in Areas 7 and 9.

Summaries of the maximum predicted values of total vertical subsidence, tilt and curvature are provided in Table 4.2 for Area 7 and Table 4.3 for Area 9. The total values represent the accumulated movements within the Study Area due to the extraction of the existing, approved and proposed longwalls.

**Table 4.2 Maximum predicted total vertical subsidence, tilt and curvature within the Study Area resulting from the mining of LW702 to LW711**

After longwalls	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
LW702 to LW708B	1450	7.5	0.07	0.15
LW709	1550	7.0	0.07	0.15
LW710A	1550	7.0	0.07	0.15
LW710B	1550	6.5	0.07	0.15
LW711	1550	7.0	0.08	0.15

**Table 4.3 Maximum predicted total vertical subsidence, tilt and curvature within the Study Area resulting from the mining of LW901 to LW905**

After longwalls	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
LW901 to LW904	1250	6.5	0.07	0.15
LW905	1300	6.5	0.07	0.15

The maximum predicted total vertical subsidence within the Study Area due to the mining of the existing, approved and proposed longwalls is 1550 mm in Area 7 and 1300 mm in Area 9. The maximum vertical subsidence of 1550 mm occurs above LW707B and it represents 46 % of the proposed extraction height of 3.4 m in this location.

The maximum predicted total vertical subsidence directly above the proposed longwalls (and not above the previously extracted longwalls) is 1400 mm above LW709 to LW711 and 800 mm directly above LW905. The maximum predicted subsidence above LW905 is less than the maximum predicted value above LW709 to LW711, as it is the last longwall in the series and therefore it has not developed additional subsidence due to the mining subsequent longwalls in the series.

The maximum predicted total tilt within the Study Area is 7 mm/m (i.e. 0.7 %, or 1 in 143), which occurs adjacent to the maingate of LW711. The maximum predicted total conventional curvatures within the Study Area are 0.08 km<sup>-1</sup> hogging and 0.15 km<sup>-1</sup> sagging, which represent minimum radii of curvature of 13 km and 7 km, respectively.

The predicted conventional subsidence effects vary across the Study Area as the result of, among other factors, variations in the longwall geometry, depths of cover, seam thickness and overburden geology. To illustrate this variation, the predicted profiles of vertical subsidence, tilt and curvature have been determined along two prediction lines. The predicted profiles of total vertical subsidence, tilt and curvature along Prediction Lines 1 and 2 are shown in Figs. C.01 and C.02, respectively, in Appendix C. The locations of these prediction lines are shown in Drawings Nos. MSEC1117-15 and MSEC1117-16.

### 4.3. Predicted strains

The prediction of strain is more difficult than the predictions of subsidence, tilt and curvature. The reason for this is that strain is affected by many factors, including ground curvature and horizontal movement, as well as local variations in the near-surface geology, the locations of pre-existing natural joints in bedrock, and the depth of bedrock. Survey tolerance can also represent a substantial portion of the measured strain, in cases where the strains are of a low order of magnitude. The profiles of observed strain, therefore, can be irregular even when the profiles of observed subsidence, tilt and curvature are relatively smooth.

In previous MSEC subsidence reports, predictions of conventional strain were provided based on the best estimate of the average relationship between curvature and strain. Similar relationships have been proposed by other authors. The reliability of the strain predictions was highlighted in these reports, where it was stated that measured strains can vary considerably from the predicted conventional values.

Adopting a linear relationship between curvature and strain provides a reasonable prediction for the conventional tensile and compressive strains. The locations that are predicted to experience hogging or convex curvature are expected to be net tensile strain zones and locations that are predicted to experience sagging or concave curvature are expected to be net compressive strain zones. In the Southern Coalfield, it has been found that a factor of 15 provides a reasonable relationship between the predicted maximum curvatures and the predicted maximum conventional strains.

The maximum predicted conventional strains resulting due to the mining of LW709 to LW711 and LW905, based on applying a factor of 15 to the maximum predicted curvatures, are 1.2 mm/m tensile and 2.3 mm/m compressive. These strains represent typical values when the ground subsides regularly with no localised or elevated strains due to near-surface geological structures or valley closure effects. The maximum strains can be much greater than these typical values, especially in the locations of near-surface geological structures, on steep slopes or within valleys.

At any point, however, there can be considerable variation from the linear relationship, resulting from non-conventional movements or from the normal scatters which are observed in strain profiles. When expressed as a percentage, observed strains can be many times greater than the predicted conventional strain for low magnitudes of curvature. In this report, therefore, we have provided a statistical approach to account for the variability, rather than just providing a single predicted conventional strain.

The range of potential strains above the proposed longwalls has been determined using monitoring data from the extracted longwalls at Appin Colliery and other nearby collieries, where the mining geometry and overburden geometry are similar. The range of strains measured during the extraction of these longwalls should, therefore, provide a reasonable indication of the range of potential strains for the proposed longwalls.

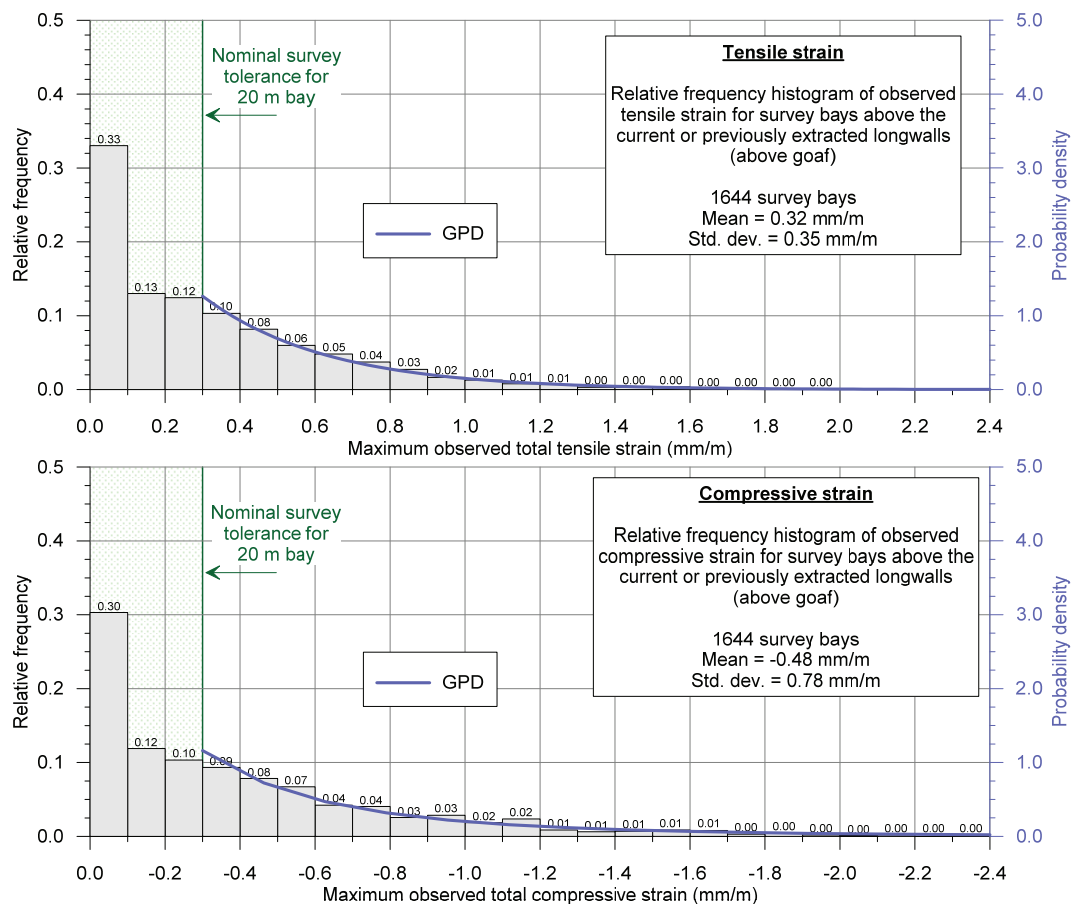
The data used in the analysis of measured strains included those resulting from both conventional and non-conventional anomalous movements but did not include those resulting from valley-related effects, which are addressed separately in this report. The strains resulting from damaged or disturbed survey marks have been excluded.

#### 4.3.1. Analysis of strains measured in survey bays

For features that are in discrete locations, such as building structures, farm dams and heritage sites, it is appropriate to assess the frequency of the maximum strains measured in individual survey bays.

The survey database has been analysed to extract the maximum tensile and compressive strains that have been measured at any time during the extraction of longwalls at the mine, for survey bays that were located directly above goaf or the chain pillars that are located between the extracted longwalls, which has been referred to as “above goaf”.

A histogram of the maximum total tensile and compressive strains measured in survey bays located above goaf is provided in Fig. 4.1. A number of probability distribution functions have been fitted to the empirical data. It was found that a *Generalised Pareto Distribution (GPD)* provided a good fit to the raw strain data, which have also been shown in this figure.



**Fig. 4.1 Distributions of the maximum measured tensile and compressive strains during the extraction of longwalls in the Southern Coalfield for survey bays located directly above goaf**

Confidence levels have been determined from the empirical strain data using the fitted GPDs. In the cases where survey bays have been measured multiple times during a longwall extraction, the maximum tensile strain and the maximum compressive strain have been used in the analysis (i.e. single tensile strain and single compressive strain measurement per survey bay).

A summary of the probabilities of exceedance for tensile and compressive strains for survey bays located above goaf, based on the fitted GPDs, is provided in Table 4.4. The analysis does not include the strains resulting from valley-related effects, which are discussed separately in the impact assessments for the natural and built features provided in Chapters 5 and 6.

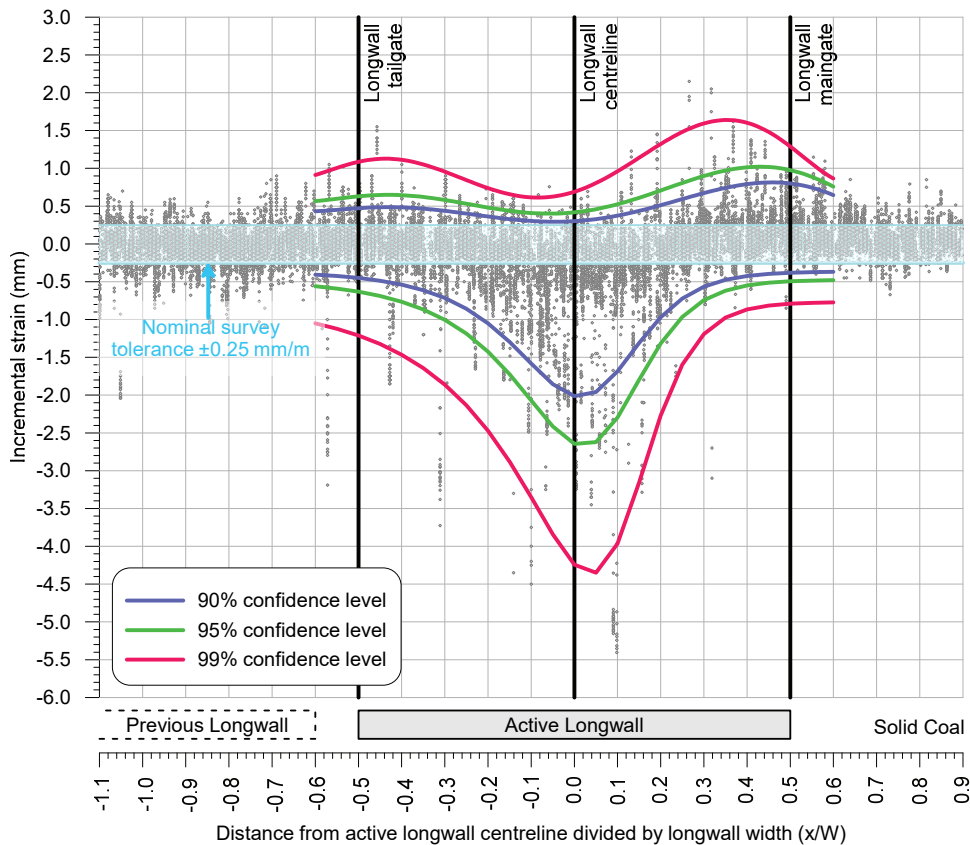
**Table 4.4 Probabilities of exceedance for strain for survey bays located directly above goaf**

Type	Strain (mm/m)	Probability of exceedance
Compression	-8.0	1 in 1300
	-6.0	1 in 550
	-4.0	1 in 170
	-2.0	1 in 30
	-1.0	1 in 8
	-0.5	1 in 3
	-0.3	1 in 2
Tension	+0.3	1 in 2
	+0.5	1 in 4
	+1.0	1 in 20
	+1.5	1 in 90
	+2.0	1 in 430
	+3.0	1 in 2000

The 95 % confidence levels for the maximum total strains that the individual survey bays above goaf experienced at any time during mining are 1.0 mm/m tensile and 1.6 mm/m compressive. The 99 % confidence levels for the maximum total strains that the individual survey bays above goaf experienced at any time during mining are 1.5 mm/m tensile and 3.3 mm/m compressive.

The probabilities for survey bays located above goaf are based on the strains measured anywhere above the extracted longwalls at the mine. As described previously, tensile strains are more likely to develop in the locations of hogging curvature and compressive strains are more likely to develop in the locations of sagging curvature.

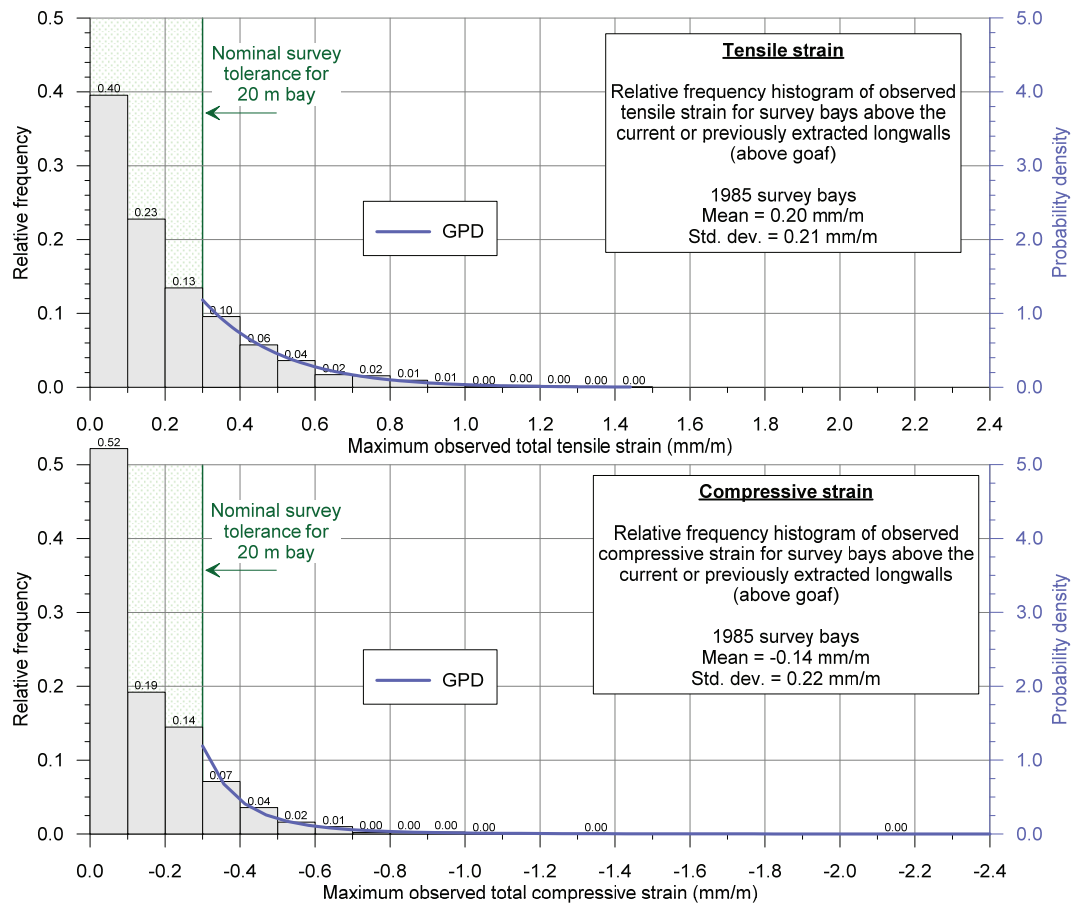
The distribution of incremental strains measured above previously extracted longwalls in the Southern Coalfield is illustrated in Fig. 4.2 (after Barbato, 2017). The distances have been normalised, so that the locations of the measured strains are shown relative to the longwall maingate and tailgate sides. The approximate confidence levels for the incremental tensile and compressive strains are also shown in this figure, to help illustrate the variation in the data.



**Fig. 4.2 Measured incremental strains versus normalised distance from the longwall maingate for extracted longwalls in the Southern Coalfield**

The survey database has also been analysed to extract the maximum tensile and compressive strains that have been measured at any time during the extraction of the longwalls at Appin Colliery and other nearby collieries, for survey bays that were located outside and within 250 m of the nearest longwall goaf edge, which has been referred to as “above solid coal”.

A histogram of the maximum total tensile and compressive strains measured in survey bays located above solid coal is provided in Fig. 4.3. The probability distribution functions, based on the fitted GPDs, have also been shown in this figure.



**Fig. 4.3 Distributions of the maximum measured tensile and compressive strains during the extraction of longwalls in the Southern Coalfield for survey bays located above solid coal**

Confidence levels have been determined from the empirical strain data using the fitted GPDs. In the cases where survey bays have been measured multiple times during a longwall extraction, the maximum tensile strain and the maximum compressive strain have been used in the analysis (i.e. single tensile strain and single compressive strain measurement per survey bay).

A summary of the probabilities of exceedance for tensile and compressive strains for survey bays located above solid coal, based the fitted GPDs, is provided in Table 4.5. The analysis does not include the strains resulting from valley-related effects, which are discussed separately in the impact assessments for the natural and built features provided in Chapters 5 and 6.

**Table 4.5 Probabilities of exceedance for strain for survey bays located above solid coal**

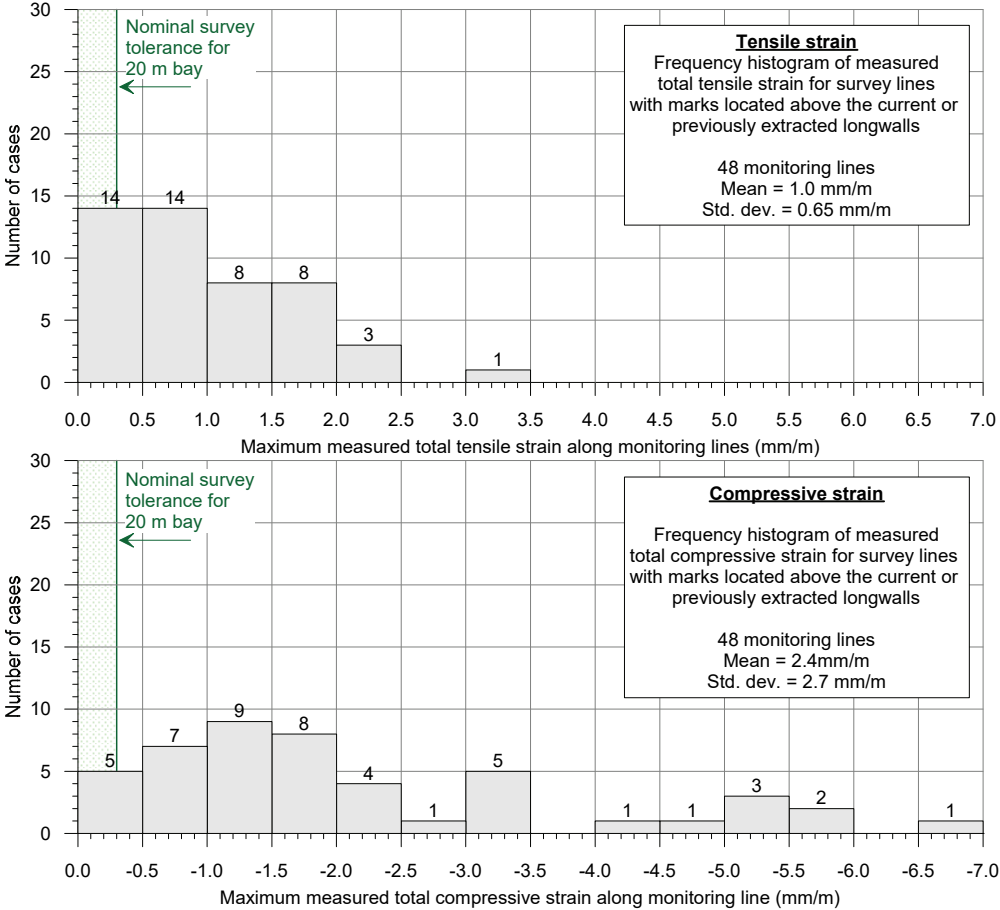
Type	Strain (mm/m)	Probability of exceedance
Compression	-2.0	1 in 2350
	-1.5	1 in 890
	-1.0	1 in 230
	-0.5	1 in 28
	-0.3	1 in 7
Tension	+0.3	1 in 4
	+0.5	1 in 11
	+1.0	1 in 150
	+1.5	1 in 2700

The 95 % confidence levels for the maximum total strains that the individual survey bays above solid coal experienced at any time during mining are 0.6 mm/m tensile and 0.4 mm/m compressive. The 99 % confidence levels for the maximum total strains that the individual survey bays above solid coal experienced at any time during mining are 0.9 mm/m tensile and 0.8 mm/m compressive.

**4.3.2. Analysis of strains measured along whole monitoring lines**

For linear features such as roads, cables and pipelines, it is more appropriate to assess the frequency of the maximum strains measured along whole monitoring lines, rather than for individual survey bays. That is, an analysis of the maximum strains measured anywhere along the monitoring lines, regardless of where the strain occurs.

A histogram of maximum measured total tensile and compressive strains measured anywhere along the monitoring lines, at any time during or after the extraction of longwalls at Appin Colliery and other nearby collieries, is provided in Fig. 4.4.



**Fig. 4.4 Distributions of maximum measured tensile and compressive strains along the monitoring lines during the extraction of longwalls in the Southern Coalfield**

It can be seen from the above figure, that 28 of the 48 monitoring lines (i.e. 58 %) had recorded maximum total tensile strains of 1.0 mm/m or less, and that 44 of the 48 monitoring lines (i.e. 92 %) had recorded maximum total tensile strains of 2.0 mm/m, or less. It can also be seen, that 29 of the 48 monitoring lines (i.e. 60 %) had recorded maximum compressive strains of 2.0 mm/m, or less, and that 39 of the 48 monitoring lines (i.e. 81 %) had recorded maximum compressive strains of 4.0 mm/m, or less.

**4.3.3. Analysis of shear strains**

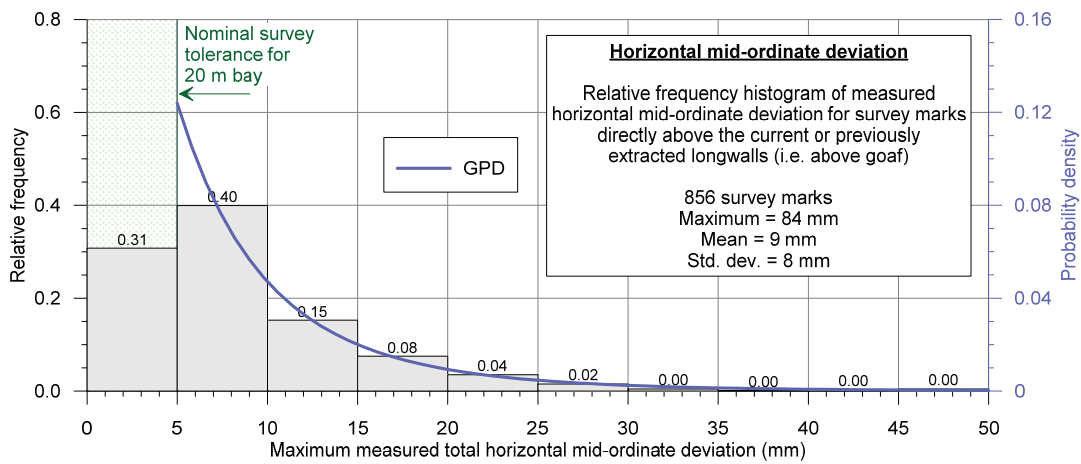
As described in Section 3.2, ground strain comprises two components, being normal strain and shear strain, which can be interrelated using Mohr’s Circle. The magnitudes of the normal strain and shear strain components are, therefore, dependent on the orientation in which they are measured. The maximum normal strains (i.e. principal strains) are those in the direction where the corresponding shear strain is zero.

Normal strains along monitoring lines can be measured using 2D and 3D techniques, by taking the change in horizontal distance between two points on the ground and dividing by the original horizontal distance between them. This provides the magnitude of normal strain along the orientation of the monitoring line and, therefore, this strain may not necessarily be the maximum (i.e. principal) strain.

Shear deformations are more difficult to measure, as they are the relative horizontal movements perpendicular to the direction of measurement. However, 3D monitoring techniques provide data on the direction and the absolute displacement of survey marks and, therefore, the shear deformations perpendicular to the monitoring line can be determined. But, in accordance with rigorous definitions and the principles of continuum mechanics, (e.g. Jaeger, 1969), it is not possible to determine horizontal shear strains in any direction relative to the monitoring line using 3D monitoring data from a straight line of survey marks.

As described in Section 3.2, shear deformations perpendicular to monitoring lines can be described using various parameters, including horizontal tilt, horizontal curvature, horizontal mid-ordinate deviation, angular distortion and shear index. In this report, horizontal mid-ordinate deviation has been used as the measure for shear deformation, which is defined as the differential horizontal movement of each survey mark, perpendicular to a line drawn between two adjacent survey marks.

The frequency distribution of the maximum total horizontal mid-ordinate deviations measured at survey marks above goaf, for previously extracted longwalls at Appin Colliery and other nearby collieries, is provided in Fig. 4.5. As the typical survey bay length was 20 m, the calculated mid-ordinate deviations were over a chord length of 40 m. The probability distribution function, based on the fitted GPD, has also been shown in this figure.



**Fig. 4.5** Distribution of maximum measured mid-ordinate deviation during the extraction of previous longwalls in the Southern Coalfield for marks located above goaf

A summary of the probabilities of exceedance for total horizontal mid-ordinate deviation for survey bays located above goaf, based the fitted GPD, is provided in Table 4.6. The analysis does not include the strains resulting from valley-related effects, which are discussed separately in the impact assessments for the natural and built features provided in Chapters 5 and 6.

**Table 4.6** Probabilities of exceedance for mid-ordinate deviation for survey marks above goaf for monitoring lines in the Southern Coalfield

	Horizontal mid-ordinate deviation (mm)	Probability of exceedance
Mid-ordinate deviation over a 40 m chord length	10	1 in 3
	20	1 in 15
	30	1 in 40
	40	1 in 110
	50	1 in 250
	60	1 in 550
	70	1 in 1,000
	80	1 in 1,900

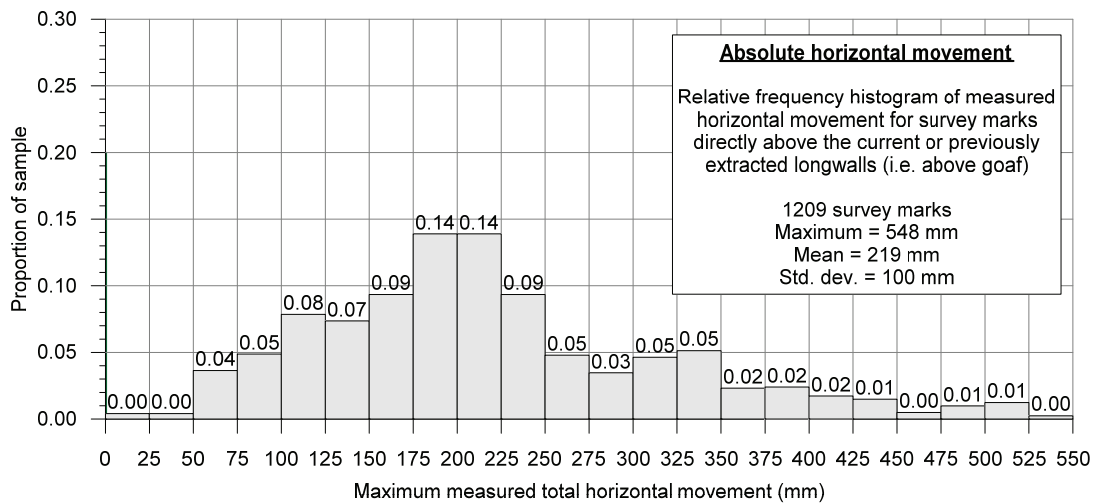
The 95 % and 99 % confidence levels for the maximum total horizontal mid-ordinate deviation that the individual survey marks located above goaf experienced at any time during mining were 23 mm and 39 mm, respectively.

#### 4.4. Predicted conventional horizontal movements

The predicted conventional horizontal movements over the existing and proposed longwalls are calculated by applying a factor to the predicted conventional tilt values. In the Southern Coalfield a factor of 15 is generally adopted, being the same factor as that used to determine the conventional strains from the conventional curvatures, and this has been found to give a reasonable correlation with measured data. This factor will vary and will be higher at low tilt values and lower at high tilt values. The application of this factor will therefore lead to over-prediction of horizontal movements where the tilts are high and under-prediction of the movements where the tilts are low.

The maximum predicted conventional tilt for the proposed longwalls is 7 mm/m. The maximum predicted conventional horizontal movement, therefore, is approximately 100 mm, i.e. 7 mm/m multiplied by a factor of 15. Greater movements can develop in incised terrain, due to the increased horizontal movements that develop in the downslope direction.

The distribution of the maximum measured horizontal movements for the 3D survey marks located directly above the longwalls in Appin Colliery and other nearby collieries is provided in Fig. 4.6. It can be seen from this figure, that horizontal movements have been measured up to approximately 550 mm, with an average measured value of approximately 220 mm.



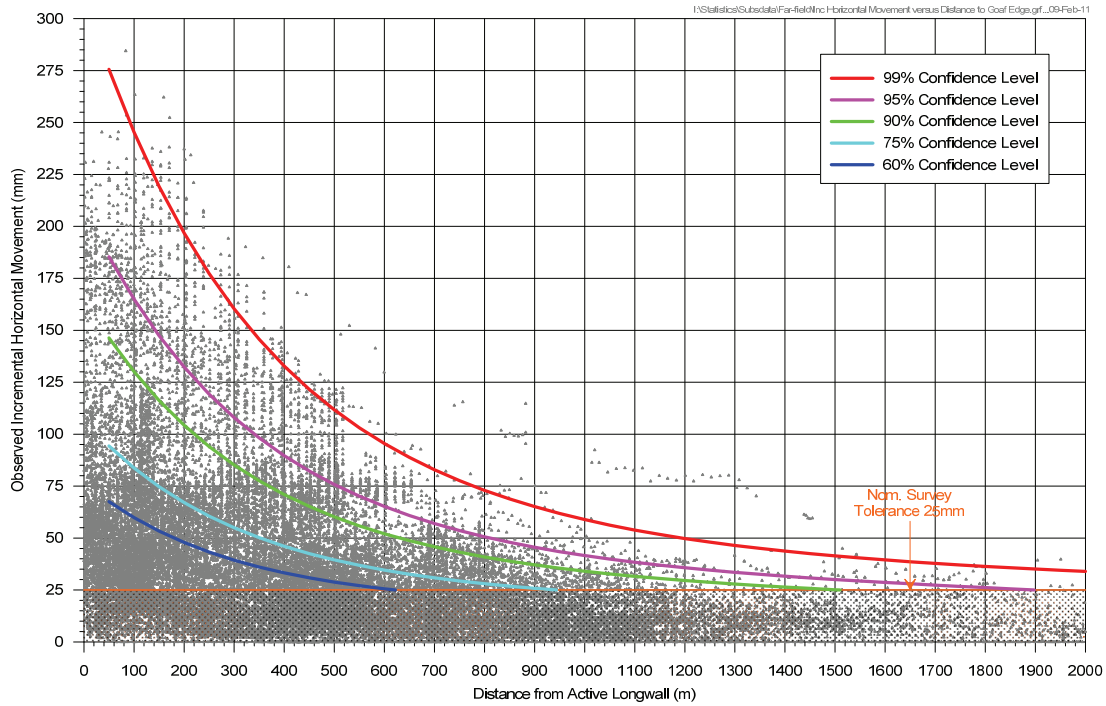
**Fig. 4.6** Distribution of the maximum measured horizontal movements for the 3D marks located directly above the longwalls in the Southern Coalfield

Conventional horizontal movements do not directly impact on natural and built features, rather impacts occur as the result of differential horizontal movements. Strain is the rate of change of horizontal movement. The impacts of strain on the natural features and items of surface infrastructure are addressed in the impact assessments for each feature, which have been provided in Chapters 5 and 6.

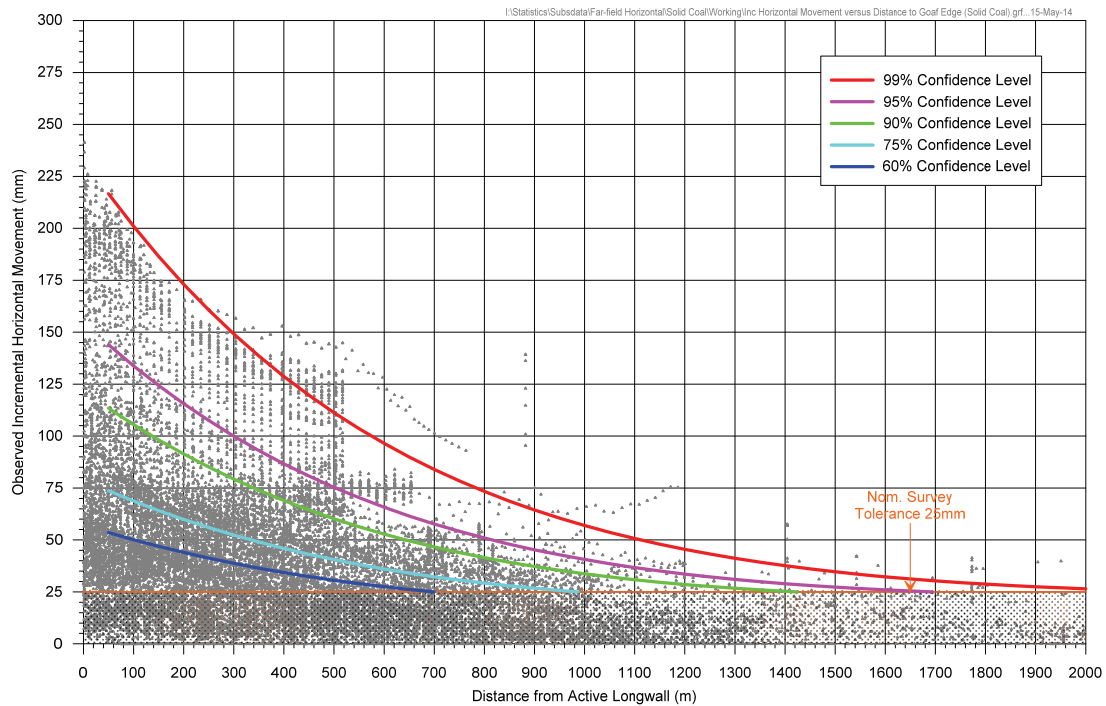
#### 4.5. Predicted far-field horizontal movements

In addition to the conventional subsidence movements that have been predicted above and adjacent to the proposed longwalls, and the predicted valley-related effects along the streams, it is also likely that far-field horizontal movements will be experienced during the extraction of the proposed longwalls.

The measured incremental far-field horizontal movements resulting from the extraction of each longwall, in any location above goaf (i.e. above the currently mined or previously mined longwalls) or above solid coal (i.e. unmined areas of coal) are provided in Fig. 4.7. The measured incremental far-field horizontal movements above solid coal only, i.e. outside the extents of extracted longwalls, are provided Fig. 4.8. The confidence levels based on fitted GPDs have also been shown in these figures to illustrate the spread of the data. It can be seen from Fig. 4.7 and Fig. 4.8 that the magnitude of the measured far-field horizontal movements over solid unmined areas of coal are lower and more consistent than the measured far-field horizontal movements over previously extracted longwalls.



**Fig. 4.7 Measured incremental far-field horizontal movements above goaf or solid coal**



**Fig. 4.8 Measured incremental far-field horizontal movements above solid coal only**

As successive longwalls within a series of longwalls are mined, the magnitudes of the incremental far-field horizontal movements tend to decrease. The total far-field horizontal movement is not, therefore, the sum of the incremental far-field horizontal movements for the individual longwalls.

The predicted far-field horizontal movements resulting from the extraction of the longwalls are very small and could only be detected by precise surveys. Such movements tend to be bodily movements towards the extracted goaf area and are accompanied by very low-levels of strain, which are generally less than survey tolerance. The impacts of far-field horizontal movements on the natural features and items of surface infrastructure within the vicinity of the Study Area are not expected to be significant, except where they occur at large structures which are sensitive to small differential movements.

#### 4.6. Non-conventional ground movements

It is likely non-conventional ground movements will occur within the Study Area, due to near-surface geological conditions, steep topography and valley-related effects, which are discussed in Section 3.4. These non-conventional movements are often accompanied by elevated tilts and curvatures that are likely to exceed the conventional predictions.

Specific predictions of upsidence, closure and compressive strain due to the valley-related effects are provided for the streams in Section 5.3. The impact assessments for the streams are based on both the conventional and valley-related effects. The potential for non-conventional movements associated with steep topography is discussed in the impact assessments for the steep slopes provided in Section 5.7.

In most cases, it is not possible to predict the exact locations or magnitudes of the non-conventional anomalous movements due to near-surface geological conditions. For this reason, the strain predictions provided in this report are based on a statistical analysis of measured strains in the NSW coalfields, including both conventional and non-conventional anomalous strains, as previously discussed in Section 4.3. In addition to this, the impact assessments for the natural features and items of surface infrastructure, which are provided in Chapters 5 and 6, include historical impacts resulting from previous longwall mining which have occurred as the result of both conventional and non-conventional subsidence movements.

The following sections provide the descriptions, predictions and impact assessments for the natural features located within the Study Area. All significant natural features located outside the Study Area, which may be subjected to far-field or valley-related effects and may be sensitive to these movements, have also been included as part of this assessment.

### 5.1. Catchment Areas and Declared Special Areas

There are no drinking water catchment areas, or declared special areas within the Study Area.

### 5.2. Rivers

There are no rivers within the Study Area or within 600 m of the proposed longwalls. The closest river is the Nepean River which is located to the south and to the east of the proposed longwalls. The thalweg (i.e. centreline) of the Nepean River is located 1.5 km south of the commencing (i.e. western) end of LW710A and 1.6 km east of the finishing (i.e. eastern) end of LW709, at its closest points to the proposed longwalls.

The predicted subsidence effects for the Nepean River, due to the mining of LW709 to LW711 and LW905, are less than 20 mm vertical subsidence, less than 20 mm upsidence and less than 20 mm closure. While the river could experience very low levels of vertical subsidence or valley-related effects, it is not predicted to experience measurable tilts, curvatures or strains.

It is unlikely, therefore, that the Nepean River would experience adverse physical impacts due to the mining-induced movements from LW709 to LW711 and LW905. Gas release zones have been observed along the river during the mining of longwalls in Areas 7 and 9. Further gas release zones could develop due to the mining of the proposed longwalls.

### 5.3. Creeks and tributaries

#### 5.3.1. Descriptions of the creeks and tributaries

The locations of the named creeks are shown in Drawing No. MSEC1117-07.

The NSW Government's Strategic Review into the Impacts of Underground Coal Mining on Natural Features in the Southern Coalfield (DoP, 2008) recommended that risk management zones (RMZs) be applied to all streams of third order or above, in the Strahler stream classification.

The creeks that have third order sections located within the Study Area or within 600 m of the proposed longwalls are Foot Onslow Creek, Harris Creek, Navigation Creek and Navigation Creek Tributary 1. There are no creeks with sections greater than third order located within the Study Area or within 600 m of the proposed longwalls.

A summary of the third order creeks that are located within the Study Area is provided in Table 5.1.

**Table 5.1 Third order creeks located within the Study Area**

Name	Location	Total length of third order section above the mining area (km)	Total length of third order section within the Study Area (km)
Foot Onslow Creek	Directly above LW708B, LW709 and LW710B	1.3	2.1
Harris Creek	Outside mining area, adjacent to LW706	0.0	0.4
Navigation Creek	Directly above LW711	1.2	2.1
Navigation Creek Tributary 1	Directly above LW709, LW710B and LW711	1.7	2.2
<b>Total</b>	-	<b>4.2</b>	<b>6.8</b>

The upper reaches of the third order creeks have shallow incisions into the surface soils, which have been derived from the Wianamatta Group, with some sandstone outcropping. The lower reaches of the third order creeks have substantial incisions into the surface soils, with exposed sandstone platforms in the bases and rock outcropping in the valley sides. Pools have naturally developed along the lengths of these creeks within the surface soils and rock platforms. Farm dams have also been established in some locations.

Photographs of Foot Onslow Creek, Harris Creek and Navigation Creek are provided in Fig. 5.1 to Fig. 5.4 (Source: IMC).



**Fig. 5.1 Foot Onslow Creek (Source: IMC)**



**Fig. 5.2 Harris Creek (Source: IMC)**



**Fig. 5.3 Upper reaches of Navigation Creek (Source: IMC)**



**Fig. 5.4 Lower reaches of Navigation Creek (Source: IMC)**

The natural gradients of the third order sections of the creeks within the Study Area typically vary between 5 mm/m (i.e. 0.5 %, or 1 in 200) and 40 mm/m (i.e. 4 %, or 1 in 25). The average natural gradients for these sections of creek vary between 10 mm/m (i.e. 1 %, or 1 in 100) and 20 mm/m (i.e. 2 %, or 1 in 50).

There are also first and second order creeks and tributaries located across the Study Area. These streams have shallow incisions into the natural surface soils which have been derived from the Wianamatta Group. The natural gradients of the first and second order creeks and tributaries vary between 20 mm/m (i.e. 2 %, or 1 in 50) and 400 mm/m (i.e. 40 %, or 1 in 2.5).

### 5.3.2. Predictions for the creeks and tributaries

The predicted profiles of total vertical subsidence, upsidence and closure along Foot Onslow Creek, Harris Creek, Navigation Creek and Navigation Creek Tributary 1 are shown in Figs. C.03 to C.06, respectively, in Appendix C. The predicted total profiles after the completion of LW702 to LW708B and LW901 to LW904 are shown as cyan lines. The predicted total profiles after the mining of each of LW709 to LW711 and LW905 are shown as the blue lines.

Summaries of the maximum predicted values of total vertical subsidence, tilt and curvature for the third order sections of Foot Onslow Creek, Harris Creek, Navigation Creek and Navigation Creek Tributary 1 are provided in Table 5.2 to Table 5.5, respectively. The values are the maximum predicted subsidence effects within the Study Area due to the mining of the existing, approved and proposed longwalls in Areas 7 and 9.

**Table 5.2 Maximum predicted total vertical subsidence, tilt and curvature for the third order section of Foot Onslow Creek**

Name	After longwall	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
Foot Onslow Creek	LW709	1350	5.5	0.07	0.14
	LW710A	1350	6.0	0.07	0.14
	LW710B	1400	6.0	0.07	0.14
	LW711	1400	6.0	0.07	0.14
	LW905	1400	6.0	0.07	0.14

**Table 5.3 Maximum predicted total vertical subsidence, tilt and curvature for the third order section of Harris Creek**

Name	After longwall	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
Harris Creek	LW709	500	3.0	0.02	0.01
	LW710A	500	3.0	0.02	0.01
	LW710B	500	3.0	0.02	0.01
	LW711	500	3.0	0.02	0.01
	LW905	500	3.0	0.02	0.01

**Table 5.4 Maximum predicted total vertical subsidence, tilt and curvature for the third order section of Navigation Creek**

Name	After longwall	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
Navigation Creek	LW709	< 20	< 0.5	< 0.01	< 0.01
	LW710A	30	< 0.5	< 0.01	< 0.01
	LW710B	30	< 0.5	< 0.01	< 0.01
	LW711	950	6.5	0.07	0.14
	LW905	950	6.5	0.07	0.14

**Table 5.5 Maximum predicted total vertical subsidence, tilt and curvature for the third order section of Navigation Creek Tributary 1**

Name	After longwall	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
Navigation Creek Tributary 1	LW709	130	0.5	0.02	< 0.01
	LW710A	130	0.5	0.02	< 0.01
	LW710B	950	6.0	0.06	0.11
	LW711	1350	7.0	0.07	0.12
	LW905	1350	7.0	0.07	0.12

The maximum predicted total tilt for the third order sections of the creeks within the Study Area is 7 mm/m (i.e. 0.7 %, or 1 in 143). The maximum predicted total conventional curvatures are 0.07 km<sup>-1</sup> hogging and 0.14 km<sup>-1</sup> sagging, which represent minimum radii of curvature of 14 km and 7 km, respectively.

The maximum predicted conventional strains for the third order sections, based on applying a factor of 15 to the maximum predicted conventional curvatures, are 1.0 mm/m tensile and 2.0 mm/m compressive. The distribution of the predicted strains due to the extraction of the longwalls is described in Section 4.3. The maximum predicted strains directly above the mining area are 1.0 mm/m tensile and 1.6 mm/m compressive based on the 95 % confidence levels.

Non-conventional movements can also occur and have occurred in the NSW coalfields as a result of, amongst other things, anomalous movements. The analysis of strains provided in Chapter 4 includes those resulting from both conventional and non-conventional anomalous movements.

The creeks will also experience valley-related effects. A summary of the maximum predicted upsidence and closure for the sections of third order creeks located is provided in Table 5.6. The values are the maximum predicted valley-related effects within the Study Area due to the mining of the existing, approved and proposed longwalls in Areas 7 and 9.

**Table 5.6 Maximum predicted total valley-related effects for the third order sections of the creeks**

Name	Maximum predicted total upsidence (mm)	Maximum predicted total closure (mm)
Foot Onslow Creek	300	250
Harris Creek	350	300
Navigation Creek	350	475
Navigation Creek Tributary 1	550	800

The maximum predicted values of valley-related closure for the third order sections of the creeks within the Study Area vary between 250 mm and 800 mm. These creeks could experience compressive strains due to these valley-related effects. The predicted strains due to valley-related effects have been determined from an analysis of ground monitoring lines for valleys with similar heights located directly above extracted longwalls at Appin Colliery and other nearby collieries.

The third order sections of the creeks located above the mining area have effective valley heights typically ranging between 10 m and 40 m, with a small section of Navigation Creek having an effective valley height up to 60 m. The maximum compressive strain measured at similar streams located directly above extracted longwalls at Appin Colliery and other nearby collieries is 7 mm/m based on the 95 % confidence level.

The first and second order sections of the creeks are located across the Study Area and, therefore, could experience the full range of predicted subsidence movements. A summary of the maximum predicted conventional subsidence effects within the Study Area is provided in Chapter 4.

**5.3.3. Impact assessments for the creeks and tributaries**

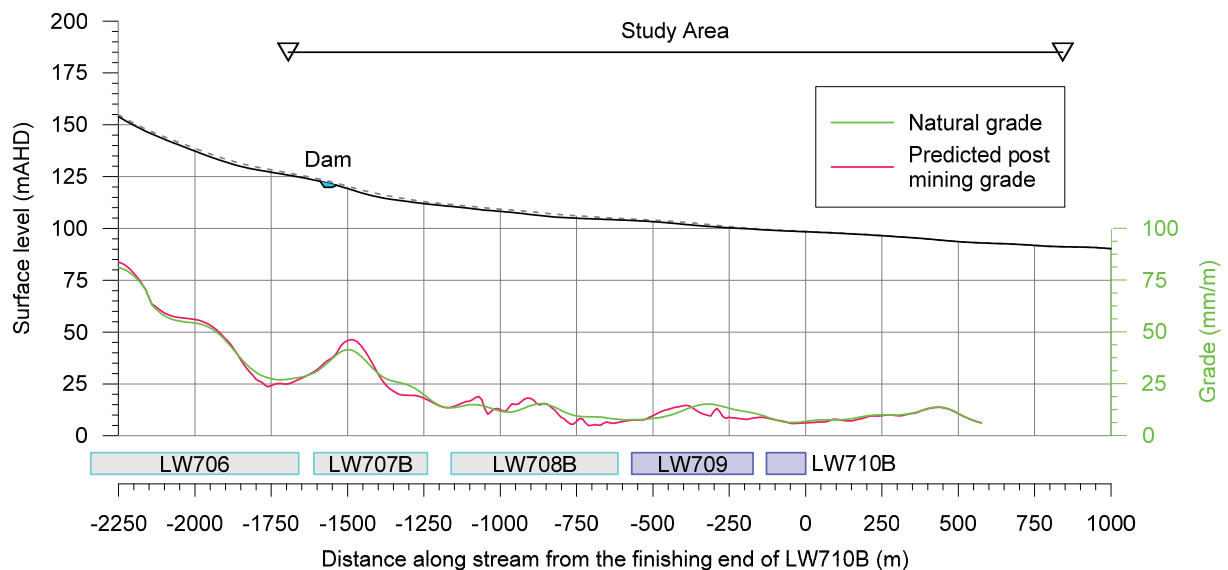
The impact assessments for the creeks are provided in the following sections. The assessments provided in this report should be read in conjunction with the assessments provided in the reports by the other specialist consultants on the project.

*Potential for increased levels of ponding, flooding and scouring due to the mining-induced tilts*

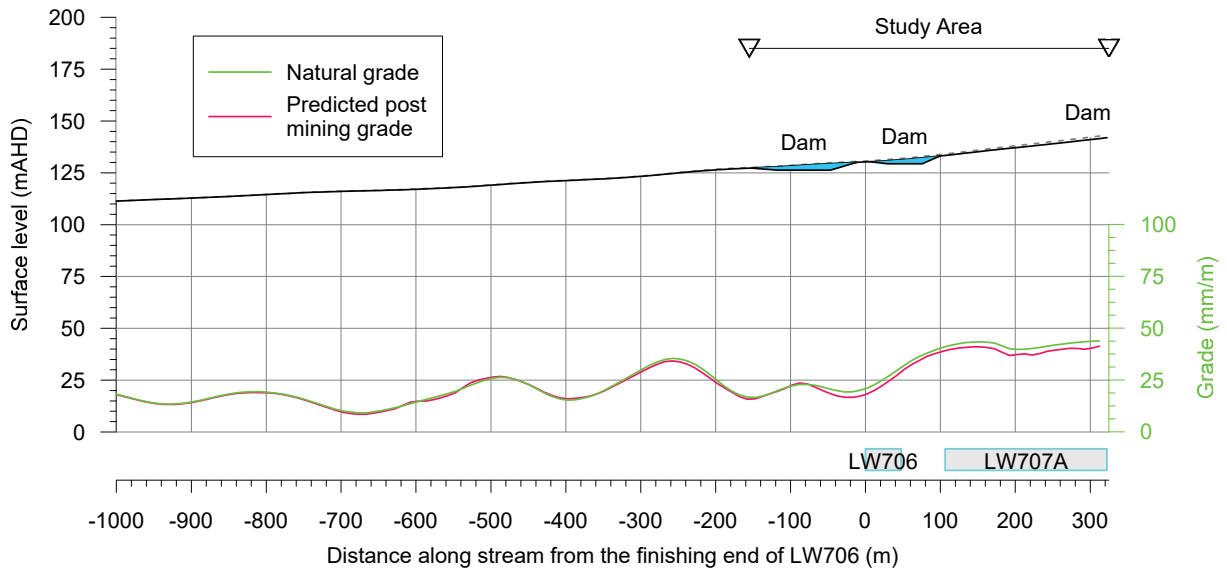
Mining can result in increased levels of ponding in locations where the mining-induced tilts oppose and are greater than the natural drainage line gradients that exist before mining. Mining can also potentially result in an increased likelihood of scouring of the banks in the locations where the mining-induced tilts considerably increase the natural drainage line gradients that exist before mining.

The maximum predicted tilt for the creeks within the Study Area is 7 mm/m (i.e. 0.7 %, or 1 in 143). The predicted mining-induced tilts are less than the average natural gradients of the creeks which vary between 10 mm/m (i.e. 1 %, or 1 in 100) and 20 mm/m (i.e. 2 %, or 1 in 50) for the third order sections and between 20 mm/m (i.e. 2 %, or 1 in 50) and 400 mm/m (i.e. 40 %, or 1 in 2.5) for the first and second order sections

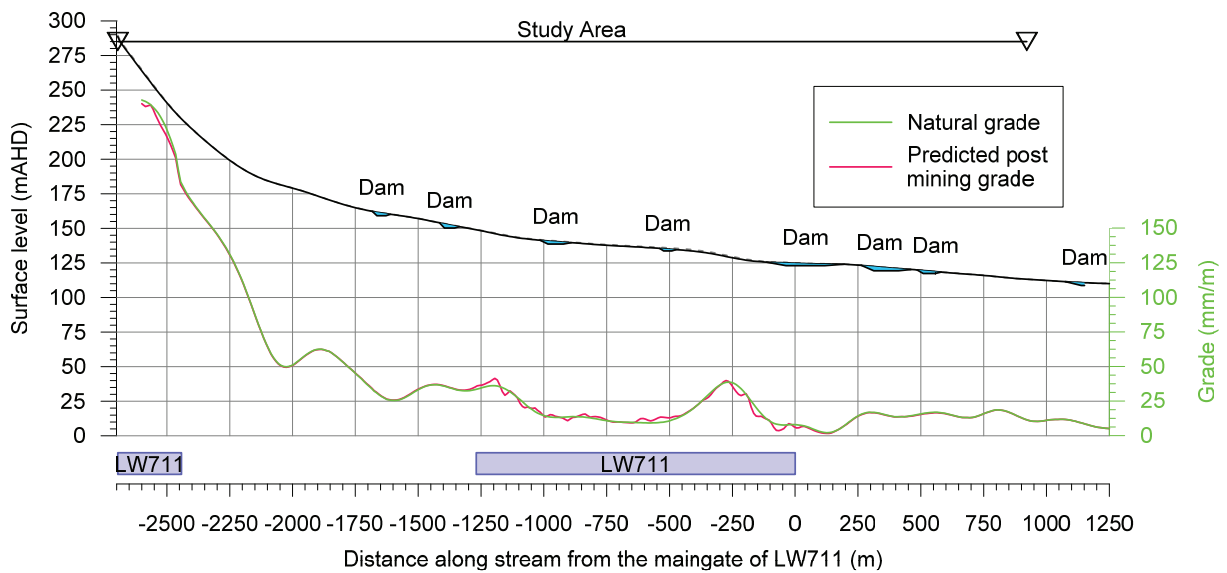
The natural grades and the predicted post-mining grades along drainage lines Foot Onslow Creek, Harris Creek, Navigation Creek and Navigation Creek Tributary 1 are illustrated in Fig. 5.5 to Fig. 5.8, respectively. The locations of these creeks are shown in Drawing No. MSEC1117-07.



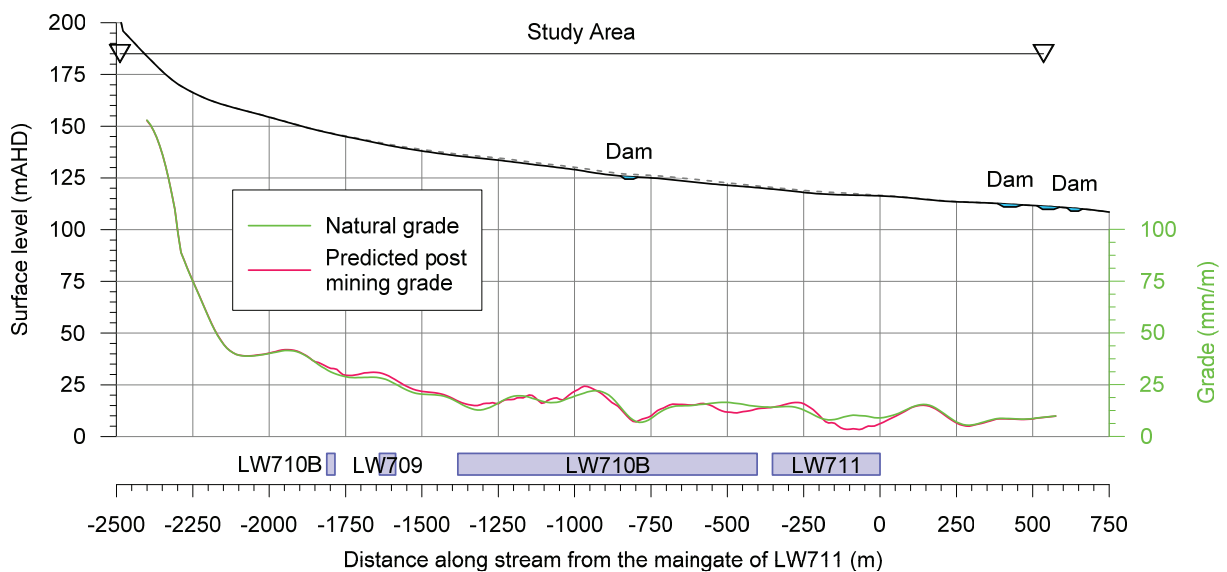
**Fig. 5.5 Natural and predicted post-mining surface levels along Foot Onslow Creek**



**Fig. 5.6** Natural and predicted post-mining surface levels along Harris Creek



**Fig. 5.7** Natural and predicted post-mining surface levels along Navigation Creek



**Fig. 5.8** Natural and predicted post-mining surface levels along Navigation Creek Tributary 1

There are no predicted reversals of stream grade along Foot Onslow Creek, Harris Creek, Navigation Creek or Navigation Creek Tributary 1. Similarly, there are no predicted reversals of stream grade along the first and second order creeks and tributaries located elsewhere within the Study Area.

It is unlikely, therefore, that there would be large-scale adverse changes in the levels of ponding or scouring of the banks along the creeks and tributaries within the Study Area due to the mining-induced tilt. It is possible that localised increased ponding could develop in some isolated locations, where the natural grades are small, and upstream of the chain pillars and the edges of the mining area.

The potential impacts of increased ponding and scouring of the drainage lines, therefore, are expected to be minor and localised. Impacts resulting from changes in surface water flows due to mining-induced tilt are expected to be small in comparison with those which occur during natural flooding conditions.

*Potential for cracking in the creek bed and fracturing of bedrock*

The third order sections of the creeks are predicted to experience a maximum conventional tensile strain of 1.0 mm/m based on the 95 % confidence level. The maximum predicted compressive strain due to the valley-related effects is 7 mm/m based on the 95 % confidence level. The first and second order sections of the creeks and tributaries could experience similar levels of strains.

Fracturing in bedrock has been observed due to previous longwall mining in the Southern Coalfield where the tensile strains have been greater than 0.5 mm/m or where the compressive strains have been greater than 2 mm/m. Fracturing could therefore develop along the creeks and tributaries due to the mining of the proposed LW709 to LW711 and LW905. Fracturing will predominately occur where the creeks and tributaries are located directly above the mining area. Impacts can also occur outside the mining area, with minor and isolated fracturing occurring at distances up to approximately 400 m outside the longwalls, as previously observed at Appin Colliery and elsewhere in the Southern Coalfield.

The mining-induced compression due to valley closure effects can also result in dilation and the development of bed separation in the topmost bedrock, as it is less confined. This additional dilation due to valley closure is expected to develop predominately within the top 10 m to 20 m of the bedrock. Compression can also result in buckling of the topmost bedrock resulting in heaving in the overlying surface soils.

Surface water flow diversions could occur along the creeks and tributaries that are located directly above the mining area. In times of heavy rainfall, the majority of the runoff would flow over the fractured bedrock and soil beds and would not be diverted into the dilated strata below. In times of low flow, however, surface water flows can be diverted into the dilated strata below the beds. The creeks and tributaries are ephemeral and, therefore, surface water flows only occur during and for short periods after rain events.

Creeks and tributaries have been directly mined beneath by the completed longwalls in Appin Areas 7 and 9 and at other nearby collieries. A summary of the experience of mining beneath ephemeral creeks and tributaries in the Southern Coalfield is provided in Table 5.7.

**Table 5.7 Experience of mining beneath creeks and tributaries in the Southern Coalfield**

Longwalls	Streams	Measured movements	Observed impacts
Appin Area 3 (LW301 and LW302)	2.7 km of streams directly mined beneath	650 mm subsidence 4.5 mm/m tilt 1.0 mm/m tensile strain 3.0 mm/m comp. strain (Measured M & N-Lines)	No reported fracturing which resulted in surface water flow diversions
Appin Area 4 (LW401 to LW409)	3.8 km of streams directly mined beneath, including Creek 2A, Rocky Ponds Creek and Simpsons Creek	700 mm subsidence 5.0 mm/m tilt 1.0 mm/m tensile strain 2.0 mm/m comp. strain (Measured A6000-Line)	No reported fracturing which resulted in surface water flow diversions
Appin Area 7 (LW701 to LW708A)	12 km of streams directly mined beneath, including upper reaches of Harris Creek	1300 mm subsidence 8.5 mm/m tilt 1.5 mm/m tensile strain 8.5 mm/m comp. strain (Measured ARTC-Line and M31-Lines)	No reported fracturing which results in surface water flow diversions
Appin Area 9 (LW901 and LW902)	3.9 km of streams directly mined beneath including tributaries to the Nepean River	525 mm subsidence 3.0 mm/m tilt 1.5 mm/m tensile strain 3.0 mm/m comp. strain (Measured ARTC-Line)	No reported fracturing which results in surface water flow diversions
West Cliff Area 5 (LW29 to LW37)	5.6 km of streams directly mined beneath, including Unnamed Creek, Ousedale Creek, Mallaty Creek and Leafs Gully	1350 mm subsidence 11 mm/m tilt 1.5 mm/m tensile strain 5.5 mm/m comp. strain (Measured B-Line)	Fracturing observed in the bed of Mallaty Creek, loss of water holding capacity in one pool (Ref. MC109).

Based on the experience of mining beneath ephemeral creeks and tributaries in the Southern Coalfield, it is likely that some fracturing will occur along the streams within the Study Area, particularly those located directly above or adjacent to the mining area. Some standing pools could experience a reduction or loss of water holding capacity.

Further assessments of the potential impacts on surface water are provided in the reports by the specialist surface water and groundwater consultants on the project.

#### 5.3.4. Recommendations for the creeks and tributaries

IMC has developed management strategies for creeks and tributaries that have been directly mined beneath by longwalls in Appin Areas 7 and 9. It is recommended that these management strategies are reviewed and updated to incorporate the proposed LW709 to LW711 and LW905. It is also recommended that periodic inspections are carried out along the streams during active subsidence.

### 5.4. Aquifers and known groundwater resources

Shallow groundwater aquifers have been identified within the Study Area. The descriptions and assessment of potential impacts on the groundwater resources are provided in the report by the specialist groundwater consultant on the project.

### 5.5. Cliffs

#### 5.5.1. Descriptions of the cliffs and minor cliffs

The definition of a cliff provided in the project approval (BSO, 2011) are:

*“Cliff* Continuous rock face, including overhangs, having a minimum height of 10 metres and a minimum slope of 2 to 1, ie having a minimum angle to the horizontal of 63°

The definitions of cliffs and minor cliffs provided in the NSW DPIE *Standard and Model Conditions for Underground Mining* (DPIE, 2012) are:

*“Cliff* Continuous rock face, including overhangs, having a minimum length of 20 metres, a minimum height of 10 metres and a minimum slope of 2 to 1 (>63.4°)

*Minor Cliff* A continuous rock face, including overhangs, having a minimum length of 20 metres, heights between 5 metres and 10 metres and a minimum slope of 2 to 1 (>63.4°); or a rock face having a maximum length of 20 metres and a minimum height of 10 metres”

The cliffs and minor cliffs within the Study Area have been identified from the LiDAR surface level contours and from site investigations. The locations of the cliffs are shown in Drawing No. MSEC1117-08.

Cliffs have been identified in the western part of the Study Area along Razorback Range. These cliffs have formed in the sandstone members (Rwb and Rwbh) of the Wianamatta Group. A summary of the cliffs within the Study Area is provided in Table 5.8.

**Table 5.8 Details of the cliffs located within the Study Area**

Cliff Ref.	Overall length (m)	Maximum height (m)	Location
RR-CL1	25	15	470 m west of LW710A
RR-CL2	50	13	420 m west of LW710A
RR-CL3	30	20	Directly above LW710A
RR-CL4	30	13	150 m south of LW710A and 420 m west of LW905
RR-CL5	90	20	Directly above LW710A
RR-CL6	30	12	Directly above LW905
RR-CL7	35	12	Directly above LW905

The cliffs have overhang depths ranging between 1 m and 3 m. Natural fracturing and dislodged rocks are evident in Cliffs RR-CL2, RR-CL3, RR-CL6 and RR-CL7. There is also minor water seepage visible on the faces of the cliffs.

The cliffs within the Study Area were inspected by the IMC field team on 23 and 24 July 2020. Photographs of these cliffs are provided in Fig. 5.9 to Fig. 5.14 (Source: IMC).



**Fig. 5.9** Cliff RR-CL1 (Source: IMC)



**Fig. 5.10** Cliff RR-CL2 (Source: IMC)



**Fig. 5.11** Cliff RR-CL3 (Source: IMC)



**Fig. 5.12** Cliff RR-CL4 (Source: IMC)



**Fig. 5.13** Cliff RR-CL6 (Source: IMC)



**Fig. 5.14** Cliff RR-CL7 (Source: IMC)

Cliffs have also been identified along the Nepean River and Harris Creek; however, these features are located outside the Study Area and are at distances of more than 1 km from the proposed longwalls.

Minor cliffs and rock outcrops have also been identified within the Study Area. These rock features are generally located along Razorback Range and the steeper sections of the streams.

### 5.5.2. Predictions for the cliffs and minor cliffs

A summary of the maximum predicted values of total vertical subsidence, tilt and curvature for the cliffs within the Study Area is provided in Table 5.9. The values are the maximum predicted subsidence effects within 20 m of the mapped extents of each of the cliffs due to the mining of the existing, approved and proposed longwalls in Areas 7 and 9.

