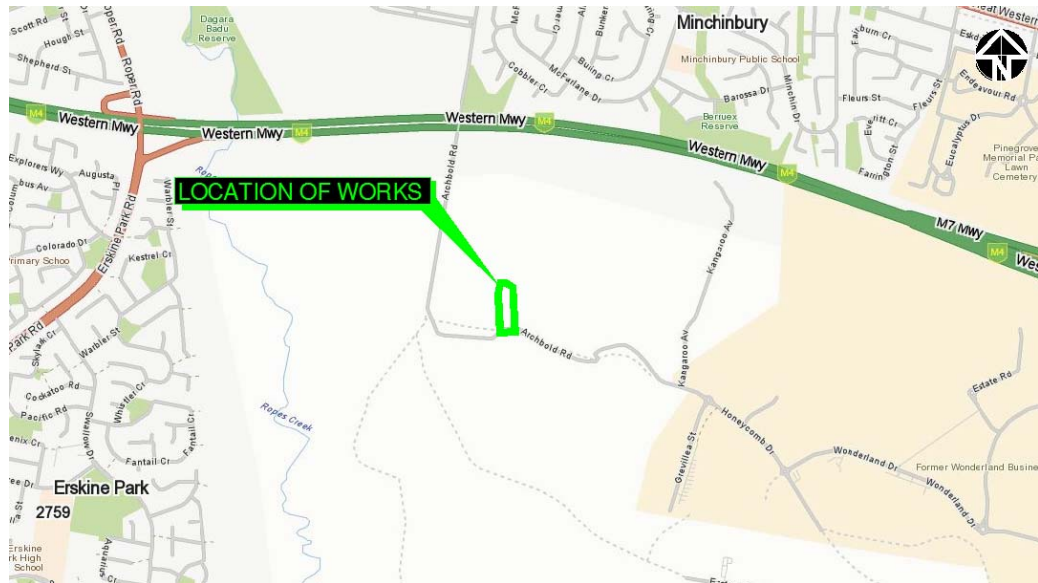


Pre-Sorting Centre, Eastern Creek



Section 75W Approval - Stormwater Management

Author: Frank Xie

Approver: Anthony McLandsborough



Report no: 14-232-5001-02

Revision 02

Date: May 2015

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Document registration

Document title	Section 75W Approval - Stormwater Management Report
Document file name	14-232-4001-01-S75W-Stormwater Management Report
Section	Civil Engineering
Document author	Frank Xie

Issue	Description	Date
01	Issue for Section 75W Approval	15/12/2014
02	Issue for Section 75W Approval	18/05/2015

Finalisation signatures

The design described in this report is considered to have been finalised.

Signature

Date

Frank Xie
Civil Engineer (Author)

15/12/14

Anthony McLandsborough
Director

15/12/14

Notes: The finalisation signatures shown above do not provide evidence of approval to the design. Approval signatures are shown on the title sheet of the design plans.

Civil Engineers & Project Managers

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1 Introduction

1.1 Project Description

The proposed development involves the construction of a Pre-Sorting Centre for Dial a Dump Industries Pty Ltd (DADI) in Eastern Creek approximately 36km west of the Sydney CBD.

The development involves the construction and operation of a Pre-Sorting Centre north of the Energy from Waste Plant (EWP).



Figure 1 - Site Location Plan

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2 INTRODUCTION

At&L have been engaged by DADI to under the Civil Design and Documentation for a Section 75W Application for the above mentioned Pre-Sorting Centre.

The purpose of this report is the reassess the existing drainage infrastructure capacity and determine of upgrade is required.

This report should be read in conjunction with the following AT&L Development Applications 14-187 drawings dated December 2014 (Appendix E):

DAC000	COVER SHEET AND LOCALITY PLAN
DAC001	NOTES AND LEGENDS
DAC003	BULK EARTHWORKS CUT AND FILL PLAN
DAC004	TYPICAL SECTIONS SHEET 1
DAC005	TYPICAL SECTIONS SHEET 2
DAC010	GENERAL ARRANGEMENT PLAN
DAC011	SITEWORKS AND STORMWATER DRAINAGE PLAN SHEET 1
DAC012	SITEWORKS AND STORMWATER DRAINAGE PLAN SHEET 2
DAC013	SITEWORKS AND STORMWATER DRAINAGE PLAN SHEET 3
DAC020	PAVEMENT PLAN
DAC021	EROSION AND SEDIMENTATION CONTROL
DAC030	SITEWORKS DETAILS
DAC031	DRAINAGE DETAILS

2.1 SCOPE

This report generally covers the following items:

- Assess existing Southern On-Site Detention (OSD) Basin Capacity.
- Assess existing Water Sensitive Urban Design (WSUD).
- Review and compare previous documentations to determine if proposed design was incorporated.
- Document the proposed onsite stormwater drainage design.

2.2 Previous Documents

Site Surface Water Management Plan

Prepare by Storm Consulting Pty Ltd, November 2008 - Appendix A

Volume Capacity Southern Basin

Prepared by G R Hawkes and Associates, 2010 – Appendix B

Consolidated Stormwater Management Plan

Prepared by Martens and Associates Pty Ltd, October 2011 - Appendix B

Updated Basin Survey consolidated with overall existing site model.

3 STORMWATER MANAGEMENT

3.1 The Site

The subject site is located south west of the existing Genesis MPC facility. The development has a total operational area of approximately 1.64 Ha which includes the warehouse and hardstand areas, with a landscaped batters approximately 1.1 Ha. The site is located within the Blacktown City Council Local Government area (LGA).

The site is currently undeveloped and is located north of the proposed EFW Plant. The site is located adjacent to a car park to the east and an access road to the north. The site is located within an earth mound which generally falls to the east towards the car parking. This is then drained into a swale to the south and flows to the west into the southern On-Site Detention Basin.

3.2 Council Requirements

As the site falls within the Blacktown City Council LGA the civil and stormwater design principles have been designed to comply with the BCC Engineering Guide for Development. The proposed site falls within Catchment 4 in the all other areas of the Hawkesbury river catchment.

A summary of Council requirements adopted is listed below:

Catchments 4 (All Other Hawkesbury River Sub-Catchments)	% impervious surface	
	100%	80%
Max. Permissible Site Discharge(l/s/ha)	147	56
Site Storage Requirements (m ³ /ha)	264	473

Table 1 - On Site Detention Requirements

- WSUD to achieve target reductions:
 - 85% Total Suspended Solids (TSS)
 - 65% Total Phosphorus (TP)
 - 45% Total Nitrogen (TN)
 - 90% Total Hydrocarbons
 - 90% Gross Pollutants (GP)
- Finished Floor Levels (FFL) to have minimum 300mm freeboard to 100 year overland flows.

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- In accordance with Blacktown City Council DCP Part R, rainwater tanks must be installed within the developed site with the aim to reduce the water demand for the development by a minimum of 80%. Rainwater tanks are an effective system to provide non-potable water for reuse for irrigation, toilets and other non-potable water uses. The rainwater tanks should be designed in accordance with *Rainwater Tank Design and Installation Handbook*, Australian Rainwater Industry Development Group, November 2008.

4 Existing OSD Basin Capacity

4.1 On-Site Detention (OSD)

There are two OSD basin (northern and southern) mentioned in previous reports; Site Surface Water Management Plan, prepare by Storm Consulting Pty Ltd, November 2008 (Appendix A) and Consolidated Stormwater Management Plan, prepared by Martens and Associates Pty Ltd, October 2011 (Appendix B). The focus of this report will be the southern basin, as the proposed development falls within this catchment.

The southern basin capacity table (derived from Land Partners Survey, April 2012).

Elevation (mAHD)	Description	Volume of Basin (m ³)	Volume of Storage (m ³)
58.48	Permanent Water Level	746	0
59.60	Top of Surcharge Pit	2369	1623
60.50	Top of Overflow Weir	3973	3227

Table 2 - On Site Detention Basin Capacity

Existing outlets details:

Orifice - 475 Diameter centre elevations of 59.337

Weir - Crest length of 10m at RL 60.5.

As mentioned within Section 3.2 it is a requirement of Blacktown City Council to have the OSD for this site be designed to comply with the Upper Parramatta River Catchment Trust (Catchment 4, others).

4.2 Results

As a result the following OSD parameter and conditions apply to this development as per Table 1:

- Maximum PSD (Permitted Site Discharge) = **147L/sec/Ha**
- SSR (Site Storage Requirement) = **264m³/Ha**

Given the total developed site is 4.75 Ha the following rates would apply:

Maximum PSD = $147 \times 4.75 = \mathbf{698 \text{ L/sec (for 100 year ARI event)}}$

And SSR = $264 \times 4.75 = \mathbf{1254m^3}$

Results

The existing basin has a maximum storage of 3227 m³ results of the drains model also indicate the following targets are achieved:

Duration	50 YR ARI		100 YR ARI	
	(m ³ /s)		(m ³ /s)	
	Existing	Developed	Existing	Developed
Peak	1.39	0.644	1.60	0.689

Table 2- Pre-Post Developed Flows (With OSD)

These results are very similar to the Martens' report in appendix B, as the developed catchment areas are lower. This is due to the EWP development to the south, as a portion of the previously calculated areas (Martens report) is directed to EWP drainage catchment.

5 Existing Water Quality

5.1.1 Policy and Guidelines

Water Sensitive Urban Design encompasses all aspects of urban water cycle management, including water supply, wastewater and stormwater management. WSUD is intended to minimise the impacts of development upon the water cycle and achieve more sustainable forms of urban development.

The stormwater design considers the following guidelines:

- Australian Rainfall Quality (2006)
- Department of Environment and Climate Change NSW (DECC), Management Urban Stormwater: Urban Design (Consultation Draft, 2008)
- Blacktown City Council Stormwater Quality Control Policy (2001, reviewed 2009)
- Landcom Water Sensitive Urban Design Policy (2009)

5.1.2 MUSIC Analysis

The MUSIC Model for Urban Stormwater Improvement Conceptualisation (MUSIC, Version 5.00.10) was used to evaluate pollutant loads from the developed site for Post-development (treated) conditions based on the proposed site development.

A conceptual view of the MUSIC model used in this report can be found in Appendix B.

5.1.3 Catchment Areas and MUSIC Parameters

Catchments used for nodes are outlined below:

Node Name	Description	Impervious (ha)	Pervious (ha)
West Landscape	Landscaped areas and Batters	0	3.88
East Landscape	Landscaped areas and Batters	0	1.05
Road Area - Existing	Existing Access Road and Parking Area	1.496	0
Roof Areas - Existing	Existing Sheds	0.131	0
Estate Road	Portion of estate road in separate approval.	0.435	0

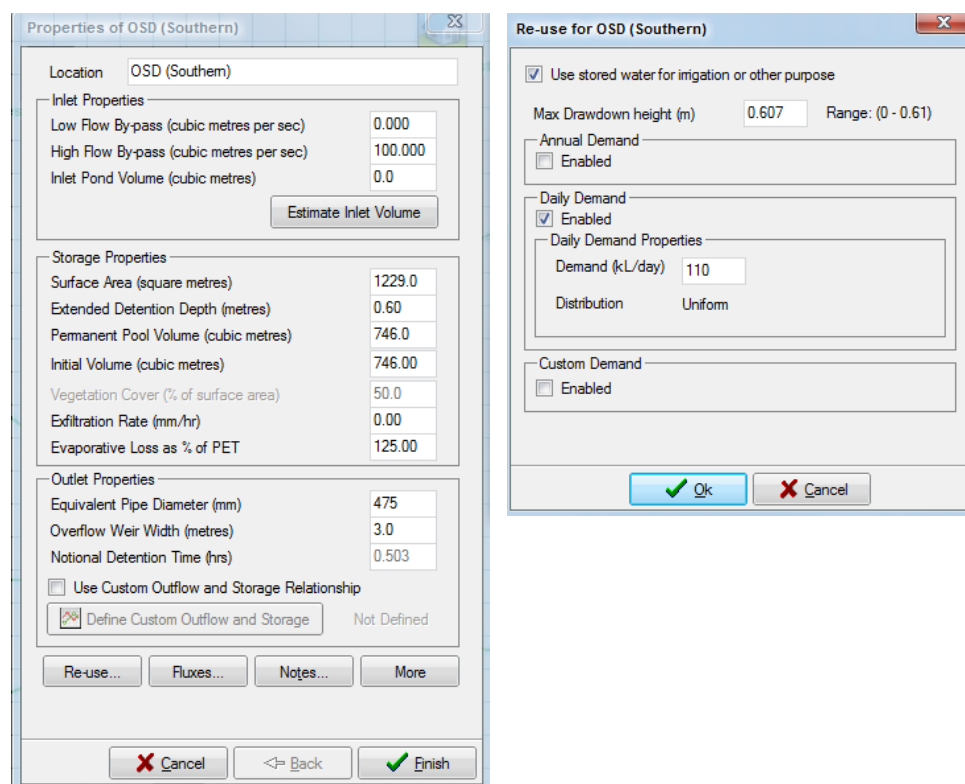
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Hardstand - New	Proposed Hardstand	0.692	0
Roof - New	Proposed Warehouse Roof	0.945	0

Table 4- Music Model Catchments

Southern OSD Basin

The Basin parameters are modelled as below:



Properties of OSD (Southern)

Location: OSD (Southern)

Inlet Properties

Low Flow By-pass (cubic metres per sec): 0.000
 High Flow By-pass (cubic metres per sec): 100.000
 Inlet Pond Volume (cubic metres): 0.0
 Estimate Inlet Volume

Storage Properties

Surface Area (square metres): 1229.0
 Extended Detention Depth (metres): 0.60
 Permanent Pool Volume (cubic metres): 746.0
 Initial Volume (cubic metres): 746.00
 Vegetation Cover (% of surface area): 50.0
 Exfiltration Rate (mm/hr): 0.00
 Evaporative Loss as % of PET: 125.00

Outlet Properties

Equivalent Pipe Diameter (mm): 475
 Overflow Weir Width (metres): 3.0
 Notional Detention Time (hrs): 0.503
☐ Use Custom Outflow and Storage Relationship
☒ Define Custom Outflow and Storage Not Defined

Re-use... Fluxes... Notes... More

Cancel Back Finish

Re-use for OSD (Southern)

☒ Use stored water for irrigation or other purpose

Max Drawdown height (m): 0.607 Range: (0 - 0.61)

Annual Demand

☐ Enabled

Daily Demand

☒ Enabled

Daily Demand Properties

Demand (kL/day): 110
 Distribution: Uniform

Custom Demand

☐ Enabled

Ok Cancel

The extended detention depth provided an additional 500m³ storage. This is for re-use during dry seasons. DADI confirmed average daily use as 110kL per day as noted in Martens stormwater report in Appendix B.

Rainwater Tank

Assumed size of 5kL tank are modelled, with minimal rainwater re-use for landscape.

Gross Pollutant Trap

The assumed GPT used in the MUSIC model was adopted from parameters for a "CDS 1015" GPT unit.

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5.2 Results

MUSIC modellings results presented as mean annual loads at the receiving node indicate that adopted target reductions are achieved. These results were similar to Martens' report in Appendix B.

The catchment sizes and reduction both differ to both "Storm Consultants" and "Martens". This is largely due to the Estate Road and the EWP proposal to the south. This is drained to a Bioretention swale/basin further south. Refer to Appendix D for Music catchment plans.

Treatment Train Effectiveness - Post-Development			
	Sources	Residual Load	% Reduction
Flow (ML/yr)	37.3	17.6	52.9
Total Suspended Solids (kg/yr)	5820	975	83.3
Total Phosphorus (kg/yr)	10.2	2.96	71
Total Nitrogen (kg/yr)	76.5	33.5	56.2
Gross Pollutants (kg/yr)	705	0	100

Table 5- Music Model Results

6 Conclusion

The results presented in this report demonstrates the OSD basin with extended detention (permanent) has enough capacity to satisfy both the detention and the water quality requirements.

This report has demonstrated that a storm water system consistent with good management practices can be provided for the proposed development.

Appendix A

Site Surface Water Management Plan
Storm Consulting Pty Ltd

Site Surface Water Management Plan

Report Prepared for:
Alexandria Landfill Pty Ltd

November 2008
Project No. 717

Prepared by:
STORM CONSULTING PTY LTD

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Document Verification

Project title	Light Horse SWA	ACN 080 852 231 ABN 73 080 852 231
Document title	Surface Water Management Report	Project number 717
Description	Light Horse Waste Facility – Surface Water Assessment	
Client Contact	Chris Biggs, DADI	

	Name	Signature	Issue:	Date
Prepared by	Georgia de Biasi / Nicola Lo		A	15/12/07
Checked by	Brad Farr / Nicola Lo		B	07/02/08
Issued by	Georgia de Biasi		C	27/3/08
Filename	717 Light Horse SW_21Nov08.doc		D	24/11/08

Document History

	Issue A		Issue B		Issue C		Issue D	
Issue to:	Date	No. Copies	Date	No. Copies	Date	No. Copies		
Jodi Kelehear (ERM)	15/12/07	PDF	7/2/08	PDF				
Rene Bakeri					27/3/08	PDF		
Chris Biggs (DADI)							24/11/08	PDF

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EXECUTIVE SUMMARY

This Surface Water Management report was prepared as part of the Environmental Assessment for the Light Horse Business Centre which is proposed to include the construction and operation of a resource recovery facility and a landfill facility. It was revised following a request for further information. The Project site is located at Eastern Creek in western Sydney and comprises four separate parcels of land, identified as Lot 2 DP 262213, Lot 1 DP 400697, Lot W DP 419612, and Lot 10, DP 241859. Storm has prepared a Surface Water Management Plan (SWMP) taking into account Blacktown City Council's stormwater management objectives and also the Director-General's Requirements for the Project.

The SWMP addresses erosion and sediment control, water quantity and water quality.

In summary, the proposed stormwater management system includes:

- ☞ Each building should have its own rainwater tank (min. 10kL volume) to harvest roof water for reuse on site, including for toilet flushing and wheel wash top up;
- ☞ Runoff generated from the Materials Processing Centre and green waste area are to be diverted to sewer and managed accordingly;
- ☞ Stormwater runoff from the other operational areas of the site will be treated through a gross pollutant trap prior to discharge to a combined on-site detention (OSD) basin with wetland treatment for water quality
- ☞ Additional volume is allowed in the OSD / treatment basin for irrigation water storage; drawdown would occur regularly for irrigation and dust suppression.
- ☞ The proposed OSD storage requirements for the operational area is 370m³/Ha (5500m³ based on 14.8ha impervious area) and has been designed to manage peak flows up to the 1 in 100yr ARI storm event.
- ☞ A sprinkler system is proposed to be located along the berms and utilised for both dust suppression and irrigation purposes.
- ☞ Stormwater runoff control within the quarry pit is to be used to assist in reducing leachate volumes. A collection basin is proposed which can be drawn down following storm events for reuse for dust suppression by water carts.

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Surface Water Management Plan

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Blue Book calculations

APPENDIX C

Model outcomes – RAFTS

APPENDIX D

Model – MUSIC layout

1.0 INTRODUCTION

1.1. Background

This is a revised report commissioned by Alexandria Landfill Pty Ltd to provide additional information to that supplied in STORM_CONSULTING's *Site Surface Water Management Plan* dated February 2008.

ThaQuarry Pty Ltd and ACN 114 843 453 Pty Ltd seek project approval for the construction and operation of resource recovery facility (including a materials processing centre (MPC) and waste transfer station (WTS)), and a Class 2 inert and solid waste landfill at Eastern Creek, in Sydney's west. Project approval is sought under Part 3A of the *Environmental Planning and Assessment Act, 1979*. The application process is to be managed on behalf of both parties by ThaQuarry Pty Ltd under the project name Light Horse Business Centre.

STORM_CONSULTING was commissioned by Environmental Resources Management Australia Pty Ltd (ERM) on behalf of the proponent to prepare a site Surface Water Management Plan for the Project, as part of the overall Environmental Assessment. This report has been revised to include additional information and amended data following a request for additional information from Blacktown City Council.

The site's location is shown in Figure 1-1. It is within the Blacktown City Council (BCC) Local Government Area. The Pioneer Quarry previously operated at the site. It has now reached the end of its economic life and all quarrying activities at the site ceased in September 2006, though the quarry void remains.

State Environmental Planning Policy No 59 – Central Western Sydney Economic and Employment Area (SEPP 59) applies to a number of landholdings in western Sydney, including the Project site, which lies within the Eastern Creek Precinct of the SEPP 59 lands.



Figure 1-1: Site Location

1.2. Development Overview

For the purposes of this assessment, the area of operations has been divided into two areas, termed the operational area and the quarry area. The operational area is depicted in Figure 1-2 and will be bounded by berms to the north, west and south, and by the quarry pit to the east. It will incorporate the following features:

- ☞ Access and internal roads;
- ☞ Car and truck parking areas;
- ☞ Administration and workshop buildings;
- ☞ Weighbridge and associated building;
- ☞ Materials Processing Centre (MPC) and Waste Transfer Station (WTS);
- ☞ Green waste processing/stockpile area; and
- ☞ Drop-off zone and materials stockpile areas.

The quarry area refers to the existing quarry pit, which is the main feature of the site. It is proposed to become a licensed class 2 inert and solid waste landfill.

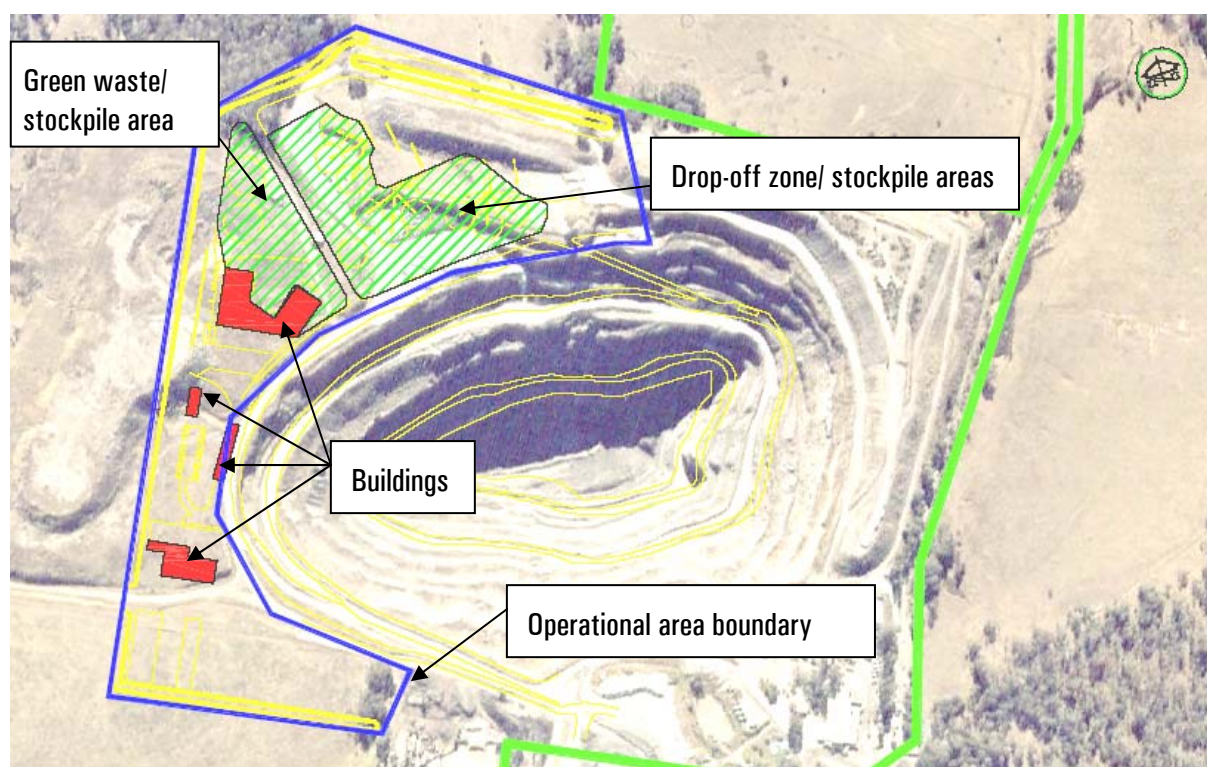


Figure 1-2. Site layout

1.3. Project Scope

STORM's scope of works for this surface water assessment report included:

- ☞ Preparation of site catchment plans and justification for any proposed redistribution between catchment areas, as defined in the Precinct Plan;
- ☞ Development of a concept stormwater drainage plan, including provision of water sensitive urban design (WSUD) elements where possible;
- ☞ Preparation of a soil and water management plan in accordance with Landcom's (2004) *Managing Urban Stormwater- Soils and Construction* – "The Blue Book";

- ☞ Stormwater detention calculations to determine relevant details of basins and drainage works;
- ☞ Water balance/ water management for wet, dry and average years, including water requirements (quantity, quality and sources) and proposed stormwater and wastewater disposal, including type, volumes, proposed treatment and management methods and reuse options;
- ☞ Identification of the quantity and physico-chemical properties of potential water pollutants and the risks they pose;
- ☞ Review of flood reports;
- ☞ Preparation of a Surface Water Assessment Report.

1.4. Planning Controls and Policy Objectives

1.4.1. Director-General's Requirements

The Director-General's (DG's) requirements issued on 22 June 2006 require a detailed assessment of specified key issues. In this report STORM addresses the DG's requirements for surface water, which are included in the Soil and Water category of Key Issues. These include:

- ☞ Surface water impacts;
- ☞ Stormwater management, including detailed consideration of any potential offsite drainage and flooding impacts;
- ☞ Erosion and sediment control;
- ☞ Salinity, in the context of site surface water management.

Other items identified in the DG's requirements (including groundwater, soil contamination, and other aspects of salinity at the site) are beyond the scope of this report and have been addressed in the Environmental Assessment Report prepared by ERM (2008).

Where necessary, STORM has consulted with Blacktown City Council with respect to regulatory requirements.

1.4.2. Eastern Creek Precinct Plan

The *Eastern Creek Precinct Plan – Stage 3* has been prepared in accordance with the provisions of SEPP 59 for land identified as Release Area 3 within the Eastern Creek Precinct (inclusive of the Project site). The draft Precinct Plan was adopted by Council on 7 December 2005, and came into force on 14 December 2005. It outlines the provisions relating to development of the Stage 3 Release Area, to ensure the SEPP aims are met.

This report aims to ensure the Project meets the relevant Environmental, Urban Amenity, Engineering and Economic objectives as set out in Section 5.5 of the Precinct Plan.

1.4.3. Other Relevant Documents

Other documents considered in the preparation of this report include:

- ☞ Institution of Engineers (2000) *Australian Rainfall and Runoff*;
- ☞ Blacktown City Council (2005) *Engineering Guide for Development*;
- ☞ Landcom (2004) *Managing Urban Stormwater: Soils and Construction Volume 1*, 4th Edition and Volume 2B – Waste Landfills (currently available as a draft for consultation only);
- ☞ Blacktown City Council (2005) *Stormwater Quality Control Policy P01100*;
- ☞ SMEC (2004) *SEPP59 Landholder Group Eastern Creek Precinct Plan Stormwater Management Strategy*.

2.0 SITE DESCRIPTION

2.1. Location and Land Use

The site (refer Figure 2-1) covers an area of approximately 122ha and comprises 4 lots:

- ☞ Lot 2, DP262213
- ☞ Lot 10, DP241856
- ☞ Lot 1, DP400697
- ☞ Lot W, DP419612

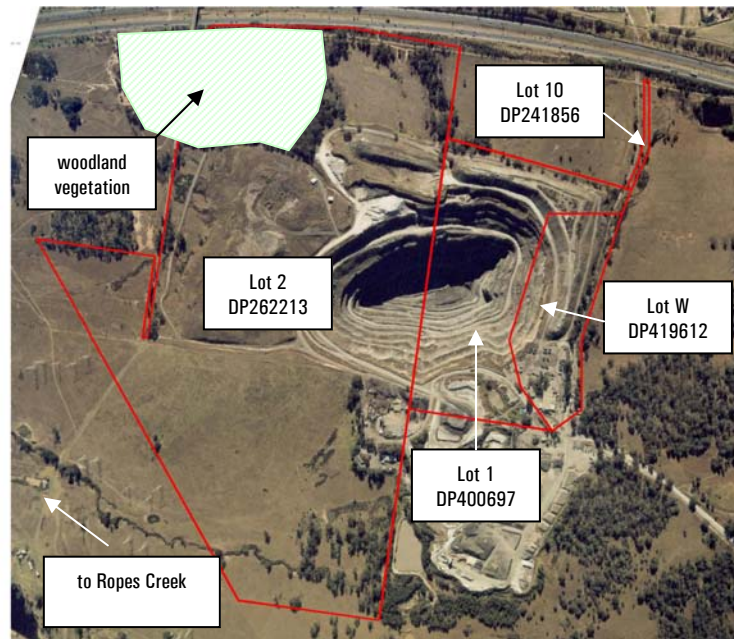


Figure 2-1: Site Boundaries

It is noted that the development footprint as assessed for this report will be restricted to the central portion of the site, as indicated on Figure 1-2.

The site is largely cleared of vegetation and is generally undeveloped beyond the existing quarry pit with associated overburden stockpiles. It is bounded by the M4 motorway to the north, a tributary of Ropes Creek to the south, Archbold Road to the west and open paddocks and the Hanson Asphalt Batching Plant and Hanson yard ('Hanson site') to the east.

In the area proposed for development, a low ridge divides the northern and north-western portions of the site. Native vegetation is largely limited to sparse trees in the north east, far south and west of the site, in addition to an area of remnant woodland in the north west of the site.

2.2. Catchments, Hydrology and Drainage

General overland flow direction across the site is to the north-west and ultimately reaches Ropes Creek approximately 1km west of the site. Ropes Creek flows northwards and is located along the western boundary of the Precinct with a total catchment area of approximately 127Ha. There is an ephemeral drainage line in the northern portion of the site that flows west towards Ropes Creek. To the south of the quarry and beyond the extents of the proposed site operations, overland drainage is generally south to south-west towards a tributary of Ropes Creek.

The site surface water drainage network is characterised by wide, flat and generally poorly defined drainage lines, which is fairly typical of drainage in western Sydney, where low topographic relief and meandering drainage lines dominate the natural landscape.

The Eastern Creek Precinct comprises nine major catchments as identified in Figure 10 of Council's Employment Lands Precinct Plan (2005), with the site located across four of the main catchments (refer Figure 2-2 and Table 2-1):

- ☞ Catchment 1: Quarry Catchment;
- ☞ Catchment 2: Quarry North Catchment;
- ☞ Catchment 3: Upper Angus Creek Catchment; and
- ☞ Catchment 6: Ropes Creek Tributary Catchment.

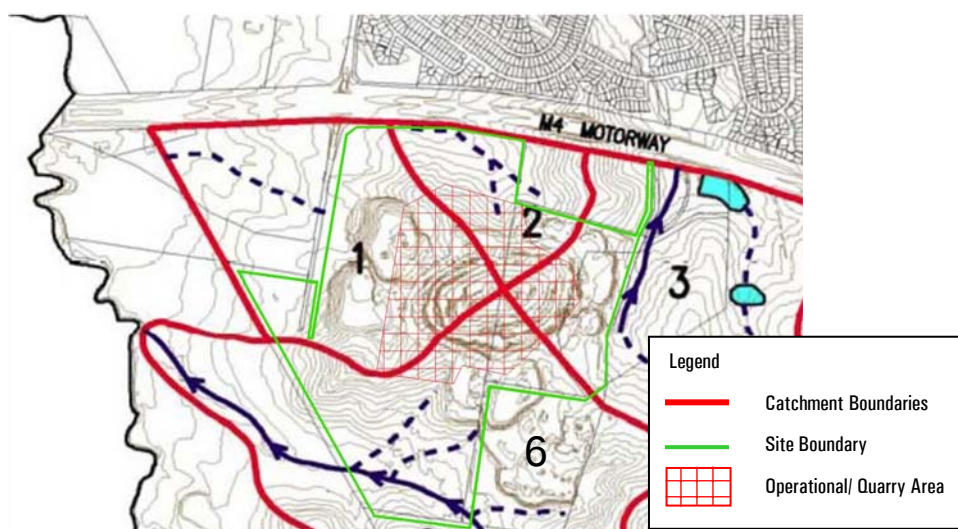


Figure 2-2: Catchment Areas (source: BCC, 2005)

Quarry Catchment (sub-catchment Ropes Creek)

The Quarry Catchment is located within the northwest corner of the Employment Lands Precinct, immediately south of the M4 Motorway. The total catchment area is approximately 72Ha, and drains to Ropes Creek via the existing contours on the site. A large portion of the current catchment runoff is reduced due to the presence of the quarry.

The site area that falls within the boundaries of the Quarry Catchment is approximately 41Ha (including part of the quarry).

Quarry North Catchment

The Quarry North Catchment is approximately 28Ha and is located immediately south of the M4 and east of the Quarry Catchment. The quarry void would intercept a large portion of the runoff from this catchment. Stormwater from this catchment drains through culverts located under the M4, to the area north of the M4.

The area of the Quarry North Catchment included within the site boundaries is approximately 19Ha.

Upper Angus Creek Catchment

The Upper Angus Creek Catchment is located in the northern section of the Precinct and has an area of 89Ha and drains northwards beneath the M4 Motorway.

The site area that lies in the Upper Angus Creek Catchment is 16Ha. It consists primarily of the quarry, whilst the remaining area is not subject to any development proposed under this DA.

Ropes Creek Tributary Catchment

Ropes Creek Tributary flows from east to west. There is a small farm dam located near the top of its catchment. Some signs of erosion are present near the dam.

The site area that falls within the Ropes Creek Tributary Catchment comprises quarry and undeveloped lands of approximately 44Ha.

Table 2-1: Site Catchment

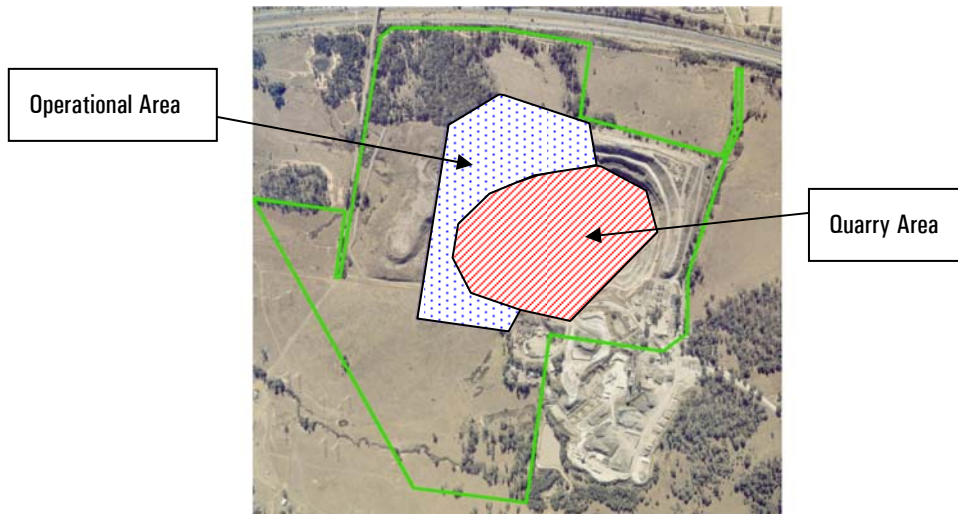
Catchment Number	Catchment Name	Site Area in Catchment (Ha)	Total Catchment Area (ha) ¹
1	Quarry Catchment	41	72
2	Quarry North Catchment	19	28
3	Upper Angus Creek Catchment	17.6	89
6	Ropes Creek Tributary Catchment	44	127
	TOTAL	121.6	316

¹ Source: Blacktown City Council Eastern Creek Precinct Plan 2005

For the purposes of modelling for this surface water assessment, the development area has been divided into two catchment areas. It has been assumed that the remaining site area will retain its current characteristics (pre-development state). The two developable areas comprise:

- 1) Operational area of the resource recovery facility (RRF), 14.8ha
- 2) Quarry area, approximately 26.5Ha.

Surface water flows from these developed areas will be managed to discharge towards the west to the Quarry catchment. This is discussed further in Section 3.5.



3.0 SURFACE WATER MANAGEMENT

3.1. Background

Part of the analysis required for successful development of the Resource Recovery Facility (RRF) and Landfill Facility includes planning of surface water management for the site. As water is both an input and output (waste product) of site activities, site planning needs to adopt an integrated approach to water management.

The key issues concerning site surface water management comprise:

- ☞ Segregation and management of 'clean' (water from operational areas) and 'dirty' runoff (i.e. leachate, or water that has come into contact with mixed wastes, green and timber wastes and uncovered landfilled wastes);
- ☞ Erosion and sediment control including protection of the drainage system from sediment influx;
- ☞ Quarry pit/haul road water management;
- ☞ Water quality control; and
- ☞ Provision of adequate on-site detention for the proposed operations.

Additionally, the *Precinct Plan* and *Engineering Guide to Development* require that pipe sizes be based on a 20 year ARI design flow and that the major drainage system be designed to safely convey the critical 100 year event under normal operating conditions.

Surface runoff generated on-site will fall into two categories

- 1) 'clean' (not leachate) – available for reuse (following roof water collection in rainwater tanks or runoff from clean operational areas which may require treatment for sediment only), and
- 2) 'dirty' (leachate) – generated from the base of the landfill, green waste areas and run off that has come into contact with mixed wastes, green and timber wastes and uncovered landfilled wastes.

Given the recent and impending changes to climate (including pronounced drought conditions), it is intended that the site remains as independent as possible of external water sources, and that the potential for off-site impacts to local receiving waters is minimised. The site layout for stormwater management is presented in Appendix A.

3.2. Soil and Water Management

Appendix A contains the Soil and Water Management Plan.

3.2.1. General

Site soil and water management will be required throughout the life of the project. The SWMP will adhere to the following principles:

- ☞ It is proposed to direct all operational area (hardstand clean) surface runoff (excluding water managed within the quarry pit) towards the Quarry catchment;
- ☞ Sediment-laden stormwater from the materials stockpile area will be directed through permanent sediment capture sumps or mini-basins along surface drainage to intercept sediment prior to reduce sediment 'slugs' reaching the GPT. Site grading is to be used to direct sediment-laden drainage away from hardstand areas;
- ☞ The MPC work floor and green waste area is to be diverted to sewer;

- ☞ Truck access to and from the unsealed areas are to be stable and designed to prevent influx of run-on and escape of untreated flows where possible;
- ☞ Runoff from site operational areas (as defined in Section 3.3) is to be directed through treatment devices (sediment traps and low-flow wetland treatment) and OSD for reuse prior to release to the site's drainage network. Overland flow paths for flows in excess of the design event are to follow natural drainage lines to the west of the site;
- ☞ Treatment devices around the site would provide sediment capture, gross pollutants where necessary, and must also be capable of capturing oil and fuel spills. Proprietary devices such as CDS, Humeceptor or similar can be selected and designed in consultation with the manufacturer to accommodate the required treatment;
- ☞ The treatment devices proposed for soil and water management are:
 - ☞ Small sediment sumps or mini-basins along swales to trap sediment 'slugs' if entrained in stormwater flow;
 - ☞ Sediment traps, e.g. proprietary gross pollutant trap (GPT) (CDS, Humeceptor or similar) or baffled settlement tank capable of retaining gross pollutants, sediment, oils and grease;
 - ☞ Within OSD basin: allowance for wet storage component, as a low-flow wetland for low-flow water quality treatment to remove fine suspended sediments as well as nutrients.
- ☞ Energy dissipation in the OSD basin settling basin for pre-treatment before entry to the OSD basin will provide further attenuation and capture of sediment that may reach the detention basin.

3.2.2. Stockpile and green waste areas

Sediment controls installed within the materials stockpile area will be maintained to prevent clogging and to prevent excessive sediment and nutrients entering the drainage system. These controls are to include:

- ☞ Small sediment sumps or mini-basins along swales to trap sediment 'slugs' if entrained in stormwater flow;
- ☞ Treatment through a GPT or baffled sediment settlement underground tank at the drainage outlet of these two areas,
- ☞ Protection of drains within these areas using:
 - vehicle exclusion,
 - stabilisation or lining of drains,
 - check-devices such as sediment sumps or mini-basins approximately every 50 metres to attenuate flows and encourage sediment dropout.
- ☞ Regular maintenance of drains and sediment traps to reduce loads within the system.

Runoff within the MPC work floor/ green waste collection area is to be managed as described in Section 3.2.

The green waste area, MPC floor, and materials stockpile area are graded away from the quarry to reduce the risk of overflows entering the quarry/landfill area.

3.3. Operational Areas

Surface runoff from the operational area will be managed separately from runoff generated in the quarry pit and haul road. Sources of stormwater runoff from the operational area include:

- ☞ Building roofs – workshop, MPC/ WTS, administration building and weighbridge shed – clean;
- ☞ Roads, car parks and other hardstand areas – clean, containing sediment;
- ☞ MPC work floor/ green waste stockpiles – dirty (to be directed to sewer);

☞ Materials stockpiles / drop off zones– clean, containing sediment.

Runoff collected from the clean or sediment-only areas will be reused on site, for uses including building internal uses (toilet flushing), wheel wash facility, dust suppression (*via* water carts) and irrigation/dust suppression from sprinkler systems around the site. (A water balance which utilises runoff generated on-site and estimated demands for the above uses have been developed and are discussed in Section 5.0.)

Drainage from the MPC work floor/green waste area is to remain in a ‘closed loop’ system with connections only to sewer. Drainage from this area is not considered in detail in this report.

Run off from other areas of the MPC/ WTS and stockpile/drop-off zones is considered to be “clean operational waters” but runoff from these areas will be subject to treatment (sediment removal) prior to reuse. Clean runoff from roofs will be collected in rainwater tanks for reuse on-site. Runoff from other parts of the operational area (e.g. roads, open areas away from stockpiles and buildings) will also be considered clean runoff and suitable for treatment and reuse on-site. This water may be directed to the OSD basin or storage tanks on-site (location to be determined), subject to satisfactory water quality.

Stormwater runoff will be conveyed by a combination of major and minor drainage systems, as shown in Appendix A, including:

- ☞ An underground piped system with provision for overland flow in swales and along roads;
- ☞ Stormwater detention and pollution control structures, and
- ☞ The natural drainage systems including creeks and overland flow.

