

## **HYDROGEOLOGICAL ASSESSMENT**

### **Property Address**

2 Fishburn Crescent, Castle Hill NSW 2154

### **Prepared for**

ARADA Development Management Pty Ltd

### **Date**



23 April 2026

## DOCUMENT CONTROL REGISTER

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- Appendix C:** Groundwater Level and Daily Rainfall
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- Appendix E:** Architectural Drawings
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## 1.0 INTRODUCTION

### 1.1 General

ARADA Development Management Pty Ltd (the client) appointed Foundation Earth Sciences Pty Ltd (FES) to conduct a Hydrogeological Assessment (HGA) for the proposed development located at 2 Fishburn Crescent, Castle Hill NSW 2154 (the site). The HGA report aims to estimate potential groundwater inflow to the proposed bulk excavation and ensure compliance with requirements set by the local city council and Water New South Wales (WaterNSW) regarding the basement design and future dewatering activities. This report has been prepared in accordance with the WaterNSW guideline titled *Minimum requirements for building site groundwater investigations and reporting* (PUB20/940, revision V02.2210, October 2022), issued by the Department of Planning and Environment (DPE), addressing groundwater-related risks relevant to the project site.

### 1.2 Scope of work

The following scope of work was carried out for the HGA:

- Collected the ground conditions including groundwater details from the geotechnical investigation report.
- Assessed and adopted the existing groundwater wells, identified as GW1 (BH2), GW2 (BH12) and GW3 (BH11), which were installed by FES during the geotechnical investigation.
- Conducted raising head permeability test in the groundwater wells.
- Assessed the standing groundwater level in the groundwater wells.
- Conducted groundwater modelling using the Finite Element Analysis (FEA) method to estimate the potential groundwater inflow into the proposed bulk excavation.
- Prepared this HGA report for the proposed development.

A site plan showing approximate locations of the groundwater well and sections of the groundwater modelling are annexed as Appendix A. The borehole log and their explanatory note from the borehole drilling is annexed as Appendix B.

### 1.3 Provided Information

The following information was made available to FES during the preparation of this report:

- Geotechnical Investigation report titled “Project 16-20 Carrington Road, Castle Hill NSW 2154”, prepared by FES, reference to G702-1R, revision 1, and dated 23 April 2026.
- Architectural drawings, project titled “Carrington Road, 2 Fishburn Crescent, Castle Hill NSW 2154”, prepared by Turner, reference to project number 23104, and dated 10 and 17 April 2025.
- Survey Plan, titled “6-20 Carrington Rd / 2-12 Middleton Ave / 4-6 Fishburn Cres / 25-31 Sexton Ave, Castle Hill”, prepared by East Coast Positioning, reference to drawing number ECP2725.D.01, and dated 16 March 2026.

## 2.0 BACKGROUND

### 2.1 Site Description

The site is located within Sydney metropolitan area, approximately 25km northwest of Sydney CBD. Details of the site are summarised in Table 1 below.

**Table 1: The Site Details**

Site	Details	
Location	2 Fishburn Crescent, Castle Hill NSW 2154	
Lot and DP	Lot 1 in DP 1316896	
Local Government Area	The Hills Shire Council	
Shape and Area	Irregular shape of land with approximate size of 14,223.6m <sup>2</sup>	
Slope	Sloping from the southeast to the northwest	
Existing Structures	Fifteen 1- to 2-storey residential dwellings with associated structures	
Closest Watercourse	Cattai River is located approximately 320m west to the site	
Special Features	No special feature was observed	
Type of Neighbouring Properties	North	Carrington Road's carriage way and road reserve
	East	Residential properties and Sexton Avenue's carriage way and road reserve
	South	Fishburn Crescent's carriage way and road reserve
	West	Middleton Avenue's carriage way and road reserve

### 2.2 Proposed Development

The architectural drawings listed above indicate that the proposed development is a multi-storey high-raised buildings with one basement level, lower ground level and ground level. The RL of basement level is at 95.9m AHD. Due to the sloping nature of the natural ground level, it is inferred that approximately 3m to 10m of excavation is required for the proposed development.

### 2.3 Local Geology

Local geology was assessed using a geological map, **Penrith** 1:100,000 Geological Series Sheet 9030 (Edition 1) 1991, from the Geological Survey of New South Wales. The map indicates that site is part of the **Rwa** geology, Ashfield Shale, Wianamatta Group, Middle Triassic age and is described as *Dark-grey to black claystone-siltstone and fine sandstone-siltstone laminite*. The local geology description does not account any fill or soil that may be present at the site.

Further, site is located eastly to **Rh** geology, Hawkesbury Sandstone, Wianamatta Group, Middle Triassic age and is described as *Medium to very coarse-grained quartz sandstone, minor laminated mudstone and siltstone lenses*.

### 3.0 GROUND CONDITIONS

Summary of encountered ground profiles within the assessed borehole are provided in Table 2 below. However, reference should be made to the borehole log attached as Appendix B of this report for more details.

**Table 2: Summary of Ground Profile**

Unit	Details	Depth (m)			
		BH2	BH3	BH11	BH12
Top of the Borehole (Approximate RL m AHD)		100.7	103.0	101.9	105.0
Unit 1	FILL, combination of Clay, Silt, and Gravel, low to medium plasticity, fine to medium grained, moist	0.0 – 0.5	0.0 – 0.2	0.0 – 0.7	0.0 – 0.5
	Residual Soil, Silty CLAY, high plasticity, moist	0.5 – 2.4	0.2 – 1.4	0.7 – 2.0	0.5 – 2.2
Unit 2	Class V SANDSTONE, extremely weathered, interbedded with clay bands, moist	2.4 – 6.0	1.4 – 6.2	2.0 – 5.6	2.2 – 6.0
Unit 3	Class IV SANDSTONE, highly weathered, with shale layers	6.0 – 7.1	6.2 – 8.2	5.6 – 6.0	6.0 – 8.7
Unit 4	Class III SANDSTONE, moderately weathered	–	8.2 – 10.2	6.0 – 11.9	8.7 – 11.7

Groundwater seepage was observed at approximate depth of 7.0m below ground level during the augering of the borehole BH2. Furthermore, standing groundwater level was recorded within the installed groundwater wells.

Standing groundwater levels are also subject to a piezometric head at the drilled locations. Further, it should be noted that the levels may be subject to seasonal fluctuations, rainfall, prevailing weather conditions, and future developments of the areas and landforms. Therefore, levels may not be representative of natural groundwater conditions of the site.

## 4.0 GROUNDWATER ASSESSMENT

The groundwater wells GW1(BH2), GW2 (BH12), and GW3 (BH11) have been adopted for the groundwater assessment.

### 4.1 Groundwater Wells

The following information was considered for the groundwater well installation, and details of the wells are provided in Table 3 below:

- Ground conditions encountered within the borehole during the drilling.
- Groundwater seepage levels in the existing wells at the site.
- Proposed bulk excavation level of the development.

**Table 3: Groundwater Well Construction Details**

Well	Total Depth (m)	Screened Length (m)	Surface Level (RL m AHD)	Water Bearing
GW1 (BH2)	7.0	1.0 – 7.0	100.7	Clay and Sandstone
GW2 (BH12)	11.65	1.0 – 11.65	105.0	Clay, Shale, and Sandstone
GW3 (BH11)	11.85	1.0 – 11.85	101.9	Clay, Shale, and Sandstone

The following installation methodology was adopted for groundwater wells:

- 50mm diameter, Class 18PVC threaded and flush joined casing, and 0.45 machine-slotted screens were used.
- Coarse, washed sand and gravel placed in the annulus surrounding the piping to a height of the screen.
- Bentonite pellets were placed in the annulus to form an impermeable plug near the top of the well to prevent surface runoff from entering directly into the well.
- Bentonite pellets placed in the annulus to form an impermeable plug near the top encountered bedrock.
- PVC cap was placed on the casing.

- 100mm diameter stainless steel flushed covers used for all well finishes and concreted onto the ground surface.

## 4.2 Groundwater Levels

The recorded groundwater levels are summarised in Table 4 below.

**Table 4: Standing Groundwater Levels**

Date	Depth (m)		
	GW1	GW2	GW3
3/04/2024	5.8	7.6	6.3
10/04/2024	4.1	7.0	6.8
18/04/2024	5.0	7.0	6.8
24/04/2024	5.6	7.7	7.2
1/05/2024	6.3	8.2	7.0
8/05/2024	6.2	8.6	7.0
16/05/2024	6.8	8.8	7.5
23/05/2024	6.3	8.9	7.4
29/05/2024	6.6	9.3	7.3
6/06/2024	6.5	9.5	7.6
13/06/2024	6.8	9.6	7.9
20/06/2024	6.7	9.8	7.8
25/06/2024	6.5	9.7	7.8
3/07/2024	6.8	9.5	7.7
11/07/2024	6.7	9.7	7.6

### 4.3 Daily Rainfall Survey

A daily rainfall survey was conducted using publicly available data from the Bureau of Meteorology (BOM). The rainfall data was collected from the closest station, located at North Rocks (Muirfield Golf Club) (Station ID 67112), during the groundwater assessment period and listed in Table 5 below.