**Table 5.9 Maximum predicted total vertical subsidence, tilt and curvature for the cliffs within the Study Area**

Feature	Reference	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
Cliffs	RR-CL1	< 20	< 0.5	< 0.01	< 0.01
	RR-CL2	< 20	< 0.5	< 0.01	< 0.01
	RR-CL3	150	2.0	0.03	< 0.01
	RR-CL4	125	< 0.5	< 0.01	< 0.01
	RR-CL5	225	3.0	0.03	< 0.01
	RR-CL6	775	4.5	0.01	0.08
	RR-CL7	600	4.5	0.03	0.04

The maximum predicted total tilt for the cliffs within the Study Area is 4.5 mm/m (i.e. 0.45 %, or 1 in 222). The maximum predicted total conventional curvatures are 0.03 km<sup>-1</sup> hogging and 0.08 km<sup>-1</sup> sagging, which represent minimum radii of curvature of 33 km and 13 km, respectively.

The maximum predicted conventional strains for the cliffs, based on applying a factor of 15 to the maximum predicted conventional curvatures, are 0.5 mm/m tensile and 1.0 mm/m compressive. The distribution of the predicted strains due to the extraction of the longwalls is described in Section 4.3. The maximum predicted strains directly above the mining area are 1.0 mm/m tensile and 1.6 mm/m compressive based on the 95 % confidence levels. The maximum predicted strains outside the mining area are 0.6 mm/m tensile and 0.4 mm/m compressive based on the 95 % confidence levels.

Non-conventional movements can also occur and have occurred in the NSW coalfields as a result of, amongst other things, anomalous movements. The analysis of strains provided in Chapter 4 includes those resulting from both conventional and non-conventional anomalous movements.

The minor cliffs are located across the Study Area and, therefore, are expected to experience the full range of predicted subsidence movements. A summary of the maximum predicted conventional subsidence movements within the Study Area is provided in Chapter 4.

### 5.5.3. Impact assessments for the cliffs and minor cliffs

The total length of cliffs that are located within the Study Area is 290 m. These cliffs are predicted to experience mine subsidence effects up to 775 mm vertical subsidence, 4.5 mm/m tilt and 0.03 km<sup>-1</sup> hogging curvature and 0.08 km<sup>-1</sup> sagging curvature.

It is difficult to assess the likelihood of cliff instabilities based upon predicted ground movements. The likelihood of a cliff becoming unstable is dependent on many factors that are difficult to quantify. Some of these factors include jointing, inclusions, weathering within the rockmass, groundwater pressure and seepage flow behind the rockface. Even if these factors could be determined, it would still be difficult to quantify the extent to which these factors may influence the stability of a cliff naturally or when it is exposed to mine subsidence movements. It is therefore possible that cliff instabilities may occur during mining that may be attributable to either natural causes, mine subsidence, or both.

The likelihood of cliff instabilities within the Study Area has been assessed based on case studies where longwall mining has previously occurred directly beneath cliffs in the Southern Coalfield.

- *Tower Longwalls 1 to 17 beneath the Cataract and Nepean Rivers*

Tower Longwalls 1 to 17 mined directly beneath the valleys of the Cataract and Nepean Rivers. The total length of cliffs that were directly mined beneath or located within the 35° angle of draw line from these longwalls was greater than 5 km. The overall heights of the cliffs varied between 10 m and 60 m and they had formed within the Hawkesbury Sandstone Sedimentary Group.

Tower Longwalls 1 and 17 had void widths varying between 110 m and 210 m and solid chain pillar widths varying between 35 m and 50 m. The longwalls were extracted from the Bulli Seam at depths of cover varying between 400 m and 540 m. The width-to-depth ratios of these longwalls therefore typically ranged between 0.4 and 0.5.

There were a total of 10 cliff instabilities recorded along the Cataract and Nepean Rivers, due to the mining of Tower Longwalls 1 to 17, all of which occurred where the longwalls mined directly beneath the cliffs. The details of the cliff instabilities are provided in Table 5.10.

**Table 5.10 Recorded cliff instabilities along the Cataract and Nepean River Valleys due to the mining of Tower Longwalls 1 to 17**

Colliery	River valley	Longwalls	Number of recorded cliff instabilities due to mining	Total length of recorded cliff instabilities due to mining	Total length of cliff within 0.7 times depth of cover from the longwalls	Observed rate of cliff instabilities due to mining (%)
Tower	Cataract	LW16 & LW17	8	160	3700	4.3
	Nepean	LW1 to LW17	2	40	1875	2.1
<b>Total</b>			<b>10</b>	<b>200</b>	<b>5575</b>	<b>3.5</b>

The total length of cliff instabilities due to the mining of Tower Longwalls 1 to 17 was approximately 4 % of the total length of the cliffs. All the recorded cliff instabilities occurred where the cliffs were directly mined beneath.

- *Tahmoor Longwalls 14 to 19 beneath the Bargo River*

Tahmoor Longwalls 14 to 19 mined directly beneath the Bargo River. The total length of cliffs that were directly mined beneath or located within the 35° angle of draw line from these longwalls was approximately 2.5 km. The overall heights of the cliffs varied between 10 m and 25 m and they had formed within the Hawkesbury Sandstone Sedimentary Group.

Tahmoor Longwalls 14 to 19 had void widths of 240 m and solid chain pillar widths of 37 m. The longwalls were extracted from the Bulli Seam at depths of cover varying between 380 m and 390 m. The width-to-depth ratios of these longwalls therefore typically ranged between 0.6 and 0.65.

No cliff instabilities were observed during the mining period.

Based on the experience of mining beneath cliffs at Tower and Tahmoor Collieries, it is estimated that on average between 3 % and 5 % of the total length, or between 1 % and 3 % of the total face area of cliffs that are located directly above the mining area would experience adverse impacts. It is also anticipated that less than 3 % of the total face area of minor cliffs located directly above the proposed longwalls would experience adverse impacts.

Cliffs RR-CL3, RR-CL5, RR-CL6 and RR-CL7 are located directly above the proposed LW710A and LW905. The total length of these cliffs is approximately 185 m and the total face area is approximately 3180 m<sup>2</sup>.

There are no building structures or other built features located directly beneath Cliffs RR-CL3, RR-CL5, RR-CL6 and RR-CL7. The risk to public safety, therefore, is considered to be low. However, it is recommended that, subject to landholder access, IMC carry out periodic inspections of the cliffs at a safe distance during active subsidence, to identify any fracturing, unstable rocks and rockfalls.

Cliffs RR-CL1, RR-CL2 and RR-CL4 are located outside the mining area at distance varying between 150 m and 470 m. At these distances, the cliffs are not predicted to experience measurable conventional tilts, curvatures or strains. It is not anticipated, therefore, that adverse impacts would occur to Cliffs RR-CL1, RR-CL2 and RR-CL4. However, it is possible that isolated rock falls could occur at these cliffs, but it is expected that this would represent on average less than 1 % of the total length or total face area of the cliffs located outside the mining area.

#### 5.5.4. Recommendations for the cliffs and minor cliffs

It is recommended that a Landscape Management Plan be developed for the cliffs within the Study Area to monitor and manage any impacts that result from cliff instabilities.

#### 5.6. Rock Outcrops

Rock outcrops are defined as exposed rockfaces with heights of less than 10 m or slopes of less than 2 in 1. There are rock outcrops located across the Study Area, primarily along the Razorback Range and the incised creeks and tributaries.

The rock outcrops are located across the Study Area and, therefore, are expected to experience the full range of predicted subsidence movements. A summary of the maximum predicted conventional subsidence movements within the Study Area is provided in Chapter 4.

The extraction of the proposed longwalls is likely to result in fracturing of the rock outcrops and, where the rock is marginally stable, this could then result in instabilities. Previous experience in the Southern Coalfield indicates that the percentage of rock outcrops that are likely to be impacted by mining is very small, representing on average less than 3 % of the total surface area.

The potential for isolated rockfalls, however, could result in a public safety risk where houses or infrastructure are located beneath large rock outcrops.

IMC has developed a *Cliff and Steep Slope Management Plan* for LW701 to LW708B and LW901 to LW904. The Management Plan addresses monitoring, response action, reporting and public safety. It is recommended that the management plan be reviewed and, where required, revised to include the proposed LW709 to LW711 and LW905.

#### 5.7. Steep slopes

##### 5.7.1. Descriptions of the steep slopes

The definition of a steep slope provided in the project approval (BSO, 2011) is: "*An area of land having a gradient between 1 in 3 (33% or 18.3°) and 2 in 1 (200% or 63.4°)*". The locations of the steep slopes were identified from the 1 m surface level contours which were generated from a LiDAR survey of the area.

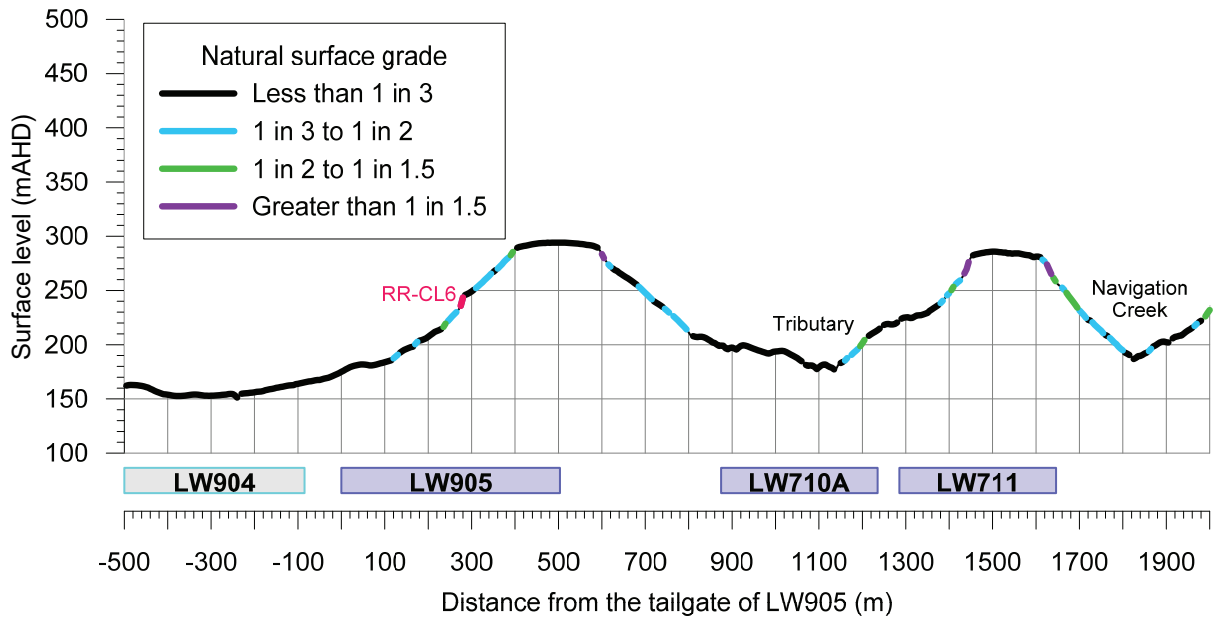
The areas identified as having steep slopes are shown in Drawing No. MSEC1117-08.

The steep slopes within the Study Area have been identified along Razorback Range above LW710A, the western end of LW711 and above LW905. A photograph of Razorback Range in Area 9 is provided in Fig. 5.15.



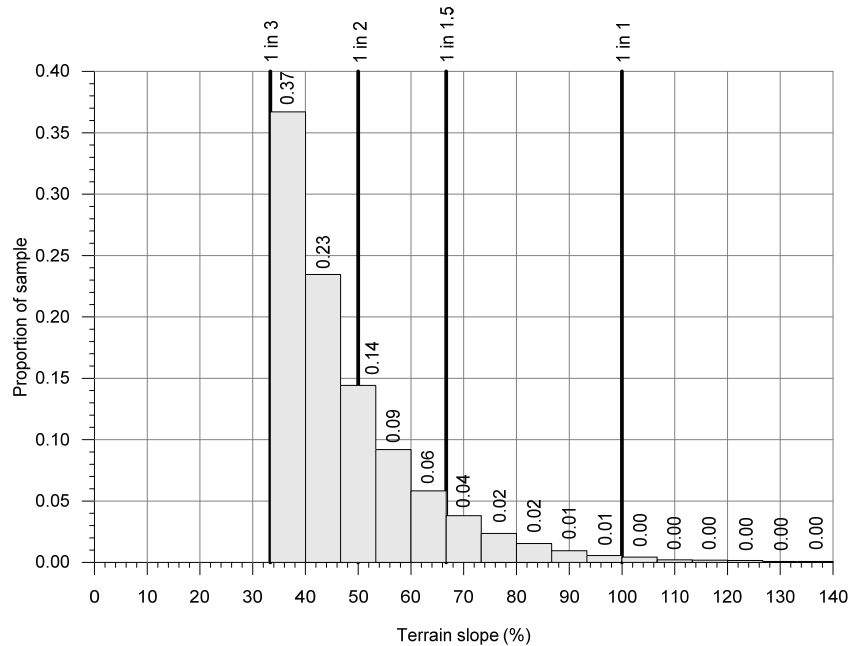
**Fig. 5.15** Razorback Range in Area 9

A section showing the surface topography and the natural grades through Razorback Range is illustrated in Fig. 5.16. The section has been taken across the range (i.e. in the direction of the maximum terrain slope) and oblique to the proposed longwalls.



**Fig. 5.16 Section through Razorback Range and Appin Area 9**

The distribution of the natural grades of the steep slopes along Razorback Range in Appin Area 9 is illustrated in Fig. 5.17. The steep slopes have natural grades typically ranging between 1 in 3 and 1 in 2 (88 % of the surface area), but there are locations with natural grades ranging between 1 in 2 and 1 in 1.5 (8 % of the surface area), between 1 in 1.5 and 1 in 1 (3 % of the surface area) and greater than 1 in 1 (1 % of the surface area).



**Fig. 5.17 Distribution of the natural grades for the steep slopes along Razorback Range in Area 9**

Steep slopes are also located above the proposed LW709, LW710B and the eastern end of LW711, and along the ridgeline above the approved LW902 to LW904. The natural grades of these steep slopes typically vary up to approximately 1 in 2 (i.e. 27° or 50 %), with isolated areas with natural grades up to 1 in 1.5 (i.e. 34° or 67 %).

The surface soils for the steep slopes along Razorback Range have formed from the Wianamatta Group. The extent of the Wianamatta Shale within the Study Area has been mapped and further discussions are provided in the report by GHD (2021d).

### 5.7.2. Predictions for the steep slopes

A summary of the maximum predicted values of total vertical subsidence, tilt and curvatures for the steep slopes along Razorback Range is provided in Table 5.11. The values in this table are the maximum predicted subsidence effects within the Study Area due to the mining of the existing, approved and proposed longwalls.

**Table 5.11 Maximum predicted total vertical subsidence, tilt and curvatures for the steep slopes along Razorback Range**

Location	After longwall	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
Steep slopes along Razorback Range	LW709	950	6.0	0.06	0.10
	LW710A	950	6.0	0.06	0.10
	LW710B	950	6.0	0.06	0.10
	LW711	950	7.0	0.08	0.14
	LW905	1150	7.0	0.08	0.14

The maximum predicted total tilt for the steep slopes along Razorback Range is 7 mm/m (i.e. 0.7 %, or 1 in 143). The maximum predicted total conventional curvatures are 0.08 km<sup>-1</sup> hogging and 0.14 km<sup>-1</sup> sagging, which represent minimum radii of curvature of 13 km and 7 km, respectively.

The maximum predicted conventional strains for the steep slopes along Razorback Range, based on applying a factor of 15 to the maximum predicted conventional curvatures, are 1.0 mm/m tensile and 2.0 mm/m compressive. The distribution of the predicted strains due to the extraction of the longwalls is described in Section 4.3. The maximum predicted strains directly above the mining area are 1.0 mm/m tensile and 1.6 mm/m compressive based on the 95 % confidence levels.

Non-conventional movements can also occur and have occurred in the NSW coalfields as a result of, amongst other things, anomalous movements. The analysis of strains provided in Chapter 4 includes those resulting from both conventional and non-conventional anomalous movements.

The steep slopes located elsewhere with the Study Area could experience the full range of predicted subsidence movements. A summary of the maximum predicted conventional subsidence effects within the Study Area is provided in Chapter 4.

### 5.7.3. Impact assessments for the steep slopes

The maximum predicted tilt for the steep slopes is 7.0 mm/m (i.e. 0.7 %, or 1 in 143) for Razorback Range and 7 mm/m (i.e. 0.7 %, or 1 in 143) elsewhere within the Study Area. The predicted changes in grade are small when compared to the natural grades of the steep slopes, which are greater than 1 in 3 and, therefore, the mining-induced tilts themselves are unlikely to result in adverse impacts on the stability of the steep slopes.

The steep slopes are more likely to be impacted by the mining-induced curvatures and strains. The potential impacts would generally result from the increased horizontal movements in the downslope direction, causing tension cracks to appear at the tops and along the sides of the slopes and compression ridges to form at the bottoms of the slopes.

The maximum predicted curvatures for the steep slopes are 0.08 km<sup>-1</sup> hogging and 0.14 km<sup>-1</sup> sagging for the Razorback Range and 0.08 km<sup>-1</sup> hogging and 0.15 km<sup>-1</sup> sagging elsewhere within the Study Area. The maximum predicted strains are 1.0 mm/m tensile and 1.6 mm/m compressive based on the 95 % confidence levels. The maximum predicted curvatures and strains for the steep slopes within the Study Area are similar to those experienced for previous longwall mining in the Southern Coalfield.

There is extensive experience of mining beneath steep slopes in the Southern Coalfield. These include steep slopes along the Cataract, Nepean, Bargo and Georges Rivers. No large-scale slope failures have been observed along these slopes, even where longwalls have been mined directly beneath them. Although no large-scale slope failures have been observed in the Southern Coalfield, tension cracking has been observed at the tops and along the sides of steep slopes as the result of increased horizontal movements in the downslope direction.

Cracks resulting from downslope movements at depths of cover greater than 400 m, such as the case in the Study Area, have been observed with typical widths in the order of 25 mm to 50 mm. Larger cracks have also been observed at the tops of very steep slopes and adjacent to large rock formations, having typical widths in the order of 100 mm to 150 mm or greater. A photograph of a tension crack near the top of a steep slope in the Southern Coalfield is provided in Fig. 5.18.



**Fig. 5.18 Example of surface tension cracking along the top of a steep slope**

The soils within the Study Area are generally derived from the Wianamatta Shale Group and there is extensive natural erosion in some locations. A photograph of natural erosion within the Study Area is provided in Fig. 5.19.



**Fig. 5.19 Photograph of natural soil erosion within the Study Area**

As the slopes along the Razorback range are steep, exhibit natural soil erosion and are predicted to experience the full range of predicted subsidence movements, it is likely that the extraction of the proposed longwalls would result in large surface cracks near the tops and along the sides of these slopes. Detailed site investigations and studies of the potential impacts on the steep slopes along the Razorback Range have been undertaken and are described in the reports by GHD (2021a, 2021b, 2021c and 2021d).

If tension cracks were to develop, as the result of the extraction of the proposed longwalls, it is possible that soil erosion could occur if these cracks were left untreated. It is possible, therefore, that some remediation would be required, including infilling of surface cracks with soil or other suitable materials, or by locally regrading and recompacting the surface. In some cases, erosion protection measures may be needed, such as the planting of additional vegetation in order to stabilise the slopes in the longer term.

While in most cases, impacts on steep slopes are likely to consist of surface cracks, there remains a low probability of large-scale downslope movements. Experience indicates that the probability of mining induced large-scale slippages is extremely low due to the significant depth of cover within the Study Area.

While the risk is extremely low, some risk remains and it is recommended that attention is paid to any features or items of infrastructure located in the vicinity of steep slopes directly above the proposed longwalls, which include the:

- local roads;
- powerlines and copper telecommunications cables;
- houses;
- rural and other structures;
- survey control marks.

The locations of the surface infrastructure in the vicinity of steep slopes are shown in Drawing No. MSEC1117-08. The risks associated with the proximity of the steep slopes are discussed in the impact assessments for each item of infrastructure.

A Landslide Risk Assessment (LRA) for the proposed LW709 to LW711 and LW905 has been carried out by GHD (2021d). The key conclusion of the LRA is that the *“results of the preliminary investigations indicate that the structures located on the identified steep slopes should be able to be effectively managed, using the techniques and management tools available and previously employed in similar geotechnical circumstances, to permit Appin Mine to extract the proposed longwalls”*.

GHD (2021d) categorised the properties into *“those not requiring additional appraisal, those requiring a brief site visit (contingent upon landholder approval for access) to confirm the site setting not observable from the public roadways, to more detailed appraisals for residences in high hazard environments”*. GHD (2021d) states that the *“residential sites that require appraisals can be conducted on a longwall-by-longwall basis”* so that *“the hazards can be managed through each subordinate longwall’s Structures Management Plan”*.

#### **5.7.4. Recommendations for the steep slopes**

IMC has engaged a team of consultants to assess the existing stability and the potential for subsidence impacts on the steep slopes associated with the Razorback Range. The assessed risks and recommendations are provided in the reports by GHD (2021a, 2021b, 2021c and 2021d).

IMC has developed a *Cliff and Steep Slope Management Plan* for LW701 to LW708B and LW901 to LW904. The existing Management Plan addresses monitoring, response action, reporting and public safety. It is recommended that the management plan be reviewed and, where required, revised to include the proposed LW709 to LW711 and LW904.

### **5.8. Escarpments**

The Razorback Range forms a well-defined escarpment in the western part of the Study Area. The descriptions, predictions and impact assessments for the rock outcrops and steep slopes along the Razorback Range are provided in Sections 5.6 and 5.7, respectively.

### **5.9. Land prone to flooding and inundation**

The catchment areas of the streams within the Study Area are relatively small and the land drains freely into the Nepean River. There are no major flood prone areas identified within the Study Area. The predicted changes in the surface levels of the streams is predicted to only have a marginal effect on their natural gradients, as described in 5.3.

### **5.10. Swamps, wetlands and water related ecosystems**

There are no swamps or wetlands within the Study Area. There are water related ecosystems within the Study Area associated with the major streams. Discussions on the water related ecosystems are provided in the reports by Cardno (2021) and SLR (2020 and 2021).

### **5.11. Flora and fauna**

The potential for impacts on the vegetation in the mining area is dependent on the surface cracking, changes in surface water and changes in groundwater. Assessments of the physical impacts due to the proposed longwall are provided in Sections 5.2 to 5.10. Assessments of the environmental consequences have been provided by the other specialist consultants on the project.

Assessments for the aquatic and terrestrial ecology are provided by Cardno (2021).

The following sections provide the descriptions, predictions and impact assessments for the built features within the Study Area. All significant features located outside the Study Area, which may be subjected to far-field or valley-related effects and may be sensitive to these movements, have also been included as part of these assessments.

## 6.1. Main Southern Railway

### 6.1.1. Descriptions of the Main Southern Railway

The locations of the Main Southern Railway and its associated infrastructure are shown in Drawing No. MSEC1117-09.

The Main Southern Railway is a key national transport route that carries significant freight and passenger services between Sydney and Melbourne. The Main Southern Railway is leased by Australian Rail Track Corporation (ARTC), who is responsible for maintaining the track.

The Main Southern Railway is located to the east of the proposed longwalls. The railway is situated immediately adjacent to the south-eastern corner of LW709 and it is 0.4 km from LW710B and 0.9 km from LW711, at its closest points. The total length of railway within the Study Area is approximately 1.3 km, which extends between kilometrages 68.2 km and 69.5 km.

The railway crosses directly above the completed and approved longwalls in Appin Areas 7 and 9. The total lengths of railway located above these existing longwalls are approximately 3.0 km above LW703 to LW708B and 2.1 km above LW901. The potential impacts on the railway due to these existing longwalls have been managed with the implementation of the approved Management Plans (Reports Nos. MSEC642 and MEC706).

The railway line is a dual track consisting of 60 kg rail on concrete sleepers with a mix of straight and curved track sections within the Study Area. The maximum speed limits on both tracks are 115 km/h for normal services and 125 km/h for XPT services. A photograph of a section of the railway within the Study Area is provided in Fig. 6.1.



*Photograph courtesy of BloorRail*

**Fig. 6.1 Main Southern Railway at 69.0 km above LW708B**

There are two railway cuttings located within the Study Area. Details of the cuttings are provided in Table 6.1 and a photograph is provided in Fig. 6.2. The cutting faces generally consist of Wianamatta Shale and clays.

**Table 6.1 Railway cuttings within the Study Area**

Label	Location	Kilometrages (km)	Length (m)	Maximum height (m)
68.6 km (Cutting 6)	Above LW708B and adjacent to LW709	68.40 to 68.85	450	6 (up rail) 4 (down rail)
69.6 km (Cutting 5)	Above LW706 and LW707	69.30 to 69.75	450	8 (up rail) 4 (down rail)



**Fig. 6.2 Cutting at 68.6 km with gabion wall and race on down side**

There is one railway embankment located within the Study Area. Details of this embankment are provided in Table 6.2.

**Table 6.2 Railway embankments within the Study Area**

Label	Location	Kilometrages (km)	Length (m)	Maximum height (m)
69.0 km (Embankment 2)	Above LW707B and LW708B	68.85 to 69.30	360 (up rail) 450 (down rail)	13

The base of the embankment at 69.0 km has been substantially widened by IMC to reduce the potential for impacts due to the extraction of LW707B to LW709. Photographs of the widened embankment are provided in Fig. 6.3.



Photographs courtesy of BloorRail

**Fig. 6.3 Embankment at 69.0 km**

There are three railway culverts located within the Study Area. Details of these culverts are provided in Table 6.3. A photograph of the culvert at 69.000 km is provided in Fig. 6.4.

**Table 6.3 Railway culverts within the Study Area**

Label	Location	Kilometrage (km)	Type	Diameter (mm)
68.621 km (Culvert 9)	110 m east of LW709	68.621 km	Concrete pipes	450 (2 off)
69.000 km (Culvert 8)	Directly above LW708B	69.000 km	Brick arch culvert with drop down pit	1300
69.455 km (Culvert 7)	Directly above LW707B	69.455 km	Concrete pipe	1000



**Fig. 6.4 Culvert at 69.0 km (prior to completion of mitigation works)**

Other infrastructure associated with the railway within the Study Area include electrical, signalling and communications systems.

### 6.1.2. Predictions for the Main Southern Railway

The predicted profiles of total vertical subsidence, tilt and curvature along the Main Southern Railway are shown in Fig. C.07, in Appendix C. The predicted total profiles after the completion of LW702 to LW708B and LW901 to LW904 are shown as cyan lines. The predicted total profiles after the mining of each of LW709 to LW711 and LW905 are shown as the blue lines.

A summary of the maximum predicted values of total vertical subsidence, tilt and curvature for the Main Southern Railway is provided in Table 6.4. The values are the maximum predicted subsidence effects within the Study Area due to the mining of the existing, approved and proposed longwalls in Areas 7 and 9.

**Table 6.4 Maximum predicted total vertical subsidence, tilt and curvature for the Main Southern Railway within the Study Area due to the mining in Areas 7 and 9**

Name	After longwalls	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
Main Southern Railway	LW702 to LW708B	1350	6.0	0.07	0.13
	LW709 to LW711 and LW905	1400	6.5	0.07	0.13

The majority of the predicted subsidence effects for the section of railway within the Study Area occur due to the mining of LW702 to LW708B, as it is located directly above these existing longwalls. Only low level additional movements are predicted due to the mining of the LW709 to LW711 and LW905, as the railway is located outside the extents of these proposed longwalls.

A summary of the maximum predicted values of additional vertical subsidence, tilt and curvature for the Main Southern Railway is provided in Table 6.5. The values are the maximum predicted additional subsidence effects for the railway due to the mining of the proposed LW709 to LW711 and LW905 only.

**Table 6.5 Maximum predicted additional vertical subsidence, tilt and curvature for the Main Southern Railway due to the mining of LW709 to LW711 and LW905 only**

Name	Due to longwalls	Maximum predicted additional vertical subsidence (mm)	Maximum predicted additional tilt (mm/m)	Maximum predicted additional hogging curvature (km <sup>-1</sup> )	Maximum predicted additional sagging curvature (km <sup>-1</sup> )
Main Southern Railway	LW709 to LW711 and LW905 only	150	0.5	0.01	< 0.01

The maximum predicted additional tilt for the railway due to the mining of LW709 to LW711 and LW905 only is 0.5 mm/m (i.e. 0.05 %, or 1 in 2000). The maximum predicted additional conventional curvatures are 0.01 km<sup>-1</sup> hogging and less than 0.01 km<sup>-1</sup> sagging, which represent minimum radii of curvature of 100 km and greater than 100 km, respectively.

The maximum predicted additional conventional strains for the railway due to the mining of LW709 to LW711 and LW905 only, based on applying a factor of 15 to the maximum predicted conventional curvatures, are less than 0.5 mm/m tensile and compressive. The distribution of the predicted strains above solid coal is described in Section 4.3.1. The maximum predicted strains outside the extents of the proposed longwalls are 0.6 mm/m tensile and 0.4 mm/m compressive based on the 95 % confidence levels.

Non-conventional movements can also occur and have occurred in the NSW coalfields as a result of, amongst other things, anomalous movements. The analysis of strains provided in Chapter 4 includes those resulting from both conventional and non-conventional anomalous movements.

Summaries of the maximum predicted values of additional vertical subsidence, tilt and curvature for the railway cuttings, embankments and culverts are provided in Table 6.6 to Table 6.8, respectively. The values are the maximum predicted additional subsidence effects for the railway infrastructure due to the mining of the proposed LW709 to LW711 and LW905 only.

**Table 6.6 Maximum predicted additional vertical subsidence, tilt and curvature for the railway cuttings due to the mining of LW709 to LW711 and LW905 only**

Feature	Reference	Maximum predicted additional vertical subsidence (mm)	Maximum predicted additional tilt (mm/m)	Maximum predicted additional hogging curvature (km <sup>-1</sup> )	Maximum predicted additional sagging curvature (km <sup>-1</sup> )
Railway cuttings	68.6 km	100	0.5	0.01	< 0.01
	69.6 km	20	< 0.5	< 0.01	< 0.01

**Table 6.7 Maximum predicted additional vertical subsidence, tilt and curvature for the railway embankments due to the mining of LW709 to LW711 and LW905 only**

Feature	Reference	Maximum predicted additional vertical subsidence (mm)	Maximum predicted additional tilt (mm/m)	Maximum predicted additional hogging curvature (km <sup>-1</sup> )	Maximum predicted additional sagging curvature (km <sup>-1</sup> )
Railway embankment	69.0 km	150	0.5	0.01	< 0.01

**Table 6.8 Maximum predicted additional vertical subsidence, tilt and curvature for the railway culverts due to the mining of LW709 to LW711 and LW905 only**

Feature	Reference	Maximum predicted additional vertical subsidence (mm)	Maximum predicted additional tilt (mm/m)	Maximum predicted additional hogging curvature (km <sup>-1</sup> )	Maximum predicted additional sagging curvature (km <sup>-1</sup> )
Railway culverts	68.621 km	60	< 0.5	< 0.01	< 0.01
	69.000 km	150	0.5	0.01	< 0.01
	69.455 km	30	< 0.5	< 0.01	< 0.01

The railway cuttings, embankments and culverts are predicted to experience additional vertical subsidence up to 150 mm due to the mining of LW709 to LW711 and LW905. Only low level additional movements are predicted due to their distances outside the proposed mining area.

### 6.1.3. Impact assessments for the Main Southern Railway

IMC and the Australian Rail Track Corporation (ARTC) have developed detailed risk management plans for managing potential mine subsidence impacts on the Main Southern Railway due to the extraction of LW703 to LW708B and LW901 to LW904 at Appin Colliery.

The management measures described in these plans are similar to those that have been developed in consultation with ARTC and successfully implemented during the mining of LW25 to LW32 at Tahmoor Colliery.

A Rail Technical Committee has been coordinated to develop the risk management strategies. This Technical Committee includes representatives from ARTC, IMC, and specialist consultants in the fields of railway track engineering, geotechnical engineering, structural engineering, track signalling, mine subsidence, risk assessment and project management. The Technical Committee consults with the Resources Regulator, Department of Regional NSW and the Office of the National Rail Safety Regulator (ONRSR).

Works by the Rail Technical Committee include:

- identification of potential impacts on the railway;
- undertaking a risk management approach, where identified risks are assessed and risk control measures are implemented; and
- development of management measures that include mitigation and preventive works, monitoring plans, triggered response plans and communication plans.

The following sections provide subsidence predictions and likely management measures that will likely be used to manage potential impacts on rail infrastructure during the mining of LW709 to LW711 and LW905.

### 6.1.4. Changes in track geometry

The mining of the proposed LW709 to LW711 could result in changes in track geometry. Changes to track geometry are described using the following parameters:

- vertical misalignment (top) – vertical deviation of the track from design;
- horizontal misalignment (line) – horizontal deviation of the track from design;
- changes in track cant – changes in super-elevation across the rails of each track from design; and
- track twist – changes in super-elevation over a length of track from design.

The Australian Rail Track Corporation's (ARTC) National Code of Practice provide allowable deviations in track geometry. The predicted total changes in track geometry for the Main Southern Railway have been determined using the predicted conventional subsidence effects provided in 6.1.2.

A summary of the maximum allowable and maximum predicted changes in track geometry is provided in Table 6.9. The predicted values are the maximum for the section of railway within the Study Area due to the mining of LW702 to LW711 and LW901 to LW905.

**Table 6.9 Allowable and predicted maximum changes in track geometry for the Main Southern Railway based on conventional subsidence movements**

Track geometry parameter	Description	Maximum allowable (mm)		Maximum predicted based on convention movements (mm)
		Speed limit is first applied	Trains are stopped	
Top	Vertical mid-ordinate deviation over a 10 m chord	38	46	2
Line	Horizontal mid-ordinate deviation over an 8 m chord	35	53	3
Change in cant	Deviation from design super-elevation across rails spaced 1.435 m apart	41	75	8
Long twist	Changes in cant over a 14 m chord	43	65	< 2

The predicted changes in track geometry are considerably less than the maximum allowable deviations specified in the National Code of Practice, if conventional subsidence occurs. For example, the maximum allowable changes in cant across the rails are 41 mm and 75 mm before the trains are respectively slowed and then stopped. In mining terminology, this represents tilts of approximately 30 mm/m to 50 mm/m (i.e. 3 % to 5 %), which are substantially greater than the maximum predicted conventional tilt for the section of railway within the Study Area of 6.5 mm/m.

The changes in track geometry could be greater than those presented in Table 6.9 if non-conventional movements develop along the Main Southern Railway. The potential rates of development of non-conventional movements have been assessed using the ground monitoring data from the previously extracted longwalls at Appin Colliery and elsewhere in the Southern Coalfield.

An example of substantial non-conventional movement in the Southern Coalfield occurred above Appin LW408. In this case, a low angle thrust fault was re-activated in response to mine subsidence movements, resulting in differential vertical and horizontal movements across the fault. Observations at the site showed that the non-conventional movements developed gradually and over a long period of time.

Regular ground monitoring across the low angle thrust fault indicated that the rate of differential movement was less than 0.5 mm/day at the time non-conventional movements could first be detected. Subsequently as mining progressed, the rate of differential movement increased to a maximum of 28 mm/week. In comparison with the National Code of Practice, the maximum allowable deviations in track geometry are much larger than the measured daily rates of change due to mining.

Localised non-conventional subsidence events have adversely impacted on track geometry along the Main Southern Railway. Differential subsidence movements developed gradually at each site, such that visual inspections could detect small changes at an early stage. This allows time to resurface the track in between the passing of trains and return track geometry parameters to within safety limits.

It is therefore considered that while non-conventional movements may potentially result in adverse changes to track geometry, the potential risk to track safety can be managed through early detection via monitoring and early response through the implementation of triggered response plans. A number of management measures are proposed to manage changes in track geometry:

- assess pre-mining track condition and adjust track (if necessary) so that pre-mining track geometry is within normal operating standards prior to the development of subsidence;
- identify potential sites of non-conventional movement, such as creeks and geological structures;
- extend the existing monitoring system, which includes among other things, the monitoring of ground movements, rail stress, rail temperature, switch displacement and track geometry;
- regularly review and assess the monitoring data;
- conduct regular visual inspections of the track; and
- adjust the track in response to monitoring results during mining, if required, to keep the track well within safety limits.

Visual inspections and regular track geometry recordings during the mining of Appin LW703 to LW707, Appin LW901 to LW903 and Tahmoor Longwalls 25 to 32, have confirmed that the impact of normal subsidence movements on track geometry has generally been very low and these very small changes in track geometry developed very gradually.

Significant changes in track geometry have been previously observed at isolated locations along the Main Southern Railway. The key observations are provided below:

- most significant changes in track geometry have been experienced at isolated locations within railway cuttings, where the track formation is founded directly on rock. The majority of the railway cuttings have consisted of weathered shale, as present in the cuttings that are located within the Study Area;
- while substantial differential subsidence movements have developed in the natural ground beneath embankments, there has been little noticeable adverse impact on track geometry. It is considered that the embankment fill buffers the ground movements. An exception to the above experiences was observed at the Embankment at 90.676 km above Tahmoor Colliery, where a change in lateral alignment of approximately 50 mm occurred;
- differential subsidence movements have developed gradually during the mining of each longwall beneath the railway. Rates of change have, however been significant at isolated locations. The track has been resurfaced multiple times during these periods, with Temporary Speed Restrictions imposed as a precaution;
- visual inspections could detect small changes at an early stage, at levels well below the trigger levels;
- the monitoring systems could detect very small changes in the ground and on the track. On some occasions, it has been found that changes in track geometry have occurred when longitudinal ground strains have been relatively minor. Surveys across the cuttings, however, have measured closure of the cutting faces. Surveys are now undertaken along and across the cutting during periods of active subsidence. This includes the cutting at 68.6 km within the Study Area;
- the frequency of track geometry trolley surveys was increased when cutting closure has been observed;
- resurfacing work could be undertaken between trains; and
- localised changes in track geometry in areas of good track condition are more noticeable to drivers. This can result in rough ride reports from train drivers and imposition of Temporary Speed Restrictions well before trigger levels are reached.

With an appropriate management plan in place, it is considered that potential impacts on track geometry can be managed during the mining of LW709 to LW711 and LW905, even if actual subsidence movements are greater than the predictions or substantial non-conventional movements occur.

#### 6.1.5. Changes in track grade

The section of the Main Southern Railway within the Study Area climbs gradually in the southbound direction. The natural surface level is at approximately 123.8 mAHD at kilometrage 68.2 km and is approximately 137.7 mAHD at kilometrage 69.5 km. The maximum gradient for the section of railway within the Study Area is approximately 1 in 30 near kilometrage 68.5 km adjacent to the proposed LW709.

The maximum predicted additional tilt along the Main Southern Railway, due to the mining of LW709 to LW711 and LW905, is 0.5 mm/m (i.e. 0.05 %, or 1 in 2000). The predicted change in grade for the railway is very small and unlikely to result in adverse impacts.

#### 6.1.6. Changes in rail stress

The Railway comprises continuously welded rail and, therefore, changes in temperature induce stresses in the rail. Higher temperatures result in increased compression and lower temperatures result in increased tension in the rail. The Stress Free Temperature (SFT) is the rail temperature at which it is at zero stress.

The predicted change in SFT for continuously welded rail due to mine subsidence has been determined using the following empirical formula:

$$\text{Equation 1: Predicted change in SFT (}^{\circ}\text{C): } E_t = \frac{85.5C_t}{L}$$

where  $C_t$  = change in length due to strain  
 $L$  = long bay length

The review of the ground and rail monitoring data along the Main Southern Railway at Appin and Tahmoor Collieries has found that there is good correlation between the measured and predicted changes in rail SFT when: the change in length due to strain ( $C_t$ ) is determined over 100 m long bay lengths (i.e.  $L = 100$  m); and 100 % transfer of ground strain to rail strain is assumed (i.e.  $\epsilon_{\text{track}} = \epsilon_{\text{ground}}$ ).

A summary of the maximum predicted incremental changes in rail SFT, due to the mining of LW709 to LW711 and LW905, is provided in Table 6.7. The values in this table represent the additional movements due to the mining of each of the proposed longwalls.

**Table 6.10 Maximum predicted incremental changes in rail SFT due to the mining of each of the proposed LW709 to LW711 and LW905**

Location	Longwall	Maximum predicted incremental increase in rail SFT (deg)	Maximum predicted incremental decrease in rail SFT (deg)
Main Southern Railway	LW709	8	-3
	LW710A	< 1	< 1
	LW710B	1	< 1
	LW905	< 1	< 1

The maximum predicted incremental changes in rail SFT are +8°C and -3°C due to the mining of LW709. The predicted changes in rail SFT could be sufficient to result in rail break and possibly rail instability if no preventive measures were to be implemented. The predicted changes in rail SFT for the other proposed longwalls are ±1°C or less. The likelihood of rail break or rail instability is considered to be low.

Management of rail stress during active subsidence has been a primary focus of the Rail Technical Committee. Traditionally, rail stress has been managed in Australia and overseas by rail strain or stress monitoring. Once measured changes in rail stress reach defined triggers, the stress is dissipated by unclipping the rails from the sleepers, cutting the rails and adding steel to, or removing steel from the rails as required, followed by re-stressing the rails to their desired stress. This process is effective but it is labour intensive and very difficult to undertake on busy tracks such as the Main Southern Railway, particularly if the frequency of required rail re-stressing is likely to be more often than weekly, as would be expected during the mining of the proposed longwalls.

For this reason, the Rail Technical Committee has introduced a combination of rail expansion switches and zero toe load clips to dissipate mining and temperature related rail stress during mining. Rail expansion switches consist of a tapered joint in the track, which allow the rails on each side of the joints to slide independently. Maximum allowable displacements of expansion switches vary between different types of switches and those that have been employed above the existing longwalls at Appin Colliery have a capacity of approximately 310 mm. Expansion switches are standard rail equipment and operate in non-subsidence applications in Australia and overseas to accommodate, for example, differential thermal movements between bridges and natural ground. A photograph of a rail expansion switch is shown in Fig. 6.5.



**Fig. 6.5 Rail expansion switch**

Zero toe load clips allow the rails to slide longitudinally along the track while maintaining lateral stability. In combination, the rails are able to expand or contract in response to mine subsidence and thermal loads into and out of the expansion switches. It is estimated that the switches will be spaced between 200 m and 400 m apart along the track within the subsidence area.

The combination of expansion switches and zero toe load clips has been successfully employed during the mining of LW703 to LW707B, LW901 and LW902 at Appin Colliery and LW25 to LW32 at Tahmoor Colliery.

A significant advantage of using rail expansion switches and zero toe load clips is that the system is flexible and can be adjusted during mining should the tolerance of the switches reach their design limits. The rails can be cut and steel can be either added or removed as necessary to restore capacity in the switches. The process is significantly faster than conventional re-stressing work as the clips do not have to be removed and reinstated and no stressing work is required. The process can be safely achieved in between the passage of trains without delaying the operation of trains.

It is likely that the following management measures will be used to manage changes in rail stress:

- assess pre-mining track condition and adjust track if required so that pre-mining track geometry and sleeper arrangements are at or close to design prior to the development of subsidence;
- identify potential sites of non-conventional movement, such as creeks and geological structures;
- assess the required spacing of expansion switches based on the predicted ground movements;
- install the expansion switches and zero toe load clips;
- install a monitoring system, which includes, among other things, the monitoring of ground movements, rail stress, rail temperature, switch displacement and track geometry;
- regularly review and assess the monitoring data;
- conduct regular visual inspections of the track, switches and clips; and
- adjust the track in response to monitoring results during mining if required.

It is considered that with the adoption of appropriate management measures, changes to rail stress can be managed, even if actual subsidence movements are greater than the predictions or substantial non-conventional movements occur.

#### **6.1.7. Potential impacts on the railway cuttings**

The maximum predicted additional vertical subsidence for the railway cuttings, due to the mining of LW709 to LW711 and LW905, are 100 mm at 68.6 km and 20 mm at 69.6 km. Only low level additional movements are predicted at the cuttings as they are located outside the extents of the proposed longwalls.

Fracturing could occur in the cutting at 68.6 km, where it is located closest to LW709, and this could result in spalling of rock if it is marginally stable. Fracturing in cutting 69.6 km is less likely due to its distance from the proposed longwalls and the very low levels of predicted movement.

In the unlikely event that the faces of these cuttings are impacted by mine subsidence, the failure is likely to be very minor, in the form of small fragments of rock, and likely to fall into the clear area adjacent to the railway, referred to as *the cess* (Christie, 2010).

The Rail Technical Committee will consider mitigation measures before the cuttings experience subsidence effects. Mitigation works could include, for example, scaling the cutting faces and removing debris from the cess. The following management measures could be used to manage potential impacts on the cuttings:

- assess condition of the cuttings prior to mining;
- consider and implement mitigation measures such as scaling the cutting faces and removing debris from the cess;
- install a monitoring system, which includes, among other things, the monitoring of ground movements at the cuttings;
- regularly review and assess the monitoring data;
- conduct regular visual inspections of the cuttings; and
- clear the cess of debris if required based on observations during mining.

It is considered that with the adoption of appropriate management measures, the potential impacts on the railway cuttings can be managed, even if actual subsidence movements are greater than the predictions or substantial non-conventional movements occur.

#### **6.1.8. Potential impacts on the railway embankment**

The maximum predicted additional vertical subsidence for the railway embankment at 69.0 km, due to the mining of LW709 to LW711 and LW905, is 150 mm. This embankment is located above the completed LW707B and LW708B and it is predicted to have already experienced vertical subsidence up to 1150 mm due to the mining of those longwalls.

IMC has appointed an embankment sub-committee, which has completed detailed studies and reviews on potential changes to embankment stability as a result of mine subsidence for the embankment at 69.0 km. Reviews were completed in 2016, 2019 and 2020.

- detailed site surveys of embankment geometry;
- geotechnical investigations of the embankment material;
- finite element modelling of potential changes to embankment stress and strain due to mine subsidence;
- slope stability analyses of existing embankment condition and potential condition if a tension crack formed in the embankment; and
- reviews of monitoring results during the mining of LW707B.

Following the review in 2016, IMC implemented additional controls to manage potential impacts on the embankment. The additional controls included earthworks to provide additional stability at the deepest sections on both sides of the embankment near the culvert. The earthworks were completed in late 2016, prior to the influence of LW707B. The Embankment at 69.000 km experienced minor changes during the mining of LW707B.

Prior to the influence of LW708B in 2020, the sub-committee conducted another review. It confirmed that there is sufficient robustness in the current stability of the embankment, with existing strengthening measures in place, to accommodate the projected subsidence movements and impacts from the extraction of LW 708B, and ensure that planned contingency measures can be effectively implemented in a timely manner to ensure that the embankment remains safe and serviceable during mining. Additional works were recommended by ARTC at the drop pit to improve drainage at the site and these have been implemented by IMC.

The Rail Technical Committee will consider whether further investigations or mitigation measures should be carried out prior to the mining of the proposed LW709 to LW711 and LW905. The following management measures will be considered to manage potential impacts on the embankment:

- assess pre-mining condition of the embankments;
- consider and implement mitigation measures such as cleaning out of culverts and strengthening of the culverts to prevent collapse;
- install a monitoring system, which includes, among other things, the monitoring of ground movements at the embankments;
- regularly review and assess the monitoring data;
- conduct regular visual inspections of the embankments and culverts; and
- provide additional culvert support in response to actual measurements and observations during mining.

It is considered that with the adoption of appropriate management measures, the potential impacts on the railway embankment can be managed, even if actual subsidence movements are greater than the predictions or substantial non-conventional movements occur.

#### **6.1.9. Potential impacts on the railway culverts**

The maximum predicted additional vertical subsidence for the railway culverts, due to the mining of LW709 to LW711 and LW905, are 60 mm at 68.621 km, 150 mm at 69.000 km and 30 mm at 69.455 km. Only low level additional movements are predicted at the culverts as they are located outside the extents of the proposed longwalls. The culverts at 69.000 km and 69.455 km are located above the completed LW707B and LW708B and they are predicted to have already experienced vertical subsidence up to 1100 mm due to the mining of those longwalls.

The culvert at 69.000 km is a brick arch culvert and there is potential for physical impacts to occur. The potential for impact is greatest due to the mining of LW708B directly beneath it; however, lesser impacts could occur due to the mining of the proposed LW709. It is possible that this culvert could experience some cracking and spalling of the masonry due to the mining of the proposed longwalls. Cracking may occur in the masonry arch or in the headwalls. While predicted movements are not considered likely to result in collapse of the culvert, IMC inserted a reinforced concrete pipe sleeve inside the culvert in 2016 prior to the influence of LW707B. Modifications have also been made to the drop down pit to improve drainage flows.

The culverts at 68.621 km and 69.455 km are concrete culverts. Only low level additional movements are predicted at these culverts, up to 60 mm vertical subsidence, due to their distances outside the extents of the proposed longwalls. The potential for adverse impacts on these two culverts is less than that for the brick arch culvert.

However, given the potentially severe consequences of culvert collapse, the Rail Technical Committee will consider mitigation measures prior to each culvert experiencing subsidence effects. The following management measures will be considered to manage potential impacts on the railway culverts:

- assess pre-mining condition of culverts;
- consider and implement mitigation measures to reduce or avoid the potential for culvert collapse. In the case of the culvert at 69.000 km, a reinforced concrete pipe has been inserted into the culvert;
- install a monitoring system, which includes, among other things, the monitoring of ground movements around the culvert and change in track geometry and rail stress;
- regularly review and assess the monitoring data;
- conduct regular visual inspections of the culverts; and
- provide additional track and/or culvert support in response to actual measurements and observations during mining.

It is considered that with the adoption of appropriate management measures, potential impacts on railway culverts can be managed, even if actual subsidence movements are greater than the predictions or substantial non-conventional movements occur.

#### **6.1.10. Recommendations for the Main Southern Railway**

IMC has developed management strategies for the Main Southern Railway for LW703 to LW708B and LW901 to LW903. It is recommended that these management plans are reviewed and updated to incorporate the proposed LW709 to LW711 and LW905.

### **6.2. M31 Hume Motorway**

The locations of the M31 Hume Motorway and its associated infrastructure are shown in Drawing No. MSEC1117-09.

The M31 Hume Motorway is located outside and to the east of the Study Area. The motorway is located 470 m from the south-eastern corner of LW709, at its closest point to the proposed longwalls. The motorway is located directly above the existing LW703 to LW708B.

The predicted additional vertical subsidence for the M31 Hume Motorway due to the mining of the proposed LW709 to LW711 and LW905 is less than 20 mm. While the motorway could experience very low levels of vertical subsidence, it is not predicted to experience measurable tilts, curvatures or strains.

It is not anticipated therefore that the M31 Hume Motorway would experience adverse impacts due to the mining of the proposed LW709 to LW711 and LW905. The discussions on the Nepean Twin Bridges are provided in Section 6.4 and the road bridges over the motorway are provided in Section 6.5.

### **6.3. Local roads**

#### **6.3.1. Descriptions of the local roads**

The locations of the local roads are shown in Drawing No. MSEC1117-09.

The main local road within the Study Area is Menangle Road. This road provides a connection between the township of Campbelltown, located north-east of the Study Area, and Picton Road, to the south-west of the Study Area. There are also a number of local roads located across the Study Area.

A summary of the main local roads within the Study Area is provided in Table 6.11. There are other local roads within the Study Area that are not included in this table.

**Table 6.11 Main local roads located within the Study Area**

Name	Location	Total length above the proposed longwalls (km)	Total length within the Study Area (km)
Cummins Road	Directly above LW711	0.1	0.6
Carrolls Road	Directly above LW709, LW710B and LW711	1.3	2.3
Donalds Range Road	Outside proposed mining area	-	1.2
Finns Road	Partially above LW711	0.1	0.5
Hawkey Road	Directly above LW710B and LW905	0.8	2.1
Menangle Road	Directly above LW709, LW710B and LW711	1.2	5.1
Quirkas Road	Directly above LW711	1.8	2.4
Top Ridge/Gibraltar Road	Directly above LW710A and LW711	0.7	1.1

The local roads have single carriageways with bitumen seals. Some local roads also have concrete kerb and guttering. A photograph of Menangle Road is provided in Fig. 6.6.



**Fig. 6.6 Photograph of Menangle Road in Area 9**

The local roads are owned and maintained by the Wollondilly Shire Council.

### **6.3.2. Predictions for the local roads**

The predicted profiles of total vertical subsidence, tilt and curvature along Menangle Road and Carrolls Road are shown in Figs. C.08 and C.09, respectively, in Appendix C. The predicted total profiles after the completion of LW702 to LW708B and LW901 to LW904 are shown as cyan lines. The predicted total profiles after the mining of each of LW709 to LW711 and LW905 are shown as the blue lines.

Summaries of the maximum predicted values of total vertical subsidence, tilt and curvature for the Menangle Road and Carrolls Road are provided in Table 6.12 and Table 6.13, respectively. The values are the maximum predicted subsidence effects within the Study Area due to the mining of the existing, approved and proposed longwalls in Areas 7 and 9.

**Table 6.12 Maximum predicted total vertical subsidence, tilt and curvature for Menangle Road within the Study Area due to the mining in Areas 7 and 9**

Name	After longwalls	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
Menangle Road	LW708B	1300	5.5	0.06	0.12
	LW709 to LW711 and LW905	1400	5.0	0.06	0.12

**Table 6.13 Maximum predicted total vertical subsidence, tilt and curvature for Carrolls Road within the Study Area due to the mining in Areas 7 and 9**

Name	After longwalls	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
Carrolls Road	LW708B	70	< 0.5	< 0.01	< 0.01
	LW709	650	3.5	0.05	0.08
	LW710A/B	1050	5.0	0.06	0.11
	LW711 and LW905	1300	6.0	0.07	0.12

The other local roads are located across the Study Area and, therefore, could experience the full range of predicted subsidence movements. A summary of the maximum predicted conventional subsidence effects within the Study Area is provided in Chapter 4.

The maximum predicted total tilt for the local roads within the Study Area is 7 mm/m (i.e. 0.7 %, or 1 in 143). The maximum predicted total conventional curvatures are 0.08 km<sup>-1</sup> hogging and 0.15 km<sup>-1</sup> sagging (refer to Table 4.2), which represent minimum radii of curvature of 13 km and 7 km, respectively.

The maximum predicted conventional strains for the local roads, based on applying a factor of 15 to the maximum predicted conventional curvatures, are 1.0 mm/m tensile and 2.0 mm/m compressive. The distribution of the predicted strains due to the extraction of the longwalls is described in Section 4.3. The maximum predicted strains directly above the mining area are 1.0 mm/m tensile and 1.6 mm/m compressive based on the 95 % confidence levels.

Non-conventional movements can also occur and have occurred in the NSW coalfields as a result of, amongst other things, anomalous movements. The analysis of strains provided in Chapter 4 includes those resulting from both conventional and non-conventional anomalous movements.

### 6.3.3. Impact assessments for the local roads

There is extensive experience of mining directly beneath local roads in the Southern Coalfield which indicates that impacts can be managed with the implementation of suitable management strategies. In all cases the local roads have remained in safe and serviceable conditions and have been remediated using normal road maintenance techniques.

A summary of the experience of mining beneath local roads at Appin Colliery and other nearby collieries is provided in Table 6.14. the predicted subsidence effects for the local roads within the Study Area are similar to the measured values for these case studies.

**Table 6.14 Experience of mining beneath local roads at Appin Colliery and other nearby collieries**

Road	Length mined beneath and longwalls	Measured movements	Observed impacts
Appin Road	3.9 km mined beneath by West Cliff LW5A3, LW5A4 and LW29 to LW37	1350 mm subsidence 11 mm/m tilt 1.5 mm/m tensile strain 5.5 mm/m comp. strain (Measured B-Line)	Localised depression above LW5A3. Bumps up to 100 mm and cracking up to 10 mm in pavement above LW32 to LW37
Brooks Point Road	2.4 km mined beneath by Appin LW1, LW2 and LW405 to LW409	700 mm subsidence 5 mm/m tilt 1 mm/m tensile strain 2 mm/m comp. strain (Measured A6000-Line)	Bump approximately 100 mm in pavement above LW408
Menangle Road	2.2 km mined beneath by Appin LW707A, LW708A and LW903	875 mm subsidence 8.5 mm/m tilt 1.5 mm/m tensile strain 8.5 mm/m comp. strain (Measured MR-Line)	Compressive heaving developed above LW707A near base of hill. Minor crack in pavement elsewhere along road
Moreton Park Road	3.8 km mined beneath by Appin LW702 to LW707B	1300 mm subsidence 7.5 mm/m tilt 1.0 mm/m tensile strain 5.0 mm/m comp. strain (Measured MPR-Line)	Minor cracking and localised bumps in pavement
Wilton Road	2.6 km mined beneath by Appin LW1, LW2, LW15, LW16, LW301 and LW302	650 mm subsidence 4.5 mm/m tilt 1 mm/m tensile strain 3 mm/m comp. strain (Measured M & N-Lines)	Some minor impacts to the road surface were observed above LW301 and LW302

The impacts on these local roads did not present an immediate public safety risk and were remediated using normal road maintenance techniques. Photographs of the impacts observed along Appin Road are provided in Fig. 6.7 and Fig. 6.8.



**Fig. 6.7 Bump in slip lane along Appin Road above West Cliff Longwall 32 (Source: Colin Dove)**



**Fig. 6.8 Tension crack in Appin Road above West Cliff Longwall 32 (Source: Colin Dove)**

The mining of LW707A and LW708A beneath Menangle Road resulted in a dip developing in the pavement near the top of Spaniards Hill and a compression bump developing near the base of the hill. A photograph of these impacts is provided in Fig. 6.9.



**Fig. 6.9 Compression bumps in Menangle Road (Source: Network Solutions)**

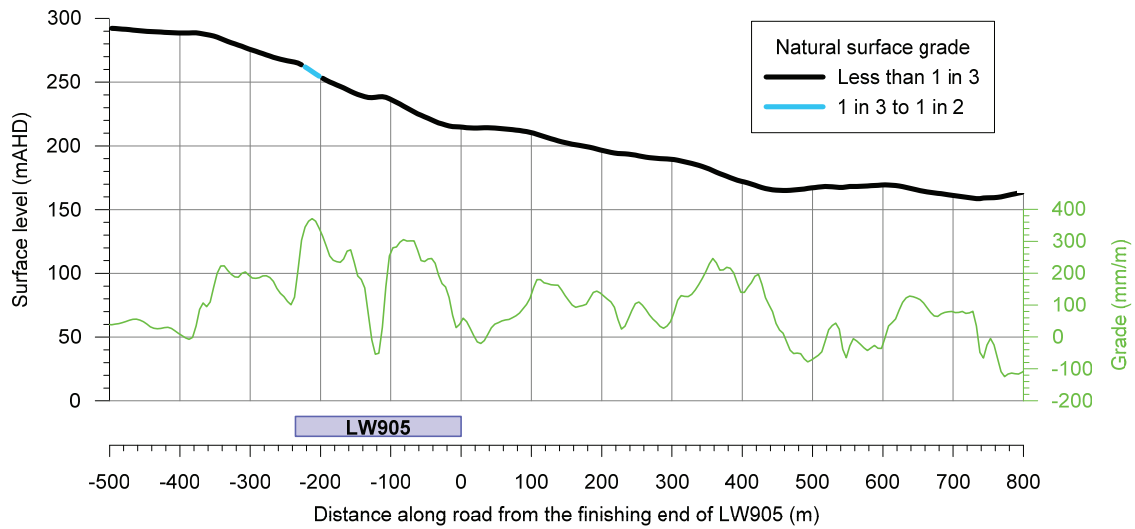
Menangle Road was maintained in safe and serviceable conditions using weekly ground monitoring and visual inspections. Preventive measures were carried out including milling of the compression bump and pavement repairs. Warning signs were also installed along the road during active subsidence.

There is also extensive experience of mining beneath local roads at Tahmoor Colliery. Longwalls 22 to 21 have mined directly beneath more than 28 km of local roads and a total of 52 impact sites have been observed. The observed rate of impact on the local roads equates to an average of one impact for every 540 m of pavement. In most cases, the impacts were relatively minor and were remediated by locally resurfacing the pavements.

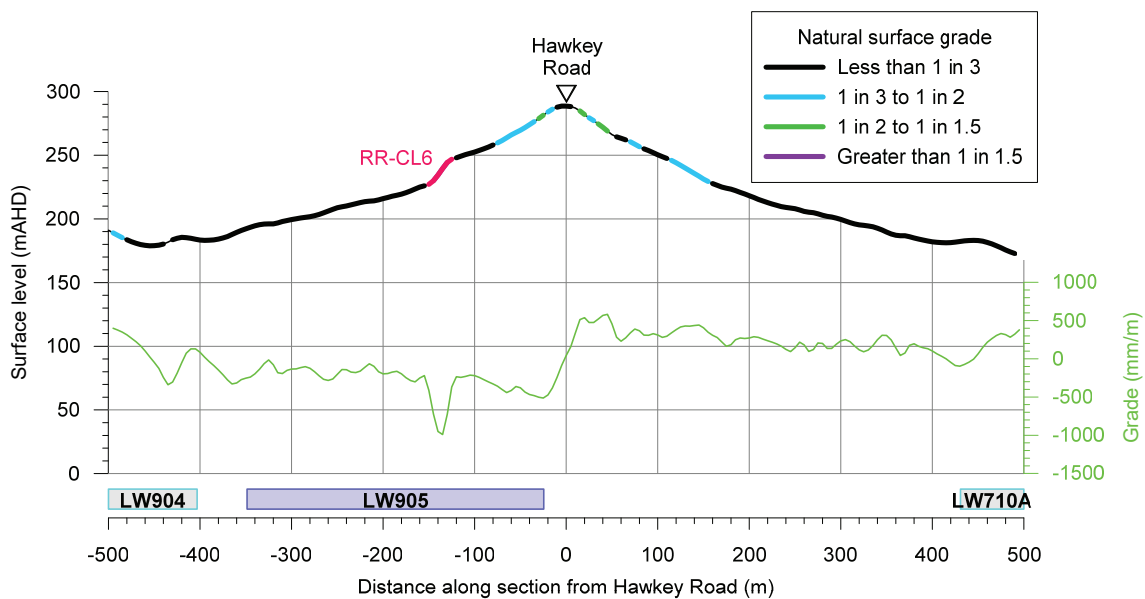
The most severe impacts at Tahmoor Colliery were located where substantial non-conventional movements had developed. These impact sites were identified using visual and ground monitoring and remediation was undertaken during active subsidence to maintain these roads in safe and serviceable conditions. Impacts have also been observed to concrete kerbs, gutters and drainage pits. The impacts are most commonly focussed on driveway laybacks and involve cracking, spalling or buckling.

The western section of Hawkey Road traverses a ridgeline of Razorback Range above the maingate of the proposed LW905. Top Ridge Road, Gibraltar Drive other local roads are located at the top of Razorback Range above the proposed LW710A and the western end of the proposed LW711.

The surface level and natural grade along the alignment of the alignment of Hawkey Road are illustrated in Fig. 6.10. The surface level and natural grade across the alignment of the road, at the top of Razorback Range, are illustrated in Fig. 6.11.



**Fig. 6.10** Surface level and natural grade along the alignment of Hawkey Road



**Fig. 6.11** Surface level and natural grade across the alignment of Hawkey Road

The natural grades along the alignment of Hawkey Road (refer Fig. 6.10) are typically less than 1 in 3 (i.e. 18° or 33 %). The natural grades on the ridgeline either side of Hawkey Road vary between 1 in 3 and 1 in 1.5 (i.e. 34° or 67 %). A cliff is also located on the southern side of the ridgeline, approximately 130 m from the centreline of the road.

The mining of LW905 beneath the western section of Hawkey Road could result in increased horizontal movements in the downslope direction causing tensile cracking in the road pavement. Similarly, tensile cracking could also develop in the other local roads located on the top of the ridgeline and above the proposed longwalls. It is recommended that ground movement and visual monitoring are carried out along these roads during active subsidence to identify increased or irregular movements.

Further discussions on the potential impacts on the steep slopes are provided in the reports by GHD (2021a, 2021b, 2021c and 2021d).

#### 6.3.4. Recommendations for the local roads

IMC has developed management strategies for the local roads for LW703 to LW708B and LW901 to LW903. It is recommended that these management plans are reviewed and updated to incorporate the proposed LW709 to LW711 and LW905.

#### 6.4. Nepean Twin Bridges

The locations of the road bridges are shown in Drawing No. MSEC1117-09.

The Nepean Twin Bridges are located where the M31 Hume Motorway crosses the Nepean River. These twin bridges are 1 km south of the completed LW901 and 1.8 km south-west of the completed LW703. The bridges are located 2.5 km south LW905, at their closest point to the proposed longwalls.

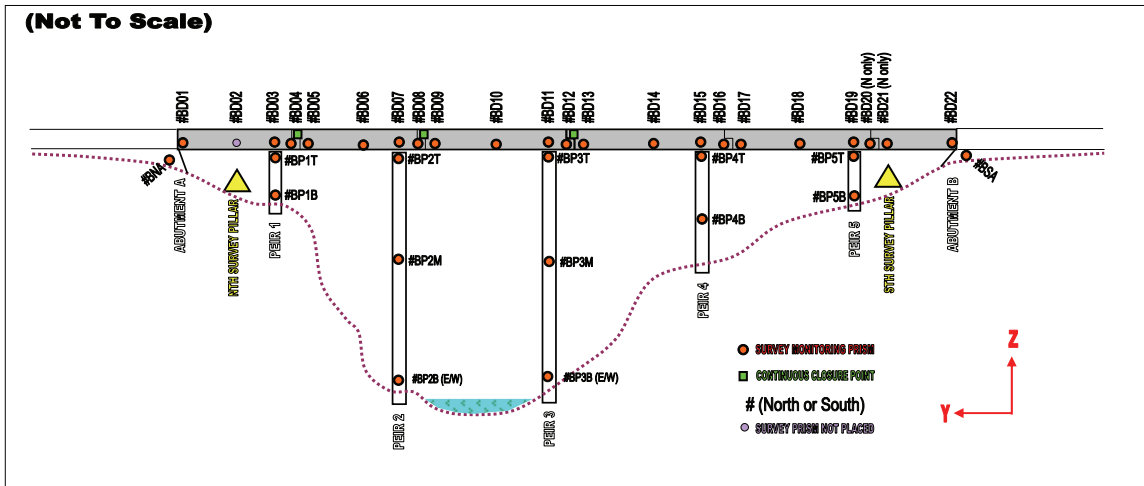
The Nepean Twin Bridges are located outside the Study Area for LW709 to LW711 and LW905. However, they could experience far-field horizontal movements and could be sensitive to these effects. These bridges have therefore been included in the impact assessments.