BCC requirements are that piped networks are designed to convey 1 in 20 year flows without surcharge. Drainage overflows (greater than 1 in 100yr flows) from both these areas will be discharged away from the quarry pit via overland flow paths. Alternatively if required, overflows will be treated and sent to sewer (at an increased capacity if required).

Vehicle entry points for MPC work floor, green waste and materials stockpile / drop-off areas are to be located to minimise uncontrolled runoff and sediment release outside these areas.

Overland flow paths around the site are to remain stable in 100 year critical flows.

3.4. Quarry Pit / Landfill

The in pit haul road will be graded towards the quarry wall. The haul road will be graded towards the quarry wall and will follow a dish drain along the length of the road to a sediment basin proposed for the base of the quarry. Small check dams (e.g. sand bags or aggregate material approx 50mm diameter) located along the dish drain will assist in controlling flow velocities and erosion. At the base of the haul road, a temporary settlement sump is to be installed (e.g. concrete tank or temporary basin lined with geotextile and rock that can be moved as required) to slow down flows and to allow sediment to drop out prior to diversion to a clean area for pumping out (during initial 10m lift) or diversion to the in-pit basin (later stages of landfill management)

A storage basin will be required in the quarry pit to collect clean runoff from quarry walls, haul road and capped landfill areas. This basin is to be progressively relocated throughout landfilling, however no basin is proposed for the first 10m landfill lift.

Runoff collected from these areas will be suitable for reuse if it has not come into contact with waste, and it is expected that the water carts will be able to draw from the sediment basin and reuse this water for dust suppression on haul roads.

3.4.1. Quarry Pit Storage Basin

Managing Urban Stormwater: Soils and Construction, Vol 2B (Waste Landfills) (NSW DECC, Draft, 2007) acknowledges that stormwater from the areas of the landfill that have daily, intermediate or final cover applied can be directed to the sediment basin/s for treatment, rather than managing this water as leachate. Only stormwater which has come into contact with waste or other leachate needs to be managed as leachate, therefore water could be transported out of the pit basin for dust suppression, stockpile watering and similar on site activities within the site's operational area outside of the pit.

1) Initial Stage

The proposed stormwater basin in the quarry will not be placed at the first 10 metres of lift. During the initial stages where the first 10 metres of landfill lift is placed, stormwater influx to the landfilling areas is to be minimised using a sump and high rated pump to capture water from the sides of the quarry. All water falling on the landfill area itself is to be treated as leachate during the first 10m lift.

2) Later Stages

Due to the proposed landfill location being within the existing quarry pit, sediment control *per se* of the landfill area is not essential as the risk of environmental damage from sedimentation is low within the quarry pit itself. Rather, the primary aim of a collection basin within the quarry pit is to assist in controlling the volume of stormwater runoff that comes into contact with waste or the active landfill area (hence minimising leachate generation). Reuse of this water was also reviewed in a water balance model (Section 5.4.1) for its ability to meet demand for dust suppression, to maximise reuse potential.

Volume 2B of the 'Blue Book' for Waste Landfills (draft for consultation only) states that sediment basins and water storages should not be located on landfilled areas. However, the unavoidable constraint of being within the quarry pit, and the need to manage runoff effectively within the pit, necessitates the use of temporary stormwater controls and storage within the quarry pit.

The use of suitable grading and bunding and inclusion of a leachate trench to separate leachate from stormwater from capped areas within the landfill is also necessary to minimise surface water flows into active landfill areas. Erosion across capped areas and sediment influx into any temporary storage at capped areas must also be accommodated.

Forward planning for the location and size of the basin is important for effective runoff and sediment control. Its location should be determined at the development of each landfill lift, taking into account that a sealed basin area is necessary to prevent infiltration, and that it is not possible to excavate through capping and back into landfilled materials. Initial shaping or grading of capped/covered areas is necessary to allow for a suitable placement for the basin to create a catchment with a low point designed into the intermediate capped areas, to drain away from the active tip face / daily cover areas and allows placement of a liner for a basin without disturbing existing capped material.

3) Basin Sizing

Basin calculations were undertaken in accordance with the Blue Book for the quarry pit (26.5Ha).

The maximum total basin volume based on the total quarry pit footprint (including settling zone and sediment zone) that may be required is approximately 4,362.5m³ which equates to 165m³ per hectare of catchment area, which may include quarry walls that drain into the pit. Assumptions and spreadsheets used for sediment basin sizing including rainfall percentiles are presented in Appendix B and include the use of 5-day, 80th percentile rainfall and 2-month sediment accumulation.

Table 3-1 presents the basin data.

Table 3-1. Quarry pit basin information

Detail	Basin Option 1
Volume per hectare runoff capture (m ³ /ha)	165
Size (m ³) – for 26.5ha area quarry footprint	4,362
Rainfall – overflows downstream (landfill protection)	5-day, 80 th percentile (16.5mm)
Dust suppression uses - % demand met at full basin size	Refer Table 5-5

Sediment influx can be reduced by including a controlled, stabilised inlet to the basin and installing and maintaining effective erosion controls around the haul road outlet and around the boundary of the basin.

A series of basins may be installed to capture flows from sub-catchments of the quarry depending on available space within the quarry. The sub-basins will need to meet minimum storage requirements of 165m³/Ha of catchment draining to each basin.

Based on the basin sizing assumptions used, drawdown of water within the basin would need to occur within 5 days of a storm event occurring, to follow the basin design requirements and also to minimise the time that water is stored at the landfill area.

Water collected in the basin should be used initially for in-pit dust control or other uses requiring water in the pit area. Basin(s) may be drawn down by the water carts for dust suppression purposes or used in dump truck on-board reservoirs.

3.5. Flooding

A review of SMEC's *Eastern Creek Precinct Plan Stormwater Management Strategy (2004) – Appendix B Hydraulic Analyses* was undertaken to inform the flooding assessment. Within the site boundaries, there is only one distinct overland flow path identified, which is in the Quarry North catchment. Flows in other catchment areas are not affected by the proposed activities.

Peak flows from the site into the Quarry catchment drainage will be detained in OSD storage (refer to Section 4.0) to match pre-development flow levels. No drainage is proposed to be directed to the Quarry North catchment. No changes to the existing flooding regime are anticipated.

3.6. Proposed Changes to Catchment Drainage

The catchment boundaries in the Eastern Creek Precinct area as set out in BCC's Precinct Plan are based on old topographic boundaries which have been extensively modified since the construction of the quarry. Drainage in the remaining portion of the modified Quarry catchment (west) has been modified due to the presence of large overburden banks that act to redirect and prevent smaller flows from draining easily through the Quarry catchment area to Ropes Creek.

The change in catchment boundaries during site operations was assessed using site plans and proposed operational catchment areas. BCC has advised that all site flows may be directed west to the Quarry catchment. A RAFTS hydrological model was used to assess catchment flows as a result of the proposed drainage design at the site (see Chapter 4).

4.0 ON-SITE DETENTION (OSD)

4.1. Background

A series of regional detention basins are proposed in BCC's Eastern Creek *Precinct Plan*. One of these regional detention basins is proposed to be located at the site, within the Quarry North Catchment adjacent to the northern site boundary and the M4 Motorway. Another is proposed to the west of the site towards Ropes Creek.

Discussions with Council indicated that these regional basins were still subject to investigation therefore site basin(s) would be required for any proposed development in the interim.

This section presents the results of site specific OSD modelling.

4.2. Methodology

An XP-RAFTS computer model was generated to replicate pre- and post-development flows for the operational area which is subject to change in land-use following construction for the proposed operational area, to calculate OSD volume requirements. This was based on the assumption that the remaining site area will not change form or characteristics from the pre-development situation, and hence, any flows generated in these areas will remain the same as for the pre-development scenario.

Council guidelines require post-development peak flows to match pre-development peak flows up to the 100yr storm events. The model was run for the 2 year and 100 year ARI storm event to derive the required OSD volumes.

XP-RAFTS software allows the user to optimise OSD volume requirements with the use of a storage node receiving flows from the subject catchment. A two-stage discharge (2yr and 100yr) was modelled to check preliminary discharge calculations for peak flow hydrographs.

4.3. Assumptions

The operational area (including berms) was modelled in XP-RAFTS and incorporated an area of 14.8Ha. The operational area was divided into two separate catchments to reduce the total anticipated basin size. Basin 1 catchment is the northern section of the operational area with a modelled area of 10.03ha. Basin 2 catchment occupies the southern section of the operational area with a modelled area of 4.74ha.

The catchments were considered to be 100% pervious in the pre-development model and 100% impervious post-development. These assumptions would result in conservative estimates for flow and OSD storage requirements.

Other XP-RAFTS modelling assumptions are documented in Table 4-1.

Table 4-1: RAFTS modelling criteria for on-site detention determination

Parameter	Pre-development	Post-development
Initial Loss/Continuing Loss (assumes wet antecedent conditions and is a conservative approach)	15mm/3mm	5mm/1mm
Roughness value across site	0.04	0.02
Proportion impervious (%)	0	100

4.4. Results

Peak flows from the site operational areas were calculated using RAFTS for the predevelopment and post-development scenarios. This was used to calculate the required OSD storage volume to prevent downstream hydraulic impacts as a result of site development and allow matching of pre- and post-development flows off site. Table 4.4 shows the results of peak flow modelling.

Table 4-2: Results for OSD modelling

Catchment	ARI	Pre-development Peak Flows (m ³ /s)	Post-development Peak Flows without detention (m ³ /s)	Post-development Peak Flows with detention (m ³ /s)	Required OSD Storage Volume (m ³)
Basin 1 Catchment	100 yr	1.156	5.277	1.110	3,900
	2yr	0.416	2.643	0.403	
Basin 2 Catchment	100yr	0.605	2.511	0.621	1,600
	2yr	0.215	1.249	0.206	

Staged discharge was initially determined using the orifice equation to estimate an orifice outlet diameter, then modelled in several iterations to ensure that pre-development and post-development flows and hydrographs were as close as possible for the 2 year and 100 year ARI. Charts showing pre- and post-development hydrographs and basin hydrographs are presented in Appendix C.

4.5. Discussion

Based on the OSD modelling results presented in Table 4-2, an OSD basin storage volume of 5,500m³ is required for the proposed operational area. The quarry area itself will not require detention storage and following final completion and capping of the landfill, drainage from the area is to be diverted around detention storages. In the event that there is a change in impervious area, an OSD volume of 370m³/Ha may be adopted based on the modelling in this report.

4.6. Dam Safety Committee requirements

The New South Wales Dam Safety Committee (DSC) *Risk Management Policy Framework for Dam Safety* (2006) was reviewed for requirements and criteria for risk assessment.

Among other goals, the DSC states that its mission is to develop and implement effective policies and procedures for regulation of dam safety. In general, dam safety is initially determined through a risk assessment that uses the probability of failure per dam in one year (with probabilities ranging from 10^{-7} to 10^{-3}) and the number of fatalities that would occur as a result of dam failure. An appropriate dam safety assessment would need to be undertaken at the relevant detailed design stage for the OSD basin.

For this site, the proposed OSD basin sizes are 3,900m³ and 1,600m³, smaller than several of the existing dams at the Eastern Creek Precinct. Generally basins will be constructed so that maximum water levels will be at most 1 metre above existing downstream ground levels, overland flow travels across rural land towards Ropes Creek.

Flows from either basin could be classed as “slow and shallow” in relation to overland flow paths, non-defined drainage lines allowing flow dispersion, and relatively long overland flow paths over un-occupied land to the nearest defined drainage line.

Moreover, STORM notes that the intended construction of a much larger regional detention basin in the vicinity of the proposed OSD basins. The larger OSD basin may present higher risks than that proposed for this site for the operations phase of the development, and will also require scrutiny particularly as the structure is intended to be in place through the long term.

In a Probable Maximum Flood the dam will have already overtopped from a smaller 1:100 event as part of its design. In a PMF event, the volume of catchment flows from further up the catchment beyond the site are likely to be having a greater impact at this point in the catchment, in which the contribution of any (unlikely) dam failure would be negligible.

As a result, these factors are likely to contribute to a negligible risk.

5.0 WATER CYCLE MANAGEMENT

5.1. Water Balance Methodology and Concept

A daily water balance analysis was used to determine the feasibility of the proposed rain and stormwater harvesting scheme and in particular the effects of various storage sizes for stormwater harvesting along with changes to demand. The water balance utilised flows generated using a simple runoff calculation using historical rainfall data, analysed for various rainfall patterns including dry, mean and wet rainfall years.

The purpose for modelling dry, mean and wet years was to assess the performance of various tank sizes given the changes to rainfall patterns. It is noted that with the potential effects of climate change and the current trend of dry rainfall patterns, the need to consider lower annual rainfalls for rain and stormwater harvesting reuse schemes is becoming more and more necessary. In addition, any excess stormwater produced (especially during wet season periods) need to be considered for the management of on-site surface waters.

A concept diagram for the proposed re-use scheme on site is shown in Figure 5-1 below.

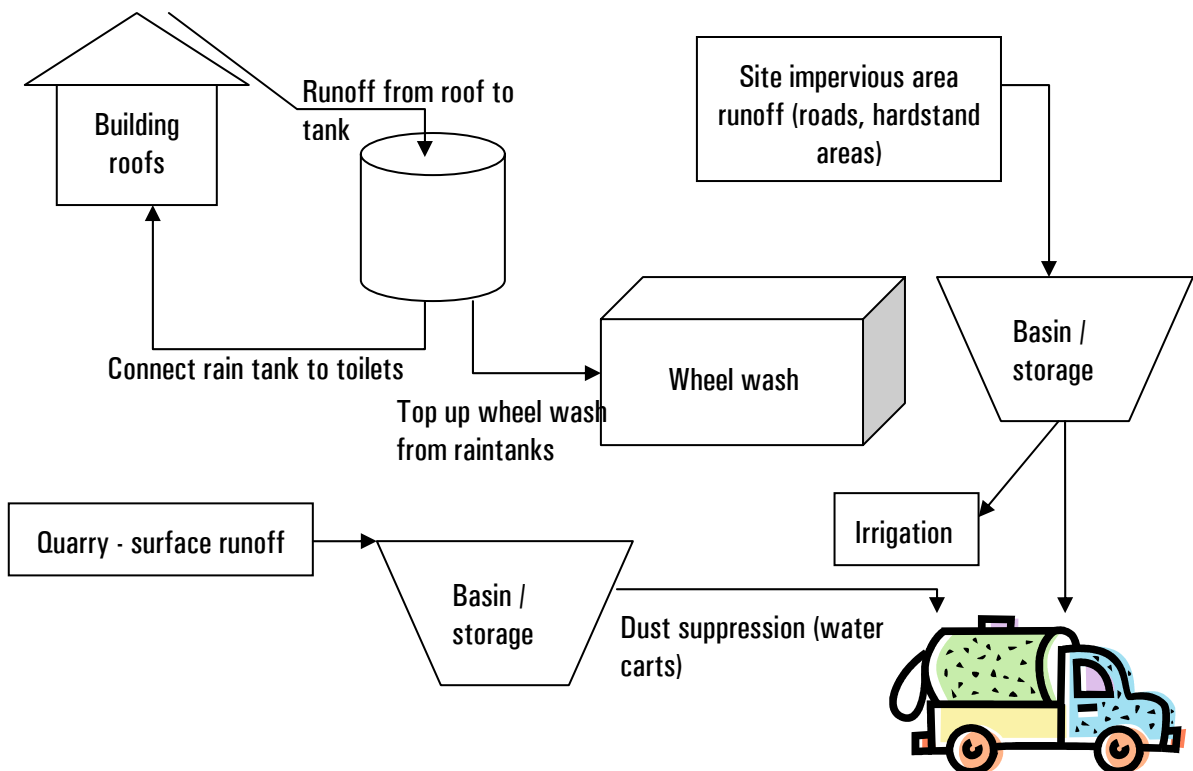


Figure 5-1: On-site water reuse concept

5.2. Modelling Inputs

5.2.1. Rainfall

Data from St Clair (BOM station #67102) was used in this analysis. Seventeen years of daily rainfall data (1985 – 2002) was assessed to determine a dry, median and wet rainfall sequence for use in the water balance model.

The following dry, median and wet year rainfalls were derived, and compared against long term averages for Prospect.

Table 5-1: Rainfall Records

Annual Rainfall (mm)		
	Modelled average rainfall and years	Prospect (long term average)
Dry	553 (1994 / 1995 / 2001/ 2002)	562
Median	851 (1987 / 1989 / 1991)	831
Wet	1104 (1986 / 1987 / 1988 / 1989 / 1990)	1183

Note: median rainfall at St Clair is below Sydney Observatory's average of 1162mm/yr.

5.2.2. Harvestable areas

The proposed roof and stormwater reuse scheme can harvest runoff from the operational area catchment, comprising the areas identified in Table 5-2. This is conservative (under-estimates area available) and excludes the proposed green waste area.

Table 5-2: Harvestable Areas

Precinct	Area (Ha)	Initial loss (mm)
Building roofs	0.6	1
Remaining Site Operational Area	13.1	5
Quarry	26.5	10

5.2.3. Water demands

The demands for harvested water for reuse includes toilet flushing, dust suppression, sprinklers (irrigation) and the wheel wash. Estimated water demands used in the water balance model are presented in Table 5-3 below.

Table 5-3: Modelled Demands

	Annual Demand (ML/yr)			Modelling Assumptions
	Dry years	Mean years	Wet years	
Toilets	0.4	0.4	0.4	34 staff on-site x 6 flushes/day x 4.5L/flush
Dust suppression	25.8	24.1	24.0	Average application = 80kL/day (assumes no application if daily rainfall exceeds 2mm)
Sprinklers (irrigation)	9.7	9.1	9.0	Average application = 30kL/day (assumes no application if daily rainfall exceeds 2mm)
Wheel wash	0.3	0.3	0.3	Water use = 25kL/month
TOTAL	36.2	33.9	33.7	

5.3. Results - Catchment runoff

Based on harvesting stormwater from 13.1ha operational catchment area, calculations undertaken by STORM for a dry, median and wet year sequence provide the runoff volumes shown in Table 5-4. The actual runoff that can be harvested for reuse will not be the entire volume generated due to losses from the system from overflows, and is dependent on storage behaviour (i.e. if the storage volume reaches 100% capacity, overflows will occur rather than further collection). The performance of varying storage volumes is presented in Section 5.4.

Table 5-4: Potential Runoff Generation

Rainfall Scenario	Potential Runoff Generated (ML/yr)		
	Dry	Median	Wet
Building Roofs	3.0	4.7	6.2
Quarry	39.1	71.2	124.9
Remaining Site Operational Area	44.9	73.2	236.8
TOTAL	87.0	149.1	367.9

5.4. Storage sizing

The water balance model was set up to determine the amount of runoff generated from the catchment under the various rainfall scenarios, with the aim of assessing the performance of various storage sizes.

5.4.1. Raintanks and building roofs

Figure 5-2 demonstrates the results of capturing roof runoff from buildings and reusing it for internal uses (toilet flushing i.e. 0.9kL/day) and topping up of the wheel wash facility (1kL/day).

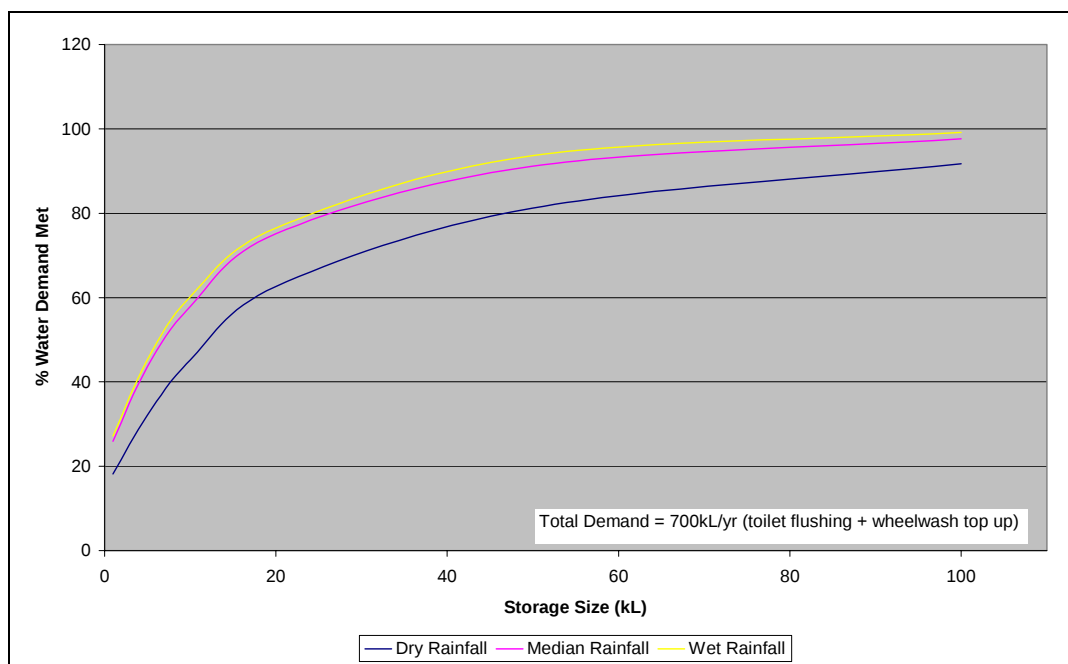


Figure 5-2: Roofwater reuse for toilet flushing + wheelwash

Based on Figure 5-2, overall tank storage volumes of up to 40kL would meet over 75% of the site's toilet flushing and wheel wash demands for the dry, median and wet rainfall scenarios e.g. a 40kL storage volume would meet 88% of the 700kL demand under median rainfall conditions. It is recommended that each of the four buildings on-site install a 10kL tank (minimum) to maximise potential roof runoff collection for reuse.

5.4.2. Surface runoff from operational area

There is opportunity to collect surface runoff from the internal roads/hardstand areas and remaining site operational area. Runoff from these areas may be directed towards the OSD basins which are proposed to include a storage component and be drawn down for reuse on site following storm events. A water balance was prepared for the water demand scenario of:

- ☞ Dust suppression for watering carts + truck on-board reservoirs (40kL/day) and spray mists / sprinkler system for irrigation or dust suppression (30kL/day).

Note: it is assumed that the water quality will be of adequate standard for reuse and will not pose a risk to human or environmental health.

It was assumed that on days where daily rainfall exceeds 2mm there is no demand for dust suppression.

A range of reuse storage volumes (within the OSD basin, as additional storage to OSD volume) under dry, median and wet rainfall scenarios were modelled.

Figure 5-3 shows the volume of rainwater supplied for a range of storage volumes under a dry, median and wet rainfall scenario. Figure 5-4 shows potential water supply and percent water demand met for dust suppression and sprinkler irrigation on site. As storage volume increases, the ability of the storage supply to meet demand will increase.

Current indicative basin size in the site drawings (Appendices A & B) allows for approximately 1000kL from Basins 1 and 2 combined, which would meet approximately 55% of the assumed water demand for dust suppression and irrigation combined. If required the storage volume could be increased at the detailed design stage.

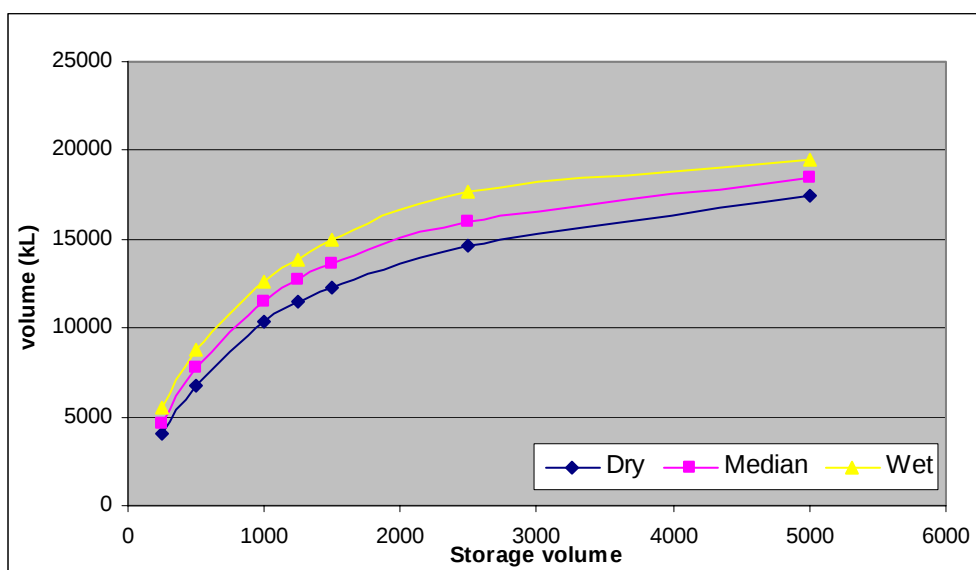


Figure 5-3. Rainwater volume supplied based on storage volume, kL/yr

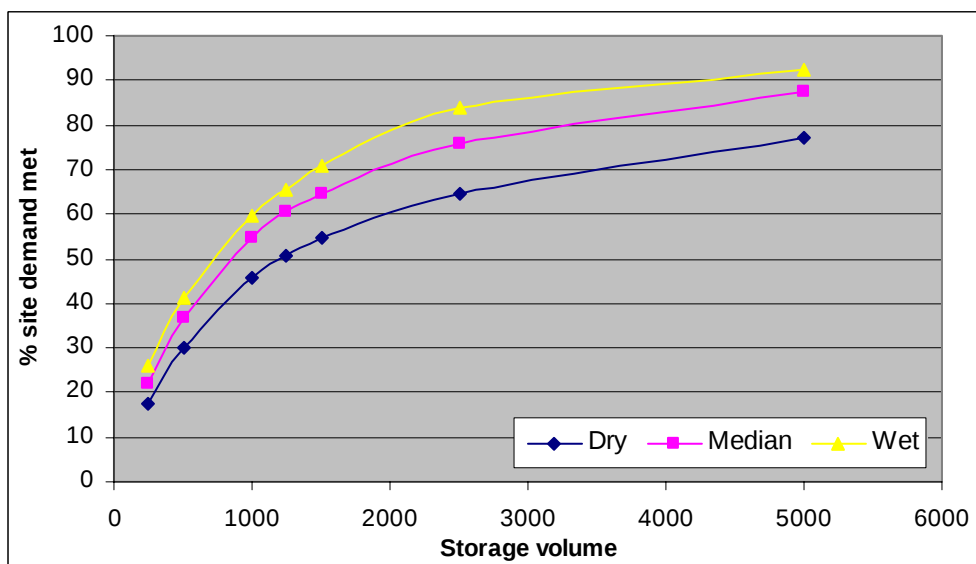


Figure 5-4. Percent water demand met annually, dust suppression and irrigation only

5.4.3. Surface runoff from quarry

Captured runoff in the quarry basin will be used for dust suppression via water carts. The available water volume for reuse from the basin will vary depending on rainfall and the stage of landfill operation, as the basin size is intended to increase in proportion to the capped landfill catchment area and runoff from quarry walls as required.

The modelled volume of reuse for dust suppression per day was 40kL/day.

Table 5-5 shows the per cent demand met from a basin sized to capture runoff from the 26.5ha quarry area. In practice the basin size may vary in relation to the area of capped landfill that is its catchment (at a rate of 165m³/ha). For this reason it was modelled separately to the storage options within the OSD basin.

Runoff collected from these areas will be suitable for reuse such as dust suppression if it has not come into contact with waste.

Table 5-5. Quarry basin reuse - % demand met

Rainfall scenario	Basin (4,362m ³)	
	Total water demand ML/yr	% water demand met
Dry	12.97	72%
Median	12.07	87%
Wet	12.13	91%

5.5. Summary of Storage Volumes

The following recommended storage volumes are based on the analysis above:

- ☞ Each building should have its own rainwater tank (min. 10kL volume) to harvest roof water runoff for reuse including toilet flushing and wheel wash top up;
- ☞ The OSD storage proposed for the operational area is of sufficient volume (min. 370m³/Ha) to contain the 1 in 2 year storm event 1 in 100yr storm event and by use of additional depth in the basin (nominal 0.5m in indicative basin sizes supplied) to act as storage for reuse on-site. It is anticipated that drawdown will occur regularly for dust suppression (water carts and sprinkler) and irrigation.
- ☞ The proposed sediment basin in the quarry has been sized using the Blue Book (approx. 165m³/Ha) and can be drawn down following storm events for dust suppression (water carts).

6.0 WATER QUALITY

6.1. Water Quality Management

The stormwater management controls for the site including water quality management measures are presented in Appendix A.

6.1.1. Pollutant Treatment Priorities

Table 1 in BCC's Stormwater Quality Control Policy (2005) presents treatment priorities for a range of pollutants generated from various land uses. The proposed development is deemed industrial and as such the pollutant treatment priorities are identified in Table 6-1, based on Table 1 in the Policy. The Policy also states that for developments on sites greater than 5Ha, the pollution treatment methods selected must treat all pollutants cited with emphasis on the first three priority pollutants.

Table 6-1 Pollutant Treatment Priorities for Industrial Areas

Development Type	Litter (Gross Pollutants)	Coarse Sediment	Nutrients	Fine Sediment	Hydrocarbons, Motor Spirit, Oil & Grease
Industrial	3	4	5	1	2

Table 6-2 outlines the pollutant retention criteria for development sites, based on Table 2 in BCC's (2005) Stormwater Quality Control Policy. MUSIC modelling (refer to Section 6.2) was undertaken to assess the effectiveness of the proposed treatment system based on the information in this table.

Table 6-2 Pollutant Retention Criteria

Pollutant	Description	Retention Criteria for Development Sites
Fine Sediment	Contaminant particles 0.1mm diameter or less	50% of the total annual load
Hydrocarbons, Motor Spirit, Oil & Grease		Whichever is greater: 1. 90% of the total annual load; or 2. Total discharge from site of TPH ¹ < 10mg/L at all times.
Litter (gross pollutants)	Trash litter and vegetation larger than 5mm	90% of the total annual load
Coarse sediment	Contaminant particles between 0.1mm and 5mm diameter	80% of the total annual load
Nutrients	Total phosphorus and total nitrogen	45% of the total annual load for each nutrient
Notes: 1. TPH – Total petroleum hydrocarbons		

Table 6-3 is based on Table 3 in BCC's (2005) Stormwater Quality Control Policy and outlines the qualitative operational objectives for new developments, and how the proposed Stormwater Management Plan meets these objectives.

Table 6-3. Water Quality Management Objectives

Pollutant/Issue	Management Objectives	SMP addresses objectives
1. Runoff volumes and flow rates	Impervious areas are not to be directly connected to the stormwater drainage system unless uncontrolled property runoff needs to be constrained	OSD to be utilized to address runoff from site developed areas including road drainage and other paved areas.
2. Stormwater quality	Reuse of stormwater for non-potable uses maximised	Yes (addressed in Section 5.0)
	Vegetated flow paths or similar are to be used to connect impervious areas to the stormwater system	A vegetated wetland (end of line system) in each OSD basin will be used to treat stormwater runoff prior to discharge to the environment. Where feasible in detailed design, rock-lined or grass swales adjacent to berms will direct site operational area runoff to treatment/OSD basin. Sediment/Gross pollutant traps and low-flow treatment through wetland (as wet storage component of OSD basin) to be utilized for operational areas
	Use of stormwater infiltration 'at source' where soil types allow.	Infiltration will occur for smaller storm events ARIs. Soil types on site (heavy clays) inhibit use of infiltration for larger ARIs. Site use not conducive to stormwater infiltration as WQ control.
3. Riparian vegetation and aquatic habitat	Protect and maintain (i.e. no demonstrated adverse impact on) natural drainage features ¹. All natural (or modified) drainage channels within the site that possess either: • base flow • defined bed and/or banks • locally occurring native riparian vegetation are to be protected and maintained.	Drainage paths within site catchments are poorly defined with no base flow. Nil to very little native riparian vegetation is present at drainage paths within site. There are no modifications proposed for existing riparian vegetation and aquatic habitat at drainage paths in the Quarry North

	'Natural channel designs' should be adopted in lieu of floodways in areas where there is no natural (or unmodified) channel.	and Quarry site catchments
4. Flow	Natural flow paths, discharge points and runoff volumes from the site should not be altered. Frequency of bank-full flows should not increase as a result of development. Generally, no increase in the 2-yr and 100-yr ARI peak flows.	Key discharge points for site are to be maintained or will remain unaffected by site development. OSD is proposed to maintain peak discharges at pre-development levels. Staged discharge modelling undertaken for 2yr & 100 yr events
5. Amenity	Multiple uses of stormwater facilities to the degree compatible with other management objectives.	OSD aligns with requirements for onsite detention in Eastern Creek Precinct. Site OSD needs could be integrated with regional detention basin in future following assessment. No clashes with other management objectives Stormwater harvesting and reuse to reduce potable water demands and peak runoff volumes.
6. Natural bushland	No demonstrated adverse impact from stormwater into urban bushland area.	No stormwater to be directed to new discharge points including bushland areas. Stormwater from site operational areas is directed through sediment trap, wetland and OSD prior to release at existing discharge points.
Notes: 1. Wetlands, watercourses and riparian corridors.		

6.1.2. Maintenance and Monitoring

The developer will be responsible for the maintenance of the proposed stormwater controls. A maintenance plan will be developed during the detailed design phase. In general, the maintenance plan should allow for:

- ☞ Regular visual inspection of the stormwater treatment measures, for example on a monthly basis and after rain events;
- ☞ OSD Basin and GPT cleaning program – more frequently as site settles from development and then based on results of regular visual inspections. Cleaning generally to consist of:
 - ☞ Sediment and weed removal from the OSD basin and its associated sediment control/stilling basin, and
 - ☞ checking integrity of in-pit stormwater basin, plus sediment removal

- ☞ (Optional) water sampling of OSD basin and in-pit stormwater collection basin, e.g. on a quarterly basis for the first year of the basin's operation as each basin is developed, then 6-monthly in following years, to ensure reused/released water is of suitable quality for end-use, e.g. in irrigation equipment (if necessary can refer to ANZECC guidelines and relevant NSW guidance;

A maintenance and monitoring check-sheet should be developed that allows for the data entry, location of stormwater management device on site (e.g. based on a map with numbered locations), type of inspection (visual, water sampling, etc), outcome (e.g. all clear, device needs cleaning), actions taken, and any follow up required.

Site salinity management with reference to water collected within the quarry pit is addressed in Section 6.3.

The quality of the water released (if any) should be in accordance with the site's Environment Protection Licence. Typically the licence will only permit discharge once the water in storage has been tested to ensure it complies with specified water quality standards for discharge. Sampling requirements may include total suspended solids (TSS), Electrical conductivity, Turbidity, Ammonia, Biochemical Oxygen Demand (BOD), Total Nitrogen and Total Phosphorus.

6.2. MUSIC modelling

6.2.1. Methodology

The MUSIC model was chosen to model water quality. This model has been released by the Cooperative Research Centre for Catchment Hydrology (CRCCH) and is a standard industry model for this purpose. MUSIC (the Model for Urban Stormwater Improvement Conceptualisation) is suitable for simulating catchment areas of up to 100 km² and utilises a continuous simulation approach to model water quality.

By simulating the performance of stormwater management systems, MUSIC can be used to determine if these proposed systems and changes to land use are appropriate for their catchments and are capable of meeting specified water quality objectives (CRC 2002). The water quality constituents modelled in MUSIC and of relevance to this report include Total Suspended Solids (TSS), Total Phosphorus (TP) and Total Nitrogen (TN).

Only the site operational area and quarry area were modelled as these undergo the greatest change in land use. The post-development model was used to compare the pollutant loads generated from the proposed development with, and without treatment controls.

The pollutant retention criteria set out in BCC's Stormwater Policy were used as a basis for assessing the effectiveness of the selected treatment trains.

The layout of the MUSIC model is presented in Appendix D.

6.2.2. Assumptions

Rainfall

Rainfall data was obtained from the Bureau of Meteorology (BOM) for Prospect Dam (BOM station #67109) the closest station to the site with continuous rainfall data. Meteorological data from 1984 - 2004 (slightly above average rainfall conditions) was used in the model in an attempt to replicate climatic conditions typical of the site.

The MUSIC User Manual (CRCCH, 2004) suggests that the time-step should not be greater than the time of concentration of the smallest sub catchment, but consideration should also be given to the smallest detention time of treatment nodes in the system. To accurately model the performance of the treatment nodes, a daily time step was chosen.

Soil Properties

Various parameters are required to be entered in MUSIC regarding soil properties. The soil profile of the existing site is fairly uniform and soil parameters were set to default Sydney values throughout the modelling.

Event Mean Concentrations

The default MUSIC Event Mean Concentration (EMC) values have been adjusted to reflect more recent data available by Duncan (2004) for specific land uses such as roads, roofs and urban areas. The parameter concentrations adopted are shown in Table 6-4.

Table 6-4: MUSIC Modelling EMCs

Land Use ¹	EMC (mg/L)		
	TSS	TP	TN
Roofs	20	0.13	2
Other site areas	270	0.5	2.2

¹ Fletcher, T., Duncan, H., Poelsma, P. & Lloyd, S. (CRC, 2004)

Proposed Treatment Method

Treatment measures modelled include gross pollutant traps, treatment through a wetland system and inclusion of stormwater runoff reuse.

6.2.3. Results

The results of the post-development model are shown in Table 6-5. The reduction rate is expressed as a percentage and compares the post-development pollutant loads **without** treatment versus post-development loads **with** treatment. When a positive reduction percentage is achieved there is a net decrease in pollutant loads as a result of development. The development can then be considered to have a beneficial effect. However, if a negative reduction percentage occurs then there is an increase in pollutant loads in that particular post-development scenario.

Table 6-5: Flow and Pollutant Load Reductions

Parameter	A Post-Development Results (without treatment controls)	B Post-Development Results (with treatment controls)	Reduction % (A-B)/A
Flow (ML/yr)	64.80	42.7	34.1
Total Suspended Solids (kg/yr)	15600.00	770	95.1
Total Phosphorus (kg/yr)	29.70	6.33	78.7

Total Nitrogen (kg/yr)	141.00	69.4	50.8
Gross Pollutants (kg/yr)	2100.00	0	100

The model results (Table 6-5) indicate that pollutant load reductions for Total Suspended Solids, Total Phosphorus and Total Nitrogen will meet BCC's Stormwater Quality Control Policy (2005).

The detention storage for the proposed wetland can be wholly contained within the basin in addition to OSD. Site drawings in Appendix A show the indicative cross section for OSD-wetland configuration with provision for storage and reuse.

6.2.4. Discussion

Based on the water quality modelling undertaken, measures including gross pollutant traps, treatment through a wetland system and stormwater runoff reuse will enable stormwater discharged from the site to be treated to a standard that meets water quality objectives as set out by Council.

It is considered that other pollutants such as hydrocarbons are not expected to cause any significant impacts on site under every day operations. Under extreme circumstances (e.g. a petrochemical spill during refuelling), operational management plans will be in place which identify strategies for remediation. Selection of a suitable GPT will allow some oils and grease to be retained.

Other measures (under the Environmental Management Plan) would include a covered and bunded area being provided for any refuelling (and materials storage) facilities on the site. Bunds should be capable of containing the full storage volume of the container plus an additional 10%.

6.3. Salinity

There is presently no visible indication of salinity at the ground surface around the site. The *Precinct Plan* suggests that adverse impacts on salinity would be expected if the groundwater level were to be raised significantly over a period of time. In this way, contributing factors may include prolonged flooding, removal of deep-rooted vegetation, over-irrigation, disruption of natural drainage lines, stormwater infiltration and leaky pipes. Some areas of the site may be more susceptible to developing soil salinity problems due to their geology.

The pit is likely to have contributed to some extent to lowering the groundwater table in the vicinity of the site by creating a groundwater 'sink' (IGGC) and this may result in the possibility that saline drainage from sub-soils and bedrock will reach the quarry pit and walls and contribute to saline runoff collected in the pit.

Please refer to the IGGC report for more information about salinity in groundwater and saline groundwater impacts.

6.3.1. Site Water Management for Salinity

Water quality in the proposed temporary sedimentation basin located on progressively capped areas of the landfill within the pit is to be assessed as per monitoring requirements. If salinity or TDS results for water quality in the basin proves too saline for site irrigation or related surface uses, its use is to be restricted to suitable areas of the site, e.g. dust suppression within and around the quarry pit.

In general, the Department of Primary Industries (Agriculture) recommend that acceptable salinity levels for pasture (assuming that any irrigated areas at the site will primarily be turf) are in the order of 2200 μ S/cm before growth begins to be affected.

Potential impacts on salinity will be managed in the following ways:

- ☞ Modifications to existing site vegetation are kept to a minimum. Most site development is to occur on existing disturbed or cleared areas adjacent to the quarry pit;
- ☞ Minimise additions to groundwater table by avoiding waterlogged areas and over-irrigation; and
- ☞ The proposed OSD basin serving the proposed development, at a depth of approximately 3m below existing ground surface, is not likely to intercept potentially saline groundwater.

7.0 SUMMARY

7.1. Summary of Stormwater Management Measures

Water on site is to be managed according to the goals and methods outlined in Table 7.1 and takes into account site needs and BCC requirements. The general layout for site surface water management is presented in Appendix A.