**Table 5: Daily Rainfall Survey**

Date	Rainfall (mm)	Date	Rainfall (mm)	Date	Rainfall (mm)
3/04/2024	4.5	7/05/2024	2.5	22/06/2024	22.0
5/04/2024	36.5	8/05/2024	2.0	23/06/2024	6.5
6/04/2024	122.5	9/05/2024	4.0	24/06/2024	0.5
10/04/2024	15.0	11/05/2024	8.0	25/06/2024	0.5
17/04/2024	0.5	12/05/2024	18.5	30/06/2024	9.5
18/04/2024	2.5	3/05/2024	1.5	1/07/2024	2.0
24/04/2024	0.5	18/05/2024	0.5	2/07/2024	4.5
1/05/2024	4.5	19/05/2024	0.5	3/07/2024	2.0
2/05/2024	3.0	2/06/2024	80.5	4/07/2024	8.0
3/05/2024	3.0	6/06/2024	6.0	5/07/2024	8.0
4/05/2024	7.5	7/06/2024	36.0	6/07/2024	2.5
5/05/2024	5.0	8/06/2024	2.0	8/07/2024	0.5
6/05/2024	48.5	15/06/2024	15.5	9/07/2024	3.0

A plot of the daily rainfall and groundwater levels recorded is presented in Appendix C of this report.

#### 4.4 Hydraulic Conductivity

A raising head hydraulic conductivity test was carried out within the wells on 9 May 2024 to evaluate the hydraulic conductivity (k) of the site's subsurface condition. The hydraulic conductivity at each well was calculated from the field test results using the Hvorslev Slug Test method and is summarised in Table 6 below.

**Table 6: Results of Hydraulic Conductivity Test**

Groundwater Well	Hydraulic Conductivity, k (m/s)	Water Bearing
GW1	$2.41 \times 10^{-7}$	Clay and Sandstone
GW2	$4.09 \times 10^{-9}$	Clay, Shale, and Sandstone
GW3	$3.84 \times 10^{-8}$	Clay, Shale, and Sandstone

## 5.0 GROUNDWATER INFLOW ANALYSIS

### 5.1 Methodology

Based on the provided architectural drawings, FES selected three sections, identified as Section 1 and Section 2, to carry out the groundwater inflow analysis. The analysis consisted of two-dimensional (2D) Finite Element Analysis (FEA) using a commercial application called "Plaxis 2D Ultimate" to estimate potential groundwater inflow into the bulk excavation of the proposed development. A Site Plan, annexed as Appendix A, presents those nominated sections.

The FEA of the designated sections was performed employing a "Plane Strain" 2-dimensional model with "15-Noded" triangular elements. Within this model, the proposed bulk excavation is modelled as a "Trench Excavation" assuming an infinite extension in the third dimension (z-axis). The Mohr-Coulomb Model was used to describe the behaviours of the soils and rocks adopted for the FEA.

The natural ground level of the site and area immediately adjacent to the site has been adopted from the survey plan, which is annexed as Appendix F in this report. Since the local natural ground levels are gently and consistently sloping, areas outside the site are extrapolated to a maximum distance of 15m from each excavation boundary in all sections. Furthermore, the subsurface conditions, including the groundwater levels for the extrapolated areas, are adopted with consistency with the excavation boundary in the FEA. Given that the site has reasonably shallow bedrock geology, the 15m distance is inferred to be adequate, and FEA will assume that no drawdown effect will be found at this 15m distance.

The excavation boundaries will normally be support by shoring wall system to ensure their stability. However, to enhance the groundwater assessment, the FEA is modelled with "Flow Only" option and no displacement of the ground has been considered in this assessment. Furthermore, the bulk excavation area has been adopted as a "Dry Condition" in the FEA to computer the groundwater inflow.

## 5.2 Subsurface Conditions

The subsurface conditions, as provided in hydrogeological assessment borehole, along with details of the installed groundwater well, have been reviewed to determine the appropriate subsurface conditions for the FEA. Due to size of the site, two sections, identified as Section 1 and Section 2, have been adopted to represent the site conditions for the FEA. The depths and elevations of the subsurface conditions for each section are summarised in Table 7 below.

**Table 7: Subsurface Conditions**

Units	Depth (m)			
	Section 1A	Section 1B	Section 2A	Section 2B
Existing Ground Level (RL m AHD)	100.7	102.5	105.3	99.5
Unit 1	0.0 – 2.4	0.0 – 2.1	0.0 – 2.0	0.0 – 2.2
Unit 2	2.4 – 6.0	2.1 – 5.8	2.0 – 6.2	2.2 – 5.8
Unit 3	6.0 – 8.2	5.8 – 7.4	6.2 – 8.5	5.8 – 6.7
Unit 4	8.2 – 15.7	7.4 – 17.5	8.5 – 20.3	6.7 – 14.5

## 5.3 Groundwater

The groundwater assessment revealed that the highest groundwater level was recorded at a depth of 4.1m below the existing ground level. In accordance with WaterNSW guideline, the highest groundwater level was adopted as the groundwater table across the site for the FEA.

The Slug Test results from the fieldwork testing were found to be consistent to the subsurface condition encountered. Therefore, typical recommended hydraulic conductivity values were adopted for the FEA, and they are summarised in Table 8 below.

**Table 8: Adopted Hydraulic Conductivity**

Units	Hydraulic Conductivity, k (m/s)		$k_y/k_x$
	$k_x$	$k_y$	
Unit 1	$1 \times 10^{-8}$	$1 \times 10^{-8}$	1
Unit 2	$1 \times 10^{-7}$	$1 \times 10^{-7}$	1
Unit 3	$1 \times 10^{-8}$	$1 \times 10^{-8}$	1
Unit 4	$1 \times 10^{-9}$	$1 \times 10^{-9}$	1

The groundwater flow in the Hawkesbury Sandstone and Shale in the Sydney region mainly occurs through defects, which are commonly found to be horizontal. Therefore, the horizontal permeable in these bedrocks are significantly higher than the vertical permeability. However, in the FEA analysis, highest  $k_y$  value has been adopted for all ground profiles to assess the most possible adverse effect.

#### 5.4 Proposed Basement

No structural drawings or briefs were available during the preparation of this report. However, it is understood that the “Drained Basement” system is the preferred option for the proposed development. Therefore, a soldier pile wall with a shotcrete infill system can be considered for the encountered subsurface condition.

The architectural drawings indicate that the proposed Basement Floor level will be at RL 95.9m AHD. Considering that an additional 0.5m excavation is required for the shallow foundation system, it is inferred that the proposed Bulk Excavation Level (BEL) will be at RL 95.4m AHD. Furthermore, the basement dimensions are inferred to be 120m long and 80m wide.

## 6.0 RESULTS

The analysis was conducted for three sections, identified as Section 1 and Section 2, to ensure coverage of all excavation boundaries. The results of the analysis are annexed as Appendix D and summarised in Table 9 below.

**Table 9: Results of the Analysis**

Section	Excavation Boundary	Boundary Length (m)	Inflow (m <sup>3</sup> /s/m)	Inflow (m <sup>3</sup> /day)	Boundary Adopted	Inflow (m <sup>3</sup> /day)
Section 1	1A	80	1.07 x 10 <sup>-8</sup>	0.08	Yes	0.08
	Bottom	80	1.24 x 10 <sup>-8</sup>	0.09	Yes	0.09
	1B	80	1.20 x 10 <sup>-8</sup>	0.09	Yes	0.09
Section 2	2A	120	2.33 x 10 <sup>-8</sup>	0.25	Yes	0.25
	Bottom	120	3.50 x 10 <sup>-9</sup>	0.04	No	–
	2B	120	0	0	Yes	0
<b>Total</b>						<b>0.51</b>

Since both sections provided the estimated groundwater inflow for the bottom excavation boundary, a larger volume of the inflow is incorporated into the final calculation to adopt a safe approach. Therefore, the inflow from the Section 2 bottom excavation boundary has been excluded in the final inflow.

Based on the results it is found that a total inflow of the 510 Liters/day groundwater can seep into the basement excavation in the worst-case scenario. This is estimated to be 0.19ML/year of the groundwater inflow into the basement excavation. If the construction of the development extends to approximately six months, the estimated groundwater removal during the construction stage is around 0.10ML/180 days.

## 7.0 CONCLUSIONS AND RECOMMENDATIONS

This report presents the findings of the preliminary hydrogeological assessment at the nominated sections, identified as Section 1 and Section 2, for the proposed development at 2 Fishburn Crescent, Castle Hill NSW 2154. The groundwater inflow analysis indicates that dewatering will be required for this project, with estimated rates of:

- Construction Phase: **0.10ML over 180 days**.
- Operation Phase: **0.19ML per year**, which is below the 3ML per year threshold.

The assessment further concludes that the proposed development, incorporating a **drained-basement** design, is feasible at the site using industry-standard design and construction methodologies. Based on the estimated groundwater inflow, it is inferred that **Sump and Pump** dewatering system can be adopted for the construction phase. A qualified Hydraulic Engineering Consultant should design an appropriate drainage system for the operation phase based on the findings of this report.

The analysis is based on the short-term groundwater assessment and the likely hydraulic conductivity of the encountered subsurface conditions. There will certainly be variations in the adopted parameters and assumptions, and it is found that the results of the analysis can be within one order of the magnitude. Since the parameters are adopted with higher-end value, based on our experience, the actual groundwater inflow into the basement is more likely to be less than what is estimated in this report. However, if large open water-bearing defects are encountered within the basement excavation, they can be controlled by grouting the defects. The analysis indicates that a **Soldier Pile** wall with a shotcrete infill system is feasible for the encountered groundwater.

This report has been prepared based on a short-term groundwater assessment utilising only two groundwater monitoring wells. Accordingly, the installation of a third well and implementation of long-term monitoring are required to comply with WaterNSW requirements. Furthermore, the report does not demonstrate that the proposed shoring wall system is adequate to accommodate potential ground movement. It is recommended that

the anticipated ground movement be assessed and verified for the selected shoring wall system prior to the construction phase.