The Nepean Twin Bridges have an overall length of approximately 235 m for the southbound carriageway and 285 m for the northbound carriageway. Both bridges consist of reinforced concrete bridge decks and piers. A photograph of these bridges is provided in Fig. 6.12.



**Fig. 6.12 Nepean Twin Bridges along the M31 Hume Motorway over the Nepean River**

An indicative elevation of the Nepean Twin Bridges is provided in Fig. 6.13 (Source: IMC), showing the relative locations of the supporting columns, which are spaced approximately 50 m apart. The figure also shows the locations of the 3D relative monitoring points that have been used to measure the movements during the mining of LW701 to LW708B and LW901 to LW903. The marks at the bases of the column were also measured during the extraction of Tower Longwalls 16 and 17.

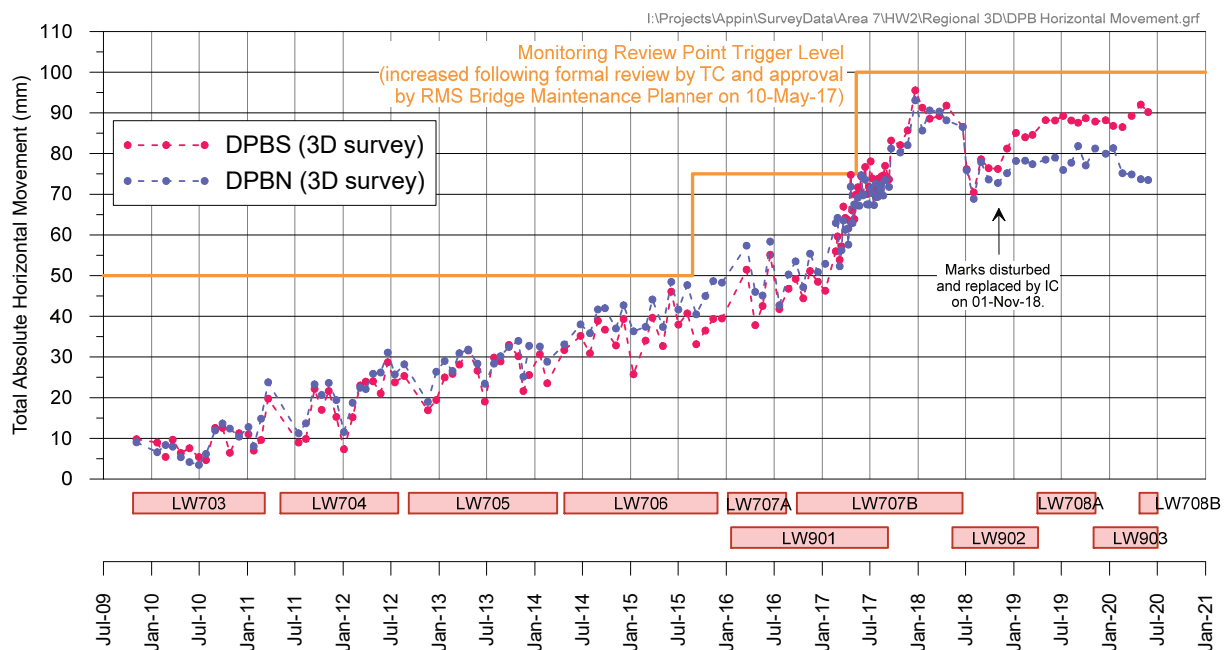


**Fig. 6.13 Indicative elevation of the Nepean Twin Bridges (Source: IMC)**

Monitoring has been established on the Nepean Twin Bridges and has been measured during the mining of LW701 to LW707B and LW901 to LW903. The monitoring includes absolute 3D bridge monitoring points, relative 3D bridge monitoring points, inclinometer monitoring, bridge joint monitoring and visual inspections.

Management Plans have been developed for the Nepean Twin Bridges for the mining of longwalls in Appin Areas 7 and 9. A Technical Committee has been established, to review the monitoring data and carry out actions in accordance with the Trigger Action Response Plan (TARP).

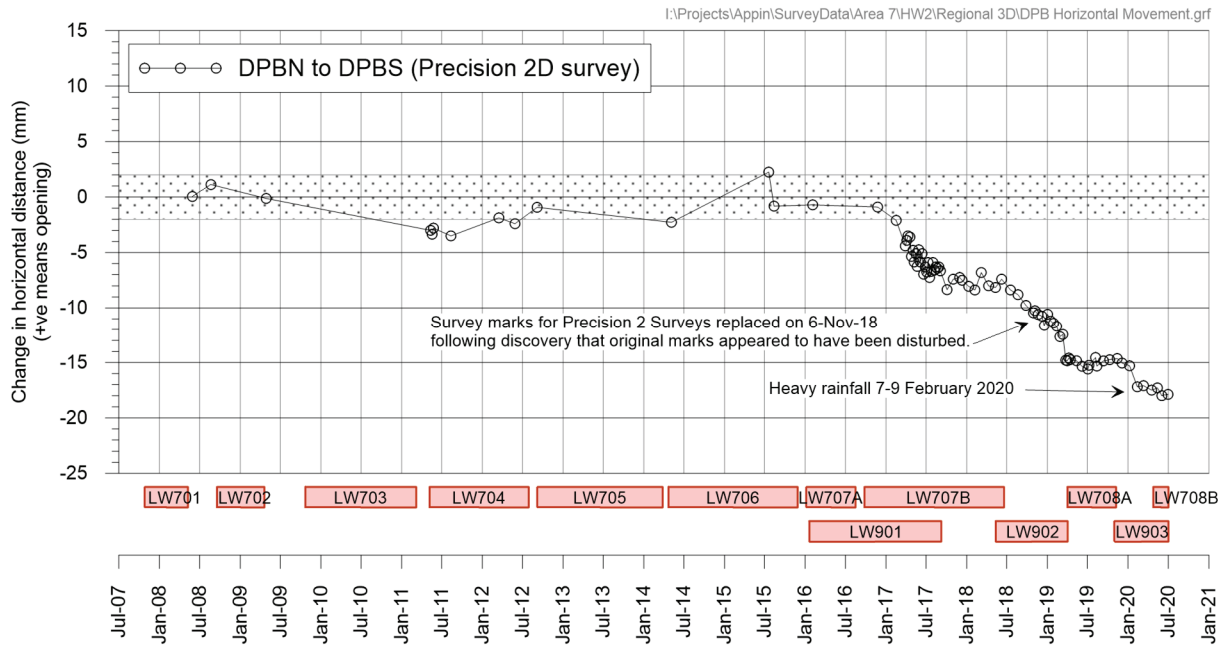
The measured absolute horizontal movements at the 3D marks at the northern end (DPBN) and southern end (DPBS) of the Nepean Twin Bridges are illustrated in Fig. 6.14. These marks have been monitored during the mining of LW701 to LW707B, LW901 and LW902. The Monitoring Review Point trigger level is also shown as the orange line in this figure.



**Fig. 6.14 Measured absolute horizontal movements at Marks DPBN and DPBS**

The Technical Committee has carried out formal reviews and agreed to increase the Monitoring Review Point trigger level on two occasions, from 50 mm to 75 mm and then 75 mm to 100 mm during mining. The decisions were based on the surveys of the bridges, monitoring of displacement sensors and FBGs at the bridge joints, which indicated no measurable differential lateral movement.

The measured relative movements between the Marks DPBN and DPBS at either end of the Nepean Twin Bridges are illustrated in Fig. 6.15.



**Fig. 6.15 Measured relative horizontal movements between Marks DPBN and DPBS**

The maximum measured relative horizontal movement between Marks DPBN and DPBS exceeded the Level 1 trigger of 5 mm. In response to this trigger, additional relative 3D surveys of the bridges were carried out and it was found that the measured lateral alignments of the bridges were within the allowable tolerances. IMC and RMS (now Transport for NSW) commissioned an engineering inspection and review of the Douglas Park Bridges by Cardno (2020). The review included a detailed measurement of the gaps at the expansion joint finger plates of both bridges by RMS. The review found that there remains up to 55 mm of additional capacity to accommodate valley closure movements at the bridges, considering the capacity to accommodate thermal expansion of the bridges.

The Nepean Twin Bridges will experience additional low-level far-field horizontal movements due to the mining of the proposed LW709 to LW711 and LW905. The longwall series in Areas 7 and 9 are mining away from the bridges and, therefore, the incremental movements decrease for successive longwalls.

IMC has an approved management plan for the Nepean Twin Bridges for LW701 to LW708B and LW901 to LW903. It is recommended that this management plan is reviewed and updated to incorporate the proposed LW709 to LW711 and LW905.

## 6.5. Road bridges

Moreton Park Road Bridge (North) is located 1.5 km north-east of the proposed LW709, LW710B and LW711. Moreton Park Road Bridge (South) is located 0.6 km south-west of the completed LW703 and is more than 2 km south of the proposed longwalls.

Moreton Park Road Bridge (North and South) are located outside the Study Area for LW709 to LW711 and LW905. However, they could experience far-field horizontal movements and could be sensitive to these effects. These bridges have therefore been included in the impact assessments.

The two bridges cross the M31 Hume Motorway and have overall lengths of approximately 100 m. The bridges are supported by three single piers, spaced at approximately 30 m apart, with concrete abutments at each end. Photographs of the Morten Park Road Bridge (North) and Moreton Park Road Bridge (South) are provided in Fig. 6.16 and Fig. 6.17, respectively.



**Fig. 6.16 Moreton Park Road Bridge (North)**



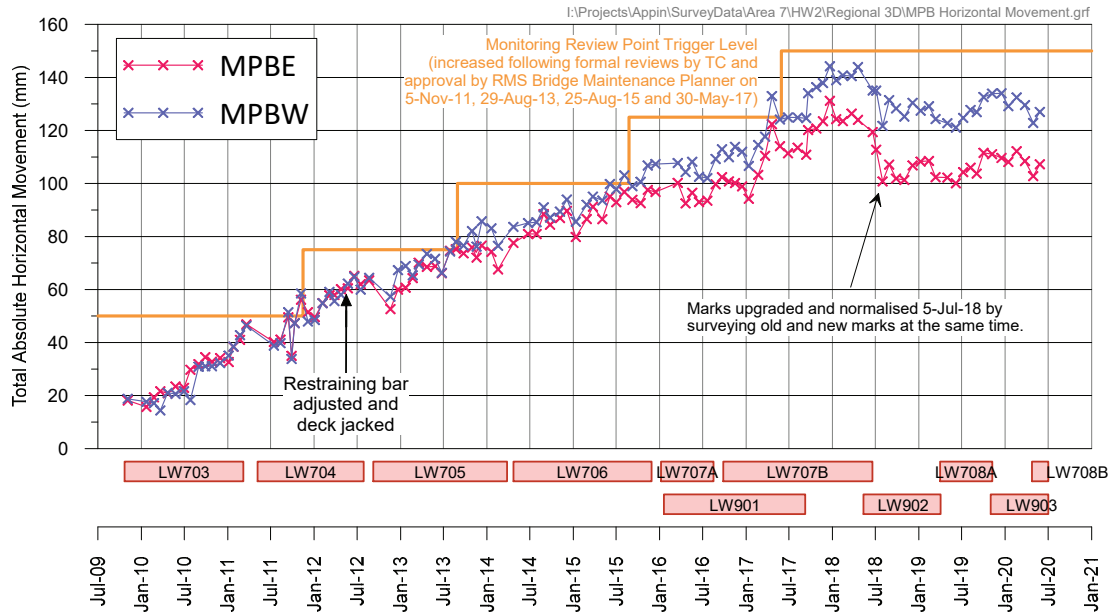
**Fig. 6.17 Moreton Park Road Bridge (South)**

The discussions on the Nepean Twin Bridges where the M31 Hume Motorway crosses the Nepean River are provided in Section 6.4.

Monitoring has been established on Moreton Park Road Bridge (South) and has been measured during the mining of LW701 to LW707B and LW901 to LW903. The monitoring includes absolute 3D bridge monitoring points, relative 3D bridge monitoring points and visual inspections.

Management Plans have been developed for the bridge for the mining of longwalls in Appin Areas 7 and 9. A Technical Committee has been established, to review the monitoring data and carry out actions in accordance with the Trigger Action Response Plan (TARP).

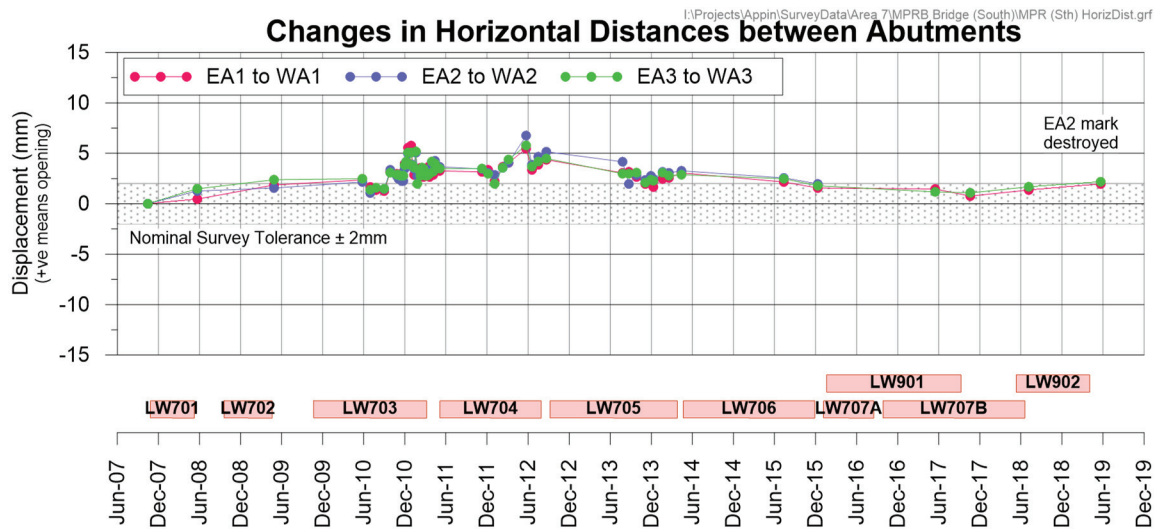
The measured absolute horizontal movements at the 3D marks at the eastern end (MPBE) and western end (MPBW) of Moreton Park Road Bridge (South) are illustrated in Fig. 6.18. These marks have been monitored during the mining of LW703 to LW707B, LW901 and LW902. The Monitoring Review Point trigger level is also shown as the orange line in this figure.



**Fig. 6.18 Measured absolute horizontal movements at Marks MPBE and MPBW**

The Technical Committee has carried out formal reviews and agreed to increase the Monitoring Review Point trigger level on four occasions. The decisions were based on the relative, which indicated that the differential movements were in the order of survey tolerance.

The measured relative movements between the Marks MPBE and MPBW at either end of Moreton Park Road Bridge (South) are illustrated in Fig. 6.19.



**Fig. 6.19 Measured relative horizontal movements between Marks MPBE and MPBW**

There was a small amount of abutment spreading, in the order of +5 mm, that developed during the extraction of LW703 to LW705. The results vary slightly between surveys and the cause is thought to be related to changes in moisture and/or temperature. The measured total changes in horizontal distance between the bridge abutments were less than  $\pm 2$  mm at the completion of LW707B and LW902. The total measured movements, therefore, were within the order of survey tolerance at the completion of this longwall.

Moreton Park Road Bridge (South) will experience additional low-level far-field horizontal movements due to the mining of the proposed LW709 to LW711 and LW905. The longwall series in Areas 7 and 9 are mining away from this bridge and, therefore, the incremental movements decrease for successive longwalls. Moreton Park Road Bridge (North) will also experience low-level far-field horizontal movements due to the mining of the proposed longwalls.

IMC has an approved management plan for Moreton Park Road Bridge (South and North) for LW701 to LW708B and LW901 to LW903. It is recommended that this management plan is reviewed and updated to incorporate the proposed LW709 to LW711 and LW905.

## 6.6. Road culverts

Road culverts are located where the local roads cross the streams or other topographical depressions. The culverts comprise concrete pipes or concrete box culverts with concrete headwalls.

The road culverts are located across the Study Area and, therefore, are expected to experience the full range of predicted subsidence movements. A summary of the maximum predicted conventional subsidence movements within the Study Area is provided in Chapter 4.

The maximum predicted tilt for road culverts is 7 mm/m (i.e. 0.7 %, or 1 in 143). It is unlikely that the mining-induced tilts would result in adverse impacts on the serviceability of the culverts, as the changes in grade are less than 1 %. If the flow of water through the culverts were to be adversely affected, due to the proposed mining, they could be remediated by re-levelling the affected culverts.

The predicted curvatures and strains could be of sufficient magnitudes to result in cracking in the culverts or the headwalls. It is unlikely, however, that these movements would adversely impact on the stabilities or structural integrities of the culverts. The potential impacts on the road culverts could be managed by visual inspection and, where required, the affected culverts can be repaired or replaced.

The road culverts are located along the streams and therefore could experience valley related effects. The culverts are orientated along the alignments of the streams and, therefore, the upsidence and closure movements are orientated perpendicular the main axes of the culverts and unlikely therefore to result in adverse impacts.

Experience of mining beneath road culverts in the Southern Coalfield indicates that the incidence of impacts is low. Impacts have generally been limited to cracking in the concrete headwalls which can be readily remediated. In some cases, however, cracking in the culvert pipes occurred which required the culverts to be replaced.

IMC has an approved management plan for the local roads, including the drainage culverts, for LW701 to LW708B and LW901 to LW903. It is recommended that this management plan is reviewed and updated to incorporate the proposed LW709 to LW711 and LW905.

## 6.7. Water infrastructure

The potable water infrastructure is shown in MSEC1117-12. The locations and details of this infrastructure are based on a Dial Before You Dig search (DB4YD, 2020) and follow up correspondence with Sydney Water.

There is no potable water infrastructure identified within the Study Area. Potable water pipelines are located within Douglas Park to the south of the Study Area. These pipelines are located more than 1.1 km from the proposed LW709 to LW711 and LW905. At this distance, the potable water pipelines are not predicted to experience measurable conventional subsidence effects. It is not anticipated, therefore, that adverse impacts would occur to these pipelines due to the proposed mining.

## 6.8. Sewerage infrastructure

The sewerage infrastructure is shown in MSEC1117-12. The locations and details of this infrastructure are based on a Dial Before You Dig search (DB4YD, 2020) and follow up correspondence with Sydney Water.

There is no sewerage infrastructure identified within the Study Area. A local sewer network is proposed to be constructed in the Menangle region in the next one to two years. The new infrastructure would need to be designed and constructed to accommodate mine subsidence effects prescribed by SA NSW.

## 6.9. Gas infrastructure

The gas infrastructure is shown in MSEC1117-12. The locations and details of this infrastructure are based on a Dial Before You Dig search (DB4YD, 2020) and follow up correspondence with AGL.

There is no gas infrastructure identified within the Study Area. A local gas distribution network services the houses located along Menangle Road and Finns Road to the north of the Study Area. These services are located more than 500 m from the proposed mining area. At this distance, the local gas distribution network is not predicted to experience measurable conventional subsidence effects. It is not anticipated, therefore, that adverse impacts would occur to the local gas distribution network due to the proposed mining.

## 6.10. Electrical infrastructure

### 6.10.1. Descriptions of the powerlines

The locations of the electrical infrastructure are shown in Drawing No. MSEC1117-10.

The electrical infrastructure within the Study Area comprises 66 kV powerlines, 11 kV powerlines and low voltage powerlines. There are no transmission lines located within the Study Area.

There are three 66 kV powerline located within the Study Area. All three powerlines cross directly above the proposed LW709, LW710B and LW711 and one of these powerlines also crosses directly above the proposed LW905. The 11 kV and low voltage powerlines are located across the Study Area and the generally follow the alignments of the local roads.

A summary of the lengths of the powerlines within the Study Area is provided in Table 6.15.

**Table 6.15 Powerlines within the Study Area**

Type	Location	Total length above the proposed longwalls (km)	Total length within the Study Area (km)
66 kV powerlines	Directly above LW709, LW710B, LW711 and LW905	3.3	8.4
11 kV powerlines	Directly above LW709 to LW711 and LW905	6.6	17.1
Low voltage powerlines	Directly above LW709 to LW711 and LW905	4.3	9.6

The 66 kV, 11 kV and low voltage powerlines consist of aerial copper cables supported on timber poles. A photograph of the 66 kV powerline (right-hand side) and an 11 kV powerline (left-hand side) along Menangle Road is provided in Fig. 6.20.



**Fig. 6.20 66 kV and 11 kV powerlines along Menangle Road**

The powerlines are owned and operated by Endeavour Energy.

### 6.10.2. Predictions for the powerlines

The predicted profiles of total vertical subsidence, tilt along and tilt across the alignment of the 66 kV powerline Branches 1, 2 and 3 are shown in Figs. C.10 to C.12, respectively, in Appendix C. The predicted total profiles after the completion of LW702 to LW708B and LW901 to LW904 are shown as cyan lines. The predicted total profiles after the mining of each of LW709 to LW711 and LW905 are shown as the blue lines.

A summary of the maximum predicted values of total vertical subsidence, tilt along and tilt across the 66 kV powerlines is provided in Table 6.16. The values are the maximum predicted subsidence effects within the Study Area due to the mining of the existing, approved and proposed longwalls in Areas 7 and 9.

**Table 6.16 Maximum predicted total vertical subsidence, tilt along and tilt across the 66 kV powerlines due to the mining in Areas 7 and 9**

Location	Reference	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt along alignment (mm/m)	Maximum predicted total tilt across alignment (mm/m)
66 kV powerlines	Branch 1	1500	6.5	4.0
	Branch 2	1400	7.0	3.0
	Branch 3	1300	6.0	4.0

The 11 kV and low voltage powerlines are located across the Study Area and, therefore, could experience the full range of predicted subsidence movements. A summary of the maximum predicted conventional subsidence effects within the Study Area is provided in Chapter 4.

The maximum predicted total tilts for the 66 kV powerlines are 7.0 mm/m (i.e. 0.7 %, or 1 in 143) along the alignments and 4.0 mm/m (i.e. 0.4 %, or 1 in 250) across the alignments. The maximum predicted total tilt in any direction for the 11 kV and low voltage powerlines within the Study Area is 7 mm/m (i.e. 0.7 %, or 1 in 143).

The maximum predicted horizontal movement of the ground associated with the maximum predicted tilt is 100 mm, i.e. 15 times maximum tilt of 7 mm/m. The maximum predicted horizontal movement at the tops of the poles (assuming a height of 15 m) therefore is 200 mm, i.e. horizontal movement at the ground plus the maximum tilt of 7 mm/m times 15 m pole height.

### 6.10.3. Impact assessments for the powerlines

The 66 kV, 11 kV and low voltage powerline will not be directly affected by the ground strains, as the cables are supported by the power poles above ground level. However, the cables may be affected by the changes in bay lengths, i.e. the distances between the poles at the levels of the cables, resulting from the differential subsidence, horizontal movements and tilt at the pole locations. The stabilities of the poles and the cable clearances may also be affected by the mining-induced tilts and the changes in the catenary profiles of the cables.

The maximum predicted tilt for the powerlines is 7 mm/m (i.e. 0.7 %, or 1 in 143). A rule of thumb used by some electrical engineers is that the tops of the poles may displace up to two pole diameters horizontally before remediation works are considered necessary. Based on pole heights of 15 m and pole diameters of 250 mm, the maximum tolerable tilt at the pole locations is in the order of 20 mm/m.

There is extensive experience of mining directly beneath powerlines at Appin Colliery and elsewhere in the Southern Coalfield. Examples of cases where the measured subsidence effects are similar to the predicted values for the powerlines within the Study Area is provided in Table 6.17.

**Table 6.17 Experience of mining beneath powerlines in the Southern Coalfield**

Colliery and longwalls	Length of powerline directly mined beneath (km)	Maximum measured movements at the powerlines	Observed impacts
Appin Area 1 (LW1 to LW12)	5.2 km of 11 kV 104 power poles	850 mm subsidence 6 mm/m tilt (Measured WX-Line)	No significant impacts
Appin Area 2 (LW14 to LW29)	1.0 km of 66 kV 4.6 km of 11 kV 76 power poles	1200 mm subsidence 7.0 mm/m tilt (Measured A-Line)	No significant impacts
Appin Area 3 (LW301 and LW302)	0.6 km of 66 kV 0.2 km of 11 kV 14 power poles	650 mm subsidence 4.5 mm/m tilt (Measured M & N-Lines)	No significant impacts
Appin Area 4 (LW401 to LW408)	3.4 km of 66 kV 0.6 km of 33 kV 2.9 km of 11 kV 96 power poles	700 mm subsidence 5.0 mm/m tilt (Measured A-Line)	No significant impacts
Appin Area 7 (LW702 to LW708A)	3.6 km of 66 kV 9.9 km of 11 kV 171 power poles	1300 mm subsidence 8.5 mm/m Tilt (Measured ARTC-Line and M31-Lines)	No significant impacts
Appin Area 9 (LW901 and LW902)	1.2 km of 66 kV 2.1 km of 11 kV 67 power poles	525 mm subsidence 3.0 mm/m tilt (Measured ARTC-Line)	No significant impacts

Colliery and longwalls	Length of powerline directly mined beneath (km)	Maximum measured movements at the powerlines	Observed impacts
Dendrobium Area 2 (LW3 and LW4)	0.8 km of 33 kV	1100 mm subsidence 40 mm/m tilt (Measured 2000-Line)	No significant impacts
Tahmoor North (LW22 to LW31)	Approx. 41 km of electrical cables and 1060 power poles	1375 mm subsidence 12 mm/m tilt (Extensive street monitoring)	Some minor adjustments to cable catenaries, pole tilts and consumer cables required.
Tower (LW1 to LW10)	6.0 km of 66 kV 4.3 km of 11 kV 112 power poles	400 mm subsidence 3 mm/m tilt (Measured T & TE-Lines)	No significant impacts
West Cliff Areas 4 & 5 (LW5A3 to LW5A4 & LW29 to LW37)	1.8 km of a 66 kV 8.5 km of 11 kV 170 power poles	1350 mm subsidence 11 mm/m tilt (Measured B-Line)	No significant impacts

The past experiences demonstrate that there have only been minor impacts on aerial powerlines that have been directly mined beneath by previously extracted longwalls in the Southern Coalfield. Some remedial measures were required, which included adjustments to cable catenaries, pole tilts and to consumer cables which connect between the powerlines and houses. The incidence of these impacts was very low.

Based on this experience, it is likely that the extraction of the proposed longwalls would result in only minor impacts on the powerlines within the Study Area. It is expected that the remedial measures would include some adjustments of the cable catenaries, pole tilts and the consumer cables, as has been undertaken in the past, but any other impacts are expected to be relatively infrequent.

11 kV and low voltage powerlines follow Top Ridge Road and Gibraltar Drive at the top of Razorback Range above the proposed LW710A and the western end of the proposed LW711. Increased horizontal movements could develop in the downslope direction due to mining beneath the steep slopes. It is recommended ground movement and visual monitoring are carried out along these powerlines during active subsidence to identify increased or irregular movements.

#### 6.10.4. Recommendations for the powerlines

IMC has an approved management plan for the powerlines for LW701 to LW708B and LW901 to LW903. It is recommended that this management plan is reviewed and updated to incorporate the proposed LW709 to LW711 and LW905.

### 6.11. Telecommunications infrastructure

#### 6.11.1. Descriptions of the telecommunications infrastructure

The locations of the telecommunications infrastructure are shown in Drawing No. MSEC1117-11.

There are optical fibre cables owned by Telstra and Optus that are located within the Study Area. A summary of these optical fibre cables is provided in Table 6.18. The Telstra cables have been numbered from east to west within the Study Area, as shown in Drawing No. MSEC1117-11.

**Table 6.18 Optical fibre cables located within the Study Area**

Owner	Reference	Location	Total length above the proposed longwalls (km)	Total length within the Study Area (km)
Telstra	Branch 1	Outside proposed mining area, adjacent to eastern end of LW709	0.0	1.3
	Branch 2	Directly above LW709, LW710B and LW711	1.2	3.3
	Branch 3	Outside the proposed mining area, 400 m south of LW709	0.0	1.5
	Branch 4	Directly above LW709 and LW710B	0.8	1.9
	Branch 5	Outside proposed mining area, 160 m west of LW711	0.0	1.2
Optus	-	Outside the proposed mining area, 380 m south of LW709	0.0	1.5

An AAPT optical fibre cable is located just outside the eastern boundary of the Study Area, on the western side of the M31 Hume Motorway. This cable is located 400 m east of LW709, at its closest point to the proposed longwalls. Optical fibre cables owned by Telstra, NextGen and Optus are also located outside the Study Area, on the eastern side of the M31 Hume Motorway. These cables are located more than 500 m east of LW709, at their closest points to the proposed longwalls.

Copper telecommunications cables are also located within the Study Area. These underground cables generally follow the local roads and they service the properties within the Study Area.

There is one telecommunications tower located within the Study Area, as shown in Drawing No. MSEC1117-11. The tower is located on Razorback Range and it is outside and immediately adjacent to the main gate of the proposed LW905. A photograph of this tower is provided in Fig. 6.21.



**Fig. 6.21 Mobile phone telecommunications tower**

The telecommunications tower supports GSM antennae and microwave dishes owned by Telstra and Optus which are used for mobile telephone communications. There are also light-weight shed structures associated with the telecommunications tower.

#### **6.11.2. Predictions for the telecommunications infrastructure**

The predicted profiles of total vertical subsidence, tilt and curvature along the Telstra optical fibre cable Branches 2, 3 and 4 are shown in Figs. C.13 to C.15, respectively, in Appendix C. The predicted total profiles after the completion of LW702 to LW708B and LW901 to LW904 are shown as cyan lines. The predicted total profiles after the mining of each of LW709 to LW711 and LW905 are shown as the blue lines.

The predicted profiles along the Optus optical fibre cable are similar to the profiles for the Telstra optical fibre cable Branch 3, as illustrated in Fig. C.14, in Appendix C. The Telstra optical fibre cable Branches 1 and 5 are located outside the proposed mining area and, therefore, are predicted additional subsidence effects due to the proposed longwalls are less than for the other optical fibre cables.

A summary of the maximum predicted values of total vertical subsidence, tilt and curvature for the optical fibre cables is provided in Table 6.19. The values are the maximum predicted subsidence effects within the Study Area due to the mining of the existing, approved and proposed longwalls in Areas 7 and 9.

**Table 6.19 Maximum predicted total vertical subsidence, tilt and curvature for the optical fibre cables within the Study Area due to the mining in Areas 7 and 9**

Feature	Reference	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
Optical fibre cables	Telstra Branch 1	1400	6.5	0.07	0.13
	Telstra Branch 2	1350	5.0	0.07	0.12
	Telstra Branch 3	1400	5.0	0.06	0.12
	Telstra Branch 4	1350	4.0	0.06	0.12
	Telstra Branch 5	30	< 0.5	< 0.01	< 0.01
	Optus	1400	5.0	0.06	0.12

The maximum predicted total tilt for the optical fibre cables within the Study Area is 6.5 mm/m (i.e. 0.65 %, or 1 in 154). The maximum predicted total conventional curvatures are 0.07 km<sup>-1</sup> hogging and 0.13 km<sup>-1</sup> sagging, which represent minimum radii of curvature of 14 km and 8 km, respectively.

The maximum predicted conventional strains for the optical fibre cables, based on applying a factor of 15 to the maximum predicted conventional curvatures, are 1.0 mm/m tensile and 2.0 mm/m compressive. The distribution of the predicted strains due to the extraction of the longwalls is described in Section 4.3. The maximum predicted strains directly above the mining area are 1.0 mm/m tensile and 1.6 mm/m compressive based on the 95 % confidence levels.

Non-conventional movements can also occur and have occurred in the NSW coalfields as a result of, amongst other things, anomalous movements. The analysis of strains provided in Chapter 4 includes those resulting from both conventional and non-conventional anomalous movements.

The Telstra Branch 1, Telstra Branch 3, Telstra Branch 5 and Optus optical fibre cables are located outside the proposed mining area. A summary of the maximum predicted values of additional vertical subsidence, tilt and curvature for these optical fibre cables is provided in Table 6.20. The values are the maximum predicted subsidence effects within the Study Area due to the mining of the proposed longwalls only.

**Table 6.20 Maximum predicted additional vertical subsidence, tilt and curvature for the optical fibre cables within the Study Area due to the mining of LW709 to LW711 and LW905**

Feature	Reference	Maximum predicted additional vertical subsidence (mm)	Maximum predicted additional tilt (mm/m)	Maximum predicted additional hogging curvature (km <sup>-1</sup> )	Maximum predicted additional sagging curvature (km <sup>-1</sup> )
Optical fibre cables	Telstra Branch 1	180	1.0	0.01	< 0.01
	Telstra Branch 3	130	1.0	0.01	< 0.01
	Telstra Branch 5	30	< 0.5	< 0.01	< 0.01
	Optus	140	1.0	0.01	< 0.01

The maximum predicted additional tilt for the Telstra Branch 1, Telstra Branch 3, Telstra Branch 5 and Optus optical fibre cables is 1.0 mm/m (i.e. 0.1 %, or 1 in 1000). The maximum predicted additional conventional curvatures are 0.01 km<sup>-1</sup> hogging and less than 0.01 km<sup>-1</sup> sagging, which represent minimum radii of curvature of 100 km and greater than 100 km, respectively.

The maximum predicted conventional strains for the optical fibre cables, based on applying a factor of 15 to the maximum predicted conventional curvatures, are less than 0.5 mm/m tensile and compressive. The distribution of the predicted strains due to the extraction of the longwalls is described in Section 4.3. The maximum predicted strains outside the mining area are 0.6 mm/m tensile and 0.4 mm/m compressive based on the 95 % confidence levels.

The copper telecommunications cables are located across the Study Area and, therefore, could experience the full range of predicted subsidence movements. A summary of the maximum predicted conventional subsidence effects within the Study Area is provided in Chapter 4.

The optical fibres and copper telecommunications cables cross the streams directly above the proposed mining area and therefore could experience valley-related effects. The maximum predicted valley-related effects for these cables are 25 mm upsidence and 50 mm closure.

A summary of the maximum predicted values of total vertical subsidence, tilt and curvature for telecommunications tower is provided in Table 6.20. The values are the maximum predicted subsidence effects within 20 m of the tower due to the mining of the existing, approved and proposed longwalls in Areas 7 and 9.

**Table 6.21 Maximum predicted total vertical subsidence, tilt and curvature for the telecommunications towers due to the mining of LW709 to LW711 and LW905**

Feature	After longwall	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
Telecom. tower	LW709	30	< 0.5	< 0.01	< 0.01
	LW710A	60	< 0.5	< 0.01	< 0.01
	LW711	60	< 0.5	< 0.01	< 0.01
	LW905	350	3.0	0.05	0.01

The maximum predicted total tilt for the telecommunications tower is 3.0 mm/m (i.e. 0.3 %, or 1 in 333). The maximum predicted total conventional curvatures are 0.05 km<sup>-1</sup> hogging and 0.01 km<sup>-1</sup> sagging, which represent minimum radii of curvature of 20 km and 100 km, respectively.

The maximum predicted conventional strains for the telecommunications tower, based on applying a factor of 15 to the maximum predicted conventional curvatures, are 1.0 mm/m tensile and less than 0.5 mm/m compressive. The distribution of the predicted strains due to the extraction of the longwalls is described in Section 4.3. The maximum predicted strains directly outside the mining area are 0.9 mm/m tensile and 0.8 mm/m compressive based on the 95 % confidence levels.

### 6.11.3. Impact assessments for the telecommunications cables

The optical fibre cables are direct buried and therefore could potentially be impacted by ground strains. The greatest potential for impacts will occur as the result of localised ground strains due to non-conventional movements or valley-related effects.

The tensile strains in the optical fibre cables could be higher than predicted, where the cables connect to the support structures, which may act as anchor points, preventing any differential movements that may have been allowed to occur with the ground. Tree roots have also been known to anchor cables to the ground. The extent to which the anchor points affect the ability of the cables to tolerate the mine subsidence movements depends on the cable size, type, age, installation method and ground conditions.

In addition to this, optical fibre cables contain additional fibre lengths over the sheath lengths, where the individual fibres are loosely contained within tubes. Compression of the sheaths can transfer to the loose tubes and fibres and result in “micro-bending” of the fibres constrained within the tubes, leading to higher attenuation of the transmitted signal. If the maximum predicted compressive strains were to be fully transferred into the optical fibre cables, the strains could be of sufficient magnitude to result in the reduction in capacities of the cables or transmission loss.

The strains transferred into the optical fibre cables can be monitored using Remote Fibre Monitoring Systems (RFMS) such as Optical Time Domain Reflectometry (OTDR), which can be used to notify the infrastructure owners of strain concentrations due to non-conventional ground movements or valley-related effects.

There is extensive experience of mining directly beneath optical fibre cables at Appin Colliery and elsewhere in the Southern Coalfield. Examples of cases where the measured subsidence effects are similar to the predicted values for the optical fibre cables within the Study Area is provided in Table 6.22.

**Table 6.22 Examples of mining beneath optical fibre cables in the Southern Coalfield**

Colliery and longwalls	Length of optical fibre cable directly mined beneath (km)	Maximum measured movements at the optical fibre cables	Pre-mining mitigation, monitoring and observed impacts
Appin Area 3 (LW301 and LW302)	0.8	650 mm subsidence 1.0 mm/m tensile Strain 3.0 mm/m comp. Strain (Measured M & N-Lines)	600 m aerial cable on standby. Ground survey, visual, OTDR. No reported impacts.
Appin Area 7 (LW703 to LW708A)	32.7 total for five cables	1300 mm subsidence 1.5 mm/m tensile Strain 8.5 mm/m comp. Strain (Measured ARTC-Line and M31-Lines)	New cable redirection to avoid potential impacts to old optical fibre cable. Ground survey, visual, OTDR. Strain concentrations detected in three cables, attenuation losses were relieved by locally exposing the cables or by building a bypass cable.
Tahmoor North (LW22 to LW32)	4.6	1375 mm subsidence 3.0 mm/m tensile strain 8.0 mm/m comp. strain (Extensive street monitoring)	Ground survey, visual, OTDR, SBS. No reported impacts.
Tower (LW1 to LW10)	1.7	400 mm subsidence 3 mm/m tilt 0.5 mm/m tensile Strain 1 mm/m comp. Strain	No reported impacts
West Cliff Areas 4 & 5 (LW5A3, LW5A4 and LW29 to LW37)	3.6	1350 mm subsidence 1.5 mm/m tensile strain 5.5 mm/m comp. strain (Measured B-Line)	Survey, visual, OTDR, SBS. No reported impacts.

*Note: SBS is a method of monitoring optical fibres and means Stimulated Brillouin Scattering*

The experience of mining beneath optical fibre cables at Appin Colliery and elsewhere in the Southern Coalfield indicates that the potential impacts can be managed with the implementation of suitable management and monitoring strategies. With an appropriate management plan in place, it is considered that potential impacts on the optical fibre cables due to the proposed mining can be managed, even if actual subsidence movements are greater than the predictions or substantial non-conventional movements occur.

The copper telecommunications cables are located across the Study Area. There is extensive experience of mining beneath copper telecommunications cables at Appin Colliery and elsewhere in the Southern Coalfield. There has been no reported impacts on the direct buried copper telecommunications cables in these cases. It is not anticipated, therefore, that the copper telecommunications cables within the Study Area will experience adverse impacts.

Copper telecommunications cables follow Top Ridge Road and Gibraltar Drive at the top of Razorback Range above the proposed LW710A and the western end of the proposed LW711. Increased horizontal movements could develop in the downslope direction due to mining beneath the steep slopes. It is recommended ground movement and visual monitoring are carried out along these underground cables during active subsidence to identify increased or irregular movements.

#### 6.11.4. Impact assessments for the telecommunications tower

The maximum predicted tilt for the telecommunications tower is 3.0 mm/m (i.e. 0.3 %, or 1 in 333). The tilt is small and unlikely to adversely impact on the tower structure itself or the shed structures containing the telecommunications equipment.

The microwave dishes that link each GSM tower are sensitive to small angular deviations. The predicted angular deviation due to the mining-induced tilt of 3.0 mm/m is approximately 0.2°. The maximum predicted vertical subsidence of the tower is 350 mm. The predicted angular deviation due to the vertical subsidence, based on a minimum line of site distance of say 2 km, is less than 0.1°.

The maximum predicted angular deviation due to both vertical subsidence and tilt of the tower sites is therefore approximately 0.3°. It is expected that the angular deviations of the microwave dishes can be managed by making any necessary adjustments to the lines of sight as the proposed LW905 is mined adjacent to it.

The maximum predicted curvatures for the tower are 0.05 km<sup>-1</sup> hogging and 0.01 km<sup>-1</sup> sagging, which represent minimum radii of curvature of 20 km and 100 km, respectively. The maximum predicted strains are 0.9 mm/m tensile and 0.8 mm/m compressive based on the 95 % confidence levels.

It is recommended that the predicted subsidence effects for the telecommunications tower is provided to the infrastructure owner so that a detailed structural analysis can be undertaken of the tower and its associated infrastructure. It is also recommended that suitable preventive measures be established, in consultation with the infrastructure owner, so that the tower and associated infrastructure can be maintained in safe and serviceable conditions throughout the mining period.

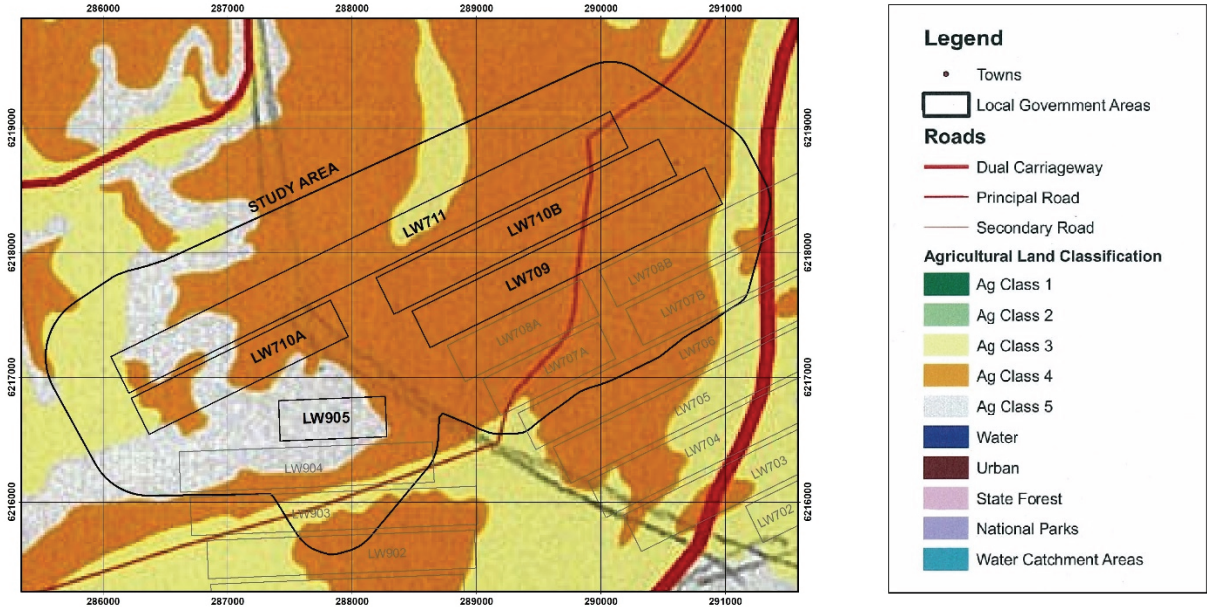
**6.11.5. Recommendations for the telecommunications infrastructure**

IMC has an approved management plan for the telecommunications infrastructure for LW701 to LW708B and LW901 to LW903. It is recommended that this management plan is reviewed and updated to incorporate the proposed LW709 to LW711 and LW905.

It is also recommended that the predicted subsidence effects for the telecommunications tower are provided to the infrastructure owner so that a detailed structural analysis can be undertaken. It is recommended that monitoring and management measures is implemented so that the tower and associated infrastructure are maintained in safe and serviceable conditions.

**6.12. Agricultural utilisation**

The agricultural land classification types within the Study Area are illustrated in Fig. 6.22.



**Fig. 6.22 Agricultural Land Classification within the Study Area (Source DTIRIS, November 2008)**

There are three main agricultural land classification types within the Study Area:

- Class 3 – grazing land or land well suited to pasture improvement,
- Class 4 – land suitable for grazing but not for cultivation, and
- Class 5 – land unsuitable for agriculture, or at best suited only to light grazing.

The flatter areas of land within the Study Area have been predominately cleared and are used for light agricultural and residential purposes. The hillier areas within the Study Area, including the Razorback Range, have not been cleared of the natural vegetation.

**6.13. Rural structures**

**6.13.1. Descriptions of the rural structures**

The locations of the rural structures are shown in Drawing No. MSEC1117-13.

There are 581 rural structures that have been identified within the Study Area. These structures include sheds, garages, carports and other non-residential building structures. Details of the rural structures are included in Table D.01, in Appendix D. A summary of the rural structures within the Study Area is provided in Table 6.23.

**Table 6.23 Details of the rural structures within the Study Area**

Type	Number located above the existing and approved longwalls	Number located above the proposed LW709 to LW711 and LW905	Number located outside the mining area
Sheds	97	180	193
Garages/carports	9	33	28
Awnings/Pergolas	1	12	15
Other	1	6	6
<b>Total</b>	<b>108</b>	<b>231</b>	<b>242</b>

There are 339 rural structures (i.e. 58 %) that are located above the existing and proposed longwalls and 242 rural structures (i.e. 42 %) that are located outside the mining area.

### 6.13.2. Predictions for the rural structures

Predictions of conventional vertical subsidence, tilt and curvature have been made at the centroid and at the vertices of each structure, as well as eight equally spaced points placed radially around the centroid and vertices at a distance of 20 m. In the case of a rectangular shaped structure, predictions have been made at a minimum of 45 points within and around the structure.

The maximum predicted values of total vertical subsidence, tilt and curvature for each of the rural structures within the Study Area are provided in Table D.02, in Appendix D. The predicted tilts represent the maximum values in any direction after the completion of each of the proposed LW709 to LW711 and LW905. The predicted curvatures are the maximum values in any direction at any time during or after the extraction of each of the proposed longwalls.

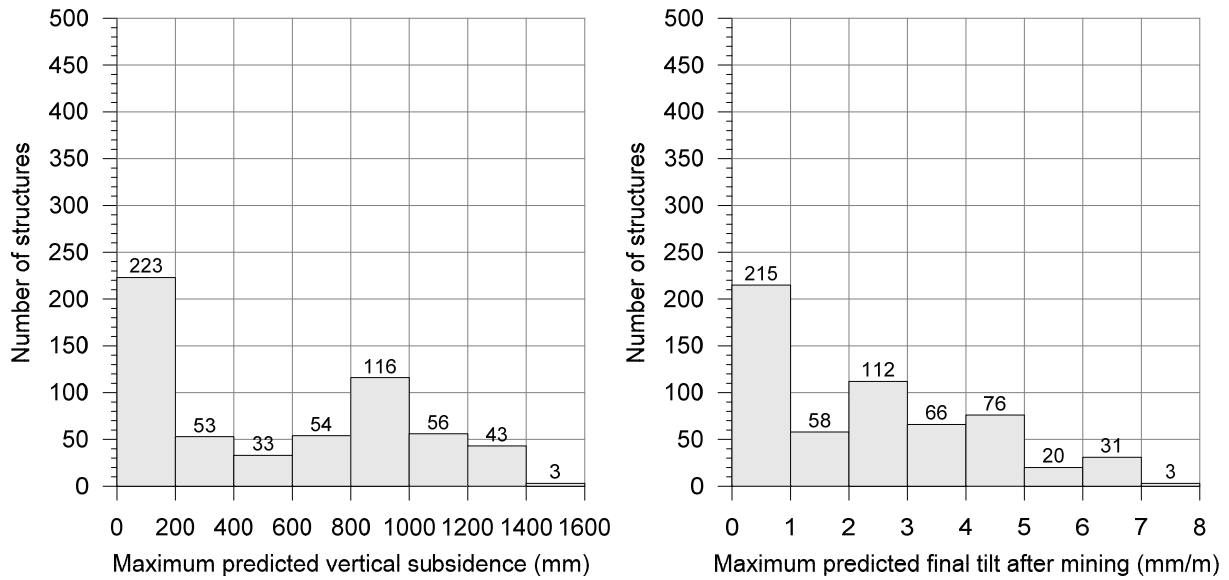
A summary of the maximum predicted values of total vertical subsidence, tilt and curvatures for the rural structures is provided in Table 6.24. The table provides the maximum predicted values for the structures at any time during or after the extraction of each longwall.

**Table 6.24 Maximum predicted total vertical subsidence, tilt and curvatures for the rural structures**

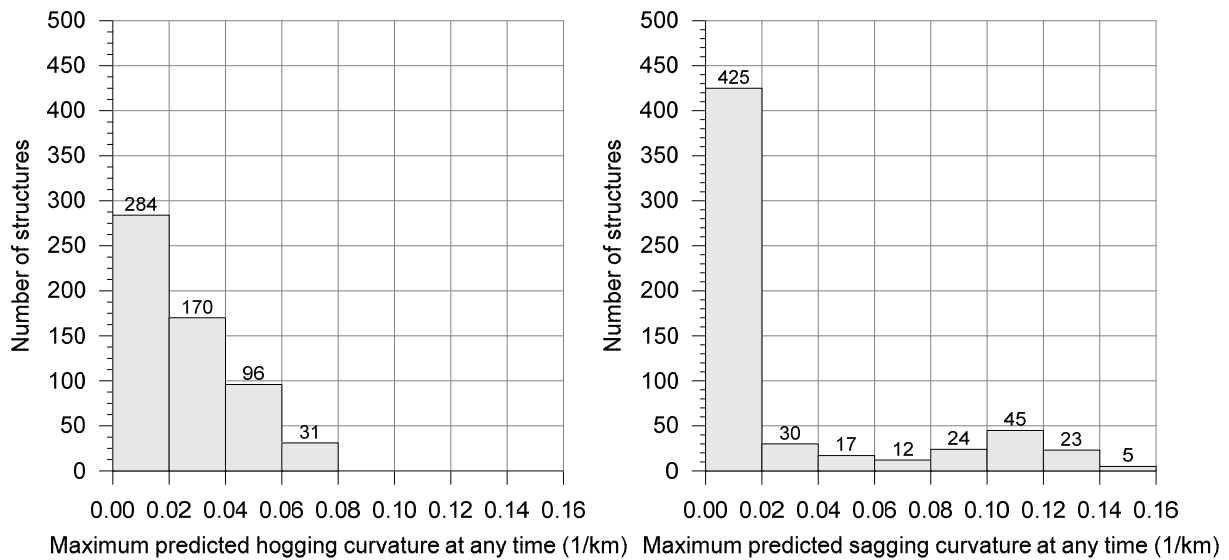
After longwall	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature ( $\text{km}^{-1}$ )	Maximum predicted total sagging curvature ( $\text{km}^{-1}$ )
LW709	1400	6.5	0.06	0.13
LW710A	1400	6.5	0.06	0.13
LW710B	1425	6.5	0.06	0.14
LW711	1425	7.0	0.07	0.14
LW905	1425	7.0	0.07	0.14

The maximum predicted tilt for the rural structures is 7.0 mm/m (i.e. 0.7 %, or 1 in 143). The maximum predicted curvatures are 0.07  $\text{km}^{-1}$  hogging and 0.14  $\text{km}^{-1}$  sagging, which represent minimum radii of curvature of 14 km and 7 km, respectively.

Distributions of the predicted vertical subsidence, tilt and curvatures for the rural structures within the Study Area are illustrated in Fig. 6.23 and Fig. 6.24.



**Fig. 6.23 Maximum predicted vertical subsidence (left-hand side) and final tilt (right-hand side) for the rural structures**



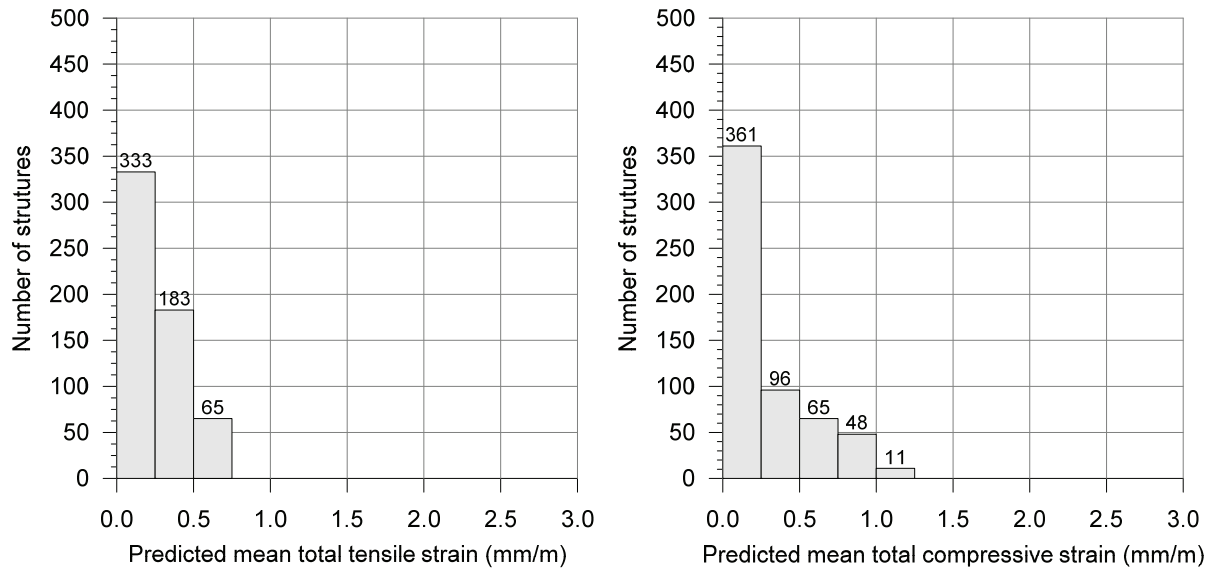
**Fig. 6.24 Maximum predicted hogging curvature (left-hand side) and sagging curvature (right-hand side) at any time for the rural structures**

The maximum predicted conventional strains for the rural, based on applying a factor of 15 to the maximum predicted conventional curvatures, are 1.0 mm/m tensile and 2.0 mm/m compressive. Higher strains could develop at the structures due to irregular ground movements or topographic effects.

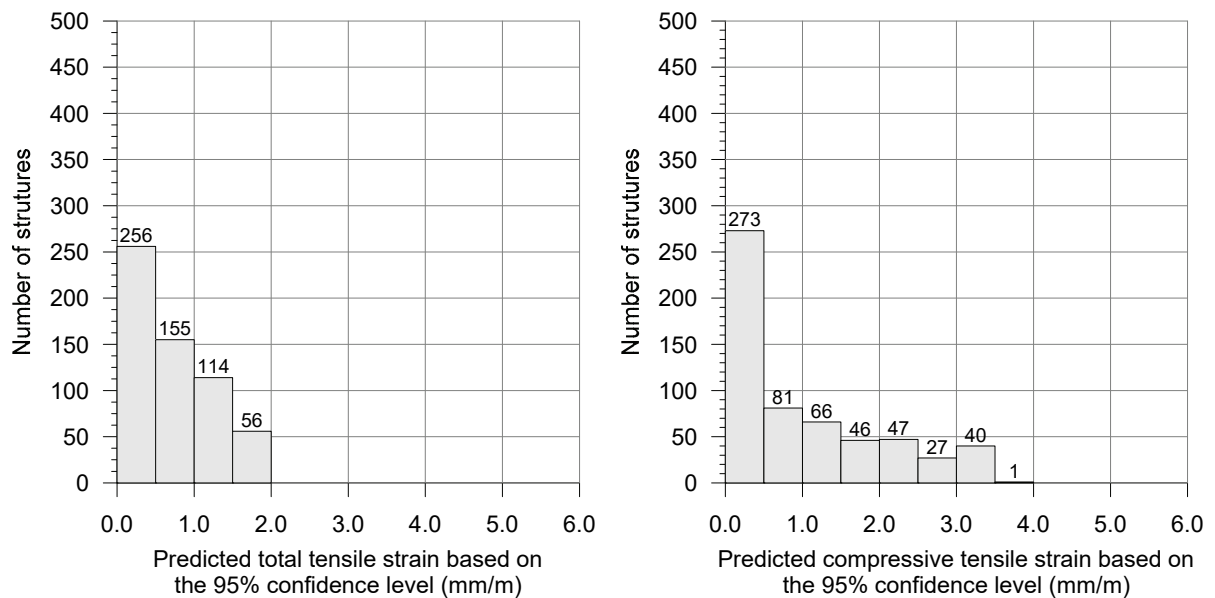
The predicted distributions of strain due to the proposed mining are described in Chapter 4. The rural structures are at discrete locations and, therefore, the most relevant distribution of strain is the maximum strains measured in individual survey bays above previous longwall mining, which is summarised in Section 4.3.1. The maximum predicted total strains directly above the proposed longwalls are 1.0 mm/m tensile and 1.6 mm/m compressive based on the 95 % confidence levels.

The strains have been predicted for each of the rural structures using the method described by Barbato (2017). This method considers the position of each structure relative to the longwalls, the surface slope, surface lithology and the potential for irregular anomalous movements.

The predicted total strains for each of the rural structures within the Study Area are provided in Table D.02, in Appendix D. Distributions of the predicted total strains based on the mean and on the 95 % confidence levels are provided in Fig. 6.25 and Fig. 6.26, respectively.



**Fig. 6.25 Predicted total tensile strain (left-hand side) and total compressive strain (right-hand side) for the rural structures based on the mean**



**Fig. 6.26 Predicted total tensile strain (left-hand side) and total compressive strain (right-hand side) for the rural structures based on the 95 % confidence level**

The rural structures within the Study Area are predicted to experience total tensile strains between 0.2 mm/m and 1.7 mm/m and total compressive strains between 0.2 mm/m and 3.5 mm/m based on the 95 % confidence levels. The predicted mean values range between 0.2 mm/m and 1.1 mm/m tensile and compressive.

### 6.13.3. Impact assessments for the rural structures

The majority of the rural structures within the Study Area are of lightweight construction and are expected to tolerate the predicted mining-induced tilt. It has been found from past longwall mining experience, that tilts of the magnitudes predicted for the proposed longwalls generally do not result in adverse impacts on rural structures. Some minor serviceability impacts could occur at the higher levels of predicted tilt, including door swings and issues with roof and pavement drainage. These serviceability impacts can generally be remediated using normal building maintenance techniques.

There is extensive experience of mining directly beneath rural structures at Appin Colliery and elsewhere in the Southern Coalfield. This experience indicates that the incidence of impacts on rural structures is very low and these structures have remained in safe and serviceable conditions. This is not surprising as rural structures are generally small in size and of light-weight construction, which makes them less susceptible to impact than houses which are typically more rigid.

The nearby Tahmoor Colliery has mined directly beneath more than 2000 rural structures and 1900 associated residential structures of similar construction during the mining of Longwalls 22 to 31. It has managed the mining-induced impacts with the implementation of suitable management strategies. The structures have remained safe and serviceable during mining.

Based on this experience, it is expected that the rural structures within the Study Area would remain safe and serviceable during the mining period, provided that they are in sound existing condition. The risk of impact could be greater if the structures are in poor existing condition, though the chances of there being a public safety risk remains very low. A number of rural structures that were in poor existing condition have been directly mined beneath and these structures have not experienced adverse impacts during mining.

Impacts on the rural structures that occur due to the mining of the proposed LW709 to LW711 and LW905 are expected to be remediated using well established building techniques. With these remediation measures available, it is unlikely that there would be long term impacts on rural structures due to the extraction of the proposed longwalls.

#### 6.13.4. Recommendations for the rural structures

IMC has approved management plans for building structures for LW701 to LW708B and LW901 to LW903. It is recommended that these management plans are reviewed and updated to incorporate the proposed LW709 to LW711 and LW905.

The management plans provide for identification (where landholder access is provided) of buildings in poor pre-mining condition that are hazardous or may become hazardous due to mining, and monitoring of structures during active subsidence. If impacts occur, the structure will be repaired in accordance with the *Coal Mine Subsidence Compensation Act 2017*.

#### 6.14. Tanks

The locations of the tanks are shown in Drawing No. MSEC1117-13.

There are 339 tanks that have been identified within the Study Area. These include water storage and gas tanks on the properties. Details of the tanks are included in Table D.01, in Appendix D. A summary of the tanks within the Study Area is provided in Table 6.25.

**Table 6.25 Details of the tanks within the Study Area**

Type	Number located above the existing and approved longwalls	Number located above the proposed LW709 to LW711 and LW905	Number located outside the mining area
Tanks	59	138	142

The maximum predicted values of total vertical subsidence, tilt and curvature for each of the tanks within the Study Area are provided in Table D.02, in Appendix D. The predicted tilts represent the maximum values in any direction after the completion of each of the proposed longwalls. The predicted curvatures are the maximum values in any direction at any time during or after the extraction of each of the proposed longwalls.

A summary of the maximum predicted values of total vertical subsidence, tilt and curvatures for the tanks is provided in Table 6.26. The table provides the maximum predicted values for the tanks at any time during or after the extraction of each longwall.

**Table 6.26 Maximum predicted total vertical subsidence, tilt and curvatures for the tanks**

After longwall	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
LW709	1400	6.5	0.06	0.13
LW710A	1400	6.5	0.06	0.13
LW710B	1425	6.5	0.06	0.14
LW711	1425	7.0	0.08	0.14
LW905	1425	7.0	0.08	0.14

The maximum predicted tilt for the tanks is 7.0 mm/m (i.e. 0.7 %, or 1 in 143). The maximum predicted curvatures are 0.08 km<sup>-1</sup> hogging and 0.14 km<sup>-1</sup> sagging, which represent minimum radii of curvature of 13 km and 7 km, respectively.

The maximum predicted conventional strains for the tanks, based on applying a factor of 15 to the maximum predicted conventional curvatures, are 1.0 mm/m tensile and 2.0 mm/m compressive. The distribution of the predicted strains due to the extraction of the longwalls is described in Section 4.3. The maximum predicted strains directly above the mining area are 1.0 mm/m tensile and 1.6 mm/m compressive based on the 95 % confidence levels.

Non-conventional movements can also occur and have occurred in the NSW coalfields as a result of, amongst other things, anomalous movements. The analysis of strains provided in Chapter 4 includes those resulting from both conventional and non-conventional anomalous movements.

The tanks themselves are typically constructed above ground level and, therefore, are unlikely to experience the full ground movements resulting from the proposed mining. It is possible that any buried water pipelines associated with the tanks within the Study Area could be impacted by the ground strains, if they are anchored by the tanks, or by other structures in the ground. Any impacts are expected to be of a minor nature and easily repaired.

IMC has approved management plans for structures (including tanks) for LW701 to LW708B and LW901 to LW903. It is recommended that these management plans are reviewed and updated to incorporate the proposed LW709 to LW711 and LW905.

## 6.15. Fences

There are fences located across the Study Area. The fences are constructed in a variety of ways, generally using either timber or metal materials. Fences are generally flexible in construction and can usually tolerate mine subsidence movements in the Southern Coalfield.

There is extensive experience of mining beneath fences at Appin Colliery and elsewhere in the Southern Coalfield. A higher incidence of impacts to urban fences has been observed, which is considered to be due to their typical type of construction, namely Colorbond fences and security gates that are fitted tightly between fences and houses. Rural fences are typically more flexible in construction by comparison. No impacts to fences securing livestock were reported. Damaged fences are relatively easy to rectify by re-tensioning of fencing wire, straightening of fence posts, and if necessary, replacing some sections of fencing.

The most vulnerable sections of farm fences are gates, particularly long gates or those with latches, as they are less tolerant to differential horizontal movements and tilts between the gate posts and the ground. If any gates are adversely impacted during the extraction of the proposed longwalls, they can be easily and quickly repaired.

## 6.16. Farm dams

### 6.16.1. Descriptions of the farm dams

The locations of the farm dams are shown in Drawing No. MSEC1117-13.

There are 239 farm dams that have been identified within the Study Area. Details of the dams are included in Table D.03, in Appendix D. A summary of the farm dams within the Study Area is provided in Table 6.27.

**Table 6.27 Details of the farm dams within the Study Area**

Type	Number located above the existing and approved longwalls	Number located above the proposed LW709 to LW711 and LW905	Number located outside the mining area
Farm dams	53	104	82

The details of the farm dams are included in Table D.03, in Appendix D. The dams have maximum dimensions ranging between 4 m and 136 m and plan areas ranging between 13 m<sup>2</sup> and 995 m<sup>2</sup>.

The farm dams are typically of earthen construction and have been established by localised cut and fill operations within the natural streams. The dams are generally shallow, with the dam wall heights generally being less than 3 m.

### 6.16.2. Predictions for the farm dams

Predictions of conventional vertical subsidence, tilt and curvature have been made at the centroid and at the vertices of each farm dam, as well as eight equally spaced points placed radially around the centroid and vertices at a distance of 20 m. In the case of a rectangular shaped dam, predictions have been made at a minimum of 45 points within and around the feature.

The maximum predicted values of total vertical subsidence, tilt and curvature for each of the farm dams within the Study Area are included in Table D.03, in Appendix D. The predicted tilts represent the maximum values in any direction after the completion of each of the proposed LW709 to LW711 and LW905. The predicted curvatures represent the maximum values in any direction at any time during or after the extraction of each of the proposed longwalls.