Table 7-1: Stormwater Management

Stormwater Management Measure	Management goal	Methods
Site Stormwater Drainage System	Piped and open drainage structures to convey the major and minor storm events (1 in 100 year and 1 in 20 year respectively) to storage and reuse facilities or off-site as required, via on-site treatment and/or detention facilities where necessary.	All clean or sediment-only surface runoff to be directed to two detention basins in the Quarry catchment, <i>via</i> constructed drainage. See Section 3.1.1 for definitions of clean and dirty runoff. Piped networks will be designed to convey 1 in 20 year flows without surcharge. The MPC work floor/green waste stockpile area is to be directed to sewer. Drainage overflows (greater than 1 in 100yr flows) from both these areas is directed away from the quarry pit via overland flow paths. Vehicle entry points for MPC work floor / green waste and materials stockpile & drop-off areas to be located to minimise uncontrolled runoff and sediment release outside these areas. Overland flow paths around the site are to remain stable in 100 year critical flows.
On-site Detention	On-site detention is required to match post-development flows with pre-development flows.	OSD is required to match post-development flows with pre-development flows from the developed operational area (14.8Ha). The remaining site area flows will not be detained as there will be no change in land use in these areas. The required OSD volume to contain 1 in 100 year flows from the 14.8Ha surface operational area is 5500m ³ .
Stormwater Management	Minimise generation of leachate and contaminated runoff	All clean surface runoff to be diverted to two OSD basins (operational area flows) or an in-pit basin (quarry area flows). Surface run-on to and from or sediment-generating operational areas at the surface, and to the quarry pit, is to be minimised through the use of diversion bunds and site grading. This will include grading the site such that all surface runoff up to the 100 year (or design) event is directed away from quarry pit. Drainage from the MPC work floor/green waste area is to be connected to sewer. Runoff from the stockpile/drop-off area is to be managed as clean surface stormwater, with additional sediment control. The MPC work floor/green waste area will be bunded to

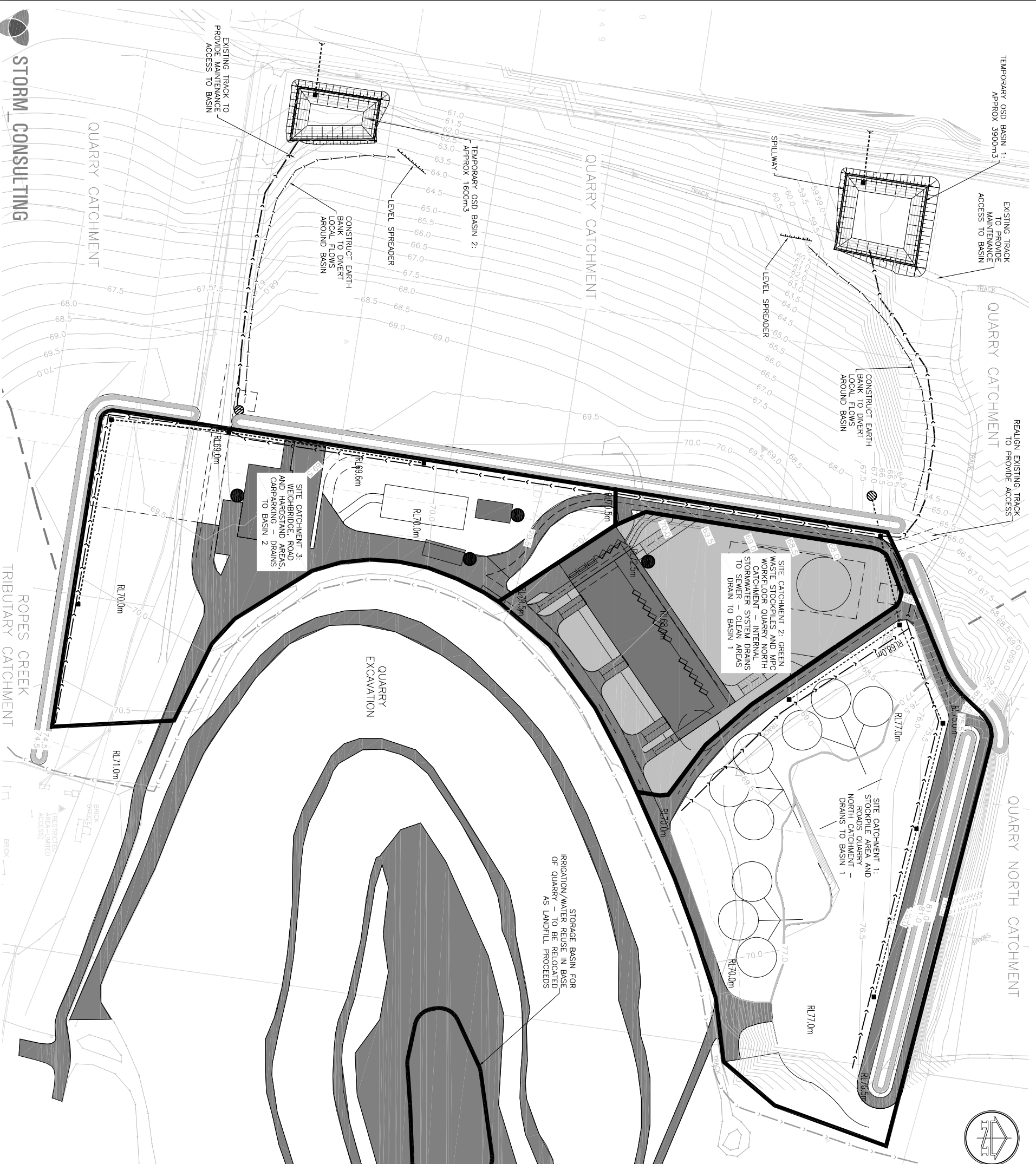
		prevent stormwater entering the area.
Water Quality and Reuse	Treat, store and reuse runoff on site where possible	Any runoff water coming into contact with waste as defined in Section 3.1 is to be treated as leachate. Reuse purposes include: 1. toilets and other building internal uses 2. outdoor uses including dust suppression, stockpile management, irrigation. See Chapter 5, Water Cycle Management for further discussion. Water management practices for incidental waste storage (e.g. office waste storage areas, 240L 'Otto' bins), vehicle wash down areas and materials storage areas to follow Appendix D in BCC <i>Stormwater Quality Control Policy</i>.
Reduce mains water demand.	Roof water is to be captured in rainwater tanks for reuse on site. A network of water storages will be located on site to provide water supply to the facility as determined by the water balance model and site demands.	Harvested roof water to be used in buildings for appropriate end-uses (e.g. toilet flushing and localised irrigation). Roof water will also be used to top up the wheel wash. Stormwater harvested from the OSD basin would be used for dust suppression, irrigation around the berms, stockpile management and other non-potable water uses. Water from proposed quarry pit detention basin installed after the initial 10m lift to be reused for dust suppression, in water carts on haul roads or in dump truck on-board reservoirs. See Chapter 5, Water Cycle Management
Site Monitoring	Monitor water quality and drainage systems	Periodic checking and maintenance of site drainage and water quality controls to be undertaken to reduce likelihood of drain blockage and overflows. To ensure water quality is suitable for equipment used in irrigation or stockpile spraying/management, monitoring of water quality may take place by sampling from the site OSD basin, and if necessary from the proposed in-pit basin.

8.0 REFERENCES

- ☞ Blacktown City Council (2005) Eastern Creek Precinct Plan – Stage 3
- ☞ Blacktown City Council (2005) Engineering Guide for Development
- ☞ Blacktown City Council (2005) Stormwater Quality Control Policy P01100
- ☞ Institution of Engineers (2000) Australian Rainfall and Runoff
- ☞ Landcom (2004) Managing Urban Stormwater: Soils and Construction Volume 1, 4th Edition
- ☞ NSW DECC (2007) Managing Urban Stormwater: Soils and Construction Volume 2B – Waste Landfills (currently available as a draft for consultation only)
- ☞ SMEC (2004) SEPP59 Landholder Group Eastern Creek Precinct Plan Stormwater Management Strategy

APPENDIX A

Surface Water Management Plan



LEGEND

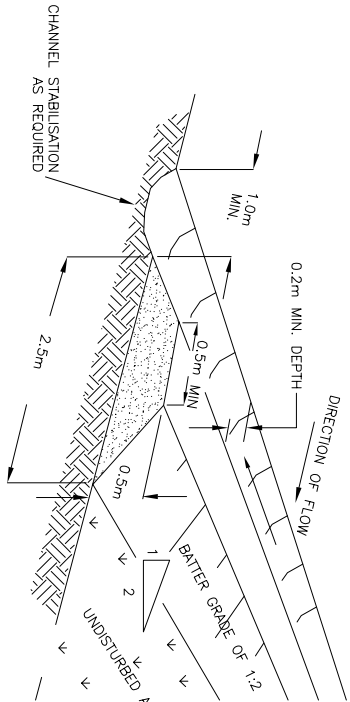
- | | |
|---|--|
|  | INDICATIVE STORMWATER PIT LOCATION |
|  | INDICATIVE STORMWATER PIPE LOCATION |
|  | OVERLAND FLOW / SWALE DIRECTION |
|  | OVERLAND FLOW SWALE TO DETENTION BASIN |
|  | GROSS POLLUTANT TRAP |
|  | DESIGN CONTOURS (0.5m INTERVAL) |
|  | MIN 10% RAINTANKS FOR BUILDINGS (INTERNAL REUSE – TOILET FLUSHING, LOCAL IRRIGATION, TOP UP OF WHOLE WASH) |
|  | INDICATIVE FUTURE OVERLAND FLOW SWALE FROM FINAL LANDFILL FORMATION |

NOTES

1. ALL RADS AND CONSTRUCTED PLATFORMS MINIMUM GRADE 0.5%.
2. ALL CONSTRUCTED PLATFORMS AND ROADS TO BE DRAINED BY PIT/PIPE DESIGNED FOR 20 YEAR ARI CAPACITY, AND OPEN SWALE DESIGNED FOR 100 YEAR ARI CAPACITY.
3. ALL SWALES AND CULVERT OUTLETS TO BE ARMOURRED/VEGETATED AS PER DETAILED DESIGN.
4. ALL AREAS TO DRAIN TO DETENTION BASINS TO THE WEST OF THE SITE AS PER COUNCIL INSTRUCTIONS.
5. IRRIGATION/ DUST SUPPRESSION LINE TO BE INSTALLED ALONG BERMS.
6. TYPICAL SECTION OF DETENTION BASIN ON DRAWING D02.
7. BASIN VOLUME GIVEN IS IN ADDITION TO WETLAND PERMANENT STORAGE AND REUSE STORAGE.
8. REFER TO DRAWING D02 FOR EARTH BANK DETAIL AND LEVEL SPREADER DETAIL.

SITE CATCHMENTS

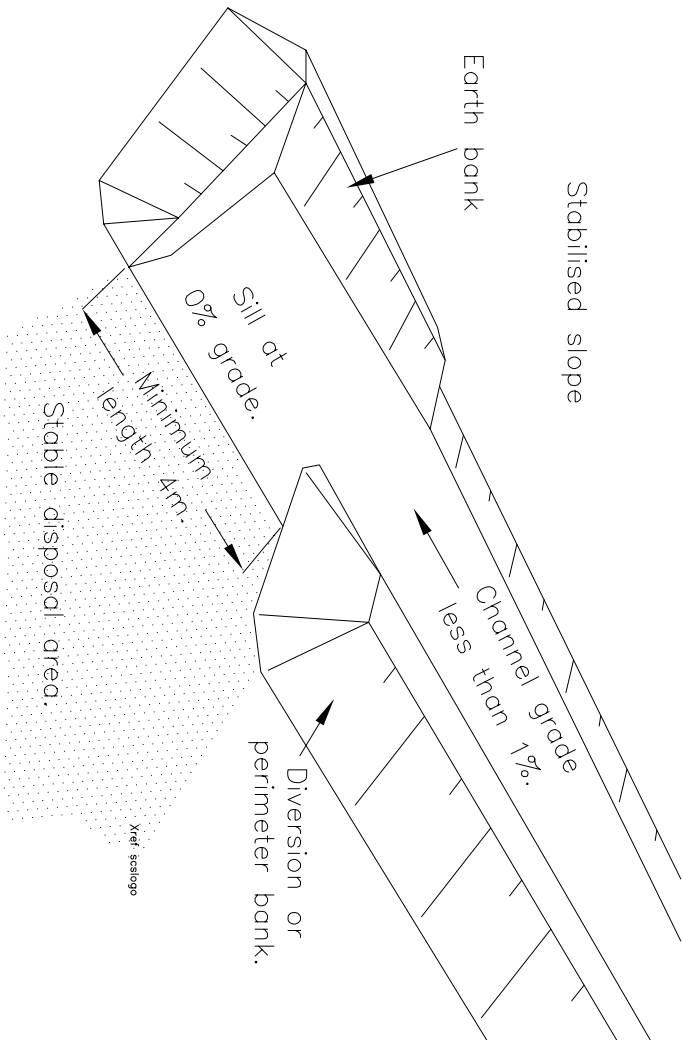
	CATCHMENT 1 [INCLUDES CRUSHED MATERIAL STOCKPILE AREA]	CATCHMENT 2 [GREEN WASTE STOCKPILE AREA AND MPC WORKSHOP]	CATCHMENT 3 [WEIGHBRIDGE, HARDSTAND AREAS, OTHER BUILDINGS]
TOTAL AREA	6.5HA	3.3 HA	4.8 HA
TEMPORARY OSD BASIN VOL. (BASED ON 400mm/4h WITH STAGED DISCHARGE OUTLETS AT BASIN)	3,900m ³ FOR TOTAL AREA OF CATCHMENTS 1 AND 2. VERY CONSERVATIVE OSD VOLUME AS MAJORITY OF RUNOFF FROM CATCHMENT 2 UP TO 100 YEAR EVENT WILL BE COLLECTED AND PUMPED TO LEACHATE TREATMENT SYSTEM		
			1,600m ³



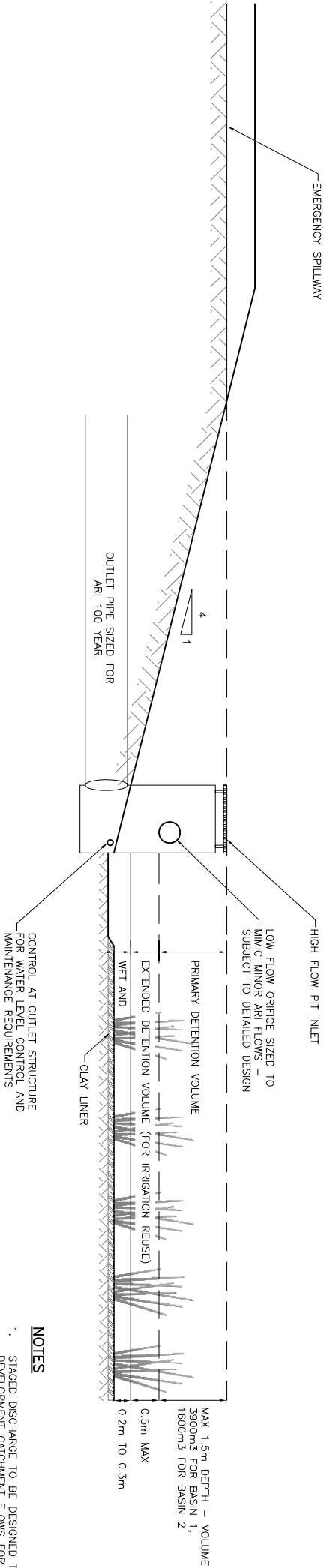
Earth Bank Detail
NTS

Earth Bank Construction Notes

1. Construct at a gradient between 1 and 5°
2. Avoid removing trees and shrubs if possible – work around them
3. Ensure the structures are free of projections or other irregularities that could impede water flow.
4. Build the drains with circular, parabolic or trapezoidal cross sections, not V-shaped.
5. Ensure the banks are properly compacted to prevent failure.
6. Complete permanent or temporary stabilisation with 10 days of construction.
7. Where discharging to erodible lands, ensure they outlet through a properly constructed level spreader.
8. Construct level spreader at a gradient of less than 1%
9. Where possible, ensure they discharge waters onto either stabilised or undisturbed disposal sites with the same subcatchment area from which the water originated. Approval might be required to discharge into other subcatchments.



Level Spreader
NOT TO SCALE



Schematic of Detention Basin and
Staged Discharge Outlet
Scale 1:50

- NOTES
1. Staged discharge to be designed to match pre and post development catchment flows for major and minor ARI rainfall events up to 100 years.
 2. Primary detention volume to cater for up to 100 year ARI.
 3. Extended detention (reuse) volume to be included depending on final water balance and reuse requirements – subject to detailed design.
 4. Wetland design and planting subject to detailed design.



LAND USE	LIMITATIONS	COMMENTS
CONSTRUCTION AREAS	DISTURBANCE TO BE NO FURTHER THAN FIVE (5) AND PREFERABLE TWO (2) METRES FROM THE EDGE OF ANY ESSENTIAL ENGINEERING ACTIVITY AS SHOWN ON THE PLAN	ALL SITE WORKERS WILL CLEARLY RECOGNISE THESE ZONES THAT, WHERE APPROPRIATE, ARE IDENTIFIED WITH BARRIER FENCING (UPSLOPE) AND SEDIMENT FENCING (DOWNSLOPE) OR SIMILAR MATERIALS
ACCESS AREAS	LIMITED TO A MAXIMUM WIDTH OF TEN (10) METRES	THE SITE MANAGER WILL DETERMINE AND MARK THE LOCATION OF THESE ZONES INSTANTLY. THE LOCATION WILL BE TO BE MARKED THEIR BOUNDARIES THAT, WHERE APPROPRIATE, ARE MARKED WITH BARRIER MESH, SEDIMENT FENCING, OR SIMILAR MATERIALS.

DRAWING: **P03**
SHEET: 3 OF 3

APPENDIX B

Blue Book calculations

SWMP Commentary, Detailed Calculations

Note: These "Detailed Calculation" spreadsheets relate only to high erosion hazard lands as identified in figure 4.6 or where the designer chooses to use the RUSLE to size sediment basins. The "Standard Calculation" spreadsheets should be used on low erosion hazard lands as identified by figure 4.6 and where the designer chooses not to run the RUSLE in calculations.

1. Site Data Sheet

Site Name: Light Horse Business Centre

Site Location: Blacktown City Council

Precinct: Eastern Creek

Description of Site: Quarry pit, steep walls w likely low sediment yield now, require clean water run-on capture from intermediate capped landfill area & wall runoff where nec to reduce leachate generation

Site area	Site						Remarks
Total catchment area (ha)	26.5						
Disturbed catchment area (ha)	26.5						

Soil analysis

% sand (fraction 0.02 to 2.00 mm)	10						Soil texture should be assessed through mechanical dispersion only. Dispersing agents (e.g. Calgon) should not be used
% silt (fraction 0.002 to 0.02 mm)	20						
% clay (fraction finer than 0.002 mm)	70						
Dispersion percentage	30.0						E.g. enter 10 for dispersion of 10%
% of whole soil dispersible	24						See Section 6.3.3(e)
Soil Texture Group	D						See Section 6.3.3(c), (d) and (e)

Rainfall data

Design rainfall depth (days)	5						See Sections 6.3.4 (d) and (e)
Design rainfall depth (percentile)	80						See Sections 6.3.4 (f) and (g)
x-day, y-percentile rainfall event	25						See Section 6.3.4 (h)
Rainfall intensity: 2-year, 6-hour storm	10.1						See IFD chart for the site

RUSLE Factors

Rainfall erosivity (R -factor)	2250						Automatic calculation from above data RUSLE data can be obtained from Appendixes A, B and C
Soil erodibility (K -factor)	0.038						
Slope length (m)	100						
Slope gradient (%)	5						
Length/gradient (LS -factor)	1.35						
Erosion control practice (P -factor)	1.3	1.3	1.3	1.3	1.3	1.3	
Ground cover (C -factor)	1	1	1	1	1	1	

Calculations

Soil loss (t/ha/yr)	150						
Soil Loss Class	1						See Section 4.4.2(b)
Soil loss (m ³ /ha/yr)	115						
Sediment basin storage volume, m ³	520						See Sections 6.3.4(i) and 6.3.5 (e)

2. Storm Flow Calculations

Peak flow is given by the Rational Formula:

$$Q_y = 0.00278 \times C_{10} \times F_y \times I_{y,tc} \times A$$

- where:
- Q_y is peak flow rate (m^3/sec) of average recurrence interval (ARI) of "Y" years
 - C_{10} is the runoff coefficient (dimensionless) for ARI of 10 years. Rural runoff coefficients are given in Volume 2, figure 5 of Pilgrim (1998), while urban runoff coefficients are given in Volume 1, Book VIII, figure 1.13 of Pilgrim (1998) and construction runoff coefficients are given in Appendix F
 - F_y is a frequency factor for "Y" years. Rural values are given in Volume 1, Book IV, Table 1.1 of Pilgrim (1998) while urban coefficients are given in Volume 1, Book VIII, Table 1.6 of Pilgrim (1998)
 - A is the catchment area in hectares (ha)
 - $I_{y,tc}$ is the average rainfall intensity (mm/hr) for an ARI of "Y" years and a design duration of "tc" (minutes or hours)

Time of concentration (t_c) = $0.76 \times (A/100)^{0.38}$ hrs (Volume 1, Book IV of Pilgrim, 1998)

Note: For urban catchments the time of concentration should be determined by more precise calculations or reduced by a factor of 50 per cent.

Peak flow calculations, 1

Site	A (ha)	tc (mins)	Rainfall intensity, I, mm/hr						C_{10}
			1 _{yr,tc}	5 _{yr,tc}	10 _{yr,tc}	20 _{yr,tc}	50 _{yr,tc}	100 _{yr,tc}	
	26.5	28	36.2	60	68	78	91	101	0.85

Peak flow calculations, 2

ARI (yrs)	Frequency factor (F_y)	Peak flows						Comment
		(m^3/s)	(m^3/s)	(m^3/s)	(m^3/s)	5 (m^3/s)	(m^3/s)	
1 yr,tc	0.8	1.813						Page B-11 DEC Eng guidelines for development
5 yr,tc	0.95	3.569						
10 yr,tc	1	4.258						
20 yr,tc	1.05	5.129						
50 yr,tc	1.15	6.553						
100 yr,tc	1.2	7.589						

4. Volume of Sediment Basins, Type D and Type F Soils

Basin volume = settling zone volume + sediment storage zone volume

Settling Zone Volume

The settling zone volume for Type F and Type D soils is calculated to provide capacity to contain all runoff expected from up to the y-percentile rainfall event. The volume of the basin's settling zone (V) can be determined as a function of the basin's surface area and depth to allow for particles to settle and can be determined by the following equation:

$$V = 10 \times C_v \times A \times R_{x\text{-day, } y\text{-}\%ile} \text{ (m}^3\text{)}$$

where:

10 = a unit conversion factor

C_v = the volumetric runoff coefficient defined as that portion of rainfall that runs off as stormwater over the x-day period

$R_{x\text{-day, } y\text{-}\%ile}$ = is the x-day total rainfall depth (mm) that is not exceeded in y percent of rainfall events. (See Sections 6.3.4(d), (e), (f), (g) and (h)).

A = total catchment area (ha)

Sediment Storage Zone Volume

In the detailed calculation on Soil Loss Classes 1 to 4 lands, the sediment storage zone can be taken as 50 percent of the settling zone capacity. Alternately designers can design the zone to store the 2-month soil loss as calculated by the RUSLE (Section 6.3.4(i)(ii)). However, on Soil Loss Classes 5, 6 and 7 lands, the zone must contain the 2-month soil loss as calculated by the RUSLE (Section 6.3.4(i)(iii)).

Place an "X" in the box below to show the sediment storage zone design parameters used here

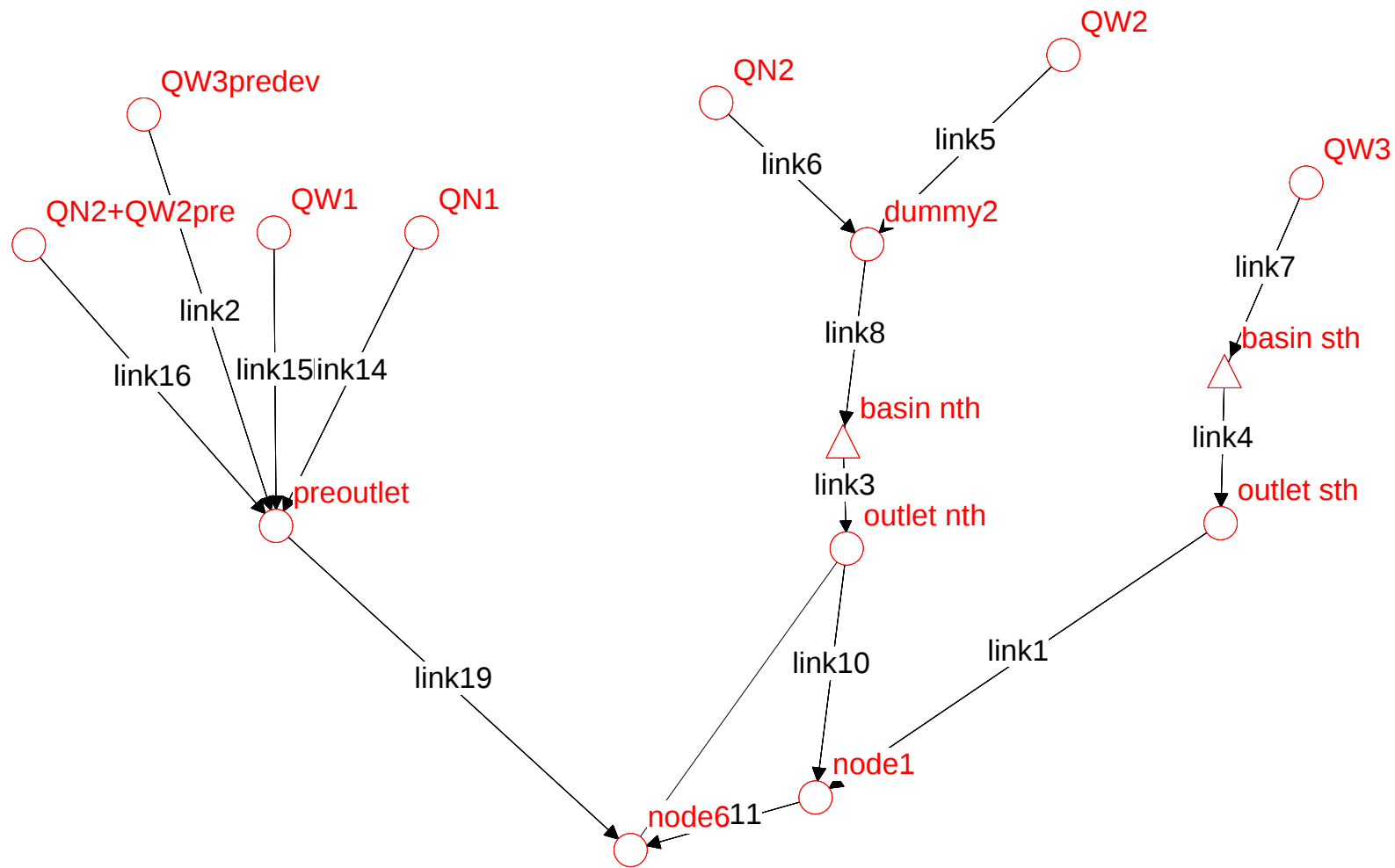
	50% of settling zone capacity,
X	2 months soil loss calculated by RUSLE

Total Basin Volume

Site	C_v	$R_{x\text{-day, } y\text{-}\%ile}$	Total catchment area (ha)	Settling zone volume (m ³)	Sediment storage volume (m ³)	Total basin volume (m ³)
	0.58	25	26.5	3842.5	520	4362.5

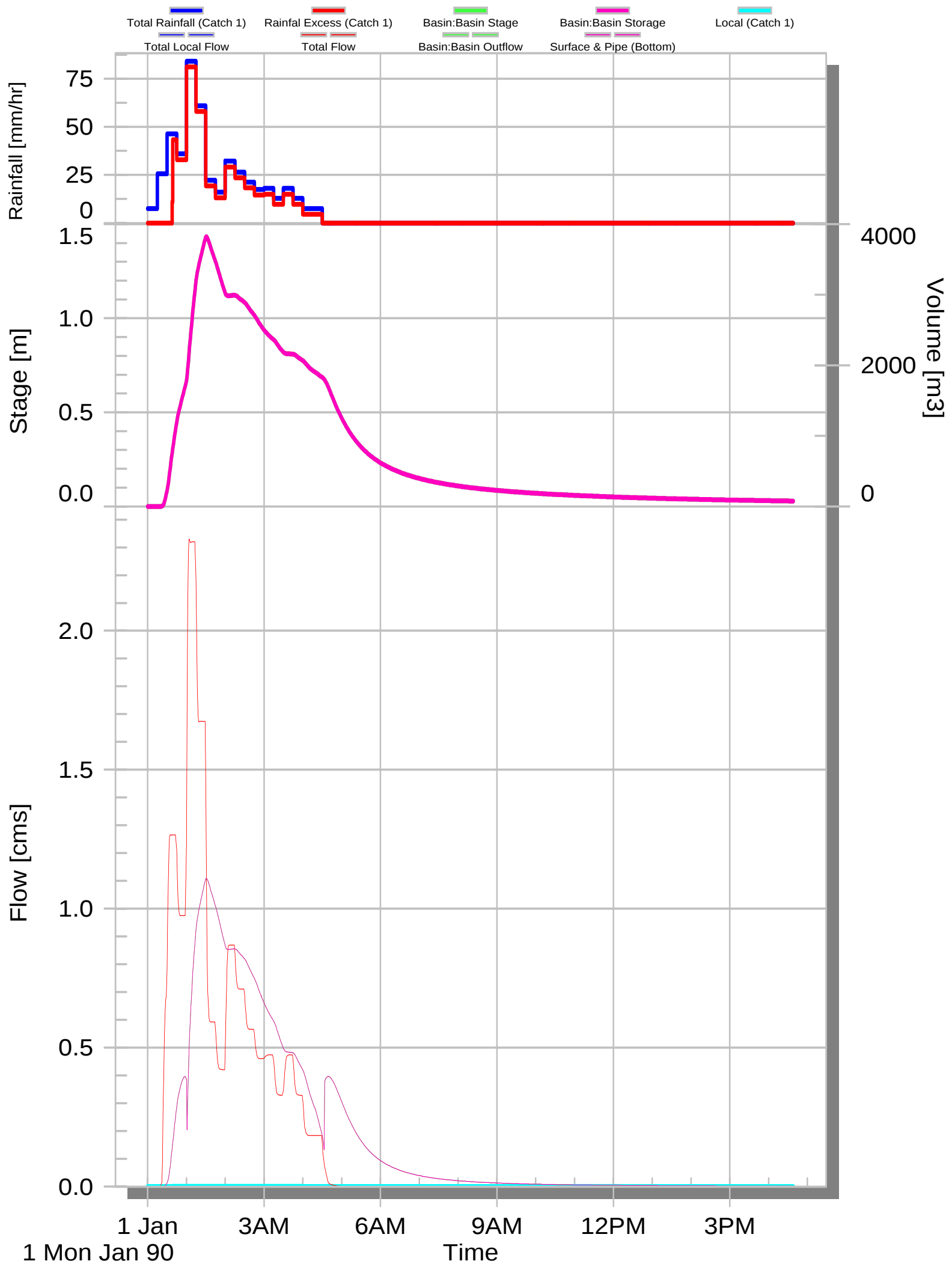
APPENDIX C

Model outcomes – RAFTS

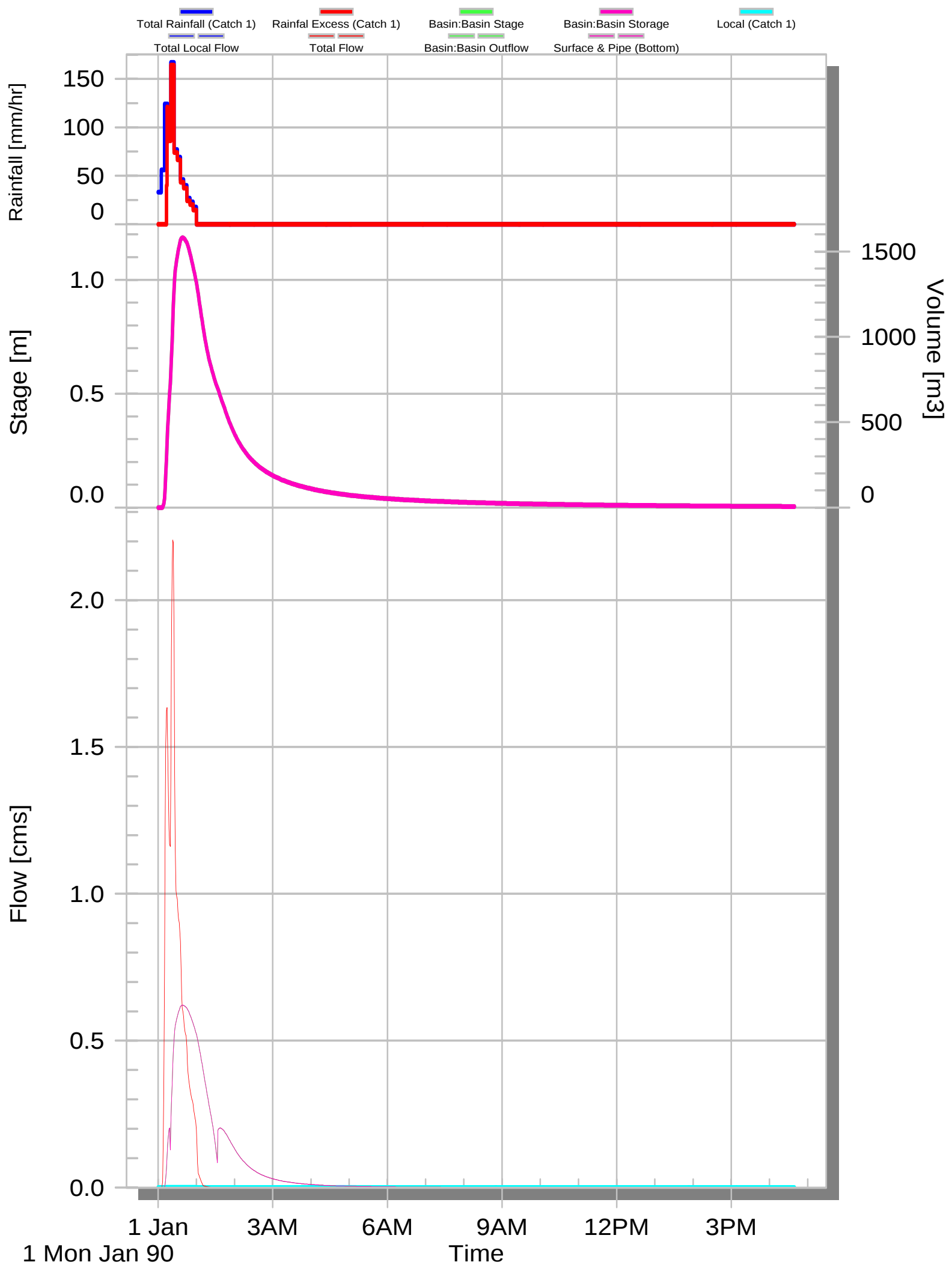


basin 1

Max - Basin:Basin Stage[1.434] Basin:Basin Storage[3825.2] Local (Catch 1)[0.000] Total Local Flow[0.000] Total Flow[2.331]

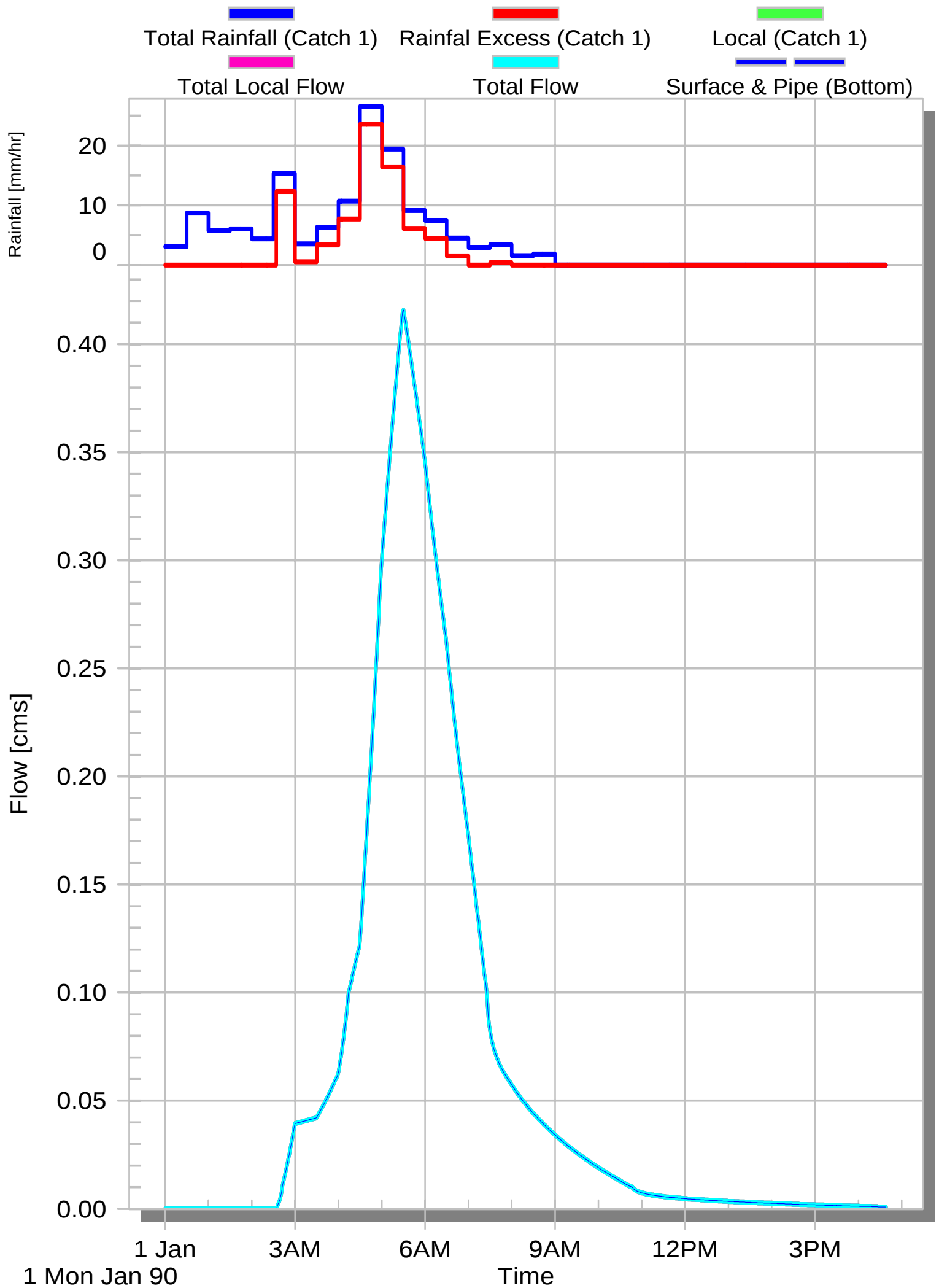


Max - Basin:Basin Stage[1.186] Basin:Basin Storage[1580.7] Local (Catch 1)[0.000] Total Local Flow[0.000] Total Flow[2.205]



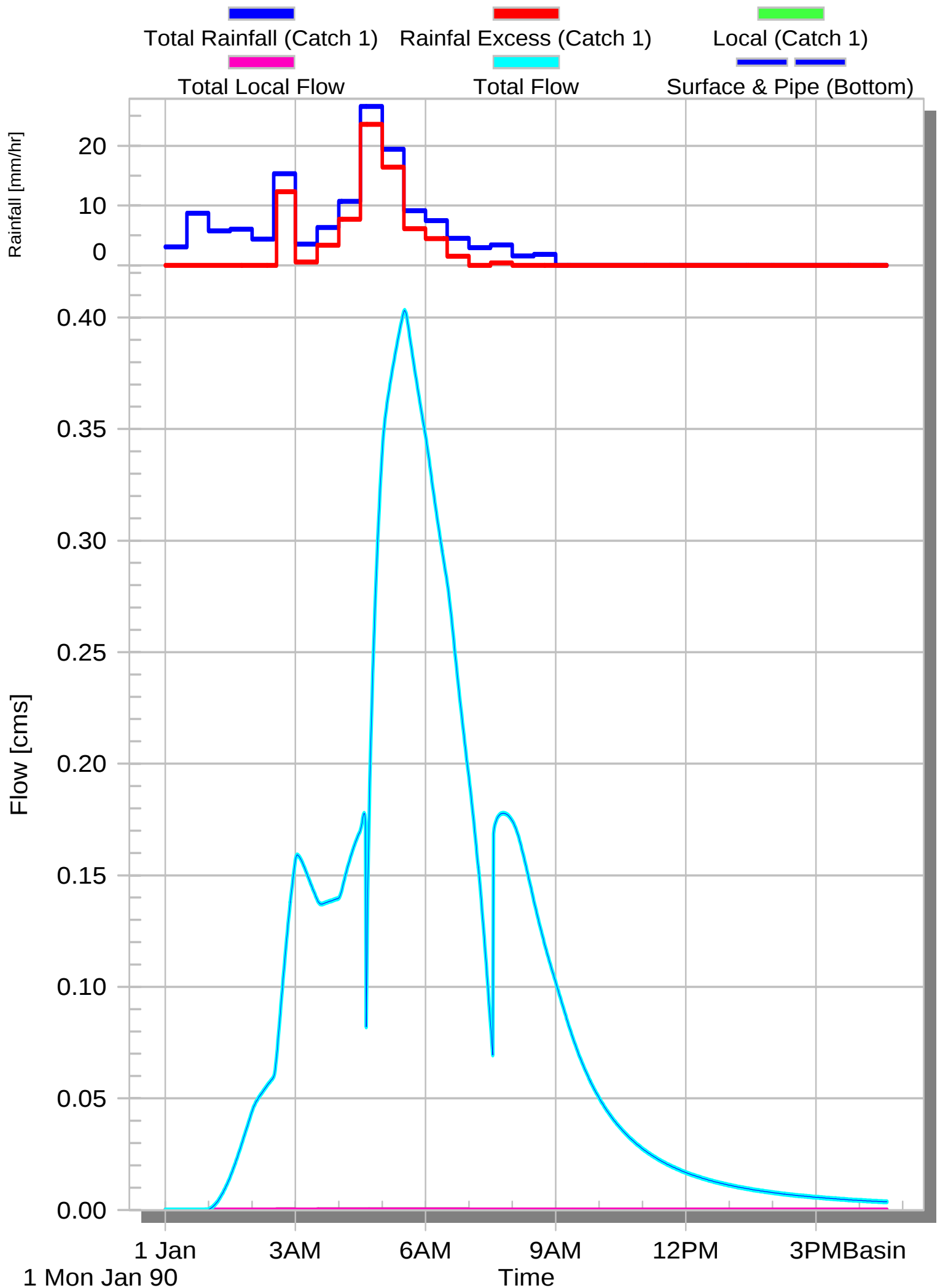
Basin 1 catchment pre-dev 2-yr

Max - Local (Catch 1)[0.416] Total Local Flow[0.416] Total Flow[0.416]