The above conclusion is valid subject to the adoption of the followings and should constitute a "Hold Point":

- **Water Quality Testing:** Prior to completing the design, it is recommended to conduct laboratory testing on collected groundwater samples to ensure they comply with WaterNSW groundwater quality requirements. Furthermore, this testing procedure is required during the construction stage to confirm the groundwater quality is still within the acceptable limit.
- **Drainage System:** The basement design and construction should incorporate an adequate drainage system to prevent groundwater inflow into the basement and mitigate potential adverse effects.
- **Dewatering Management Plan:** During the construction stage, groundwater inflow into the basement excavation should be monitored. If present, dewatering of groundwater can be conducted using the sump and pump method until the drainage system is completed. The volume of dewatered groundwater should be measured to verify that the designed drainage system is sufficient to manage the groundwater inflow throughout the building's design life. Additionally, the quality of the groundwater before disposal should be tested prior to disposal to ensure compliance with WaterNSW guidelines. A Dewatering Management Plan should be prepared in accordance with WaterNSW guideline and implement during both the construction phase and the building's design life.

For and on behalf of Foundation Earth Sciences

**Prepared by**



**Chandrasekaran Muralitharan**

Principal Geotechnical Engineer

**Reviewed By**



**Ben Buckley**

Director

## 8.0 LIMITATIONS

The assessment of the sub-surface profile within the proposed development area and the recommendations presented in this report are based on limited information available to date.

The recommendations and advice presented in this report on soil and rock condition is considered to be indicative only as only very limited areas were assessed on site to date. Site inspection by a consulting Geotechnical Engineer or Engineering Geologist are to be undertake when further investigation works are to be carried out to confirm the condition of founding materials in which this geotechnical assessment recommends.

Anecdotal evidence and Information provided by client is assumed to be relevant and to the best of knowledge be appropriate for its interpretation.

There is a possibility that the actual geotechnical and groundwater conditions across the site could differ from the inferred geotechnical assumptions and derivations on which our recommendations are presented in this report. In that case, Foundation Earth Sciences should be contacted for further advise and review of the information provided in this report. Foundation Earth Sciences does not accept any liabilities for the conditions not provided and/or accessible during the preparation of this report. Any ensuring liability resulting from use of this report by third parties cannot be transferred to Foundation Earth Sciences.

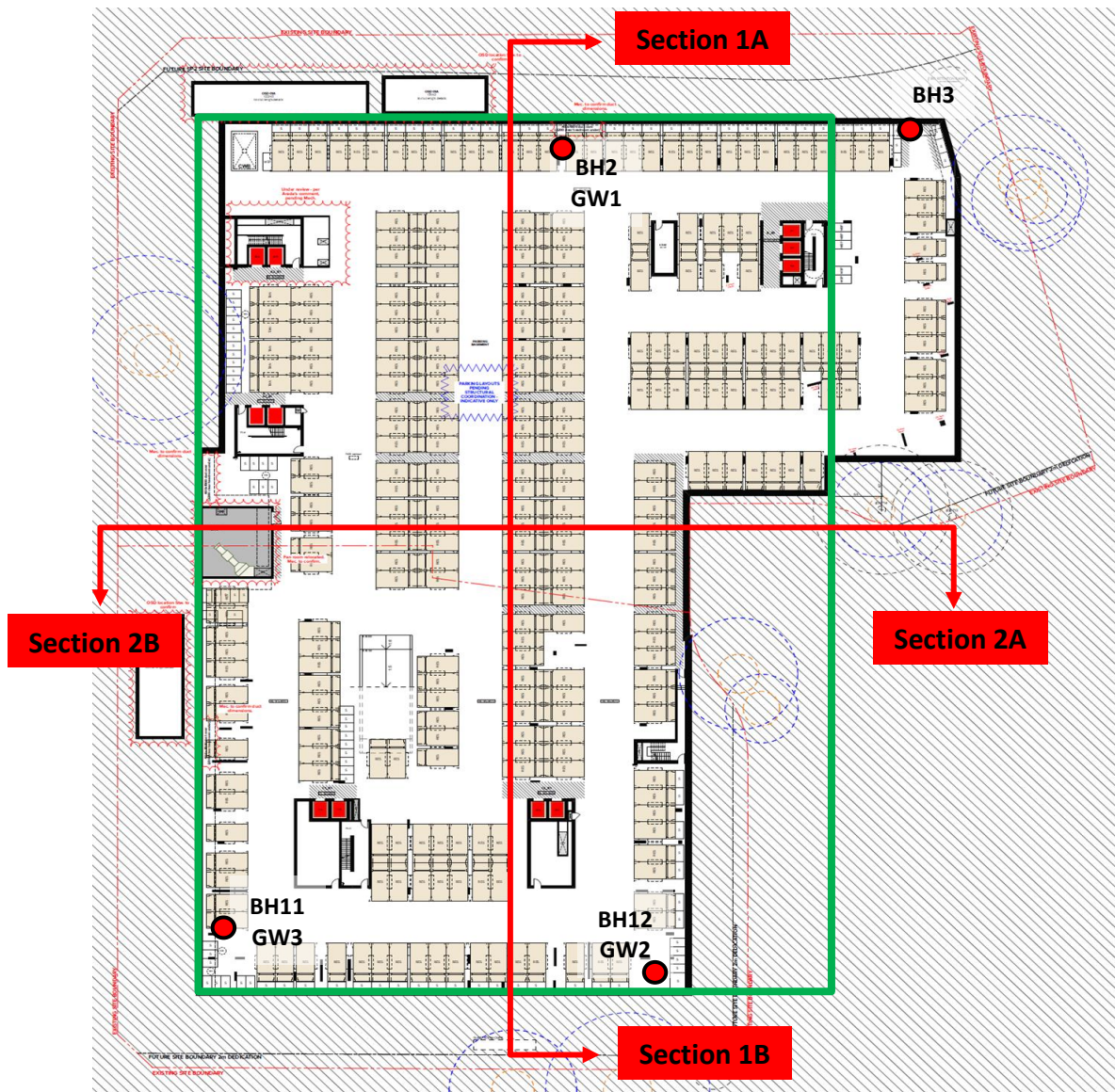
## 9.0 REFERENCES

1. Water NSW guideline titled “Minimum requirements for building site groundwater investigations and reporting” was issued by Department of Planning and Environment (DPE), with referenced to number PUB20/940, revision V02.2210, and dated October 2022
2. Australian Standard – AS1726-2017 “Geotechnical Site Investigation”.
3. Australian Standard – AS 1170.4-2024 “Structural Design Actions – Part 4: Earthquake actions in Australia”.
4. Australian Standard – AS 2870-2011 “Residential slabs and footings”.
5. Australian Standard – AS 2159-2009 “Piling - Design and installation”.
6. Pells, P.J.N, Mostyn, E and Walker, B F – Foundations on Sandstone and Shale in the Sydney Region, Australian Geomechanics Journal, Dec 1998.
7. Pells, P.J.N, Douglas D.J, Rodway, B, Thorne C, McManon B.K – Design Loadings for Foundations on Shale and Sandstone in the Sydney Region. Australian Geomechanics Journal, 1978.

# Appendix A

## Site Plan

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**Key**

● Approximate borehole locations

Not to scale  
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Ref #

G702-3

MM

Site Plan

ARADA Development Management Pty Ltd

2 Fishburn Crescent, Castle Hill NSW 2154

# Appendix B

## Boreholes Logs

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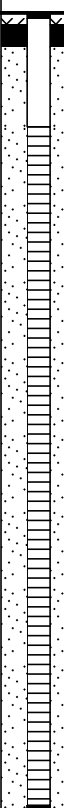





**CLIENT NAME:** ARADA **JOB NUMBER:** G702

**SITE ADDRESS:** Carrington Rd [Middleton/Fishburn/Sexton Ave], Castle Hill NSW **PROJECT:** Geotechnical Investigation

**Date Started :** 6/03/2024 **Completed :** 6/03/2024 **Logged By :** KV/AB **Checked By :** LM

**Borehole Location :** Refer to Site Plan **Surface RL :** 100.7 **Datum :** m AHD

**Equipment :** FICO Drilling Rig **Borehole Size :** 100mm **Slope :** -90°

Method	Water	Well Details	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture	Consistence	Samples Tests Remarks	Additional Observations	Depth (m)
ADT	Water observed during augering at 7m		100.2	0.50			FILL, Silty Clay, medium plasticity, brown/orange	M	[S-F]		Fill	
			99.9	0.80		CL-CH	Silty CLAY, medium plasticity, orange, with some medium gravel	M	[S-F]	Aggressivity + Atterberg Sample	Residual Soil	
			99.5	1.20		CL-CH	Silty CLAY, medium to high plasticity, brown/orange	M	[F-St]			
				2.00		CL-CH	CLAY, medium plasticity, grey/orange, with some ironstone bands and extremely weathered sandstone layers	M	[St]			
			98.3	2.40			SANDSTONE, extremely weathered, fine grained, brown/orange, interbedded with clay and trace of ironstone bands				Sandstone	
			93.6	7.10			Borehole BH2 terminated at 7.10m					

**Comments:**

D - Dry      VS - Very Soft      VL - Very Loose  
M - Moist    S - Soft              L - Loose  
W - Wet      F - Firm              MD - Medium Dense  
                 St - Stiff            D - Dense  
                 VSt - Very Stiff    VD - Very Dense  
                 H - Hard





CLIENT NAME: ARADA JOB NUMBER: G702

SITE ADDRESS: Carrington Rd [Middleton/Fishburn/Sexton Ave], Castle Hill NSW PROJECT: Geotechnical Investigation