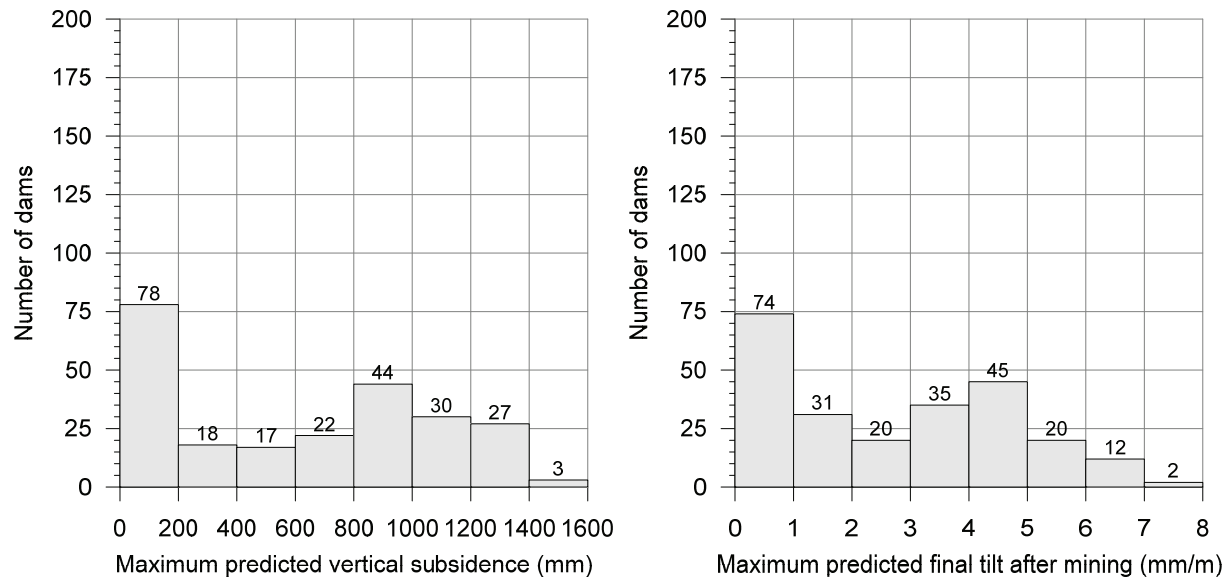
A summary of the maximum predicted values of total vertical subsidence, tilt and curvatures for the farm dams is provided in Table 6.28. The table provides the maximum predicted values for the dams at any time during or after the extraction of each proposed longwall.

**Table 6.28 Maximum predicted total vertical subsidence, tilt and curvatures for the farm dams**

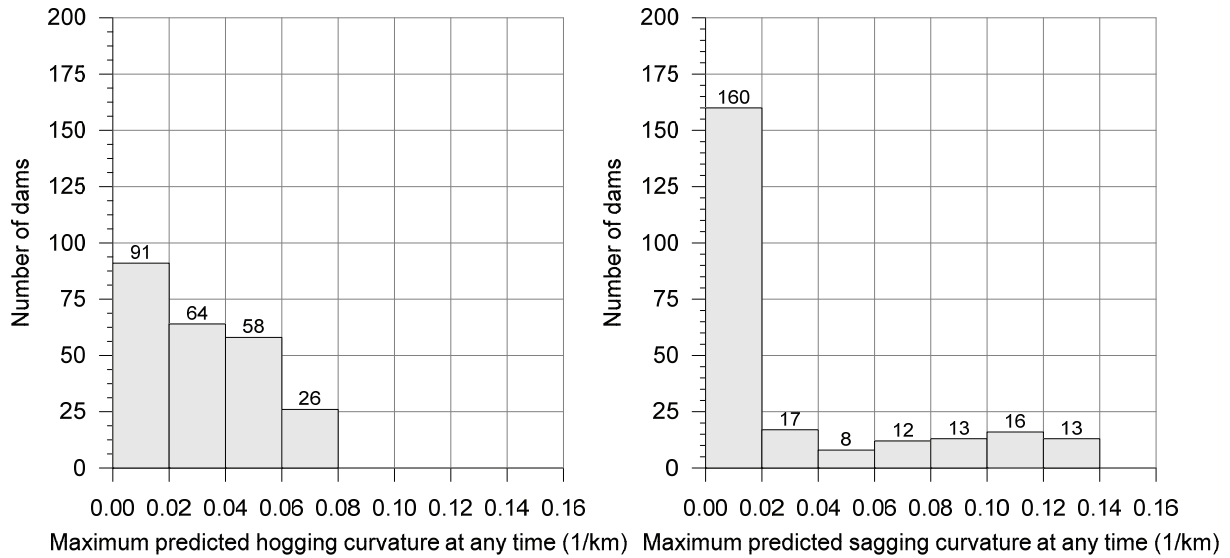
After longwall	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
LW709	1450	7.0	0.07	0.13
LW710A	1450	7.0	0.07	0.13
LW710B	1450	6.5	0.07	0.13
LW711	1450	7.0	0.07	0.14
LW905	1450	7.0	0.07	0.14

The maximum predicted tilt for the farm dams is 7.0 mm/m (i.e. 0.7 %, or 1 in 143). The maximum predicted curvatures are 0.07 km<sup>-1</sup> hogging and 0.14 km<sup>-1</sup> sagging, which represent minimum radii of curvature of 14 km and 7 km, respectively.

Distributions of the predicted vertical subsidence, tilt and curvatures for the farm dams within the Study Area are illustrated in Fig. 6.27 and Fig. 6.28.



**Fig. 6.27 Maximum predicted vertical subsidence (left-hand side) and final tilt (right-hand side) for the farm dams**



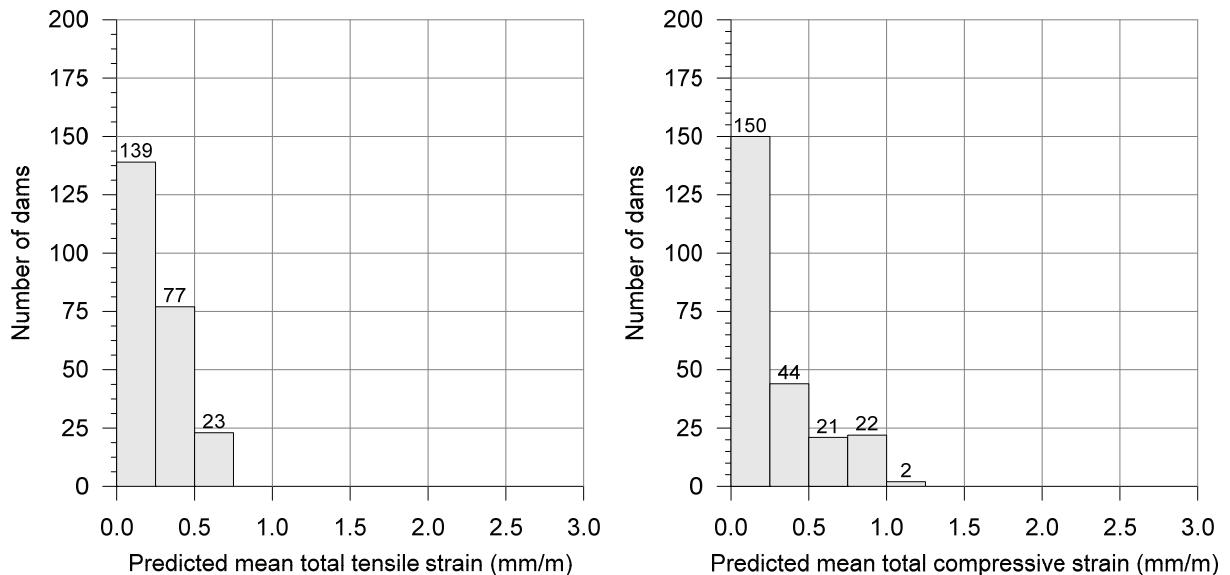
**Fig. 6.28 Maximum predicted hogging curvature (left-hand side) and sagging curvature (right-hand side) at any time for the farm dams**

The maximum predicted conventional strains for the farm dams, based on applying a factor of 15 to the maximum predicted conventional curvatures, are 1.0 mm/m tensile and 2.0 mm/m compressive. Higher strains could develop at the structures due to irregular ground movements or topographic effects.

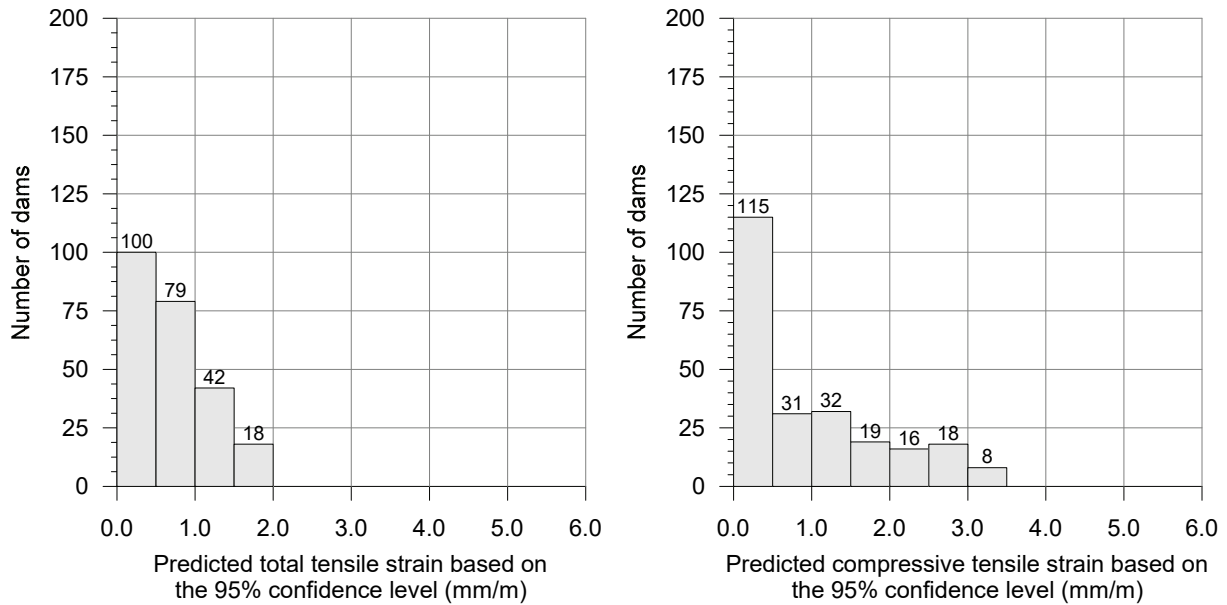
The predicted distributions of strain due to the proposed mining are described in Chapter 4. The rural structures are at discrete locations and, therefore, the most relevant distribution of strain is the maximum strains measured in individual survey bays above previous longwall mining, which is summarised in Section 4.3.1. The maximum predicted total strains directly above the proposed longwalls are 1.0 mm/m tensile and 1.6 mm/m compressive based on the 95 % confidence levels.

The strains have been predicted for each of the farm dams using the method described by Barbato (2017). This method considers the position of each feature relative to the longwalls, the surface slope, surface lithology and the potential for irregular anomalous movements.

The predicted total strains for each of the farm dams within the Study Area are provided in Table D.03, in Appendix D. The distributions of the predicted total strains based on the mean and on the 95 % confidence levels are provided in Fig. 6.29 and Fig. 6.30, respectively.



**Fig. 6.29 Predicted total tensile strain (left-hand side) and total compressive strain (right-hand side) for the farm dams based on the mean**



**Fig. 6.30 Predicted total tensile strain (left-side) and total compressive strain (right-side) for the farm dams based on the 95 % confidence level**

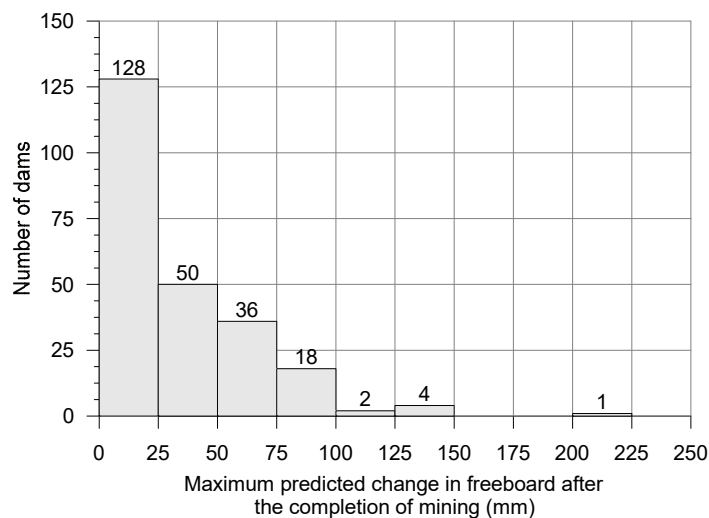
The farm dams within the Study Area are predicted to experience total tensile strains between 0.2 mm/m and 1.7 mm/m and total compressive strains between 0.2 mm/m and 3.4 mm/m based on the 95 % confidence levels. The predicted mean values range between 0.2 mm/m and 1.0 mm/m tensile and compressive.

The farm dams have typically been constructed along the alignments of streams and, therefore, may be subjected to valley-related effects due to the proposed mining. The equivalent valley heights at the dams are small and it is expected, therefore, that the predicted valley-related upsidence and closure movements at the dam walls would be considerably less than the predicted conventional subsidence effects and would not be substantial.

### 6.16.3. Impact assessments for the farm dams

The maximum predicted tilt for the farm dams within the Study Area is 7.0 mm/m (i.e. 0.7 %, or 1 in 143). Mining-induced tilts can affect the water levels around the perimeters of farm dams, with the freeboard increasing on one side and decreasing on the other. Tilt can potentially reduce the storage capacity of farm dams, by causing them to overflow, or can affect the stability of the dam walls.

The predicted changes in freeboard at the farm dams within the Study Area have been determined by taking the difference between the maximum predicted vertical subsidence and the minimum predicted vertical subsidence anywhere around the perimeter of each farm dam. The maximum predicted changes in freeboard for the farm dams are provided in Table D.03, in Appendix D, and are illustrated in Fig. 6.31.



**Fig. 6.31 Predicted changes in freeboard for the farm dams within the Study Area**

The predicted changes in freeboard for the farm dams within the Study Area are small, varying from less than 50 mm to 200 mm. It is unlikely that the dams would experience adverse impacts on the storage capacities due to these small changes in freeboard.

The farm dams located directly above the proposed mining area could experience cracking in the bases or their walls due to the mining-induced curvatures and strains.

There is extensive experience of mining directly beneath farm dams in the Southern Coalfield, which indicates that the incidence of impacts on these features is low. Farm dams are commonly constructed with cohesive materials in the bases and walls which can absorb the conventional subsidence movements typically experienced in the Southern Coalfield without the development of substantial cracking. Non-conventional movements can result in localised cracking and deformations at the surface and, where coincident with farm dams, could result in impacts.

IMC has mined directly beneath more than 200 farm dams in Appin Areas 3, 4, 7 and 9 and West Cliff Area 5. Loss of water was reported for only a small number of dams. Similarly, Tahmoor Colliery has mined Longwalls 22 to 31 beneath a total of 103 dams. While a small number of landowners have advised of impacts, there has been one claim to SA NSW for impacts on farm dams at the time of the report.

Cracking in the dam bases or walls could be repaired by reinstating them with cohesive materials. If any of the farm dams within the Study Area were to lose water due to the proposed mining, the mine would provide an alternative water source until the completion of repairs in accordance with the *Coal Mine Subsidence Compensation Act 2017*.

#### 6.16.4. Recommendations for the farm dams

IMC has approved management plans for farm dams for LW701 to LW708B and LW901 to LW903. It is recommended that these management plans are reviewed and updated to incorporate the proposed LW709 to LW711 and LW905.

#### 6.17. Groundwater bores

The locations of the registered groundwater bores are shown in Drawing No. MSEC1117-14. The locations and details of these were obtained from the Australian Groundwater Explorer, which is publicly available on line (BOM, 2020).

A summary of the registered groundwater bores identified within the Study Area is provided in Table 6.29.

**Table 6.29 Registered groundwater bores within the Study Area**

Ground licence number	Location	Depth (m)	Type*
GW072196	270 m north-west of LW711	N/A	HUSE
GW072874	Above the tailgate of LW710B	189	HUSE
GW100289	430 m north-west of LW711	30	STOK
GW100673	400 m south-west of LW710A	104	STOK
GW101986	210 m south-west of LW710B	210	HUSE
GW104661	80 m south-west of LW707A	219	HUSE
GW105376	Directly above LW710B	219	HUSE
GW105388	Directly above LW709	230	HUSE
GW105531	430 m north-west of LW711	210	HUSE
GW105534	Directly above LW709	207	HUSE
GW105574	Directly above LW711	210	HUSE
GW106574	Above the maingate of LW709	238	HUSE
GW106675	120 m north-west of LW711	183	HUSE
GW108907	120 m north-west of LW711	210	HUSE
GW112381	Directly above LW711	152	HUSE / STOK
GW112441	Directly above LW707A	294	(Decommissioned)

**Note:** \* denotes type HUSE refers to *Water supply for household needs e.g. washing, toilet Water Access Right* and type STOK refers to *Water supply for livestock Basic Access Right*.

The groundwater bores could experience adverse impacts due to the extraction of the proposed LW709 to LW711 and LW905, particularly the bores located directly above the proposed mining area. Impacts could include lowering of the piezometric surface, blockage of the bores due to differential horizontal displacements at different horizons within the strata and changes to groundwater quality.

More detailed assessments are provided in the report by the specialist groundwater consultant for the project (SLR, 2021).

## 6.18. Industrial, commercial and business establishments

The locations of the commercial structures are shown in Drawing No. MSEC1117-13.

There are two commercial structures identified within the Study Area on property F17 (Refs. F17C01 and F17C01). These structures are associated with the telecommunications tower located near the main gate of the proposed LW905. The discussions of these structures are provided in Section 6.11.

## 6.19. Exploration drill holes

There are exploration drill holes located across the Study Area. Exploration drill holes to seam level are grouted and capped prior to the proposed longwalls mining directly beneath them.

## 6.20. Aboriginal heritage sites

### 6.20.1. Descriptions of the Aboriginal heritage sites

The locations of the Aboriginal heritage sites are shown in Drawing No. MSEC1117-14. The details of the heritage sites have been provided by Biosis (2021).

There are three Aboriginal heritage sites that have been identified within the Study Area, being Refs. 52-2-4226, 52-2-4227 and 52-2-4630. There are six additional sites that are located within the 600 m boundary from the proposed mining area, being Refs. 52-2-3191, 52-2-3192, 52-2-3194, 52-2-3687, 52-2-3688 and 52-2-4508.

Details of the Aboriginal heritage sites located within Study Area and the 600 m boundary are provided in Table 6.30.

**Table 6.30 Aboriginal heritage sites identified within the Study Area and the 600 m boundary**

Reference	Type	Location relative to the longwalls
52-2-4226	Open site	Directly above LW711
52-2-4227	Open site	Directly above LW711
52-2-3191	Open site	470 m north-east of LW711
52-2-3192	Open site	430 m north of LW711
52-2-3194	Open site	470 m north of LW711
52-2-3687	Open site	600 m north of LW710B
52-2-3688	Open site	600 m north of LW710B
52-2-4508	Open site	420 m north of LW711
52-2-4630	Open site	180 m west of LW709

Further details on the Aboriginal heritage sites are provided in the report by Biosis (2021).

### 6.20.2. Predictions for the Aboriginal heritage sites

A summary of the maximum predicted total vertical subsidence, tilt and curvatures for the Aboriginal heritage sites located within the Study Area and the 600 m boundary is provided in Table 6.31. The values are the maximum predicted subsidence effects within 20 m of the identified locations of each of the sites due to the mining of the existing, approved and proposed longwalls in Areas 7 and 9.

**Table 6.31 Maximum predicted total vertical subsidence, tilt and curvatures for the Aboriginal heritage sites**

Reference	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
52-2-4226	275	3.0	0.05	0.01
52-2-4227	475	5.5	0.06	0.01
52-2-3191	< 20	< 0.5	< 0.01	< 0.01
52-2-3192	< 20	< 0.5	< 0.01	< 0.01
52-2-3194	< 20	< 0.5	< 0.01	< 0.01
52-2-3687	< 20	< 0.5	< 0.01	< 0.01
52-2-3688	< 20	< 0.5	< 0.01	< 0.01
52-2-4508	< 20	< 0.5	< 0.01	< 0.01
52-2-4630	70	0.5	0.01	< 0.01

Sites 52-2-4226 and 52-2-4227 are located directly above the western end of LW711. The maximum predicted total tilt for these two sites is 5.5 mm/m (i.e. 0.55 %, or 1 in 182). The maximum predicted total conventional curvatures are 0.06 km<sup>-1</sup> hogging and 0.01 km<sup>-1</sup> sagging, which represent minimum radii of curvature of 17 km and 100 km, respectively.

The maximum predicted conventional strains for Sites 52-2-4226 and 52-2-4227, based on applying a factor of 15 to the maximum predicted conventional curvatures, are 1.0 mm/m tensile and less than 0.5 mm/m compressive. The distribution of the predicted strains due to the extraction of the longwalls is described in Section 4.3. The maximum predicted strains directly above the mining area are 1.0 mm/m tensile and 1.6 mm/m compressive based on the 95 % confidence levels.

Non-conventional movements can also occur and have occurred in the NSW coalfields as a result of, amongst other things, anomalous movements. The analysis of strains provided in Chapter 4 includes those resulting from both conventional and non-conventional anomalous movements.

The remaining sites are located at distances varying between 180 m and 600 m outside the proposed mining area. Site Ref. 52-2-4630 is predicted to experience 70 mm vertical subsidence and the remaining sites are predicted to experience less than 20 mm vertical subsidence due to the proposed mining. The maximum predicted strains outside the mining area are 0.6 mm/m tensile and 0.4 mm/m compressive based on the 95 % confidence levels.

The Aboriginal heritage sites are located on the valley sides of the streams. These sites are unlikely therefore to experience the valley-related upsidence or the compressive strain due to valley closure that occur near the valley base.

### 6.20.3. Impact assessments for the Aboriginal heritage sites

The Aboriginal heritage sites within the Study Area comprise open sites. Surface cracking could develop above the proposed longwalls. Fracturing of exposed bedrock could also occur along the streams at distances up to approximately 400 m outside the proposed mining area.

It is unlikely that the artefacts and deposits themselves would be impacted by surface cracking. It is possible, however, that if remediation of the surface were required after mining, that these works could potentially impact the Aboriginal heritage sites.

Further assessments of the potential impacts on the Aboriginal heritage sites is provided by the specialist heritage consultant on the project (Biosis, 2021).

### 6.20.4. Recommendations for the Aboriginal heritage sites

It is recommended that IMC develop an Aboriginal Heritage Management Plan in consultation with the registered parties for the Aboriginal heritage sites.

## 6.21. Survey control marks

The locations of the survey control marks are shown in Drawing No. MSEC1117-14. The locations and details of the survey control marks were obtained from *Spatial Services* using the *SCIMS Online* website (SCIMS, 2020).

Survey control marks are located across the Study Area. The marks located directly above the proposed mining area could experience the full range of predicted movements. The marks located outside the mining area could experience far-field vertical and far-field horizontal effects.

It is possible that the survey control marks could be affected by far-field horizontal movements at distances of 1 km to 2 km outside the extents of the proposed longwalls. Far-field horizontal movements and the methods used to predict such movements are described further in Sections 3.3 and 4.5.

It is recommended that the survey control marks that are required for future use are re-established after the completion of mining in the area and after the ground has stabilised. Consultation between IMC and Spatial Services will be required to ensure that these survey control marks are reinstated at the appropriate time, as required.

## 6.22. Houses

### 6.22.1. Descriptions of the houses

The locations of the houses are shown in Drawing No. MSEC1117-13.

There are 177 houses that have been identified within the Study Area. The details of the houses are included in Table D.01, in Appendix D. The locations, sizes, and construction details of the houses were determined from aerial photographs of the area, kerbside inspections and *Google Street View*®.

A summary of the locations of the houses within the Study Area is provided in Table 6.32.

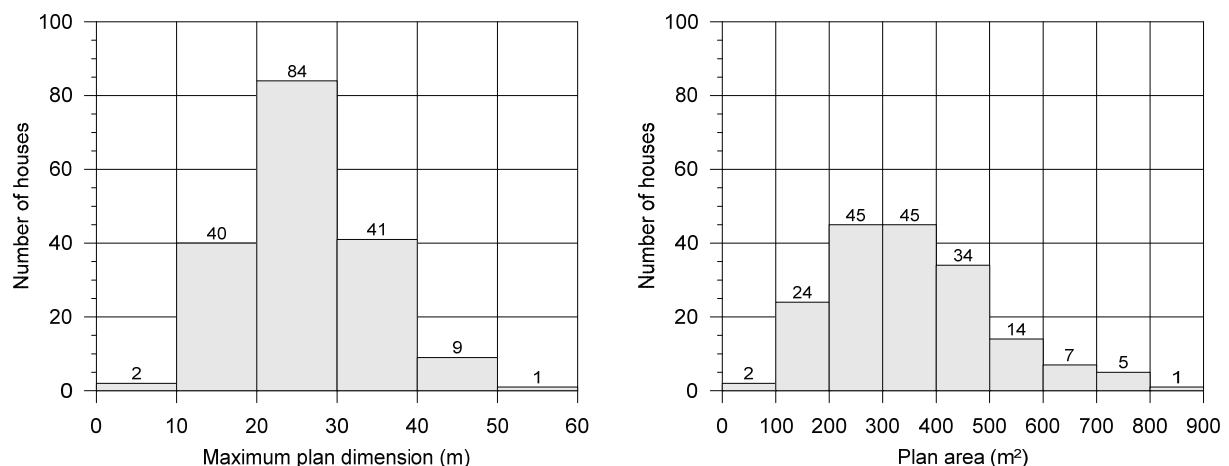
**Table 6.32 Locations of the houses within the Study Area**

Type	Number located above the existing and approved longwalls	Number located above the proposed LW709 to LW711 and LW905	Number located outside the mining area
Houses	30	76	71

There are 106 houses (i.e. 60 %) that are located above the existing and proposed longwalls and 71 houses (i.e. 40 %) that are located outside the mining area.

#### *Maximum plan dimension, plan area and height*

Distributions of the maximum plan dimensions and plan areas of the houses within the Study Area are provided in Fig. 6.32. The majority of the houses have maximum dimensions between 10 m and 40 m, with an average value of approximately 26 m. The majority of the houses have plan areas between 200 m<sup>2</sup> and 500 m<sup>2</sup>, with an average value of approximately 350 m<sup>2</sup>.



**Fig. 6.32 Distribution of houses by maximum plan dimension (left-side) and plan area (right-side)**

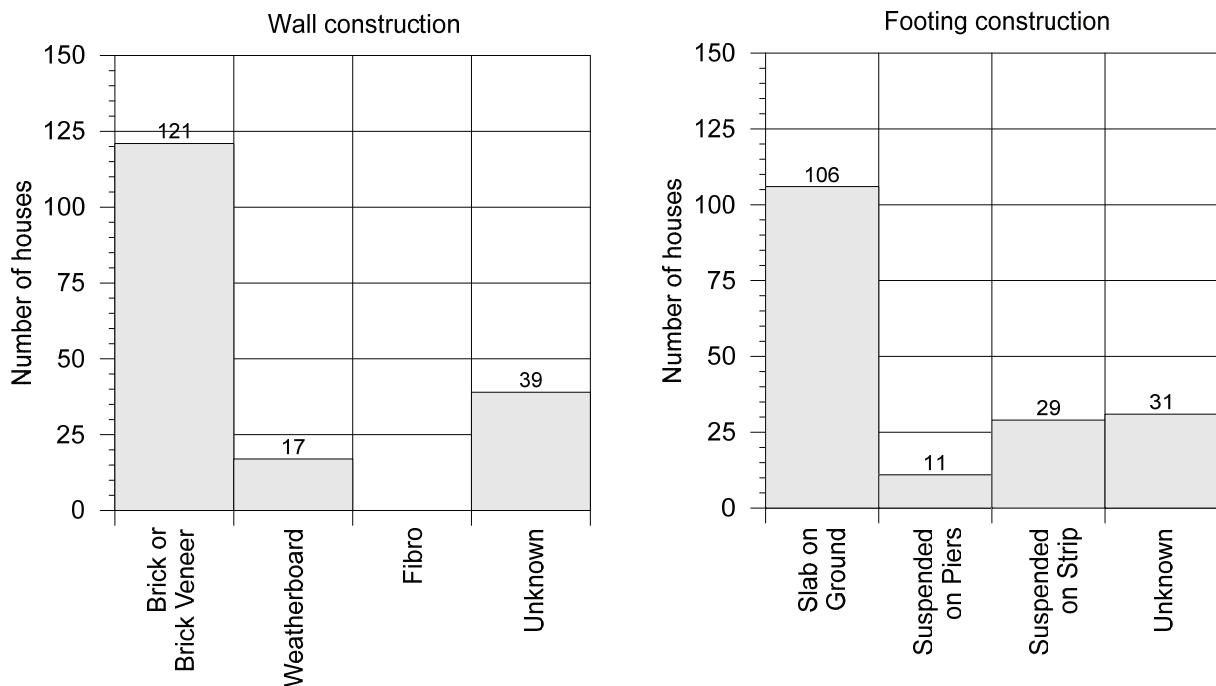
The houses have been categorised into four groups, on the basis of their maximum plan dimension and the number of stories. A summary of these house type categories is provided in Table 6.33. It is noted that two-storey houses include split-level houses.

**Table 6.33 House type categories**

House Type	Description	Number	Percentage
H1	Single-storey with maximum plan dimension less than 30 m	108	61 %
H2	Single-storey with maximum plan dimension of 30 m or greater	47	27 %
H3	Two-storey with maximum plan dimension less than 30 m	13	7 %
H4	Two-storey with maximum plan dimension of 30 m or greater	9	5 %

*Type of construction*

Distributions of the wall and footing constructions of the houses within the Study Area are provided in Fig. 6.33. The majority of the houses within the Study Area comprise brick or brick-veneer structures founded on slabs on ground. There are 39 houses with unknown wall constructions and 31 houses with unknown footing constructions. The impact assessments have conservatively adopted brick structures on strip footings for these houses.



**Fig. 6.33 Distributions of wall and footing construction for houses within the Study Area**

Following a review of impacts to houses due to longwall mining in the Southern Coalfield, it was found that there was a noticeable difference in structural performance in response to mine subsidence movements between the following construction types:

- brick or brick-veneer houses constructed on a ground slab;
- brick or brick-veneer houses constructed on strip footings; and
- weatherboard or fibro houses constructed on either ground slabs or strip footings.

A summary of houses by construction type is provided in Table 6.34. It is noted that some houses have been constructed with masonry walls at basement level, with weatherboard linings for the main living areas above. These houses have been reported as brick in Table 6.34.

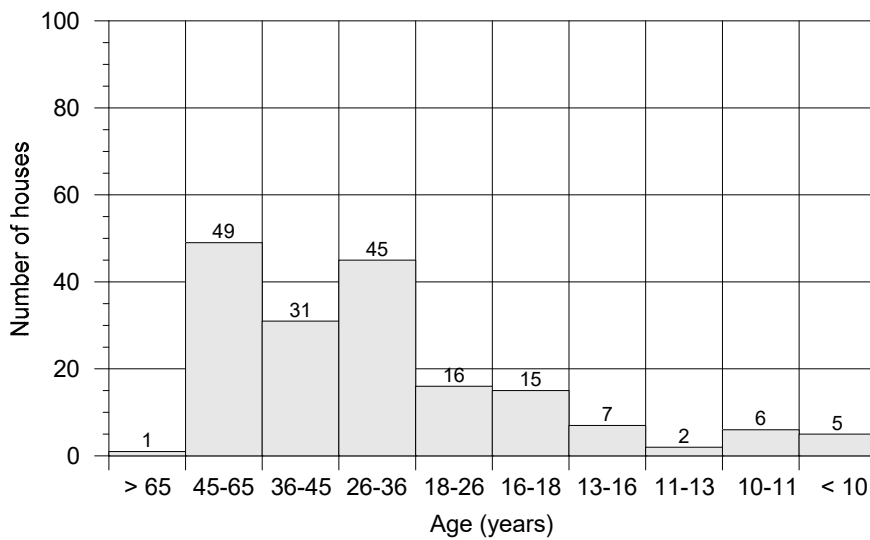
**Table 6.34 Distribution of houses by construction type**

Description	Number	Percentage
Brick or brick-veneer houses constructed on a ground slab	91	51 %
Brick or brick-veneer houses constructed on strip footings	69	39 %
Weatherboard or fibro houses constructed on either ground slabs or strip footings or other	17	10 %

*Age of houses*

The ages of the houses have been determined by examination of a series of historical aerial photographs provided by Land and Property Information, IMC and Nearmap. The photographs that were available over the Study Area were taken from 1955 to 2020.

A histogram showing the distribution of houses by age is shown in Fig. 6.34.



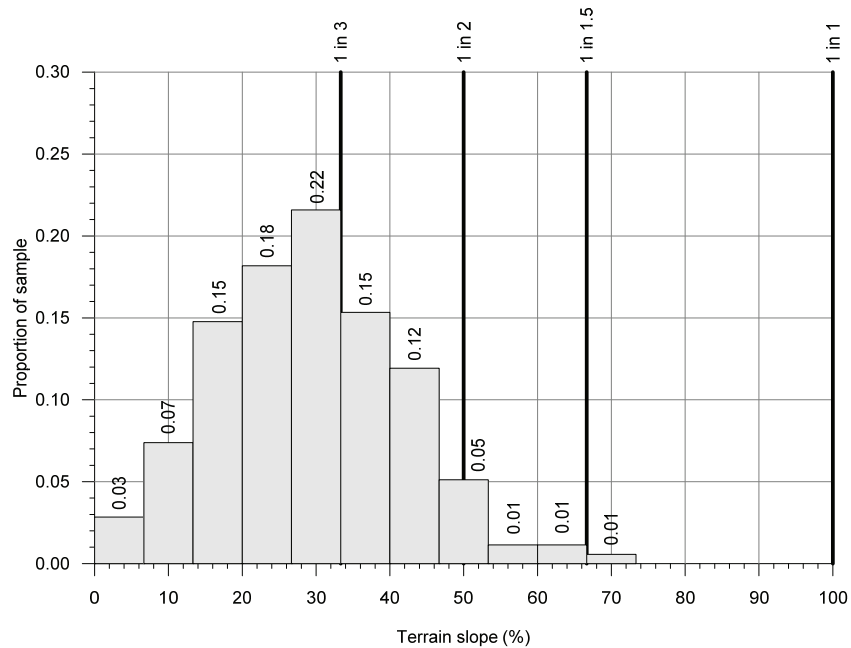
**Fig. 6.34 Distribution of houses by age**

*Houses on steep slopes*

Houses are located on or near steep slopes within the Study Area. These include houses at the top and near the base of Razorback Range in the western part of the Study Area. There are also other houses located on or near steep slopes in the eastern part of the Study Area.

The houses located on or within 25 m of the steep slopes are shown in Table D.01, in Appendix D. The locations of these houses are shown in Drawing No. MSEC1117-02. There are 77 houses within the Study Area located on or near the steep slopes, of which 26 are located at the tops, 23 are located on the sides and 28 are located near the bases.

The distribution of the maximum natural surface grades within 25 m of each of the houses within the Study Area is illustrated in Fig. 6.35.



**Fig. 6.35 Distribution of the maximum natural surface grades within 25 m of each of the houses within the Study Area**

The maximum natural grades for 114 houses (i.e. 65 % of cases) are less than 1 in 3, i.e. less than the value used to define steep slopes. These houses therefore are not considered to be in close proximity to steep slopes. The maximum natural grades for the remaining houses are between 1 in 3 and 1 in 2 for 52 houses (i.e. 30 % of cases), between 1 in 2 and 1 in 1.5 for 9 houses (i.e. 5 % of cases) and greater than 1 in 1.5 for 1 house (i.e. 1 % of cases).

A summary of the 10 houses that are located within 25 m of steep slopes with natural grades of 1 in 2 or greater is provided in Table 6.35.

**Table 6.35 Houses located within 25 m of steep slopes with natural grades of 1 in 2 or greater**

Reference	Location	Steep slopes within 25 m of house	
		Maximum natural grade (%)	Location relative to steep slope
D44a	Above LW708A	52	On
F06h01	Outside mining area	56	Top
F42a	Outside mining area	68	Bottom
F51h01	Outside mining area	51	On
N15h01	Above LW904	51	Bottom
N16h01	Above LW904	66	Bottom
O08h01	Above LW711	58	Top
O17h01	Above LW710A	51	Top
R01h01	Outside mining area	64	Top
S11h01	Outside mining area	52	On

There are five houses within the Study Area that are located directly above the proposed longwalls and are within 25 m of steep slopes with natural grades of 1 in 2 or greater. Two of these houses (Refs. O08h01 and O17h01) are located at the tops of the steep slopes, one house (Ref. D44a) is located on the steep slope and two houses (Refs. N15h01 and N16h01) are located at the bottoms of the steep slopes.

The remaining five houses (Refs. F06h01, F42a, F51h01, R01h01 and S11h01) are located outside the proposed mining area.

### Houses outside declared Mine Subsidence Districts

The locations of the declared Mine Subsidence Districts (MSD) are shown in Drawing No. MSEC1117-13.

There are 164 houses (i.e. 93 %) that are located within the Wilton MSD, which was declared on the 7 November 1979. The remaining 13 houses (i.e. 7%) are located within the South Campbelltown MSD, which was declared on 30 June 1976.

It is estimated that less than 10 % of houses within the Study Area were constructed prior to the declarations of the Wilton MSD and South Campbelltown MSD. The hazard associated with these houses is that they may be less tolerant to mine subsidence movements as their designs have not been checked and approved by SA NSW (formally the Mine Subsidence Board). Some older houses may also be in poor condition. Many of the houses are constructed with timber frames and weatherboard panels or fibro sheets.

The analysis of impacts to structures as part of ACARP Research Project C12015 did not find any significant correlation between the rate of impact and structure age. In any case, IMC will conduct a building and/or structural inspection of the houses (where access is allowed) that were constructed prior to the declaration of the mine subsidence districts and that are predicted to experience more than 20 mm of subsidence due to the extraction of the proposed longwalls.

#### 6.22.2. Predictions for the houses

Predictions of conventional vertical subsidence, tilt and curvature have been made at the centroid and at the vertices of each house, as well as eight equally spaced points placed radially around the centroid and vertices at a distance of 20 m. In the case of a rectangular shaped structure, predictions have been made at a minimum of 45 points within and around the structure.

The maximum predicted values of total vertical subsidence, tilt and curvature for each of the houses within the Study Area are included in Table D.02, in Appendix D. The predicted tilts represent the maximum values in any direction after the completion of each of the proposed LW709 to LW711 and LW905. The predicted curvatures represent the maximum values in any direction at any time during or after the extraction of each of the proposed longwalls.

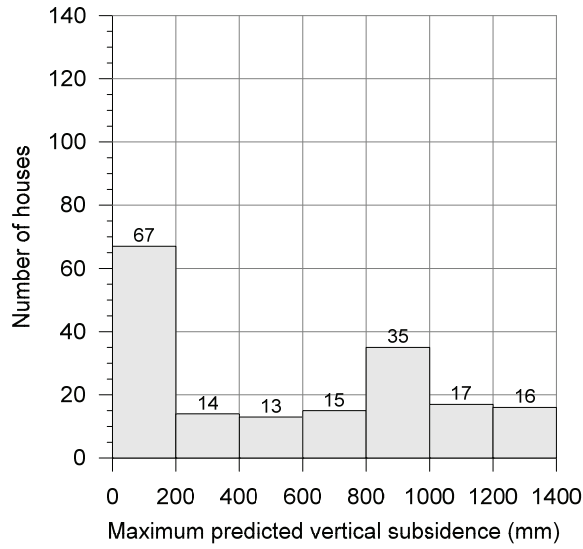
A summary of the maximum predicted values of total vertical subsidence, tilt and curvatures for the houses is provided in Table 6.36. The table provides the maximum predicted values for the houses at any time during or after the extraction of each longwall.

**Table 6.36 Maximum predicted total vertical subsidence, tilt and curvatures for the houses**

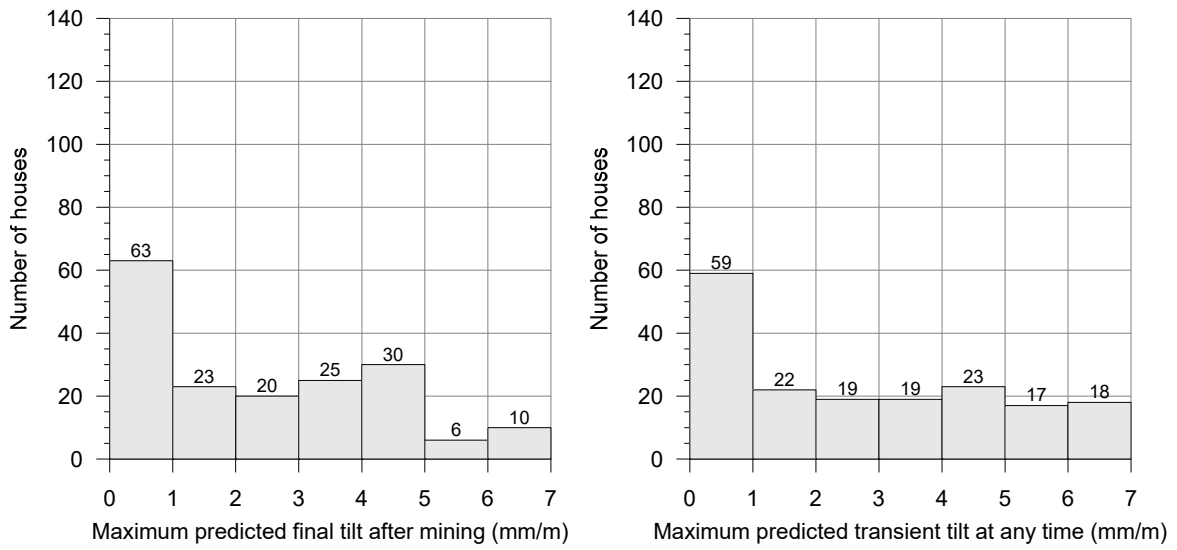
After longwall	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature ( $\text{km}^{-1}$ )	Maximum predicted total sagging curvature ( $\text{km}^{-1}$ )
LW709	1375	6.5	0.07	0.13
LW710A	1375	6.5	0.07	0.13
LW710B	1400	6.5	0.07	0.14
LW711	1400	7.0	0.07	0.14
LW905	1400	7.0	0.07	0.14

The maximum predicted tilt for the houses is 7.0 mm/m (i.e. 0.7 %, or 1 in 143). The maximum predicted curvatures are 0.07  $\text{km}^{-1}$  hogging and 0.14  $\text{km}^{-1}$  sagging, which represent minimum radii of curvature of 14 km and 7 km, respectively.

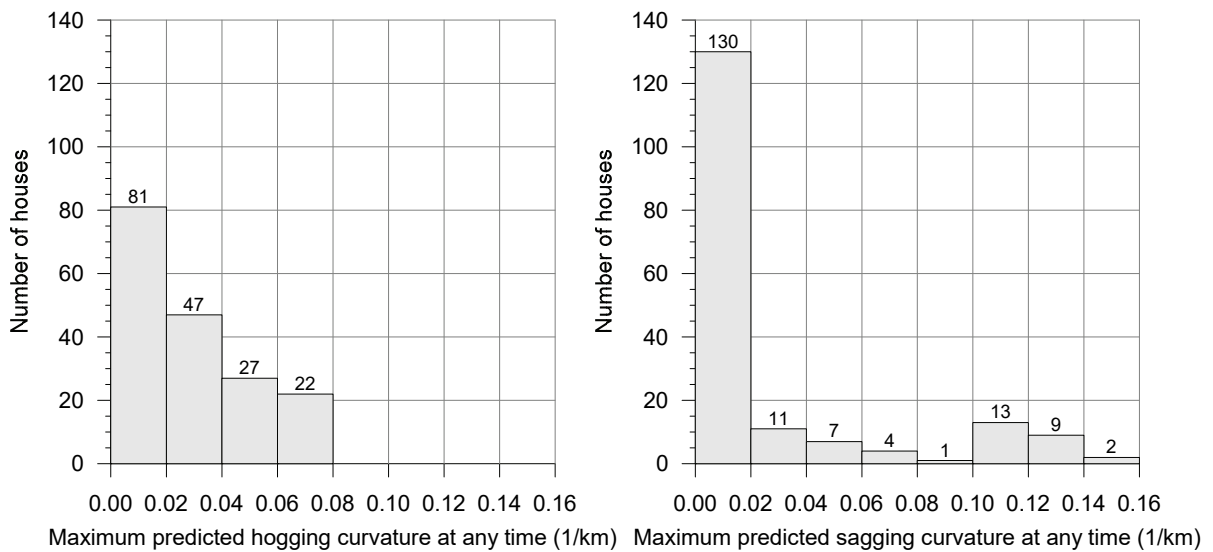
Distributions of the predicted vertical subsidence, tilt and curvatures for the houses within the Study Area are illustrated in Fig. 6.36, Fig. 6.37 and Fig. 6.38.



**Fig. 6.36** Maximum predicted vertical subsidence for the houses



**Fig. 6.37** Maximum predicted final tilt (left-side) and transient tilt (right-side) for the houses



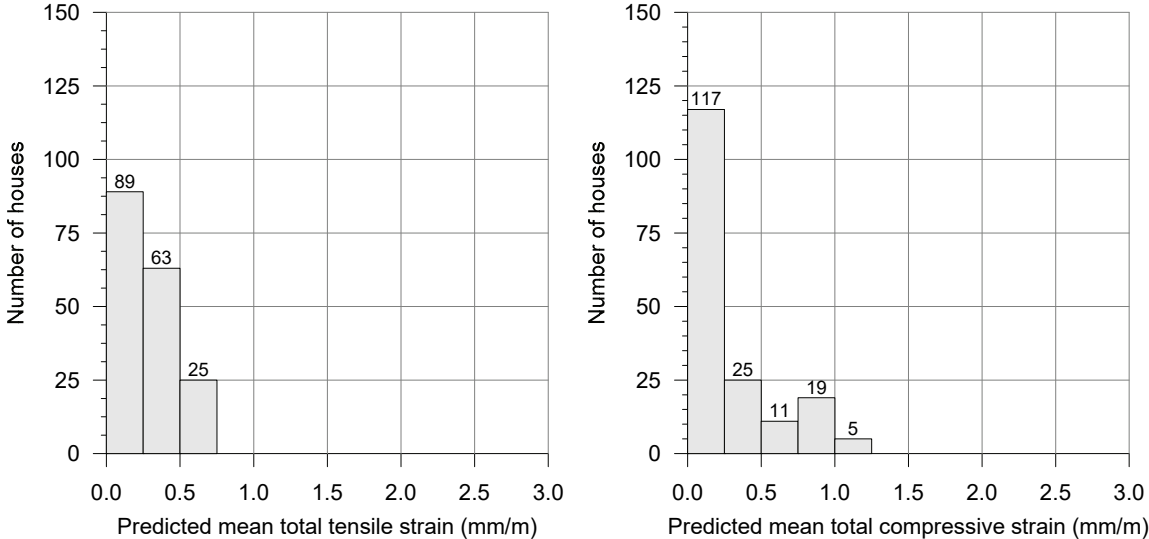
**Fig. 6.38** Maximum predicted hogging curvature (left-side) and sagging curvature (right-side) at any time for the houses

The maximum predicted conventional strains for the houses, based on applying a factor of 15 to the maximum predicted conventional curvatures, are 1.0 mm/m tensile and 2.0 mm/m compressive. Higher strains could develop at the houses due to irregular ground movements or topographic effects.

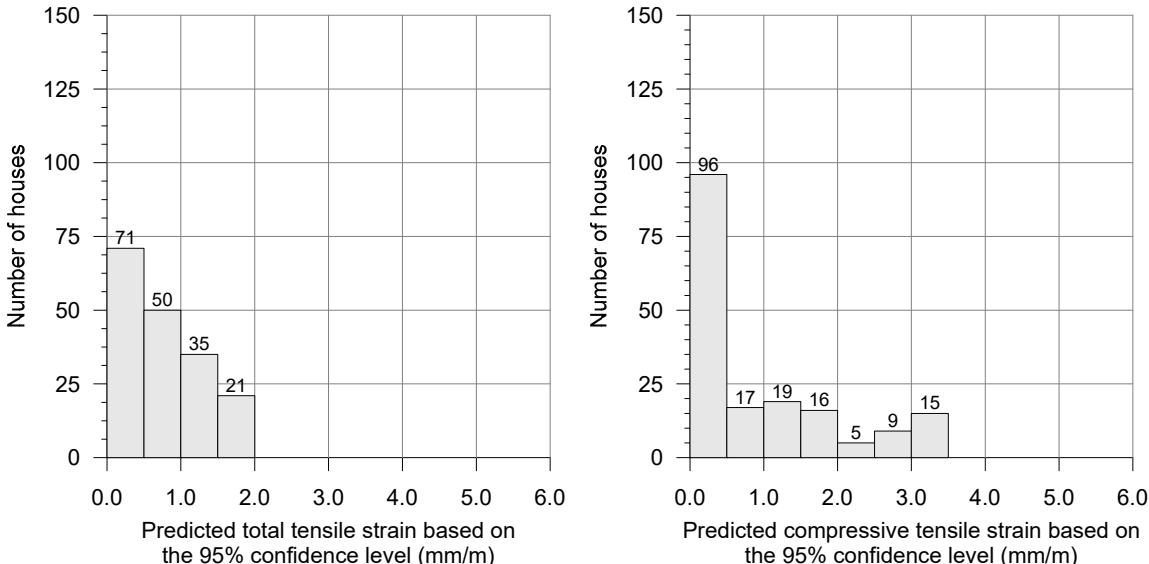
The predicted distributions of strain due to the extraction of LW709 to LW711 and LW905 are described in Chapter 4. The houses are at discrete locations and, therefore, the most relevant distribution of strain is the maximum strains measured in individual survey bays above previous longwall mining, which is summarised in Section 4.3.1. The maximum predicted total strains directly above the proposed longwalls are 1.0 mm/m tensile and 1.6 mm/m compressive based on the 95 % confidence levels.

The strains have been predicted for each of the houses using the method described by Barbato (2017). This method considers the position of each house relative to the longwalls, the surface slope, surface lithology and the potential for irregular anomalous movements.

The predicted total strains for each of the houses within the Study Area are provided in Table D.02, in Appendix D. The distributions of the predicted total strains based on the mean and on the 95 % confidence levels are provided in Fig. 6.39 and Fig. 6.40, respectively.



**Fig. 6.39 Predicted total tensile strain (left-hand side) and total compressive strain (right-hand side) for the houses based on the mean**



**Fig. 6.40 Predicted total tensile strain (left-hand side) and total compressive strain (right-hand side) for the houses based on the 95 % confidence level**

The houses within the Study Area are predicted to experience total tensile strains between 0.2 mm/m and 1.7 mm/m and total compressive strains between 0.2 mm/m and 3.5 mm/m based on the 95 % confidence levels. The predicted mean values range between 0.2 mm/m and 1.0 mm/m tensile and compressive.

### 6.22.3. Reported impacts for houses at Appin Area 7

The existing LW702 to LW708A have mined directly beneath 37 houses and there is an additional 52 houses located outside the mining area and within the 35° angle of draw. The impacts to these houses due to this existing mining should provide a guide to the potential impacts due to the proposed longwalls.

IMC has provided MSEC with details of the damage to houses that has been reported to them due to the mining of LW702 to LW708A. There are 20 claims that relate to damage of the houses that have been categorised into repair categories R0 to R5, as described in Appendix B.

A summary of the reported impacts for houses due to LW702 to LW708A is provided in Table 6.37.

**Table 6.37 Reported impacts for the houses due to LW702 to LW708A**

Location	Repair category			
	No Claim or R0	R1 or R2	R3 or R4	R5
Houses directly above the mining area (37 total)	49 % (18 houses)	24 % (9 houses)	22 % (8 houses)	5 % (2 houses)
Houses outside the mining area and within the 35° angle of draw (52 total)	98 % (51 houses)	2 % (1 house)	0 % (0 houses)	0 % (0 houses)
All houses within the 35° angle of draw (89 total)	78 % (69 houses)	11 % (10 houses)	9 % (8 houses)	2 % (2 houses)

It has been reported that 78 % of houses experienced Nil or Category R0 impacts, being 18 of the 37 houses located above the mining area and 51 of the 52 houses located outside the mining area. There were 10 houses (i.e. 11 %) that experienced Category R1 or R2 impacts, of which 9 were located above the longwalls and 1 was outside the mining area. There were also 8 houses (i.e. 9 %) that experience Category R3 or R4 impacts and 2 houses (i.e. 2 %) that experienced Category R5 impacts, all of which were located above the mining area.

In all cases, the houses remained in safe and serviceable conditions. The property owners were not exposed to immediate or sudden safety hazards due to the mining of LW702 to LW708A.

### 6.22.4. Impact assessments for the Houses

The following sections provide the impact assessments for the houses within the Study Area.

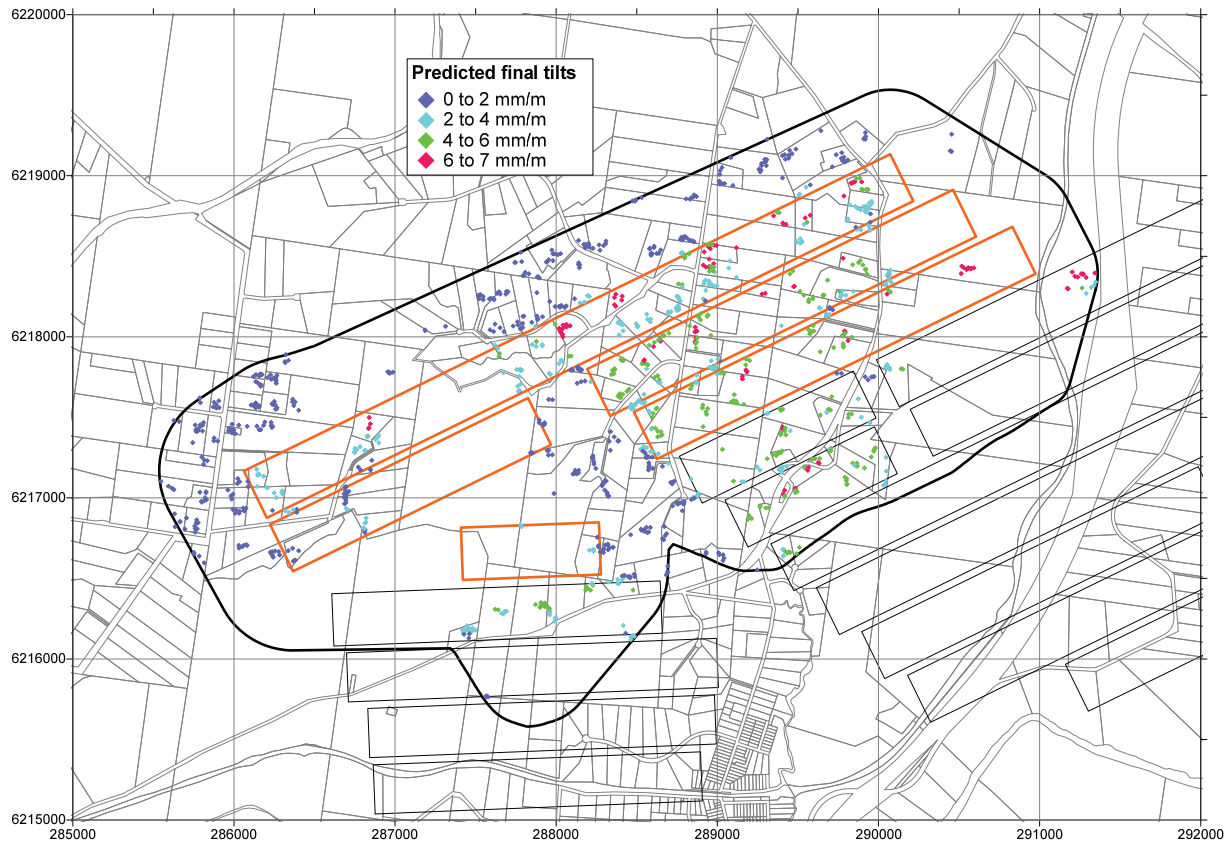
#### *Potential impacts resulting from vertical subsidence*

Vertical subsidence does not directly affect the stability or serviceability of houses. The potential for impacts on houses is affected by differential subsidence, which includes tilt, curvature and strain, and the impact assessments based on these parameters are described in the following sections.

Vertical subsidence can, in some cases, affect the heights of houses above the flood level. A detailed surface water assessment has been carried out by the special consultant on the project (SLR, 2020).

#### *Potential impacts resulting from tilt*

The maximum predicted tilt for the houses within the Study Area is 7.0 mm/m (i.e. 0.7 %, or 1 in 143). The distribution of predicted final tilts for the houses within the Study Area is provided in Fig. 6.41. The greatest tilts occur at the houses located adjacent to the maingates of the existing and proposed longwalls.



**Fig. 6.41 Distribution of predicted final total tilts for the houses within the Study Area**

It has been found from past longwall mining experience that tilts of 7 mm/m or less generally do not result in adverse impacts on houses. Some minor serviceability impacts can occur at these levels of tilt, including door swings and issues with roof gutter and wet area drainage, all of which can be remediated using normal building maintenance techniques.

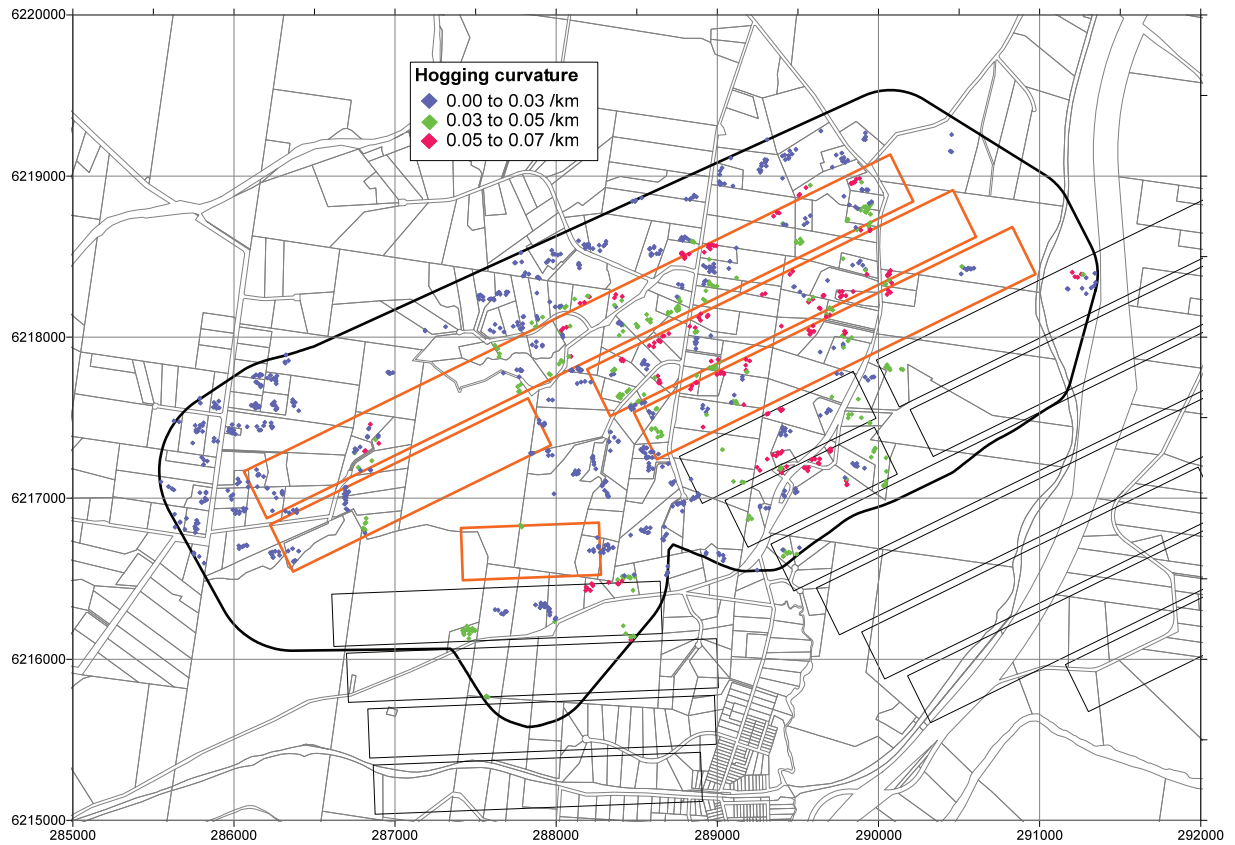
It is expected, therefore, that only minor serviceability impacts would occur for the houses within the Study Area as a result of the mining-induced tilt. It is possible, however, that more substantial serviceability impacts could develop at some houses, as a result of non-conventional ground movements, which could require the releveling of wet areas or, in some cases, the releveling of parts of the building structures.

It is expected that, in all cases, the houses within the Study Area will remain in safe and serviceable condition as a result of the mining induced tilts, as tilts by themselves rarely impact on the stability of building structures at the levels that are predicted to occur.

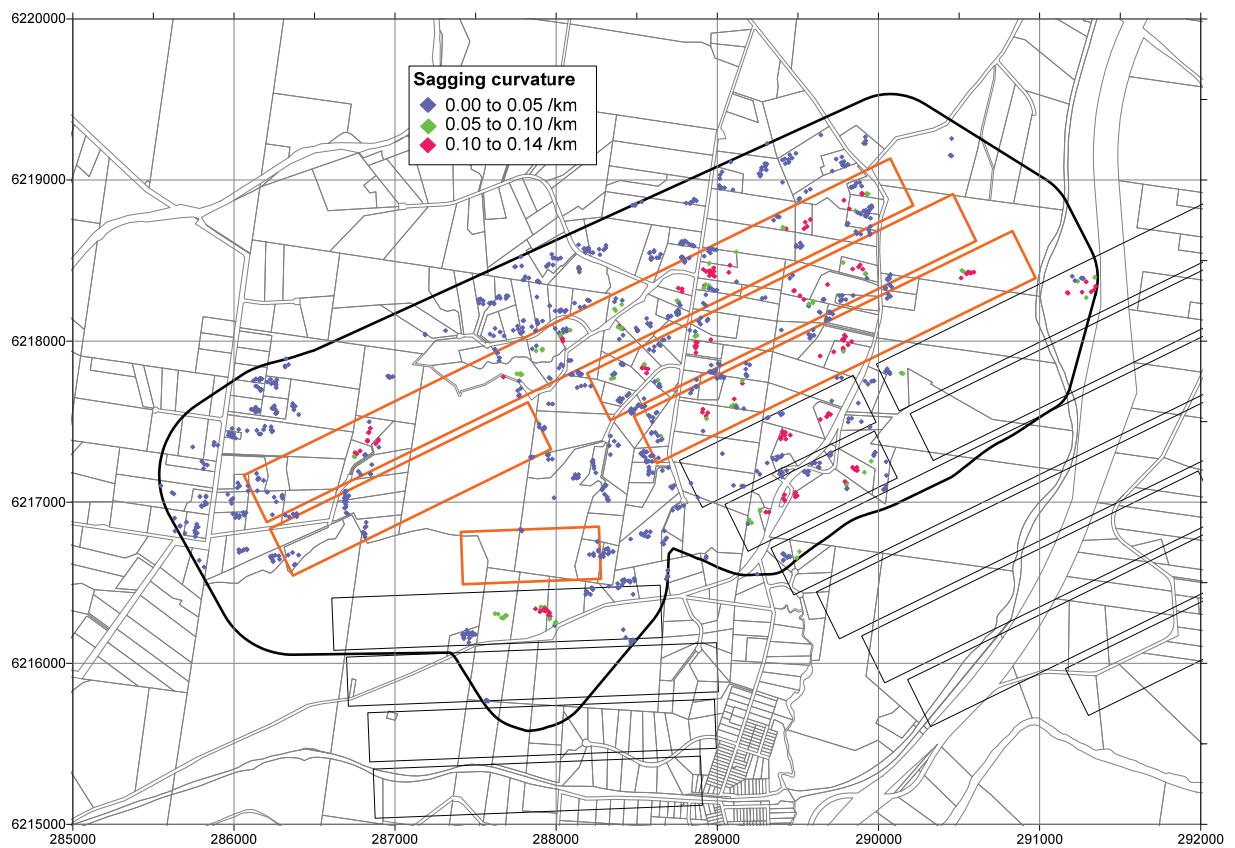
#### *Potential impacts resulting from curvature and strain*

It has been found from past longwall mining experience that the majority of impacts on houses are a result of the mining-induced curvature and strains. The maximum predicted curvatures for the houses within the Study Area are  $0.07 \text{ km}^{-1}$  hogging and  $0.14 \text{ km}^{-1}$  sagging, which represent minimum radii of curvatures of 14 km and 7 km, respectively.

The distributions of the maximum predicted total hogging and sagging curvatures for the houses within the Study Area are provided in Fig. 6.42 and Fig. 6.43. The greatest predicted hogging curvatures occur adjacent to the maingates and the greatest predicted sagging curvatures occur near the centrelines of the existing and proposed longwalls.



**Fig. 6.42** Distribution of maximum predicted total hogging curvature for the houses within the Study Area



**Fig. 6.43** Distribution of maximum predicted total sagging curvature for the houses within the Study Area

Building structures have been directly mined beneath at a number of collieries throughout the NSW coalfields. The experience gained has provided substantial information that has been used to continually develop the methods of impact assessment for houses. The assessments provided in this report are based on the latest research, which is summarised in Appendix B.

The probabilities of impacts for each house within the Study Area have been assessed using the method developed as part of ACARP Research Project C12015 (Waddington, 2009), which has been updated based on observations of impacts in the Southern Coalfield up to 2016. This method uses the primary parameters of predicted ground curvature and type of construction for each house, as identified and described in Section 6.22.1.

Trend analyses following the mining of longwalls in the Southern Coalfield indicate that the chance of impact is higher for the following houses:

- houses predicted to experience higher strains and curvatures;
- houses with masonry walls;
- masonry walled houses that are constructed on strip footings;
- larger houses; and
- houses with variable foundations, such as those with extensions added.

The probabilities of impacts for the houses within the Study Area have been assessed using the method developed as part of ACARP Research Project C12015 (Waddington, 2009), which has been updated based on observations of impacts in the Southern Coalfield up to 2016. This method uses the primary parameters of ground curvature and type of construction and is described in Appendix B. The parameter of strain is indirectly used in this method due to its relationship with curvature.