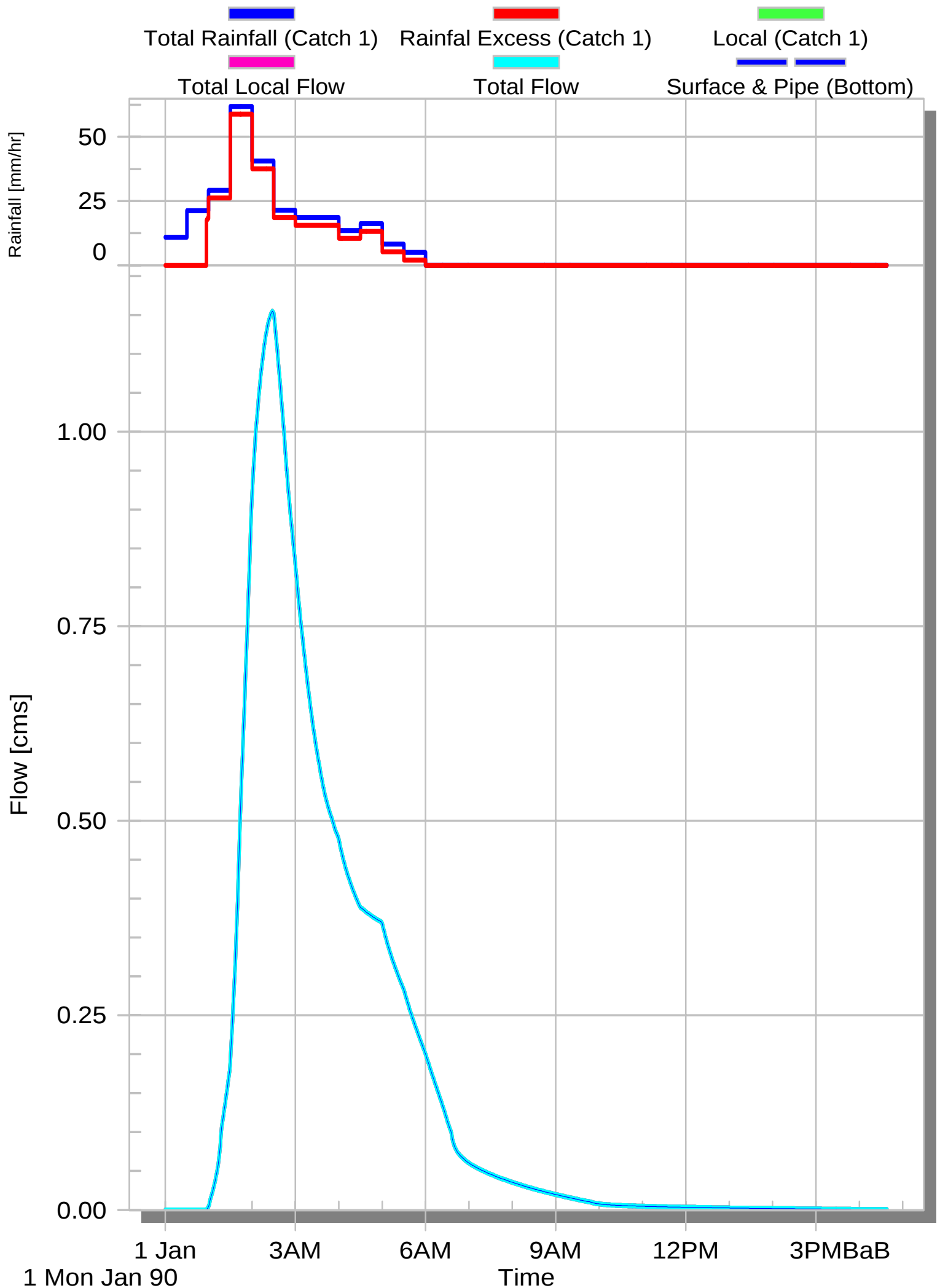
Basin 1 catchment postdev 2-yr

Max - Local (Catch 1)[0.000] Total Local Flow[0.000] Total Flow[0.403]



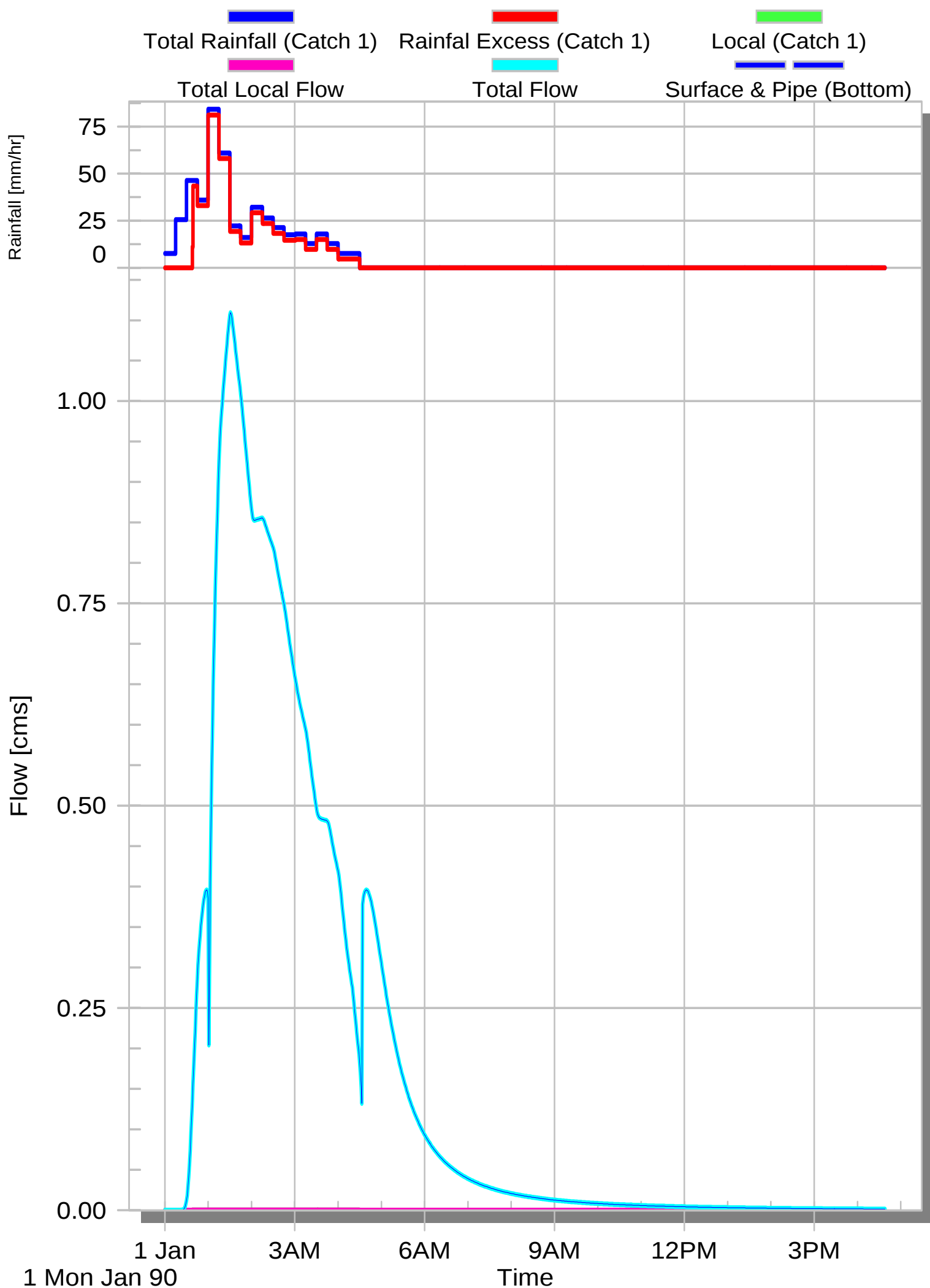
Basin 1 predev catchment 100-yr

Max - Local (Catch 1)[1.156] Total Local Flow[1.156] Total Flow[1.156]



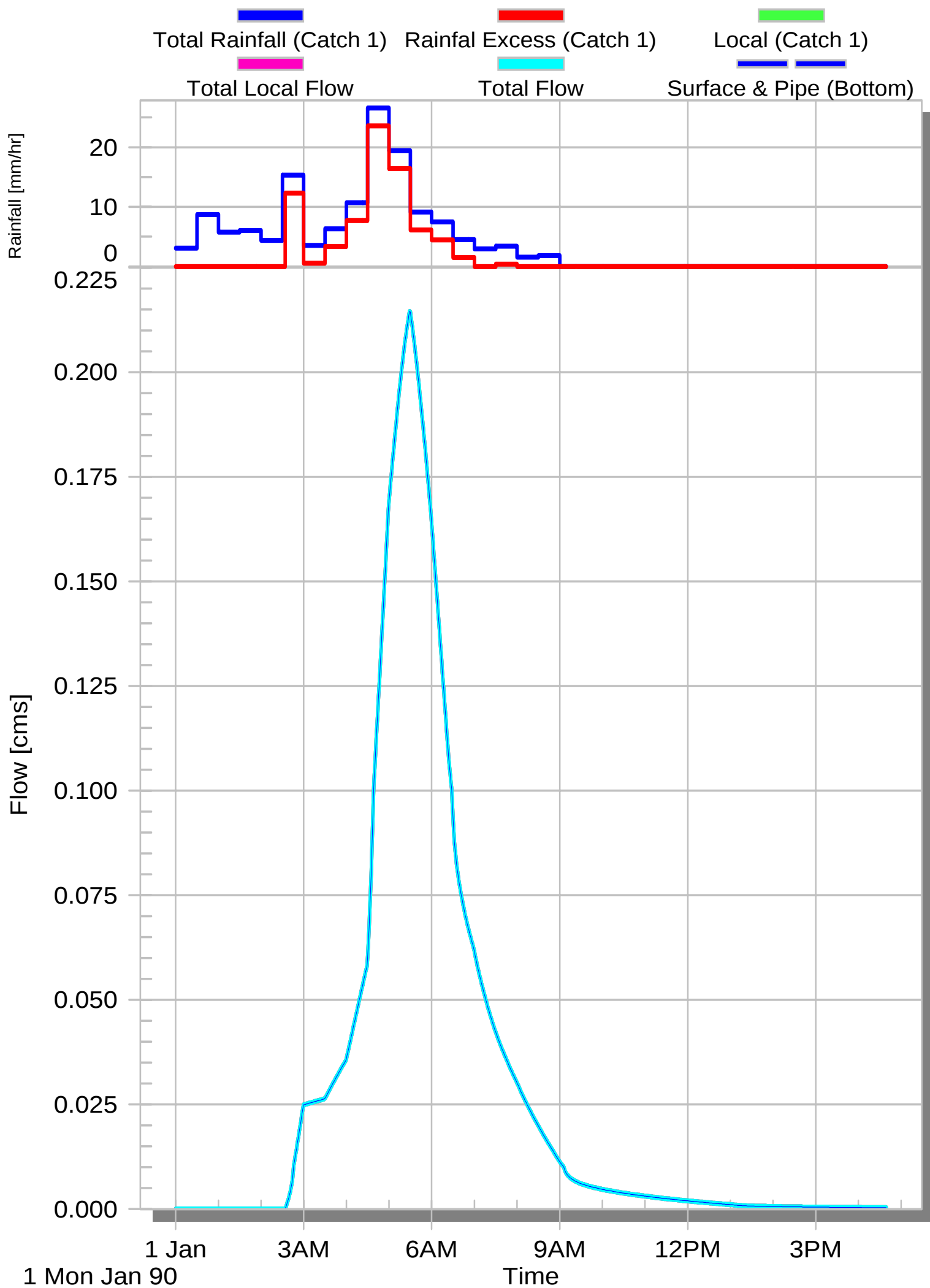
Basin 1 catchment postdev 100-yr

Max - Local (Catch 1)[0.000] Total Local Flow[0.000] Total Flow[1.110]



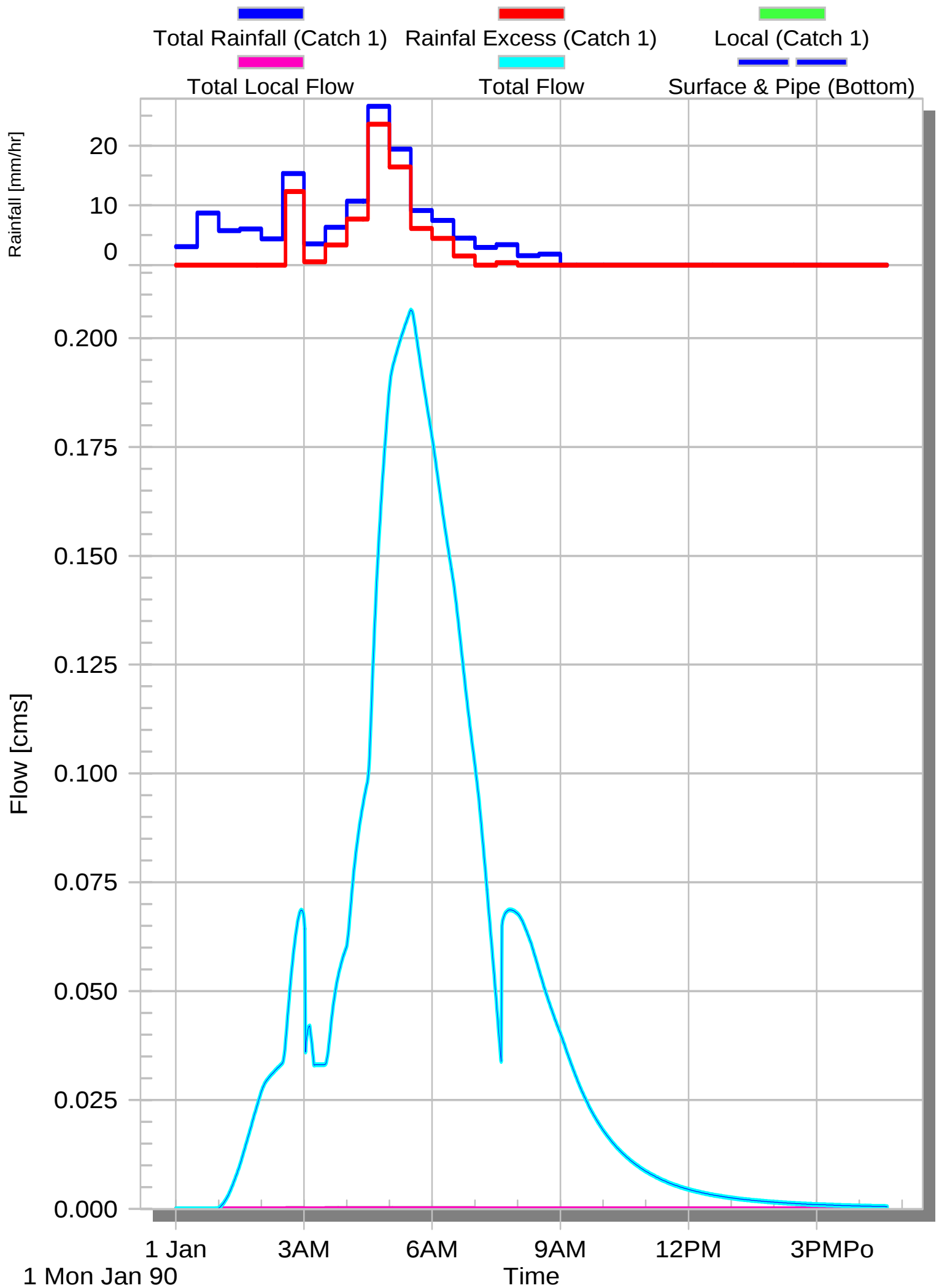
Basin 2 catchment predev, 2-yr

Max - Local (Catch 1)[0.215] Total Local Flow[0.215] Total Flow[0.215]



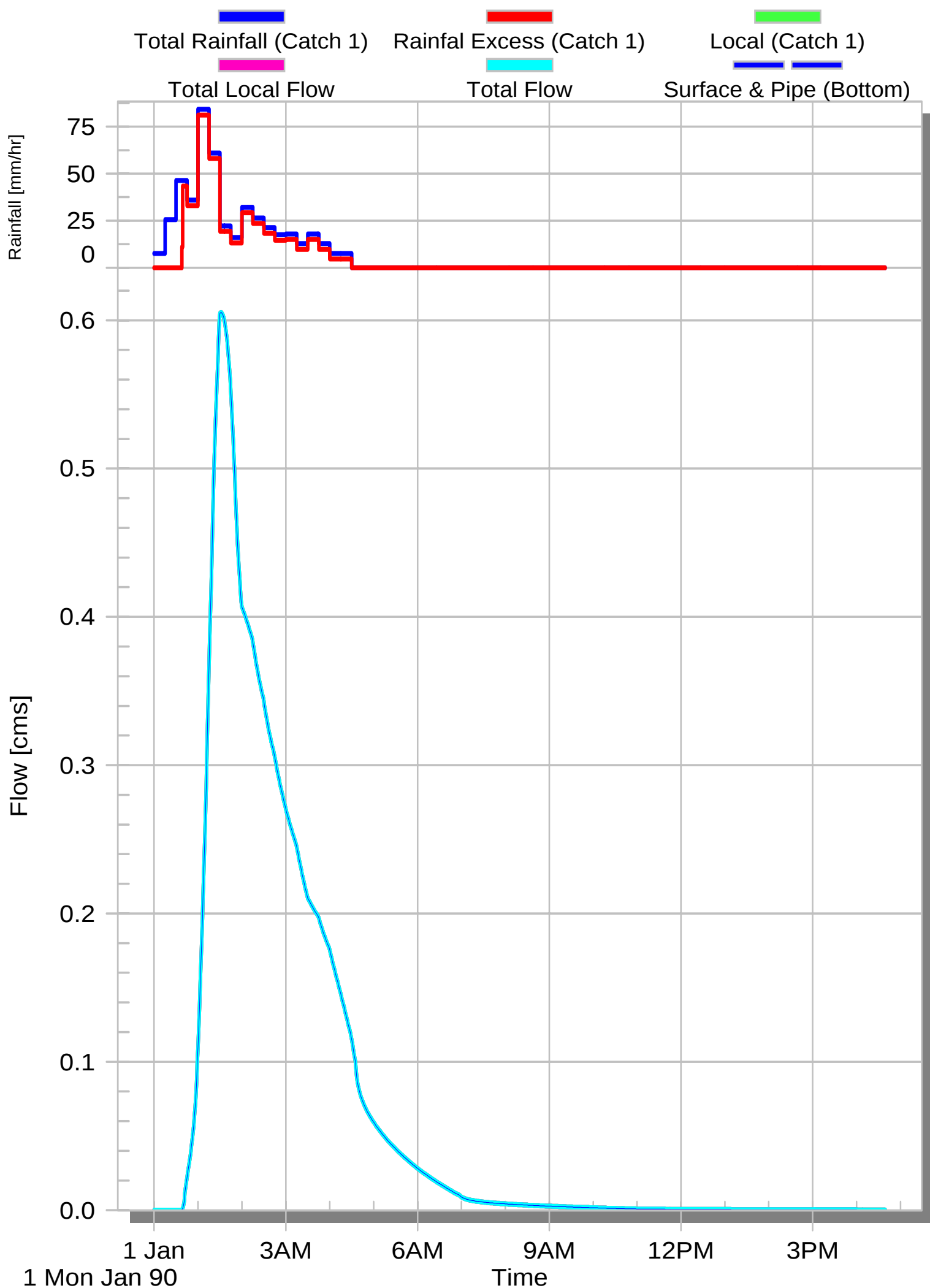
Basin 2 catchment postdev, 2-yr

Max - Local (Catch 1)[0.000] Total Local Flow[0.000] Total Flow[0.206]



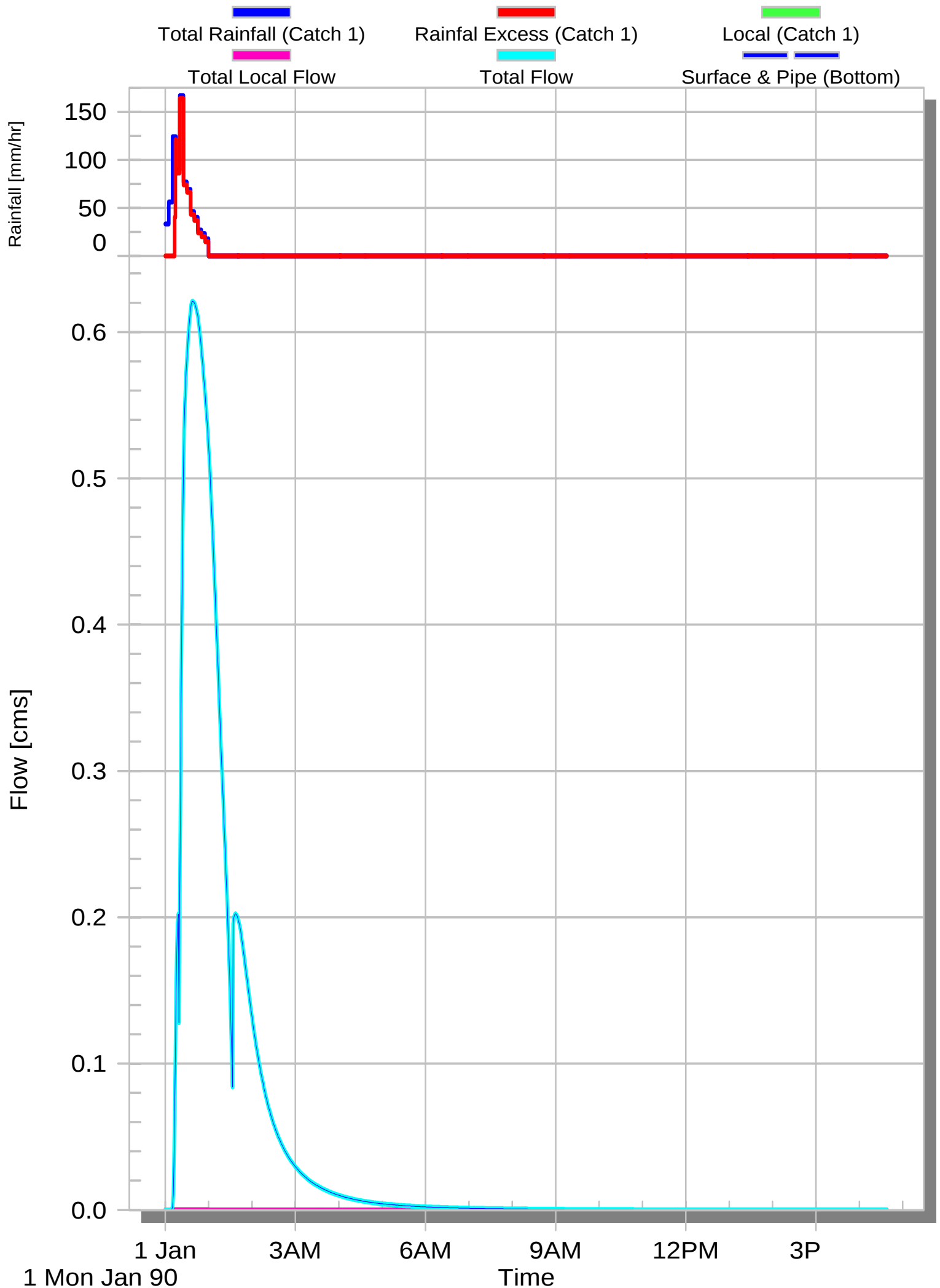
Basin 2 catchment predev 100-yr

Max - Local (Catch 1)[0.605] Total Local Flow[0.605] Total Flow[0.605]



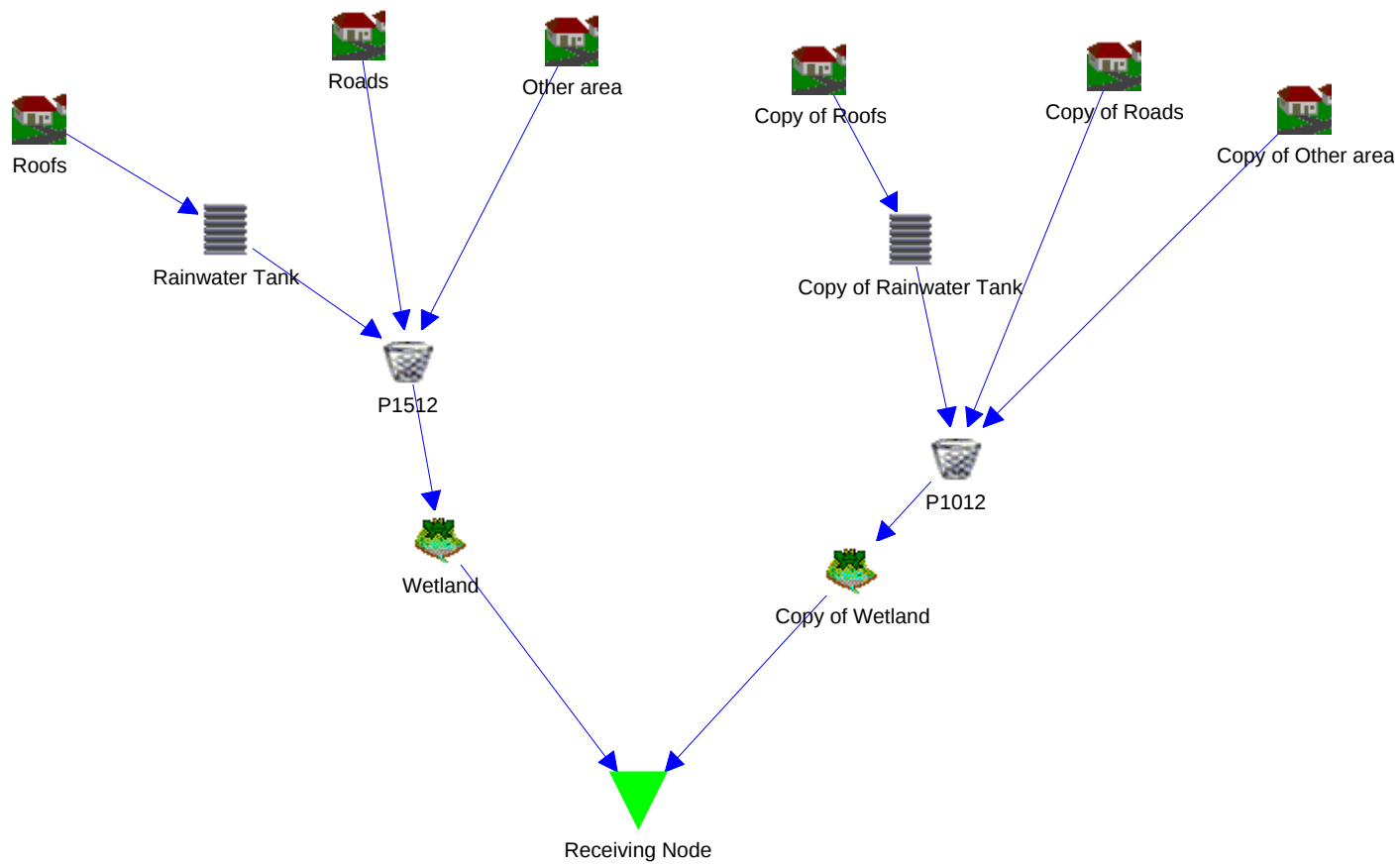
Basin 2 catchment postdev 100-yr

Max - Local (Catch 1)[0.000] Total Local Flow[0.000] Total Flow[0.621]



APPENDIX D

Model – MUSIC layout



Appendix B

Consolidated Stormwater Management Plan Martens

Dial A Dump Industries Pty Ltd



Consolidated Stormwater Management Plan

Light Horse Business Park, Eastern
Creek, NSW

ENVIRONMENTAL



WATER



WASTEWATER



GEOTECHNICAL



CIVIL



PROJECT
MANAGEMENT



P1103002JR01V05
October 2011

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Document and Distribution Status							
Author(s)		Reviewer(s)		Project Manager		Signature	
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Revision No.	Description	Status	Release Date	Document Location			
				File Copy	MA Library	DADI	
1	Client Review	Draft	-	1E,1P,1H	-	1P	
1	DoP Submission	Final	10.03.2011	1E,1P,1H	1H	1P	
2	DoP Revised Final	Final	20.06.2011	1E,1P,1H	1H	1P	
3	OSD Assessment	Final	19.07.2011	1E,1P,1H	1H	1P	
4	Water Quality Assessment and OSD amendments	Final	22.09.2011	1E,1P,1H	1H	1P	
5	Respond to BCC 11/10/2011	Final	26.10.2011	1E,1P,1H	1H	1P	

Distribution Types: F = Fax, H = hard copy, P = PDF document, E = Other electronic format. Digits indicate number of document copies.

All enquiries regarding this project are to be directed to the Project Manager.

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1 Introduction

1.1 Overview

Martens and Associates has been commissioned by Dial-A-Dump Industries (the client) to prepare a consolidated Stormwater Management Plan for the site.

1.2 Scope

This document shall:

- Reassess stormwater quantity controls through modelling of constructed OSD basins to determine their adequacy to achieve site objectives.
- Reassess stormwater quality outcomes achieved by the proposed treatment train, including the OSD basins, to confirm their adequacy and compliance with Blacktown City Council policy.
- Document the final design stormwater solution for the site.

1.3 Site Areas

1.3.1 Fill Pad – ‘Area D’

‘Area D’ lies to the north of the quarry pit and is bound by a conservation area to the west, the M4 motorway to the north and a vacant future industrial lot to the east. Prior to recent works, the area was a grassed paddock. It has since been filled and no formal use is proposed. The area shall revegetate with grass.

1.3.2 Operational Area

The operational area includes hardstand area and buildings and is located directly west of the quarry pit. The area is bound by the conservation area and Area D to the north, a large earth bund and OSD basins beyond to the west (see Section 1.3.3) and cleared grasslands to the south.

Within this area, waste is to be processed for recycling or disposal to landfill.

Design layout of the operational area is provided in previously submitted Storm Consulting documentation. Final layout plan is reproduced in Attachment A.

Processing of waste within the MPC building will ensure that waste does not come into contact with stormwater.

To the extent that water may be used within the MPC building, basal grates ensure that drainage is wholly captured within the building, with excess overflow being directed to the leachate collection system.

Processing of green waste will take place in the designated green waste area at the northwestern corner of the operational area. This area is concreted and equipped with a sump, submersible (float switch activated) pump and co-located collection tanks. Concrete berms at the entrance and exit points to a height of 30cm ensure that the bunded concrete green waste area is capable of containing stormwater from a 12 hour, 1 in 100 year storm event. In the event that the pump failed, the area has been designed and equipped with overflow pipes directing surplus leachate to the in-pit leachate collection system. All runoff from the green waste area is either recycled or discharged to the quarry pit thence trade waste sewer connection.

The greater part of the operational area will drain to the northern GPT and thereafter to the northern bioremediation basin, wetland and OSD pond. The remainder will drain towards the southern GPT and southern OSD, or to the quarry and be captured by the in-pit stormwater collection pond.

Sediment control will be provided by the use of GPTs (CDS units) and open swales, bioremediation basin and wetlands constructed within OSDs.

1.3.3 Western Section

This area lies to the west of the operational area, and contains the OSD basins servicing the operational area. There is no formal landuse proposed and the area shall be allowed to regrass.

1.4 Past Documentation

Previously prepared stormwater documentation and its relevance to this management plan is summarised as follows:

- LandPartners, 2011 'Area D Finished Surface Contours'

Provides proposed finished contours for 'Area D'. As no formal use of the areas is proposed at this stage, no OSD is required.

The Precinct plan adopted by Blacktown City Council foreshadows the construction of a regional detention basin at or near the north eastern corner of the Conservation area.

Currently a temporary sediment pond occupies this area and, in the future when Area D is developed, that future application will address details of the permanent stormwater detention facility.

- Storm Consulting, 2008 'Site Surface Water Management Plan'

This document (Attachment C) addresses the main waste processing area and details drainage requirements and, now superseded, water quality controls and OSD measures.

OSD and water quality modelling by Storm (2008) has been superseded by Martens and Associates (2011) modelling.

- Storm Consulting, 2009 'Light Horse Business Centre Pavement Setout and Drainage Plans'

We understand these plans have been prepared on the basis of more detailed modelling of site hydrology and are relied on for catchment details.

- G R Hawkes and Associates (2010) 'Volume Capacity Southern Basin' and 'Volume Capacity North Basin'.

We understand these plans provide the most accurate survey of the existing north and south basin. These plans (Attachment B) are relied on to determine 'as built' basin volumes for DRAINS hydraulic modelling.

- NSW Dam Safety Committee Advice (2010)

The Landowner has obtained confirmation from the NSW Dam Safety Committee that the OSDs in the position and at the volumes as shown are not prescribed structures. This advice is provided as Attachment D.

2 Southern Riparian Zone

2.1 Overview

Site Image Landscape Architects Pty Ltd was commissioned by the client to prepare a Riparian Zone Management Plan covering the areas located in Lot 3 DP 1145808. This area, as part of SEPP 59, has been dedicated to conservation. It contains an intermittent watercourse which runs the length of this boundary.

At the time of inspection by Site Image, this creek was found to be highly affected by erosion and sedimentation. Riparian vegetation was limited and mainly consisted of noxious weeds. An unauthorized diversion trench had been temporarily constructed to divert overflow waters from neighbouring sedimentation dams while creek works were carried out.

2.2 Reinstatement of Riparian Zone

The following management plan has been approved by DECCW and DoP and works have subsequently been carried out at the site.

- Sediment and erosion control measures were installed as required. Trees requiring protection during works identified.
- Fill material and sediment within the watercourse was removed and used to fill the diversion trench and reinstate the ground level in that area.
- The watercourse was reinstated to reflect its original channel form.
- Channel was lined with rocks and gravel to address future scouring and erosion.
- Topsoil was replaced utilising material stockpiled onsite.
- The banks, restored watercourse and other areas affected by restoration works were revegetated by spray seeding of suitable native grasses. This extends 10m either side of the creek from the creek centreline.
- Regular weed control is being undertaken.

The Precinct Plan stipulates a 40 metre buffer from top of bank on each side of a scheduled watercourse. The riparian zone abutting the

unnamed tributary to Ropes Creek is located in Lot 3 DP 1145808. This is not a project area, and no development works are proposed for this area. It is currently vacant and not cultivated.

The Precinct Plan stipulates a 10 metre set back from the Upper Angus Creek area. This riparian area is located on neighbouring land to Lot 4 DP 1145808 owned by Sumy Pty Ltd and similarly no works or development is proposed for this area.

3 Surface Water Quantity

3.1 Overview

This section provides results of hydraulic modelling of constructed basins. It assesses the existing structures adequacy to reduce post-development discharges to equal or less than pre-development discharges for the 2, 10 and 100 year ARI storms. It provides the final stormwater quantity solution for the development.

We note that hydraulic modelling results reported in this document supersedes results provided in Storm (2008) assessment. Requirements are detailed in Sections 3.3 to 3.8.

Subsequent to the submission of a previous version of this report (Version 4 dated 22.09.2011), Blacktown Council advised they required changes to input parameters in the DRAINS model. While the original input parameters are considered adequate and appropriate it was agreed, in subsequent consultation with Council, that remodelling using updated initial and continuing loss inputs would be completed.

3.2 General OSD Requirements

3.2.1 Operational Area

OSD is required for the proposed Operational Area which is subject to change in landuse resulting in an increase in impervious area.

3.2.2 'Area D and the Western Section'

OSD is not required for 'Area D' and the 'Western Section'. On completion of construction, these areas will regrass and therefore will not change hydrologically from the pre-development situation. Area D and the remainder of the western section therefore do not require OSD at this stage. Ultimately, it is assumed these areas shall undergo development for industrial/commercial purposes at which stage OSD shall be provided in accordance with preliminary requirements.

3.3 Modelling

DRAINS modelling was used to analyse site hydrology and confirm that existing OSD structures satisfy water quantity objectives outlines in Section 3.1. Input parameters were varied between modelling detailed in Version 4 of this report and the now reported figures by increasing initial and continuing losses for the pre-development model and for the undeveloped 'bund' catchments to 15 mm and 2.5 mm/hr

respectively. It is noted that Council's original requested initial loss of 25 mm was considered unacceptable and, in consultation with Council, it was agreed 15 mm would be used for modelling purposes.

3.4 Catchment Areas

The pre-development and post-development catchment areas used in DRAINS modelling are summarised in Table 1 and shown in Attachment E.

Table 1: DRAINS catchment summary

DRAINS Scenario	Catchment ID	Area (ha)	% Impervious	% Pervious	Existing Receiving Basin
Pre-Development	North Catchment	10.67	0	100	NA
	South Catchment	8.68	0	100	NA
Total		19.35			
Post-Development	North Stockpile Cat	4.27	100	0	
	North Operational Cat	3.90	100	0	North Basin
	North OSD Bund	2.17	0	100	
	South Operational Cat	5.63	100	0	South Basin
	South OSD Bund	3.47	0	100	
Total		19.44			

3.5 DRAINS Results

Results of pre- and post-development DRAINS modelling for 1 in 2, 10 and 100 year ARI storms are summarised in Table 2 (north OSD basin) and Table 3 (south OSD basin). Storm durations of 25 minutes to 9 hours (540 minutes) are assessed.

Table 2: North OSD Basin - Summary of DRAINS results (total flows) for 25-540 minute duration storm flows for design storm events

Duration (mins)	Q ₂			Q ₁₀			Q ₁₀₀		
	Pre-development peak flow (m ³ /s)	Developed peak flow (m ³ /s)	Difference (m ³ /s)	Pre-development peak flow (m ³ /s)	Developed peak flow (m ³ /s)	Difference (m ³ /s)	Pre-development peak flow (m ³ /s)	Developed peak flow (m ³ /s)	Difference (m ³ /s)
25	0.51	0.46	-0.05	1.31	0.57	-0.74	2.64	1.47	-1.17
30	0.57	0.45	-0.12	1.48	0.58	-0.90	2.83	1.60	-1.23
60	0.97	0.51	-0.46	1.91	0.65	-1.26	3.13	2.40	-0.73
90	1.13	0.52	-0.61	1.99	0.66	-1.33	3.09	2.52	-0.57
120	1.03	0.51	-0.52	2.03	0.66	-1.37	3.20	2.66	-0.54
180	0.76	0.49	-0.27	1.59	0.64	-0.95	2.56	1.62	-0.94
270	0.96	0.49	-0.47	1.59	0.64	-0.95	2.26	1.82	-0.44
360	0.82	0.48	-0.34	1.23	0.63	-0.6	1.74	1.47	-0.27
540	0.72	0.47	-0.25	1.08	0.61	-0.47	1.53	1.22	-0.31

Table 3: South OSD Basin - Summary of DRAINS results (total flows) for 25-540 minute duration storm flows for design storm events

Duration (mins)	Q ₂			Q ₁₀			Q ₁₀₀		
	Pre-development peak flow (m ³ /s)	Developed peak flow (m ³ /s)	Difference (m ³ /s)	Pre-development peak flow (m ³ /s)	Developed peak flow (m ³ /s)	Difference (m ³ /s)	Pre-development peak flow (m ³ /s)	Developed peak flow (m ³ /s)	Difference (m ³ /s)
25	0.45	0.25	-0.20	1.15	0.38	-0.77	2.31	1.02	-1.29
30	0.50	0.25	-0.25	1.27	0.39	-0.88	2.40	1.19	-1.21
60	0.82	0.31	-0.51	1.62	0.45	-1.17	2.61	1.84	-0.77
90	0.96	0.33	-0.63	1.69	0.47	-1.22	2.62	1.94	-0.68
120	0.88	0.32	-0.56	1.74	0.47	-1.27	2.68	2.06	-0.62
180	0.64	0.30	-0.34	1.36	0.45	-0.91	2.12	1.20	-0.92
270	0.79	0.31	-0.48	1.32	0.45	-0.87	1.87	1.50	-0.37
360	0.67	0.31	-0.36	1.00	0.46	-0.54	1.42	1.24	-0.18
540	0.58	0.34	-0.24	0.88	0.47	-0.41	1.25	1.17	-0.08

Results demonstrate existing OSD basins provide a reduction in downslope storm flows from post-development to pre-development for all storm durations modelled for the 2, 10 and 100 year ARI events. OSD basins therefore achieve objectives set in Section 3.1 for stormwater quantity.

3.6 OSD Storage Capacities

Stage storage relationships were determined for the OSD basins based on G R Hawkes and Associates (2010) survey (Attachment B). The

capacities from G R Hawkes are summarised in Table 4 with allowance for 500 kL permanent pool volume in the northern basin and 1000 kL in the southern - of which 500 kL is to be constructed in addition to volumes surveyed.

Table 4: OSD Capacity Details

Basin	Level (mAHD)	Measured Area (m ²)	GR Hawkes Volume ¹
North Basin	52.0	227	0
	52.9	800	500 ²
	53.0	1018	545
	54.0	1415	1763
	55.0	1830	3384
	55.5	2012	4349
South Basin	59.15	1040	1000 ²
	60.0	1687	2257 ²
	60.5	1857	3142 ²
	61.0	2025	4115 ²

Note: ¹ Volume based on survey provided by G R Hawkes and Associates with ²assumed increase of 500 kL for Southern Basin; 500kL / 1000 kL of permanent pool volume is provided below outlet invert of the Northern and Southern basins respectively.

Based on Table 4, the required OSD volumes of the North Basin and South Basin were determined to be 2.9 ML (3.4 ML including retained 0.5 ML) and 2.6 ML (3.1 ML including retained 1.0 ML) respectively at the invert of the outlet weir.

3.7 Outlet Structures

Table 5 provides details of the outlet structures modelling in DRAINS and required for the OSD basins to achieve adequate retention of flow.

Table 5: OSD Outlet Details

Basin	Low Level Outlet			Weir Data	
	Type	Diameter (mm)	Centre Elevation (mAHD)	Crest Length (m)	Crest Level (mAHD)
North Basin	Orifice	500	53.2	10	55.0
South Basin	Orifice	475	59.4	10	60.5

3.8 Conclusion

DRAINS modelling undertaken by Martens and Associates (2011) concluded that OSD basins as recommended are adequate (with appropriate outlets) to provide a reduction in downslope storm flows from post-development to pre-development for both the 'north' and 'south' catchments.