Date Started : 6/03/2024 Completed : 6/03/2024 Logged By : KV/AB Checked By : LM

Borehole Location : Refer to Site Plan Surface RL : 103 Datum : m AHD

Equipment : FICO Drilling Rig Borehole Size : 100mm Slope : -90°

Method	Water	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture	Consistence	Samples Tests Remarks	Additional Observations	Depth (m)
ADT		102.8	0.20		CL-CH	FILL, Silty Clay, medium plasticity, brown	M	[S]		Fill	
						CLAY, medium-high plasticity, light brown/grey, trace of medium gravel	M	[F-St]		Residual Soil	
		101.6	1.40			SANDSTONE, extremely weathered, fine grained, brown, interbedded with clay and trace of ironstone bands			Aggressivity + Atterberg Sample SPT 6, 21, 10/50mm	Sandstone	2
	Not Encountered										4
		96.8	6.20			Borehole BH3 continued as cored hole from 6.20m					6
											8
											10
											12

Comments:

D - Dry VS - Very Soft VL - Very Loose  
M - Moist S - Soft L - Loose  
W - Wet F - Firm MD - Medium Dense  
St - Stiff D - Dense  
VSt - Very Stiff VD - Very Dense  
H - Hard

CLIENT NAME: ARADA JOB NUMBER: G702

SITE ADDRESS: Carrington Rd [Middleton/Fishburn/Sexton Ave], Castle Hill NSW PROJECT: Geotechnical Investigation

Date Started : 6/03/2024 Completed : 6/03/2024 Logged By : KV/AB Checked By : LM

Borehole Location : Refer to Site Plan Surface RL : 103 Datum : m AHD

Equipment : FICO Drilling Rig Borehole Size : 100mm Slope : -90°

Method	Water	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering	Estimated Strength					Is (50) (MPa)	RQD %	Defect Spacing (mm)	Defect Description	Depth (m)
							EL	VL	J	M	H					
			2													2
			4													4
			6													6
NMLC	Not Encountered	96.8	6.20		SHALE, highly weathered, brown/dark grey with light grey laminations, trace of ironstone staining	HW					D A 0.21 0.39			6.26m J,P,S, 0-5° 6.35m J,P,S, 10-20° 6.40m J,P,S, 0-5° 6.41m J,P,S, 0-5° 6.43m J,P,S, 0-5° 6.46m J,P,S, 0-5° 6.51m J,P,S, 0-5° 6.57m, Clay Seam 6.66m J,P,S, 0-5° 6.68m, Clay Seam 6.73m J,P,S, 0-5° 6.78m J,P,S, 10-20° 6.81m J,Ir,S, 0-5° 6.84m J,Ir,S, 40-50° 6.90m J,P,S, 0-5° 6.93m, Fragment Zone, 100mm 7.05m, Fragment Zone, 50mm 7.13m J,Ir,S, 5-10° 7.14m J,Ir,S, 5-10° 7.17m J,Cu,S, 40-50° 7.25m, Barrell Lift 7.33m J,P,S, 0-5° 7.37m J,P,S, 0-5° 7.45m J,Ir,S, 0-5° 7.57m J,P,S, 0-5° 7.62m J,P,S, 0-5° 7.68m J,P,S, 5-10° 7.75m J,P,S, 0-5° 7.81m, Clay Seam 7.82m J,Ir,S, 40-50° 7.95m J,P,S, 0-5° 8.00m J,P,S, 0-5° 8.08m J,P,S, 5-10° 8.14m J,P,S, 0-5° 8.25m, Extremely Weathered Seam 8.36m J,P,S, 0-5° 8.45m J,P,S, 0-5° 8.58m, Fractured Zone, 30mm 8.78m, Barrell Lift 8.83m J,P,S, 0-5° 8.94m, Hand Break 8.96m, Fracture Zone, 40mm 9.00m J,Cu,S, 40-50° 9.10m J,P,S, 0-5° 9.32m J,P,S, 10-20° 9.44m, Extremely Weathered Seam 9.56m J,P,S, 10-20° 9.81m J,P,R, 40-50°		8
		94.8	8.24		SANDSTONE, highly to moderately weathered, fine grained, grey, with some ironstone staining	HW-MW					D A 0.24 0.49				10	
		92.8	10.17		BH3 terminated at 10.17m						D A 1.43 1.28				12	

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<b>Comments:</b>	Weathering	EL - Extremely Low	D - Diametral	J - Joint	MB - Mechanical Break	S - Smooth
	EW - Extremely	VL - Very Low	A - Axial	B - Bedding Plan	HB - Handling Break	R - Rough
	HW - Highly	L - Low		C - Clay Seams		P - Polished
	MW - Moderately	M - Medium		FZ - Fractured Zone	PI - Planar	
	SW - Slightly	H - High		IS - Infill Seam	Ir - Irregular	Qz - Quartz
	Fr - Fresh	VH - Very High		SS - Sheared Seam	Cu - Curved	Fe - Iron Stain
		EH - Extremely High		CZ - Crushed Zone	St - Stepped	

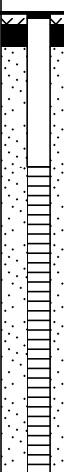


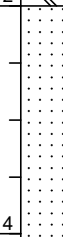
CLIENT NAME: ARADA JOB NUMBER: G702

SITE ADDRESS: Carrington Rd [Middleton/Fishburn/Sexton Ave], Castle Hill NSW PROJECT: Geotechnical Investigation

Date Started : 7/03/2024 Completed : 7/03/2024 Logged By : KV/AB Checked By : LM

Borehole Location : Refer to Site Plan Surface RL : 101.9 Datum : m AHD

Equipment : Commachio 305 Borehole Size : 100mm Slope : -90°

Method	Water	Well Details	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture	Consistence	Samples Tests Remarks	Additional Observations	Depth (m)
ADT			101.2	0.70			FILL, Clayey Silt, low to medium plasticity, brown	M	[S]		Fill	
						CL-CH	Silty CLAY, medium plasticity, brown/orange	M	[St]	Aggressivity + Atterberg Sample	Residual Soil	
			99.9	2.00			SANDSTONE, extremely weathered, fine grained, brown/grey, with some clay and trace of ironstone bands			SPT 3, 8, 16 N=24	Sandstone	2
												4
			97.7	4.20			Borehole BH11 continued as cored hole from 4.20m					
		Not Encountered		6								6
				8								8
				10								10
				12								12

Comments:

D - Dry VS - Very Soft VL - Very Loose  
M - Moist S - Soft L - Loose  
W - Wet F - Firm MD - Medium Dense  
St - Stiff D - Dense  
VSt - Very Stiff VD - Very Dense  
H - Hard

CLIENT NAME: ARADA JOB NUMBER: G702

SITE ADDRESS: Carrington Rd [Middleton/Fishburn/Sexton Ave], Castle Hill NSW PROJECT: Geotechnical Investigation

Date Started : 7/03/2024 Completed : 7/03/2024 Logged By : KV/AB Checked By : LM

Borehole Location : Refer to Site Plan Surface RL : 101.9 Datum : m AHD

Equipment : Commachio 305 Borehole Size : 100mm Slope : -90°

Method	Water	Well Details	RL (m)	Depth (m)	Graphic Log	Material Description	Weathering	Estimated Strength					Is (50) (MPa)	RQD %	Defect Spacing (mm)	Defect Description	Depth (m)
								EL	VL	J	M	H					
				2													
				4													
			97.7	4.20		SHALE, highly weathered, brown/dark grey	HW								4.20m Clay Band, 40mm		
			97.0	4.90		SANDSTONE, highly weathered, fine grained, brown/red, interbedded with shale and ironstone bands									4.29m J.P.S, 0-5°		
			96.3	5.59		SANDSTONE, highly to moderately weathered, fine grained, grey/light brown	HW-MW								4.34m J.P.S, 0-5°		
				6											4.38m Fragmented Zone, 30mm		
				8											4.47m J.P.S, 0-5°		
				10											4.50m J.P.S, 0-5°		
				12											4.58m J.P.S, 0-5°		
															4.63m Clay Band, 40mm		
															4.75m J.P.S, 0-5°		
															4.92m Clay Seam		
															5.00m J.P.S, 5-10°		
															5.09m Clay Band, 30mm		
															5.13m Barrell Lift		
															5.16m J.P.S, 0-5°		
															5.21m J.P.S, 0-5°		
															5.26m J.P.S, 0-5°		
															5.27m J.P.S, 0-5°		
															5.30m J.P.S, 0-5°		
															5.37m Extremely Weathered Seam		
															5.47m J.St.S, 10-20°		
															5.51m Clay Band, 50mm		
															5.58m J.P.S, 0-5°		
															5.70m J.P.S, 0-5°		
															6.00m Hand Break		
															6.72m J.P.S, 0-5°		
															6.80m Hand Break		
															7.00m Hand Break		
															7.23m Barrell Lift		
															7.42m J.P.S, 0-5°		
															7.54m J.P.S, 0-5°		
															7.77m J.P.S, 30-40°		
															8.00m Hand Break		
															8.79m Barrell Lift		
															9.00m Hand Break		
															9.41m J.P.S, 30-40°		
															9.67m J.P.S, 30-40°		
															10.00m Hand Break		
															10.86m J.P.S, 0-5°		
															11.00m Hand Break		
															11.24m J.P.S, 0-5°		
															11.35m J.P.S, 20-30°		
															11.39m J.P.S, 20-30°		
			90.1	11.85													

BH11 terminated at 11.85m

Comments:	Weathering	EL - Extremely Low	D - Diametral	J - Joint	MB - Mechanical Break	S - Smooth
	EW - Extremely	VL - Very Low	A - Axial	B - Bedding Plan	HB - Handing Break	R - Rough
	HW - Highly	L - Low		CS - Clay Seams		P - Polished
	MW - Moderately	M - Medium		FZ - Fractured Zone	PI - Planar	
	SW - Slightly	H - High		IS - Infill Seam	IR - Irregular	QZ - Quartz
	Fr - Fresh	VH - Very High		SS - Sheared Seam	CU - Curved	FE - Iron Stain
		EH - Extremely High		CZ - Crushed Zone	ST - Stepped	