The maximum natural grades within 25 m of each of the houses within the Study Area are less than 1 in 3 for 65 % of cases, between 1 in 3 and 1 in 2 for 30 % of cases and greater than 1 in 2 in 5 % of cases. The natural grades in close proximity to the houses within the Study Area are reasonably similar to those where houses have been directly mined elsewhere in the Southern Coalfield. The ACARP method for assessing impacts on houses should therefore provide a reasonable indication of the overall levels of potential impact for the houses within the Study Area. However, some houses that are located on or near the steep slopes could experience higher impacts and this is discussed separately in Section 6.22.5.

A summary of the maximum predicted subsidence effects and the assessed impacts for the houses within the Study Area is provided in Table D.03, in Appendix D. The overall distribution of the assessed impacts for the houses within the Study Area is provided in Table 6.38. The assessments have been conservatively based on all houses comprising brick structures on strip footings.

**Table 6.38 Assessed impacts for the houses within the Study Area**

Location	Repair category			
	No Claim or R0	R1 or R2	R3 or R4	R5
Houses directly above the mining area (103 total)	50 % ~ 60 %	25 % ~ 31 %	11 % ~ 15 %	3 % ~ 5 %
Houses outside the mining area (71 total)	83 % ~ 93 %	6 % ~ 12 %	1 % ~ 5 %	0 % ~ 1 %
All houses within the Study Area (175 total)	63 % ~ 73 %	17 % ~ 23 %	7 % ~ 11 %	2 % ~ 3 %

The repair categories R0 to R5 are described in Appendix B.

It has been assessed that approximately 63 % to 73 % of the houses within the Study Area will experience Nil or Category R0 impacts, approximately 17 % to 23 % will experience Category R1 or R2 impacts, approximately 7 % to 11 % will experience Category R3 or R4 impacts and approximately 2 % to 3 % will experience Category R5 impacts.

The reported impacts to houses due to the mining of LW702 to LW708A is described in Section 6.22.3. There were 89 houses located within the 35° angle of draw, of which, 78 % experienced Nil or Category R0 impacts, 11 % experienced Category R1 or R2 impacts, 9 % experienced Category R3 or R4 impacts and 2 % experienced Category R5 impacts.

The assessed impacts for the houses within the Study Area due to the proposed LW709 to LW711 and LW905 are similar to that reported for the existing LW702 to LW708A. While the assessed rate for minor impacts (i.e. R1 or R2) is greater for the proposed longwalls, the assessed rates for major impacts (i.e. R3, R4 or R5) are similar.

Severe impacts have previously occurred as a result of substantial non-conventional movements and in plateau areas away from incised valleys, the locations of which cannot be predicted prior to mining. The impacts, however, develop gradually such that they can be detected early and repairs can be undertaken incrementally to ensure that the houses remain safe and serviceable during mining.

As noted in Appendix B, at the time of writing ACARP Research Project C12015, the observed proportion of houses where SA NSW and affected landowners had decided to rebuild rather than repair (Category R5) impacts was less than 0.5 %. Since the publication of the research report, the proportion of houses where a decision has been made to rebuild has increased to approximately 0.7 %.

The observed proportion of houses with Category R1 to R4 impacts have also increased since the original ACARP study. This is partly due to the time lag effect between the mining impact, when damage is claimed by residents and when the nature and level of the damage requiring repairs is assessed in detail by SA NSW. The latest review includes observations up to the end of Tahmoor Longwall 29 in 2016, which was approximately two years after the completion of Tahmoor Longwall 27 and one year after the completion of Longwall 28, which was the last panel to directly mine beneath the urban areas of Tahmoor.

The primary risk associated with mining beneath houses is public safety. Historically, residents have not been exposed to immediate and sudden safety hazards as a result of impacts that occur due to mine subsidence movements in the NSW coalfields, where the depths of cover were greater than 400 m, such as the case above the proposed longwalls.

Emphasis is placed on the words “immediate and sudden” as in rare cases, some structures have experienced severe impacts, but the impacts did not present an immediate risk to public safety as they developed gradually with ample time for residents to relocate.

All houses within the Study Area are expected to remain safe throughout the mining period, provided that effective management measures are adopted during mining and these are described in the following section.

#### 6.22.5. Houses located on or near steep slopes

Houses are located on or near steep slopes within the Study Area. These include houses at the top and near the base of Razorback Range in the western part of the Study Area. There are also other houses located on or near steep slopes in the eastern part of the Study Area.

The houses located on or within 25 m of the steep slopes are shown in Table D.01, in Appendix D. There are 77 houses within the Study Area located on or near the steep slopes, of which 26 are located at the tops, 23 are located on the sides and 28 are located near the bases. The majority of these houses are located on or near steep slopes with natural grades between 1 in 3 and 1 in 2. The assessment method for houses is based on observations of mining beneath houses on steep slopes with similar ranges of grades.

A summary of the 8 houses located at the top of Razorback Range is provided in Table 6.39.

**Table 6.39 Houses at the top of Razorback Range and directly above the proposed longwalls**

Reference	Location	Maximum predicted total vertical subsidence (mm)	
		After LW710A	After LW711
O03h01	Directly above LW710A	225	600
O04h01	Directly above LW710A	150	625
O05h01	Directly above LW711	60	675
O06h01	Directly above LW711	50	950
O08h01	Directly above LW711	50	925
O17h01	Directly above LW710A	70	160
O18h01	Adjacent to LW711	40	90
O18h02	Adjacent to LW711	30	80

The steep slopes on Razorback Range could experience increased horizontal movements in the downslope direction, causing tension cracks to appear at the tops and along the sides of the slopes and compression ridges to form at the bottoms of the slopes. The houses located at the top of Razorback Range could experience increased effects due to the potential for non-conventional ground movements.

A Landslide Risk Assessment (LRA) for the proposed LW709 to LW711 and LW905 has been carried out by GHD (2021d). The key conclusion of the LRA is that the “*results of the preliminary investigations indicate that the structures located on the identified steep slopes should be able to be effectively managed, using the techniques and management tools available and previously employed in similar geotechnical circumstances, to permit Appin Mine to extract the proposed longwalls*”.

Geotechnical inspections have been carried out by GHD for three of the properties located at the top of Razorback Range (Refs. O02, O17 and O18). The geotechnical assessment reports (GHD 2021a, 2021b and 2021c) state that:

*“For the nominated structures, the risk-to-property (in a landslide risk management sense) under current conditions has been appraised as VERY LOW. This risk level is expected to be adopted by a regulator as acceptable.*

*The proposed longwall mining has been judged to produce a minor adverse additional impact upon the assessed risk-to-property estimates across the sites...”* and that

*“The [landslide] risk levels assessed for current conditions, upon which mining related impact are imposed, remain unchanged from the currently existing risk levels.*

*Risk-to-life has been appraised for selected relevant scenarios. The appraised risk-to-life levels were found to be lower than levels that would be ACCEPTABLE to a regulator.”*

GHD (2021d) has categorised other properties within the Study Area into “*those not requiring additional appraisal, those requiring a brief site visit (contingent upon landholder approval for access) to confirm the site setting not observable from the public roadways, to more detailed appraisals for residences in high hazard environments*”. GHD (2021d) states that the “*residential sites that require appraisals can be conducted on a longwall-by-longwall basis*” so that “*the hazards can be managed through each subordinate longwall’s Structures Management Plan*”.

Monitoring has been established for the houses located at the top of Razorback Range for the mining of LW904, as shown in Drawing No. MSEC1117-01.

The existing ground monitoring lines (red points shown on Drawing No. MSEC1117-01) include the: Early Warning Line above the western end of LW904, five monitoring lines on the side of Razorback Range located above and to the north of the maingate of LW904, and far-field 3D monitoring points located at the top of Razorback Range. Monitoring points will also be established around the houses located at the top of Razorback Range, subject to the approval of the individual property owners.

There are existing Global Navigation Satellite System (GNSS) units (blue points shown on Drawing No. MSEC1117-01) located above the mining area and on the side of Razorback Range, and proposed GNSS units (green points shown on Drawing No. MSEC1117-01) for three properties located at the top of Razorback Range, subject to the approval of the individual property owners.

Further ground monitoring lines and GNSS units will be established for LW709 to LW711 and LW905 as part of the Structures Management Plans for these proposed longwalls.

#### **6.22.6. Management of potential impacts on houses**

IMC has extensive experience of mining beneath houses at Appin and West Cliff Collieries. It has developed and acted in accordance with risk management plans to manage potential impacts to residential structures during the mining of LW701 to LW708 and LW901 to LW903.

The Subsidence Management Process has been developed in consideration of the following facts and observations:

1. Australian standards have been available for use in the design of structures since 1948. The majority of the houses within the Study Area have been constructed within and after the declaration of the Wilton MSD and South Campbelltown MSD;
2. there is sufficient redundancy in structural design such that ductile deformation will develop and be noticeable to residents before structural failure occurs;
3. subsidence movements develop gradually over time at Appin Colliery as they have above other previously extracted longwalls at similar depths of cover;
4. experiences during the mining at Appin Colliery and other collieries in the Southern Coalfield have found that the most effective method of managing potential impacts on the safety and serviceability of structures are by way of community consultation. Residents living within the active subsidence zone have often provided early feedback to Appin Colliery and/or SA NSW about impacts developing at their houses or along their local roads. Contact is made well before impacts develop to a level of severity sufficient to become a safety hazard;
5. on the basis of the above, there is sufficient time for residents to notify IMC or SA NSW of significant displacement or deflection well before structural failure will occur;

6. the conclusions are supported by the observation that residents have not been exposed to immediate and sudden safety hazards as a result of impacts that occur due to mine subsidence movements at Appin Colliery or other collieries in the Southern Coalfield at similar depths of cover. This includes the recent experience at Tahmoor Mine during the mining of Longwalls 22 to 32, which have affected more than 2000 houses and civil structures; and
7. while severe impacts have developed during mining, there is sufficient redundancy in structural design such that when structures have experienced severe impacts, they have developed gradually with ample time for residents to notify IMC or SA NSW to repair the structure and/or relocate residents before structural failure occurs.

While the three most important factors in managing risks to public safety are redundancy in structural design, gradual development of subsidence movements and an effective community consultation program, a number of additional management measures have been undertaken, including site specific investigations, regular surveys and inspections during mining and triggered response measures.

With appropriate management plans in place, it is considered that the houses will remain safe at all times during the extraction of the proposed longwalls, even if actual subsidence movements were greater than the predictions or substantial non-conventional movements occurred.

Impacts to the houses would be repaired or, if required, replaced in accordance with the *Coal Mine Subsidence Compensation Act 2017*.

## 6.23. Associated residential structures

### 6.23.1. Non-residential building structures

The descriptions, predictions and impact assessments for the rural structures are provided in Section 6.13.

### 6.23.2. Swimming pools

The locations of the swimming pools are shown in Drawing No. MSEC1117-13.

There are 70 privately owned swimming pools that have been identified within the Study Area. Details of the pools are included in Table D.01, in Appendix D. A summary of the swimming pools within the Study Area is provided in Table 6.40.

**Table 6.40 Details of the swimming pools within the Study Area**

Type	Number located above the existing and approved longwalls	Number located above the proposed LW709 to LW711 and LW905	Number located outside the mining area
Pools	9	26	35

There are 35 pools (i.e. 50 %) that are located above the existing and proposed longwalls and 35 pools (i.e. 50 %) that are located outside the mining area.

Predictions of conventional subsidence, tilt and curvature have been made at the centroid and around the perimeters of each pool. A summary of the maximum predicted values of conventional subsidence, tilt and curvature for each pool within the Study Area is provided in Table D.02, in Appendix D.

A summary of the maximum predicted values of total vertical subsidence, tilt and curvatures for the swimming pools is provided in Table 6.41. The table provides the maximum predicted values for the pools at any time during or after the extraction of each longwall.

**Table 6.41 Maximum predicted total vertical subsidence, tilt and curvatures for the pools**

After longwall	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
LW709	1300	5.5	0.06	0.12
LW710A	1300	5.5	0.06	0.12
LW710B	1375	5.5	0.06	0.12
LW711	1375	7.0	0.07	0.12
LW905	1375	7.0	0.07	0.12

The maximum predicted tilt for the pools is 7.0 mm/m (i.e. 0.7 %, or 1 in 143). The maximum predicted curvatures are 0.07 km<sup>-1</sup> hogging and 0.13 km<sup>-1</sup> sagging, which represent minimum radii of curvature of 14 km and 8 km, respectively.

The predicted distributions of strain due to the proposed mining are described in Chapter 4. The pools are at discrete locations and, therefore, the most relevant distribution of strain is the maximum strains measured in individual survey bays above previous longwall mining, which is summarised in Section 4.3.1. The maximum predicted total strains directly above the proposed longwalls are 1.0 mm/m tensile and 1.6 mm/m compressive based on the 95 % confidence levels.

Mining-induced tilts are more noticeable in pools than other structures due to the presence of the water line and the small gap to the edge coping, particularly when the pool lining has been tiled. Skimmer boxes are also susceptible to being lifted above the water line due to mining tilt.

The Australian Standard AS2783-1992 (Use of reinforced concrete for small swimming pools) requires that pools be constructed level ± 15 mm from one end to the other. This represents a tilt of approximately 3.3 mm/m for pools that are 10 metres in length. Australian Standard AS/NZS 1839:1994 (Swimming pools – Pre-moulded fibre-reinforced plastics – Installation) also requires that pools be constructed with a tilt of 3 mm/m or less.

It can be seen from Table D.02, that 20 pools within the Study Area (i.e. 29 % of the total) are predicted to experience final tilts greater than 3 mm/m, at the completion of the proposed longwalls, which is greater than the Australian Standard. It is likely, therefore that these pools would require remediation of the pool copings after the completion of active subsidence. It is possible, if the tilts were fully realised at the pools with the higher predicted tilts, that the final tilts could be difficult to remediate and, in these cases, the pools would need to be rebuilt.

The ranges of predicted maximum curvatures and strains for the pools are similar to those previously experienced at Appin Colliery and other nearby collieries in the Southern Coalfield. The incidence and levels of impacts on the pools within the Study Area, therefore, are expected to be similar to those previously experienced in the Southern Coalfield.

Observations during the mining of Tahmoor Mine Longwalls 22 to 32 have shown that pools, particularly in-ground pools, are more susceptible to severe impacts than houses and other structures. Pools cannot be easily repaired and most of the impacted pools need to be replaced in order to restore them to pre-mining condition or better.

As of June 2017, a total of 157 pools have experienced mine subsidence movements during the mining of Longwalls 22 to 30, of which 141 were located directly above the extracted longwalls. A total of 36 pools have reported impacts, all of which were located directly above the extracted longwalls. This represents an impact rate of approximately 23 %. A higher proportion of impacts have been observed for in-ground pools, particularly fibreglass pools. The majority of the impacts related to tilt or cracking, though in a small number of cases the impacts were limited to damage to skimmer boxes or the edge coping.

In addition to the above, a number of pool gates have been impacted by mine subsidence during the mining of Longwall 22 to 32. While the gates can be easily repaired, the consequence of breaching pool fence integrity is considered to be severe. As a result, with landholder agreement, IMC inspects the integrity of pool fences regularly during the active subsidence period.

Impacts to the pools, fences and gates would be repaired or, if required, replaced in accordance with the *Coal Mine Subsidence Compensation Act 2017*.

### 6.23.3. Tennis Courts

The locations of the tennis courts are shown in Drawing No. MSEC1117-13.

There are 9 tennis courts that have been identified within the Study Area. Details of the tennis courts are included in Table D.01, in Appendix D. A summary of the tennis courts within the Study Area is provided in Table 6.42.

**Table 6.42 Details of the tennis courts within the Study Area**

Type	Number located above the existing and approved longwalls	Number located above the proposed LW709 to LW711 and LW905	Number located outside the mining area
Tennis Courts	1	6	2

There are 7 tennis courts (i.e. 78 %) that are located above the existing and proposed longwalls and 2 tennis courts (i.e. 22 %) that are located outside the mining area.

Predictions of conventional subsidence, tilt and curvature have been made at the centroid and around the perimeters of each tennis court. A summary of the maximum predicted values of conventional subsidence, tilt and curvature for each tennis court within the Study Area is provided in Table D.02, in Appendix D.

A summary of the maximum predicted values of total vertical subsidence, tilt and curvatures for the tennis court is provided in Table 6.43. The table provides the maximum predicted values for the tennis courts at any time during or after the extraction of each longwall.

**Table 6.43 Maximum predicted total vertical subsidence, tilt and curvatures for the tennis courts**

After longwall	Maximum predicted total vertical subsidence (mm)	Maximum predicted total tilt (mm/m)	Maximum predicted total hogging curvature (km <sup>-1</sup> )	Maximum predicted total sagging curvature (km <sup>-1</sup> )
LW709	1100	6.0	0.05	0.10
LW710A	1100	6.0	0.05	0.10
LW710B	1275	5.0	0.06	0.12
LW711	1325	7.0	0.07	0.12
LW905	1325	7.0	0.07	0.12

The maximum predicted tilt for the pools is 7.0 mm/m (i.e. 0.7 %, or 1 in 143). The maximum predicted curvatures are 0.07 km<sup>-1</sup> hogging and 0.12 km<sup>-1</sup> sagging, which represent minimum radii of curvature of 14 km and 8 km, respectively.

The predicted distributions of strain due to the proposed mining are described in Chapter 4. The tennis courts are at discrete locations and, therefore, the most relevant distribution of strain is the maximum strains measured in individual survey bays above previous longwall mining, which is summarised in Section 4.3.1. The maximum predicted total strains directly above the proposed longwalls are 1.0 mm/m tensile and 1.6 mm/m compressive based on the 95 % confidence levels.

The tennis courts within the Study Area comprise both grass and concrete surfaces. It possible that the maximum predicted curvatures and strains could result in minor cracking in the grass surfaced tennis courts; however, cracking is expected to be minor and easily repairable. It is expected that ground strains of these magnitudes would arch around the concrete tennis courts and not be fully transferred into the pavements. It is possible, that some minor surface cracking could occur in the concrete surfaces, but it is expected to be of a minor nature and easily repairable.

#### 6.23.4. Rigid external pavements

Adverse impacts on rigid external pavements, such as driveways and footpaths, are often reported to SA NSW in the Southern Coalfield. This is because pavements are typically thin relative to their length and width. The design of external pavements is also not regulated by Council or SA NSW.

A study by MSEC of 120 properties at Tahmoor and Thirlmere indicated that 98 % of the properties with external concrete pavements demonstrated some form of cracking prior to mining. These cracks are sometimes difficult to distinguish from cracks caused by mine subsidence. It is therefore uncertain how many claims for damage can be genuinely attributed to mine subsidence impacts.

Impacts to external pavements would be repaired or, if required, replaced in accordance with the *Coal Mine Subsidence Compensation Act 2017*.

## 6.24. Conclusion

The assessments provided in this report indicate that the assessed levels of impact on the natural and built features can be managed by the preparation and implementation of appropriate management strategies. It should be noted, however, that more detailed assessments of some natural and built features have been undertaken by other specialist consultants, and the findings in this report should be read in conjunction with the findings in all other relevant reports.

## 7.0 GLOSSARY OF TERMS AND DEFINITIONS

Some of the more common mining terms used in the report are defined below:

<b>Angle of draw</b>	The angle of inclination from the vertical of the line connecting the goaf edge of the workings and the limit of subsidence (which is usually taken as 20 mm of subsidence).
<b>Chain pillar</b>	A block of coal left unmined between the longwall extraction panels.
<b>Cover depth (H)</b>	The depth from the surface to the top of the seam. Cover depth is normally provided as an average over the area of the panel.
<b>Closure</b>	The reduction in the horizontal distance between the valley sides. The magnitude of closure, which is typically expressed in the units of <i>millimetres (mm)</i> , is the greatest reduction in distance between any two points on the opposing valley sides. It should be noted that the observed closure movement across a valley is the total movement resulting from various mechanisms, including conventional mining-induced movements, valley closure movements, far-field effects, downhill movements and other possible strata mechanisms.
<b>Critical area</b>	The area of extraction at which the maximum possible subsidence of one point on the surface occurs.
<b>Curvature</b>	The change in tilt between two adjacent sections of the tilt profile divided by the average horizontal length of those sections, i.e. curvature is the second derivative of subsidence. Curvature is usually expressed as the inverse of the <b>Radius of Curvature</b> with the units of <i>1/kilometres (km<sup>-1</sup>)</i> , but the value of curvature can be inverted, if required, to obtain the radius of curvature, which is usually expressed in <i>kilometres (km)</i> . Curvature can be either <b>hogging</b> (i.e. convex) or <b>sagging</b> (i.e. concave).
<b>Extracted seam</b>	The thickness of coal that is extracted. The extracted seam thickness is thickness normally given as an average over the area of the panel.
<b>Effective extracted seam thickness (T)</b>	The extracted seam thickness modified to account for the percentage of coal left as pillars within the panel.
<b>Face length</b>	The width of the coalface measured across the longwall panel.
<b>Far-field movements</b>	The measured horizontal movements at pegs that are located beyond the longwall panel edges and over solid unmined coal areas. Far-field horizontal movements tend to be bodily movements towards the extracted goaf area and are accompanied by very low levels of strain.
<b>Goaf</b>	The void created by the extraction of the coal into which the immediate roof layers collapse.
<b>Goaf end factor</b>	A factor applied to reduce the predicted incremental subsidence at points lying close to the commencing or finishing ribs of a panel.
<b>Horizontal displacement</b>	The horizontal movement of a point on the surface of the ground as it settles above an extracted panel.
<b>Inflection point</b>	The point on the subsidence profile where the profile changes from a convex curvature to a concave curvature. At this point the strain changes sign and subsidence is approximately one half of S max.
<b>Incremental subsidence</b>	The difference between the subsidence at a point before and after a panel is mined. It is therefore the additional subsidence at a point resulting from the excavation of a panel.
<b>Panel</b>	The plan area of coal extraction.
<b>Panel length (L)</b>	The longitudinal distance along a panel measured in the direction of mining from the commencing rib to the finishing rib.
<b>Panel width (Wv)</b>	The transverse distance across a panel, usually equal to the face length plus the widths of the roadways on each side.
<b>Panel centre line</b>	An imaginary line drawn down the middle of the panel.
<b>Pillar</b>	A block of coal left unmined.
<b>Pillar width (Wpi)</b>	The shortest dimension of a pillar measured from the vertical edges of the coal pillar, i.e. from rib to rib.

<b>Shear deformations</b>	The horizontal displacements that are measured across monitoring lines and these can be described by various parameters including; horizontal tilt, horizontal curvature, mid-ordinate deviation, angular distortion and shear index.
<b>Strain</b>	<p>The change in the horizontal distance between two points divided by the original horizontal distance between the points, i.e. strain is the relative differential displacement of the ground along or across a subsidence monitoring line. Strain is dimensionless and can be expressed as a decimal, a percentage or in parts per notation.</p> <p><b>Tensile Strains</b> are measured where the distance between two points or survey pegs increases and <b>Compressive Strains</b> where the distance between two points decreases. Whilst mining-induced <b>strains</b> are measured <b>along</b> monitoring lines, ground <b>shearing</b> can occur both vertically, and horizontally <b>across</b> the directions of the monitoring lines.</p>
<b>Sub-critical area</b>	An area of panel smaller than the critical area.
<b>Subsidence</b>	<p>The vertical movement of a point on the surface of the ground as it settles above an extracted panel, but, 'subsidence of the ground' in some references can include both a vertical and horizontal movement component. The vertical component of subsidence is measured by determining the change in surface level of a peg that is fixed in the ground before mining commenced and this vertical subsidence is usually expressed in units of <i>millimetres (mm)</i>.</p> <p>Sometimes the horizontal component of a peg's movement is not measured, but in these cases, the horizontal distances between a particular peg and the adjacent pegs are measured.</p>
<b>Super-critical area</b>	An area of panel greater than the critical area.
<b>Tilt</b>	The change in the slope of the ground as a result of differential subsidence, and is calculated as the change in subsidence between two points divided by the horizontal distance between those points. Tilt is, therefore, the first derivative of the subsidence profile. Tilt is usually expressed in units of <i>millimetres per metre (mm/m)</i> . A tilt of 1 mm/m is equivalent to a change in grade of 0.1 %, or 1 in 1000.
<b>Uplift</b>	An increase in the level of a point relative to its original position.
<b>Upsidence</b>	Upsidence results from the dilation or buckling of near-surface strata at or near the base of the valley. The magnitude of upsidence, which is typically expressed in the units of <i>millimetres (mm)</i> , is the difference between the observed subsidence profile within the valley and the conventional subsidence profile which would have otherwise been expected in flat terrain.

## APPENDIX A. REFERENCES

## References

- Barbato (2017). *Development of improved methods for the prediction of horizontal movement and strain at the surface due to longwall coal mining*. James Barbato. PhD thesis, University of New South Wales. [http://www.unswworks.unsw.edu.au/UNSWORKS:unsworks\\_search\\_scope:unsworks\\_47542](http://www.unswworks.unsw.edu.au/UNSWORKS:unsworks_search_scope:unsworks_47542)
- Biosis (2021). *Appin Longwalls 709, 710A, 710B, 711 and 905. Heritage Impact Assessment*. Biosis Pty Ltd, Project No. 33531, dated April 2021.
- BOM, (2020). *Australian Groundwater Explorer*, as viewed in July 2020. Bureau of Meteorology at <http://www.bom.gov.au/water/groundwater/explorer/map.shtml>
- BSO (2011). *Project Approval – Section 75J of the Environmental Planning and Assessment Act 1979*. Application Number: 08\_0150, Proponent: BHP Billiton Illawarra Coal Holdings Pty Ltd, Project: Bulli Seam Operations Project.
- Cardno (2020). *Memorandum: Twin Bridges over Nepean River at Douglas Park – Review of Effects of Mining at End of LW902*. Cardno, memorandum, dated 6 May 2020.
- Cardno (2021). *Aquatic Flora and Fauna Assessment, Appin Area 7&9 Longwalls 709, 710A, 710B, 711 and 905*. Cardno, Reference 59919104, dated April 2021.
- DeBono et al. (2014) *A review of the accuracy and reliability of empirical subsidence predictions*. In: Proceedings of the ninth triennial MSTs conference on mine subsidence. Mine Subsidence Technological Society, Vol. 2, pp. 461-472.
- DoP (2008). *Impacts of Underground Coal Mining on Natural Features in the Southern Coalfield: Strategic Review*. NSW Department of Planning, July 2008.
- DB4YD (2020). Dial Before You Dig search, dated July 2020.
- DPIE (2008). *Impacts of Underground Coal Mining on Natural Features in the Southern Coalfield - Strategic Review*. State of New South Wales through the NSW Department of Planning.
- DPIE (2012). *Standard and Model Conditions for Underground Mining*. NSW Department of Planning, Industry and Environment. [http://www.planning.nsw.gov.au/Portals/0/Development/SSD\\_-\\_Draft\\_Model\\_Conditions\\_-\\_Underground\\_Mine.pdf](http://www.planning.nsw.gov.au/Portals/0/Development/SSD_-_Draft_Model_Conditions_-_Underground_Mine.pdf)
- GHD (2021a). *LW709, LW710A, LW710B, LW711, LW904 and LW905 "Steep slopes" Landslide Risk Assessment 5 Gibraltar Drive, Razorback NSW*. GHD report, dated January 2021.
- GHD (2021b). *LW709, LW710A, LW710B, LW711, LW904 and LW905 "Steep slopes" Landslide Risk Assessment 8 Gibraltar Drive, Razorback NSW*. GHD report, dated January 2021.
- GHD (2021c). *LW709, LW710A, LW710B, LW711, LW904 and LW905 "Steep slopes" Landslide Risk Assessment 10 Gibraltar Drive, Razorback NSW*. GHD report, dated January 2021.
- GHD (2021d). *Appin Area 7 and 9 Proposed Longwalls Landslide Risk Assessment relating to Mine Subsidence Influences*. GHD report, dated April 2021.
- Jaeger (1969). *Elasticity, fracture and flow*. Springer, 1969.
- Kay, D.R. and Waddington, A.A. (2014). *Effects of mine subsidence, geology and surface topography on observed valley closure movements and development of an updated valley closure prediction method*. ACARP Research Project No. C18015, July 2014.
- Niche (2021). *Appin – Longwalls 709 to 711 and 905 Biodiversity Impact Assessment*. Niche Environment and Heritage, Project No. 6049, dated April 2021.
- Patton and Hendren (1972). *General Report on Mass Movements*. Patton F.D. and Hendren A.J. Second International Congress of Engineering Geology, V-GR1-V-GR57.
- SCIMS (2020). *SCIMS – SIX Maps* website, viewed on the 24 July 2020. Spatial Services, NSW Government. <https://maps.six.nsw.gov.au/scims.html>
- SLR (2020). *Appin Mine Extraction Plan Surface Water Assessment Longwalls 709, 710A, 710B, 711 and 905*. SLR Reference: 630.30102-R01-v.6, dated April 2021.
- SLR (2021). *Appin Mine Extraction Plan Groundwater Impact Assessment*. SLR Reference: 665.10015.00003-R01-V5.0.docx, dated April 2021.
- Waddington, A.A. and Kay, D.R. (2002). *Management Information Handbook on the Undermining of Cliffs, Gorges and River Systems*. ACARP Research Projects Nos. C8005 and C9067, September 2002.
- Waddington, A.A. (2009). *ACARP The Prediction of Mining Induced Movements in Building Structures and the Development of Improved Methods of Subsidence Impact Assessment*. ACARP Research Project C12015, March 2009.

## **APPENDIX B. METHOD OF IMPACT ASSESSMENTS FOR HOUSES**

## APPENDIX B METHOD OF IMPACT ASSESSMENT FOR HOUSES

### B.1. Introduction

The methods for predicting and assessing impacts on building structures have developed over time as knowledge and experience has grown. MSEC has provided predictions and impact assessments for the building structures within the *Study Area* using the latest methods available at this time.

Longwall mining has occurred directly beneath building structures at a number of collieries in the Southern Coalfield, including Appin, West Cliff, Tower and Tahmoor Collieries. The most extensive data has come from extraction of Tahmoor Mine Longwalls 22 to 29, where approximately 1900 houses have experienced subsidence movements. The experiences gained during the mining of these longwalls, as well as longwalls at other collieries in the Southern and Newcastle Coalfields, have provided substantial additional information that has been used to further develop the methods.

The information was initially collected during the mining of Tahmoor Mine Longwalls 22 to 24A and reviewed in two parallel studies, one as part of a funded ACARP Research Project C12015 (Waddington, 2009), and the other at the request of Industry and Investment NSW (now the Department of Planning and Environment – Resources Regulator).

The outcomes of these studies include:-

- Review of the performance of the previous method,
- Recommendations for improving the method of Impact Classification, and
- Recommendations for improving the method of Impact Assessment.

Additional information was collected in 2016 after the completion of Longwall 29 and impact assessments for the houses in this report have been based on the updated information provided. A summary is provided in the following sections.

### B.2. Review of the Performance of the Previous Method

The previous method of impact assessment applied predictions of curvature on the overall length of each house to predict a crack width in the external walls that was classified based primarily in accordance with Table C1 of Australian Standard 2870-1996. This method did not include impacts to other elements, finishes or services.

Extensive data on house impacts has come from extraction of Tahmoor Mine Longwalls 22 to 25 and a comparison between predicted and observed impacts is provided in Table B.1. The comparison is based on pre-mining predictions that were provided in SMP Applications for these longwalls and the observations of impacts using the previous method of impact classification. The comparison is based on information up to 30 November 2008. At that point in time, the length of extraction of Longwall 25 was 611 metres.

A total of 1037 houses and civil structures were affected by subsidence due to the mining of Tahmoor Mine Longwalls 22 to 25 at that time. A total of 175 claims had been received by the MSB, now SA NSW (not including claims that were refused) of which 14 claims did not relate to the main residence or civil structure.

**Table B.1 Summary of comparison between observed and predicted impacts for each structure**

Strain Impact Category	Total no. of observed impacts for structures predicted to be strain impact Category 0	Total no. of observed impacts for structures predicted to be strain impact Category 1	Total no. of observed impacts for structures predicted to be strain impact Category 2	Total
No impact	483	373	20	876
Cat 0	31	70	6	107
Cat 1	8	9	1	18
Cat 2	7	11	2	20
Cat 3	2	2	0	4
Cat 4	3	5	0	8
Cat 5	3	1	0	4
Total	537	471	29	1037
<b>% claim</b>	<b>10 %</b>	<b>21 %</b>	<b>31 %</b>	<b>16 %</b>
<b>% Obs &gt; Pred</b>	<b>4 %</b>	<b>4 %</b>	<b>0 %</b>	<b>-</b>
<b>% Obs &lt;= Pred</b>	<b>96 %</b>	<b>96 %</b>	<b>100 %</b>	<b>-</b>

Given that observed impacts are less than or equal to predicted impacts in 96 % of cases, it is considered that the previous methods are generally conservative even though non-conventional movements were not considered in the predictions and assessments. However, when compared on a house by house basis, the predictions have been substantially exceeded in a small proportion of cases.

The majority, if not all, of the houses that have experienced Category 3, 4 or 5 impacts are considered to have experienced substantial non-conventional subsidence movements. The consideration is based on nearby ground survey results, where localised bumps are observed in subsidence profiles and high localised strain is observed. The potential for impact from non-conventional movements were discussed generally and not included in the specific impact assessments for each structure.

The inability to specify the number or probability of impacts due to the potential for non-conventional movements is a shortcoming of the previous method. It was considered that there was substantial room for improvement in this area and recommendations are provided to improve the previous method.

The comparison shows a favourable observation that the overall proportion of claims increased for increasing observed ground movements. This suggests that the main parameters currently used to make impact assessments (namely predicted conventional curvature and maximum plan dimension of each structure) are credible. Please note that we have stated predicted conventional curvature rather than strain, as predictions of strain were directly based on predictions of conventional curvature.

A substantial over-prediction is observed at the low end of the spectrum of impacts (Category 0 and 1). A number of causes and/or possible causes for the deviations have been identified:

- Construction methods and standards may mitigate against small differential ground movements.
- The impacts may have occurred but the residents have not made a claim for the following reasons:-
  - All structures contain some existing, pre-mining defects. A pre-mining field investigation of 119 structures showed that it is very rare for all elements of a building to be free of cracks. Cracks up to 3 mm in width are commonly found in buildings. Cracks up to 1 mm in width are very common. There is a higher incidence of cracking in brittle forms of construction such as masonry walls and tiled surfaces.
  - In light of the above, additional very slight Category 0 and 1 impacts may not have been noticed by residents. A forensic investigation of all structures before or after mining may reveal that the number of actual impacts is greater than currently known.
  - Similarly, impacts have been noticed but some residents may consider them to be too trivial to make a claim. While difficult to prove statistically, it is considered that the frequency of claims from tenanted properties is less than the frequency of claims from owner-occupied properties.
- The impacts have been noticed but some residents are yet to make a claim at this stage. It has been observed that there is a noticeable time lag between the moment of impact and the moment of making a claim. At the time of the original study in 2008, more claims were therefore expected to be received in the future within areas that have already been directly mined beneath. This has been confirmed by the findings of the most recent study based on information received in 2016. It has also been found that as assessments and repairs were progressively determined at each house, the level of impacts at each house has generally been greater than was originally reported.
- The predictive method is deliberately conservative in a number of ways.
  - Predicted subsidence movements for each structure are based on the maximum predicted subsidence movements within 20 metres of the structure.
  - An additional 0.2 mm/m of strain was added
  - Maximum strains were applied to the maximum plan dimension, regardless of the maximum predicted strain orientation.
  - The method of impact assessment does not provide for “nil impacts”. The minimum assessed level of impact is Category 0.
  - The impact data was based on double-storey full masonry structures in the UK.

Finally, it is considered that the previous method impact classification has masked the true nature and extent of impacts. It is recommended that an improved method of classification be adopted before embarking on any further analysis. This is discussed in the next chapter of this report.

### B.3. Method of Impact Classification

#### B.3.1. Previous Method

The impacts to structures were previously classified in accordance with Table C1 of Australian Standard 2870-1996, but the table has been extended by the addition of Category 5 and is reproduced below.

**Table B.2 Classification of damage with reference to strain**

Impact category	Description of typical damage to walls and required repair	Approximate crack width limit
0	Hairline cracks.	< 0.1 mm
1	Fine cracks which do not need repair.	0.1 mm to 1.0 mm
2	Cracks noticeable but easily filled. Doors and windows stick slightly	1 mm to 5 mm
3	Cracks can be repaired and possibly a small amount of wall will need to be replaced. Doors and windows stick. Service pipes can fracture. Weather-tightness often impaired	5 mm to 15 mm, or a number of cracks 3 mm to 5 mm in one group
4	Extensive repair work involving breaking-out and replacing sections of walls, especially over doors and windows. Window or door frames distort. Walls lean or bulge noticeably. Some loss of bearing in beams. Service pipes disrupted.	15 mm to 25 mm but also depends on number of cracks
5	As above but worse, and requiring partial or complete rebuilding. Roof and floor beams lose bearing and need shoring up. Windows broken with distortion. If compressive damage, severe buckling and bulging of the roof and walls.	> 25 mm

Note 1 of Table C1 states that “Crack width is the main factor by which damage to walls is categorized. The width may be supplemented by other factors, including serviceability, in assessing category of damage.

Impacts relating to tilt were classified according to matching impacts with the description in Table B.3, not the observed actual tilt. This is because many houses that had experience tilts greater than 5 mm had not made a claim to the MSB (now SA NSW).

**Table B.3 Classification of damage with reference to tilt**

Impact category	Tilt (mm/m)	Description
A	< 5	Unlikely that remedial work will be required.
B	5 to 7	Adjustment to roof drainage and wet area floors might be required.
C	7 to 10	Minor structural work might be required to rectify tilt. Adjustments to roof drainage and wet area floors will probably be required and remedial work to surface water drainage and sewerage systems might be necessary.
D	> 10	Considerable structural work might be required to rectify tilt. Jacking to level or rebuilding could be necessary in the worst cases. Remedial work to surface water drainage and sewerage systems might be necessary.

### B.3.2. Need for improvement to the previous method of impact classification

It is very difficult to design a method of impact classification that covers all possible scenarios and permutations. The application of any method is likely to find some instances that do not quite fit within the classification criteria.

Exposure to a large number of affected structures has allowed the mining industry to appreciate where improvements can be made to all aspects including the identification of areas for improvement in the previous method of impact classification.

A number of difficulties have been experienced with the previous method during the mining period. The difficulty centres on the use of crack width as the main classifying factor, as specified in Table C1 of Australian Standard 2870-1996.

A benefit of using crack width as the main factor is that it provides a clear objective measure by which to classify impact. However, experience has shown that crack width is a poor measure of the overall impact and extent of repair to a structure. The previous method of impact classification may be useful for assessing impact to newly built structures in a non-subsidence environment but further improvement and clarification is recommended before it can be effectively applied to houses impacted by mine subsidence.

The following aspects highlight areas where the previous classification system could be improved.-

- *Slippage on Damp Proof Course*

Many houses have experienced slippage along the damp proof course in Tahmoor. Slippage on some houses is relatively small (less than 10 mm) though substantial slippage has been observed in a number of cases, such as shown in Fig. B.1 below.



**Fig. B.1 Example of slippage on damp proof course**

Under the previous classification method, the “crack” width of the slippage may be very small (Category 1) but the distortion in the brickwork is substantial. Moreover, the extent of work required to repair the impact is substantial as it usually involves re-lining the whole external skin of the structure. Such impacts would be considered Category 4 based on extent of repair but only Category 1 or 2 based on maximum crack width.

There is no reference to slippage of damp proof course in the previous method of impact classification. However, if the extent of repair was used instead of using crack width as the main factor, the impact category would be properly classified as either Category 4 or Category 5.

It was recommended that slippage of damp proof courses be added to the previous impact classification table.

- *Cracks to brickwork*

In some cases, cracks are observed in mortar only. For example, movement joints in some structures have been improperly filled with mortar instead of a flexible sealant, as shown in Fig. B.2. In these situations, the measured crack width may be substantial but the impact is relatively simple to repair regardless of the crack width.



**Fig. B.2 Example of crack in mortar only**

In other cases, a small number of isolated bricks have been observed to crack or become loose. This is usually straightforward to repair. Under the previous impact classification method, a completely loose brick could be strictly classified as Category 5 as the crack width is infinitely large. This is clearly not the intention of the previous method but clarification is recommended to avoid confusion.

If a panel of brickwork is cracked, the method of repair is the same regardless of the width. While it is considered reasonable to classify large and severe cracks by its width, it is recommended that cracks less than 5 mm in width be treated the same rather than spread across Categories 0, 1 and 2.

If a brick lined structure contains many cracks of width less than 3 mm, the impact would be classified as no more than Category 2 under the previous method of impact classification. The extent of repair may be substantially more than a house that has experienced only one single 5 mm crack. However, it is recognised that it is very difficult to develop a simple method of classifying impacts based on multiple cracks in wall panels. How many cracks are needed to justify an increase in impact category?

- *Structures without masonry walls*

Timber framed structures with lightweight external linings such as weatherboard panels and fibro sheeting are not referenced in the previous classification table. If crack widths were strictly adopted to classify impacts, it may be possible to classify movement in external wall linings beyond Category 3 when in reality the repairs are usually minor.

It was recommended that the impact classification table be extended to include structures with other types of external linings.

- *Minor impacts such as door swings*

Experience has shown that one of the earliest signs of impact is the report of a sticking door. In some instances, the only observed impact is one or two sticking doors. It takes less than half an hour to repair a sticking door and impact is considered negligible.

Such an impact would be rightly classified as Category 0 based on the previous method of impact classification as there is no observed crack. However, the previous classification table suggests that sticking doors and windows occur when Category 2 crack widths develop. It was recommended that the impact classification table be amended in this respect.

### **B.3.3. Broad recommendations for improvement of previous method of impact classification**

It was recommended that crack width no longer be used as the main factor for classifying impacts. This does not mean that the use of crack width should be abandoned altogether. Crack width remains a good indicator of the severity of impacts and should be used to assist classification, particularly for impacts that are moderate or greater.

By focussing on crack width, the previous impact classification table appears to be classifying impacts from a structural stability perspective. It was recommended that a revised impact classification table be more closely aligned with all aspects of a building, including its finishes and services. Residents who are affected by impacts are concerned as much about impacts to internal linings, finishes and services as they are about cracks to their external walls and a revised impact classification method should reflect this.

With crack width no longer used as the main factor, it was recommended that the wording of the descriptions of impact in the classification table be extended to cover impacts to more elements of buildings. In keeping with the previous method of assessment, the level of impact should distinguish between cosmetic, serviceability and stability related impacts:-

- Low impact levels should relate to cosmetic impacts that do affect the structural integrity of the building and are relatively straight-forward to repair,
- Mid-level impact categories should relate to impacts to serviceability and minor structural issues, and
- High level impacts should be reserved for structural stability issues and impacts requiring extensive repairs.

### B.3.4. Revised method of impact classification

The following revised method of impact classification has been developed.

**Table B.4 Revised classification based on the extent of repairs**

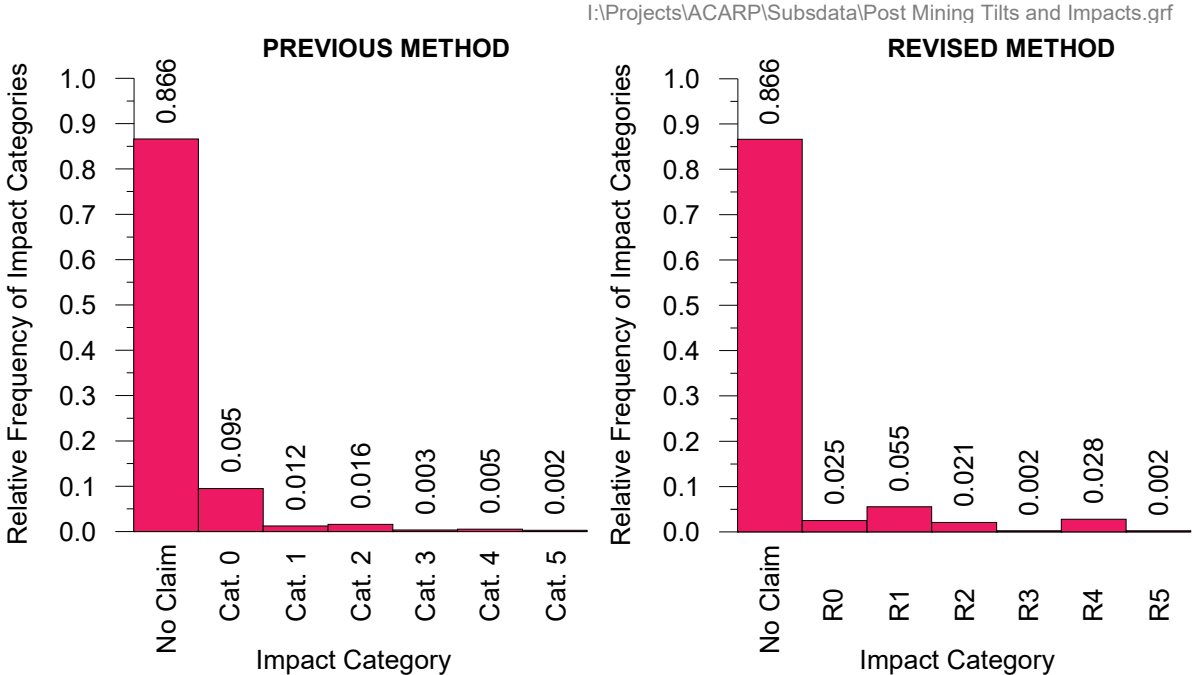
Repair Category	Extent of Repairs
<b>Nil</b>	No repairs required
<b>R0 Adjustment</b>	One or more of the following, where the damage does not require the removal or replacement of any external or internal claddings or linings:- <ul style="list-style-type: none"> <li>- <b>Door</b> or <b>window</b> jams or swings, or</li> <li>- Movement of <b>cornices</b>, or</li> <li>- Movement at external or internal <b>expansion joints</b>.</li> </ul>
<b>R1 Very Minor Repair</b>	One or more of the following, where the damage can be repaired by filling, patching or painting without the removal or replacement of any external or internal brickwork, claddings or linings:- <ul style="list-style-type: none"> <li>- Cracks in <b>brick mortar</b> only, or isolated cracked, broken, or loose <b>bricks</b> in the external façade, or</li> <li>- Cracks or movement &lt; 5 mm in width in any external or internal wall <b>claddings, linings, or finish</b>, or</li> <li>- Isolated cracked, loose, or drummy floor or wall <b>tiles</b>, or</li> <li>- Minor repairs to any <b>services or gutters</b>.</li> </ul>
<b>R2 Minor Repair</b>	One or more of the following, where the damage affects a small proportion of external or internal claddings or linings, but does not affect the integrity of external brickwork or structural elements:- <ul style="list-style-type: none"> <li>- Continuous cracking in <b>bricks</b> &lt; 5 mm in width in one or more locations in the total external façade, or</li> <li>- Slippage along the <b>damp proof course</b> of 2 to 5 mm anywhere in the total external façade, or</li> <li>- Cracks or movement ≥ 5 mm in width in any external or internal wall <b>claddings, linings, finish</b>, or</li> <li>- Several cracked, loose or drummy floor or wall <b>tiles</b>, or</li> <li>- Replacement of any <b>services</b>.</li> </ul>
<b>R3 Substantial Repair</b>	One or more of the following, where the damage requires the removal or replacement of a large proportion of external brickwork, or affects the stability of isolated structural elements:- <ul style="list-style-type: none"> <li>- Continuous cracking in <b>bricks</b> of 5 to 15 mm in width in one or more locations in the total external façade, or</li> <li>- Slippage along the <b>damp proof course</b> of 5 to 15 mm anywhere in the total external façade, or</li> <li>- Loss of bearing to isolated walls, piers, columns, or other load-bearing elements, or</li> <li>- Loss of stability of isolated structural elements.</li> </ul>
<b>R4 Extensive Repair</b>	One or more of the following, where the damage requires the removal or replacement of a large proportion of external brickwork, or the replacement or repair of several structural elements:- <ul style="list-style-type: none"> <li>- Continuous cracking in <b>bricks</b> &gt; 15 mm in width in one or more locations in the total external façade, or</li> <li>- Slippage along the damp proof course of 15 mm or greater anywhere in the total external façade, or</li> <li>- Releveling of building, or</li> <li>- Loss of stability of several structural elements.</li> </ul>
<b>R5 Re-build</b>	Extensive damage to house where the MSB (now SA NSW) and the owner have agreed to rebuild as the cost of repair is greater than the cost of replacement.

As discussed at the start of this chapter, it is very difficult to design a method of impact classification that covers all possible scenarios and permutations. While the method has been floated among some members of the mining industry, it is recommended that this table be reviewed broadly.

The recommended method has attempted to follow the current Australian Standard in terms of the number of impact categories and crack widths for Categories 3 and 4. The method is based on the extent of repairs required to repair the physical damage that has occurred, and does not include additional work that is occasionally required because replacement finishes cannot match existing damaged ones. It is therefore likely that the actual cost of repairs will vary greatly between houses depending on the nature of the existing level and type of finishes used.

The impacts experienced at Tahmoor Mine have been classified in accordance with the revised method of classification with good results. The method allowed clearer trends to be found when undertaking statistical analyses.

A comparison between the previous and revised methods is shown in Fig. B.3.



**Fig. B.3 Comparison between previous and revised methods of impact classification**

It can be seen that there was an increased proportion in the higher impact categories using the revised method. This is brought about mainly by the recorded slippage on damp proof courses, which are classified as either Category 3 or Category 4 when they were previously classified as Category 1 or 2.

There was also a noticeable reduction in proportion of Category 0 impacts and noticeable increase in proportion of Category 1 impacts using the revised method. This is because the revised method reserves Category 0 impacts for impacts that did not result in cracking any linings, while the previous method allows hairline cracking to occur.

The consistent low proportion of Category 3 impacts under both the previous and current methods raises questions as to whether this category should be merged with Category 4.

## **B.4. Method of impact assessment**

### **B.4.1. Need for improvement of the previous method**

The previous method of impact assessment provided specific quantitative predictions based on predicted conventional subsidence movements and general qualitative statements concerning the potential for impacts due to non-conventional movements. These non-conventional movements are additional to the predicted conventional movements.

This message was quite complex and created the potential for confusion and misunderstanding among members of the community who may easily focus on numbers and letters in a table that deal specifically with their house and misunderstand the message contained in the accompanying words of caution about the low level of reliability concerning predictions of conventional strain and potential for non-conventional movements.

This was unfortunately a necessary shortcoming of the previous method at the time as there was very little statistical information available to quantify the potential for impacts due to non-conventional movement. However, a great deal of statistical information was available following the mining of Tahmoor Mine Longwalls 22 to 24A at the time of the 2009 ACARP study and the method and message to the community could be improved. Additional statistical information was collected in 2016, which was approximately two years after the completion of Longwall 27 and one year after the completion of Longwall 28, which was the last panel to directly mine beneath the urban areas of Tahmoor. The timing of the data is such that it accounts for much of the time lag effect that occurs between the time of impact, when damage is claimed by residents and when the nature and level of the damage requiring repairs is assessed in detail by SA NSW.

While additional statistical information is now available, there remains limited knowledge at this point in time to accurately predict the locations of non-conventional movement. Substantial gains are still to be made in this area.

In the meantime, therefore, a probabilistic method of impact assessment has been developed. The method combines the potential for impacts from both conventional and non-conventional subsidence movement.

### **B.4.2. Factors that could be used to develop a probabilistic method of prediction**

Trend analyses have highlighted a number of factors that could be used to develop a probabilistic method. The trends examined were:-

- *Ground tilt*

This was found to be an ineffective parameter at Tahmoor Mine as ground tilts have been relatively benign and a low number of claims have been made solely in relation to tilt.

- *Ground strain*

There appears to be a clear link between ground strain and impacts, particularly compressive strain. The difficulty with adopting ground strain as a predictive factor lies in the ability to accurately predict ground strain at a point.

Another challenge with using strain to develop a probabilistic method is that there is limited information that links maximum observed strains with observed impacts at a structure. Horizontal strain is a two-dimensional parameter and it has been measured along survey lines that are oriented in one direction only.

The above issues are less problematic for curvature and the statistical analysis on the relationship between strain and curvature shows that the observed frequency of high strains increased with increasing observed curvature.

- *Ground curvature*

Curvature appears to be the most effective subsidence parameter to develop a probabilistic method. The trend analysis showed that the frequency of impacts increased with increasing observed curvature.

It should be noted that we are referring to conventional curvature and not curvatures that have developed as a result of non-conventional subsidence behaviour. This is because conventional curvature can be readily predicted with reasonable correlation with observations. It is also a relatively straight-forward exercise to estimate the observed smoothed or "conventional" mining-induced curvature that has previously been experienced at houses provided some ground monitoring is undertaken across and along extracted longwalls.

Non-conventional curvature cannot be predicted prior to mining and is accounted for by using a probabilistic method of impact assessment.

It has also been shown that the observed frequency of high strains increased with increasing observed curvature.

- *Position of structure relative to longwall*

A clear trend was understandably found that structures located directly above goaf were substantially more likely to experience impact. The calculated probabilities may be applicable for mining conditions that are similar to those experienced at Tahmoor Mine but will be less applicable for other mining conditions. An effective probabilistic method should create a link between the magnitude of differential subsidence movements and impact.

- *Construction type*

Two trends have been observed. Not surprisingly, structures constructed with lightweight flexible external linings are able to accommodate a far greater range of subsidence movements than brittle inflexible linings such as masonry. The analyses merely quantified what was already well known. The second observation was that houses constructed with strip footings were noticeably more likely to experience impacts than houses constructed with a ground slab, particularly in relation to higher levels of impact. This is because houses with strip footings are more susceptible to slippage along the damp proof course.

- *Structure size*

Trend analysis showed that larger structures attract a higher likelihood of impact. This is understandable as the chance of impacts increases with increasing footprint area. However, it is noted that the probability of severe impacts was not substantially greater for larger structures even though this would be expected if considering probabilities theoretically rather than empirically. It may be worthwhile including structure size as a factor in the development of a probabilistic method, though it is considered that it is a third order effect behind subsidence movements and construction type.

- *Structure age*

The trend analysis for structure age did not reveal any noticeable trends.

- *Extensions, variable foundations and building joints*

There is a clear trend of a higher frequency of impacts for structures that include extensions, variable foundations and building joints. The increased frequency appears to be related mainly to lower impact categories.

- *Urban or rural setting*

While trends were observed, it is considered that they can be explained by other factors. However, consideration can be made to provide a more conservative estimate of probabilities in rural areas if structure size has not been considered.

### **B.4.3. Revised method of impact assessment**

A revised method of impact assessment has been developed, based on information received in 2016 at a time when the extraction of Longwall 29 had been completed. The method is probabilistic and currently includes conventional ground curvature and construction type as input factors.

At the time of the original 2009 ACARP study, the trends in the data were difficult to determine within small ranges of curvature because of the relatively low number of buildings that reported damage at this time. A decision was therefore taken to analyse the data in a limited number of curvature ranges, so that where possible a reasonable sample size would be available in each range. The ranges of curvature originally chosen were 5 to 15 kilometres, 15 to 50 kilometres and greater than 50 kilometres.

Additional information provided in 2016 has demonstrated that the proportion of houses reporting impacts has increased. This has allowed statistical analyses to be conducted using narrower bands of observed curvatures though some inconsistencies remain in some bands due to the sample sizes. The ranges of curvature provided in this report are 2.5 to 15 kilometres, 15 to 50 kilometres and greater than 50 kilometres.

Because the incidence of damage for different construction types showed strong trends and because the sample size was reasonable for each type of structure, the data were analysed to determine the effect of radius of curvature on the incidence of damage for each of the three structure types and for each of the three curvature ranges.

The following probabilities are proposed in Table B.5.

**Table B.5 Probabilities of impact based on curvature and construction type based on the revised method of impact classification**

R (km)	Repair Category			
	No Repair or R0	R1 or R2	R3 or R4	R5
<b>Brick or brick-veneer houses with Slab on Ground</b>				
> 50	90 ~ 95 %	3 ~ 10 %	1 %	< 0.5 %
15 to 50	70 ~ 75 %	20 ~ 25 %	5 ~ 10 %	< 0.5 %
2.5 to 15	45 ~ 65 %	25 ~ 35 %	10 ~ 15 %	1 ~ 3 %
<b>Brick or brick-veneer houses with Strip Footing</b>				
> 50	85 ~ 90 %	5 ~ 15 %	1 ~ 3 %	< 2 %
15 to 50	60 ~ 75 %	20 ~ 30 %	5 ~ 15 %	1 ~ 3 %
2.5 to 15	45 ~ 65 %	25 ~ 30 %	5 ~ 15 %	5 ~ 10 %
<b>Timber-framed houses with flexible external linings of any foundation type</b>				
> 50	90 ~ 95 %	3 ~ 10 %	1 %	< 0.5 %
15 to 50	75 ~ 85 %	10 ~ 20 %	5 ~ 10 %	< 0.5 %
2.5 to 15	70 ~ 80 %	20 ~ 25 %	7 ~ 12 %	< 0.5 %

The results have been expressed as a range of values rather than a single number, recognising that the data had considerable scatter within each curvature range. While structure size and building extensions have not been included in the predictive tables, it is recommended to adopt percentages at the higher end of the range for larger structures or those with building extensions.

The percentages stated in each table are the percentages of building structures of that type that would be likely to be damaged to the level indicated within each curvature range. The levels of damage in the tables are indicated with reference to the repair categories described in the damage classification given in Table B.4.

The ranges provided in Table B.5 have been converted into a set of probability curves to remove artificial discontinuities that are formed by dividing curvatures into three categories. These are shown in Fig. B.4. The probability curves are applicable for all houses and civil structures.

At the time of writing ACARP Research Project C12015 (Waddington, 2009), the observed proportion of houses where the MSB (now SA NSW) and affected landowners had agreed to rebuild rather than repair (Category R5) impacts was less than 0.5 %. Since the publication of the research report, the proportion of houses where a decision has been made to rebuild has increased to approximately 1.1% overall and 3.2% above Longwalls 24A to 27 within the observed zone of increased subsidence. The decision to rebuild rather than repair a house is based on a variety of factors. Whilst acknowledging the significance of a decision to rebuild compared to repair a house, all houses previously impacted at Tahmoor Mine could have been repaired rather than replaced, including those where a decision has been made to rebuild them. This does not diminish the significance of this category from a social and economic impact point of view and it is important to continue recording the number of instances where a decision has been made to rebuild a house.

#### **B.4.4. Review of observed probabilities as mining continues**

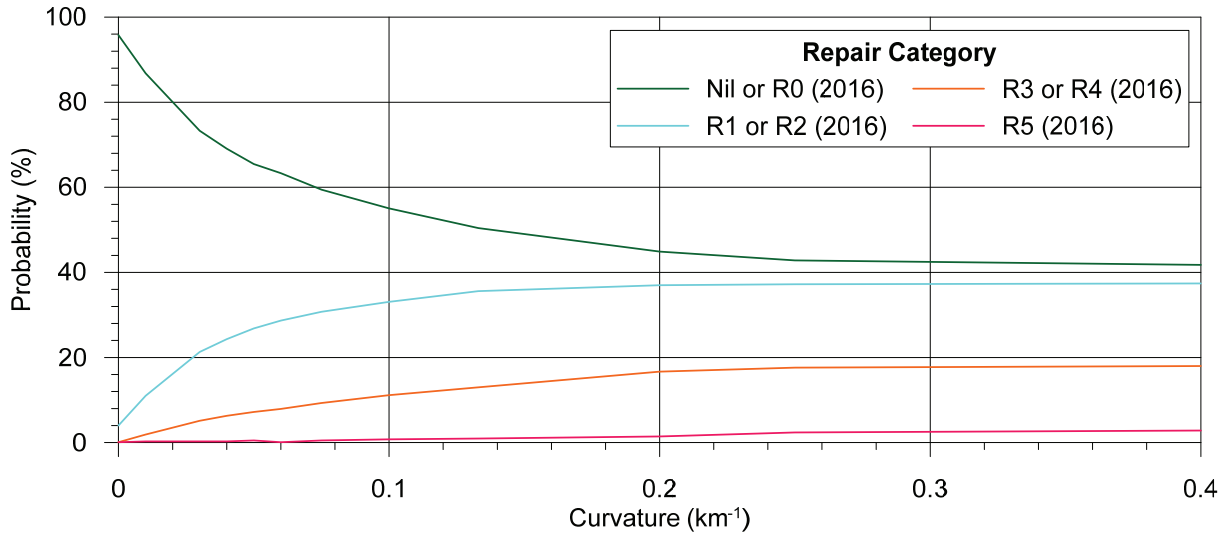
Reviews of observed probabilities are continually undertaken as Tahmoor Mine and other mines continue to extract beneath houses. The provision of additional information on impact on houses in 2016 has improved the level of understanding on the nature and frequency of impacts during the mining of Longwalls 22 to 29 compared to the information that was collected for the previous 2009 ACARP study, which was conducted after the mining of Longwalls 22 to 24A.

Additional statistical information was collected in 2016, which was approximately two years after the completion of Longwall 27 and one year after the completion of Longwall 28, which was the last panel to directly mine beneath the urban areas of Tahmoor.

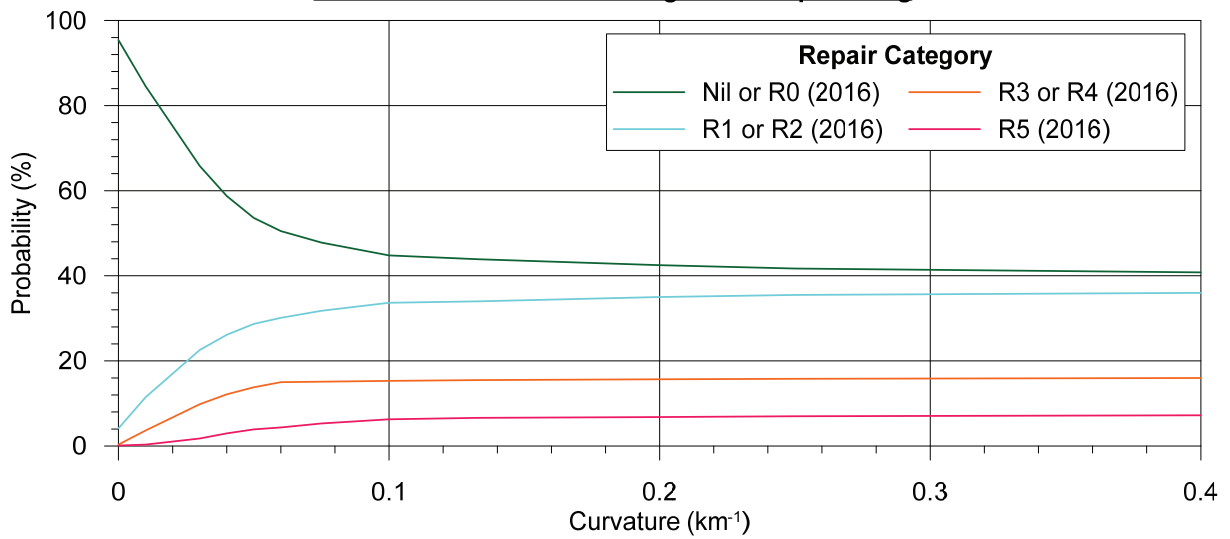
A finding from the additional information is that the proportion of houses that have experienced impacts has increased over time. The reasons for the increase are due to the time lag effect that occurs between the mining impact, when damage is claimed by residents and when the nature and level of the damage requiring repairs is assessed in detail by SA NSW.

In light of the above, it is recommended that the probabilities be revisited in the future.