4 Water Quality Management

4.1 Overview

Water quality MUSIC modelling of the operational area has been previously undertaken by Storm Consulting (2008). Due to modification to OSD basin/wetland configuration, Blacktown City Council have requested in recent correspondence (August 12, 2011) that this modelling be reassessed.

Subsequent to the assessment as completed in Version 4 of this report Council has requested further modifications and amendments to the submitted MUSIC model. These requests and responses are discussed below.

4.2 Blacktown City Council Water Quality Submission

BCC requested a number of model changes, responses to which are provided below:

- Re-use volumes assumed in modelling are 110 kL/day drawn from the two basins. This figure has been determined in consultation with the site operator (annual site usage of 40 ML) and is assigned in the model as 60 kL/day and 50 kL/day from the North and South basins respectively. This estimate is based on existing comparable operators and anticipated site processes and takes into account periods when usage will be lower due to rainfall etc.

For the purposes of modelling it is assume that the usage is evenly spread through the year as this is the simplest means to enter this parameter to MUSIC. The figure of 110 kL/day reflects 40 ML/year spread across the year, it is acknowledged that on some days a greater amount shall be used and on others a lesser amount.

- Modify treatment of OSD capacity in wetland treatment nodes – BCC considers that inclusion of the extended detention depth (OSD) in the MUSIC model shall result in over estimation of pollutant removal. Analysis of DRAINS inflow / outflow hydrographs indicates that the inclusion of the OSD volume shall result in detention of water for periods comparable to that modelled by MUSIC using input parameters relied on. The inclusion of this as a component of the model is therefore considered reasonable.

Notwithstanding Martens' position that inclusion is appropriate, remodelling without this parameter shows that the change to the final treatment train efficiency is less than 1 % for nutrients (i.e. resolution of reporting) and at most 1 % for suspended solids (northern wetland). To satisfy Council's request amended modelling has assumed a nominal 0.25 m extended detention depth.

- Impervious percentage – the areas of the site to be used for stockpiling aggregates was treated differently between the water quality and water quantity assessment in Version 4 of this report. The stockpiles were considered pervious for the frequent lower intensity water quality storms and impervious for the higher intensity rarer OSD events.

While this rationale is considered sound and reasonable, for the purposes of conservatism and to address Council's request, the stockpile area shall be treated as impervious in this revision of the assessment.

- Rainfall - Runoff and catchment generation rates – the modelling has been updated to reflect BCC requested input parameters.

4.3 Blacktown City Council Water Quality Objectives

Blacktown City Council's *Stormwater Quality Control Policy* (2005) water quality objectives are adopted as the site's water quality objectives. The following pollutant reduction objectives are set out for comparing the post-development untreated versus treated scenarios:

- 90% reduction in gross pollutants;
- 80% reduction in suspended solids; and
- 45% reduction in nitrogen and phosphorus.

4.4 Treatment Train

The proposed water quality treatment train for the site is:

1. Rainwater Tanks -

Water balance modelling undertaken by Storm (2008) calculated that 40kL of tank capacity is required to capture roofwater runoff for reuse in toilet flushing and other site uses not addressed by stormwater harvested from OSD basins.

2. Gross Pollutant Traps (GPT) -

Tank overflow, surface runoff from the remaining operational areas and parts of the site draining west are directed through gross pollutant traps (GPTs).

3. Bioremediation Area

In addition to the GPTs and water quality wetlands already constructed on-site (basins not 'fitted out' as wetlands at this stage) it is required that a bioremediation basin be constructed to further treat runoff in the northern catchment.

The required basin shall have the following specifications:

- Filter area: 450 m²
- Extended Detention Area: 450 m²
- Extended detention depth : 400 mm
- Filter Depth: 500 mm
- Filter media permeability: 200 mm/hour for specification and 100 mm/hr ultimate (as used in modelling to reflect blockage of media over time)
- Constructed with a system of underdrains discharging into the OSD basin / wetland system.

4. Water Quality Wetland

GPT discharge is directed to one of 2 wetlands to be used for OSD and stormwater treatment/reuse ('OSD North' and 'OSD South'). As earlier outlined Dial-a-dump confirms an annual average daily flow of 110 kL/day of water is required for site uses (we assume water is preferentially drawn from the northern basin). This volume is in excess of water used from roof water tanks.

The southern basin is to be deepened at the western side to provide an additional 500 kL of storage capacity below the current invert. The design basin capacity therefore shall be increased to 1000 kL below the outlet invert to maximise the stored water for re-use during dry periods. The northern basin shall remain as built (other than inlet / outlet structures and wetland planting).

4.5 Modelling Methodology

The treatment train efficiency of the trains leading to the outlets of the north and south basins was assessed using MUSIC V5.

4.6 Model Inputs

MUSIC model inputs were sourced as follows:

- A 6-minute daily time step climate file was used for the revised MUSIC modelling. This file was supplied by Blacktown City Council.
- Rainfall – Runoff parameters as provided by Council.
- OSD volumes are based on G R Hawkes and Associates (2010) survey (Attachment B) as amended by this report.

4.7 MUSIC Results

MUSIC model results are provided in Table 6 with catchments and MUSIC model layout provided in Figure 1 (Attachment F).

Table 6: Treatment train pollutant reduction rates modelled in MUSIC.

Parameter	Northern Basin (%)	Southern Basin (%)
Total Suspended Solids	87.3	84.0
Total Phosphorous	69.8	73.2
Total Nitrogen	45.3	45.0
Gross Pollutants	100	100

4.8 Conclusion

Pollutant reductions achieved project water quality objectives (Section 4.3) and therefore Blacktown City Council's requirements demonstrating that the proposed treatment train is adequate for the treatment of site runoff to an appropriate standard.

5 Conclusion

5.1 Water Quantity

DRAINS modelling undertaken by Martens and Associates (2011) confirms OSD basins specified are adequate to achieve post-development storm flows less than pre-development flows for all events required by Blacktown City Council.

5.2 Water Quality

Outcomes from MUSIC modelling confirm that water quality pollutant reduction rates achieved by the proposed treatment train and are in accordance with relevant local policy and are therefore adequate.

5.3 Water Quality Monitoring

Council's Stormwater Quality Control Policy (2005) has now been superseded and quarterly testing is unlikely to demonstrate compliance with Council policy requirements.

Accordingly, it will be necessary to monitor storm events to determine the overall pollutant load reductions. A monitoring and testing program will be implemented to the satisfaction of OEH that tests a certain agreed number of storms at the inlet to the treatment train and at the discharge point.

6 References

LandPartners (2011) *'Area D Finished Surface Contours'*

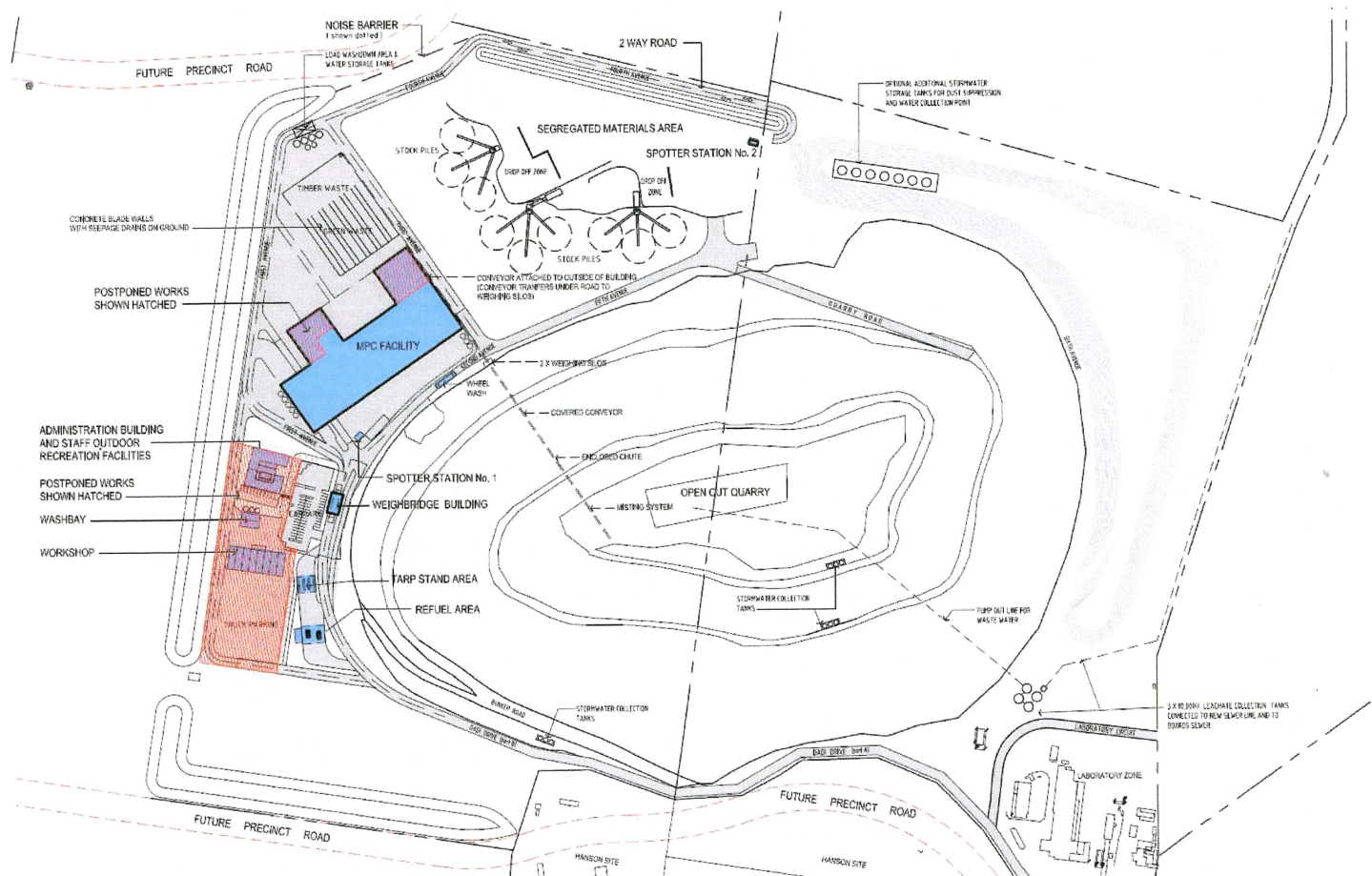
Storm Consulting Pty Ltd (2008) *'Site Surface Water Management Plan'*

Storm Consulting Pty Ltd (2009) *'Light Horse Business Centre Pavement Setout and Drainage Plans'*

G R Hawkes and Associates (2010) *'Volume Capacity Southern Basin'*
and *'Volume Capacity North Basin'*

7 Attachment A – Operational Area Layout (Storm Consulting, 2009)

APPENDIX 1 - SITE PLAN & BUILDING ELEVATIONS



LIGHT HORSE BUSINESS CENTRE
DADI - MATERIALS PROCESSING FACILITY
DIAL A DUMP INDUSTRIES
OLD WALGROVE ROAD, EASTERN CREEK, NSW



AXIS
architectural

2 / 107 Cronulla Street, Cronulla NSW 2230
p + 02 9523 7858 e + david@axisarchitects.com.au
nominated architect - David McDonald NSW ARB - 7997

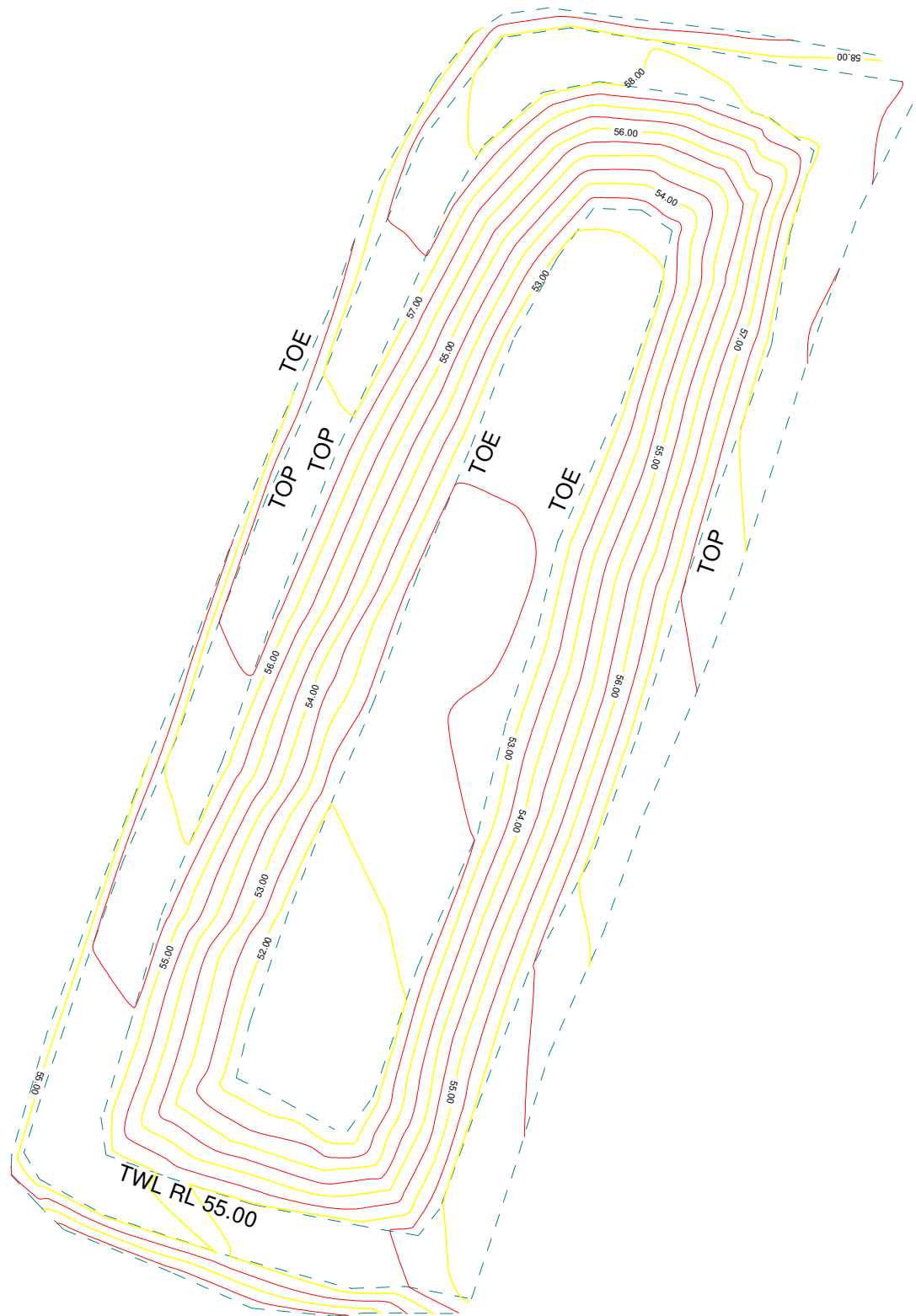


SITE LAYOUT PLAN
OVERALL SITE

MARCH 2010
1:3000 scale

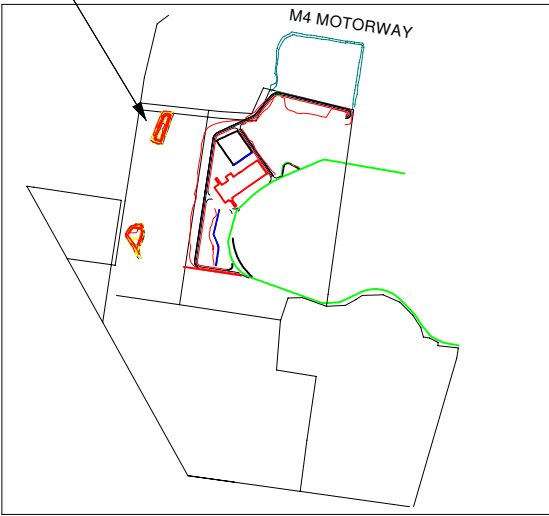
090507/DI-EP-MP A 101-8 / A

**8 Attachment B – G R Hawkes and Associates (2010) OSD
Basin Survey**



VOLUMES:	
=====	
Cut to Fill Ratio:	0.000
Cut:	0.0 (cubic metres)
Fill:	3492.8 (cubic metres)
Net:	3492.8 (cubic metres) [fill]
Litres	3,492,800L

NORTH BASIN



Revision	Date	Description	Drawn By	Approved By
B	13/01/11	Amendments: Volume	AJ	GH
A	1/09/10	Detail Survey	PE	GH

Surveyed By:



GRH Lawkes & Associates Pty Limited

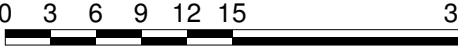
E-Mail: grh@grh.com.au | Mob: 0419 249 667

Web: www.grh.com.au

Title:

VOLUME CAPACITY NORTH BASIN
DIAL A DUMP
LIGHT HORSE BUSINESS CENTRE
EASTERN CREEK

Scale Bar:



Surveyed By:	Job No:	Contour Interval:
AJ	1084	0.50
Print Scale:	Projection:	Datum:
1: 500 @ A3	MGA	AHD

Notes:

#Volume was calculated to Top of Water Level = RL 55.00

Computer File:

S:\GRH\GRH JOB Documents\PE 1084 EASTERN CREEK FILL AREA 26 08 10

Surveyed For:

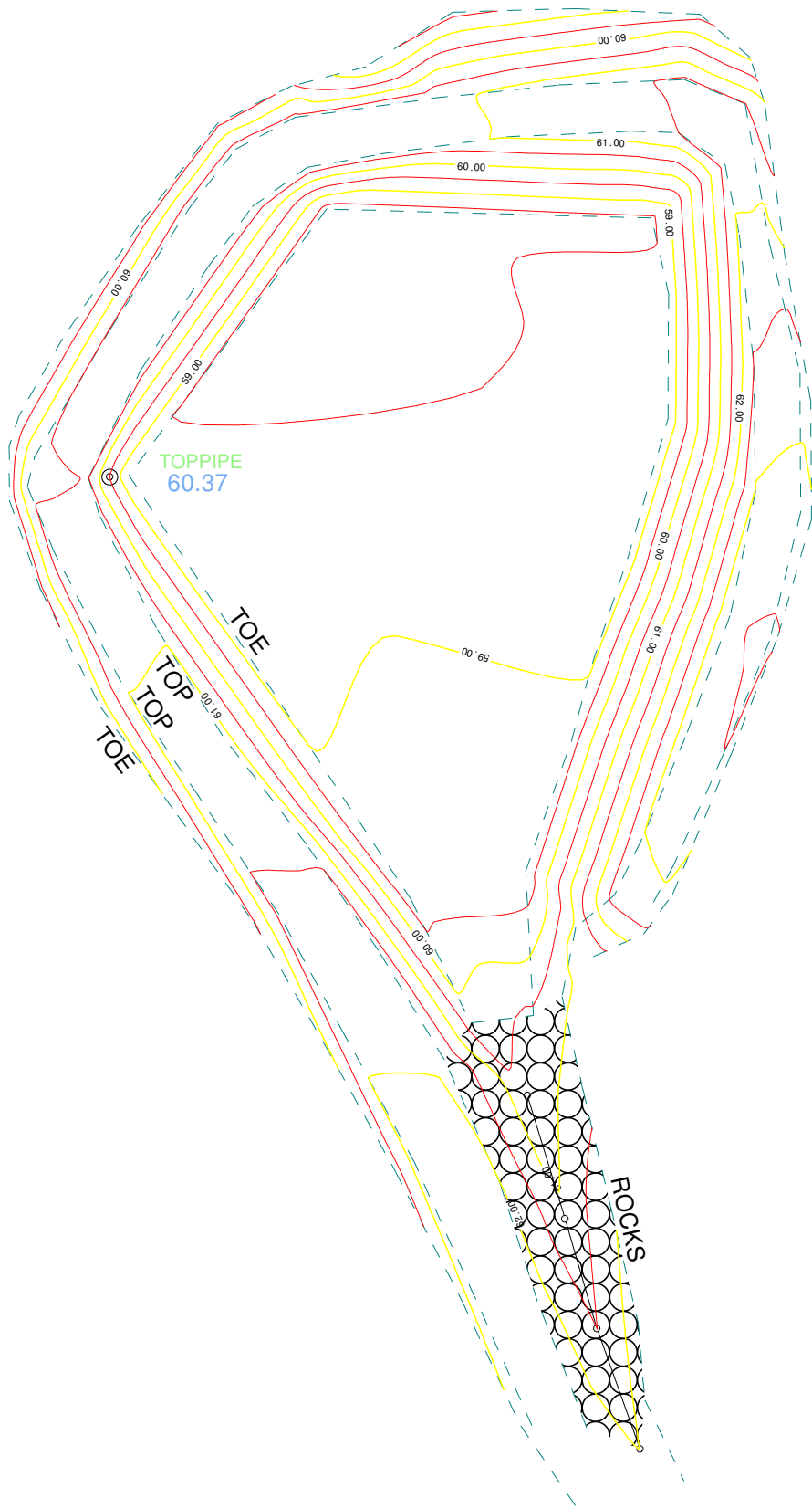


DIAL A DUMP INDUSTRIES
KEEPING AUSTRALIA CLEAN

Rev:

B

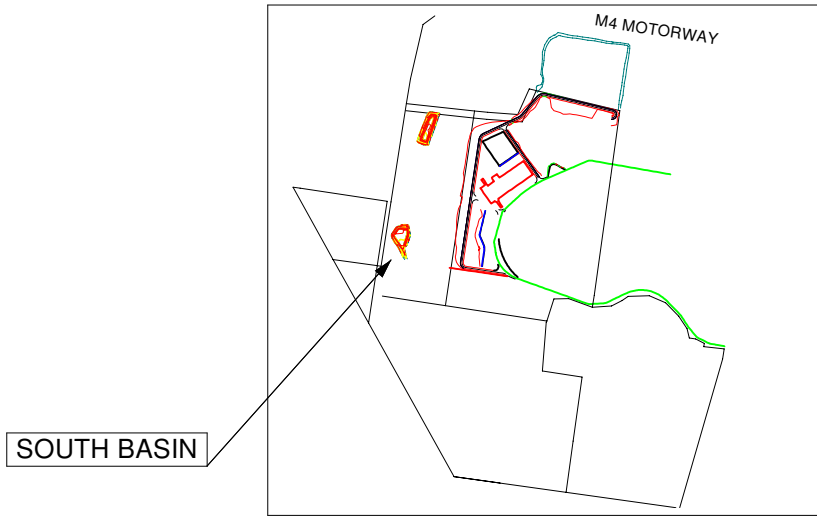
Page: 1 - 1



VOLUMES:

=====
Cut to Fill Ratio: 0.000

Cut: 0.0 (cubic metres)
Fill: 2575.5 (cubic metres)
Net: 2575.5 (cubic metres) [fill]
Liters 2,575,500L



B	13/01/11	Amendments: Information Removed	AJ	GH
A	1/09/10	Detail Survey	PE	GH
Revision	Date	Description	Drawn By	Approved By

Surveyed By:

 **Lawkes & Associates** Pty Limited

E-Mail: grh@grh.com.au | Mob: 0419 249 667
Web: www.grh.com.au

Title:

**VOLUME CAPACITY SOUTH BASIN
DIAL A DUMP
LIGHT HORSE BUSINESS CENTRE
EASTERN CREEK**

Scale Bar:

0 3 6 9 12 15 30

Surveyed By: PE Job No: 1084 Contour Interval: 0.50

Print Scale: 1: 500 @ A3 Projection: MGA Datum: AHD

Notes:

#Volume was calculated to top of outlet pipe.
#Volume is approximate only.
#Toe of basin was approximated due to the water in the basin at time of survey.

Computer File: S:\GRH\GRH JOB Documents\

Surveyed For:

 **DIAL A DUMP**
INDUSTRIES
KEEPING AUSTRALIA CLEAN

Rev: B Page: 1 - 1

**10 Attachment D – NSW Dam Safety Committee Advice
(2010)**

Megan Bowling

From: Charles Navaratne <charles@damsafety.nsw.gov.au>
Sent: Thursday, 16 June 2011 1:02 PM
To: Jacqueline Brauman
Subject: RE: D1 Form

Hi Jackie,

As per your data, the OSD's are located below ground level.
Hence DSC would not prescribe them.

Regards

Charles Navaratne
Small Dams Engineer
NSW Dams Safety Committee
Level 3, 10 Valentine Av.
Parramatta NSW 2150

Phone 02-98957848
Fax 02-98957354

From: Jacqueline Brauman [<mailto:JacquelineBrauman@dadi.com.au>]
Sent: Thursday, 16 June 2011 12:29 PM
To: charles@damsafety.nsw.gov.au
Subject: FW: D1 Form

Hi Charles,

Further to our telephone conversation earlier this morning about OSD sizes at Eastern Creek, I have attached a D1 form and drawings for each OSD.

I have also attached the Blacktown Council's Concept Masterplan with crosses marking the OSD locations. The natural slope is to the west.

I would be pleased if you could advise immediately whether the OSDs need to be prescribed.

Thanks for your help.

Regards,

Jacqueline Brauman | Solicitor
Dial A Dump Industries | Keeping Australia Clean | 32 Burrows Road Alexandria NSW 2015
Contact | **P:** (02) 9519-9999 | **F:** (02) 9516-5559 | **W:** www.dadi.com.au

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From: Charles Navaratne [<mailto:charles@damsafety.nsw.gov.au>]
Sent: Tuesday, 30 March 2010 3:05 PM
To: Steve Baxter
Subject: RE: D1 Form

Hi Steve,

11 Attachment E – DRAINS Catchment Plan



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martens
6/37 Leighton Place
Hornsby, NSW 2077 Australia
Phone: (02) 9476 9999
Fax: (02) 9476 8767
Email: mail@martens.com.au
Internet: <http://www.martens.com.au>

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ASSOCIATES PTY LTD**
Sustainable Solutions
Environmental - Geotechnical - Civil
Hydraulic - Wastewater Engineers

CLIENT
**DIAL A DUMP INDUSTRIES
PTY LTD**

THIS PLAN MUST NOT BE USED FOR CONSTRUCTION UNLESS
SIGNED AS APPROVED BY PRINCIPAL CERTIFYING AUTHORITY
All measurements in mm unless otherwise specified.

TITLE
**PRE DEVELOPMENT CATCHMENT PLAN -
LIGHT HORSE BUSINESS PARK, EASTERN CREEK, NSW**

PROJECT MANAGER:
AN

DRAWING NUMBER:
P1103002JD02V03

DESIGNED:
SL

DRAWN:
SL

REVIEWED:
AN

DATUM:
mAHD

HORIZONTAL RATIO:
1:1500 @ A1
1:3000 @ A3

VERTICAL RATIO:
1:1500 @ A1
1:3000 @ A3

SHEET
1
OF 2
SHEETS

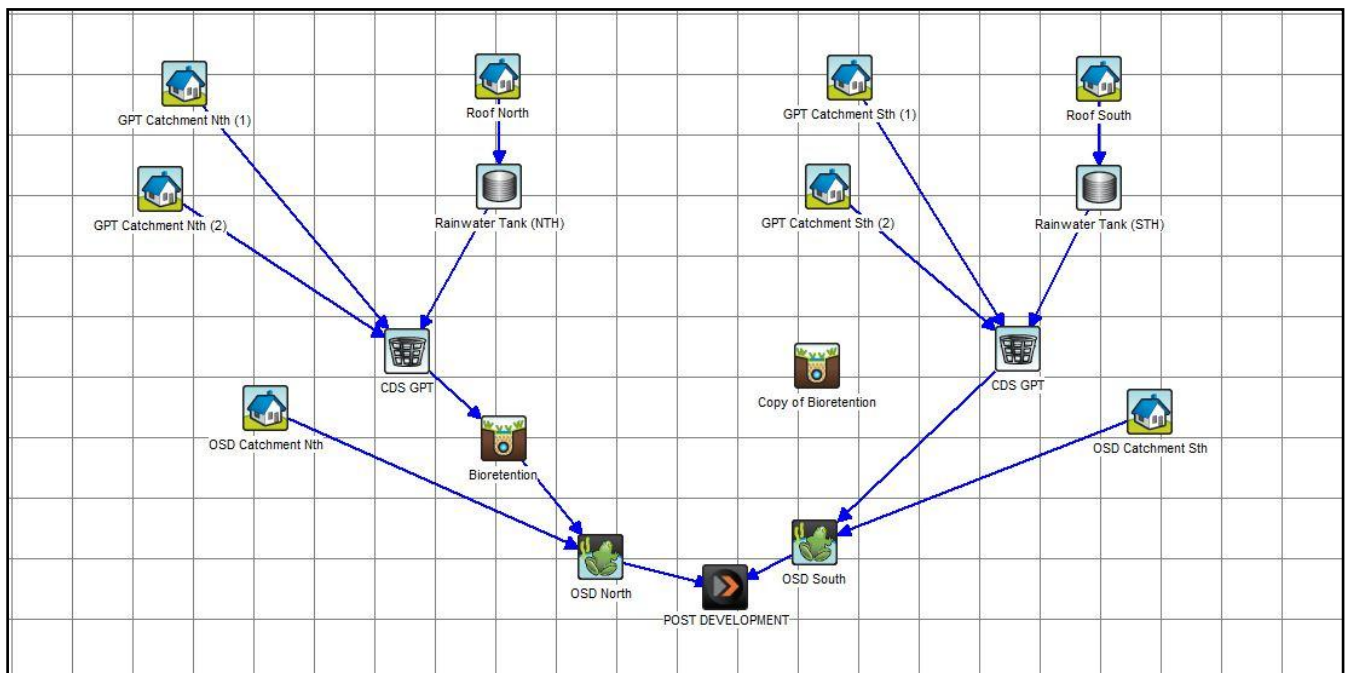
PAPER SIZE:
A1 / A3

REV.	DESCRIPTION	DATE	ISSUED
1	ATTACHMENT OF CONSOLIDATED STORMWATER MANAGEMENT PLAN	19.07.2011	SL
2	FINAL CONSOLIDATED STORMWATER MANAGEMENT PLAN	20.09.2011	AN
3	AMENDED FINAL CONSOLIDATED STORMWATER MANAGEMENT PLAN	21.09.2011	AN



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12 Attachment F – MUSIC Model Layout



Appendix C

OSD Catchment Plan



B	ISSUED FOR INFORMATION	18-05-15
A	ISSUED FOR INFORMATION	16-12-14
Issue	Description	Date

Bar Scales

THIS DRAWING CANNOT BE
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ANY FORM OR USED FOR ANY
OTHER PURPOSE OTHER THAN
THAT ORIGINALLY INTENDED
WITHOUT THE WRITTEN
PERMISSION OF AT&L

Client

DIAL A DUMP

Scales
NTS

Grid

Height
Datum

Drawn

Designed

Checked

Approved

Project

PRE SORT CENTRE
EASTERN CREEK

Title

OSD CATCHMENT
PLAN

Civil Engineers and Project Managers

at&l

Suite 702, 154 Pacific Hwy
St Leonards NSW 2065
ABN 96 130 882 405
Tel: 02 9439 1777
Fax: 02 9460 8413
www.atl.net.au
info@atl.net.au

Status

FOR INFORMATION

A1

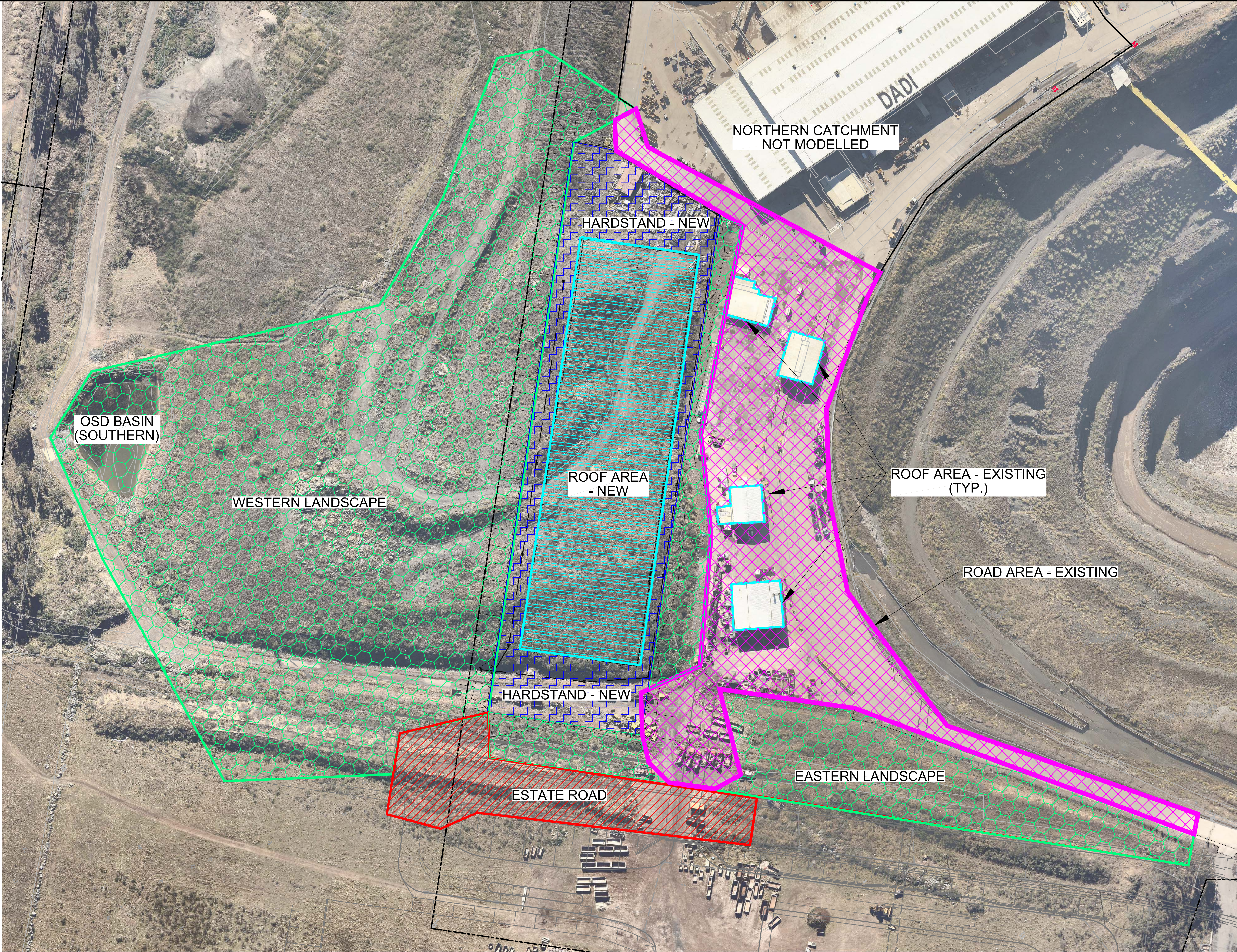
Drawing No.
SKC01

Project No.
14-232

Issue
B

Appendix D

Music Data and Catchment Plan



B	ISSUED FOR INFORMATION	18-05-15
A	ISSUED FOR INFORMATION	15-12-14
Issue	Description	Date

Bar Scales

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OTHER PURPOSE OTHER THAN
THAT ORIGINALLY INTENDED
WITHOUT THE WRITTEN
PERMISSION OF AT&L

Client

DIAL A DUMP

Scales	Drawn	FX
NTS	Designed	FX
Grid	Checked	AM
Height Datum	Approved	

Project	PRE SORT CENTRE EASTERN CREEK
Title	MUSIC CATCHMENTS

Civil Engineers and Project Managers at&l Suite 702, 154 Pacific Hwy St Leonards NSW 2065 ABN 96 130 882 405 Tel: 02 9439 1777 Fax: 02 9460 8413 www.atl.net.au info@atl.net.au		
Status	FOR INFORMATION	A1
Drawing No.	Project No.	Issue
SKC02	14-232	B

Appendix E

Civil Siteworks and Drainage Plans

PRE SORT CENTRE

PROPOSED CIVIL WORKS

FOR S75W APPROVAL

DRAWING LIST

GENERAL

- DAC001 COVER SHEET AND LOCALITY PLAN
- DAC002 NOTES AND LEGENDS

BULK EARTHWORKS

- DAC003 BULK EARTHWORKS CUT AND FILL PLAN

TYPICAL SECTIONS

- DAC004 TYPICAL SECTIONS SHEET 1
- DAC005 TYPICAL SECTIONS SHEET 2

SITEWORKS AND STORMWATER DRAINAGE PLAN

- DAC010 GENERAL ARRANGEMENT PLAN
- DAC011 SITEWORKS AND STORMWATER DRAINAGE PLAN SHEET 1
- DAC012 SITEWORKS AND STORMWATER DRAINAGE PLAN SHEET 2
- DAC013 SITEWORKS AND STORMWATER DRAINAGE PLAN SHEET 3

PAVEMENT PLANS

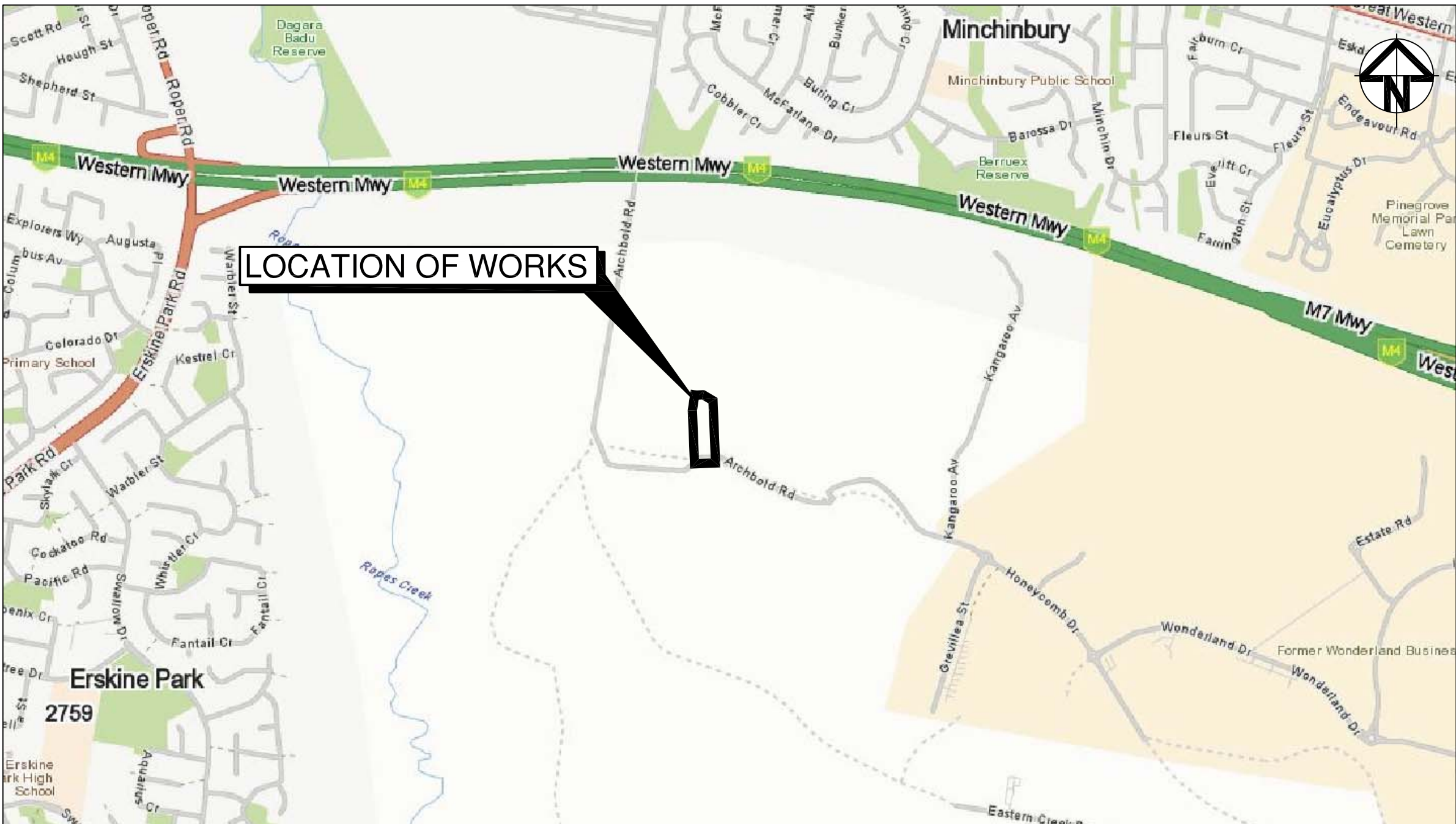
- DAC020 PAVEMENT PLAN

EROSION AND SEDIMENTATION CONTROL

- DAC021 EROSION AND SEDIMENTATION CONTROL PLAN

SITEWORKS DETAILS

- DAC030 SITEWORKS DETAILS
- DAC031 DRAINAGE DETAILS



LOCALITY PLAN
N.T.S

		Bar Scales	THIS DRAWING CANNOT BE COPIED OR REPRODUCED IN ANY FORM OR USED FOR ANY OTHER PURPOSE OTHER THAN THAT ORIGINALLY INTENDED WITHOUT THE WRITTEN PERMISSION OF AT&L	Client DIAL A DUMP	Scales NTS Grid MGA Height Datum AHD	Drawn	FX	Project PRE SORT CENTRE EASTERN CREEK	 Suite 702, 154 Pacific Hwy St Leonards NSW 2065 ABN 96 130 882 405 Tel: 02 9439 1777 Fax: 02 9460 8413 www.atl.net.au info@atl.net.au			
						Designed	FX					
						Checked	AM					
						Approved						
										Title COVER SHEET AND LOCALITY PLAN	Status FOR APPROVAL	A1
B	ISSUED FOR APPROVAL					03-03-15						
A	ISSUED FOR APPROVAL					15-12-14						
Issue	Description					Date						

THE EXISTING SITE CONDITIONS SHOWN ON THE FOLLOWING DRAWINGS HAVE BEEN INVESTIGATED BY LAND PARTNERS, BEING REGISTERED SURVEYORS. THE INFORMATION IS SHOWN TO PROVIDE A BASIS FOR DESIGN. AT & L DOES NOT GUARANTEE THE ACCURACY OR COMPLETENESS OF THE SURVEY BASE OR ITS SUITABILITY AS A BASIS FOR CONSTRUCTION DRAWINGS.