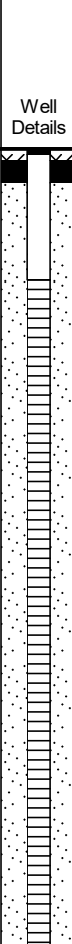






CLIENT NAME: ARADA JOB NUMBER: G702

SITE ADDRESS: Carrington Rd [Middleton/Fishburn/Sexton Ave], Castle Hill NSW PROJECT: Geotechnical Investigation

Date Started : 7/03/2024 Completed : 7/03/2024 Logged By : KV/AB Checked By : LM

Borehole Location : Refer to Site Plan Surface RL : 105 Datum : m AHD








Equipment : Commachio 305 Borehole Size : 100mm Slope : -90°

Method	Water	Well Details	RL (m)	Depth (m)	Graphic Log	Classification Symbol	Material Description	Moisture	Consistence	Samples Tests Remarks	Additional Observations	Depth (m)	
ADT			105.0	0.05			DGB Road Base, 50mm FILL, Clayey Silt, low to medium plasticity, brown	M	[S-F]		Road Base Fill		
			104.5	0.50		CL-CH	Silty CLAY, medium plasticity, brown	M	[F-St]	SPT 7, 8, 11 N=19 Aggressivity + Atterberg Sample	Residual Soil		
			104.2	0.80		CL-CH	Silty CLAY, medium plasticity, grey, interbedded with extremely weathered sandstone	M	[VSt-H]				
				2.00									2
			102.8	2.20			SANDSTONE, extremely weathered, fine grained, grey, with some clay and trace of ironstone bands					Sandstone	
			99.0	6.00			SHALE, extremely to highly weathered, dark grey, with some clay				Shale	6	
			98.0	7.00			Borehole BH12 continued as cored hole from 7.00m					8	
				10								10	
				12								12	

Comments:

D - Dry  
M - Moist  
W - Wet  
VS - Very Soft  
S - Soft  
F - Firm  
St - Stiff  
VSt - Very Stiff  
H - Hard  
VL - Very Loose  
L - Loose  
MD - Medium Dense  
D - Dense  
VD - Very Dense

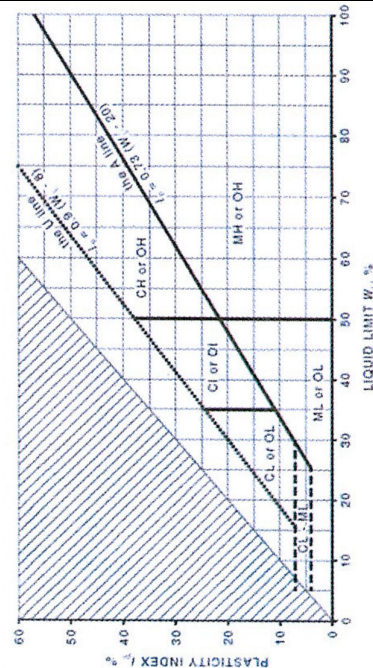


 FILL	 ORGANIC SOILS (OL, OH, Pt)	 CLAY (CL, CI, or CH)
 COUBLES or BOULDERS	 SILT (ML or MH)	 SAND (SP or SW)
 GRAVEL (GP or GW)	Combinations of these basic symbols may be used to indicate mixed materials	

## CLASSIFICATION AND INFERRED STRATIGRAPHY

Soil is broadly classified and described in borehole logs using the preferred method given in AS 1726:2017, section 6.1 soil descriptions and classification.

GROUP SYMBOLS			PARTICLE SIZE CHARACTERISTICS						
MAJOR DIVISIONS	SYMBOL	DESCRIPTION	Fraction	Components	Subdivision	Size mm			
<b>COARSE GRAINED SOILS</b> More than 65% of the soil excluding oversize fraction is greater than 0.075mm	<b>GRAVEL</b> More than 50% of the coarse fraction is >2.36mm	GW	Well graded gravel and gravel-sand mixtures, little or no fines.	Over-size	BOULDERS	>200			
		GP	Poorly graded gravel and gravel-sand mixtures, little or no fines.		COBBLES	63-200			
		GM	Silty gravel, gravel-sand-clay mixtures.		GRAVEL	Coarse Medium Fine	19-63 6.7-19 2.36-6.7		
		GC	Clayey gravel, gravel-sand-clay mixtures.		SAND	Coarse Medium Fine	0.6-2.36 0.21-0.6 0.075-0.21		
	<b>SAND</b> More than 50% of the coarse fraction is <2.36mm	SW	Well graded sand and gravelly sand, little or no fines.		SILT		0.002-0.075		
		SP	Poorly graded sand and gravelly sand, little or no fines.		CLAY		<0.002		
		SM	Silty sand, sand-silt mixtures.	<b>PLASTICITY PROPERTIES</b>					
		SC	Clayey sand, sandy-clay mixtures.						
		<b>FINE GRAINED SOILS</b> More than 35% of the soil excluding oversize fraction is less than 0.075mm	Liquid limit less <50%					ML	Inorganic silts of low plasticity, very fine sands, rock flour, silty or clayey fine sands.
								CL, CI	Inorganic silts of low to medium plasticity, gravelly clays, sandy clays, silty clays
Liquid limit > 50%	MH		Inorganic silts of high plasticity.						
	CH	Inorganic clays of high plasticity.							
	OH	Organic clays of medium to high plasticity.							
Highly organic soil	PT	Peat muck and other highly organic soils.							



## ABBREVIATIONS AND DESCRIPTIONS FOR ROCK MATERIAL AND DEFECTS

<b>MOISTURE CONDITION</b>		
<b>Symbol</b>	<b>Term</b>	<b>Description</b>
<b>D</b>	<b>Dry</b>	Non-cohesive and free running
<b>M</b>	<b>Moist</b>	Soils feel cool, darkened in colour. Soil tends to stick together.
<b>W</b>	<b>Wet</b>	Soils feel cool, darkened in colour. Soil tends to stick together, free water forms when handling.

Moisture content of cohesive soils shall be described in relation to plastic limit (PL) or liquid limit (LL) for soils with higher moisture content as follows: Moist, dry of plastic limit ( $w < PL$ ); Moist, near plastic limit ( $w = PL$ ); Moist, wet of plastic limit ( $w < PL$ ); Wet, near liquid limit ( $w = LL$ ), Wet, wet of liquid limit ( $w > LL$ ),

<b>CONSISTENCY</b>				<b>DENSITY</b>			
<b>Symbol</b>	<b>Term</b>	<b>Undrained Shear Strength (kPa)</b>	<b>SPT "N" #</b>	<b>Symbol</b>	<b>Term</b>	<b>Density Index %</b>	<b>SPT "N" #</b>
<b>VS</b>	<b>Very Soft</b>	<b>&lt; 12</b>	<b>&lt; 2</b>	<b>VL</b>	<b>Very Loose</b>	<b>&lt; 15</b>	<b>0 to 4</b>
<b>S</b>	<b>Soft</b>	<b>&gt; 12 to &lt; 25</b>	<b>&gt; 2 to &lt; 4</b>	<b>L</b>	<b>Loose</b>	<b>&gt; 15 to &lt; 35</b>	<b>4 to 10</b>
<b>F</b>	<b>Firm</b>	<b>&gt; 25 to &lt; 50</b>	<b>&gt; 4 to &lt; 8</b>	<b>MD</b>	<b>Medium Density</b>	<b>&gt; 65 to &lt; 85</b>	<b>10 to 30</b>
<b>St</b>	<b>Stiff</b>	<b>&gt; 50 to &lt; 100</b>	<b>&gt; 8 to 15</b>	<b>D</b>	<b>Dense</b>	<b>&gt; 65 to &lt; 85</b>	<b>30 to 50</b>
<b>H</b>	<b>Hard</b>	<b>&gt; 200</b>	<b>&gt; 30</b>				
<b>Fr</b>	<b>Friable</b>	<b>-</b>					

In the absence of test results, consistency and density may be assessed from correlations with the observed behaviour of the material. #SPT correlations are not stated in AS1726:2017 and may be subject to corrections for overburden pressure and equipment type.

<b>MINOR COMPONENTS</b>		
<b>TERM</b>	<b>ASSESSMENT GUIDE</b>	<b>PROPORTION BY MASS</b>
<b>Trace</b>	Presence just detectable by feel or eye but soil properties little or no difference to general properties of primary component	Coarse grained soils: <5% Fine grained soils: <15%
<b>With</b>	Presence easily detectable by feel or eye but soil properties little or no difference to general properties of primary component	Coarse grained soils: 5-12% Fine grained soils: 15-30%
<b>Prefix</b>	Presence easily detectable by feel or eye in conjunction with the general properties of primary component	Coarse grained soils: >12% Fine grained soils: >30%

### CLASSIFICATION AND INFERRED STRATIGRAPHY

Rock is broadly classified and described in borehole logs using the preferred method given in AS 1726:2017, section 6.2 Rock identification, descriptions, and classification.