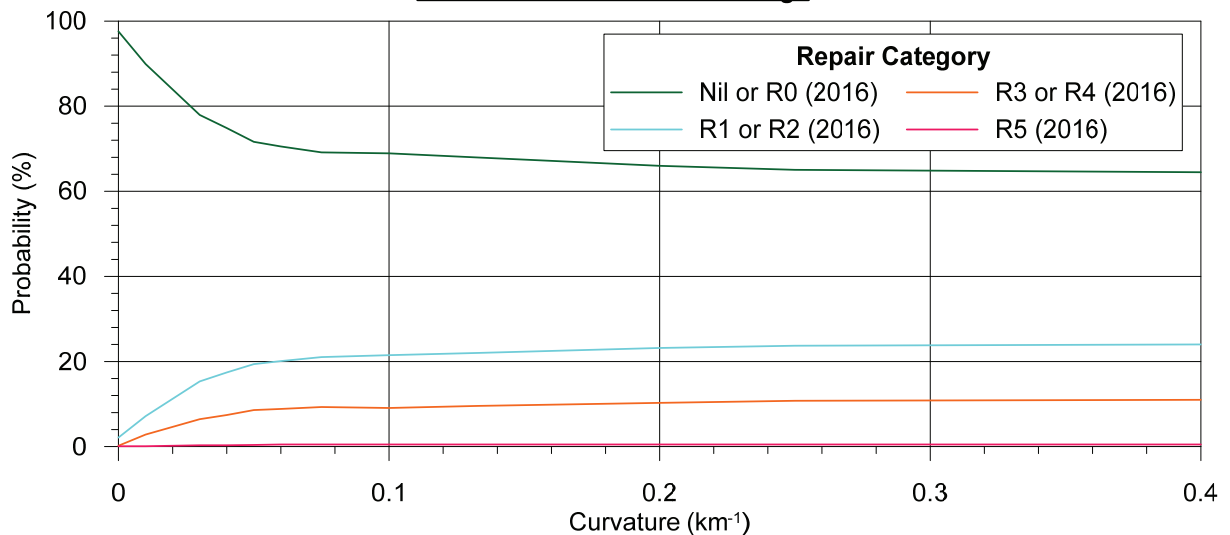
**Brick or Brick-Veneer Buildings with Slab on Ground**



**Brick or Brick-Veneer Buildings with Strip Footing**



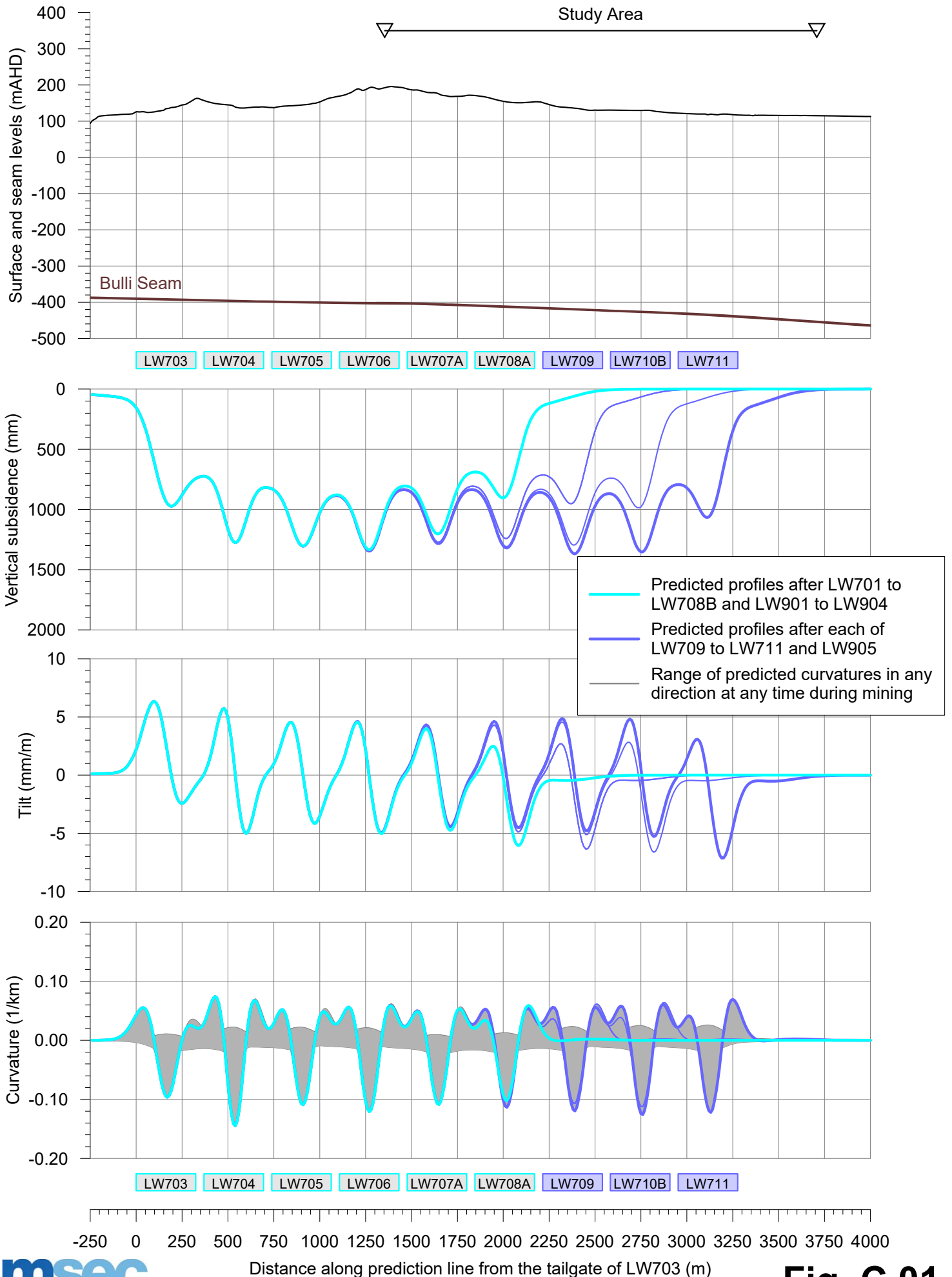
**Weatherboard or Fibro Buildings**



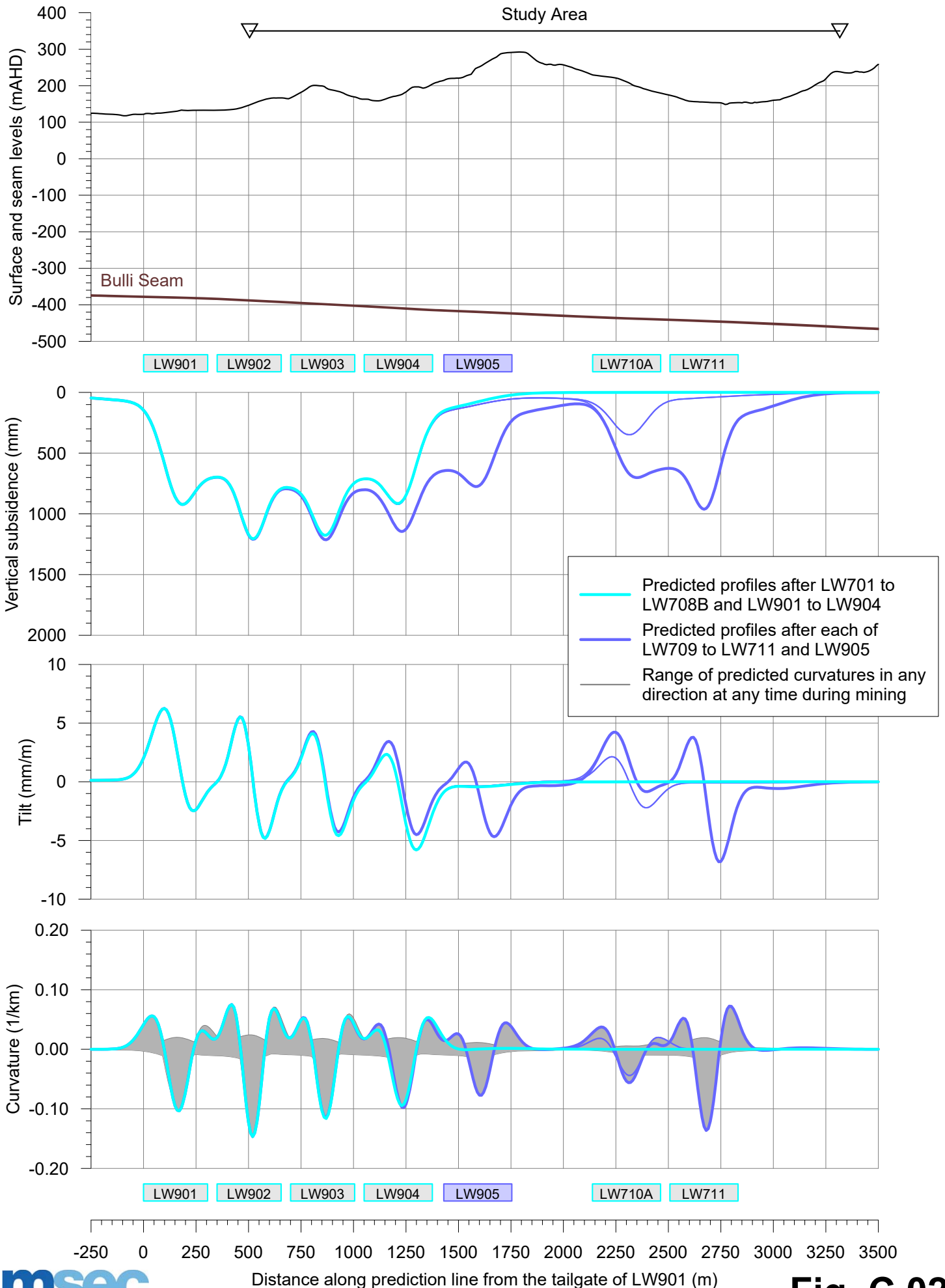
**Fig. B.4 Probability curves for impacts to buildings**

## **APPENDIX C. FIGURES**

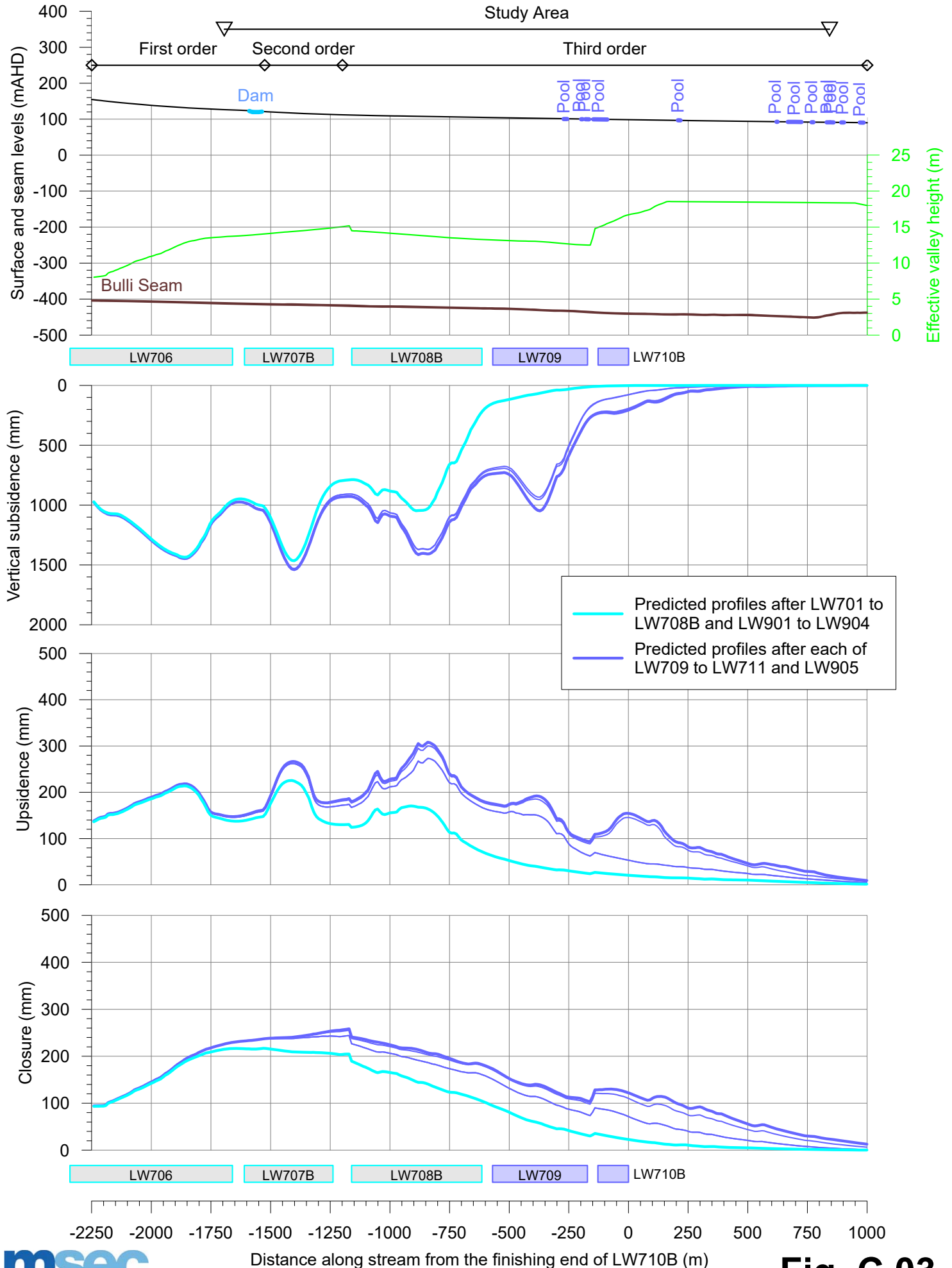
# Predicted profiles of vertical subsidence, tilt and curvature along Prediction Line 1 due to mining in Appin Areas 7 and 9



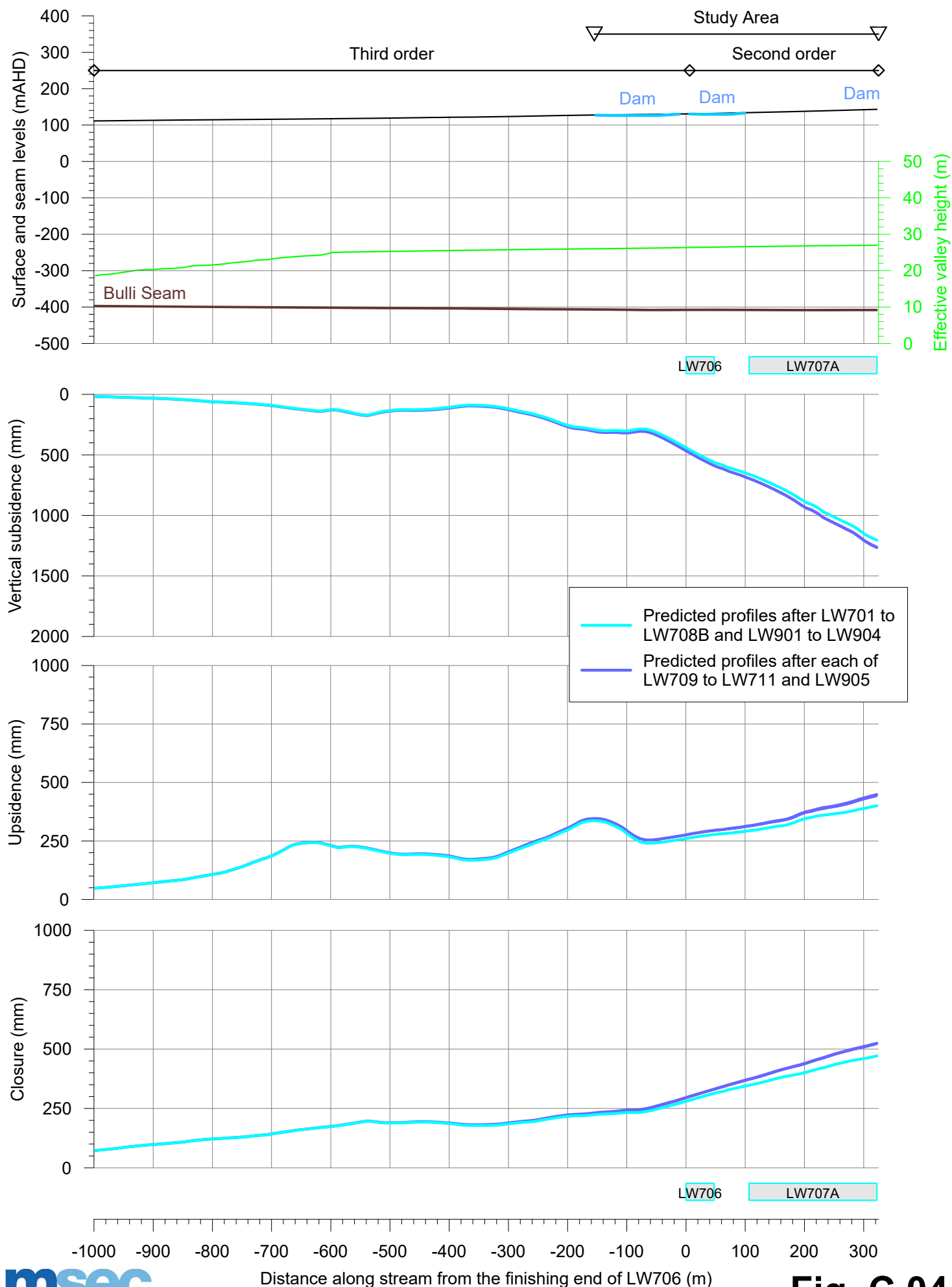
## Predicted profiles of vertical subsidence, tilt and curvature along Prediction Line 2 due to mining in Appin Areas 7 and 9



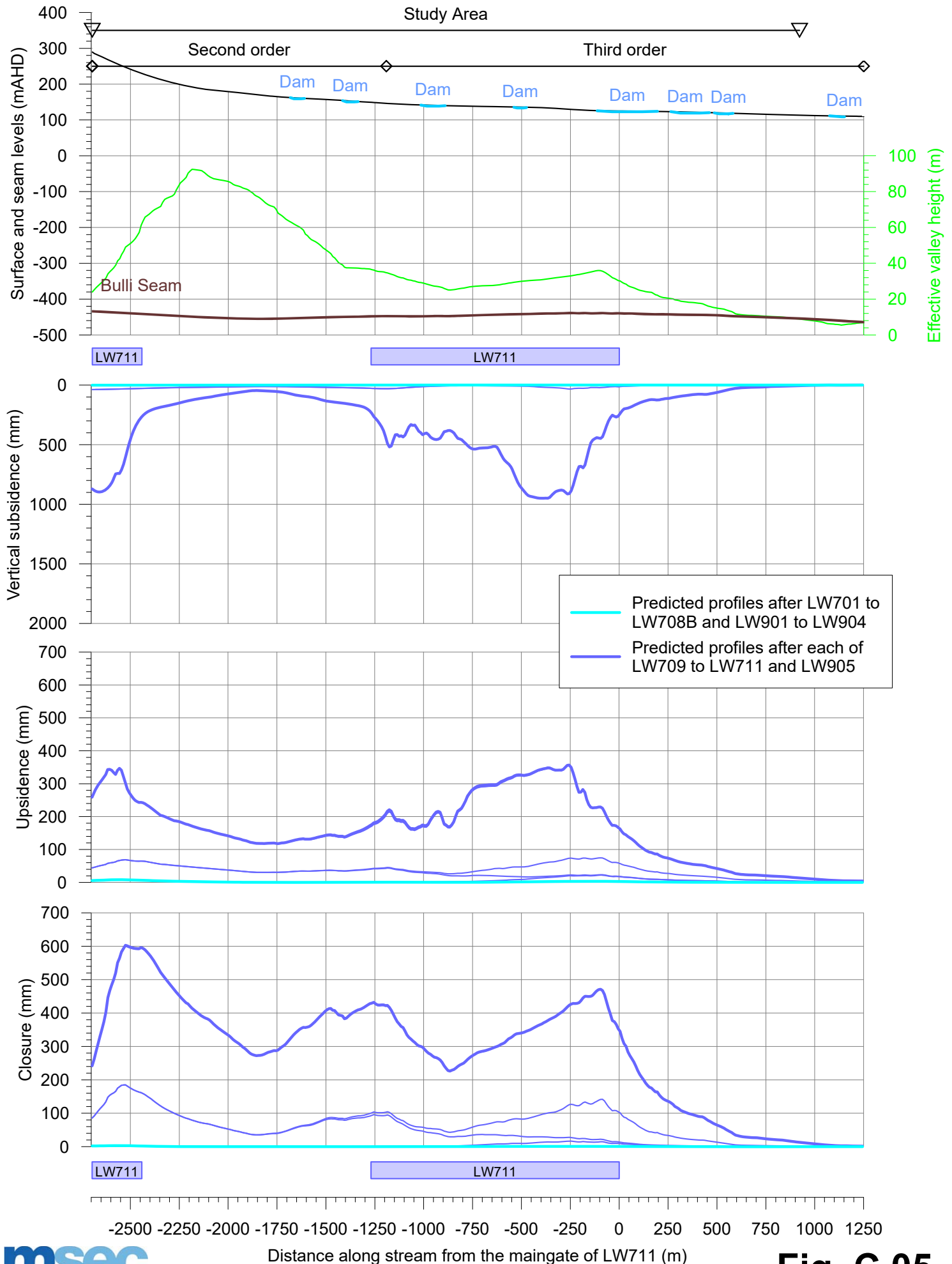
# Predicted profiles of vertical subsidence, upsidence and closure along Foot Onslow Creek due to mining in Appin Areas 7 and 9



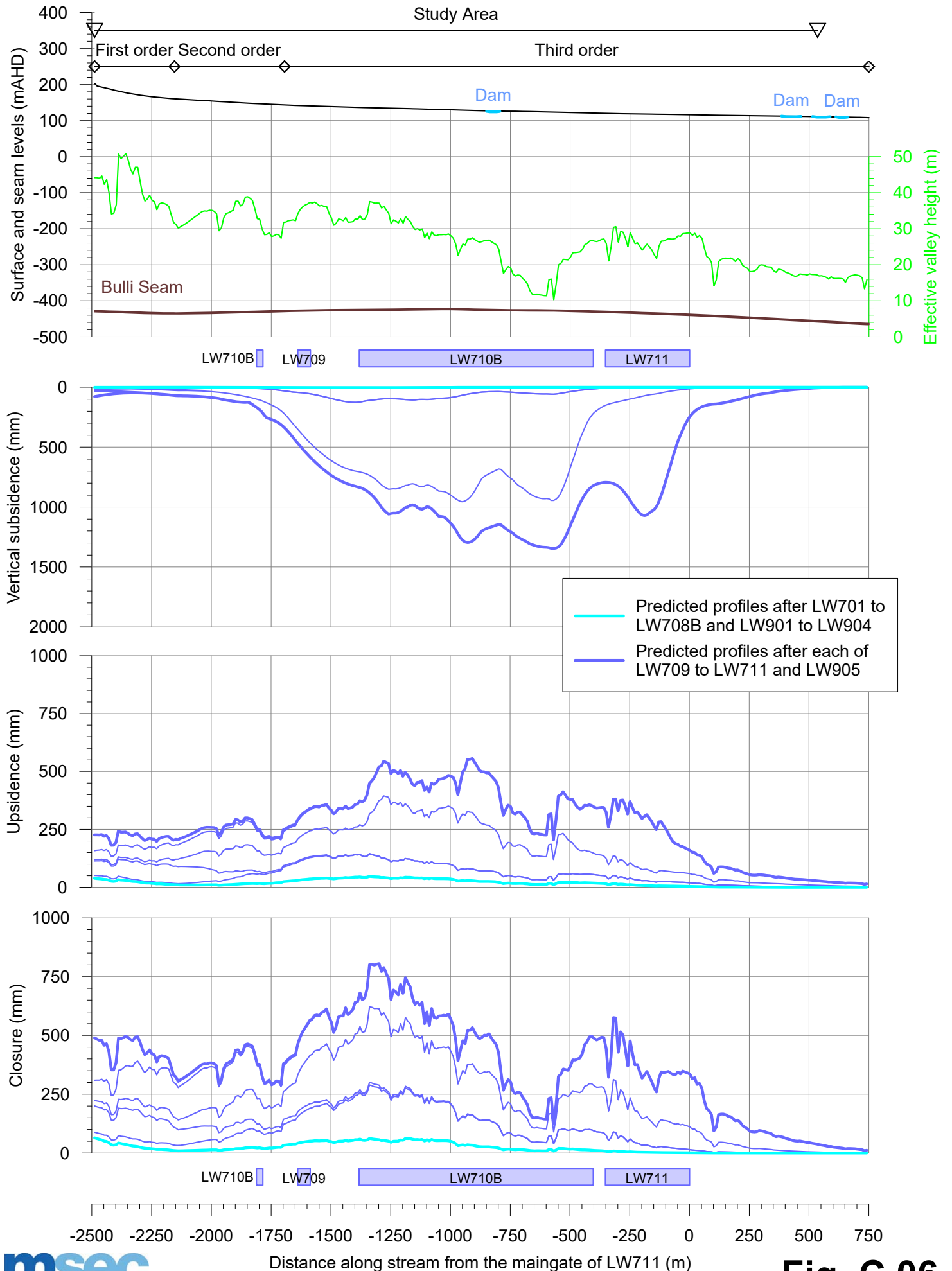
# Predicted profiles of vertical subsidence, upsidence and closure along Harris Creek due to mining in Appin Areas 7 and 9



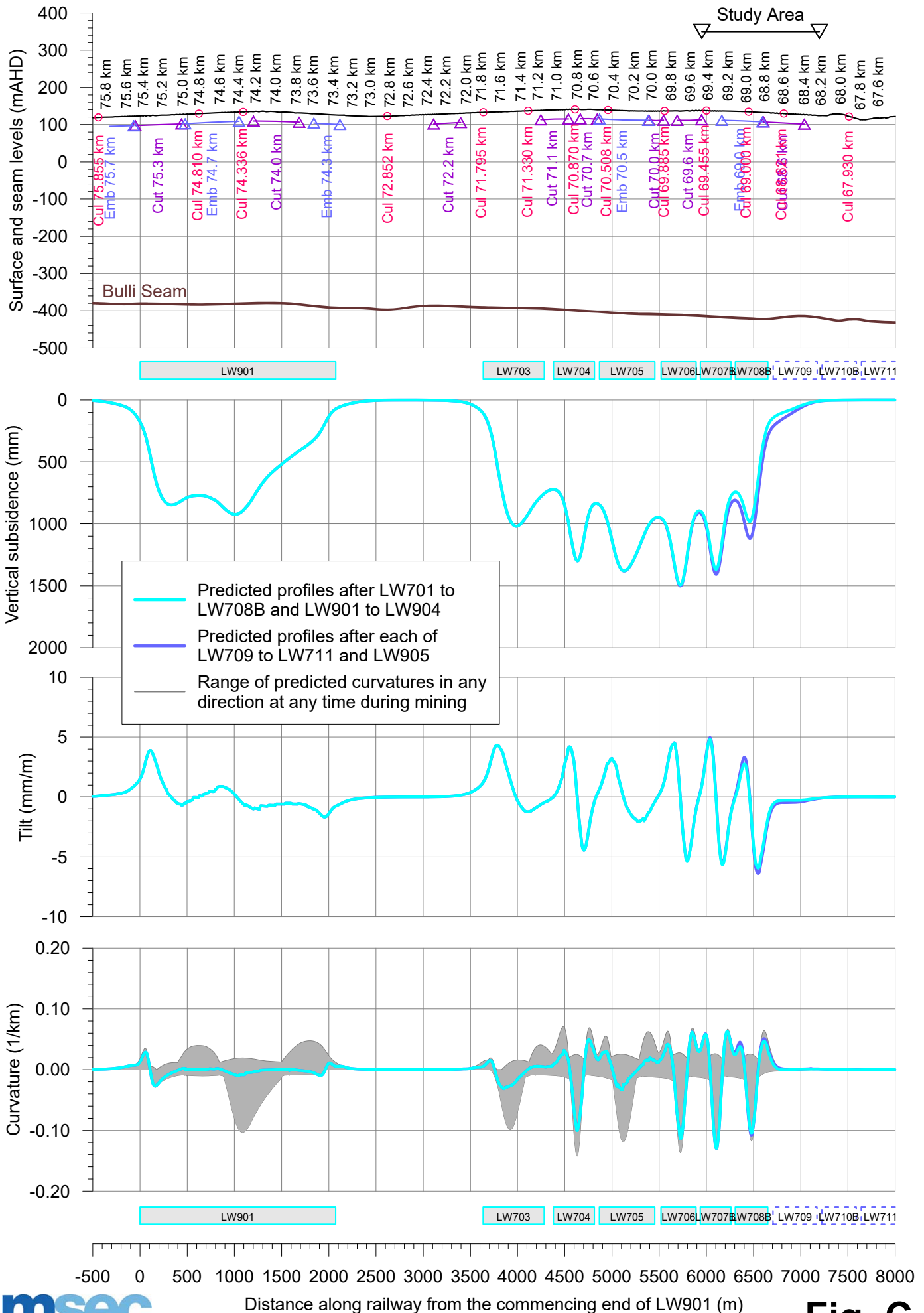
# Predicted profiles of vertical subsidence, upsidence and closure along Navigation Creek due to mining in Appin Areas 7 and 9



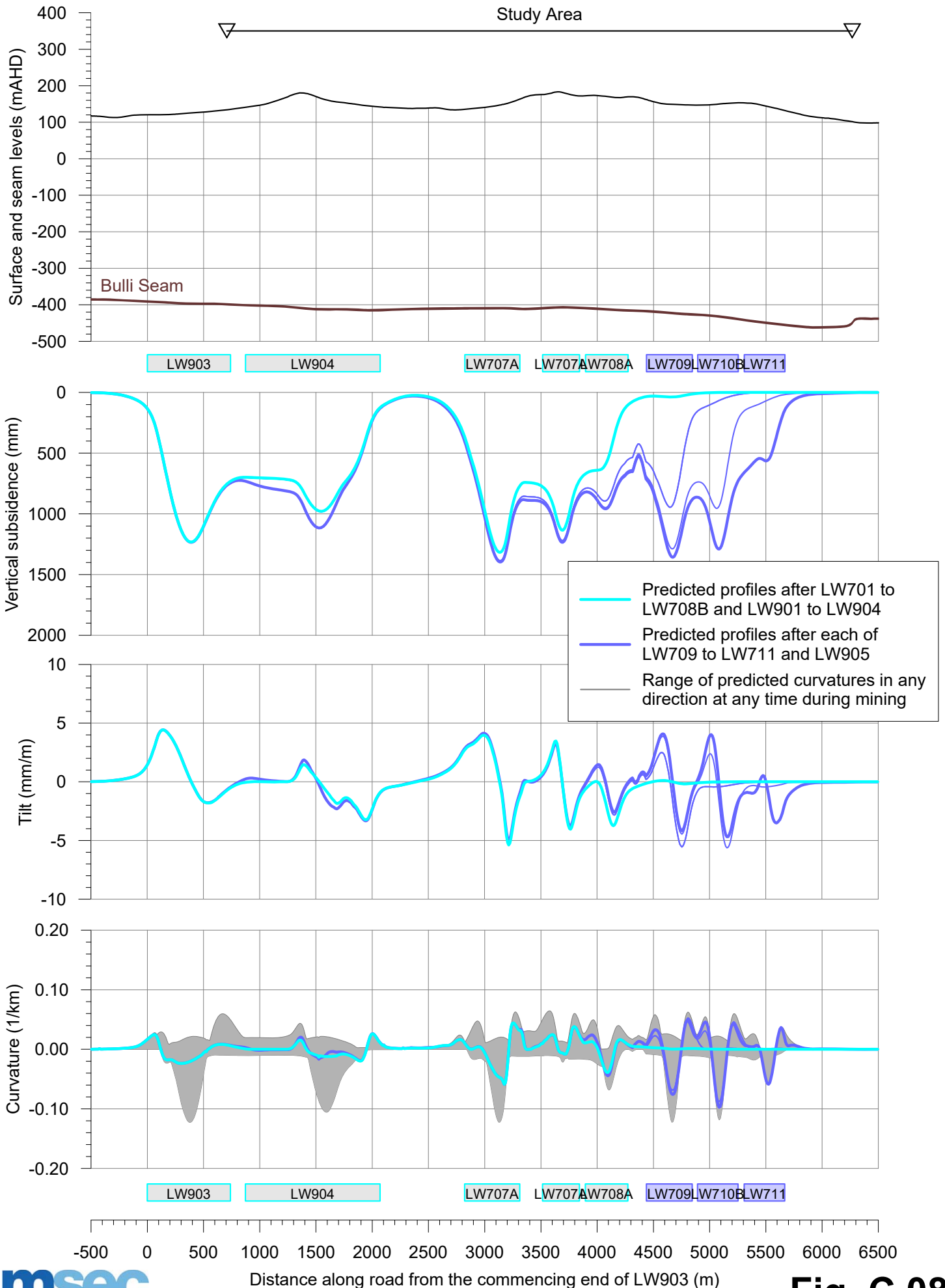
# Predicted profiles of vertical subsidence, upsidence and closure along Navigation Creek Tributary 1 due to mining in Appin Areas 7 and 9



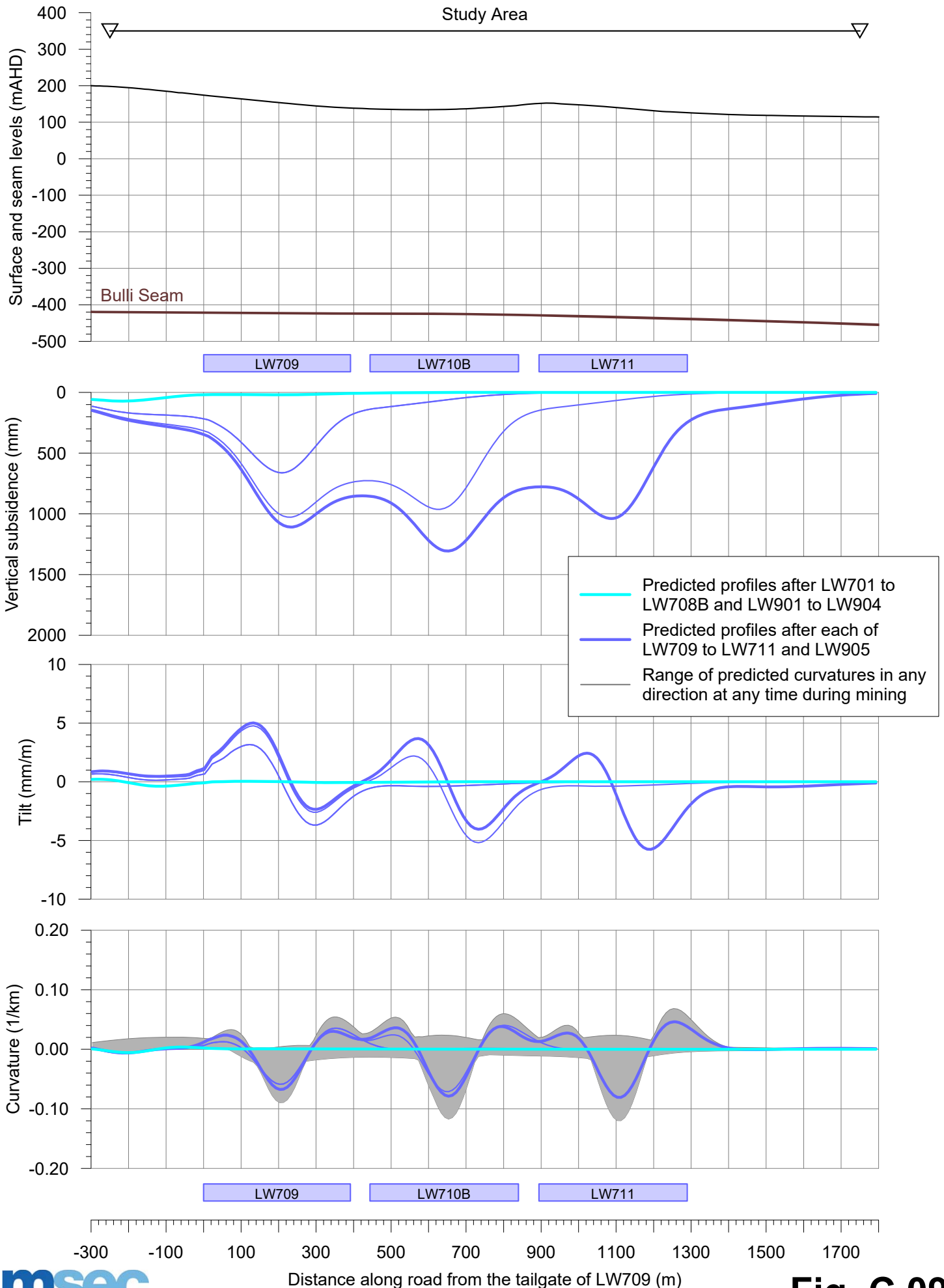
# Predicted profiles of vertical subsidence, tilt and curvature along the Main Southern Railway due to mining in Appin Areas 7 and 9



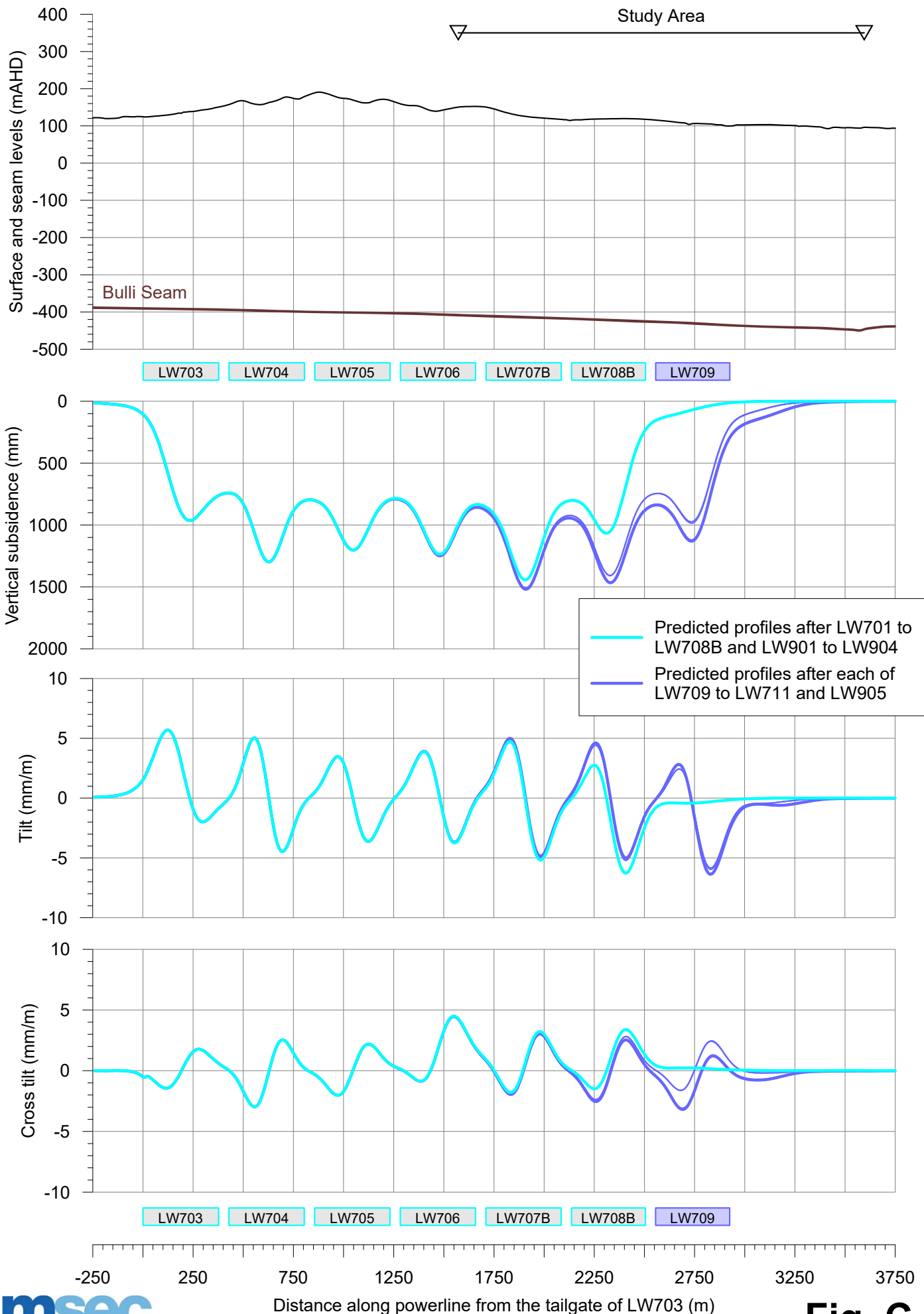
# Predicted profiles of vertical subsidence, tilt and curvature along Menangle Road due to mining in Appin Areas 7 and 9



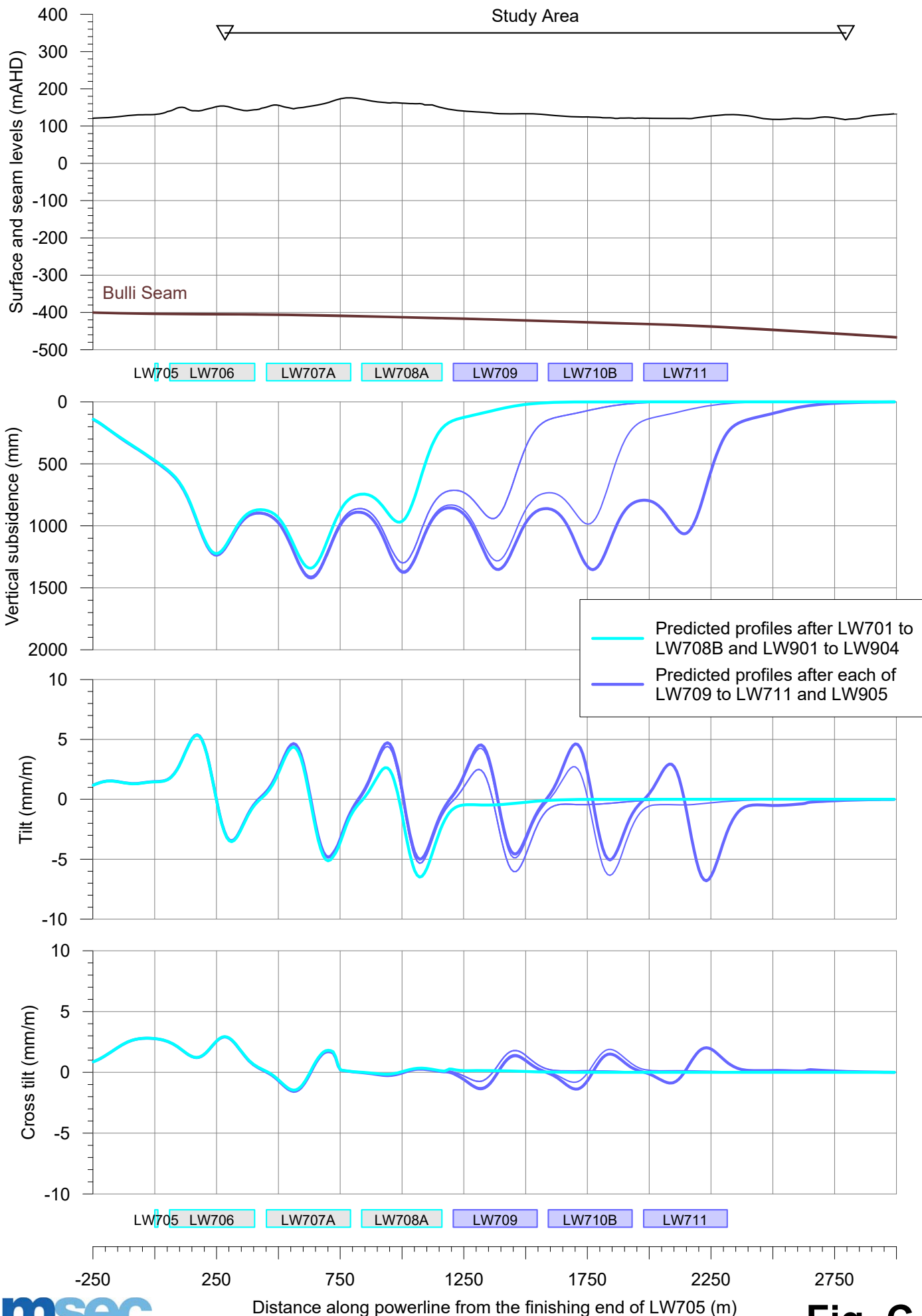
# Predicted profiles of vertical subsidence, tilt and curvature along Carrolls Road due to mining in Appin Areas 7 and 9



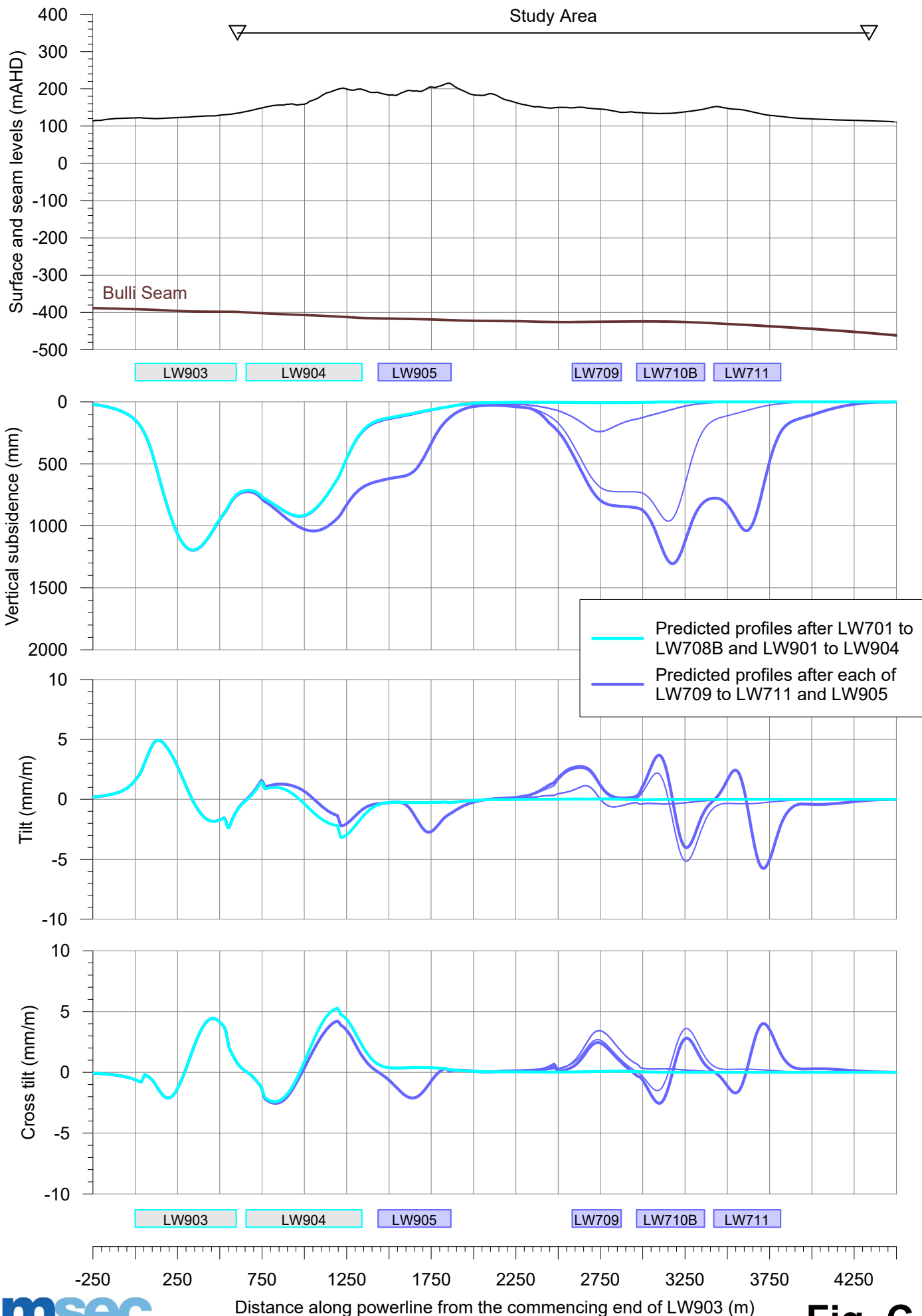
# Predicted profiles of vertical subsidence, tilt along and tilt across the 66 kV Powerline Branch 1 due to mining in Appin Areas 7 and 9



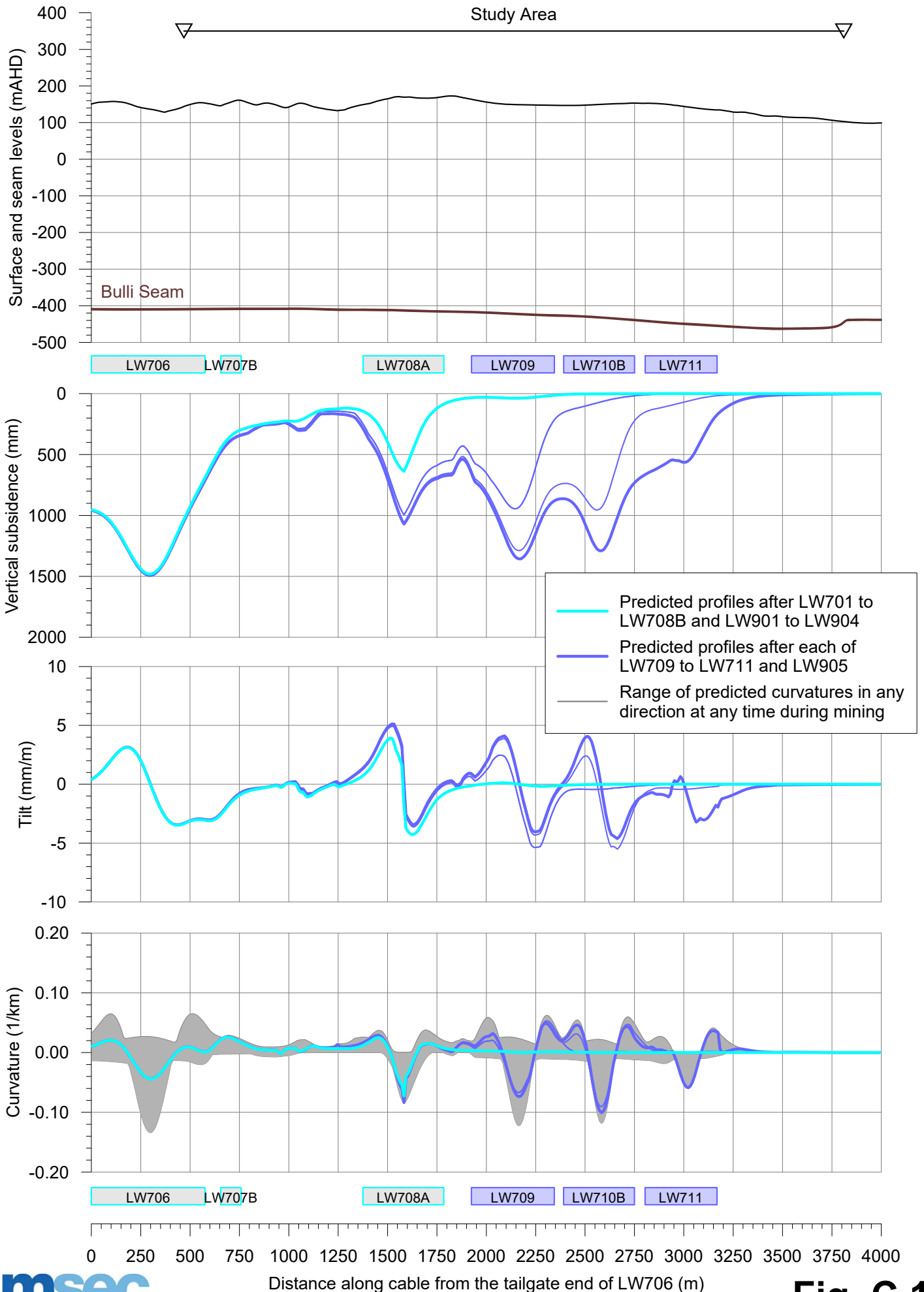
# Predicted profiles of vertical subsidence, tilt along and tilt across the 66 kV Powerline Branch 2 due to mining in Appin Areas 7 and 9



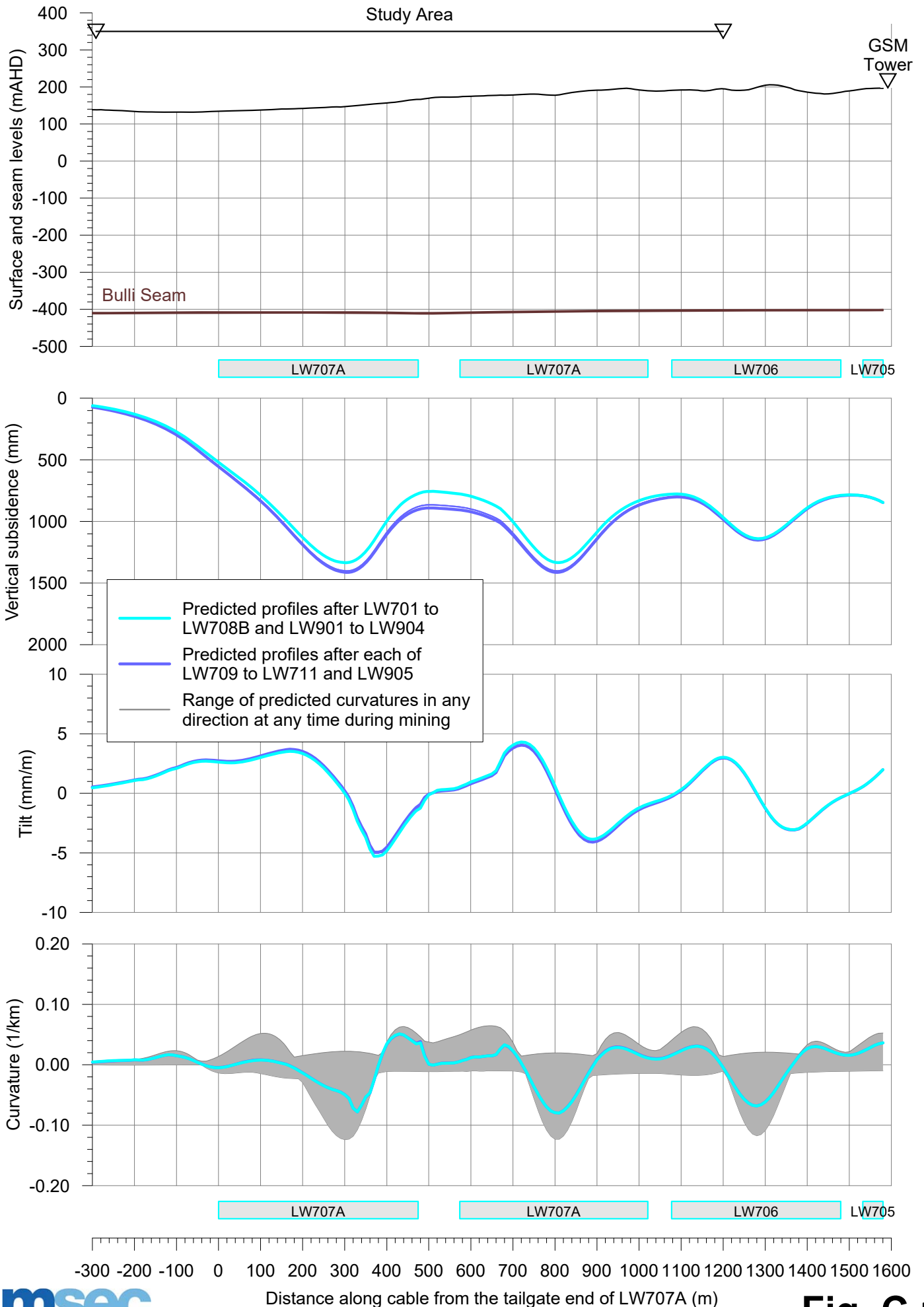
# Predicted profiles of vertical subsidence, tilt along and tilt across the 66 kV Powerline Branch 3 due to mining in Appin Areas 7 and 9



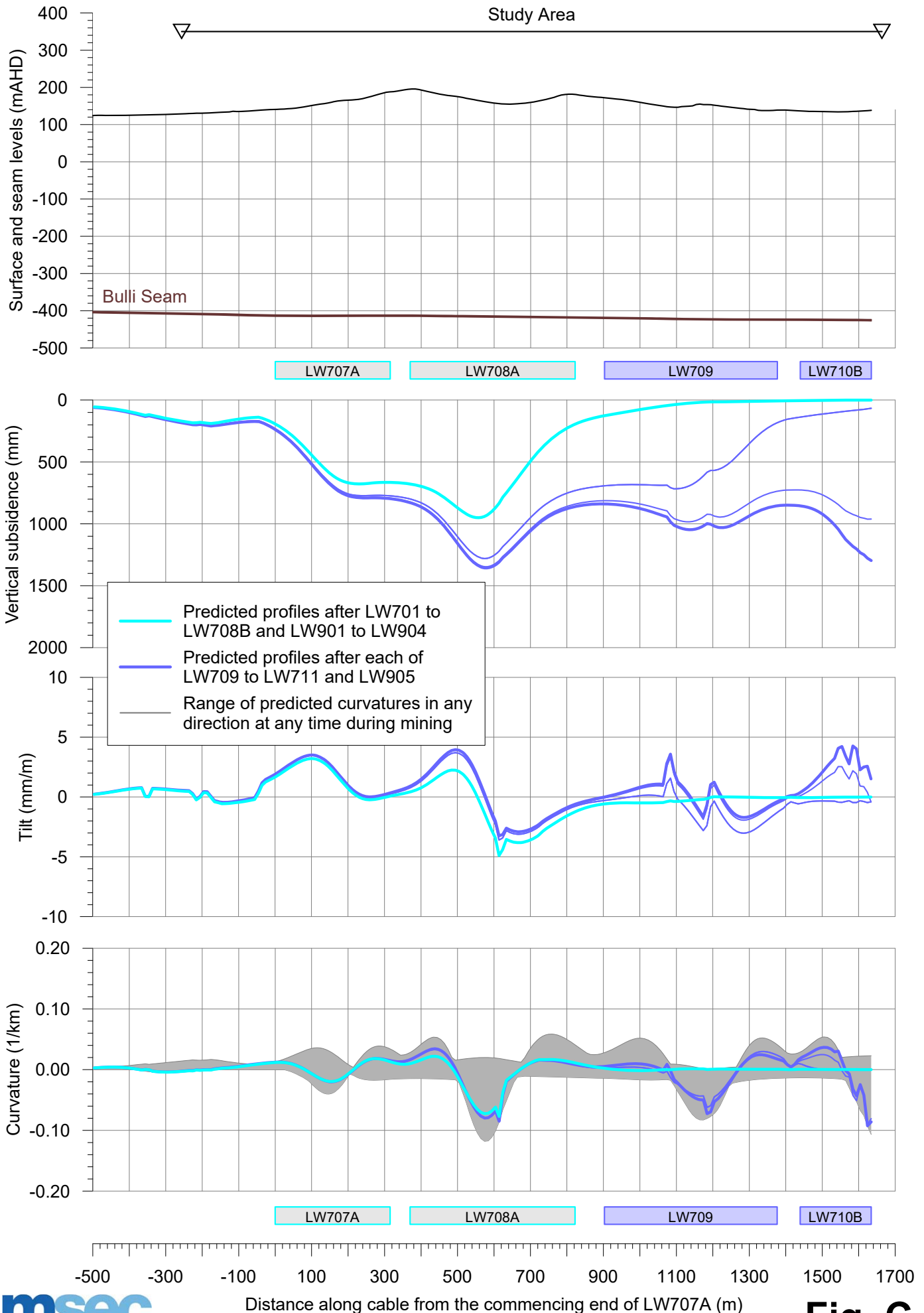
# Predicted profiles of vertical subsidence, tilt and curvature along the Telstra Optical Fibre Cable (2) due to mining in Appin Areas 7 and 9



# Predicted profiles of vertical subsidence, tilt and curvature along the Telstra Optical Fibre Cable (3) due to mining in Appin Areas 7 and 9



# Predicted profiles of vertical subsidence, tilt and curvature along the Telstra Optical Fibre Cable (4) due to mining in Appin Areas 7 and 9



## **APPENDIX D. TABLES**

# Table D.01 - Details of the structures within the Study Area

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m <sup>2</sup> )	Houses Steep Slopes		Houses Number of Storeys	Houses				House Wall Type			House Roof Type		House Footing Type			Other Structures																
							H1	H2		H3	H4	Single Storey with Length less than 30 metres	Single Storey with Length greater than 30 metres	Double Storey with Length less than 30 metres	Double Storey with Length greater than 30 metres	Brick or Brick Veneer	Weatherboard	Fibro	Tiled	Metal Sheet	Slab on Ground	Suspended on Piered Footings	Suspended on Strip Footings	Rural	Public Amenities	Public Utilities	Commercial											
A06a	291236	6218373	House	-	27	316			1	H1				1			1					1																
A06b	291202	6218403	Rural	Shed	14	117																																
A06c	291290	6218369	House	-	17	207			1																	1												
A06f	291348	6218340	Rural	Shed	7	32																																
A06g	291290	6218270	Rural	Shed	8	42																																
A06h	291264	6218303	Rural	Shed	5	14																																
A06i	291177	6218298	Rural	Shed	6	24																																
A06j	291172	6218302	Rural	Shed	4	9																																
A06k	291339	6218319	Rural	Shed	4	10																																
A06n	291343	6218396	Rural	Shed	5	9																																
A06p	291220	6218375	Rural	Shed	15	81																																
A06q	291258	6218391	Tank	-	8	48																																
A06r	291258	6218390	Tank	-	2	2																																
A06s	291270	6218388	Tank	-	2	2																																
A06t	291320	6218306	Tank	-	7	43																																
A06u	291320	6218306	House	-	26	357	On		1																													
A24h01	289802	6217107	House	-	26	357	On		1																													
A24p01	289803	6217120	Pool	-	9	43																																
A24r01	289793	6217128	Rural	Garage	11	83																																
A24r02	289804	6217082	Rural	Shed	11	61																																
A24r03	289485	6217061	Rural	Shed	10	47																																
A24r04	289496	6217040	Rural	Shed	5	23																																
A24r06	289487	6217045	Rural	Shed	9	65																																
A24t01	289484	6217033	Tank	-	5	22																																
A24t03	289805	6217087	Tank	-	2	2																																
C01a	289550	6217189	House	-	21	236	On		1																													
C01b	289565	6217189	Rural	Shed	10	84																																
C01p01	289562	6217171	Pool	-	10	42																																
C01t01	289540	6217188	Tank	-	4	13																																
C02a	289842	6217216	House	-	18	248	On		1																													
C02b	289858	6217206	Rural	Shed	12	81																																
C02c	289862	6217198	Rural	Shed	9	27																																
C02p01	289913	6217186	Rural	Shed	3	7																																
C02t01	289866	6217198	Pool	-	10	35																																
C02t02	289866	6217216	Tank	-	4	11																																
C02t03	289850	6217205	Tank	-	2	3																																
C03a	290043	6217100	House	-	30	314	On		1																													
C03b	290032	6217077	Rural	Shed	12	57																																
C03c	290050	6217166	Rural	Shed	6	12																																
C03d	290047	6217251	Rural	Shed	5	11																																
C03e	289955	6217254	Rural	Shed	4	10																																
C03f	289972	6217299	Rural	Shed	15	101																																
C03g	289980	6217310	Rural	Shed	4	12																																
C03h	289978	6217307	Rural	Shed	4	12																																
C03i01	290036	6217083	Rural	Carport	7	38																																
C03i02	290045	6217083	Tank	-	3	8																																
C03i03	289971	6217289	Tank	-	2	4																																

# Table D.01 - Details of the structures within the Study Area

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m <sup>2</sup> )	Houses Steep Slopes	Houses Number of Storeys	Houses				House Wall Type			House Roof Type		House Footing Type			Other Structures								
									H1	H2	H3	H4	Brick or Brick Veneer	Weatherboard	Fibro	Tiled	Metal Sheet	Slab on Ground	Suspended on Piered Footings	Suspended on Strip Footings	Rural	Public Amenities	Public Utilities	Commercial					
C04a	289817	6217521	House	-	29	424	Bottom	1	H1: 1	H2: 1	H3: 1	H4: 1	1	1	1	1	1	1	1	1					1				
C04b	289894	6217500	Rural	Shed	7	24																							1
C04c	289947	6217464	Rural	Shed	8	55																							1
C04p01	289834	6217521	Pool	-	9	29																							1
C04r01	289947	6217469	Rural	Shed	8	29																							1
C04t01	289812	6217498	Tank	-	5	20																							1
C08a	290056	6217828	House	-	21	263		1	H1: 1	H2: 1	H3: 1	H4: 1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	
C06b	290060	6217808	Rural	Garage	9	60																							1
C06c	290068	6217809	Rural	Shed	2	5																							1
C06d	290142	6217803	Rural	Shed	13	124																							1
C06e01	290044	6217813	Rural	Shed	4	14																							1
C06f01	290071	6217797	Tank	-	8	45																							1
C06g02	290151	6217798	Tank	-	2	4																							1
C09a	290052	6218266	House	-	37	412		2	H1: 1	H2: 1	H3: 1	H4: 1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	
C09p01	290075	6218278	Pool	-	15	76																							1
C09t01	290060	6218291	Tank	-	8	49		1	H1: 1	H2: 1	H3: 1	H4: 1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	
C10a	290060	6218377	House	-	19	239																							1
C10b	290068	6218351	Rural	Shed	7	50																							1
C10c	290045	6218333	Rural	Shed	11	97																							1
C10e	290062	6218394	Rural	Garage	6	38																							1
C10f	290067	6218387	Rural	Shed	2	24																							1
C10g	290068	6218391	Rural	Shed	2	40																							1
C10h	290075	6218413	Rural	Shed	7	17																							1
C10r01	290056	6218383	Rural	Pergola	10	53																							1
C10t01	290068	6218404	Tank	-	8	55																							1
C10r04	290081	6218336	Tank	-	2	3																							1
C10r05	290043	6218326	Tank	-	2	2																							1
C11a	290552	6218422	Rural	Shed	16	80																							1
C11b	290577	6218424	Rural	Shed	9	15																							1
C11c	290592	6218428	Rural	Shed	3	9																							1
C11d	290529	6218431	Rural	Shed	10	106																							1
C11f	290517	6218439	Rural	Shed	5	10																							1
C11g	290513	6218393	Rural	Shed	2	3																							1
C11r01	290572	6218423	Rural	Shed	5	13																							1
C11r02	290556	6218428	Rural	Shed	5	13																							1
C11t01	290547	6218418	Tank	-	2	4																							1
C11r02	290559	6218427	Tank	-	2	4																							1
C12f	290454	6219257	Rural	Shed	3	9																							1
C12g	290446	6219155	Rural	Shed	14	111																							1
C12r02	290454	6219151	Tank	-	3	7																							1
C20r01	289906	6219226	House	-	47	710	Bottom	2	H1: 1	H2: 1	H3: 1	H4: 1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	
C20p01	289919	6219225	Pool	-	12	67																							1
C20r01	289919	6219268	Rural	Shed	5	18																							1
C20r02	289921	6219244	Rural	Gazebo	9	62																							1
D03r01	289310	6219226	Rural	Shed	19	189																							1
D05a	289261	6219040	House	-	25	609		1	H1: 1	H2: 1	H3: 1	H4: 1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	