EXISTING UNDERGROUND SERVICES
NOTES

CONTRACTORS ARE TO UNDERTAKE A SERVICES SEARCH, PRIOR TO COMMENCEMENT OF WORKS ON SITE. SEARCH RESULTS ARE TO BE KEPT ON SITE AT ALL TIMES.

- (A) STORMWATER DESIGN CRITERIA:
 - (1) AVERAGE CONCENTRATION INTERVAL:
 - 100 YEARS: 100% TO SURCHARGE PIT
 - 120 YEARS: EXTERNAL PAVEMENTS
 - (2) RAINFALL INTENSITIES:
 - TIME OF CONCENTRATION: 5 MINUTES
 - 100 YEARS: 218 mm/hr
 - 120 YEARS: 167 mm/hr
 - (3) RUNOFF COEFFICIENTS:

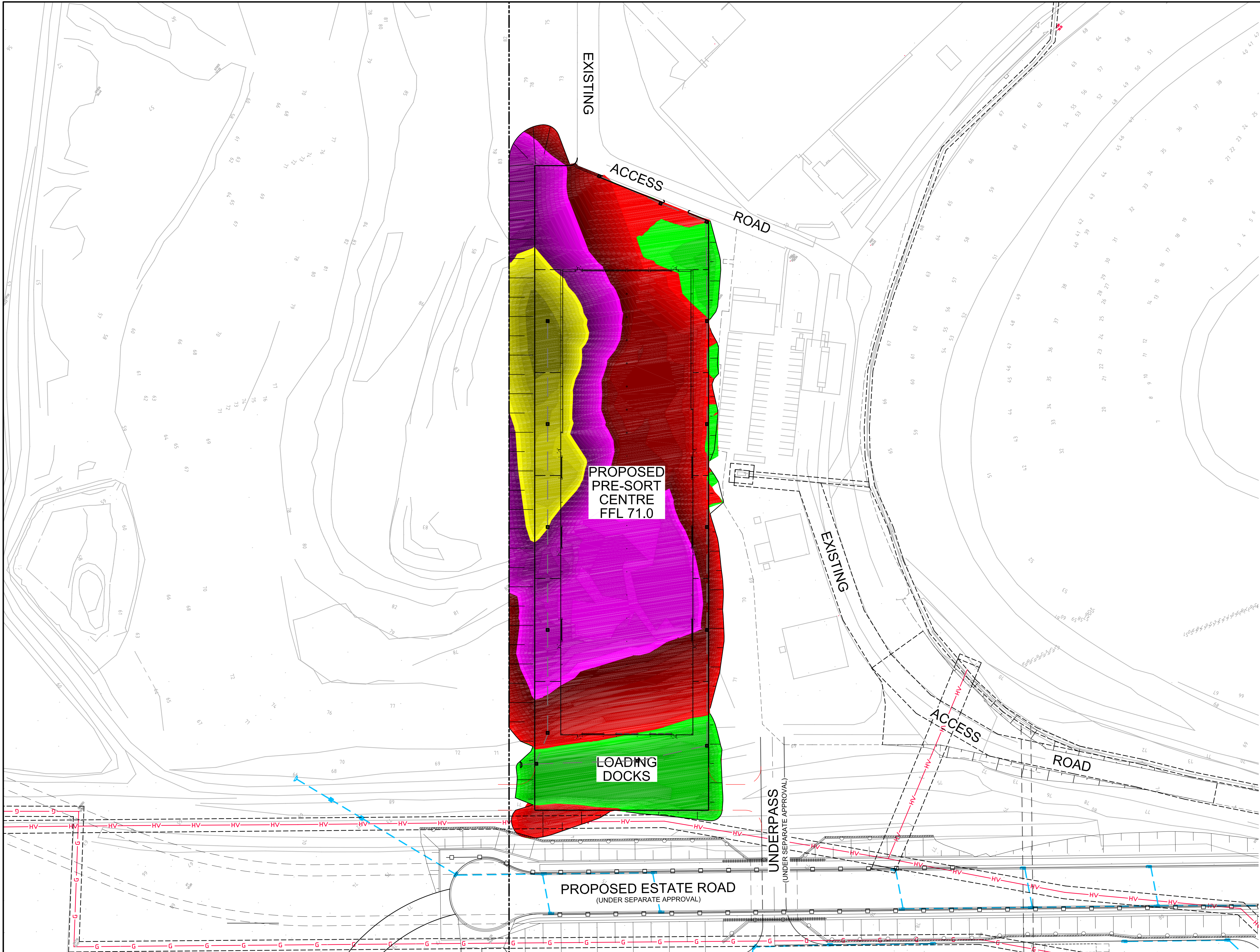
ROOF AREAS:	$\frac{C_{100}}{C_{20}}$	=10
EXTERNAL PAVEMENTS:	$\frac{C_{100}}{C_{20}}$	=10
2. PIPES 300+ DIA. AND LARGER TO BE REINFORCED CONCRETE CLASS '2' APPROVED SPIGOT AND SOCKET WITH RUBBER RING JOINTS, U.N.O.
3. PIPES UP TO 300 DIA SHALL BE SEWER GRADE uPVC WITH SOLDER WELDED JOINTS.
4. EQUIVALENT STRENGTH VCP OR FRC PIPES MAY BE USED.
5. ALL STORMWATER DRAINAGE LINES UNDER PROPOSED SLABING TO BE uPVC PRESSURE PIPE GRADE 6. ENSURE ALL VERTICALS AND DOWNPIPES ARE uPVC PRESSURE PIPE, GRADE 6 FOR A MIN OF 3.0m IN HEIGHT.
6. PIPES TO BE INSTALLED TO TYPE HS3 (ROAD) HS2 (LOTS) SUPPORT IN ACCORDANCE WITH AS 3725 (1989) IN ALL CASES BACKFILL TRENCH WITH SAND TO 300mm ABOVE PIPE. WHERE PIPE IS UNDER PAVEMENTS BACKFILL BEHIND PIPE TO TOP OF TRENCH TO UNDERSIDE OF PAVEMENT WITH SAND OR APPROVED GRANULAR MATERIAL COMPACTED IN 150mm LAYERS TO MINIMUM 98% STANDARD MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS 1289 5.2.1. (OR A DENSITY INDEX OF NOT LESS THAN 75)
7. ALL INTERNAL WORKS WITHIN PROPERTY BOUNDARIES ARE TO COMPLY WITH THE REQUIREMENTS OF AS 3500 3.1 (1998) AND AS/NZS 3500 3.2 (1998).
8. PRECAST PITS MAY BE USED EXTERNAL TO THE BUILDING SUBJECT TO APPROVAL BY AT & L.
9. ENLARGERS, CONNECTIONS AND JUNCTIONS TO BE PREFABRICATED FITTINGS WHERE PIPES ARE LESS THAN 300 DIA.
10. WHERE SUBSOIL DRAINS PASS UNDER FLOOR SLABS AND VEHICULAR PAVEMENTS, UNSLOTTED uPVC SEWER GRADE PIPE IS TO BE USED.
11. CARE IS TO BE TAKEN WITH LEVELS OF STORMWATER LINES. GRADES SHOWN ARE NOT TO BE REDUCED WITHOUT APPROVAL.
12. GRATES AND COVERS SHALL CONFORM TO AS 3996.
13. ALL INTERNAL PIT DIMENSIONS TO CONFORM TO AS3500.3 TABLE 8.2.
14. AT ALL TIMES DURING CONSTRUCTION OF STORMWATER PITS, ADEQUATE SAFETY PROCEDURES SHALL BE TAKEN TO ENSURE AGAINST THE POSSIBILITY OF PERSONNEL FALLING DOWN PITS.
15. ALL EXISTING STORMWATER DRAINAGE LINES AND PITS THAT ARE TO REMAIN ARE TO BE INSPECTED AND CLEANED DURING THIS PROCESS. ANY PART OF THE STORMWATER DRAINAGE SYSTEM THAT WARRANTS REPAIR SHALL BE REPORTED TO THE SUPERINTENDING/ENGINEER FOR FURTHER DIRECTIONS.

1. ORIGIN OF LEVELS:- REFER SURVEY NOTES.
2. CONTRACTOR MUST VERIFY ALL DIMENSIONS AND EXISTING LEVELS ON SITE PRIOR TO COMMENCEMENT OF WORK. ANY DISCREPANCIES TO BE REPORTED TO A & L.
3. MAKE SMOOTH CONNECTION WITH EXISTING WORKS.
4. ALL TRENCH BACKFILL MATERIAL SHALL BE COMPACTED TO THE SAME DENSITY AS THE ADJACENT MATERIAL.
5. ALL SERVICE TRENCHES UNDER VEHICULAR PAVEMENTS SHALL BE BACKFILLED WITH SAND TO 300mm ABOVE PIPE, WHERE PIPE IS UNDER PAVEMENTS BACKFILL REMAINDER OF TRENCH TO UNDERSIDE OF PAVEMENT WITH SAND OR APPROVED GRAVELAR MATERIAL COMPACTED IN 150mm LAYERS TO MINIMUM 98% MODIFIED MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS 1289 5.2.1. (OR A DENSITY IN INDEX OF NOT LESS THAN 75)
6. PROVIDE 10mm WIDE EXPANSION JOINTS BETWEEN BUILDINGS AND ALL CONCRETE OR UNIT PAVEMENTS.
7. ASPHALTIC CONCRETE SHALL CONFORM TO RMS. SPECIFICATION R116.
8. ALL BASECOURSE MATERIAL SHALL BE IGNEOUS ROCK QUARRIED MATERIAL TO COMPLY WITH RMS. FORM 3051 (UNBOUND), RMS. FORM 3052 (BOUND) COMPACTED TO MINIMUM 98% MODIFIED DENSITY IN ACCORDANCE WITH AS 1289 5.2.1. FREQUENCY OF COMPACTION TESTING SHALL NOT BE LESS THAN 1 TEST PER 50m² OF BASECOURSE MATERIAL PLACED.
9. ALL SUB-BASE COURSE MATERIAL SHALL BE IGNEOUS ROCK QUARRIED MATERIAL TO COMPLY WITH RMS. FORM 3051, 3051.1 AND COMPACTED TO MINIMUM 95% MODIFIED DENSITY IN ACCORDANCE WITH AS 1289 5.2. FREQUENCY OF COMPACTION TESTING SHALL NOT BE LESS THAN 1 TEST PER 50m² OF SUB-BASE COURSE MATERIAL PLACED.
10. AS AN ALTERNATIVE TO THE USE OF IGNEOUS ROCK AS A SUB-BASE MATERIAL IN (9) A CERTIFIED RECYCLED CONCRETE MATERIAL COMPLYING WITH RMS. FORM 3051 AND 3051.1 WILL BE CONSIDERED. SUBJECT TO MATERIAL SAMPLES AND APPROPRIATE CERTIFICATIONS BEING PROVIDED TO THE SATISFACTION OF A & L.
11. SHOULD THE CONTRACTOR WISH TO USE A RECYCLED PRODUCT THIS SHALL BE CLEARLY INDICATED IN THEIR TENDER AND THE PRICE DIFFERENCE BETWEEN AN IGNEOUS PRODUCT AND A RECYCLED PRODUCT SHALL BE CLEARLY INDICATED.
12. WHERE NOTED ON THE DRAWINGS THAT WORKS ARE TO BE CARRIED BY OTHERS, (eg. ADJUSTMENT OF SERVICES), THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE CO-ORDINATION OF THESE WORKS.

7. DURING WINDY WEATHER, LARGE, UNPROTECTED AREAS WILL BE KEPT MOIST (NOT WET) BY SPRINKLING WITH WATER TO KEEP DUST UNDER CONTROL.
8. FINAL SITE LANDSCAPING WILL BE UNDERTAKEN AS SOON AS POSSIBLE AND WITHIN 20 WORKING DAYS FROM COMPLETION OF CONSTRUCTION ACTIVITIES.

— G —	EXISTING GAS
— W —	EXISTING WATER
— SW —	EXISTING STORMWATER
— HV —	EXISTING ELECTRICAL HIGH VOLTAGE

			Bar Scales	THIS DRAWING CANNOT BE COPIED OR REPRODUCED IN ANY FORM OR USED FOR ANY OTHER PURPOSE OTHER THAN THAT ORIGINALLY INTENDED WITHOUT THE WRITTEN PERMISSION OF AT&L	Client	DIAL A DUMP	Scales NTS	Drawn FX	Project	PRE SORT CENTRE EASTERN CREEK	at&l Civil Engineers and Project Managers Suite 702, 154 Pacific Hwy St Leonards NSW 2065 ABN 96 130 882 405 Tel: 02 9439 1777 Fax: 02 9460 8413 www.atl.net.au info@atl.net.au		
							Designed FX						
							Grid MGA	Checked AM					
							Height Datum AHD	Approved					
										Title	NOTES AND LEGENDS	Status FOR APPROVAL	A1
B	ISSUED FOR APPROVAL	03-03-15											
A	ISSUED FOR APPROVAL	15-12-14											
Issue	Description	Date									Drawing No. DAC002	Project No. 14-232	Issue B



HWE CUT & FILL LEGEND			
Lower_value	Upper_value		Colour
-15.5	to -14.0	2	Dark Green
-14.0	to -14.5	2	
-14.5	to -13.0	2	Green
-13.0	to -12.5	2	
-12.5	to -12.0	2	Light Green
-12.0	to -11.5	2	
-11.5	to -11.0	2	Yellow-Green
-11.0	to -10.5	2	
-10.5	to -10.0	2	Yellow
-10.0	to -9.5	2	
-9.5	to -9.0	2	Orange
-9.0	to -8.5	2	
-8.5	to -8.0	2	Red-Orange
-8.0	to -7.5	2	
-7.5	to -7.0	2	Red
-7.0	to -6.5	2	
-6.5	to -6.0	2	Dark Red
-6.0	to -5.5	2	
-5.5	to -5.0	m	Brown
-5.0	to -4.5	m	
-4.5	to -4.0	m	Dark Brown
-4.0	to -3.5	m	
-3.5	to -3.0	m	Black
-3.0	to -2.5	m	
-2.5	to -2.0	m	Dark Grey
-2.0	to -1.5	m	
-1.5	to -1.0	m	Grey
-1.0	to -0.5	m	
-0.5	to 0	m	Light Grey
0	to 0.5	m	
0.5	to 1.0	m	White
1.0	to 1.5	m	
1.5	to 2.0	m	Light Blue
2.0	to 2.5	m	

TOTAL CUT/FILL

TOTAL:	CUT	127,200m³
	FILL	3,900m³
	SURPLUS SPOIL	123,300m³

- NOTES
- THE SURPLUS SPOIL FROM THE ABOVE TABLE IS INTENDED TO BE PLACED AND COMPACTED WITHIN THE FUTURE EFW SITE
- EARTHWORKS VOLUMES DENOTED ARE APPROXIMATE ONLY AND HAVE BEEN CALCULATED BETWEEN THE EARTHWORKS SURFACE LEVEL (300mm BELOW FINISHED SURFACE LEVEL) AND THE EXISTING SURFACE LEVEL. THE VOLUMES DO NOT TAKE INTO ACCOUNT THE FOLLOWING --
1. STRIPPING AND STOCKPILING OF EXISTING TOP SOIL
 2. BULKING FACTORS OF REMOVED CUT
 3. REMOVAL OF EXISTING BUILDING SLABS AND PAVEMENTS
 4. REMOVAL AND/OR REMEDIATION OF ANY EXISTING UNCONTROLLED FILL
 5. PROPOSED LANDSCAPING
 6. STORMWATER AND UTILITIES TRENCHING
 7. EXISTING DAMS OR WATER BODIES WHERE BY THE BASE HAS NOT BEEN SURVEYED DUE TO THE PRESENCE OF WATER AT TIME OF SURVEY

Bar Scales		
0 20 40 60 80m		
1 : 750 @ A1 1 : 1000 @ A3		
B	ISSUED FOR APPROVAL	03-03-15
A	ISSUED FOR APPROVAL	15-12-14
Issue	Description	Date

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Client

DIAL A DUMP

Scales	1 : 500	Drawn	FX
		Designed	FX
Grid	MGA	Checked	AM
Height Datum	AHD	Approved	

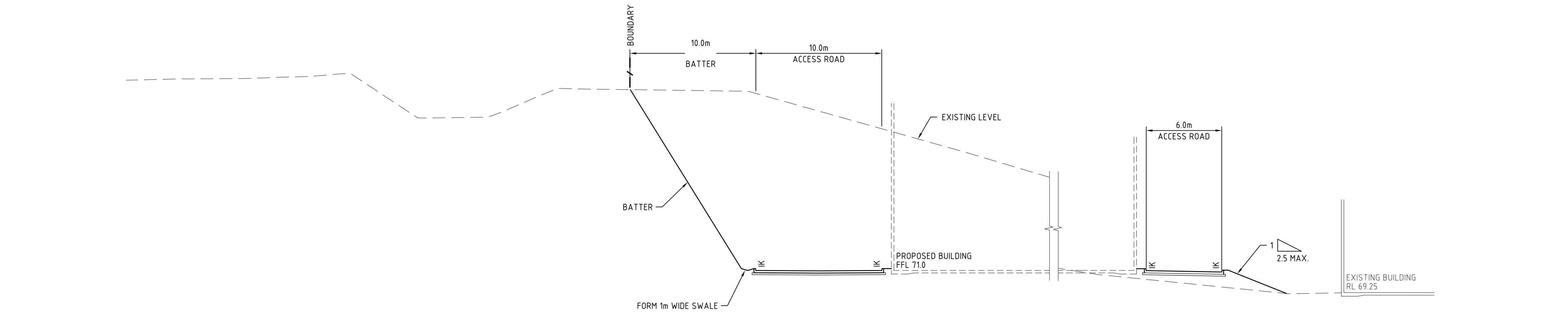
Project

PRE SORT CENTRE EASTERN CREEK

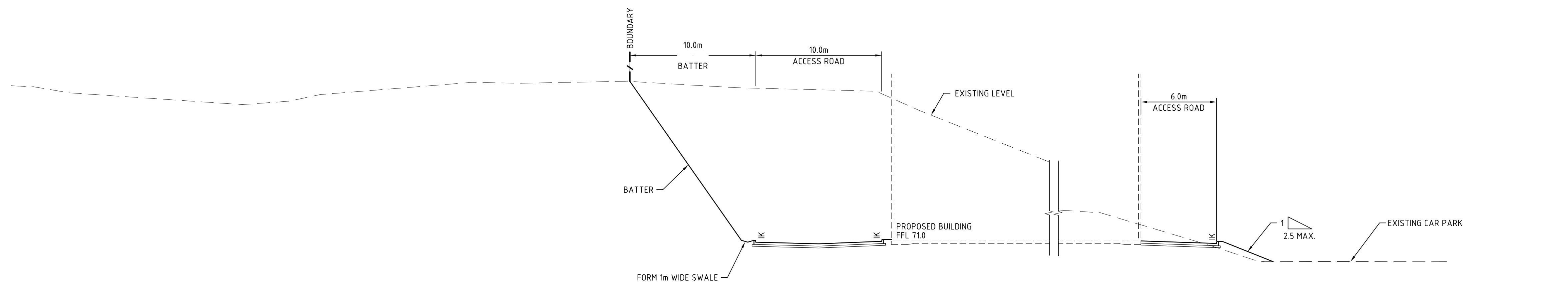
Title

BULK EARTHWORKS CUT AND FILL PLAN

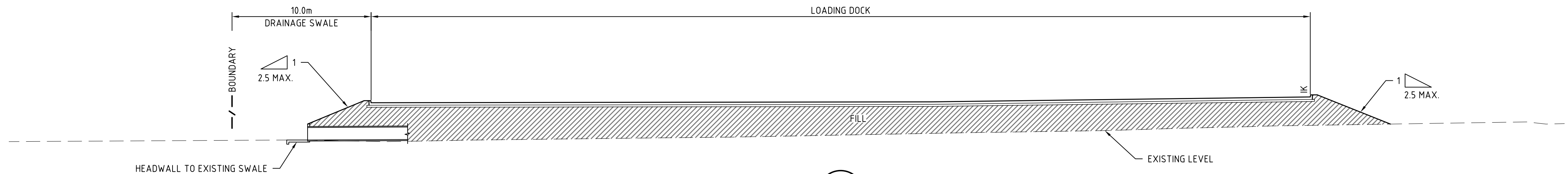
Civil Engineers and Project Managers		
at&l		
Suite 702, 154 Pacific Hwy St Leonards NSW 2065 ABN 96 130 882 405 Tel: 02 9439 1777 Fax: 02 9460 8413 www.atl.net.au info@atl.net.au		
Status	FOR APPROVAL	A1
Drawing No.	DAC003	Issue
Project No.	14-232	B



SECTION 1
1 : 200
DAC010, DAC011

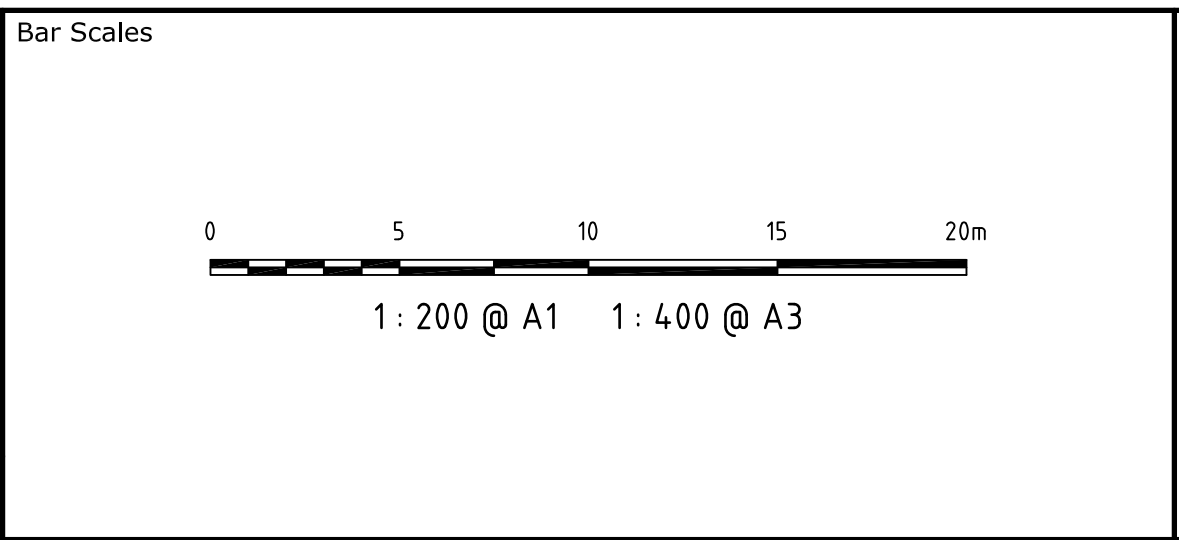


SECTION 2
1 : 200
DAC010, DAC012



SECTION 3
1 : 200
DAC010, DAC013

B	ISSUED FOR APPROVAL	03-03-15
A	ISSUED FOR APPROVAL	15-12-14
Issue	Description	Date



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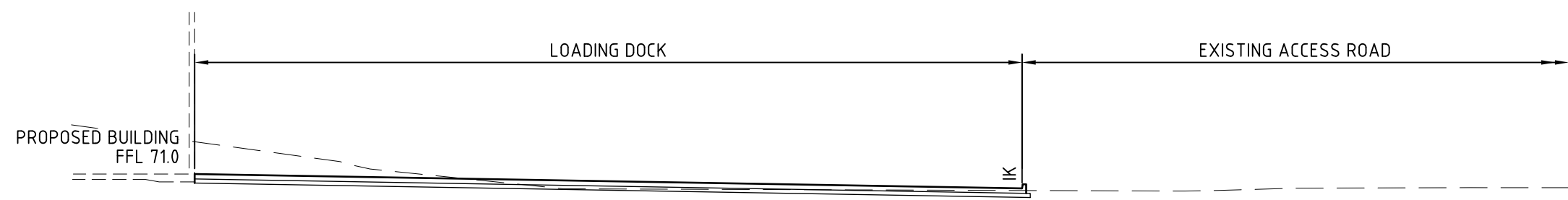
Client

DIAL A DUMP

Scales	1 : 200	Drawn	FX
		Designed	FX
Grid	MGA	Checked	AM
Height Datum	AHD	Approved	

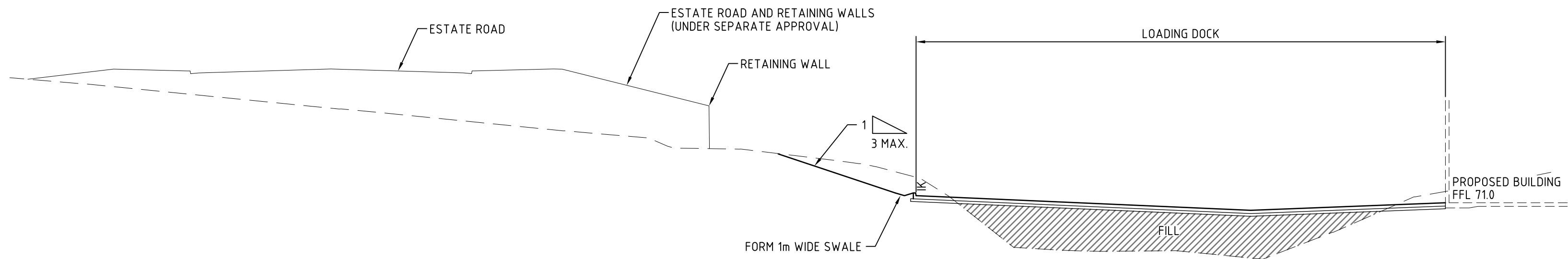
Project	PRE SORT CENTRE EASTERN CREEK
Title	TYPICAL SECTIONS SHEET 1

Civil Engineers and Project Managers		
 Suite 702, 154 Pacific Hwy St Leonards NSW 2065 ABN 96 130 882 405 Tel: 02 9439 1777 Fax: 02 9460 8413 www.atl.net.au info@atl.net.au		
Status	FOR APPROVAL	A1
Drawing No.	Project No.	Issue
DAC004	14-232	B



SECTION 4
1 : 200

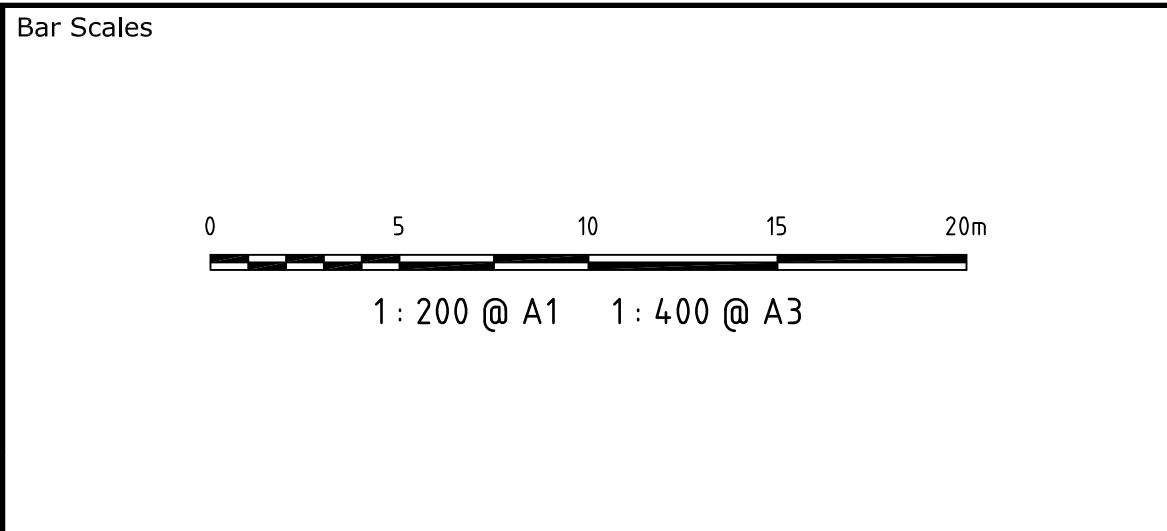
DAC010, DAC011



SECTION 5
1 : 200

DAC010, DAC013

B	ISSUED FOR APPROVAL	03-03-15
A	ISSUED FOR APPROVAL	15-12-14
Issue	Description	Date



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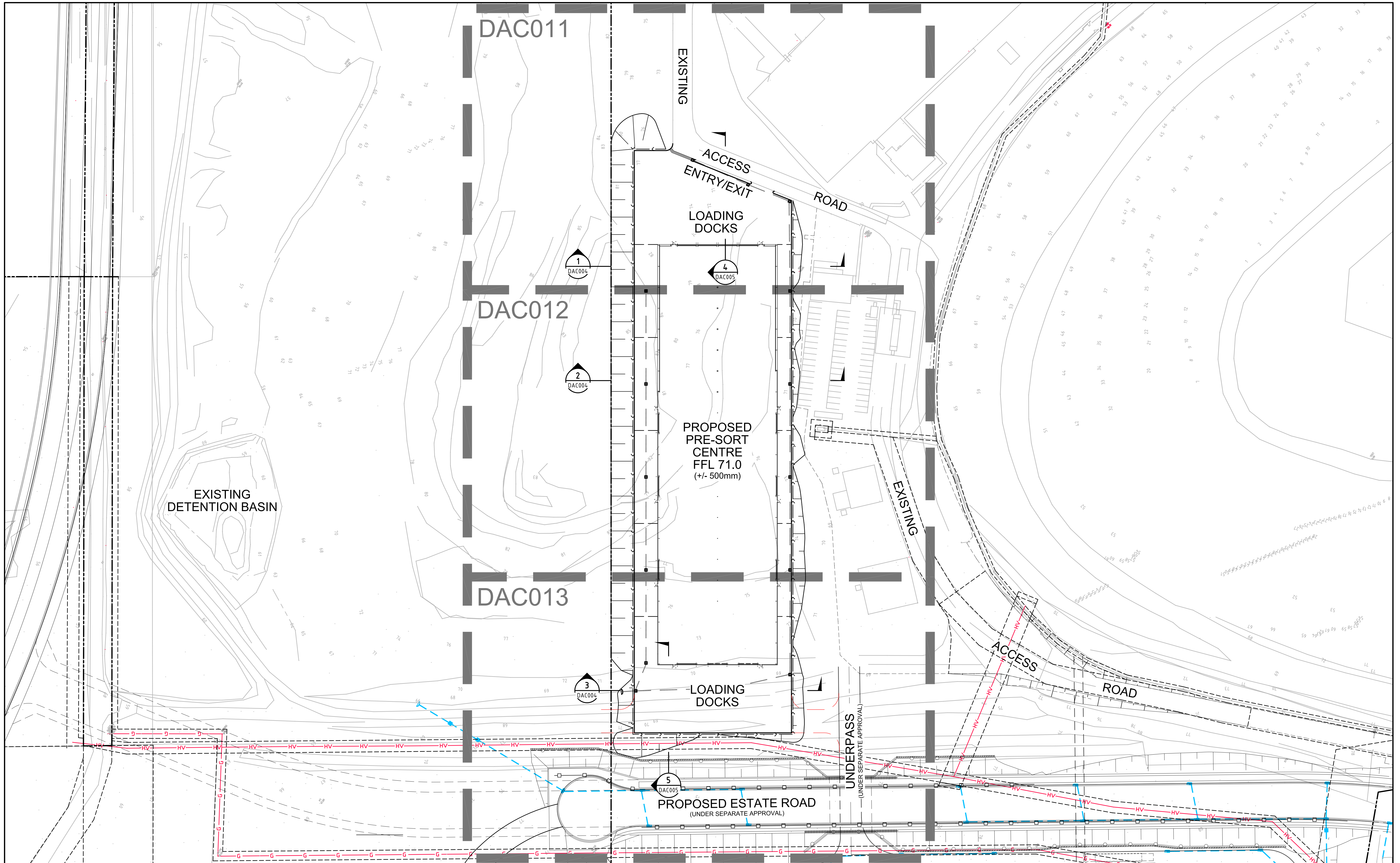
Client

DIAL A DUMP

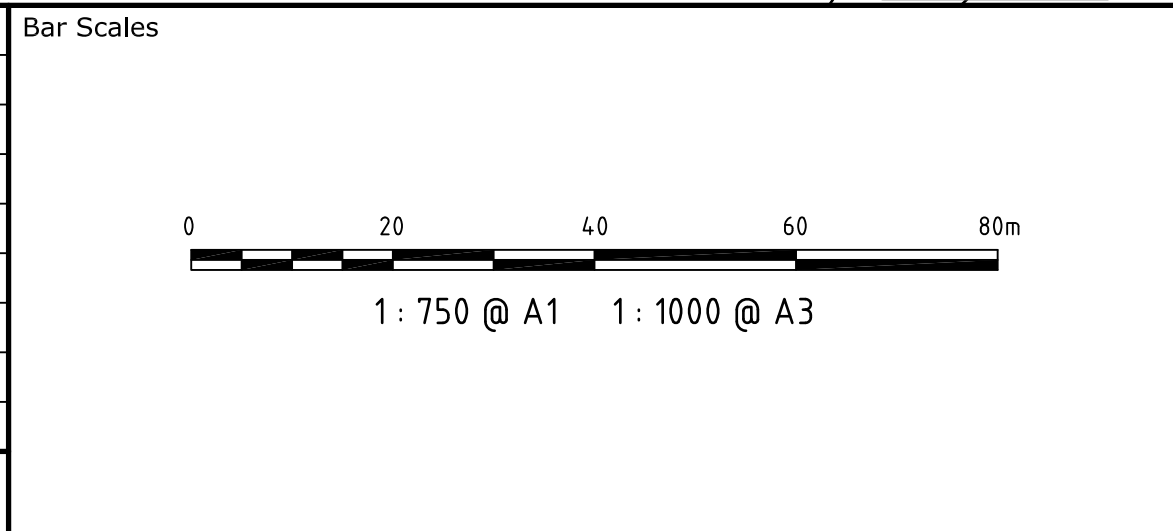
Scales	1 : 200	Drawn	FX
		Designed	FX
Grid	MGA	Checked	AM
Height Datum	AHD	Approved	

Project		Title
PRE SORT CENTRE EASTERN CREEK		
TYPICAL SECTIONS SHEET 2		

Civil Engineers and Project Managers		
at&l		
Suite 702, 154 Pacific Hwy St Leonards NSW 2065 ABN 96 130 882 405 Tel: 02 9439 1777 Fax: 02 9460 8413 www.atl.net.au info@atl.net.au		
Status	FOR APPROVAL	A1
Drawing No.	Project No.	Issue
DAC005	14-232	B



B	ISSUED FOR APPROVAL	03-03-15
A	ISSUED FOR APPROVAL	15-12-14
Issue	Description	Date

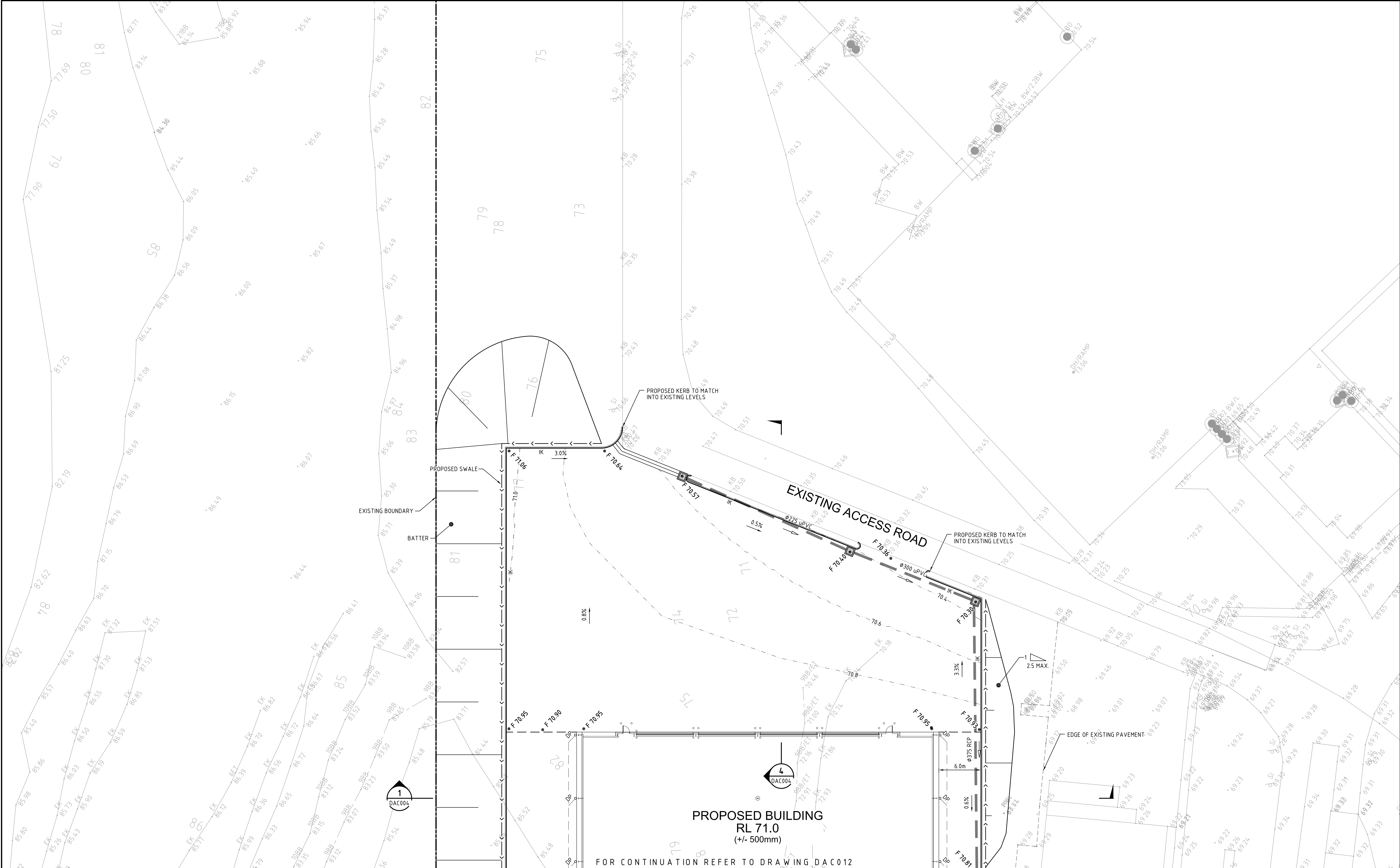


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DIAL A DUMP	

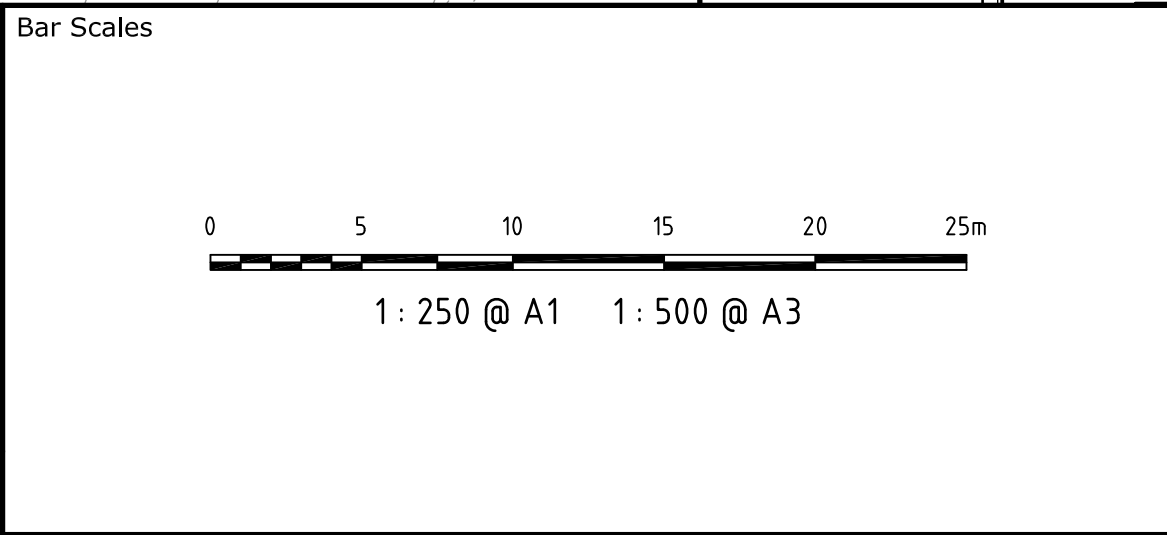
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Grid	MGA	Checked	AM
Height Datum	AHD	Approved	

Project	PRE SORT CENTRE EASTERN CREEK
Title	GENERAL ARRANGEMENT PLAN

Civil Engineers and Project Managers		
at&l		
Suite 702, 154 Pacific Hwy St Leonards NSW 2065 ABN 96 130 882 405 Tel: 02 9439 1777 Fax: 02 9460 8413 www.atl.net.au info@atl.net.au		
Status	FOR APPROVAL	A1
Drawing No.	Project No.	Issue
DAC010	14-232	B



B	ISSUED FOR APPROVAL	03-03-15
A	ISSUED FOR APPROVAL	15-12-14
Issue	Description	Date