<b>ROCK MATERIAL DESCRIPTION</b>			
<b>Layering</b>		<b>Structure</b>	
<b>Term</b>	<b>Description</b>	<b>Term</b>	<b>Spacing (mm)</b>
Massive	No Layering apparent	Thinly Laminated	<6
		Laminated	6-20
Poorly Developed	Layering just visible; little effect on properties	Very thinly bedded	20-60
		Thinly bedded	60-200
Well Developed	Layering (bedding, foliation, cleavage) distinct; rock breaks more easily parallel to layering	Medium bedded	200-600
		Thickly bedded	600-2,000
		Very thickly bedded	>2,000

ABBREVIATIONS AND DESCRIPTIONS FOR DEFECT TYPES		
Defect Type	Abbr.	Description
Joint	JT	Surface of a fracture or parting, formed without displacement, across which the rock has little or no tensile strength. May be closed or filled by air, water, soil, or rock substance, which acts as cement.
Bedded Parting	BP	Surface of fracture or parting, across which the rock has little or no tensile strength, parallel or sub-parallel to layering/bedding. Bedding refers to the layering or stratification of a rock, indicating orientation during deposition, resulting in planar anisotropy in the rock material.
Foliation	FL	Repetitive planar structure parallel to the shear direction or perpendicular to the direction of higher pressure, especially in the metamorphic rock, e.g., schistosity (SH) and Gneissosity.
Contact	CO	The surface between two types or ages of rock.
Cleavage	CL	Cleavage planes appear as parallel, closely spaced, and planar surfaces resulting from mechanical fracturing of rock through deformation or metamorphism, independent of bedding.
Sheared Seam/ Zone (Fault)	SS/SZ	Seam or zone with roughly parallel almost planar boundaries of rock substance cut by closely spaced (often <50mm) parallel and usually smooth or slickensided joints or cleavage planes.
Crushed seam/ Zone (Fault)	CS/CZ	Seam or zone composed of disoriented usually angular fragments of the host rock substance, with rough parallel near-planar boundaries. The brecciated fragments may be of clay, silt, sand or gravel sizes or mixtures of these.
Decomposed seam/Zone	DS/DZ	Seam of soil substance, often with gradational boundaries, formed by weathering of the rock material in places.
Infilled Seam	IS	Seam of soil substance, usually clay or clayey, with very distinct roughly parallel boundaries, formed by soil migrating into joint or open cavity.
Schistosity	SH	The foliation in schist or other coarse grained crystalline rock due to the parallel arrangement of platy or prismatic mineral grains, such as mica.
Vein	VN	Distinct sheet-like body of minerals crystallised within rock through typically open-space filling or crack-seal growth.

ABBREVIATIONS AND DESCRIPTIONS FOR DEFECT SHAPE AND ROUGHNESS					
Shape	Abbr.	Description	Roughness	Abbr.	Description
Planar	PI	Consistent Orientation	Polished	Pol	Shiny smooth surface
Curved	Cu	Gradual change in orientation	Slickensided	SL	Grooved or stained surface, usually polished
Undulating	Un	Wavy surface	Smooth	S	Smooth to touch. Few or no surface irregularities
Stepped	St	One or more well defined steps	Rough	RF	Many small surface irregularities, amplitude generally <1mm. Feels like fine to coarse sandpaper
Irregular	Ir	Many sharp changes in orientation	Very Rough	VR	Many large surface irregularities, amplitude generally >1mm. Feels like very coarse sandpaper.

**Orientation:**  
**Vertical Boreholes-** The dip (inclination from horizontal) of the defect.  
**Inclined Boreholes-** The inclination is measured as the acute angle to the core axis.

ABBREVIATIONS AND DESCRIPTIONS FOR DEFECT COATING			DEFECT APERTURE		
Coating	Abbr.	Description	Aperture	Abbr.	Description
Clean	CN	No Visible coating or infilling	Closed	CL	Closed.
Stain	SN	No visible coating but surfaces are discoloured by staining, often limonite (orange-brown)	Open	O	Without any infill material
Veneer	VNR	A visible coating of soil or mineral substances, usually too thin to measure (<1mm); may be patchy	Infilled	-	Soil or rock i.e., clay, talc, pyrite, quartz, etc.

### CLASSIFICATION AND INFERRED STRATIGRAPHY

Rock is broadly classified and described in borehole logs using the preferred method given in AS 1726:2017, section 6.2 Rock identification, descriptions, and classification.

### ROCK MATERIAL STRENGTH CLASSIFICATION

Symbol	Term	Point Load Index Is (50) (MPa)#	Field Guide
VL	Very Low	0.03 To 0.01	Material crumbles under firm blows with sharp end of pick; can be peeled with knife; too hard to cut triaxial sample by hand. Pieces up to 30mm can be broken by finger pressure.
L	Low	0.1 to 0.3	Easily scored with knife; indentations 1mm to 3mm show in the specimen with firm blows of pick point; has dull sound under hammer. A piece of core 150mm long by 50mm diameter may be broken by hand. Sharp edges of core may be friable and break during handling.
M	Medium	0.3 – 1	Readily scored with a knife; a piece of core 150mm long by 50mm diameter can be broken by hand with difficulty
H	High	1 to 3	A piece of core 150mm long by 50mm diameter cannot be broken by hand but can be broken with a pick with a single firm blow; rock rings under hammer.
VH	Very High	3 to 10	Hand specimen breaks with pick after more than one blow; rock rings under hammer.
EH	Extremely High	>10	Specimen requires many blows with geological pick to break through intact material; rock rings under hammer.

#Rock Strength Test Results



Point Load Strength Index, Is (50) Axial Test (MPa)



Point Load Strength Index, Is (50), Diametral test (MPa)

Relationship between rock strength test result (Is (50)) and unconfined compressive strength (UCS) will vary with rock type and strength and should be determined on a site-specific basis. However, UCS is typically 20 x Is (50)

### ROCK MATERIAL WEATHERING CLASSIFICATION

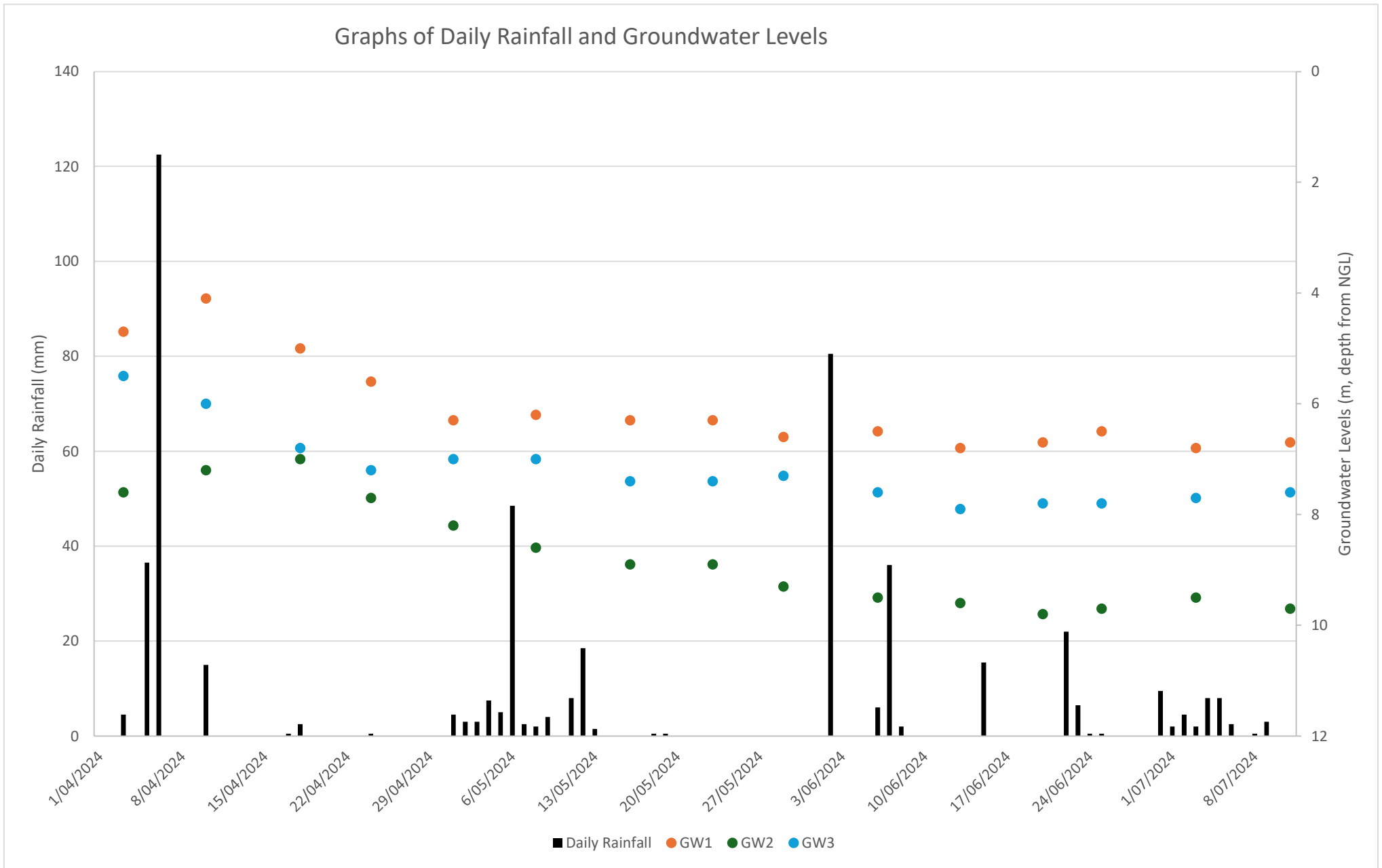
Symbol	Term	Field Guide
RS	Residual Soil	Soil developed on extremely weathered rock; the mass structure and substance fabric no longer evident; there is a large change in volume, but the soil has not been significantly transported.
XW	Extremely Weathered	Rock is weathered to such an extent that it has soil properties- i.e., it either disintegrates or can be remoulded, in water.
DW	HW	Rock strength usually changed by weathering. The rock may be highly discoloured, usually by iron staining. Porosity may be increased by leaching or may be decreased due to decomposition of weathered product in pores. In some environments it is convenient to subdivide into highly weathered and moderately weathered, with the degree of alteration typically less for MW
	MW	
SW	Slightly Weathered	Rock slightly discoloured but shows little to no change of strength relative to fresh rock.
FR	Fresh	Rock shows no sign of decomposition or staining.