# Table D.01 - Details of the structures within the Study Area

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m <sup>2</sup> )	Houses Steep Slopes		Houses Number of Storeys					Houses				House Wall Type			House Roof Type		House Footing Type			Other Structures													
							Bottom	Top	H1	H2	H3	H4	Brick or Brick Veneer	Weatherboard	Fibro	Tiled	Metal Sheet	Slab on Ground	Suspended on Piered Footings	Suspended on Strip Footings	Rural	Public Amenities	Public Utilities	Commercial															
D05b	289275	6219063	Rural	Awning	21	69																							1										
D05c	289275	6219078	Rural	Garage	13	103																										1							
D05d	289267	6219024	Rural	Shed	6	29																										1							
D05e	289300	6219087	Rural	Shed	9	31																										1							
D05f	289296	6219102	Rural	Shed	15	163																										1							
D05g	289405	6218986	Rural	Shed	4	19																										1							
D05h	289284	6219112	Rural	Shed	8	37																										1							
D05p01	289271	6219034	Pool	-	10	47																											1						
D05f01	289297	6219085	Rural	Shed	3	10																												1					
D05i01	289304	6219063	Tank	-	7	40																													1				
D05i02	289307	6219100	Tank	-	3	7																														1			
D06a	289014	6219008	House	-	30	372																														1			
D06b	289014	6218968	Rural	Shed	21	386																														1			
D06d	289045	6218952	Rural	Shed	3	8																														1			
D06e	289097	6218939	Rural	Shed	6	15																														1			
D06f	289029	6219052	Rural	Shed	4	8																														1			
D06f01	289017	6218954	Rural	Shed	4	15																														1			
D06f02	289017	6218947	Rural	Shed	5	16																														1			
D06f01	289017	6219032	Tank	-	8	47																														1			
D06f02	289026	6218953	Tank	-	4	10																														1			
D07a	289447	6219130	House	-	27	479																														1			
D07b	289441	6219110	Rural	Awning	11	31																														1			
D07c	289418	6219132	Rural	Garage	11	78																														1			
D07d	289404	6219125	Rural	Shed	4	17																														1			
D07f	289488	6219165	Rural	Shed	4	15																														1			
D07g	289642	6219280	Rural	Shed	4	16																														1			
D07p01	289445	6219114	Pool	-	9	55																															1		
D07f01	289424	6219100	Tennis court	-	33	563																															1		
D07f01	289449	6219157	Tank	-	7	39																															1		
D07f02	289464	6219136	Tank	-	4	10																															1		
D08a	289787	6219120	House	-	25	510																															1		
D08b	289794	6219086	Rural	Shed	7	49																															1		
D08c	289714	6219055	Rural	Shed	8	39																															1		
D08p01	289771	6219105	Pool	-	8	26																															1		
D08f01	289809	6219097	Rural	Shed	6	26																															1		
D08f02	289773	6219143	Rural	Shed	7	24																															1		
D08f03	289738	6219130	Rural	Shed	3	11																															1		
D08f01	289785	6219095	Tank	-	8	46																															1		
D08f02	289777	6219091	Tank	-	7	35																															1		
D09a	289551	6218926	House	-	23	367																															1		
D09b	289511	6218886	Rural	Shed	5	19																															1		
D09c	289507	6218880	Rural	Shed	6	20																															1		
D09e	289786	6218874	Rural	Shed	3	11																															1		
D09p01	289577	6218944	Pool	-	7	25																															1		
D10a	289853	6218961	House	-	41	517																															1		
D10c	289897	6218918	Rural	Garage	10	67																															1		

# Table D.01 - Details of the structures within the Study Area

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m <sup>2</sup> )	Houses Steep Slopes	Houses Number of Storeys	Houses				House Wall Type			House Roof Type		House Footing Type			Other Structures															
									H1	H2	H3	H4	Brick or Brick Veneer	Weatherboard	Fibro	Tiled	Metal Sheet	Slab on Ground	Suspended on Piered Footings	Suspended on Strip Footings	Rural	Public Amenities	Public Utilities	Commercial												
D10d	289896	6218962	Rural	Shed	4	14																														
D10f	289884	6218985	Rural	Shed	4	12																														
D10p01	289833	6218952	Pool		11	52																														
D10r01	289925	6218916	Rural	Shed	19	125																														
D10r02	289862	6218980	Rural	Carport	5	16																														
D10r03	289845	6218959	Rural	Pergola	8	44																														
D10r04	289829	6218955	Rural	Pergola	5	17																														
D10s01	289901	6218913	Tank	-	3	8																														
D10s02	289901	6218913	Tank	-	2	2																														
D10s03	289931	6218912	Tank	-	3	8																														
D10s04	289935	6218914	Tank	-	3	8																														
D11a	289948	6218914	House	-	29	366																														
D11b	289960	6218831	Rural	Shed	13	91			Bottom																											
D11c	289909	6218808	Rural	Shed	6	20																														
D11d	289819	6218821	Rural	Shed	6	27																														
D11p01	289942	6218831	Pool		9	36																														
D11r01	289935	6218803	Rural	Shed	3	8																														
D11r02	289922	6218811	Tank	-	6	26																														
D11s02	289957	6218841	Tank	-	2	5																														
D12a	289941	6218792	House	-	28	325																														
D12b	289891	6218788	House	-	31	260																														
D12c	289950	6218763	Rural	Shed	4	13																														
D12d	289812	6218732	Rural	Shed	4	13																														
D12e	289950	6218711	Rural	Shed	10	46																														
D12f	289857	6218809	Rural	Shed	5	24																														
D12r01	289804	6218722	Rural	Shed	6	18																														
D12r02	289924	6218775	Tank	-	7	39																														
D12r03	289916	6218780	Tank	-	2	2																														
D12r04	289918	6218779	Tank	-	2	2																														
D13a	289870	6218798	Tank	-	3	8																														
D13b	289947	6218673	House	-	26	271																														
D13c	289892	6218664	Rural	Shed	16	142																														
D13d	289867	6218667	Rural	Shed	9	40																														
D13r01	289859	6218680	Rural	Shed	7	19																														
D13p01	289939	6218690	Pool		7	18																														
D13r02	289948	6218661	Tank	-	4	10																														
D14a	289556	6218708	House	371	23	371																														
D14b	289577	6218755	Rural	Shed	9	58																														
D14r01	289538	6218698	Rural	Shed	3	5																														
D14r02	289546	6218738	Tank	-	7	43																														
D15a	289350	6218750	House	-	51	794																														
D15b	289405	6218706	Rural	Shed	17	230																														
D15c	289384	6218770	Rural	Gazebo	7	42																														
D15d	289427	6218699	Rural	Shed	10	70																														
D15p01	289370	6218774	Pool		13	60																														
D15r01	289411	6218699	Tank	-	2	5																														

**Table D.01 - Details of the structures within the Study Area**

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m <sup>2</sup> )	Houses Steep Slopes	Houses Number of Storeys	Houses				House Wall Type			House Roof Type			House Footing Type			Other Structures																			
									H1	H2	H3	H4	Brick or Brick Veneer	Weatherboard	Fibro	Tiled	Metal Sheet	Slab on Ground	Suspended on Piered Footings	Suspended on Strip Footings	Rural	Public Amenities	Public Utilities	Commercial																	
D17a	289525	6218591	House	-	16	163		1	H1																																
D17b	289520	6218584	Rural	Carpport	5	21																			1																
D17c	289489	6218594	Rural	Shed	9	60																																			
D17p01	289515	6218609	Pool	-	9	32																																			
D1701	289514	6218582	Tank	-	5	17																																			
D1702	289490	6218587	Tank	-	3	7																																			
D18a	289904	6218449	House	-	31	558																																			
D18b	289918	6218423	Rural	Shed	10	101																																			
D18d	289839	6218448	Rural	Shed	3	9																																			
D18e	289781	6218488	Rural	Shed	3	7																																			
D18f	289887	6218473	Rural	Shed	4	14																																			
D18p01	289878	6218462	Pool	-	12	53																																			
D1801	289920	6218416	Rural	Shed	6	20																																			
D1802	289929	6218390	Rural	Shed	8	29																																			
D19a	289796	6218257	House	-	38	828	On	2																																	
D19d	289843	6218283	Rural	Shed	4	13																																			
D19p01	289775	6218259	Pool	-	7	20																																			
D1901	289766	6218249	Rural	Carpport	6	34																																			
D1902	289758	6218282	Rural	Shed	6	31																																			
D1901	289775	6218284	Tank	-	8	56																																			
D1903	289765	6218254	Tank	-	2	4																																			
D1904	289768	6218253	Tank	-	2	4																																			
D20b	289663	6218259	Rural	Shed	9	65																																			
D20h01	289684	6218351	House	-	21	110	On	1																																	
D2001	289656	6218259	Tank	-	3	8																																			
D2002	289657	6218255	Tank	-	3	8																																			
D21a	289481	6218314	House	-	17	229																																			
D21b	289513	6218282	Rural	Shed	5	26																																			
D21c	289465	6218409	House	-	48	704	Top	1																																	
D21d	289451	6218391	Rural	Garage	2	116																																			
D2101	289474	6218312	Tank	-	2	5																																			
D22a	288944	6218555	House	-	34	387																																			
D22c	288980	6218569	Rural	Shed	12	75																																			
D22p01	288958	6218547	Pool	-	11	41																																			
D2201	288920	6218528	Rural	Garage	14	113																																			
D2202	288946	6218572	Rural	Carpport	7	21																																			
D2203	289079	6218471	Rural	Shed	4	10																																			
D2204	289118	6218553	Rural	Shed	3	10																																			
D2201	288994	6218567	Tank	-	8	48																																			
D2202	288965	6218573	Tank	-	2	2																																			
D2203	288965	6218571	Tank	-	2	2																																			
D23a	288932	6218432	House	-	24	492	Bottom	1																																	
D23b	288914	6218444	Rural	Garage	13	110																																			
D23d	288952	6218407	House	-	18	209	Bottom	1																																	
D23e	288958	6218429	Rural	Pergola	6	29																																			
D23f	288979	6218455	Rural	Shed	8	56																																			

Table D.01 - Details of the structures within the Study Area

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m2)	Houses Steep Slopes	Houses Number of Storeys	Houses				House Wall Type			House Roof Type		House Footing Type			Other Structures												
									Single Storey with Length less than 30 metres	Single Storey with Length greater than 30 metres	Double Storey with Length less than 30 metres	Double Storey with Length greater than 30 metres	H1	H2	H3	H4	Brick or Brick Veneer	Weatherboard	Fibro	Tiled	Metal Sheet	Slab on Ground	Suspended on Piered Footings	Suspended on Strip Footings	Rural	Public Amenities	Public Utilities	Commercial					
D23g	288983	6218401	Rural	Garage	11	65																1											
D23h	289121	6218369	Rural	Shed	21	252																	1										
D23p01	288957	6218419	Pool	-	12	39																											
D23r01	288972	6218413	Tennis court	-	33	510																											
D23r02	289071	6218426	Rural	Shed	6	21																											
D23r03	288950	6218483	Rural	Shed	4	11																											
D23r01	288949	6218436	Tank	-	5	16																											
D23r02	288961	6218436	Tank	-	2	4																											
D23r03	288979	6218433	Tank	-	8	48																											
D24a	288917	6218344	House	-	21	343		1															1										
D24b	288915	6218327	Rural	Garage	10	54																											
D24c	288954	6218332	Rural	Shed	3	7																											
D24d	288969	6218332	Rural	Shed	23	272																											
D24p01	288951	6218344	Pool	-	12	61																											
D24r01	288949	6218327	Rural	Gazebo	9	60																											
D24r02	288925	6218349	Rural	Pergola	11	83																											
D24r01	288998	6218316	Tank	-	8	43																											
D24r02	288982	6218313	Tank	-	2	3																											
D25a	288911	6218235	House	-	23	207	Bottom	1																									
D25b	288929	6218289	Rural	Shed	11	55																											
D25p01	288926	6218224	Rural	Gazebo	9	60	Top	1																									
D26a	289591	6218235	House	-	29	515																											
D26b	289632	6218182	Rural	Shed	12	89																											
D26p01	289594	6218248	Pool	-	9	54																											
D26r01	289639	6218177	Tank	-	3	9																											
D26r02	289567	6218218	Tank	-	2	3																											
D26r03	289566	6218220	Tank	-	2	3																											
D26r04	289566	6218231	Tank	-	8	56																											
D26r08	289537	6218258	Tank	-	3	8																											
D27a	289696	6218142	House	-	37	528		1																									
D27b	289699	6218176	Rural	Shed	10	89																											
D27p01	289678	6218141	Pool	-	11	34																											
D27r01	289676	6218127	Rural	Gazebo	9	38																											
D27r02	289716	6218168	Tank	-	6	27																											
D27r03	289697	6218183	Tank	-	2	4																											
D28a	289796	6218030	House	-	21	169		1																									
D28b	289784	6218011	Rural	Shed	8	44																											
D28c	289835	6217998	Rural	Shed	6	50																											
D28e	289639	6217908	Rural	Shed	8	29																											
D28r01	289789	6218036	Rural	Carport	7	24																											
D28r02	289784	6218006	Rural	Shed	6	24																											
D28r03	289778	6218009	Rural	Shed	11	61																											
D28r01	289791	6218021	Tank	-	2	5																											
D29a	289813	6217975	House	-	32	535	Bottom	2																									
D29b	289723	6217933	Rural	Shed	8	32																											
D29r01	289781	6217957	Tank	-	8	51																											



**Table D.01 - Details of the structures within the Study Area**

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m <sup>2</sup> )	Houses Steep Slopes	Houses Number of Storeys	Houses				House Wall Type			House Roof Type		House Footing Type			Other Structures																				
									H1	H2	H3	H4	Brick or Brick Veneer	Weatherboard	Fibro	Tiled	Metal Sheet	Slab on Ground	Suspended on Piled Footings	Suspended on Strip Footings	Rural	Public Amenities	Public Utilities	Commercial																	
D38d	289116	6217601	Rural	Shed	7	21																1																			
D38e	289114	6217590	Rural	Shed	3	8																																			
D38f	289121	6217589	Rural	Shed	4	9																																			
D38h	289157	6217735	Tennis court	-	32	501																																			
D38i	289155	6217735	Rural	Shed	4	14																																			
D38k	289164	6217581	Rural	Shed	2	25																																			
D39a	289040	6217776	Rural	Shed	5	23																																			
D39o1	289020	6217776	Rural	Shed	17	185																																			
D39o2	289023	6217783	Tank	-	2	3																																			
D39o2	289029	6217782	Tank	-	2	3																																			
D39o3	288998	6217768	Tank	-	2	3																																			
D39o4	289040	6217770	Tank	-	2	2																																			
D40a	288843	6217698	House	-	32	338	Bottom	1																																	
D40b	288865	6217717	Rural	Garage	9	60																																			
D40o1	288835	6217677	Rural	Garage	11	93																																			
D40o2	288871	6217718	Tank	-	2	3																																			
D40o3	288870	6217716	Tank	-	2	3																																			
D41a	288907	6217557	House	-	26	451	Bottom	1																																	
D41b	288931	6217530	Rural	Shed	14	214																																			
D41c	288936	6217516	Rural	Shed	8	223																																			
D41d	288911	6217442	Rural	Shed	4	13																																			
D41p01	288911	6217577	Pool	-	9	30																																			
D41o1	288941	6217555	Tank	-	9	60																																			
D42o1	289032	6217302	Rural	Shed	11	53																																			
D43a	289264	6217171	House	-	31	417	On	1																																	
D43o1	289311	6217243	Rural	Shed	12	84																																			
D44a	289247	6217192	Tank	-	8	46																																			
D44c	289425	6217390	Rural	Shed	12	77																																			
D44d	289407	6217440	Rural	Shed	6	23																																			
D44e	289393	6217392	Rural	Shed	5	26																																			
D44f	289412	6217424	Rural	Shed	8	22																																			
D44h	289415	6217426	Rural	Shed	2	3																																			
D44k	289453	6217416	Rural	Shed	3	5																																			
D44p01	289403	6217427	Pool	-	12	53																																			
D44o1	289418	6217416	Tank	-	7	36																																			
D44o2	289422	6217433	Tank	-	2	2																																			
D45a	289707	6217307	House	-	25	279	On	2																																	
D45o01	289696	6217303	Pool	-	8	26																																			
D45o1	289704	6217292	Tank	-	5	16																																			
D45o2	289699	6217291	Tank	-	3	6																																			
D46a	289610	6217242	House	-	30	386	On	1																																	
D46o1	289634	6217215	Rural	Shed	8	54																																			
D46o2	289627	6217218	Rural	Shed	7	39																																			
D46o1	289635	6217244	Tank	-	5	17																																			
D46o2	289588	6217242	Tank	-	2	3																																			



## Table D.01 - Details of the structures within the Study Area

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m <sup>2</sup> )	Houses Steep Slopes	Houses Number of Storeys	Houses				House Wall Type			House Roof Type			House Footing Type			Other Structures															
									H1	H2	H3	H4	Brick or Brick Veneer	Weatherboard	Fibro	Tiled	Metal Sheet	Slab on Ground	Suspended on Piled Footings	Suspended on Strip Footings	Rural	Public Amenities	Public Utilities	Commercial													
D5402	289009	6216660	Rural	Shed	11	43																		1													
D5403	288932	6216664	Rural	Shed	11	109																			1												
D5401	289035	6216646	Tank	-	8	56																															
D5404	288929	6216653	Tank	-	5	23																															
D55h01	289212	6216872	House	-	20	400	On	1	1							1																					
D5501	289197	6216892	Rural	Shed	7	33																															
D5501	289189	6216866	Tank	-	8	49																															
D56h01	289297	6218272	House	-	26	420	Bottom	1	1																												
D57h01	289277	6218265	Tank	-	8	46																															
D57h01	288872	6217759	House	-	15	125	On	1	1																												
E15c	289510	6216693	Rural	Shed	5	17																															
E15r01	289493	6216654	Rural	Shed	11	99																															
E16c	289411	6216679	Rural	Shed	6	36																															
E16e	289452	6216661	Rural	Shed	8	8																															
E16f	289418	6216671	Rural	Shed	4	11																															
E16h01	289432	6216656	House	-	37	388		1	1																												
E16p01	289444	6216665	Pool	-	10	34																															
E16o2	289409	6216638	Tank	-	2	5																															
E16o3	289410	6216641	Tank	-	2	5																															
E16o4	289406	6216679	Tank	-	3	9																															
E20h01	289247	6216553	House	-	34	403		1	1																												
F02h01	288696	6216577	House	-	22	274	Bottom	1	1																												
F02r01	288684	6216522	Rural	Shed	3	6																															
F02r02	288693	6216542	Rural	Shed	17	156																															
F02r02	288692	6216536	Tank	-	4	10																															
F03h01	288411	6216489	House	-	22	411	Bottom	1	1																												
F03r01	288422	6216506	Rural	Shed	9	62																															
F03r02	288448	6216513	Rural	Shed	14	138																															
F03r03	288411	6216515	Rural	Shed	18	69																															
F03r04	288388	6216468	Rural	Shed	6	10																															
F03r05	288326	6216478	Rural	Shed	5	28																															
F03r06	288477	6216428	Rural	Shed	3	5																															
F03r07	288459	6216504	Rural	Shed	6	22																															
F03r08	288488	6216512	Rural	Greenhouse	8	20																															
F03r09	288382	6216495	Rural	Shed	3	6																															
F03r10	288384	6216467	Rural	Shed	3	5																															
F03r11	288484	6216526	Rural	Shed	10	60																															
F03r01	288382	6216472	Tank	-	7	42																															
F03r04	288458	6216511	Tank	-	4	10																															
F03r05	288423	6216520	Tank	-	3	8																															
F04h01	288674	6216766	House	-	25	294	Top	1	1																												
F04r01	288686	6216777	Rural	Shed	12	77																															
F04r02	288671	6216823	Rural	Shed	3	8																															
F04r01	288667	6216739	Tank	-	8	50																															
F04t02	288660	6216747	Tank	-	2	5																															
F05h01	288563	6216817	House	-	37	562		1	1																												

**Table D.01 - Details of the structures within the Study Area**

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m <sup>2</sup> )	Houses Steep Slopes	Houses Number of Storeys	Houses				House Wall Type			House Roof Type			House Footing Type			Other Structures								
									H1 Single Storey with Length less than 30 metres	H2 Single Storey with Length greater than 30 metres	H3 Double Storey with Length less than 30 metres	H4 Double Storey with Length greater than 30 metres	Brick or Brick Veneer	Weatherboard	Fibro	Tiled	Metal Sheet	Slab on Ground	Suspended on Piered Footings	Suspended on Strip Footings	Rural	Public Amenities	Public Utilities	Commercial						
F0502	288544	6216801	Rural	Shed	13	86			H1																	1				
F0503	288463	6216771	Rural	Shed	18	130																								1
F0505	288529	6216799	Rural	Shed	4	13																								1
F0506	288527	6216805	Rural	Shed	3	7																								1
F0503	288545	6216792	Tank	-	3	8																								
F0504	288560	6216817	Tank	-	2	2																								
F0505	288565	6216785	Tank	-	3	8																								
F0506	288574	6216788	Tank	-	7	41																								
F0601	288722	6217100	House	-	22	275	Top	1	1						1															
F0601	288711	6217096	Pool	-	5	13																								
F0601	288660	6217197	Rural	Shed	11	81																								1
F0604	288719	6217090	Rural	Pergola	16	132																								1
F0605	288731	6217104	Rural	Carport	5	18																								1
F0606	288696	6217214	Rural	Shed	4	10																								1
F0701	288577	6217201	House	-	17	202		1	1						1															1
F0701	288600	6217184	Rural	Shed	11	78																								1
F0702	288575	6217190	Rural	Shed	3	32																								1
F0703	288561	6217249	Rural	Shed	14	43																								1
F0704	288574	6217217	Rural	Shed	2	5																								1
F0701	288573	6217173	Tank	-	3	7																								
F0801	288585	6217288	House	-	27	232		2							1															1
F0801	288560	6217312	Rural	Shed	5	28																								1
F08q	288560	6217255	Rural	Shed	6	23																								1
F0801	288568	6217255	Rural	Shed	6	23																								1
F0801	288577	6217255	Rural	Shed	13	84																								1
F0802	288595	6217253	Rural	Shed	5	15																								1
F0803	288560	6217258	Rural	Shed	9	34																								1
F0804	288552	6217278	Rural	Shed	15	103																								1
F0805	288534	6217294	Rural	Shed	6	19																								1
F0806	288525	6217303	Rural	Shed	4	9																								1
F0807	288542	6217311	Rural	Shed	6	22																								1
F0808	288557	6217310	Rural	Shed	10	25																								1
F0809	288544	6217318	Rural	Shed	3	8																								1
F0810	288603	6217288	Rural	Shed	5	12																								1
F0811	288634	6217274	Rural	Shed	6	19																								1
F0812	288592	6217287	Rural	Pergola	9	36																								1
F0801	288538	6217290	Tank	-	4	11																								
F0802	288560	6217253	Tank	-	3	6																								
F0803	288564	6217254	Tank	-	3	7																								
F09c	288649	6217386	Rural	Shed	6	120																								1
F09f	288656	6217390	Rural	Shed	6	33																								1
F09g	288594	6217432	Rural	Shed	11	84																								1
F09h	288616	6217433	Rural	Shed	4	10																								1
F09h01	288650	6217410	House	-	30	267	Bottom	1	1																					1
F0903	288631	6217413	Rural	Gazebo	8	38																								1
F0901	288590	6217427	Tank	-	2	3																								
F09tc	288633	6217378	Tennis court	-	31	483																								

## Table D.01 - Details of the structures within the Study Area

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m <sup>2</sup> )	Houses Steep Slopes	Houses Number of Storeys	Houses				House Wall Type			House Roof Type		House Footing Type			Other Structures									
									H1	H2	H3	H4	Brick or Brick Veneer	Weatherboard	Fibro	Tiled	Metal Sheet	Slab on Ground	Suspended on Piered Footings	Suspended on Strip Footings	Rural	Public Amenities	Public Utilities	Commercial						
F10a	288631	6217758	House	-	19	277	Bottom	1	1								1													
F10b	288646	6217716	Rural	Shed	13	70															1									1
F10c1	288640	6217720	Rural	Carport	9	18															1									1
F10c2	288612	6217748	Rural	Shed	3	10															1									1
F10c1	288640	6217733	Tank	-	8	46																								
F10c2	288636	6217716	Tank	-	2	2																								
F11a	288602	6217525	House	-	29	482		1	1							1														
F11b	288581	6217533	Rural	Shed	11	75																								
F11c	288626	6217558	Rural	Shed	5	20																								
F11d	288597	6217501	Rural	Shed	13	87																								
F11e	288575	6217539	Rural	Shed	4	15																								
F11f	288551	6217594	Rural	Shed	25	344																								
F11g	288546	6217643	Rural	Shed	7	29																								
F11p01	288618	6217553	Pool	Shed	11	48																								
F12h01	288465	6217584	House	-	22	315		1	1							1														
F12p01	288473	6217566	Pool	-	13	48																								
F12r01	288459	6217564	Rural	Garage	12	78																								
F12r02	288462	6217572	Rural	Awning	5	16																								
F12r03	288505	6217597	Rural	Shed	18	130																								
F12r04	288531	6217590	Rural	Shed	9	28																								
F12r05	288479	6217554	Rural	Shed	2	4																								
F12t01	288490	6217579	Tank	Shed	8	56																								
F12t02	288467	6217551	Tank	-	2	4																								
F12t03	288512	6217605	Tank	-	2	5																								
F13h01	288360	6217380	House	-	19	180		2		1																				
F13p01	288338	6217410	Rural	Garage	20	158																								
F13r02	288330	6217351	Rural	Shed	4	11																								
F13r01	288386	6217352	Tank	Shed	3	8																								
F13r02	288385	6217356	Tank	-	3	8																								
F13r03	288382	6217353	Tank	-	2	3																								
F13t01	288335	6217298	Tennis court	-	38	731																								
F14h01	288320	6217034	House	-	27	334		1	1							1														
F14p01	288326	6217011	Pool	Garage	8	29																								
F14p02	288321	6217010	Pool	-	2	3																								
F14r01	288301	6217049	Rural	Garage	9	76																								
F14r02	288300	6217097	Rural	Shed	20	170																								
F14t01	288308	6217107	Tank	Shed	2	5																								
F14t02	288310	6217104	Tank	-	2	5																								
F14t03	288289	6217084	Tank	-	2	5																								
F14t04	288285	6217075	Tank	-	2	5																								
F14t05	288317	6217020	Tank	-	8	48																								
F17c01	287779	6216833	Commercial	-	5	18																								
F17c02	287786	6216820	Commercial	-	3	6																								
F17h01	288119	6217170	House	-	7	18																								1
F17p01	288123	6217160	Pool	-	25	355		1	1																					1
F17r01	288109	6217154	Rural	Garage	10	95																								1

### Table D.01 - Details of the structures within the Study Area

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m <sup>2</sup> )	Houses Steep Slopes	Houses Number of Storeys	Houses				House Wall Type			House Roof Type		House Footing Type			Other Structures							
									Single Storey with Length less than 30 metres	Single Storey with Length greater than 30 metres	Double Storey with Length less than 30 metres	Double Storey with Length greater than 30 metres	Brick or Brick Veneer	Weatherboard	Fibro	Tiled	Metal Sheet	Slab on Ground	Suspended on Piered Footings	Suspended on Strip Footings	Rural	Public Amenities	Public Utilities	Commercial				
F1702	288140	6217173	Rural	Shed	10	80											1						1					
F1705	287987	6217027	Rural	Shed	6	14																						1
F1701	288126	6217149	Tank	-	7	42																						
F1702	288138	6217154	Tank	-	2	4																						
F1703	288105	6217147	Tank	-	3	7																						
F1801	287952	6217263	House	-	34	409	Bottom	1											1									
F1801	287976	6217266	Pool	-	9	38																						
F1801	287976	6217266	Pool	-	9	38																						
F1801	287952	6217263	House	-	34	409	Bottom	1																				
F1801	287976	6217266	Pool	-	9	38																						
F1801	287952	6217263	House	-	34	409	Bottom	1																				
F1801	287976	6217266	Pool	-	9	38																						
F1802	287953	6217295	Tank	-	7	43																						
F1803	287945	6217293	Tank	-	2	4																						
F1804	287842	6217282	Tank	-	2	3																						
F1805	287843	6217279	Tank	-	2	3																						
F1806	287843	6217277	Tank	-	2	3																						
F1901	287895	6217471	House	-	39	629	Bottom	1																				
F1901	287896	6217484	Rural	Carport	13	98																						1
F1902	287926	6217468	Rural	Shed	15	127																						1
F1901	287916	6217454	Tank	-	7	44																						
F1902	287922	6217452	Tank	-	4	12																						
F1903	287928	6217458	Tank	-	4	12																						
F2001	287762	6217695	House	-	19	318	On	1																				1
F2001	287773	6217667	Rural	Shed	14	108																						1
F2004	287673	6217777	Rural	Shed	10	71																						1
F2001	287780	6217706	Tank	-	8	51																						
F2002	287769	6217659	Tank	-	3	9																						
F2003	287767	6217662	Tank	-	3	9																						
F21a	288153	6217720	House	-	34	471	Top	1																				
F21b	288131	6217711	Rural	Shed	11	58																						1
F21f	287979	6217610	House	-	18	143	Bottom	1																				
F21p01	288178	6217726	Pool	-	9	41																						
F2102	288217	6217722	Tennis court	-	31	507																						
F2103	287953	6217611	Rural	Shed	4	14																						1
F22a	288132	6217800	House	-	30	238	Top	1																				1
F22b	288160	6217796	House	-	15	140	Top	1																				
F22c	288149	6217790	Rural	Shed	7	48																						1
F22d	288096	6217797	Rural	Shed	12	71																						1
F22e	288093	6217880	Rural	Shed	7	50																						1
F22p01	288134	6217815	Pool	-	9	34																						
F22r01	288146	6217799	Rural	Pergola	11	76																						1
F22r02	288101	6217791	Rural	Shed	3	7																						1
F22r03	288079	6217879	Rural	Shed	6	27																						1
F22t01	288090	6217793	Tank	-	2	2																						
F23a	288365	6217772	House	-	28	272	Top	1																				1
F23b	288365	6217798	Rural	Garage	11	62																						1
F23c	288408	6217852	Rural	Shed	17	182																						1
F23d	288417	6217852	Rural	Carport	8	37																						1
F23e	288403	6217859	Rural	Carport	8	37																						1



# Table D.01 - Details of the structures within the Study Area

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m2)	Houses Steep Slopes	Houses Number of Storeys	Houses				House Wall Type			House Roof Type			House Footing Type			Other Structures											
									H1	H2	H3	H4	Brick or Brick Veneer	Weatherboard	Fibro	Tiled	Metal Sheet	Slab on Ground	Suspended on Piered Footings	Suspended on Strip Footings	Rural	PA	PU	C									
F30b	288782	6218308	Rural	Shed	6	17																	1										
F30c01	288758	6218329	Tank	-	8	49																											
F31a	287790	6217789	House	-	23	202		2																									
F31b	287778	6217800	Rural	Pergola	11	87																											
F31c	287756	6217796	Rural	Shed	12	68																											
F31c01	287770	6217752	Rural	Shed	10	46																											
F31d01	287778	6217791	Tank	-	3	6																											
F32a	288649	6217973	House	-	15	154		2																									
F32b	288610	6217967	Rural	Shed	17	102																											
F32c	288579	6217947	Rural	Shed	11	116																											
F32p01	288635	6217981	Pool	-	11	39																											
F32r01	288637	6217978	Rural	Pergola	4	15																											
F32t01	288604	6217939	Tank	-	7	41																											
F33a	288413	6217647	House	-	18	270		2																									
F33b	288388	6217629	Rural	Cabana	6	30																											
F33c	288395	6217647	Rural	Shed	5	24																											
F33d	288453	6217676	Rural	Shed	14	81																											
F33p01	288392	6217636	Pool	-	7	28																											
F33r01	288383	6217633	Rural	Shed	2	3																											
F33t01	288438	6217661	Tank	-	7	36																											
F33t02	288399	6217621	Tank	-	2	4																											
F34a	288407	6218254	House	-	39	494		2																									
F34b	288370	6218191	Rural	Shed	20	273																											
F34r01	288408	6218228	Rural	Garage	10	77																											
F34r02	288362	6218241	Tennis court	-	34	572																											
F34t01	288370	6218264	Tank	-	9	58																											
F34t02	288362	6218199	Tank	-	4	10																											
F35a	288149	6218446	House	-	24	287		1																									
F35b	288138	6218450	Rural	Pergola	10	65																											
F35c	288140	6218460	Rural	Carport	6	41																											
F35p01	288144	6218429	Pool	-	10	42																											
F36b	288272	6218537	Rural	Garage	10	56																											
F36c	288228	6218551	Rural	Shed	13	97																											
F36d	288309	6218593	Rural	Gazabo	10	251																											
F36t01	288266	6218572	House	-	37	472		1																									
F36p01	288296	6218567	Pool	-	10	37																											
F37a	288622	6218515	House	-	21	780		1																									
F37b	288588	6218507	Rural	Shed	11	114																											
F37c	288592	6218519	Rural	Shed	11	77																											
F37t01	288606	6218555	Rural	Shed	11	114																											
F37r02	288580	6218546	Rural	Shed	5	17																											
F37t01	288638	6218518	Rural	Shed	5	17																											
F37t02	288648	6218540	Tank	-	8	52																											
F37t03	288587	6218516	Tank	-	2	3																											
F37t04	288587	6218516	Tank	-	3	7																											
F37t05	288594	6218513	Tank	-	3	7																											

Table D.01 - Details of the structures within the Study Area

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m <sup>2</sup> )	Houses Steep Slopes	Houses Number of Storeys	Houses				House Wall Type			House Roof Type			House Footing Type			Other Structures															
									Single Storey with Length less than 30 metres	Single Storey with Length greater than 30 metres	Double Storey with Length less than 30 metres	Double Storey with Length greater than 30 metres	H1	H2	H3	H4	Brick or Brick Veneer	Weatherboard	Fibro	Tiled	Metal Sheet	Slab on Ground	Suspended on Piered Footings	Suspended on Strip Footings	Rural	Public Amenities	Public Utilities	Commercial									
F37i06	288508	6218562	Tank	-	3	7																															
F37i07	288561	6218550	Tank	-	2	3																															
F39a	288826	6218516	House	-	15	188		1	1																												
F39b	288795	6218506	Rural	Shed	18	155																															
F39c	288799	6218497	Rural	Shed	7	45																															
F39d	288797	6218488	Rural	Shed	6	15																															
F39e	288775	6218521	Rural	Shed	14	33																															
F39i01	288782	6218522	Rural	Shed	4	13																															
F39i01	288777	6218517	Tank	-	3	7																															
F39i01	288307	6218578	Tank	-	7	40																															
F40a	288808	6218615	House	-	17	302	Bottom	2																													
F40b	288784	6218601	Rural	Shed	7	46																															
F40c	288829	6218601	Rural	Shed	12	109																															
F40d	288843	6218597	Rural	Shed	15	108																															
F40e	288856	6218590	Rural	Shed	3	7																															
F40i01	288791	6218614	Pool	-	12	39																															
F40i01	288799	6218624	Rural	Pergola	11	67																															
F40i01	288770	6218606	Tank	-	8	47																															
F40i02	288779	6218619	Tank	-	2	3																															
F40i03	288851	6218593	Tank	-	2	3																															
F41a	288532	6218866	House	-	25	337	On	1	1																												
F41b	288493	6218852	Rural	Shed	14	121																															
F41c	288474	6218844	Rural	Shed	4	10																															
F41i01	288489	6218843	Tank	-	2	5																															
F42a	288238	6217208	House	-	25	378	Bottom	1	1																												
F42i01	288234	6217224	Pool	-	10	31																															
F42i01	288228	6217243	Rural	Garage	20	171																															
F42i02	288246	6217182	Tank	-	8	49																															
F42i03	288223	6217249	Tank	-	2	5																															
F42i04	288224	6217253	Tank	-	2	5																															
F43a	288229	6217261	Tank	-	2	5																															
F43b	288020	6217831	House	-	34	470	On	1	1																												
F43i01	287965	6217768	Rural	Garage	17	246																															
F43i02	288041	6217847	Tank	-	8	56																															
F43i03	288032	6217853	Tank	-	2	2																															
F43i04	288030	6217855	Tank	-	2	2																															
F45i01	287956	6217773	Tank	-	2	2																															
F45i01	288871	6218872	House	-	27	347																															
F45i01	288850	6218871	Pool	-	1	40																															
F45i01	288837	6218863	Rural	Shed	15	212																															
F45i02	288860	6218881	Rural	Carport	13	121																															
F45i03	288807	6218857	Rural	Shed	10	35																															
F45i01	288830	6218854	Tank	-	3	9																															
F50i01	288190	6218575	House	-	16	185	On	1	1																												
F50i01	288186	6218560	Pool	-	6	18																															
F50i01	288178	6218575	Rural	Carport	7	45																															

**Table D.01 - Details of the structures within the Study Area**

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m <sup>2</sup> )	Houses Steep Slopes	Houses Number of Storeys	Houses				House Wall Type			House Roof Type			House Footing Type			Other Structures																
									H1	H2	H3	H4	Brick or Brick Veneer	Weatherboard	Fibro	Tiled	Metal Sheet	Slab on Ground	Suspended on Piered Footings	Suspended on Strip Footings	Rural	Public Amenities	Public Utilities	Commercial														
F5002	288186	6218569	Rural	Pergola	6	27																	1															
F5003	288140	6218599	Rural	Shed	8	41																			1													
F5004	288203	6218563	Rural	Shed	8	22																			1													
F5005	288205	6218555	Rural	Shed	9	26																			1													
F5006	288180	6218550	Rural	Shed	3	9																			1													
F5001	288171	6218573	Tank	-	3	9																																
F5002	288200	6218575	Tank	-	3	6																																
F5003	288200	6218578	Tank	-	3	6																																
F5101	288032	6218520	House	-	35	441	On	1																														
F5101	288012	6218510	Pool	-	8	24																																
F5101	287947	6218466	Rural	Shed	12	87																																
F5102	287951	6218476	Rural	Shed	9	55																																
F5103	287951	6218515	Rural	Shed	3	7																																
F5104	287980	6218472	Rural	Shed	2	3																																
F5101	287973	6218538	Tank	-	10	72																																
F5102	287980	6218515	Tank	-	7	43																																
F5103	287938	6218464	Tank	-	3	6																																
F5201	287830	6218390	House	-	25	305		1	1																													
F5202	287890	6218372	House	-	10	74	On	1																														
F5201	287807	6218401	Rural	Shed	8	69																																
F5202	287803	6218381	Rural	Shed	14	118																																
F5203	287795	6218380	Rural	Pergola	6	29																																
F5204	287883	6218372	Rural	Carport	7	29																																
F5205	287786	6218468	Rural	Shed	6	33																																
F5206	287748	6218463	Rural	Shed	5	20																																
F5207	287742	6218462	Rural	Shed	5	29																																
F5208	287782	6218525	Rural	Shed	4	11																																
F5201	287896	6218365	Tank	-	3	9																																
J01a	289704	6217547	House	-	40	407	Bottom	1																														
J01b	289639	6217515	Rural	Shed	11	69																																
J0101	289690	6217551	Tank	-	8	50																																
J0102	289680	6217533	Tank	-	2	2																																
J0103	289679	6217531	Tank	-	2	2																																
J02a	289938	6217736	House	-	27	442	Top	1																														
J02b	289918	6217752	Rural	Garage	11	96																																
J02c	289876	6217615	Rural	Shed	18	194																																
J0201	289957	6217750	Tank	-	9	61																																
J0202	289969	6217753	Tank	-	2	3																																
J0203	289972	6217753	Tank	-	2	3																																
K01a	288143	6218213	House	-	41	630	Bottom	1																														
K0101	288115	6218235	Rural	Garage	13	156																																
K0102	288194	6218254	Rural	Shed	5	13																																
K0101	288206	6218250	Tank	-	8	48																																
K0102	288198	6218255	Tank	-	2	4																																
K0103	288175	6218216	Tank	-	2	4																																
K02a	288026	6218181	House	-	31	416																																

## Table D.01 - Details of the structures within the Study Area

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m <sup>2</sup> )	Houses Steep Slopes	Houses Number of Storeys	Houses				House Wall Type	House Roof Type	House Footing Type	Other Structures									
									Single Storey with Length less than 30 metres	Single Storey with Length greater than 30 metres	Double Storey with Length less than 30 metres	Double Storey with Length greater than 30 metres				H1	H2	H3	H4	Rural	Public Amenities	Public Utilities	Commercial		
K02001	288033	6218194	Pool	-	11	31																			
K02001	288019	6218188	Rural	Pergola	10	53												1							
K02001	287960	6218219	Rural	Shed	3	7												1							
K02003	287994	6218188	Rural	Shed	2	5													1						
K02001	288062	6218191	Tank	-	8	44																			
K02002	288053	6218183	Tank	-	2	3																			
K02003	288052	6218185	Tank	-	2	3																			
K03a	287915	6218126	House	-	29	470																			
K03001	287888	6218105	Rural	Shed	12	119			1																
K03002	287886	6218113	Rural	Shed	10	38																			
K03003	287880	6218129	Rural	Shed	2	4																			
K03004	287880	6218274	Rural	Shed	3	10																			
K04a	288020	6218050	House	-	19	323																			
K0401	288065	6218075	House	-	33	482																			
K0401	288065	6218062	Pool	-	10	25																			
K0401	288043	6218071	Rural	Garage	16	134																			
K0402	288036	6218021	Rural	Shed	16	186																			
K0403	288043	6218000	Rural	Shed	12	93																			
K0404	288044	6218055	Rural	Shed	7	23																			
K0405	287994	6218078	Rural	Shed	3	10																			
K0401	288087	6218068	Tank	-	8	48																			
K0402	288025	6218032	Tank	-	8	48																			
K0403	288039	6218031	Tank	-	3	9																			
K0404	288031	6218042	Tank	-	2	4																			
K05a	287878	6217973	House	-	29	375																			
K0501	287913	6217951	Rural	Shed	10	99																			
K0501	287851	6217952	Tank	-	8	53																			
K0502	287914	6217944	Tank	-	2	3																			
K0503	287875	6217940	Tank	-	2	3																			
L01a	288859	6218057	House	-	30	451																			
L0101	288960	6218009	Rural	Shed	6	21	Bottom																		
L0101	288874	6218035	Tank	-	9	57																			
L0102	288862	6218032	Tank	-	2	3																			
L0103	288862	6218030	Tank	-	2	3																			
L0104	288828	6218067	Tank	-	2	3																			
N0501	287580	6215764	Rural	Shed	30	247																			
N0501	287562	6215763	Tank	-	3	6																			
N0502	287568	6215772	Tank	-	3	9																			
N0503	287562	6215768	Tank	-	3	9																			
N1101	288429	6216160	House	-	25	425	Bottom																		
N1101	288485	6216139	Rural	Shed	17	131																			
N1103	288464	6216121	Rural	Shed	5	22																			
N1101	288463	6216138	Tank	-	8	46																			
N1102	288478	6216146	Tank	-	3	8																			
N1103	288416	6216208	Tank	-	3	6																			
N1201	288322	6216677	House	-	23	373	Top																		













# Table D.01 - Details of the structures within the Study Area

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m <sup>2</sup> )	Houses Steep Slopes	Houses Number of Storeys	Houses				House Wall Type			House Roof Type			House Footing Type			Other Structures														
									H1	H2	H3	H4	Brick or Brick Veneer	Weatherboard	Fibro	Tiled	Metal Sheet	Slab on Ground	Suspended on Piered Footings	Suspended on Strip Footings	Rural	Public Amenities	Public Utilities	Commercial												
R05001	286144	6217717	Rural	Pergola	6	16																1							1							
R05002	286164	6217701	Rural	Shed	9	58																									1					
R05003	286166	6217693	Rural	Shed	5	52																									1					
R05001	286136	6217722	Tank	-	5	20																														
R05002	286169	6217701	Tank	-	2	3																														
R05003	286169	6217699	Tank	-	2	3																														
R06001	286136	6217765	House	-	22	267		1																												
R06001	286136	6217760	Rural	Garage	9	55																														
R06002	286121	6217753	Rural	Shed	3	10																														
R06003	286162	6217771	Rural	Shed	9	25																														
R06004	286168	6217767	Rural	Shed	9	56																														
R06005	286174	6217765	Rural	Shed	4	6																														
R06001	286159	6217757	Tank	-	2	3																														
R06002	286162	6217766	Tank	-	2	3																														
R09001	286322	6217888	House	-	34	559	Top	1																												
R09001	286317	6217847	Rural	Shed	18	104																														
R09002	286319	6217839	Rural	Shed	9	47																														
R09001	286339	6217854	Tank	-	7	42																														
R10001	286259	6217745	Pool	-	9	27																														
R10001	285890	6217581	House	-	28	271		1																												
R10001	285892	6217558	Pool	-	9	36																														
R10001	285906	6217603	Rural	Shed	7	46																														
R1002	285908	6217560	Rural	Shed	11	99																														
R1003	285871	6217564	Rural	Shed	7	47																														
R10001	285877	6217594	Tank	-	6	27																														
R11003	285788	6217621	Rural	Garage	8	56																														
R11004	285791	6217590	House	Shed	9	47																														
S01001	287793	6218082	Rural	-	41	667		1																												
S01001	287793	6218089	House	-	41	667																														
S01002	287794	6218082	Rural	Shed	16	214																														
S01002	287794	6218126	Rural	Shed	4	11																														
S01003	287799	6218128	Rural	Shed	2	4																														
S01101	287793	6218058	Tank	-	2	4																														
S01102	287871	6218064	Tank	-	2	4																														
S02001	287736	6218069	House	-	31	543		2																												
S02001	287765	6218074	Rural	Garage	12	104																														
S02001	287760	6218093	Tank	-	10	72																														
S02002	287778	6218052	Tank	-	2	2																														
S02003	287779	6218049	Tank	-	2	2																														
S03001	287609	6218033	House	-	37	518		1																												
S03001	287590	6218043	Pool	-	9	29																														
S03001	287654	6218056	Rural	Shed	18	248																														
S03002	287621	6218079	Rural	Shed	3	5																														
S03001	287676	6218032	Tank	-	8	46																														
S04001	287688	6218265	House	-	43	649	Top	1																												
S04001	287691	6218280	Pool	-	10	36																														
S04001	287654	6218267	Rural	Garage	14	127																														

**Table D.01 - Details of the structures within the Study Area**

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Type	Description	Maximum Structure Plan Length (m)	Planar Area (m <sup>2</sup> )	Houses Steep Slopes	Houses Number of Storeys	Houses				House Wall Type			House Roof Type		House Footing Type			Other Structures											
									H1	H2	H3	H4	Brick or Brick Veneer	Weatherboard	Fibro	Tiled	Metal Sheet	Slab on Ground	Suspended on Piered Footings	Suspended on Strip Footings	Rural	Public Amenities	Public Utilities	Commercial								
S04r02	287615	6218232	Rural	Shed	17	146																1					1					
S04r03	287702	6218279	Rural	Cabana	7	47																									1	
S04t01	287633	6218269	Tank	-	9	58																										
S04t02	287643	6218277	Tank	-	2	5																										
S04t03	287646	6218279	Tank	-	2	5																										
S04t04	287609	6218236	Tank	-	2	5																										
S04t05	287611	6218238	Tank	-	2	5																										
S05h01	287549	6218231	House	-	42	520	Top	1																								
S05h02	287523	6218236	House	-	15	156	Top	1	1									1	1													
S05p01	287547	6218249	Pool	-	10	41																										
S05r01	287587	6218235	Rural	Shed	13	150																								1		
S05r01	287516	6218257	Tank	-	9	58																										
S05r02	287528	6218261	Tank	-	2	5																										
S05r03	287531	6218261	Tank	-	2	5																										
S05r04	287596	6218238	Tank	-	4	11																										
S09a	287313	6218064	House	-	9	57	Bottom	1	1																							
S10r01	287187	6218038	Rural	Shed	6	32																										1
S10r02	287187	6218042	Rural	Shed	6	17																										1
S11h01	286986	6217780	House	-	17	197	On	1	1																							1
S11r01	286963	6217781	Rural	Shed	12	64																										1
S11r02	286955	6217782	Rural	Shed	4	13																										1
S11t01	286971	6217776	Tank	-	3	6																										
S11t02	286966	6217775	Tank	-	3	6																										
S11t03	286963	6217776	Tank	-	3	6																										
S13r01	287645	6217878	Rural	Shed	4	10																										1
S14h01	287638	6217927	House	-	15	168	Bottom	1	1																							1
S14r01	287649	6217897	Rural	Shed	6	15																										1
S14t01	287619	6217949	Tank	-	9	60																										

Count 108 47 13 9 121 20 1 59 116 106 11 32 581 0 0 2

# Table D.02 - Predicted subsidence effects for the structures within the Study Area

Structure Reference	Centrad MGA Easting	Centrad MGA Northing	Structure Type	Predicted total subsidence after LW9/9 (mm)	Predicted total subsidence after LW10/8 (mm)	Predicted total subsidence after LW11/7 (mm)	Predicted total after LW9/9 (mm/yr)	Predicted total after LW10/8 (mm/yr)	Predicted total after LW11/7 (mm/yr)	Predicted total subsidence after LW9/9 after LW11/7 (Δmm)	Predicted total subsidence after LW10/8 after LW11/7 (Δmm)	Predicted total sagging after LW9/9 after LW10/8 (Δmm)	Predicted total sagging after LW10/8 after LW11/7 (Δmm)	Predicted total sagging after LW9/9 after LW11/7 (Δmm)	Predicted mean total subsidence (mm/yr)	Predicted mean total subsidence (mm/yr)	Predicted C <sub>t</sub> for total strain (mm/yr)	Predicted C <sub>t</sub> for total strain (mm/yr)	Predicted Probability of impact for houses (%)	Predicted Probability of impact for houses (%)	
A06c	291126	621863	House	775	775	775	6.5	6.5	6.5	0.06	0.06	<0.01	<0.01	<0.01	0.3	0.3	0.1	0.1	29.8	15.7	4.2
A06d	291128	621864	House	900	900	900	6.0	6.0	6.0	0.06	0.06	<0.01	<0.01	<0.01	0.4	0.4	0.1	0.1	33.7	15.3	6.3
A06e	291129	621866	House	900	900	900	6.5	6.5	6.5	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.2	0.2	44.7	38.7	15.3
A06f	291129	621870	Road	975	975	975	2.5	2.5	2.5	0.02	0.02	<0.01	<0.01	<0.01	0.5	0.5	0.2	0.2	-	-	-
A06g	291133	621883	Road	975	975	975	4.0	4.0	4.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A06h	291137	621893	Road	960	960	960	6.5	6.5	6.5	0.05	0.05	<0.01	<0.01	<0.01	1.1	1.1	0.4	0.4	-	-	-
A06i	291139	621819	Road	960	960	960	2.0	2.0	2.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A06j	291143	621898	Road	860	860	860	6.5	6.5	6.5	0.02	0.02	<0.01	<0.01	<0.01	0.9	0.9	0.3	0.3	-	-	-
A06k	291148	621899	Road	860	860	860	6.5	6.5	6.5	0.02	0.02	<0.01	<0.01	<0.01	0.9	0.9	0.3	0.3	-	-	-
A06l	291152	621893	Track	700	700	700	6.5	6.5	6.5	0.06	0.06	<0.01	<0.01	<0.01	0.8	0.8	0.3	0.3	-	-	-
A06m	291170	621899	Track	725	725	725	6.5	6.5	6.5	0.05	0.05	<0.01	<0.01	<0.01	0.8	0.8	0.3	0.3	-	-	-
A06n	291175	621899	Track	725	725	725	6.5	6.5	6.5	0.05	0.05	<0.01	<0.01	<0.01	0.8	0.8	0.3	0.3	-	-	-
A06o	291179	621899	Track	700	700	700	6.5	6.5	6.5	0.05	0.05	<0.01	<0.01	<0.01	0.8	0.8	0.3	0.3	-	-	-
A06p	291180	621899	Track	950	950	950	2.5	2.5	2.5	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A06q	291180	621906	House	1300	1300	1300	5.0	5.0	5.0	0.05	0.05	<0.01	<0.01	<0.01	0.7	0.7	0.4	0.4	10.6	19.9	48.8
A06r	291181	621907	House	1300	1300	1300	5.0	5.0	5.0	0.05	0.05	<0.01	<0.01	<0.01	0.7	0.7	0.4	0.4	10.6	19.9	48.8
A06s	291182	621908	House	1325	1325	1325	4.5	4.5	4.5	0.05	0.05	<0.01	<0.01	<0.01	0.8	0.8	0.5	0.5	11.3	20.6	51.1
A06t	291182	621909	House	1325	1325	1325	4.5	4.5	4.5	0.05	0.05	<0.01	<0.01	<0.01	0.8	0.8	0.5	0.5	11.3	20.6	51.1
A06u	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A06v	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A06w	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A06x	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A06y	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A06z	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07a	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07b	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07c	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07d	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07e	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07f	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07g	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07h	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07i	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07j	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07k	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07l	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07m	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07n	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07o	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07p	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07q	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07r	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07s	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07t	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07u	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07v	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07w	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07x	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07y	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-
A07z	291182	621910	Road	1400	1400	1400	3.0	3.0	3.0	0.02	0.02	<0.01	<0.01	<0.01	0.6	0.6	0.3	0.3	-	-	-



# Table D.02 - Predicted subsidence effects for the structures within the Study Area

Structure Reference	Centroid MGA Easting	Centroid MGA Northing	Structure Type	Predicted total subsidence after LW99 (mm)	Predicted total subsidence after LW108 (mm)	Predicted total subsidence after LW111 (mm)	Predicted total subsidence after LW120 (mm)	Predicted total subsidence after LW125 (mm)	Predicted total subsidence after LW130 (mm)	Predicted total subsidence after LW135 (mm)	Predicted total subsidence after LW140 (mm)	Predicted total subsidence after LW145 (mm)	Predicted total subsidence after LW150 (mm)	Predicted total subsidence after LW155 (mm)	Predicted total subsidence after LW160 (mm)	Predicted total subsidence after LW165 (mm)	Predicted total subsidence after LW170 (mm)	Predicted total subsidence after LW175 (mm)	Predicted total subsidence after LW180 (mm)	Predicted total subsidence after LW185 (mm)	Predicted total subsidence after LW190 (mm)	Predicted mean total subsidence (mm/yr)	Predicted 95% CI for total mean subsidence (mm/yr)	Predicted 95% CI for total strain (mm/yr)	Predicted Probability of Impact for Houses (%)	Predicted Probability of Impact for Houses (%)	Predicted Probability of Impact for Houses (%)	Predicted Probability of Impact for Houses (%)	
02065	281076	621822	Rural	133	130	127	124	121	118	115	112	109	106	103	100	97	94	91	88	85	82	0.5	0.5	0.1	1.7	-	-	-	
02066	281076	621822	Rural	130	127	124	121	118	115	112	109	106	103	100	97	94	91	88	85	82	79	76	0.5	0.5	0.1	1.7	-	-	-
02067	281076	621822	Rural	127	124	121	118	115	112	109	106	103	100	97	94	91	88	85	82	79	76	0.5	0.5	0.1	1.7	-	-	-	
02068	281076	621822	Rural	124	121	118	115	112	109	106	103	100	97	94	91	88	85	82	79	76	73	0.5	0.5	0.1	1.7	-	-	-	
02069	281076	621822	Rural	121	118	115	112	109	106	103	100	97	94	91	88	85	82	79	76	73	70	0.5	0.5	0.1	1.7	-	-	-	
02070	281076	621822	Rural	118	115	112	109	106	103	100	97	94	91	88	85	82	79	76	73	70	67	0.5	0.5	0.1	1.7	-	-	-	
02071	281076	621822	Rural	115	112	109	106	103	100	97	94	91	88	85	82	79	76	73	70	67	64	0.5	0.5	0.1	1.7	-	-	-	
02072	281076	621822	Rural	112	109	106	103	100	97	94	91	88	85	82	79	76	73	70	67	64	61	0.5	0.5	0.1	1.7	-	-	-	
02073	281076	621822	Rural	109	106	103	100	97	94	91	88	85	82	79	76	73	70	67	64	61	58	0.5	0.5	0.1	1.7	-	-	-	
02074	281076	621822	Rural	106	103	100	97	94	91	88	85	82	79	76	73	70	67	64	61	58	55	0.5	0.5	0.1	1.7	-	-	-	
02075	281076	621822	Rural	103	100	97	94	91	88	85	82	79	76	73	70	67	64	61	58	55	52	0.5	0.5	0.1	1.7	-	-	-	
02076	281076	621822	Rural	100	97	94	91	88	85	82	79	76	73	70	67	64	61	58	55	52	49	0.5	0.5	0.1	1.7	-	-	-	
02077	281076	621822	Rural	97	94	91	88	85	82	79	76	73	70	67	64	61	58	55	52	49	46	0.5	0.5	0.1	1.7	-	-	-	
02078	281076	621822	Rural	94	91	88	85	82	79	76	73	70	67	64	61	58	55	52	49	46	43	0.5	0.5	0.1	1.7	-	-	-	
02079	281076	621822	Rural	91	88	85	82	79	76	73	70	67	64	61	58	55	52	49	46	43	40	0.5	0.5	0.1	1.7	-	-	-	
02080	281076	621822	Rural	88	85	82	79	76	73	70	67	64	61	58	55	52	49	46	43	40	37	0.5	0.5	0.1	1.7	-	-	-	
02081	281076	621822	Rural	85	82	79	76	73	70	67	64	61	58	55	52	49	46	43	40	37	34	0.5	0.5	0.1	1.7	-	-	-	
02082	281076	621822	Rural	82	79	76	73	70	67	64	61	58	55	52	49	46	43	40	37	34	31	0.5	0.5	0.1	1.7	-	-	-	
02083	281076	621822	Rural	79	76	73	70	67	64	61	58	55	52	49	46	43	40	37	34	31	28	0.5	0.5	0.1	1.7	-	-	-	
02084	281076	621822	Rural	76	73	70	67	64	61	58	55	52	49	46	43	40	37	34	31	28	25	0.5	0.5	0.1	1.7	-	-	-	
02085	281076	621822	Rural	73	70	67	64	61	58	55	52	49	46	43	40	37	34	31	28	25	22	0.5	0.5	0.1	1.7	-	-	-	
02086	281076	621822	Rural	70	67	64	61	58	55	52	49	46	43	40	37	34	31	28	25	22	19	0.5	0.5	0.1	1.7	-	-	-	
02087	281076	621822	Rural	67	64	61	58	55	52	49	46	43	40	37	34	31	28	25	22	19	16	0.5	0.5	0.1	1.7	-	-	-	
02088	281076	621822	Rural	64	61	58	55	52	49	46	43	40	37	34	31	28	25	22	19	16	13	0.5	0.5	0.1	1.7	-	-	-	
02089	281076	621822	Rural	61	58	55	52	49	46	43	40	37	34	31	28	25	22	19	16	13	10	0.5	0.5	0.1	1.7	-	-	-	
02090	281076	621822	Rural	58	55	52	49	46	43	40	37	34	31	28	25	22	19	16	13	10	7	0.5	0.5	0.1	1.7	-	-	-	
02091	281076	621822	Rural	55	52	49	46	43	40	37	34	31	28	25	22	19	16	13	10	7	4	0.5	0.5	0.1	1.7	-	-	-	
02092	281076	621822	Rural	52	49	46	43	40	37	34	31	28	25	22	19	16	13	10	7	4	1	0.5	0.5	0.1	1.7	-	-	-	
02093	281076	621822	Rural	49	46	43	40	37	34	31	28	25	22	19	16	13	10	7	4	1	0	0.5	0.5	0.1	1.7	-	-	-	
02094	281076	621822	Rural	46	43	40	37	34	31	28	25	22	19	16	13	10	7	4	1	0	0	0.5	0.5	0.1	1.7	-	-	-	
02095	281076	621822	Rural	43	40	37	34	31	28	25	22	19	16	13	10	7	4	1	0	0	0	0.5	0.5	0.1	1.7	-	-	-	
02096	281076	621822	Rural	40	37	34	31	28	25	22	19	16	13	10	7	4	1	0	0	0	0	0.5	0.5	0.1	1.7	-	-	-	
02097	281076	621822	Rural	37	34	31	28	25	22	19	16	13	10	7	4	1	0	0	0	0	0	0.5	0.5	0.1	1.7	-	-	-	
02098	281076	621822	Rural	34	31	28	25	22	19	16	13	10	7	4	1	0	0	0	0	0	0	0.5	0.5	0.1	1.7	-	-	-	
02099	281076	621822	Rural	31	28	25	22	19	16	13	10	7	4	1	0	0	0	0	0	0	0	0.5	0.5	0.1	1.7	-	-	-	
02100	281076	621822	Rural	28	25	22	19	16	13	10	7	4	1	0	0	0	0	0	0	0	0	0.5	0.5	0.1	1.7	-	-	-	
02101	281076	621822	Rural	25	22	19	16	13	10	7	4	1	0	0	0	0	0	0	0	0	0	0.5	0.5	0.1	1.7	-	-	-	
02102	281076	621822	Rural	22	19	16	13	10	7	4	1	0	0	0	0	0	0	0	0	0	0	0.5	0.5	0.1	1.7	-	-	-	
02103	281076	621822	Rural	19	16	13	10	7	4	1	0	0	0	0	0	0	0	0	0	0	0	0.5	0.5	0.1	1.7	-	-	-	
02104	281076	621822	Rural	16	13	10	7	4	1	0	0	0	0	0	0	0	0	0	0	0	0	0.5	0.5	0.1	1.7	-	-	-	
02105	281076	621822	Rural	13	10	7	4	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0.5	0.5	0.1	1.7	-	-	-	
02106	281076	621822	Rural	10	7	4	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.5	0.5	0.1	1.7	-	-	-	
02107	281076	621822	Rural	7	4	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.5	0.5	0.1	1.7	-	-	-	
02108	281076	621822	Rural	4	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.5	0.5	0.1	1.7	-	-	-	
02109	281076	621822	Rural	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.5	0.5	0.1	1.7	-	-	-	
02110	281076	621822	Rural	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.5	0.5	0.1	1.7	-	-	-	



# Table D.02 - Predicted subsidence effects for the structures within the Study Area

Structure Reference	Central OGA Easting	Central OGA Northing	Structure Type	Predicted total subsidence after LW99 (mm)	Predicted total subsidence after LW108 (mm)	Predicted total subsidence after LW126 (mm)	Predicted total subsidence after LW131 (mm)	Predicted total subsidence after LW136 (mm)	Predicted total subsidence after LW141 (mm)	Predicted total subsidence after LW146 (mm)	Predicted total subsidence after LW151 (mm)	Predicted total subsidence after LW156 (mm)	Predicted total subsidence after LW161 (mm)	Predicted total subsidence after LW166 (mm)	Predicted total subsidence after LW171 (mm)	Predicted total subsidence after LW176 (mm)	Predicted total subsidence after LW181 (mm)	Predicted total subsidence after LW186 (mm)	Predicted total subsidence after LW191 (mm)	Predicted total subsidence after LW196 (mm)	Predicted mean total subsidence (mm)	Predicted % C for total mean subsidence (mm)	Predicted Probability of impact for structures after LW171 (1/Ann)	Predicted Probability of impact for structures after LW176 (1/Ann)	Predicted Probability of impact for structures after LW181 (1/Ann)	Predicted Probability of impact for structures after LW186 (1/Ann)	Predicted Probability of impact for structures after LW191 (1/Ann)	Predicted Probability of impact for structures after LW196 (1/Ann)	Predicted Probability of impact for structures after LW201 (1/Ann)	Predicted Probability of impact for structures after LW206 (1/Ann)	Predicted Probability of impact for structures after LW211 (1/Ann)	Predicted Probability of impact for structures after LW216 (1/Ann)	Predicted Probability of impact for structures after LW221 (1/Ann)	Predicted Probability of impact for structures after LW226 (1/Ann)	Predicted Probability of impact for structures after LW231 (1/Ann)																				
E001A1	283000	621000	Rural	125	130	135	140	145	150	155	160	165	170	175	180	185	190	195	200	205	210	0.5	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	0.5	0.5	-	-	-	-	-	-	-	-	-							
D004A2	283500	621500	Rural	70	70	70	70	70	70	70	70	70	70	70	70	70	70	70	70	70	70	0.5	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	0.5	0.5	-	-	-	-	-	-	-	-	-	-					
D004A3	284000	622000	Rural	70	70	70	70	70	70	70	70	70	70	70	70	70	70	70	70	70	70	0.5	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01	<0.01













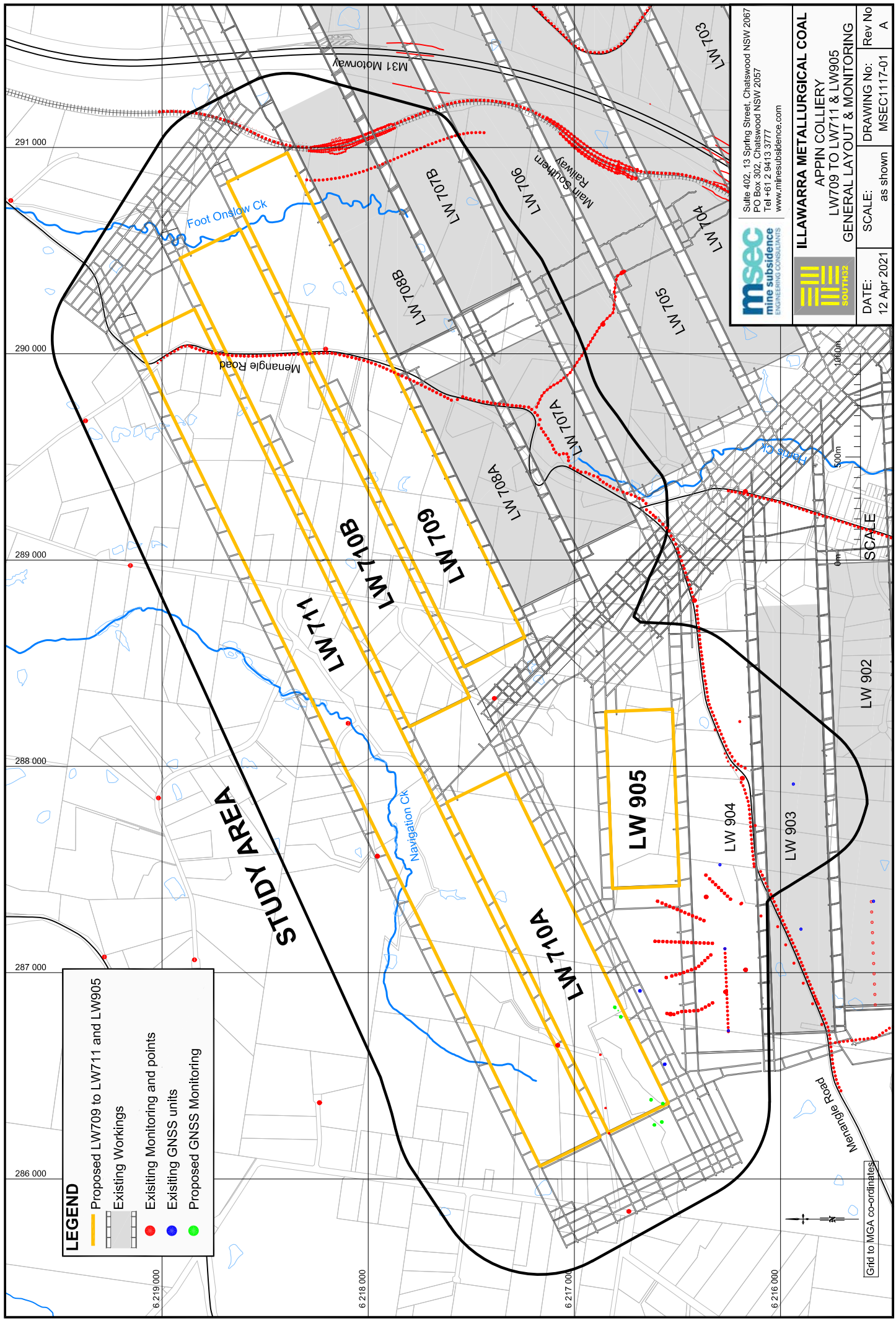








## **APPENDIX E. DRAWINGS**



**LEGEND**

- Proposed LW709 to LW711 and LW905
- Existing Workings
- Existing Monitoring and points
- Existing GNSS units
- Proposed GNSS Monitoring