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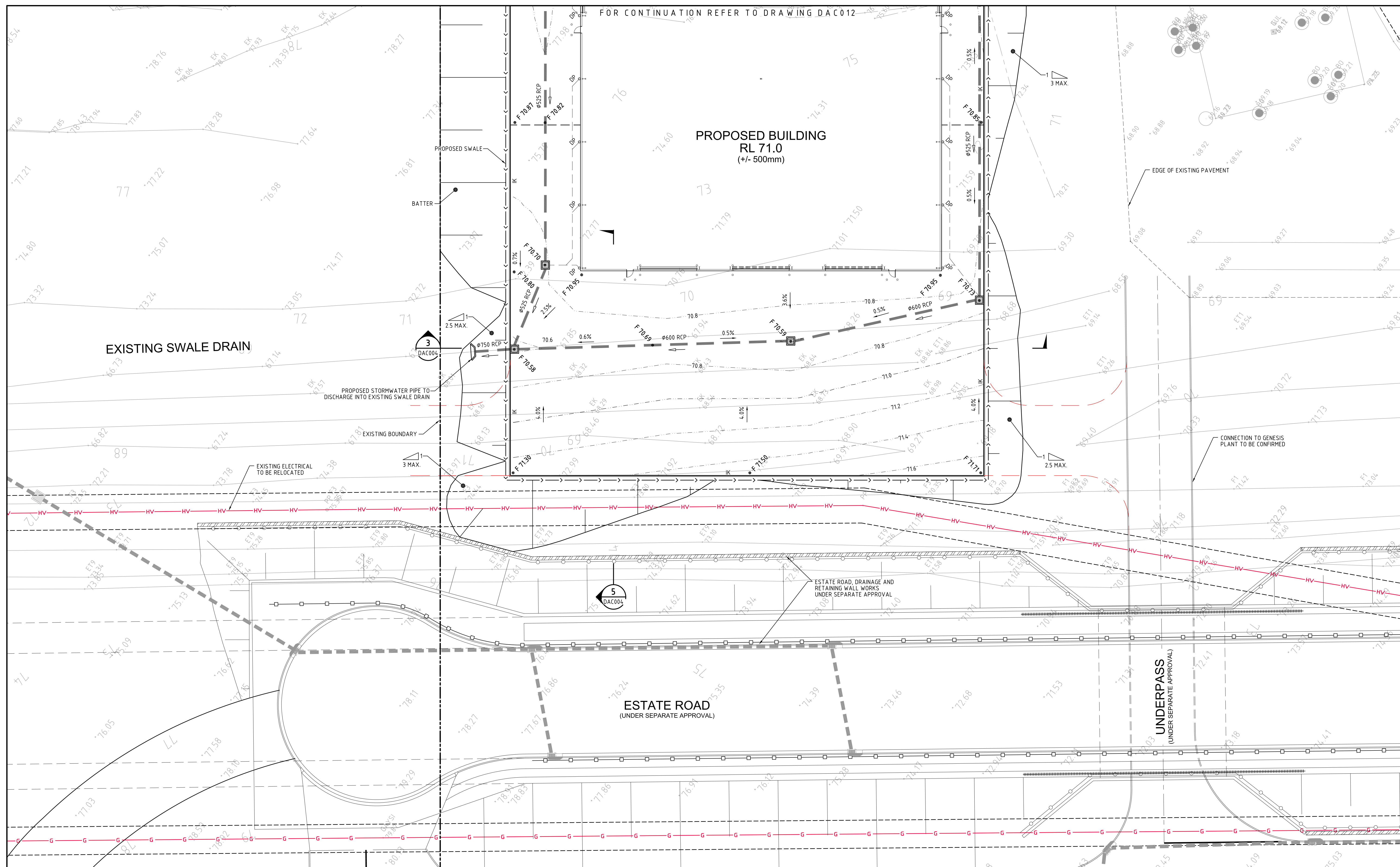
Client

DIAL A DUMP

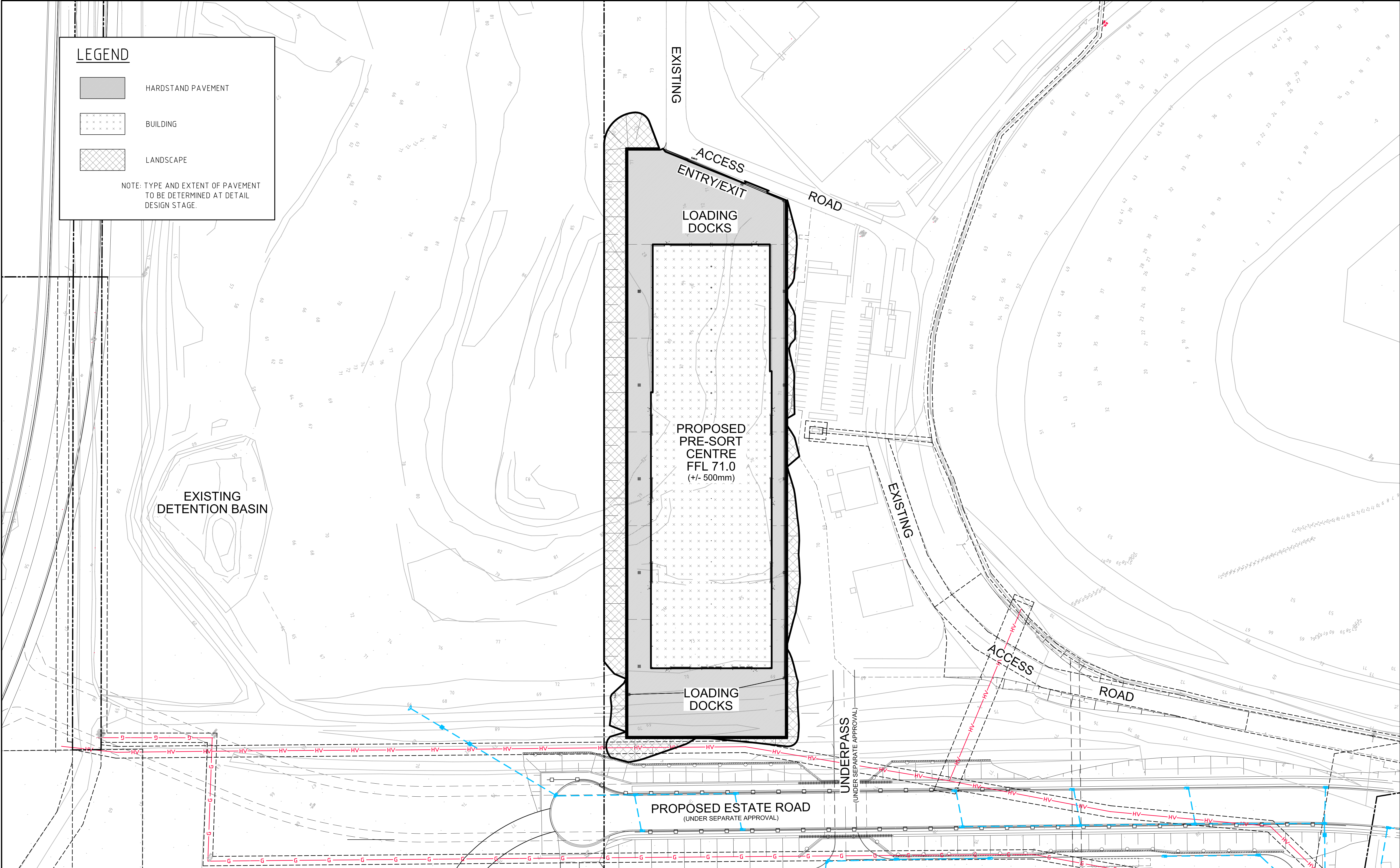
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		Designed	FX
Grid	MGA	Checked	AM
Height Datum	AHD	Approved	

Project	PRE SORT CENTRE EASTERN CREEK
Title	SITEWORKS AND STORMWATER DRAINAGE PLAN - SHEET 1

Civil Engineers and Project Managers		
at&l		
Suite 702, 154 Pacific Hwy St Leonards NSW 2065 ABN 96 130 882 405 Tel: 02 9439 1777 Fax: 02 9460 8413 www.atl.net.au info@atl.net.au		
Status	FOR APPROVAL	A1
Drawing No.	Project No.	Issue
DAC011	14-232	B



			Bar Scales	
--	--	--	--	--



LEGEND

HARDSTAND PAVEMENT

BUILDING

LANDSCAPE

NOTE: TYPE AND EXTENT OF PAVEMENT TO BE DETERMINED AT DETAIL DESIGN STAGE.

B	ISSUED FOR APPROVAL	03-03-15
A	ISSUED FOR APPROVAL	15-12-14
Issue	Description	Date

Bar Scales

020406080m

1 : 750 @ A11 : 1000 @ A3

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Client

DIAL A DUMP

Scales	1 : 750	Drawn	FX
		Designed	FX
Grid	MGA	Checked	AM
Height Datum	AHD	Approved	

Project

PRE SORT CENTRE
EASTERN CREEK

Title

PAVEMENT
PLAN

Civil Engineers and Project Managers
at&l
Suite 702, 154 Pacific Hwy
St Leonards NSW 2065
ABN 96 130 882 405
Tel: 02 9439 1777
Fax: 02 9460 8413
www.atl.net.au
info@atl.net.au

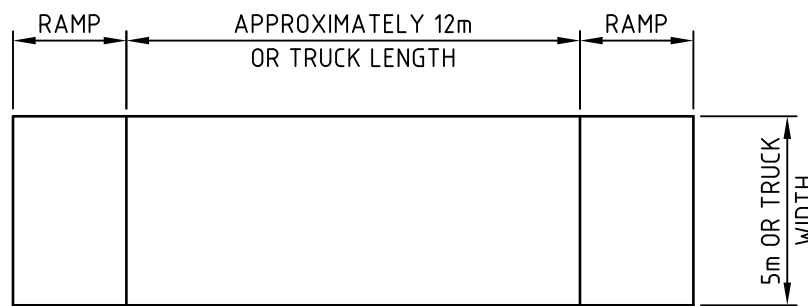
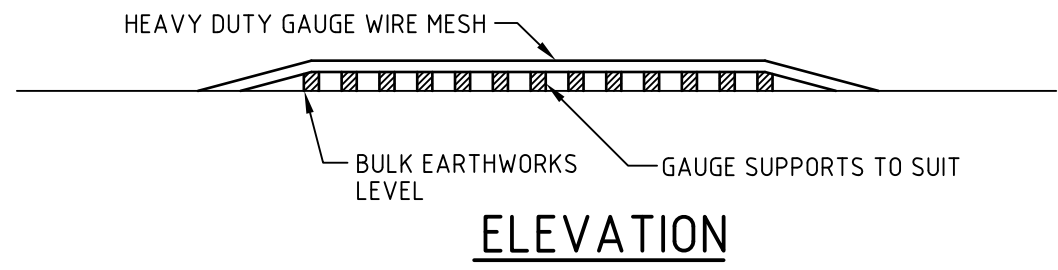
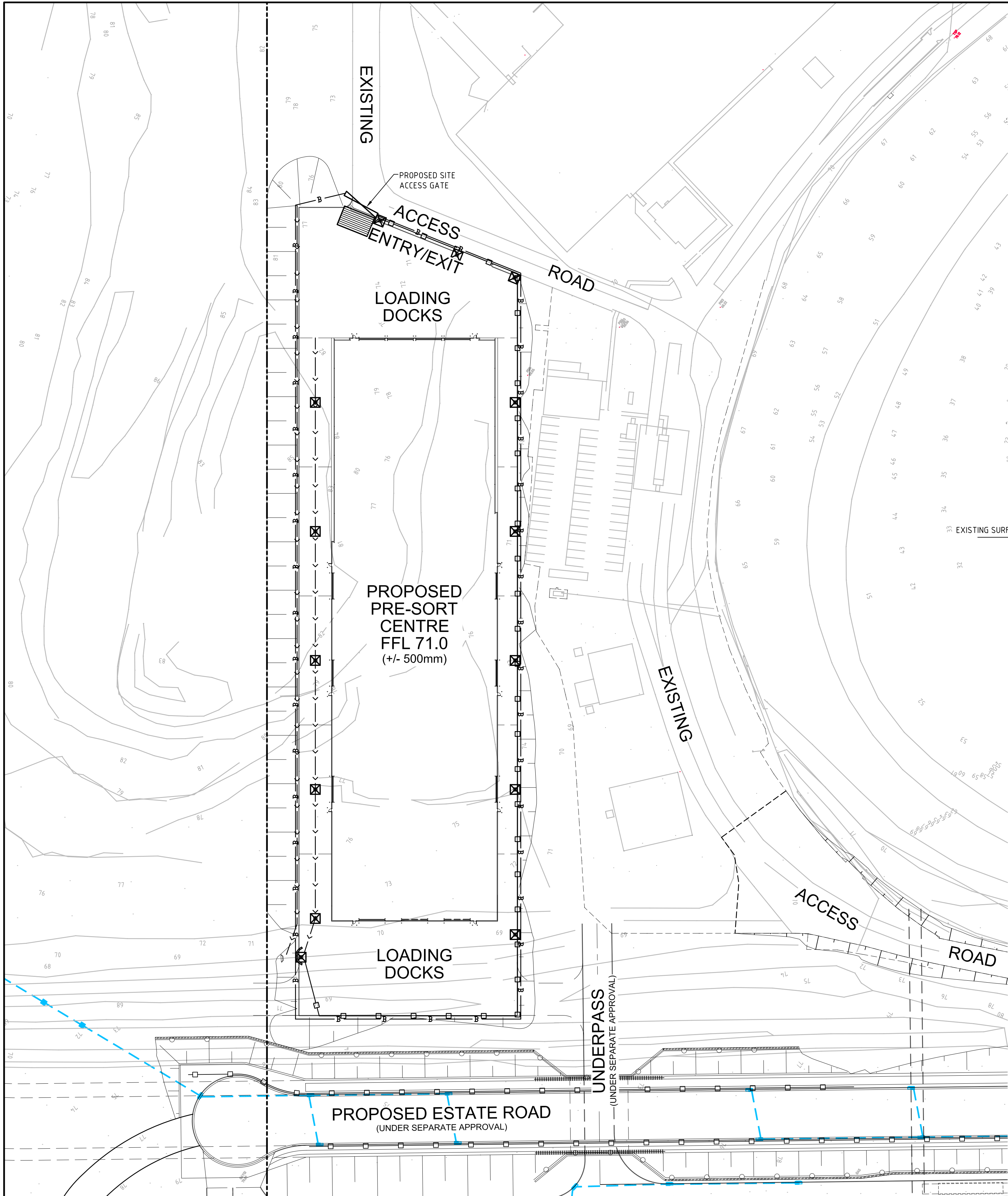
Status
FOR APPROVAL

Drawing No.
DAC020

Project No.
14-232

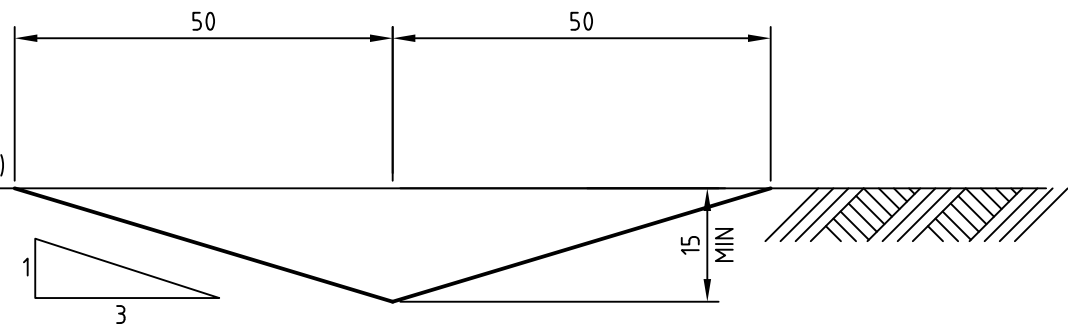
Issue
B

A1

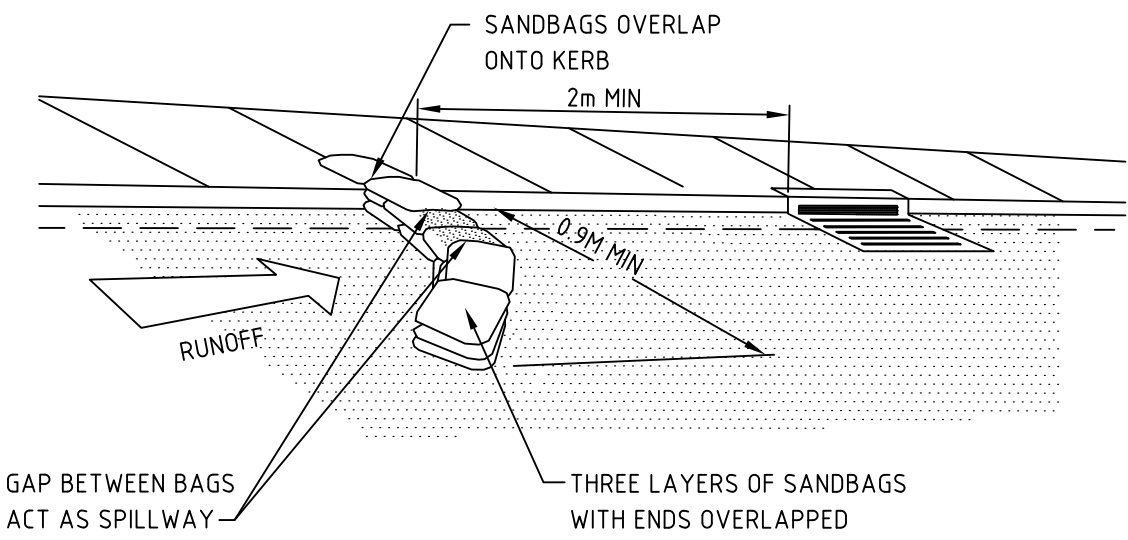


TEMPORARY CONSTRUCTION EXIT
(WASH DOWN DETAIL)

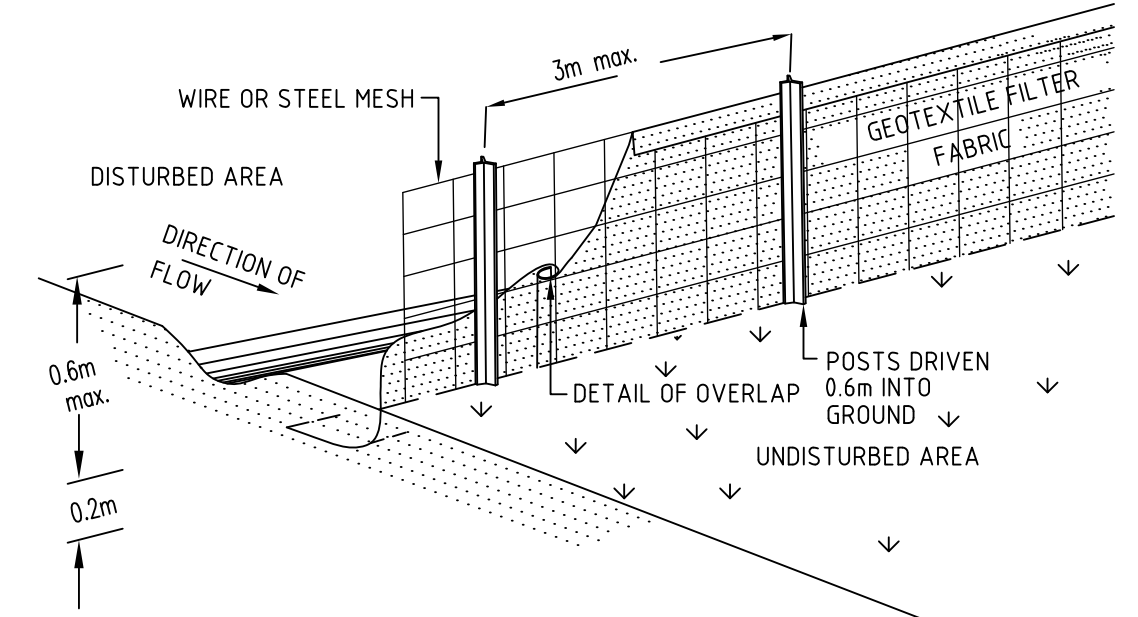
SCALE 1:200
THE EXIT SHALL BE MAINTAINED IN A CONDITION WHICH PREVENTS TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS OF WAY. THIS MAY REQUIRE REPAIR AND OR CLEANOUT OF ANY MEASURES USED TO TRAP SEDIMENT. ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC RIGHTS OF WAY MUST BE REMOVED IMMEDIATELY.



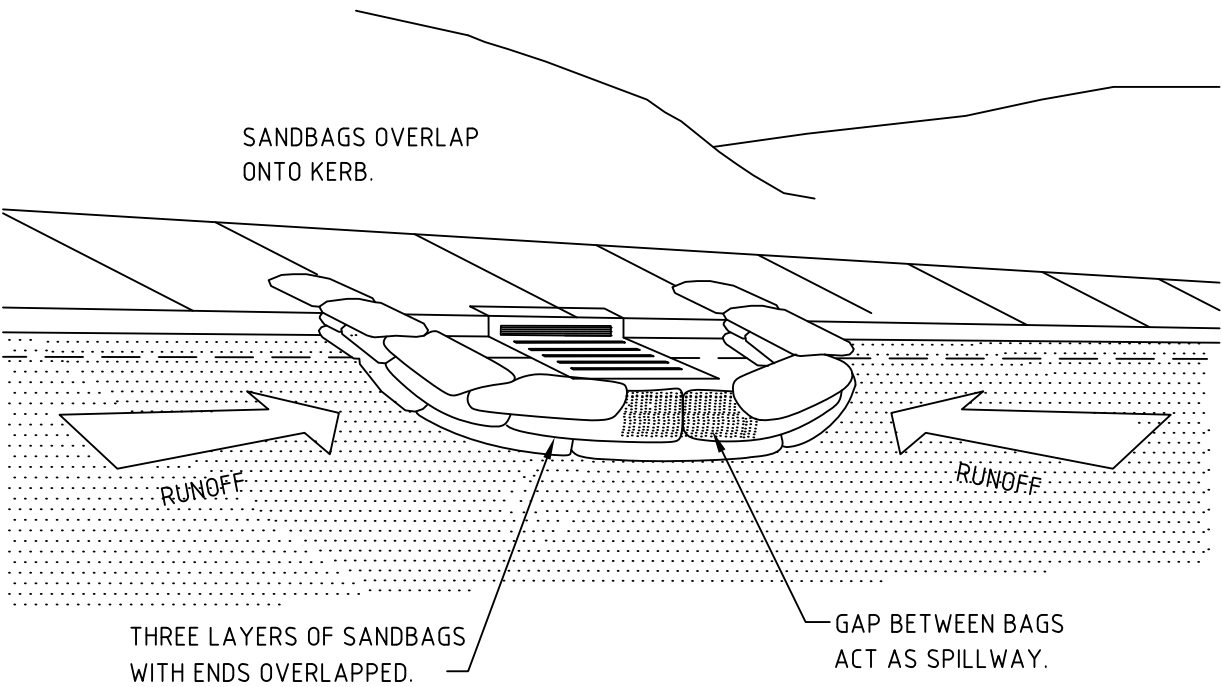
DIVERSION SWALE
(MIN FALL 1%)
SCALE 1:10



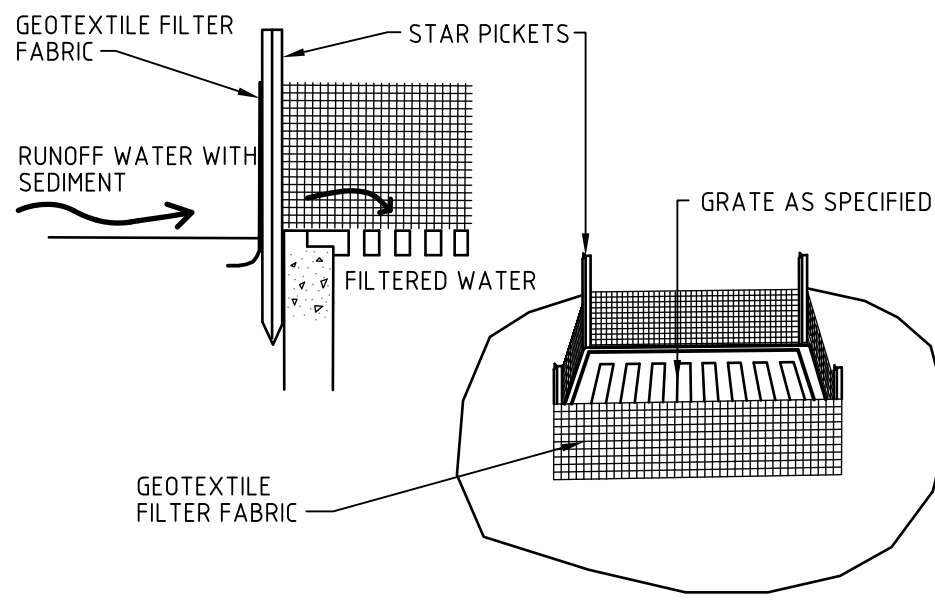
MESH AND GRAVEL INLET FILTER
FOR KERB INLET ON GRADE
NTS



SEDIMENT FENCE
NTS



SANDBAG SEDIMENT TRAP
FOR KERB INLET AT LOWPOINT
NTS



GEOTEXTILE INLET FILTER PIT SURROUND
NTS

Bar Scales		
0 20 40 60 80m		
1 : 750 @ A1 1 : 1000 @ A3		
B	ISSUED FOR APPROVAL	03-03-15
A	ISSUED FOR APPROVAL	15-12-14
Issue	Description	Date

Bar Scales		
0 20 40 60 80m		
1 : 750 @ A1 1 : 1000 @ A3		

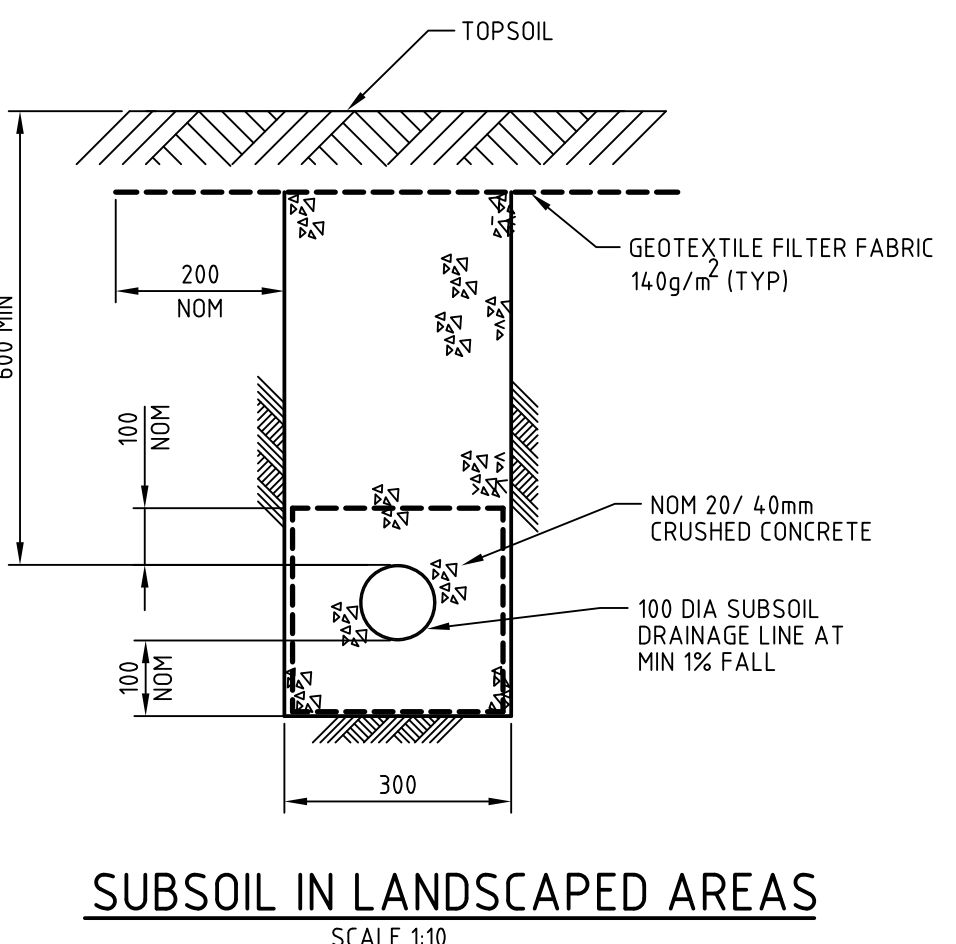
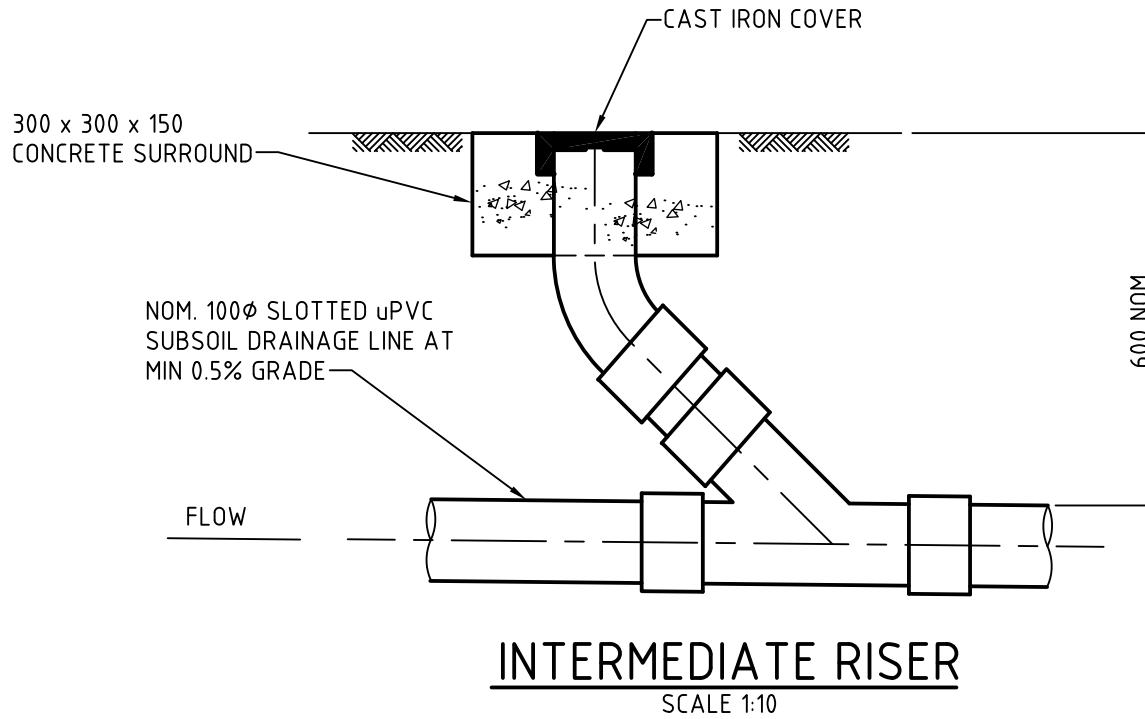
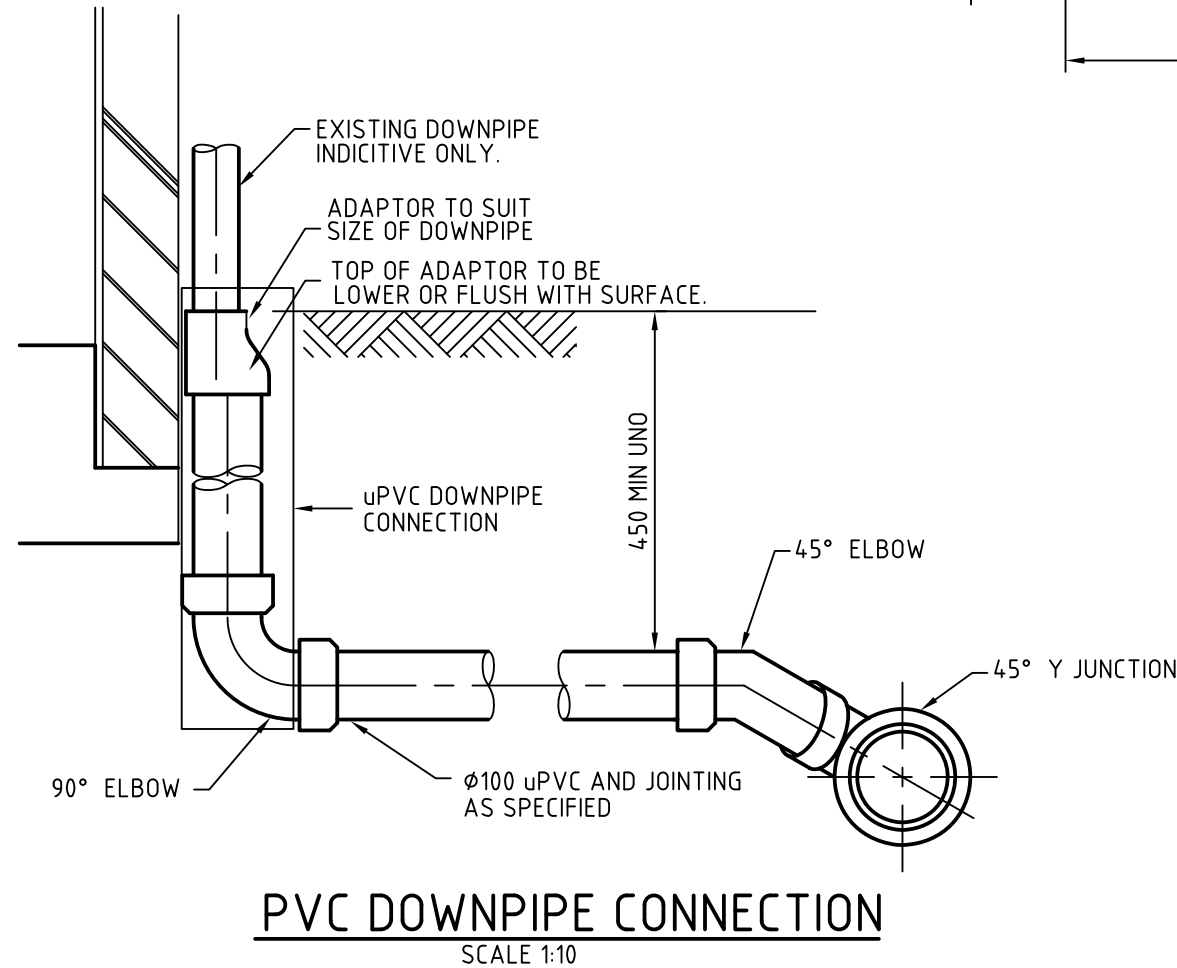
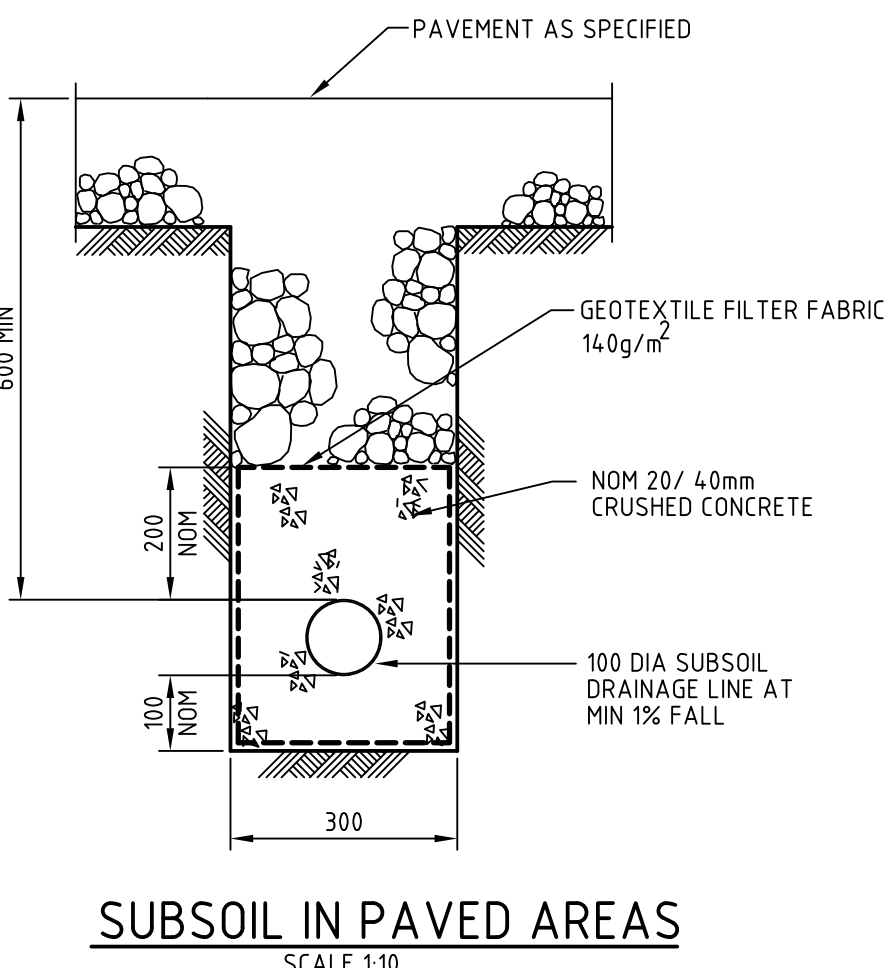
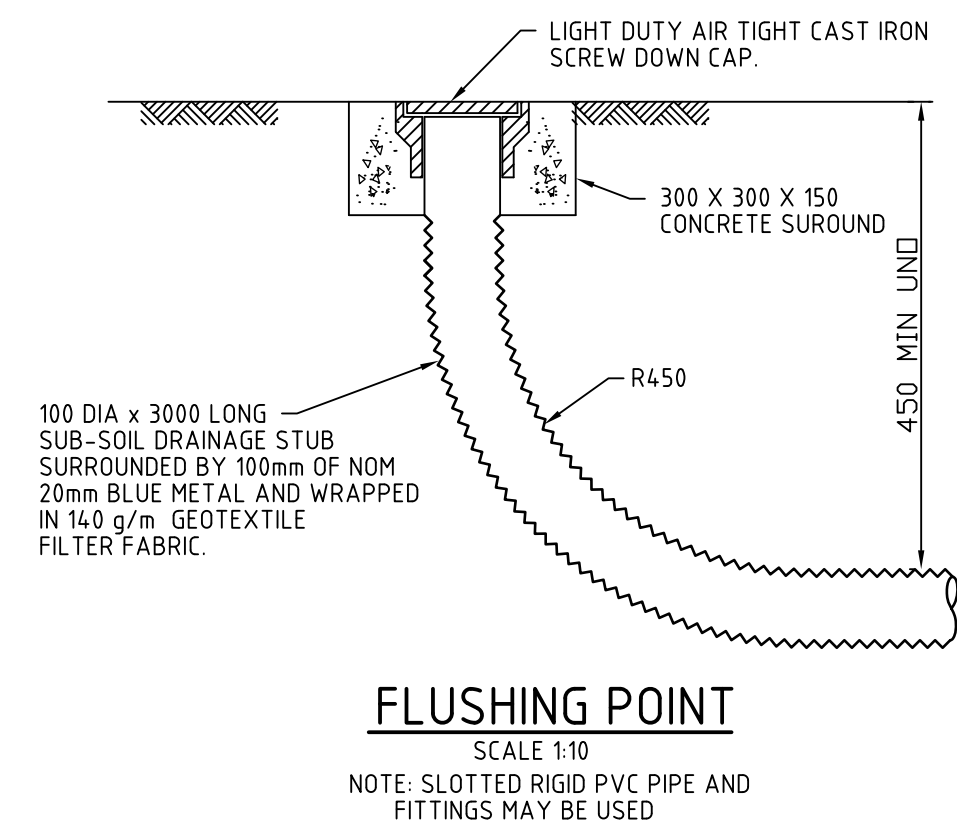
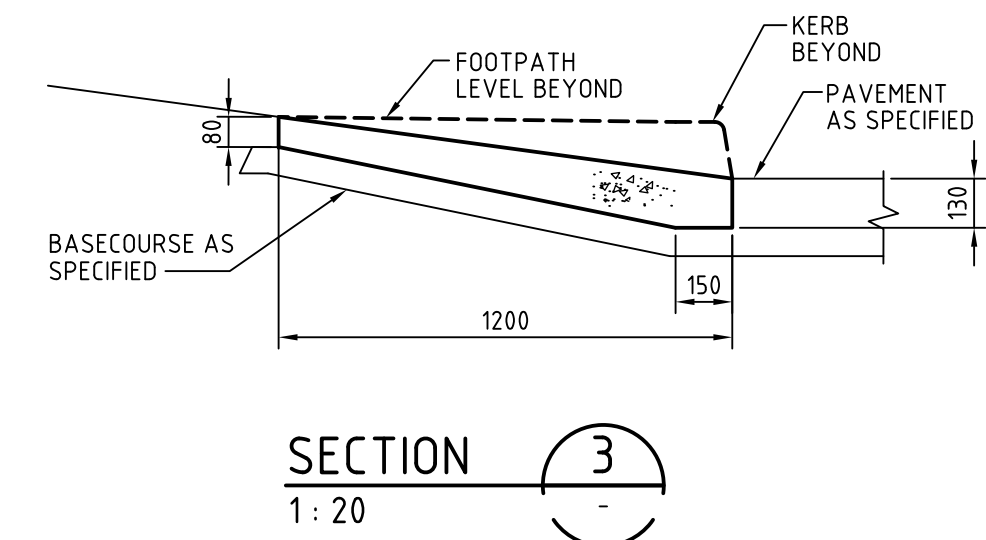
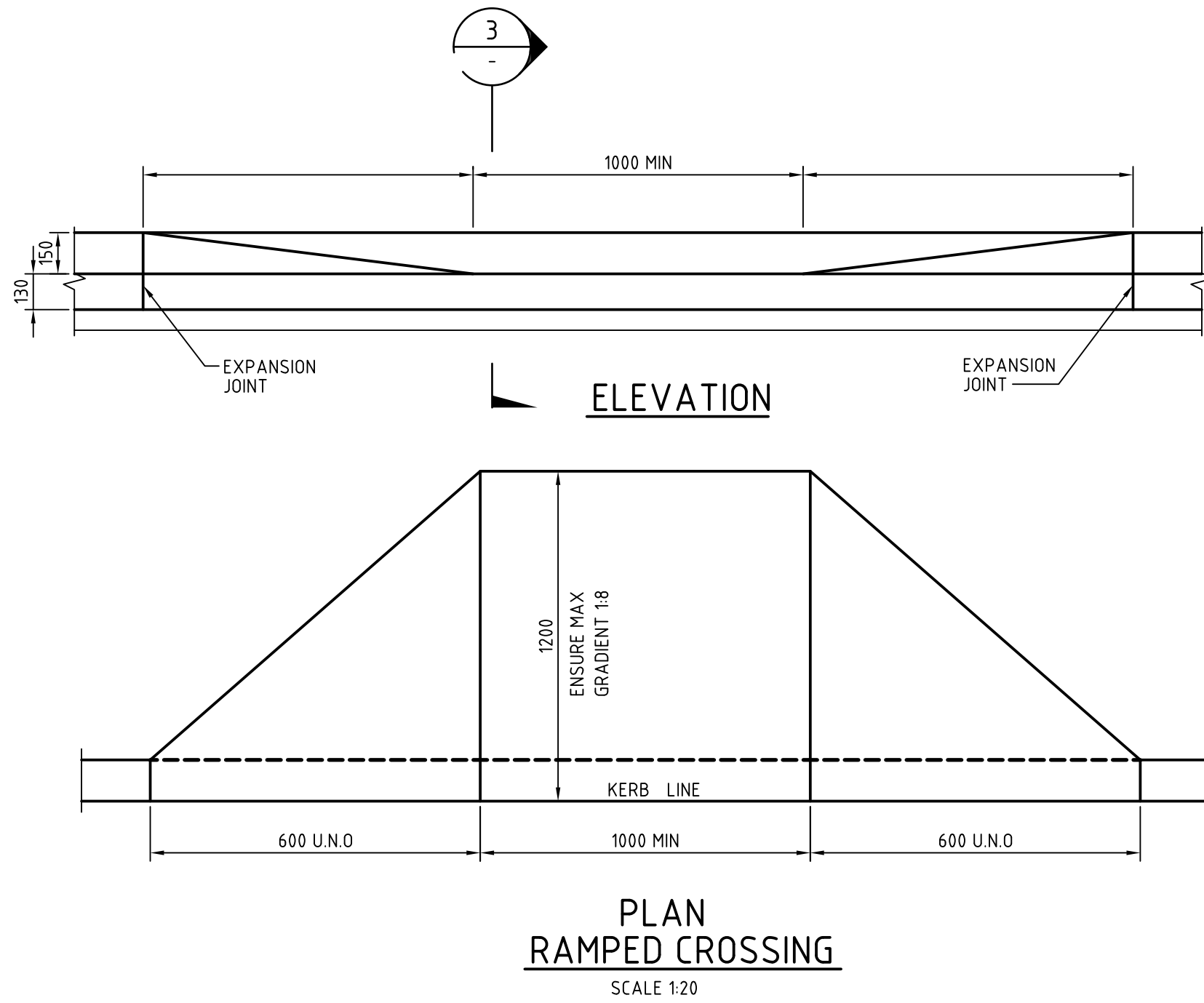
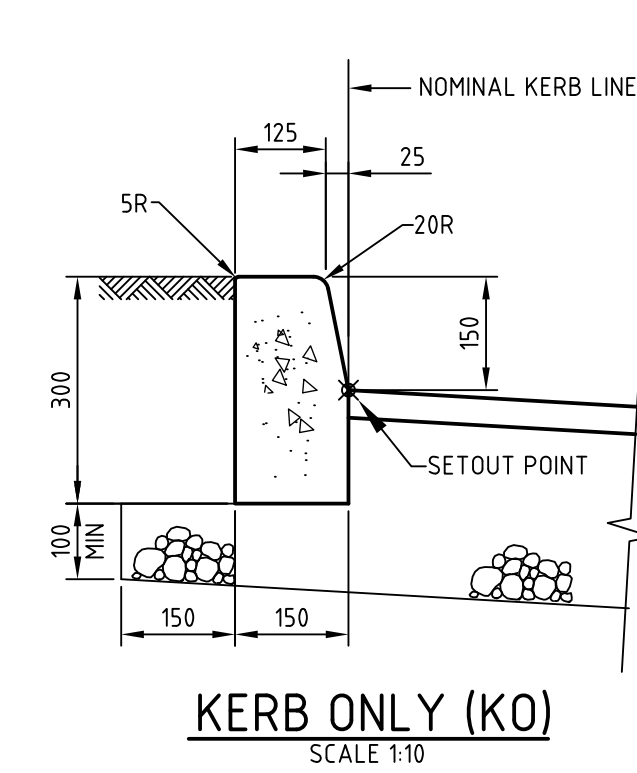
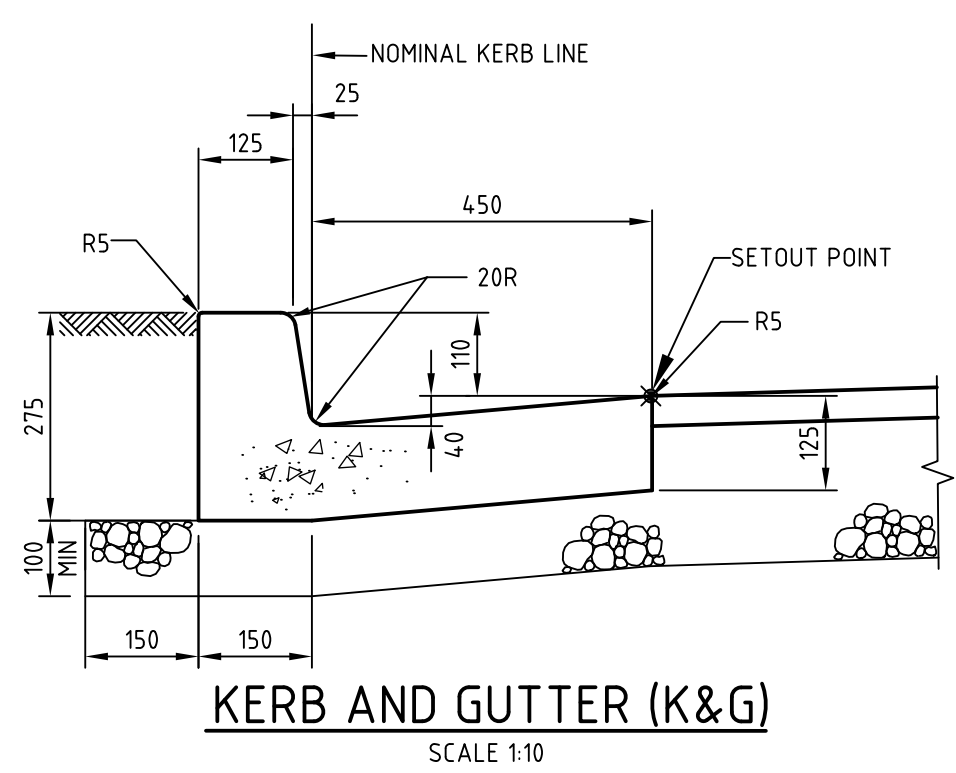
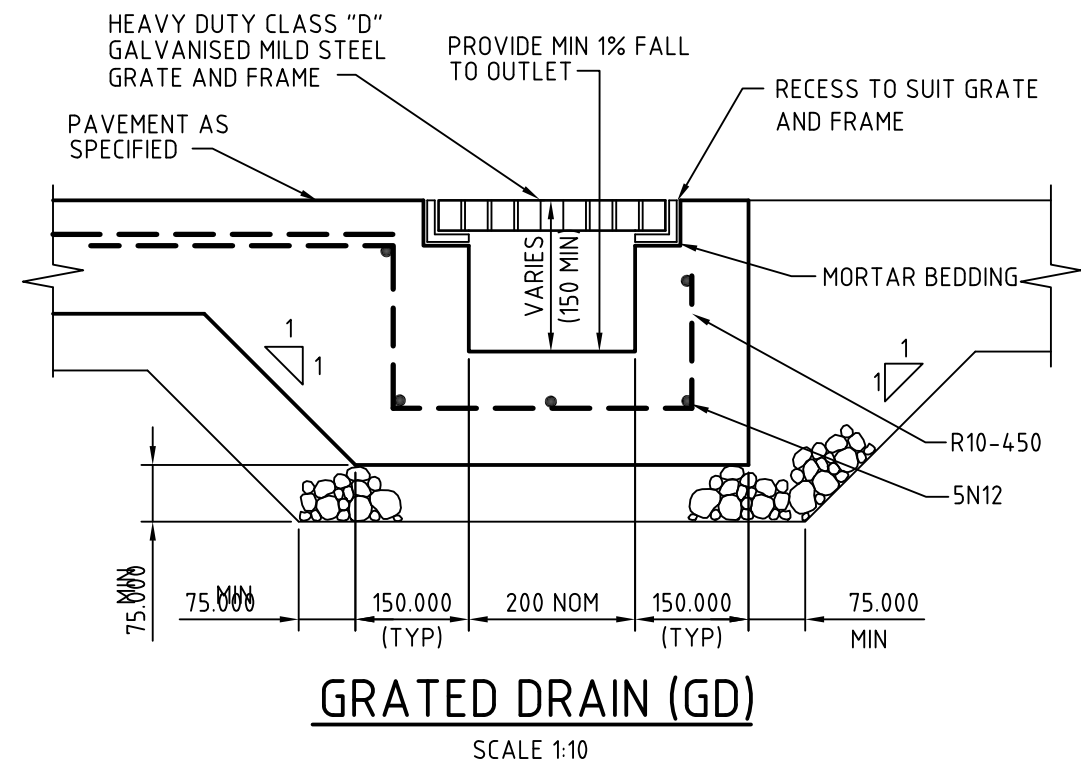
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Client
DIAL A DUMP