# Appendix C

## Groundwater Level and Daily Rainfall

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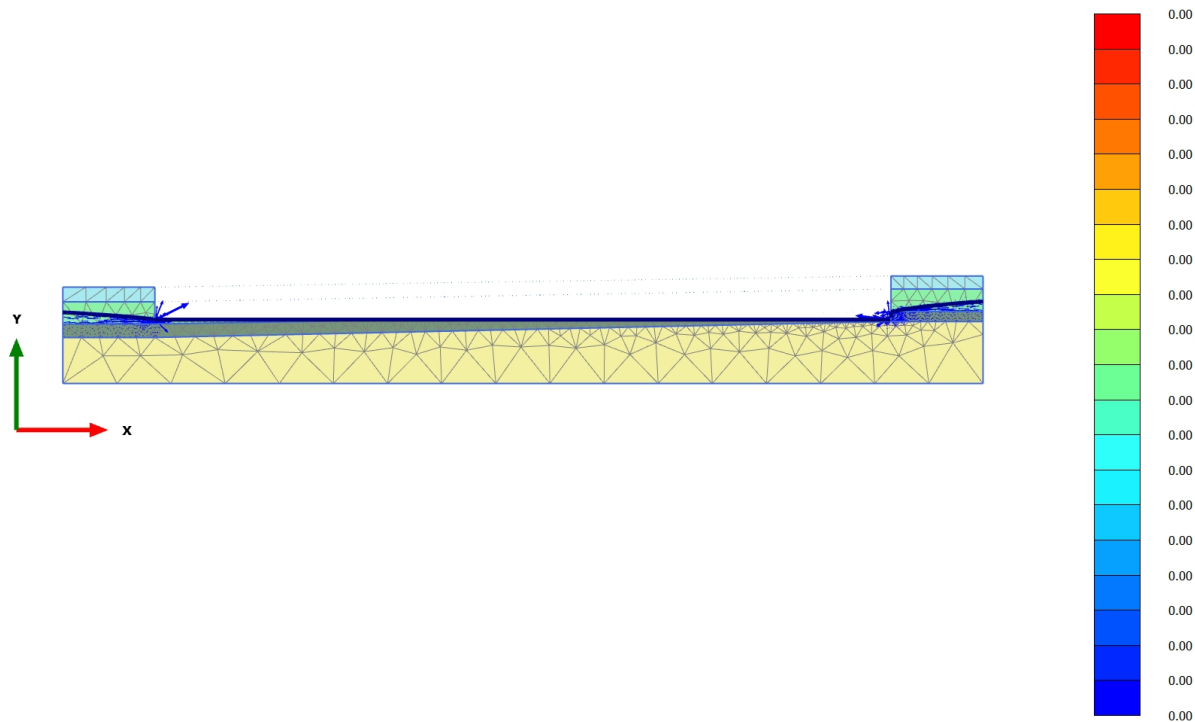
### Graphs of Daily Rainfall and Groundwater Levels



# Appendix D

## Groundwater Inflow Analysis

---

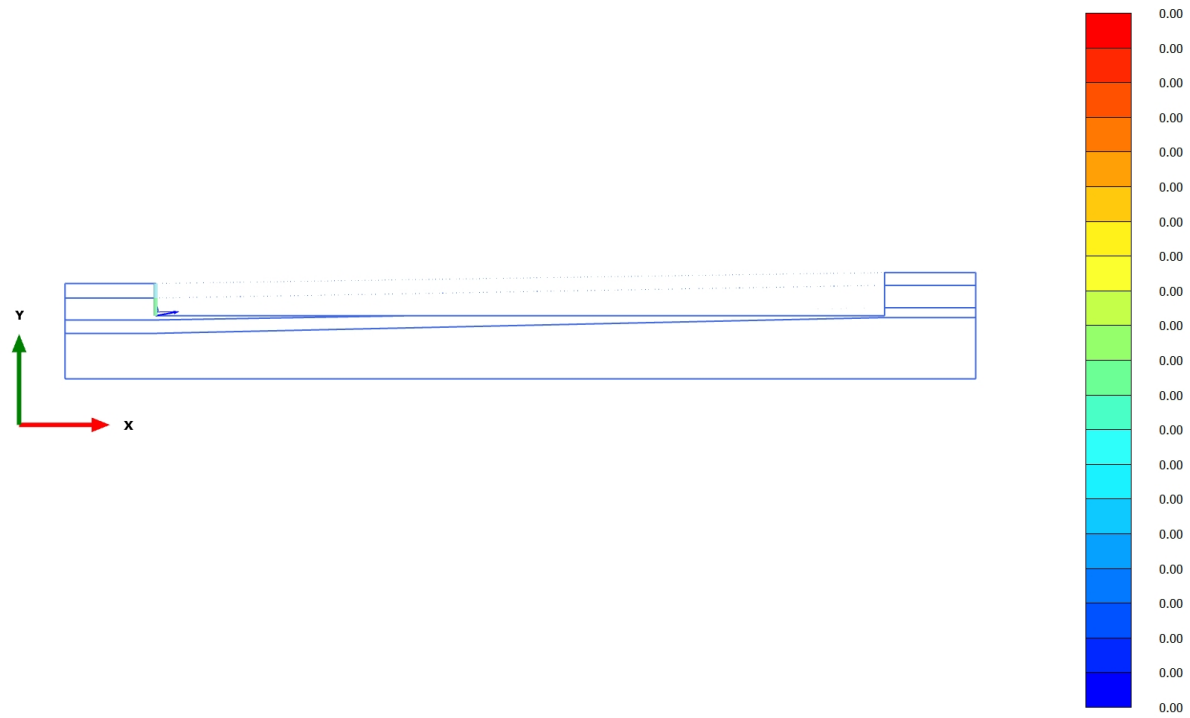
4.1.1 Calculation results, Excavation [Phase\_1] (1/1), Groundwater flow  $|q|$ 

**Groundwater flow  $|q|$  (scaled up  $200 \cdot 10^6$  times) (Time 0.000 s)**

Maximum value =  $0.03201 \cdot 10^{-6}$  m/s (Element 557 at Stress point 6674)

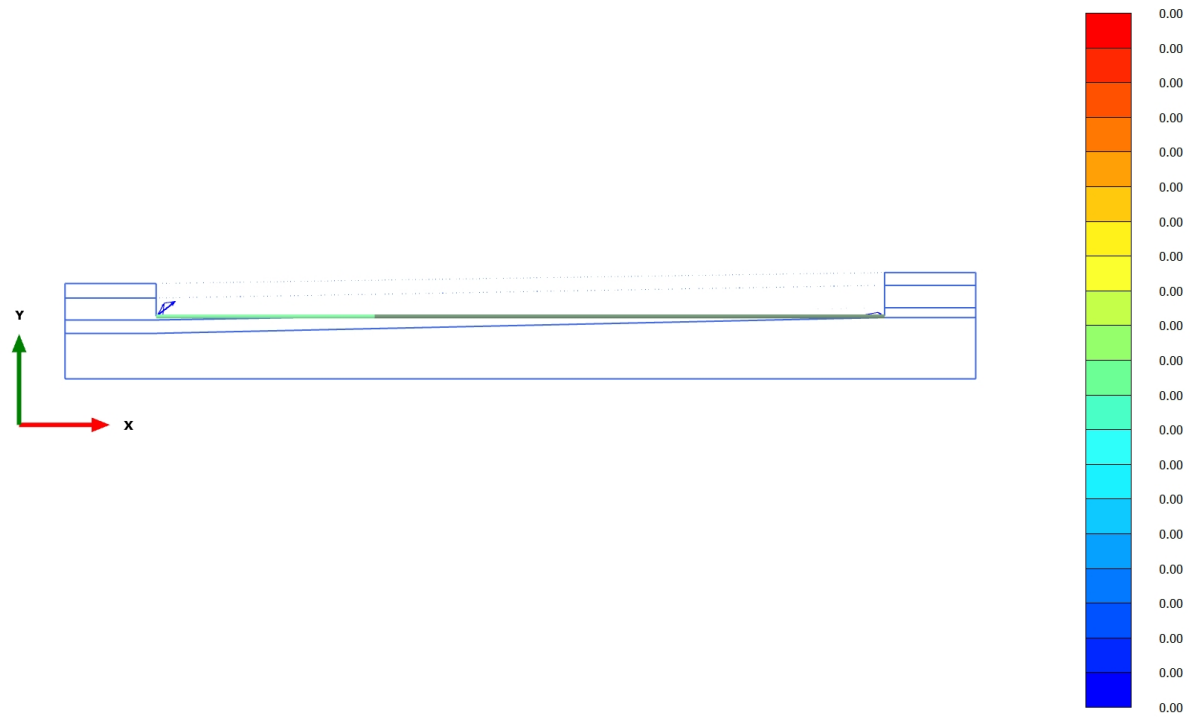
Minimum value =  $0.3779 \cdot 10^{-15}$  m/s (Element 1100 at Stress point 13198)