Suite 402, 13 Spring Street, Chatswood NSW 2067  
 PO Box 302, Chatswood NSW 2057  
 Tel +61 2 9413 3777  
 www.minesubsidence.com

APPIN COLLIERY  
 LW709 to LW711 & LW905  
 GENERAL LAYOUT & MONITORING

DATE:	12 Apr 2021	SCALE:	as shown	DRAWING No:	MSEC1117-01	Rev No:	A
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Grid to MGA co-ordinates

SCALE

LW 902

LW 903

LW 904

LW 905

LW 703

LW 704

LW 705

LW 706

LW 707A

LW 707B

LW 708A

LW 708B

LW 709

LW 710A

LW 710B

LW 711

Navigation Ck

Foot Onslow Ck

Menangle Road

M31 Motorway

Main Southern Railway

500m

286 000

287 000

288 000

289 000

290 000

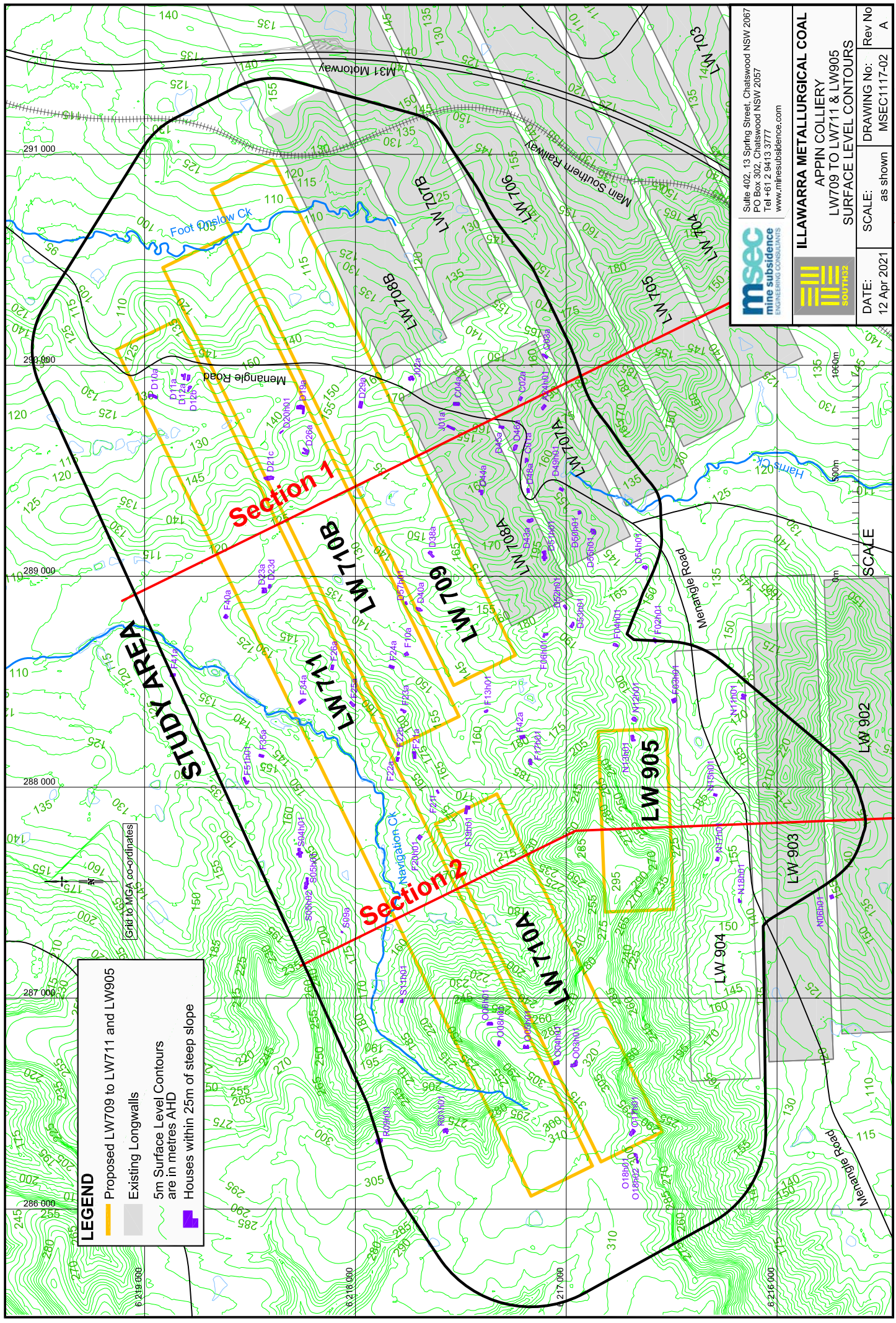
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6 219 000



**LEGEND**

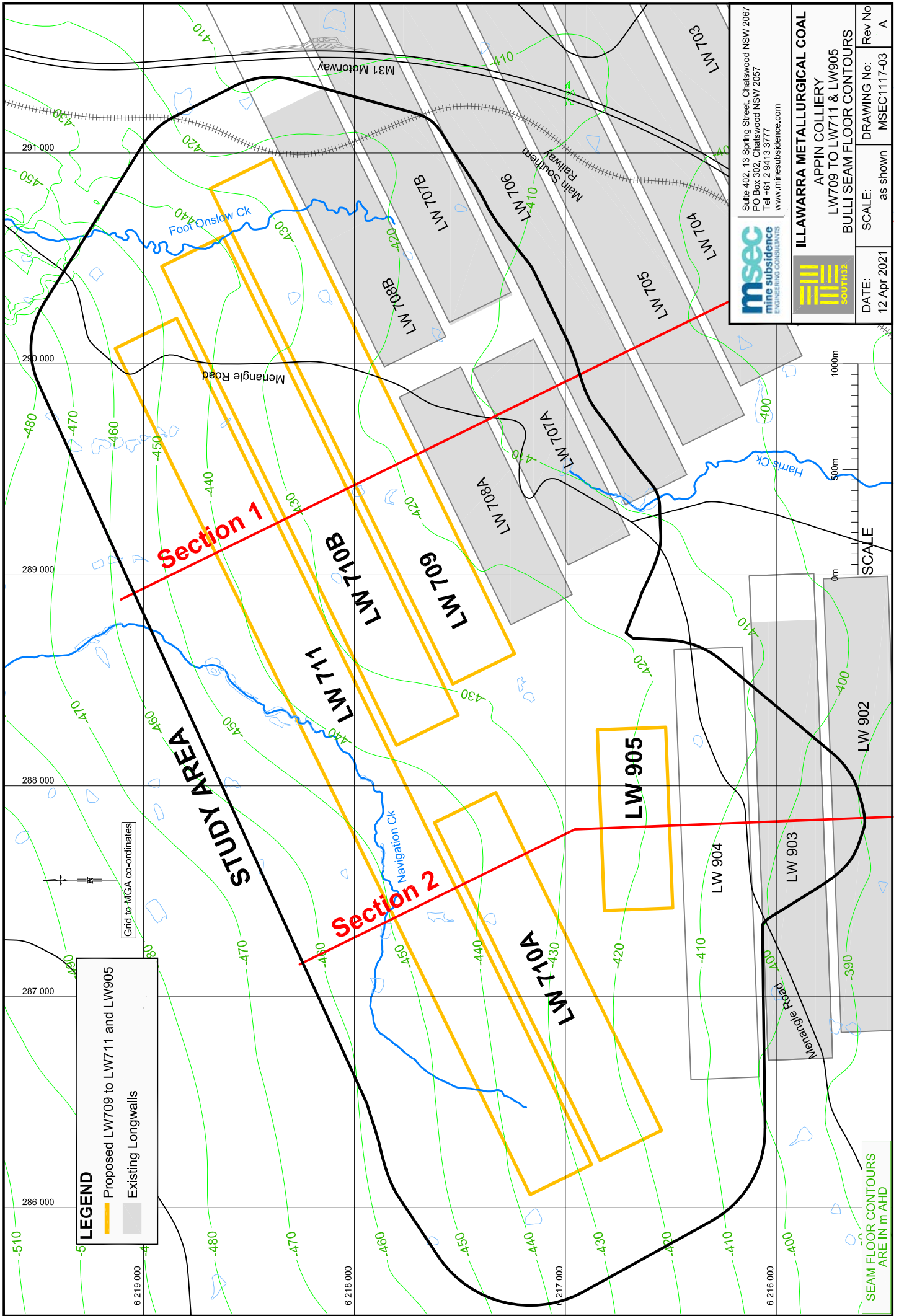
- Proposed LW709 to LW711 and LW905
- Existing Longwalls
- 5m Surface Level Contours are in metres AHD
- Houses within 25m of steep slope

**msec**  
mine subsidence  
ENGINEERING CONSULTANTS

Suite 402, 13 Spring Street, Chatswood NSW 2067  
PO Box 302, Chatswood NSW 2057  
Tel +61 2 9413 3777  
www.minesubsidence.com

**ILLAWARRA METALLURGICAL COAL**  
APPIN COLLIERY  
LW709 to LW711 & LW905  
SURFACE LEVEL CONTOURS

DATE:	12 Apr 2021	SCALE:	as shown	Rev No	A
DRAWING No:			MSEC1117-02		



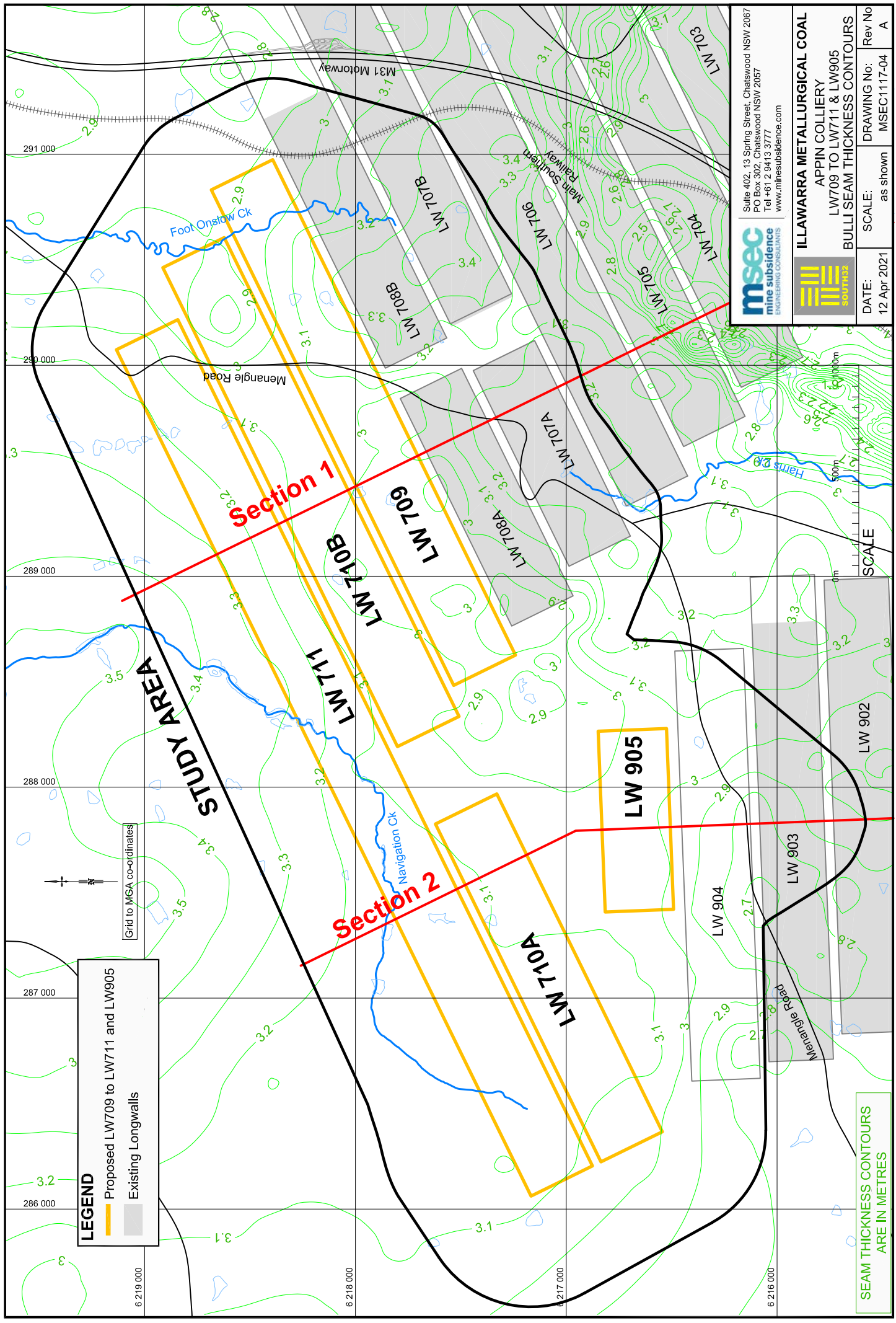
Suite 402, 13 Spring Street, Chatswood NSW 2067  
 PO Box 302, Chatswood NSW 2057  
 Tel +61 2 9413 3777  
 www.minesubsidence.com



**ILLAWARRA METALLURGICAL COAL**  
 APPIN COLLIERY  
 LW709 TO LW711 & LW905  
 BULLI SEAM FLOOR CONTOURS

DATE:	12 Apr 2021	SCALE:	as shown	DRAWING No:	MSEC1117-03	Rev No:	A
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SEAM FLOOR CONTOURS ARE IN m AHD



**LEGEND**

- Proposed LW709 to LW711 and LW905
- Existing Longwalls

Grid to MGA co-ordinates

SEAM THICKNESS CONTOURS ARE IN METRES

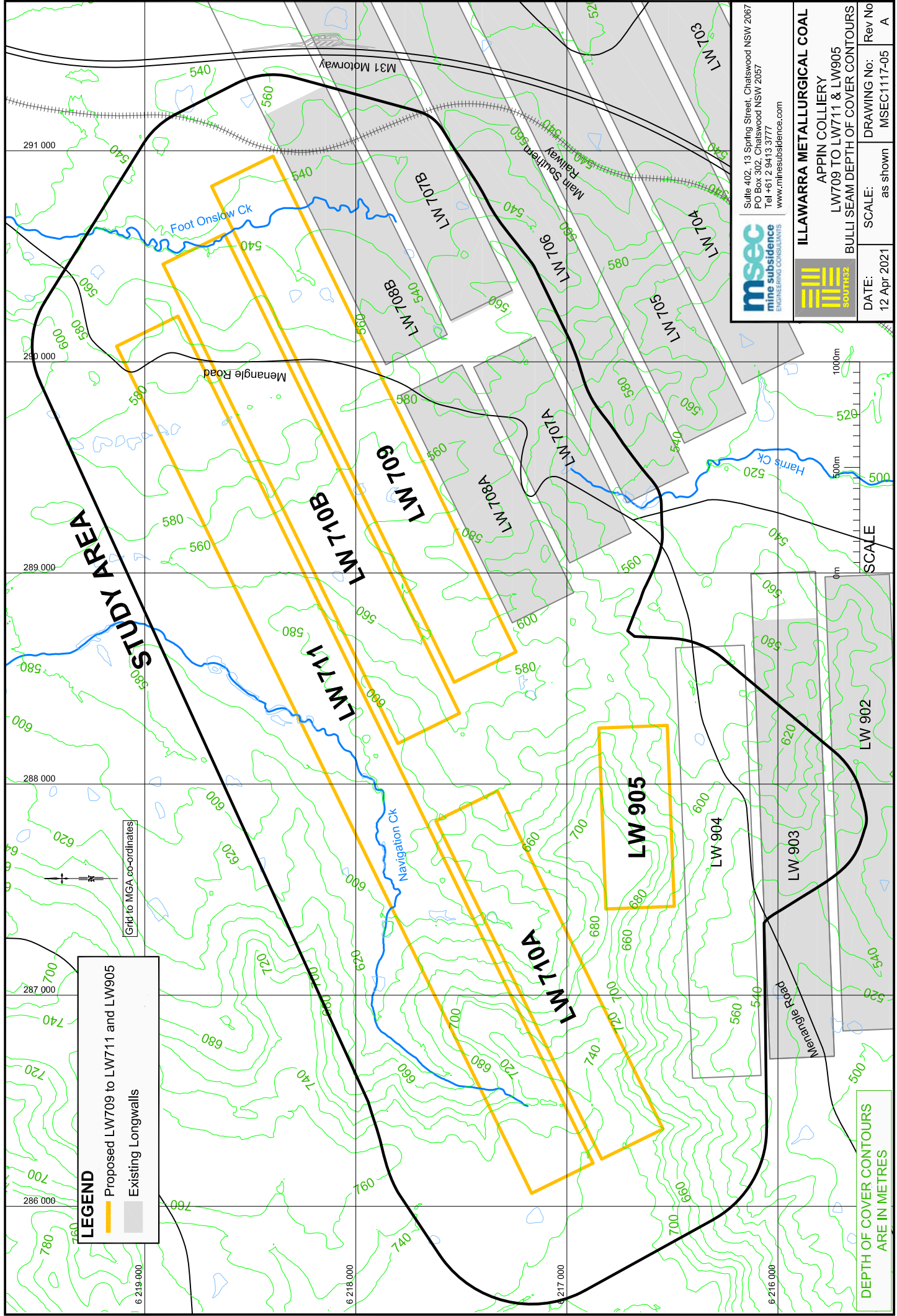


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ILLAWARRA METALLURGICAL COAL  
 APPIN COLLIERY  
 LW709 to LW711 & LW905  
 BULLI SEAM THICKNESS CONTOURS

DATE:	12 Apr 2021	SCALE:	as shown	DRAWING No:	MSEC1117-04	Rev No:	A
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**LEGEND**

- Proposed LW709 to LW711 and LW905
- Existing Longwalls

Grid to MGA co-ordinates

**STUDY AREA**



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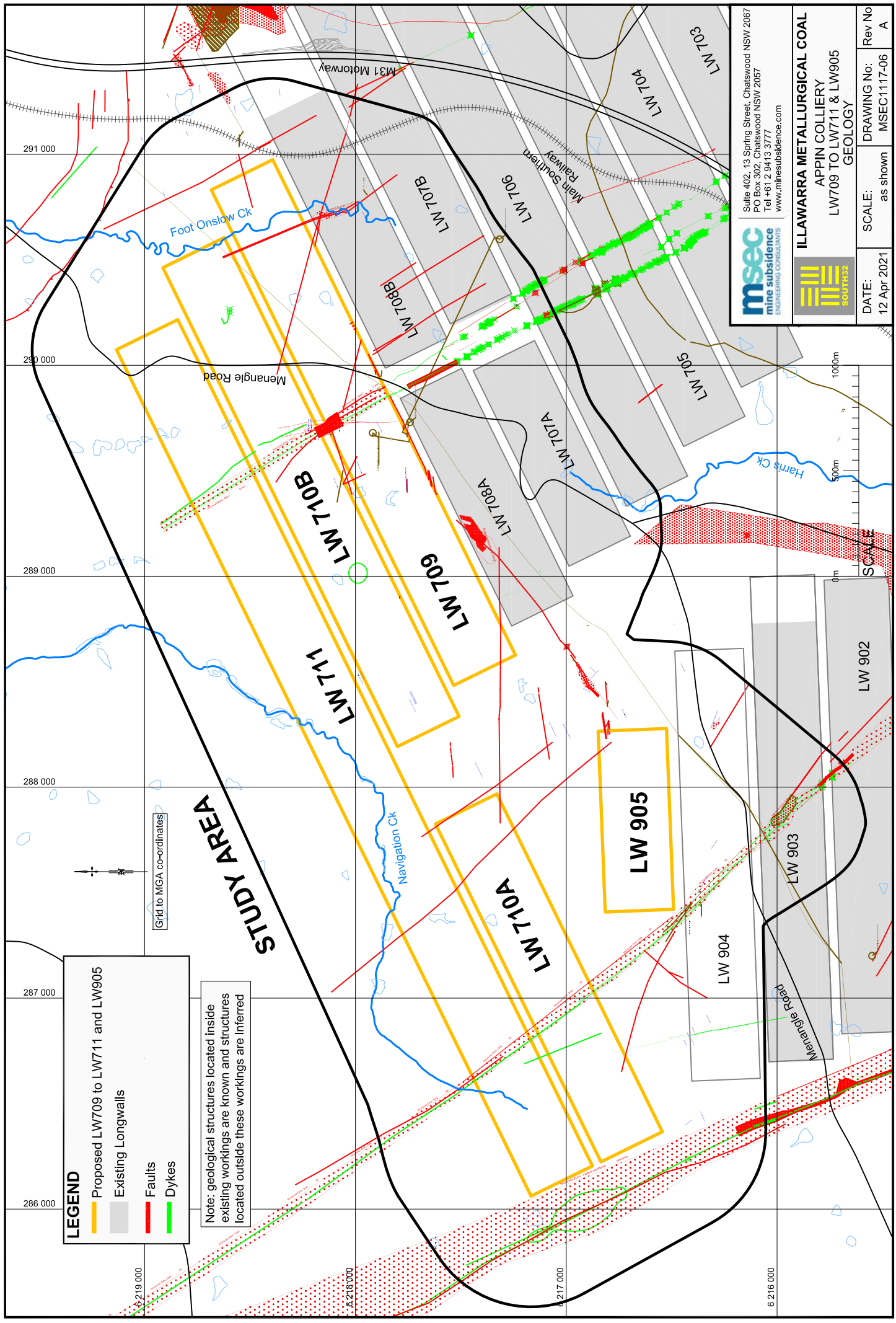


**ILLAWARRA METALLURGICAL COAL**  
 APIN COLLIERY  
 LW709 TO LW711 & LW905  
 BULLI SEAM DEPTH OF COVER CONTOURS

DATE:	12 Apr 2021	SCALE:	as shown	DRAWING No:	MSEC1117-05	Rev No:	A
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SCALE  
 0m  
 50m  
 100m

DEPTH OF COVER CONTOURS  
 ARE IN METRES



**LEGEND**

- Proposed LW709 to LW711 and LW905
- Existing Longwalls
- Faults
- Dykes

Note: geological structures located inside existing workings are known and structures located outside these workings are inferred

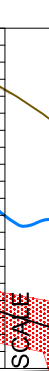
Grid to MGA co-ordinates

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ENGINEERING CONSULTANTS

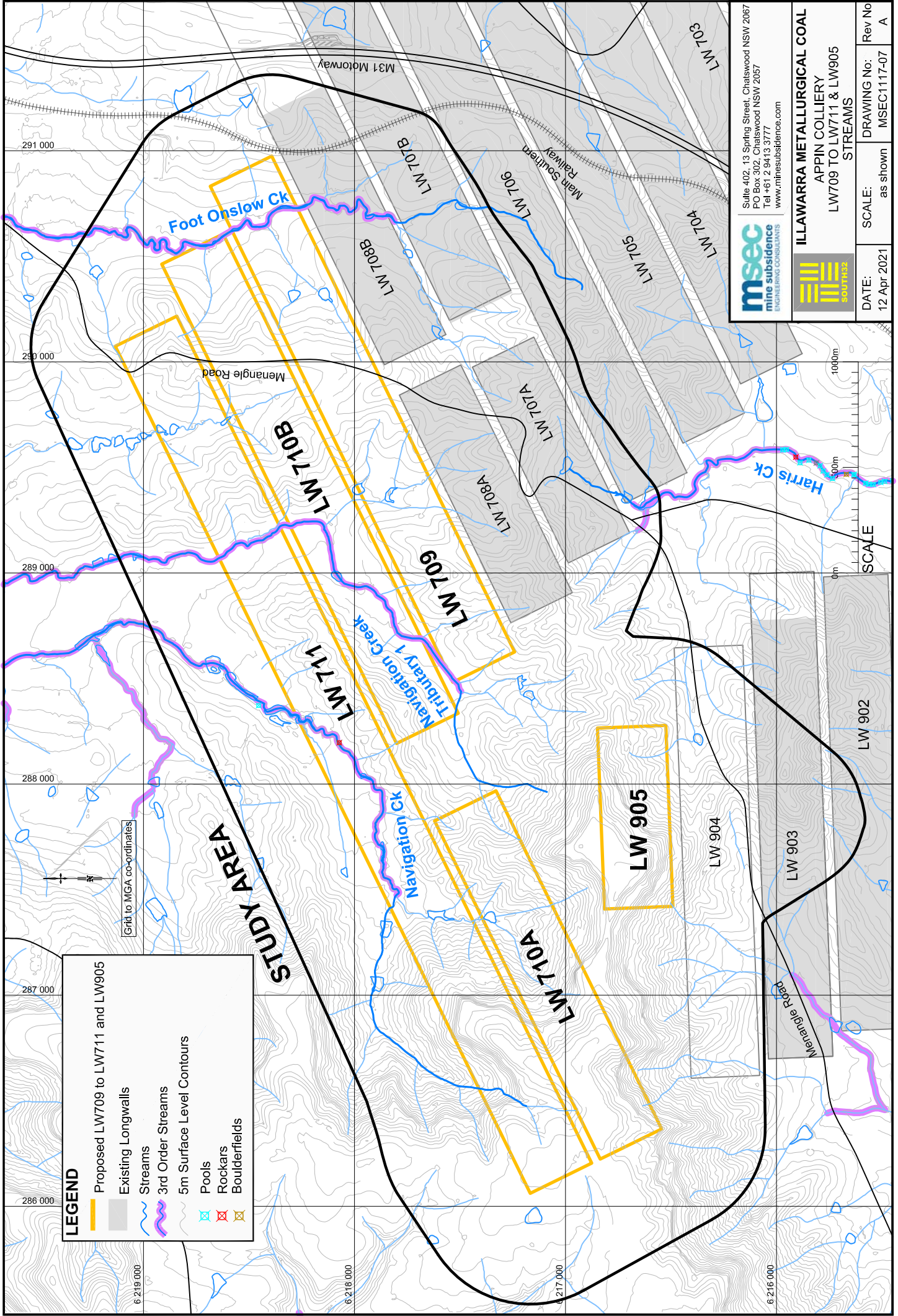
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**ILLAWARRA METALLURGICAL COAL**  
APPIN COLLIERY  
LW709 to LW711 & LW905  
GEOLOGY

DATE:	12 Apr 2021	SCALE:	as shown	DRAWING No:	MSEC1117-06	Rev No:	A
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SCALE



**LEGEND**

- Proposed LW709 to LW711 and LW905
- Existing Longwalls
- Streams
- 3rd Order Streams
- 5m Surface Level Contours
- Pools
- Rockars
- Boulderfields

Grid to MGA co-ordinates

**STUDY AREA**

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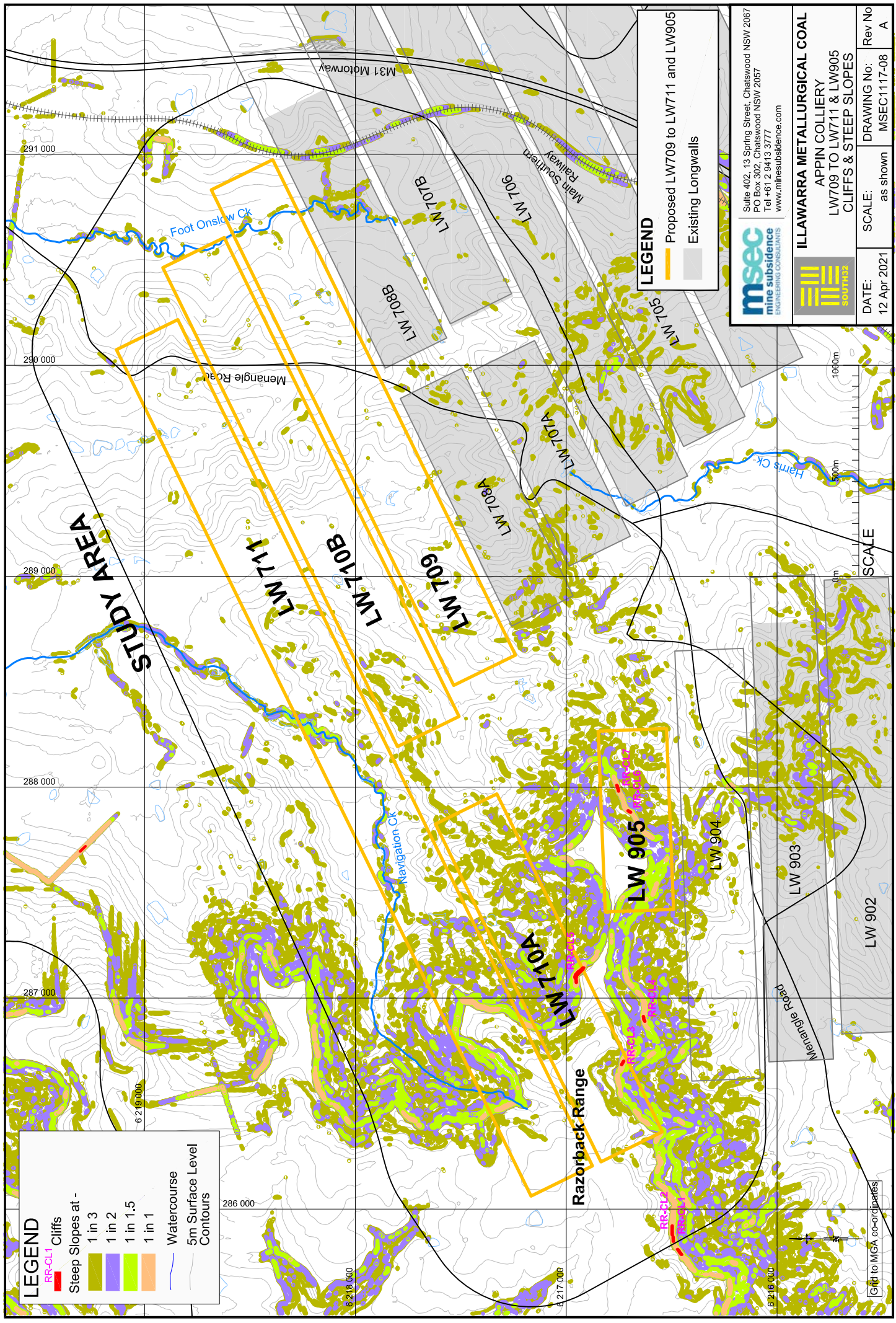
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PO Box 302, Chatswood NSW 2067  
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**ILLAWARRA METALLURGICAL COAL**  
APPIN COLLIERY  
LW709 to LW711 & LW905  
STREAMS

DATE: 12 Apr 2021

SCALE: as shown

DRAWING No: MSEC1117-07  
Rev No: A



**LEGEND**

**RR-CL1** Cliffs

Steep Slopes at -

- 1 in 3
- 1 in 2
- 1 in 1.5
- 1 in 1

Watercourse

5m Surface Level Contours

**LEGEND**

- Proposed LW709 to LW711 and LW905
- Existing Longwalls

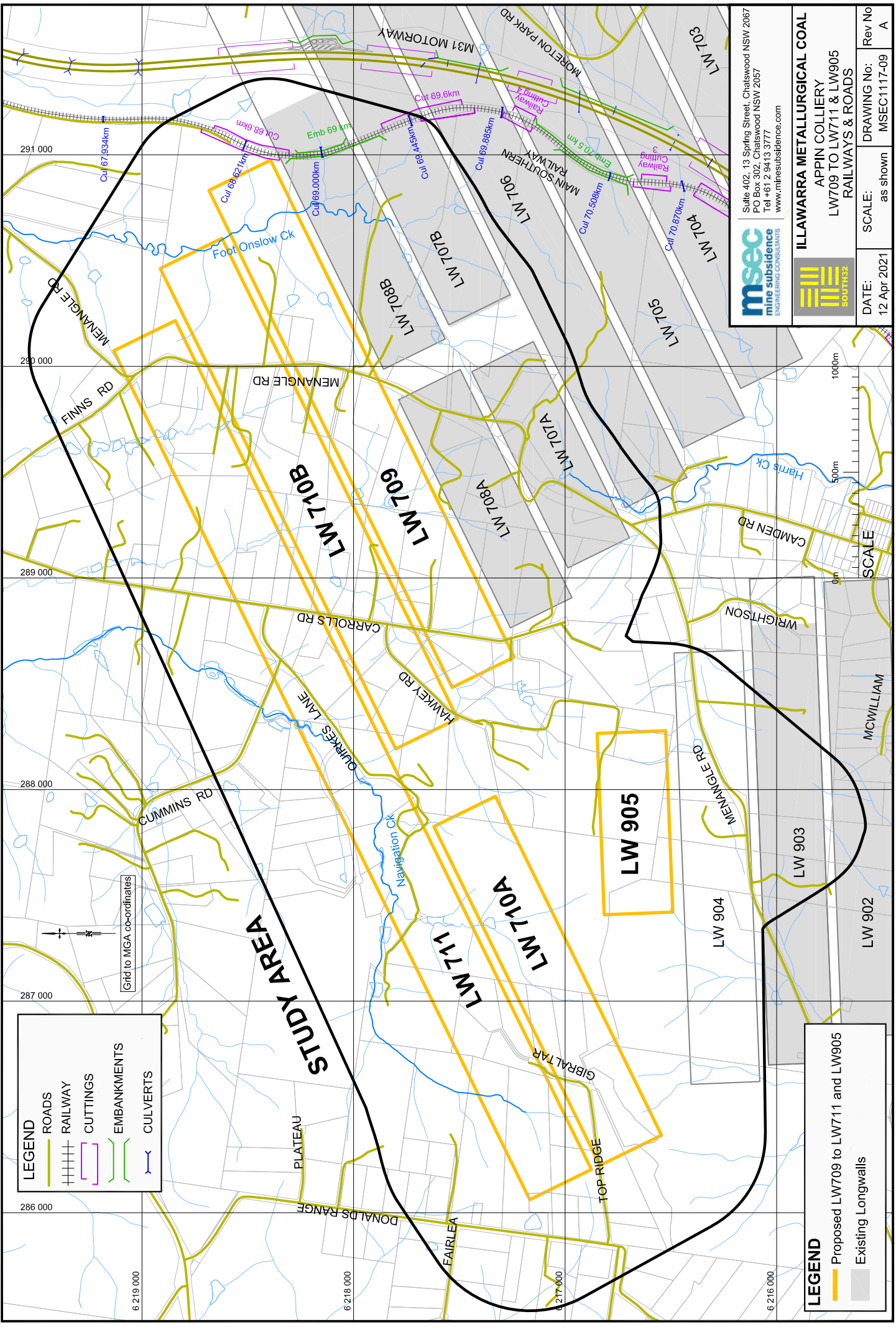
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**ILLAWARRA METALLURGICAL COAL**  
APPIN COLLIERY  
LW709 TO LW711 & LW905  
CLIFFS & STEEP SLOPES

**DATE:** 12 Apr 2021  
**SCALE:** as shown  
**DRAWING No:** MSEC1117-08  
**Rev No:** A

Grid to MGA co-ordinates



**LEGEND**

- ROADS
- RAILWAY
- CUTTINGS
- EMBANKMENTS
- CULVERTS

**LEGEND**

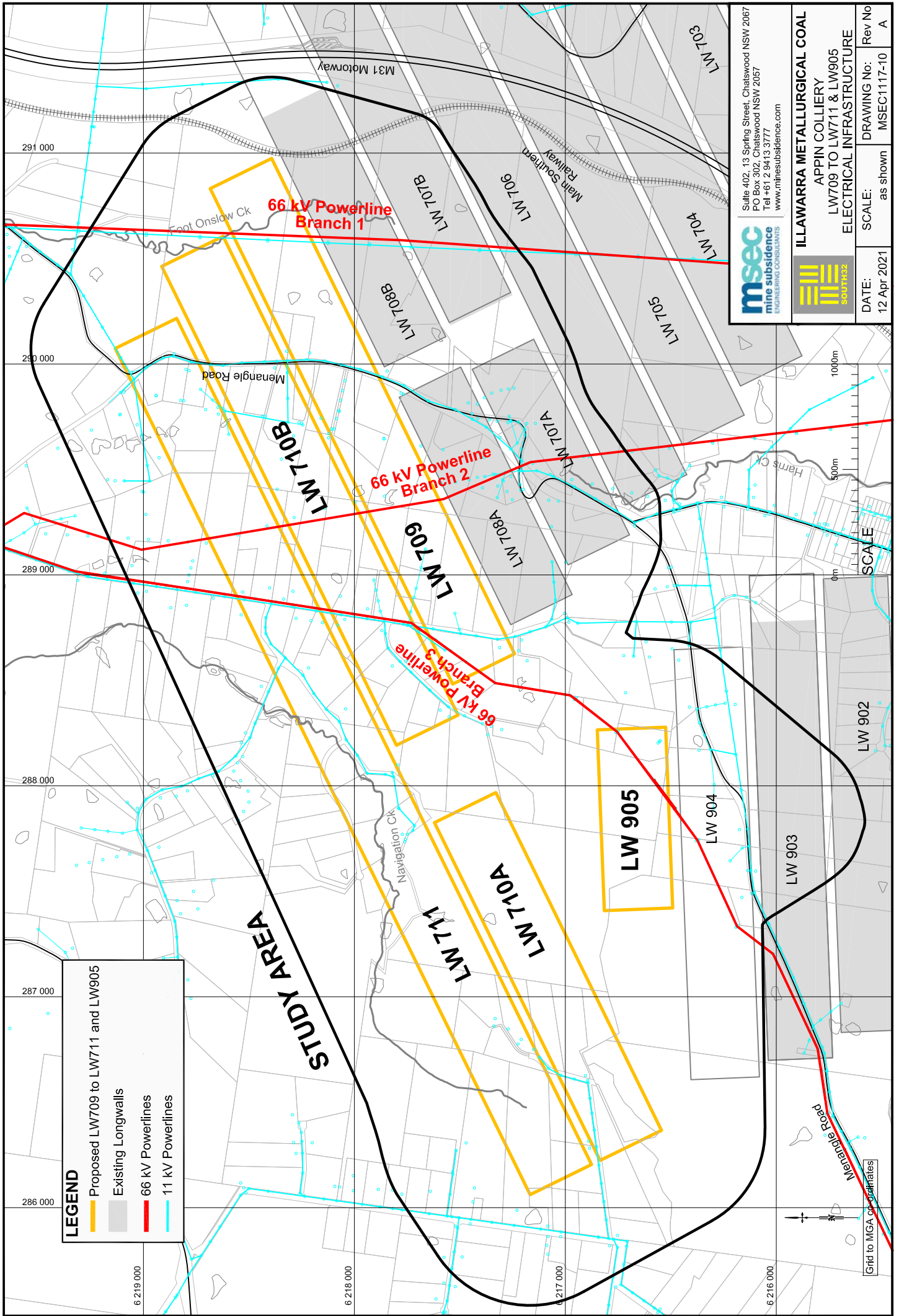
- Proposed LW709 to LW711 and LW905
- Existing Longwalls

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**ILLAWARRA METALLURGICAL COAL**  
 APPIN COLLIERY  
 LW709 to LW711 & LW905  
 RAILWAYS & ROADS

**DATE:** 12 Apr 2021  
**SCALE:** as shown  
**DRAWING No:** MSEC1117-09  
**Rev No:** A



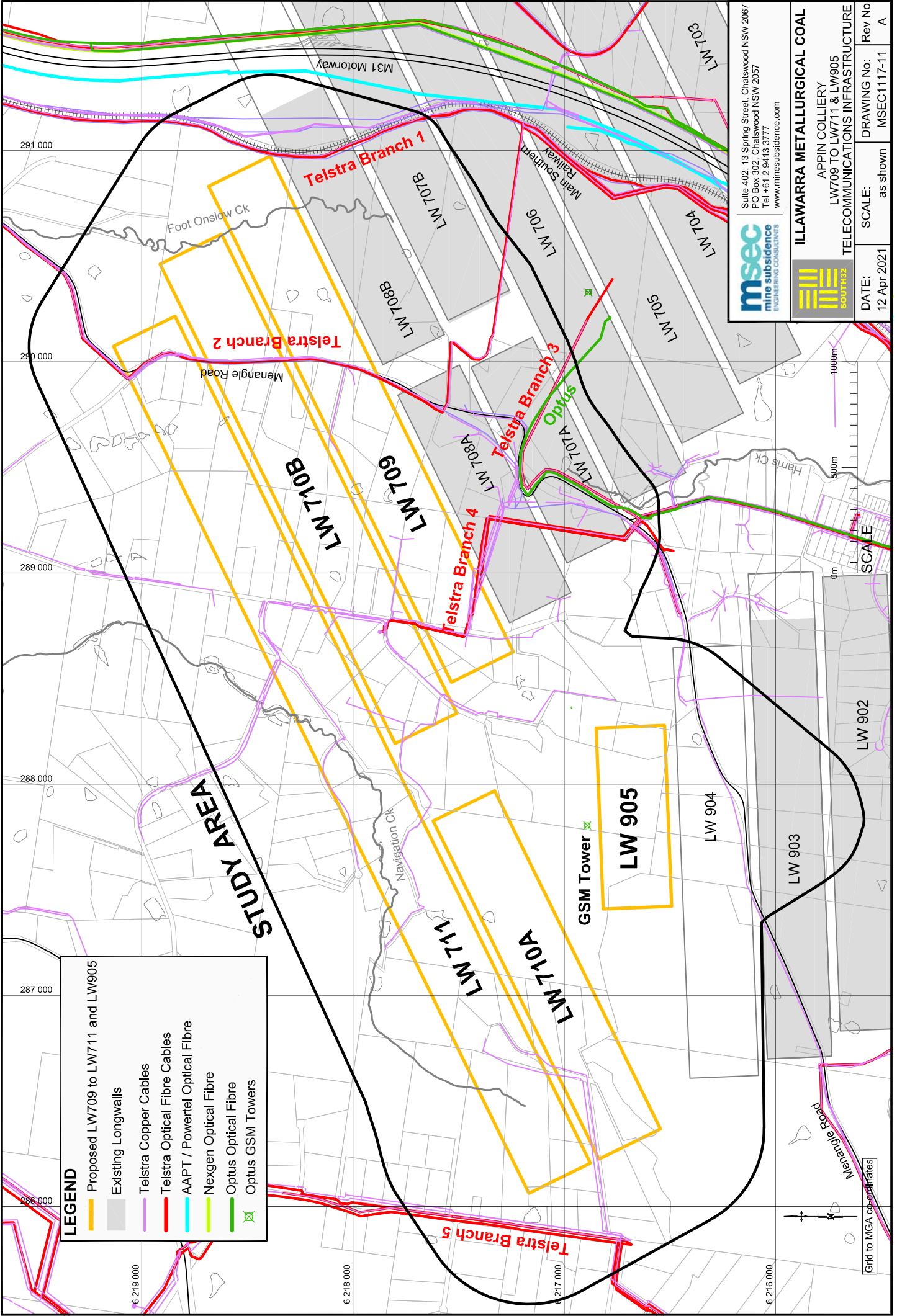
**LEGEND**

- ▬ Proposed LW709 to LW711 and LW905
- Existing Longwalls
- ▬ 66 kV Powerlines
- ▬ 11 kV Powerlines

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		ILLAWARRA METALLURGICAL COAL APPIN COLLIERY LW709 to LW711 & LW905 ELECTRICAL INFRASTRUCTURE	
DATE:	12 Apr 2021	SCALE:	as shown
DRAWING No:	MSEC1117-10	Rev No:	A



Grid to MGA coordinate



**LEGEND**

- ▬ Proposed LW709 to LW711 and LW905
- Existing Longwalls
- ▬ Telstra Copper Cables
- ▬ Telstra Optical Fibre Cables
- ▬ AAPT / PowerTel Optical Fibre
- ▬ Nexgen Optical Fibre
- ▬ Optus Optical Fibre
- X Optus GSM Towers

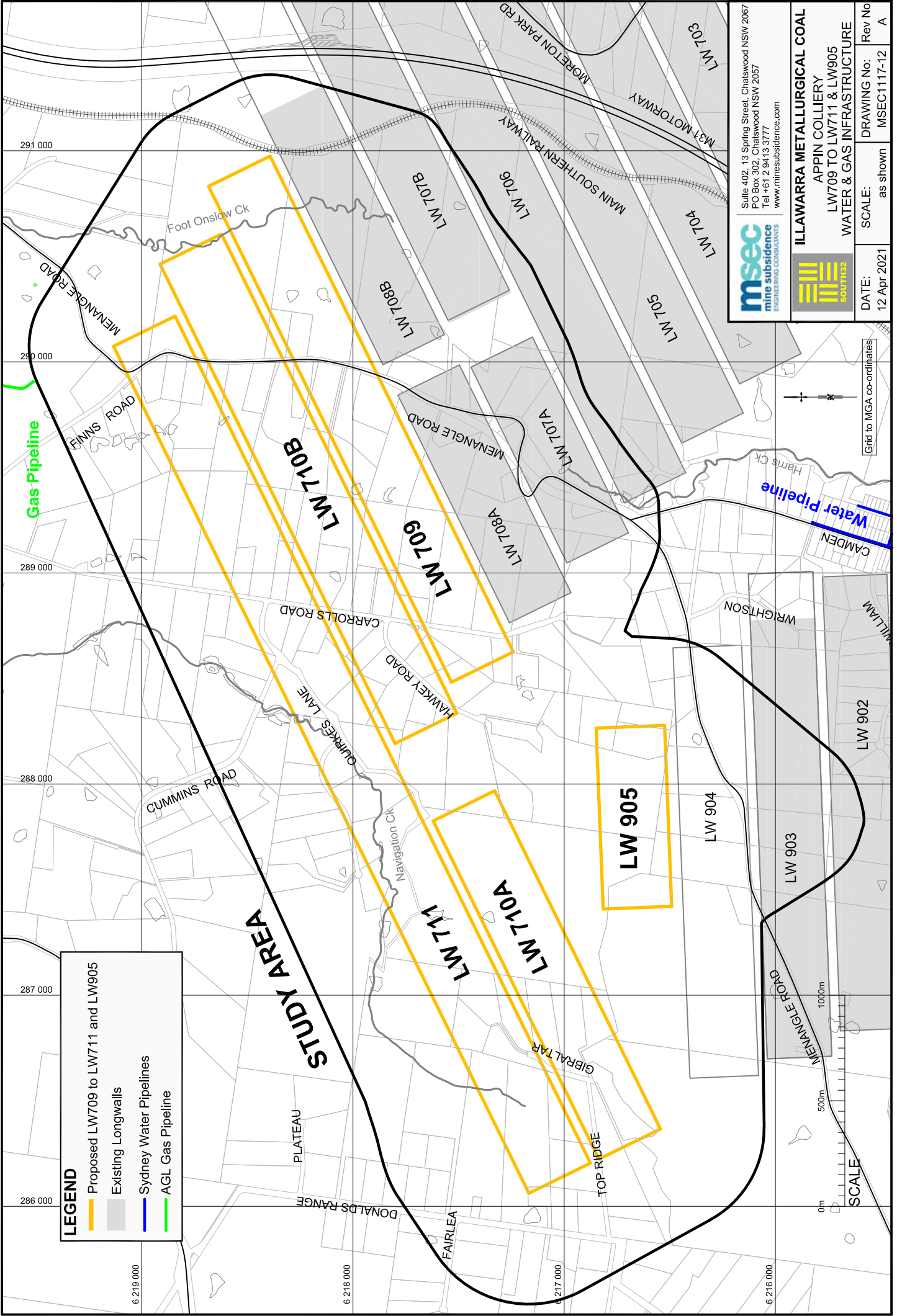



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**ILLAWARRA METALLURGICAL COAL**  
 APPIN COLLIERY  
 LW709 TO LW711 & LW905  
 TELECOMMUNICATIONS INFRASTRUCTURE

DATE:	12 Apr 2021	SCALE:	as shown	DRAWING No:	MSEC1117-11	Rev No:	A
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Grid to MGA coordinates



**LEGEND**

- ▬ Proposed LW709 to LW711 and LW905
- Existing Longwalls
- ▬ Sydney Water Pipelines
- ▬ AGL Gas Pipeline

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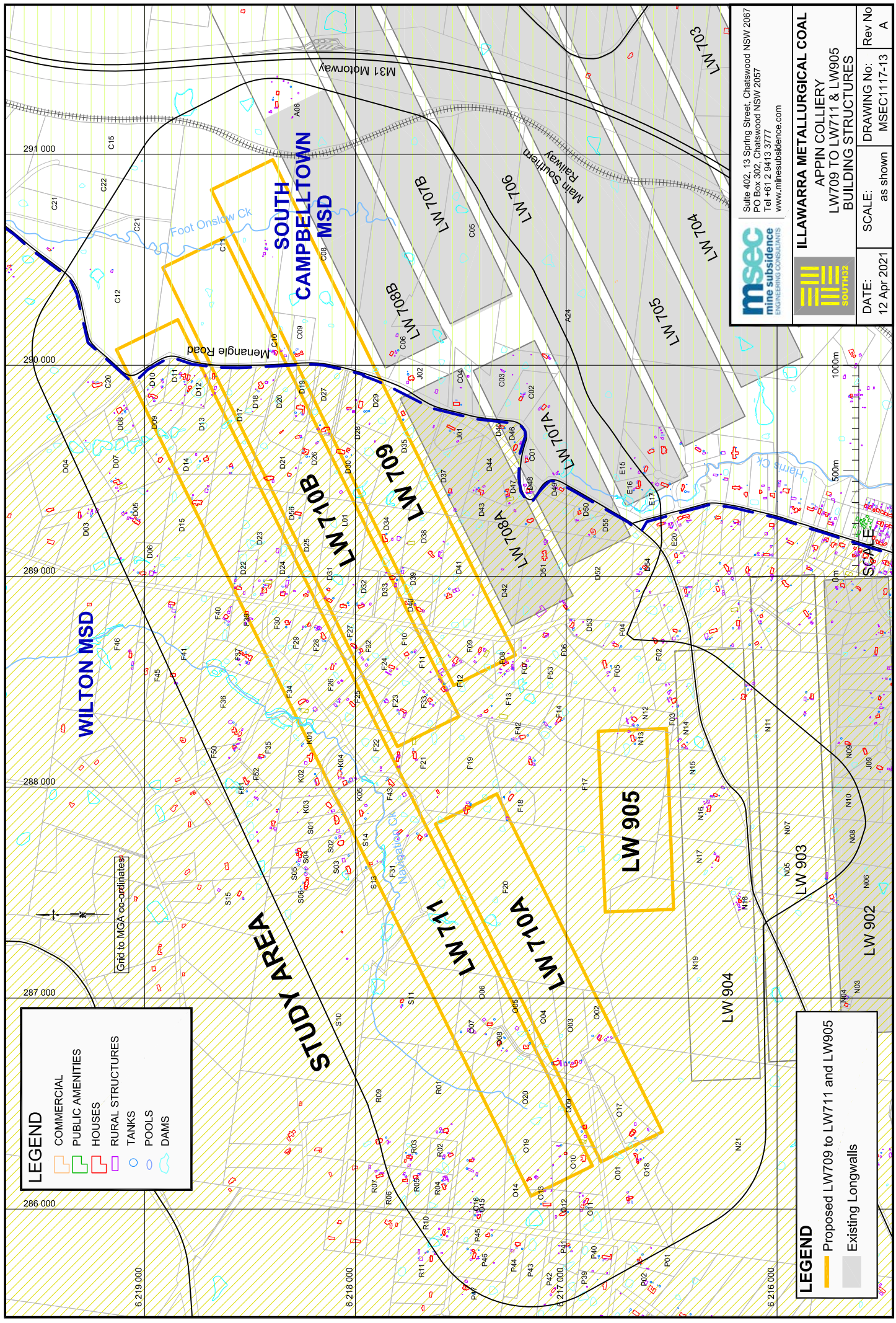
**ILLAWARRA METALLURGICAL COAL**  
APPIN COLLIERY  
LW709 TO LW711 & LW905  
WATER & GAS INFRASTRUCTURE

DATE: 12 Apr 2021  
SCALE: as shown  
DRAWING No: MSEC1117-12  
Rev No: A

Grid to MGA co-ordinates

SCALE

0m 500m 1000m



**LEGEND**

- COMMERCIAL
- PUBLIC AMENITIES
- HOUSES
- RURAL STRUCTURES
- TANKS
- POOLS
- DAMS

Grid to MGA co-ordinates

**LEGEND**

- Proposed LW709 to LW711 and LW905
- Existing Longwalls



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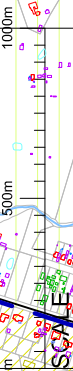


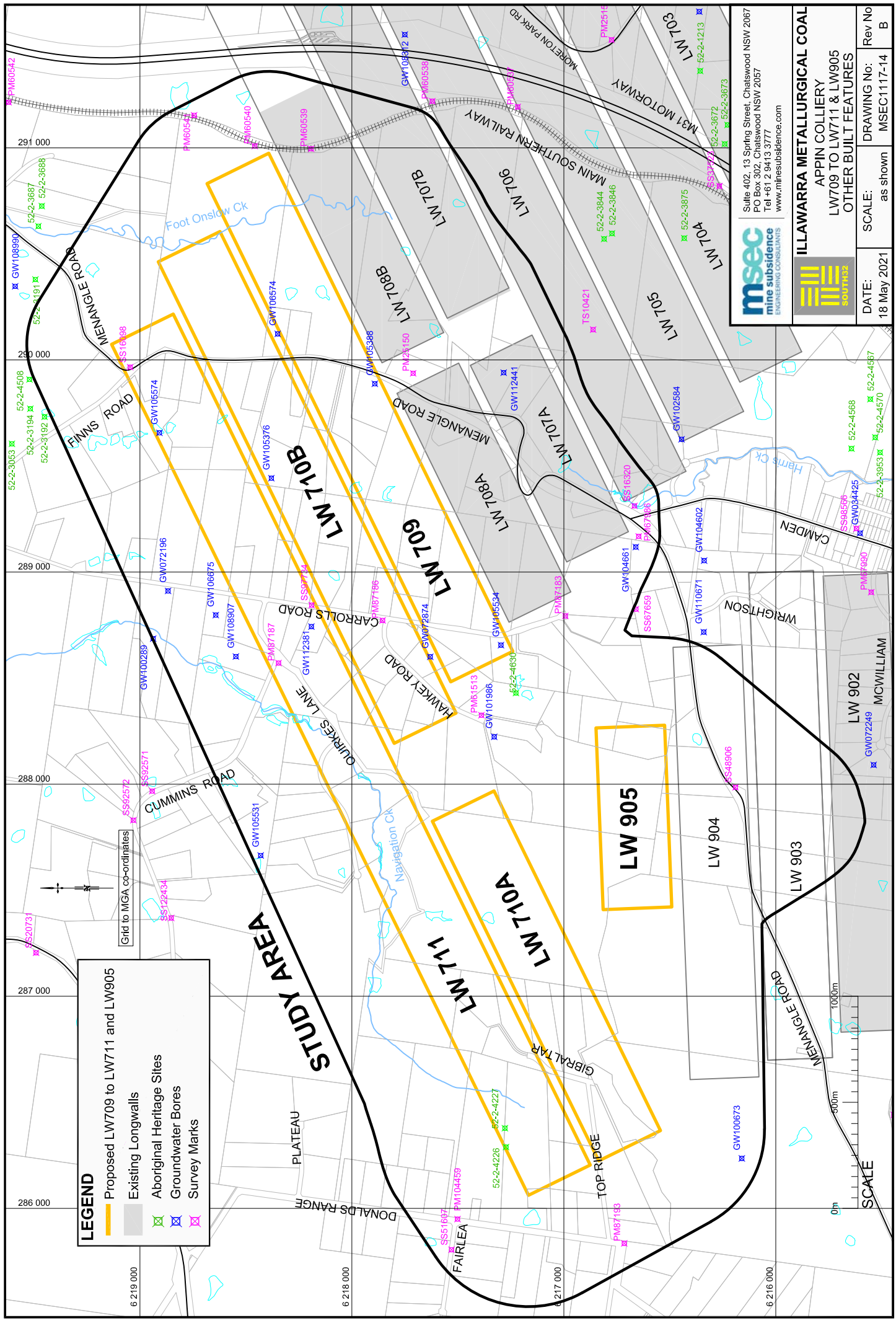
**ILLAWARRA METALLURGICAL COAL**  
 APPIN COLLIERY  
 LW709 TO LW711 & LW905  
 BUILDING STRUCTURES

DATE: 12 Apr 2021

SCALE: as shown

Rev No: A  
 DRAWING No: MSEC1117-13

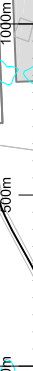
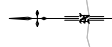




**LEGEND**

- Proposed LW709 to LW711 and LW905
- Existing Longwalls
- ✕ Aboriginal Heritage Sites
- ✕ Groundwater Bores
- ✕ Survey Marks

Grid to MGA co-ordinates



SCALE

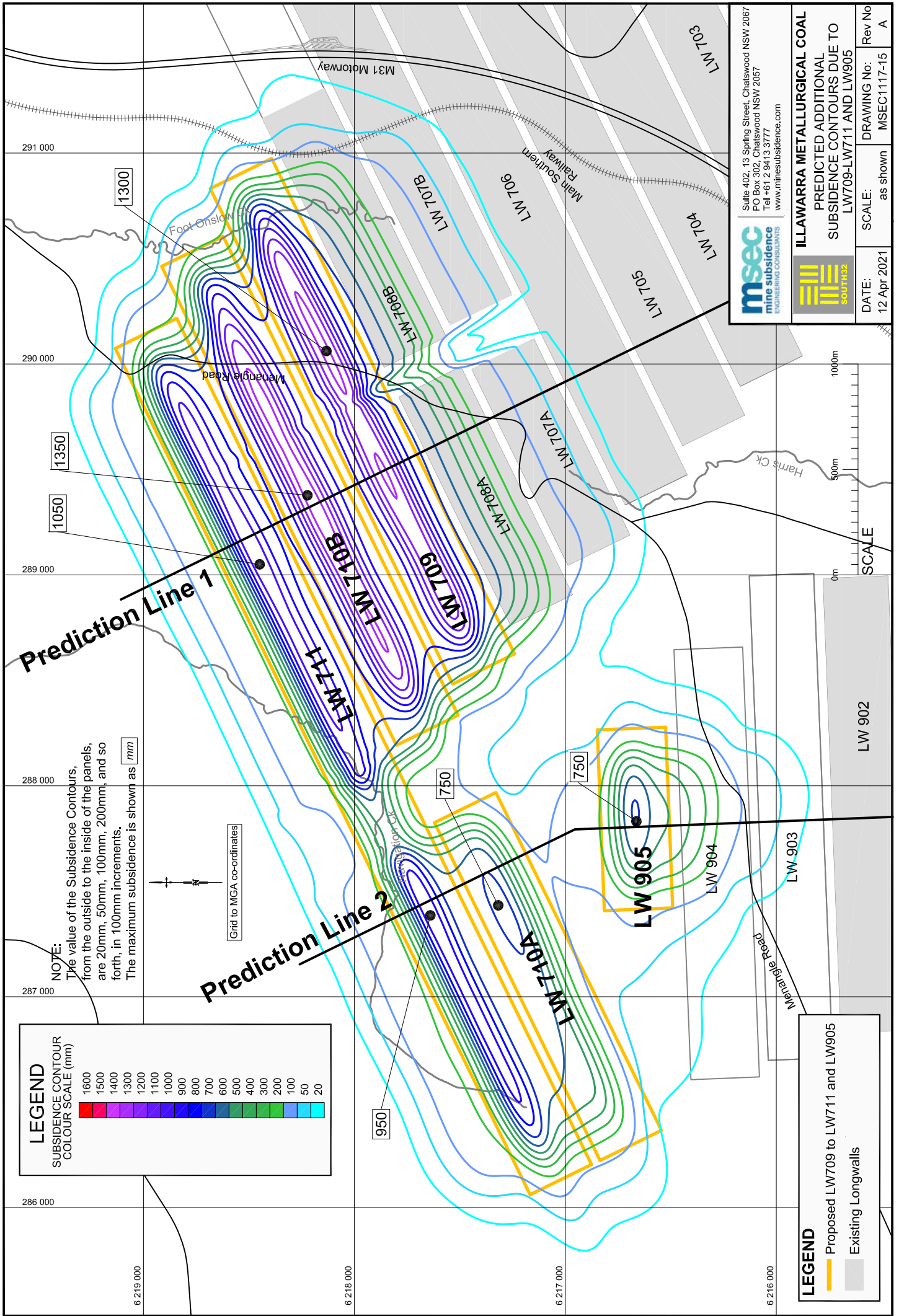


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**ILLAWARRA METALLURGICAL COAL**  
 APPIN COLLIERY  
 LW709 TO LW711 & LW905  
 OTHER BUILT FEATURES

DATE:	18 May 2021	Rev No:	B
SCALE:	as shown	DRAWING No:	MSEC1117-14

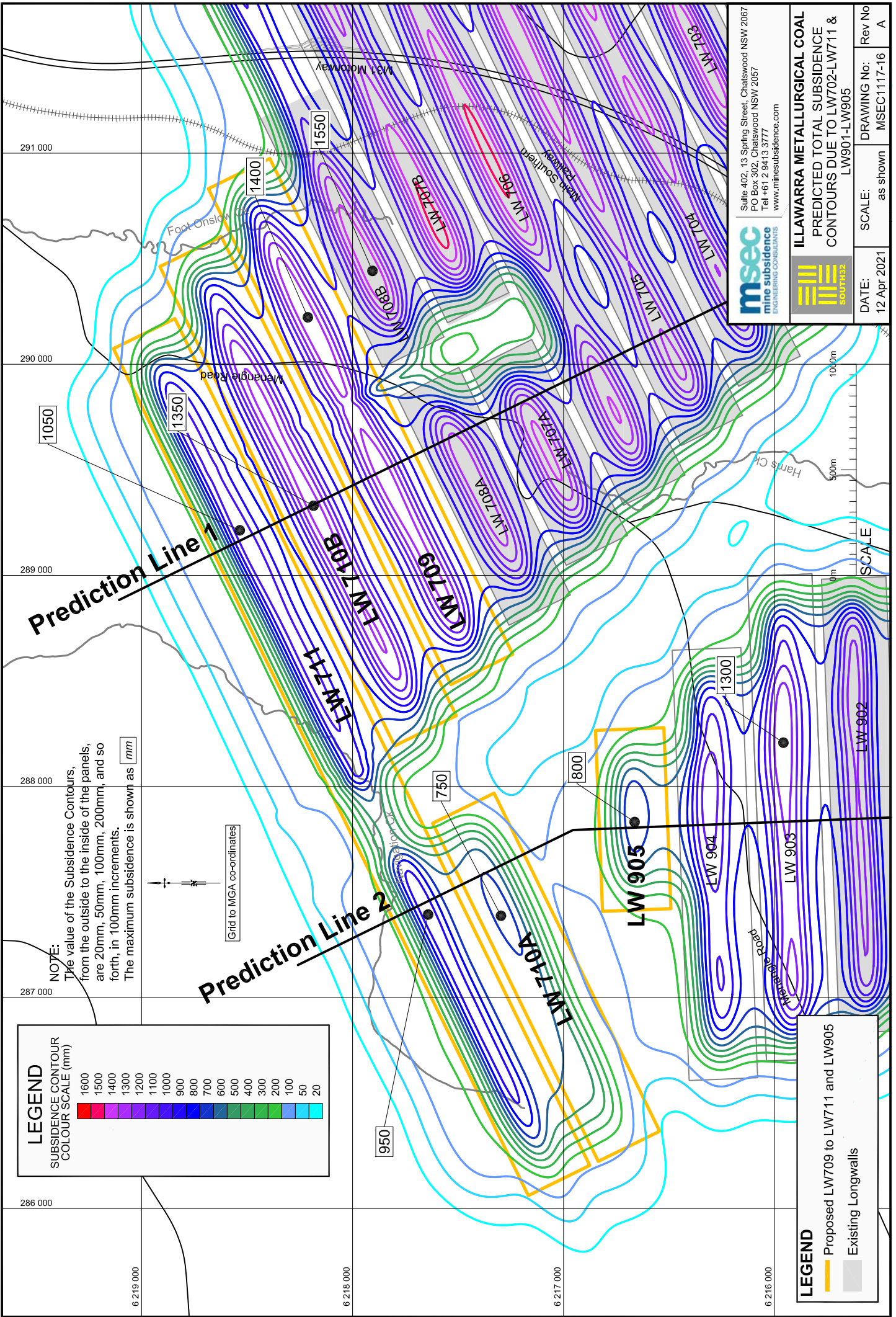


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**ILLAWARRA METALLURGICAL COAL**  
PREDICTED ADDITIONAL  
SUBSIDENCE CONTOURS DUE TO  
LW709-LW711 AND LW905

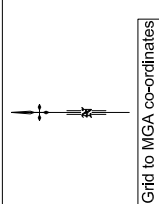
DATE: 12 Apr 2021  
SCALE: as shown  
DRAWING No: MSEC1117-15  
Rev No: A



**LEGEND**  
SUBSIDENCE CONTOUR COLOUR SCALE (mm)

1600	Red
1500	Orange
1400	Yellow
1300	Light Green
1200	Green
1100	Light Blue
1000	Blue
900	Dark Blue
800	Teal
700	Light Cyan
600	Cyan
500	Light Green
400	Green
300	Light Green
200	Yellow
100	Light Green
50	Light Green
20	Light Green

**NOTE:**  
The value of the Subsidence Contours, from the outside to the inside of the panels, are 20mm, 50mm, 100mm, 200mm, and so forth, in 100mm increments.  
The maximum subsidence is shown as mm



**LEGEND**

- Proposed LW709 to LW711 and LW905
- Existing Longwalls



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**ILLAWARRA METALLURGICAL COAL**  
PREDICTED TOTAL SUBSIDENCE CONTOURS DUE TO LW702-LW711 & LW901-LW905

DATE:	12 Apr 2021	DRAWING No:	MSEC1117-16	Rev No	A
SCALE:	as shown				

SCALE  
0m  
500m  
1000m