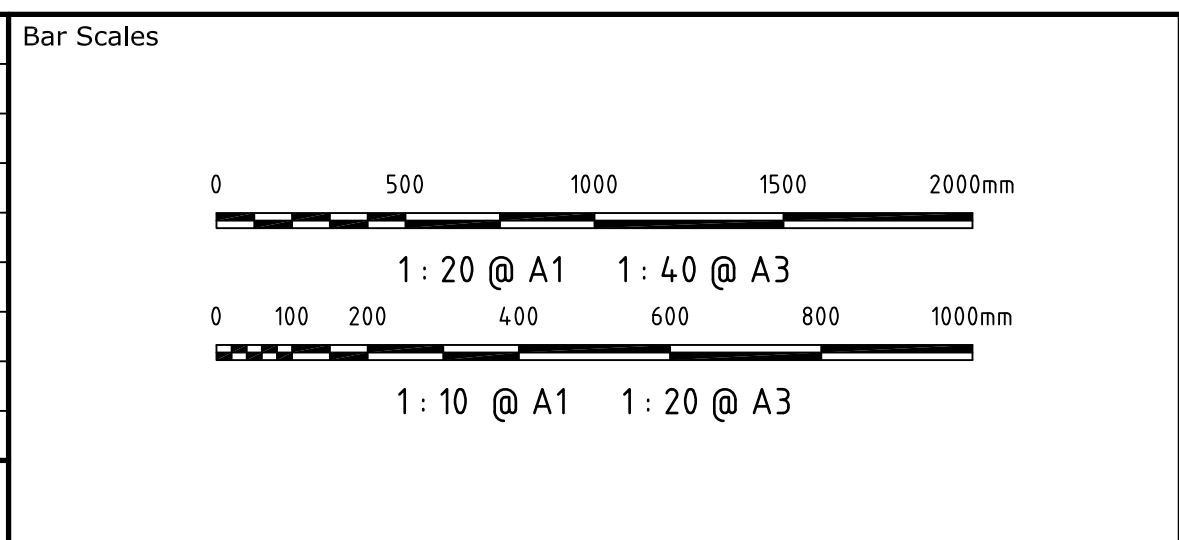
Scales 1 : 750	Drawn	FX
	Designed	FX
	Checked	AM
	Approved	
Grid MGA		
Height Datum AHD		

Project PRE SORT CENTRE EASTERN CREEK	
Title EROSION AND SEDIMENT CONTROL PLAN	

Civil Engineers and Project Managers at&l Suite 702, 154 Pacific Hwy St Leonards NSW 2065 ABN 96 130 882 405 Tel: 02 9439 1777 Fax: 02 9460 8413 www.atl.net.au info@atl.net.au		
Status FOR APPROVAL	A1	
Drawing No. DAC021	Project No. 14-232	Issue B



B	ISSUED FOR APPROVAL	03-03-15
A	ISSUED FOR APPROVAL	15-12-14
Issue	Description	Date



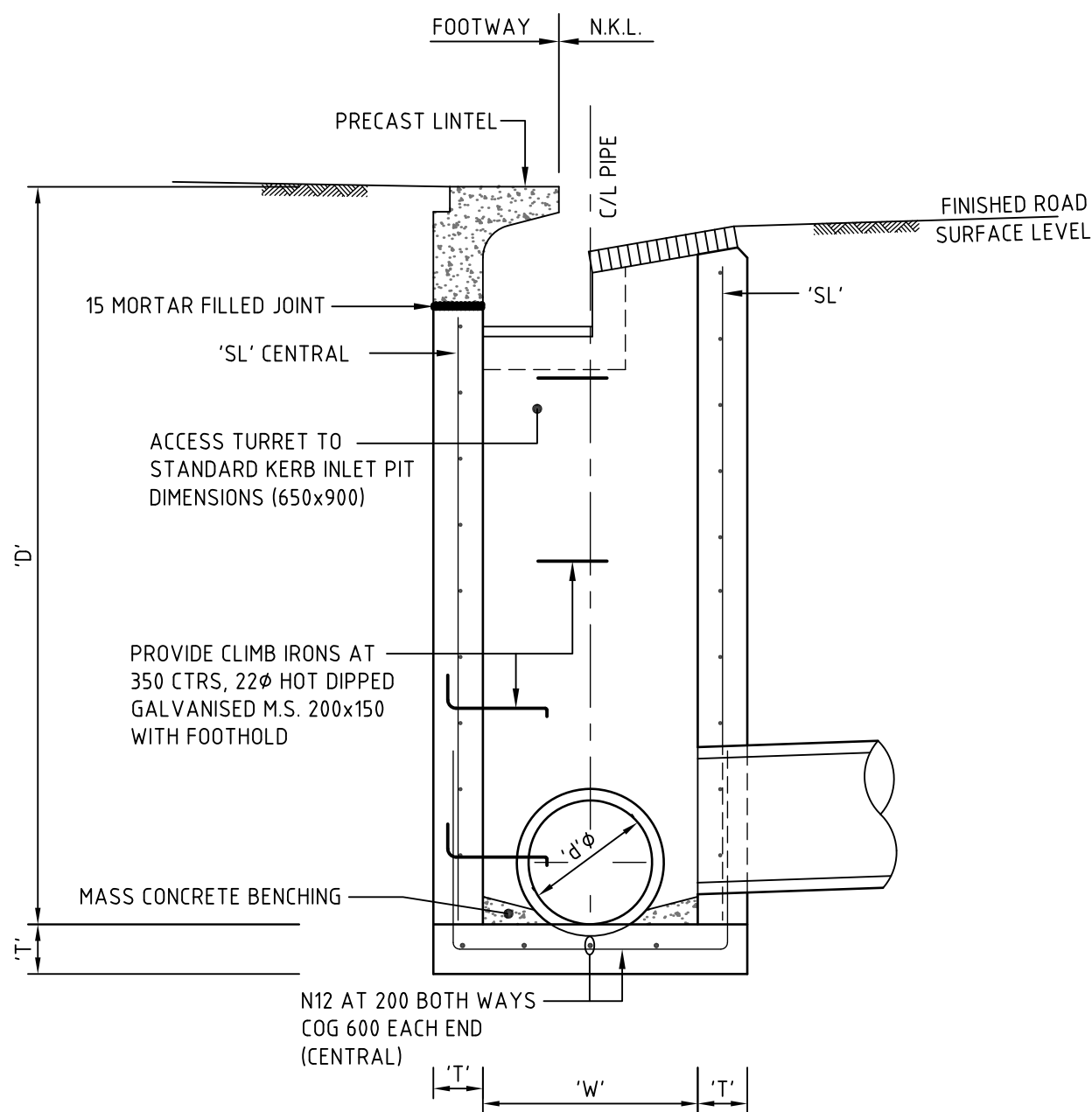
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Client
DIAL A DUMP

Scales	1:10 1:20	Drawn	FX
		Designed	FX
Grid	MGA	Checked	AM
Height Datum	AHD	Approved	

Project	PRE SORT CENTRE EASTERN CREEK
Title	SITEWORKS DETAILS

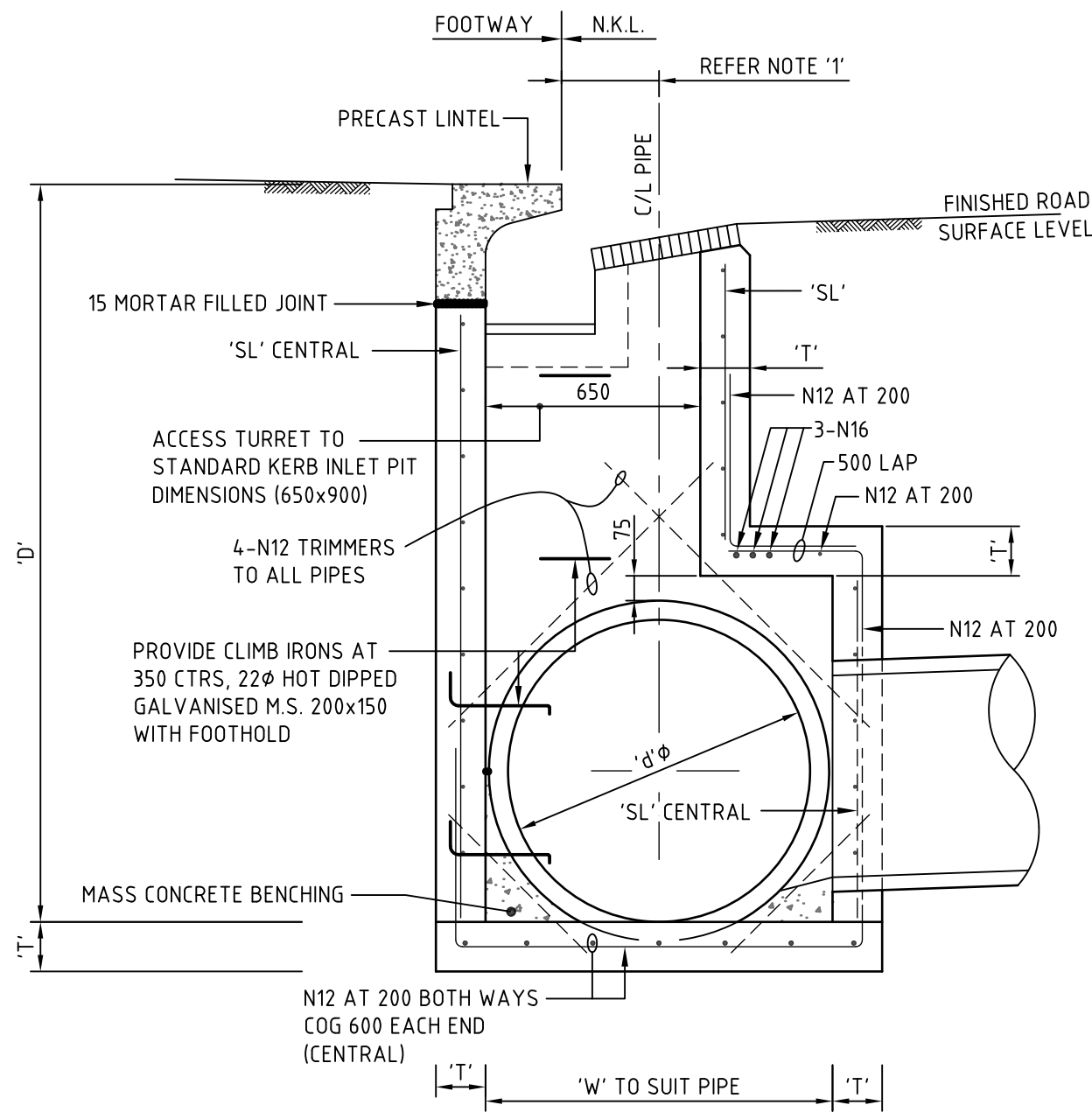
Civil Engineers and Project Managers at&l Suite 702, 154 Pacific Hwy St Leonards NSW 2065 ABN 96 130 882 405 Tel: 02 9439 1777 Fax: 02 9460 8413 www.atl.net.au info@atl.net.au		
Status	FOR APPROVAL	A1
Drawing No.	DAC030	Issue
Project No.	14-232	B



PIT TYPE 'A' DETAIL
STANDARD GRATED KERB INLET PIT
FOR PIPES UP TO 525Ø

SCALE: 1:20

PIPE DIA. 'ØØ'	WIDTH 'W'	DEPTH 'D'	WALL 'T'	FABRIC/COV 'SL'
375	650	800	150	SL72
	650	1200	150	SL72
	650	1600	150	SL72
	650	2000	150	SL72
	650	2400	150	SL72
450	650	800	150	SL72
	650	1200	150	SL72
	650	1600	150	SL72
	650	2000	150	SL72
	650	2400	150	SL72
525	650	800	150	SL72
	650	1200	150	SL72
	650	1600	150	SL72
	650	2000	150	SL72
	650	2400	150	SL72

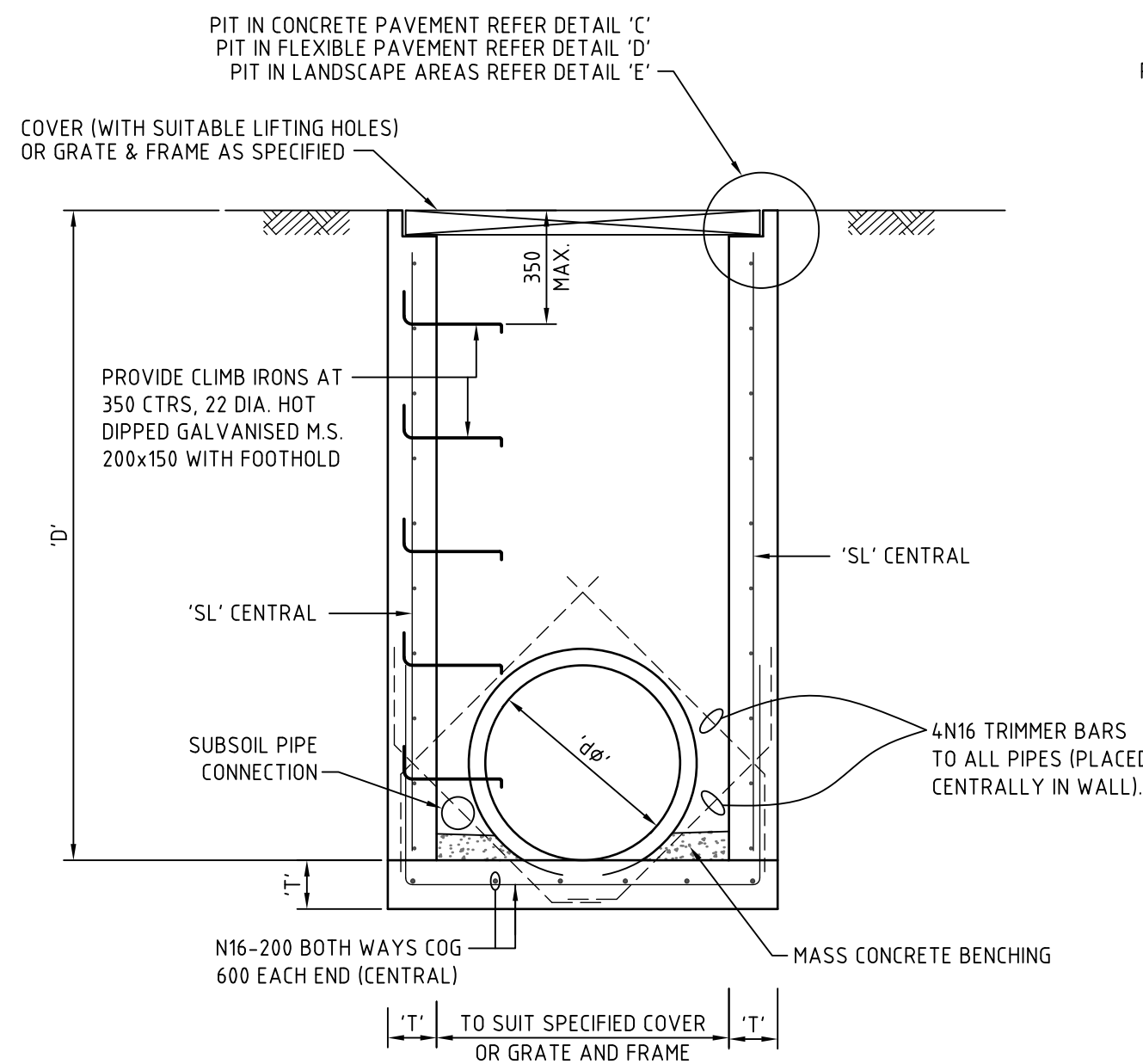


PIT TYPE 'B' DETAIL
STANDARD GRATED KERB INLET PIT
FOR PIPES FROM 600Ø TO 1350Ø

(ENLARGED CHAMBER)
SCALE: 1:20

PIPE DIA. 'ØØ'	WIDTH 'W'	DEPTH 'D'	WALL 'T'	FABRIC/COV 'SL'
600	850	1200	150	SL72
	850	1600	150	SL72
	850	2000	150	SL72
	850	2400	150	SL72
	850	2800	150	SL72
675	940	1200	150	SL72
	940	1600	150	SL72
	940	2000	150	SL72
	940	2400	150	SL72
	940	2800	150	SL72
750	1020	1200	150	SL72
	1020	1600	150	SL72
	1020	2000	150	SL72
	1020	2400	150	SL72
	1020	2800	150	SL72
825	1100	1600	150	SL72
	1100	2000	150	SL72
	1100	2400	150	SL72
	1100	2800	150	SL72
	1100	3200	150	SL72
900	1180	1600	150	SL72
	1180	2000	150	SL72
	1180	2400	150	SL72
	1180	2800	150	SL72
	1180	3200	150	SL72
1050	1345	1600	150	SL92
	1345	2000	150	SL92
	1345	2400	150	SL92
	1345	2800	150	SL92
	1345	3200	150	SL92
1200	1510	2000	150	SL102
	1510	2400	150	SL102
	1510	2800	150	SL102
	1510	3200	150	SL102
	1510	3600	150	SL102
1350	1690	2000	200	N12@150
	1690	2400	200	N12@150
	1690	2800	200	N12@150
	1690	3200	200	N12@150
	1690	3600	200	N12@150

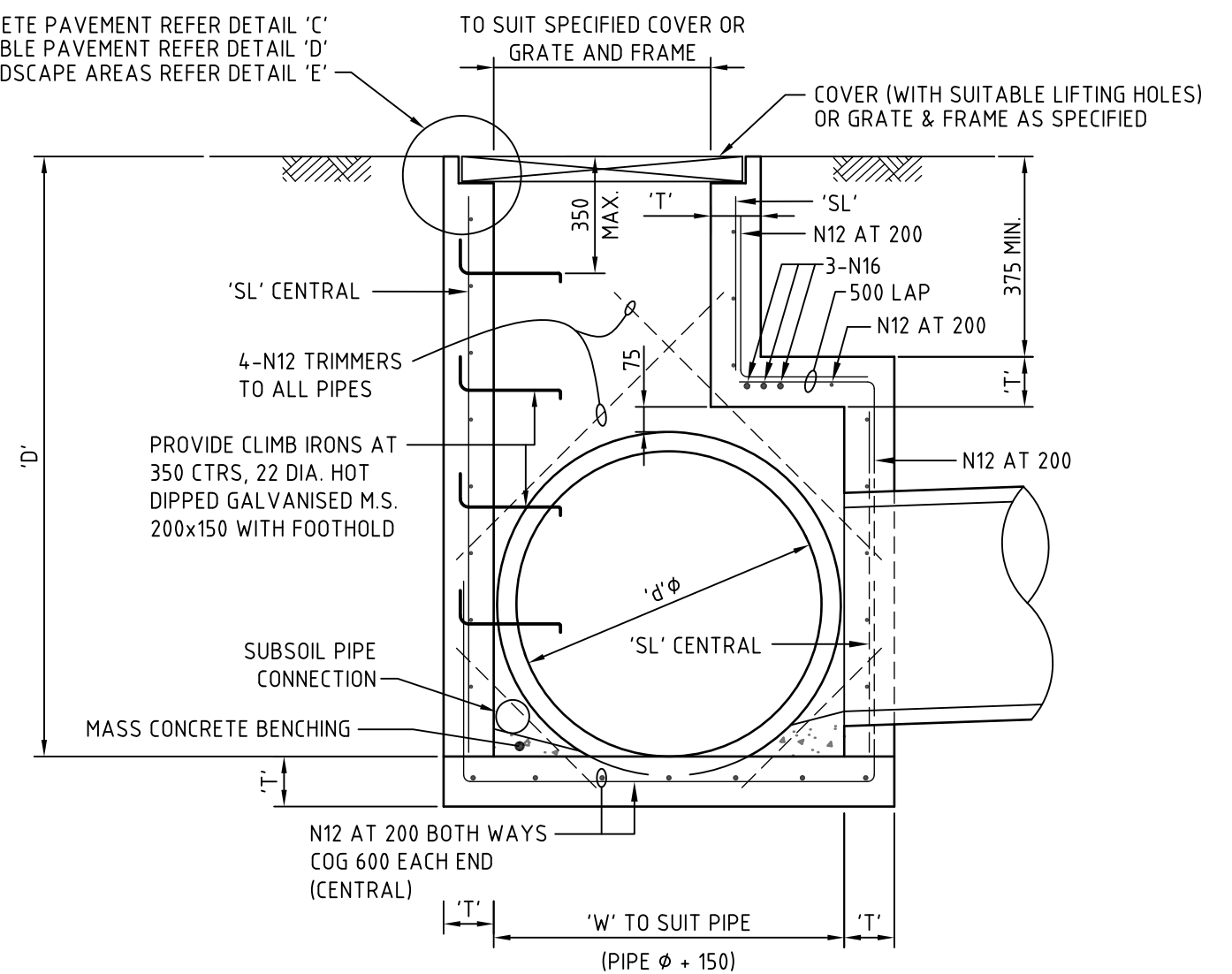
NOTE 1
150 FOR Ø600 DIA.
180 FOR Ø675 DIA.
220 FOR Ø750 DIA.
255 FOR Ø825 DIA.
370 FOR Ø1050 DIA.
445 FOR Ø1200 DIA.



PIT TYPE 'C' DETAIL
SURFACE INLET PIT
FOR PIPES UP TO 525Ø

SCALE: 1:20

PIPE DIA. 'ØØ'	WIDTH 'W'	DEPTH 'D'	WALL 'T'	FABRIC/COV 'SL'
375	650	800	150	SL72
	650	1200	150	SL72
	650	1600	150	SL72
	650	2000	150	SL72
	650	2400	150	SL72
450	650	800	150	SL72
	650	1200	150	SL72
	650	1600	150	SL72
	650	2000	150	SL72
	650	2400	150	SL72
525	650	800	150	SL72
	650	1200	150	SL72
	650	1600	150	SL72
	650	2000	150	SL72
	650	2400	150	SL72



PIT TYPE 'D' DETAIL
SURFACE INLET PIT
FOR PIPES FROM 600Ø TO 1350Ø

(ENLARGED CHAMBER)
SCALE: 1:20

PIPE DIA. 'ØØ'	WIDTH 'W'	DEPTH 'D'	WALL 'T'	FABRIC/COV 'SL'
600	850	1200	150	SL72
	850	1600	150	SL72
	850	2000	150	SL72
	850	2400	150	SL72
	850	2800	150	SL72
675	940	1200	150	SL72
	940	1600	150	SL72
	940	2000	150	SL72
	940	2400	150	SL72
	940	2800	150	SL72
750	1020	1200	150	SL72
	1020	1600	150	SL72
	1020	2000	150	SL72
	1020	2400	150	SL72
	1020	2800	150	SL72
825	1100	1600	150	SL72
	1100	2000	150	SL72
	1100	2400	150	SL72
	1100	2800	150	SL72
	1100	3200	150	SL72
900	1180	1600	150	SL72
	1180	2000	150	SL72
	1180	2400	150	SL72
	1180	2800	150	SL72
	1180	3200	150	SL72
1050	1345	1600	150	SL92
	1345	2000	150	SL92
	1345	2400	150	SL92
	1345	2800	150	SL92
	1345	3200	150	SL92
1200	1510	2000	150	SL102
	1510	2400	150	SL102
	1510	2800	150	SL102
	1510	3200	150	SL102
	1510	3600	150	SL102
1350	1690	2000	200	N12@150
	1690	2400	200	N12@150
	1690	2800	200	N12@150
	1690	3200	200	N12@150
	1690	3600	200	N12@150

NOTE

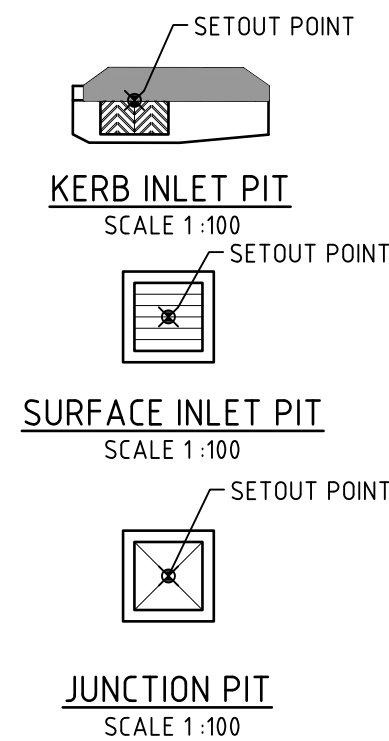
- FOR PIT SIZE REFER TO TABLE (900 MIN LONG).
- REINFORCING MESH IS TO BE BENT TO LAP 300 AROUND ALL CORNERS. VERTICAL BARS ARE NOT TO BE CUT. ALTERNATLY PROVIDE N12 "L" BARS (500x500) AT 400 VERTICAL CTS.
- COMPRESSIVE STRENGTH (F'c) FOR CAST IN SITU CONCRETE SHALL BE A MINIMUM 32 MPa AT 28 DAYS.
- TOP OF BENCHING SHALL BE 1/2 OF OUTLET PIPE DIAMETER.
- 100mm SUBSOIL DRAINAGE PIPE 3000 LONG WRAPPED IN FABRIC SOCK TO BE PROVIDE ADJACENT TO INLET PIPES.
- ALL PITS SHALL BE PROVIDED WITH A LOCKING CLIP.
- PIT GRATE TO BE 'WELDLOK' GULLY GRATE GG 78-50 OR APPROVED EQUIVALENT.
- DURING INSTALLATION OF GRATE AND FRAME CONTRACTOR IS TO ENSURE CLEARANCE BETWEEN LINTEL AND OPENED GRATE (REFER TO INSTALLATION TOLERANCE).
- PROVIDE STEP IRONS AS INDICATED FOR PITS DEEPER THAN 1200.
- N12 AT 200 CENTRAL MAY BE USED IN LEIU OF MESH. LAP 500 AT CORNERS

11. CONCRETE STRENGTH - UNLESS NOTED OTHERWISE

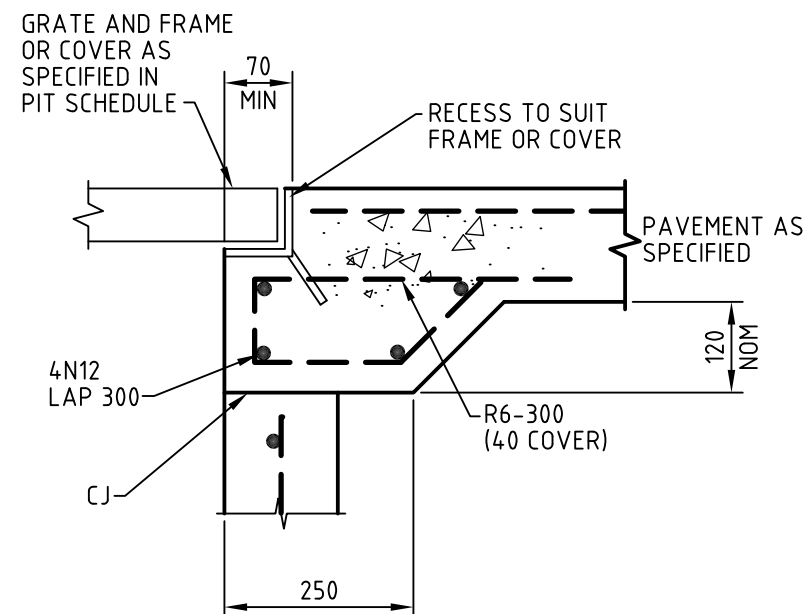
ELEMENT	f'c MPa (28 DAYS)	SLUMP	MAX AGG SIZE	CEMENT TYPE
PITS	32	80mm	20mm	GP

12. COVER - UNLESS NOTED OTHERWISE

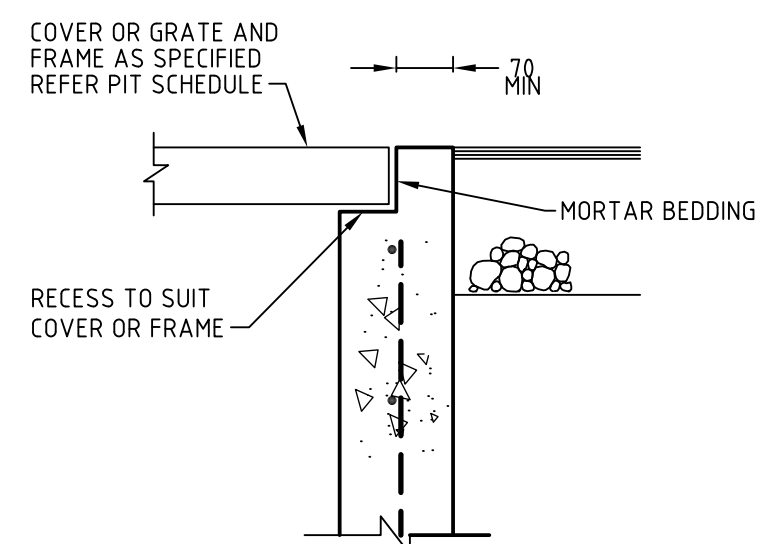
ELEMENT	INTERIOR	EXTERIOR
PITS		45mm
SLAB TOP	45mm	45mm
SLAB BOTTOM	45mm	45mm
BEAM TOP		
BEAM BOTTOM		
BEAM SIDE		
COLUMNS		



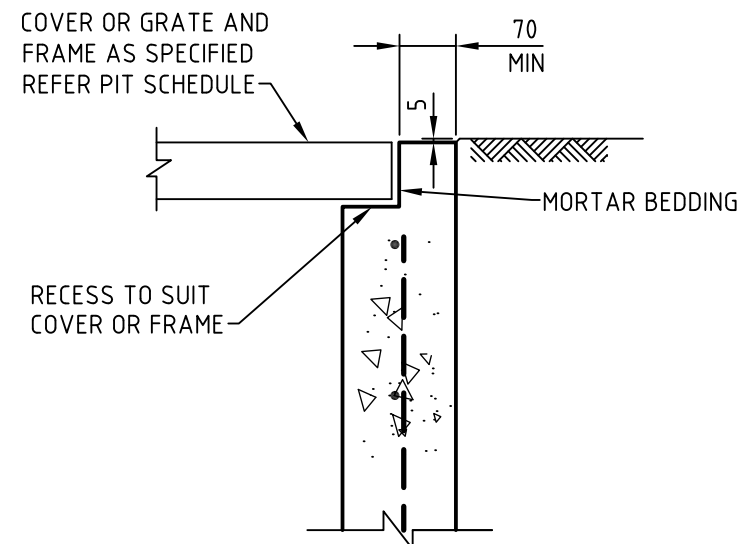
STORMWATER PIT
SETOUT POINTS



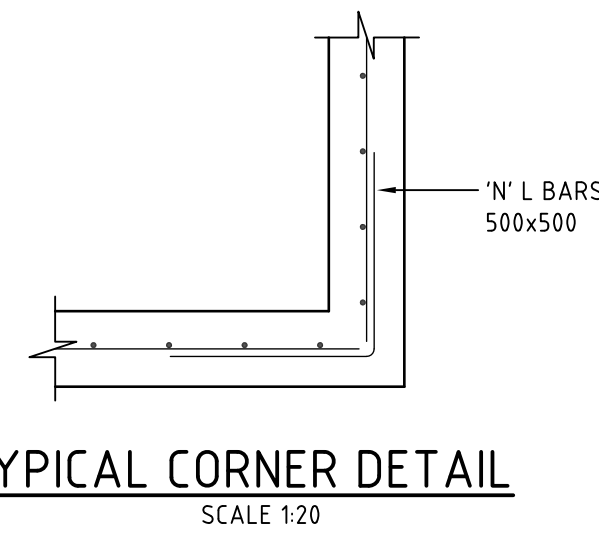
DETAIL "C"
SCALE 1:10



DETAIL "D"
SCALE 1:10



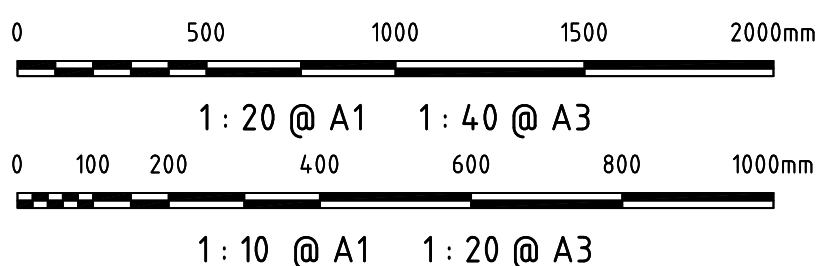
DETAIL "E"
SCALE 1:10



TYPICAL CORNER DETAIL
SCALE 1:20

Issue	Description	Date
B	ISSUED FOR APPROVAL	03-03-15
A	ISSUED FOR APPROVAL	15-12-14

Bar Scales



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Client

DIAL A DUMP

Scales

1 :10
1 :20

Drawn

FX

Designed

FX

Grid

MGA

Checked

AM

Height

AHD

Approved

Project

PRE SORT CENTRE
EASTERN CREEK

Title

DRAINAGE
DETAILS

Civil Engineers and Project Managers

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Status

FOR APPROVAL

A1

Drawing No.
DAC031

Project No.
14-232

Issue
B