### 4.2.1 Calculation results, cross section A-A\*, Excavation [Phase\_1] (1/1), Groundwater flow |q|



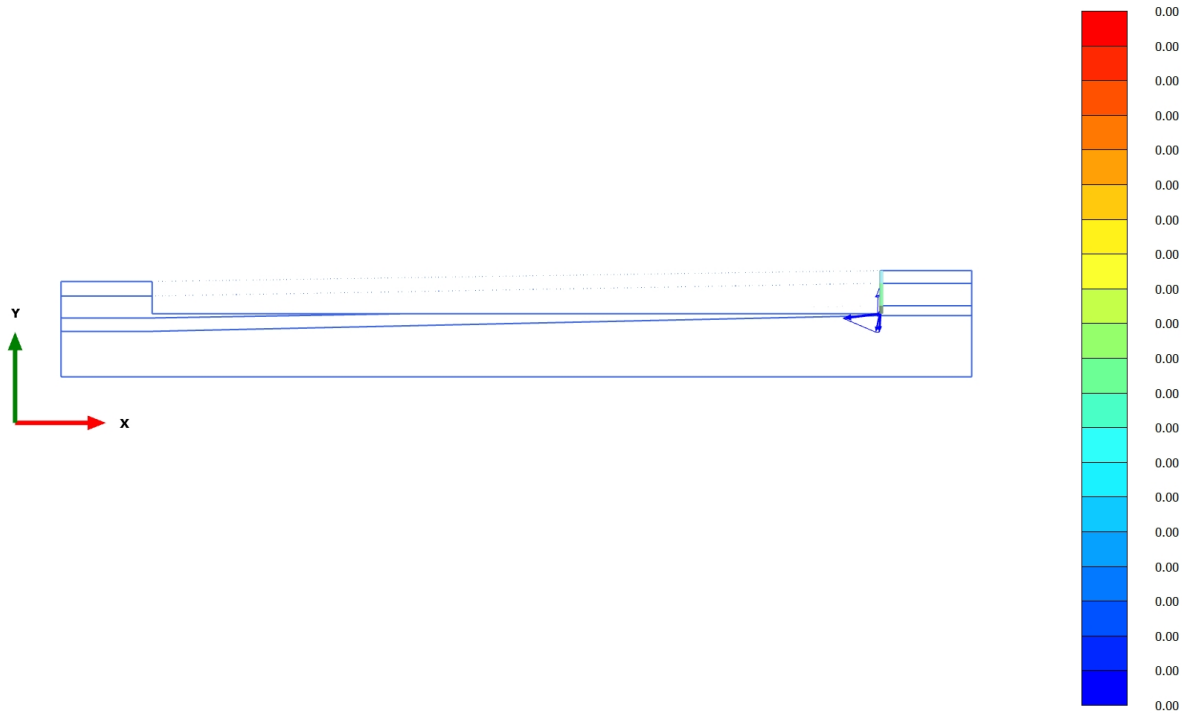
**Groundwater flow |q| (scaled up  $200 \cdot 10^6$  times) (Time 0.000 s)**  
 Maximum value =  $0.02005 \cdot 10^{-6}$  m/s  
 Minimum value =  $0.02851 \cdot 10^{-12}$  m/s  
 Total discharge is  $0.01062 \cdot 10^{-6}$  m<sup>3</sup>/s/m

### 4.1.1 Calculation results, cross section A-A\*, Excavation [Phase\_1] (1/1), Groundwater flow |q|

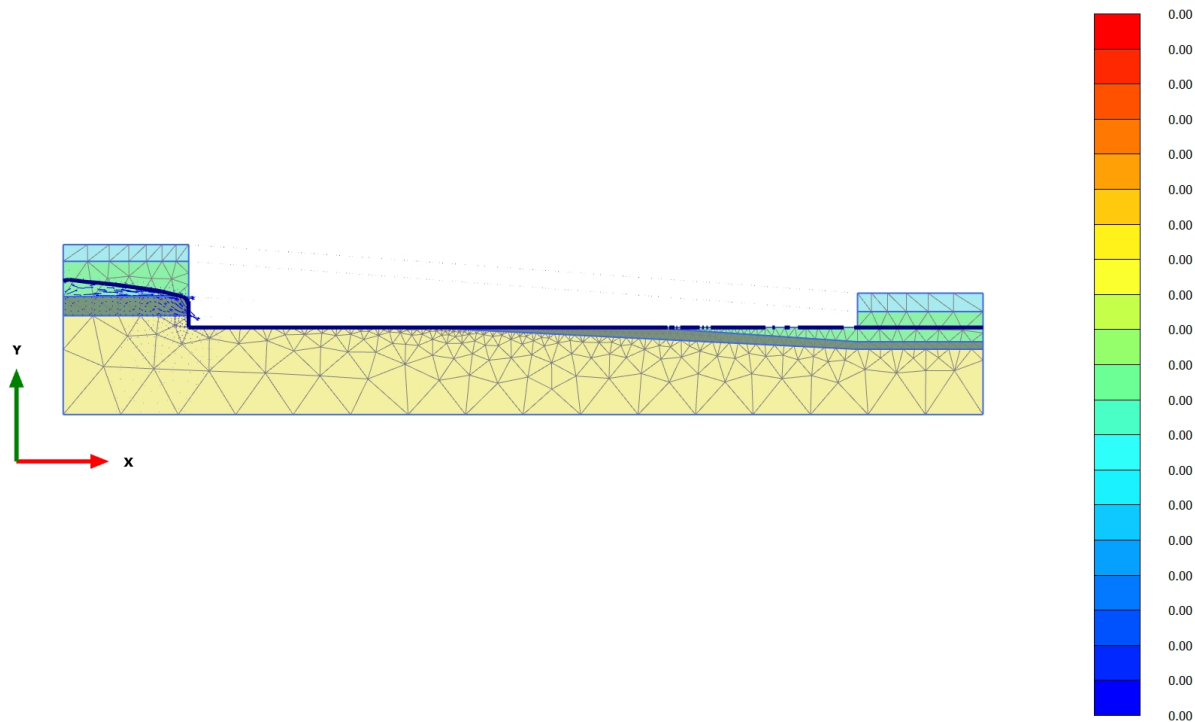


**Groundwater flow |q| (scaled up  $200 \times 10^6$  times) (Time 0.000 s)**  
 Maximum value =  $0.02036 \times 10^{-6}$  m/s  
 Minimum value =  $6.265 \times 10^{-15}$  m/s  
 Total discharge is  $0.01233 \times 10^{-6}$  m<sup>3</sup>/s/m

### 4.1.1 Calculation results, cross section A-A\*, Excavation [Phase\_1] (1/1), Groundwater flow |q|



**Groundwater flow |q| (scaled up  $500 \cdot 10^6$  times) (Time 0.000 s)**  
 Maximum value =  $0.01261 \cdot 10^{-6}$  m/s  
 Minimum value =  $0.02387 \cdot 10^{-12}$  m/s  
 Total discharge is  $-0.01196 \cdot 10^{-6}$  m<sup>3</sup>/s/m

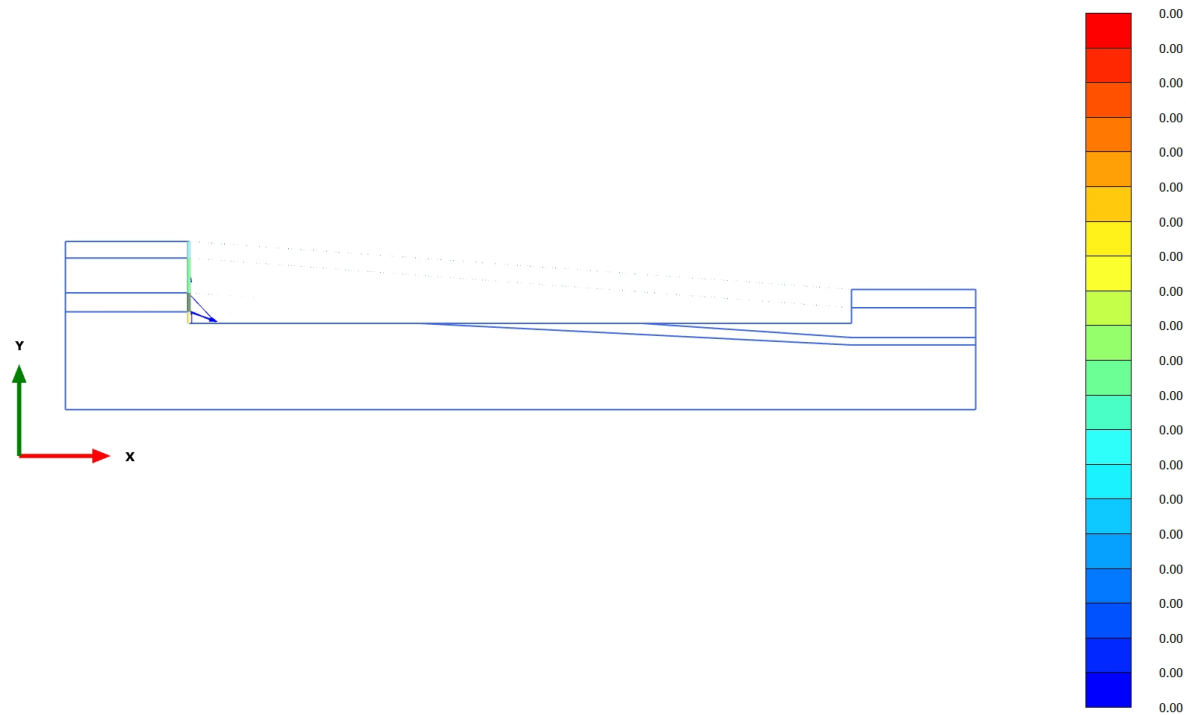
4.1.1 Calculation results, Excavation [Phase\_1] (1/1), Groundwater flow  $|q|$ 

**Groundwater flow  $|q|$  (scaled up  $100 \cdot 10^6$  times) (Time 0.000 s)**

Maximum value =  $0.02402 \cdot 10^{-6}$  m/s (Element 161 at Stress point 1929)

Minimum value =  $1.224 \cdot 10^{-21}$  m/s (Element 661 at Stress point 7931)

#### 4.2.1 Calculation results, cross section A-A\*, Excavation [Phase\_1] (1/1), Groundwater flow |q|



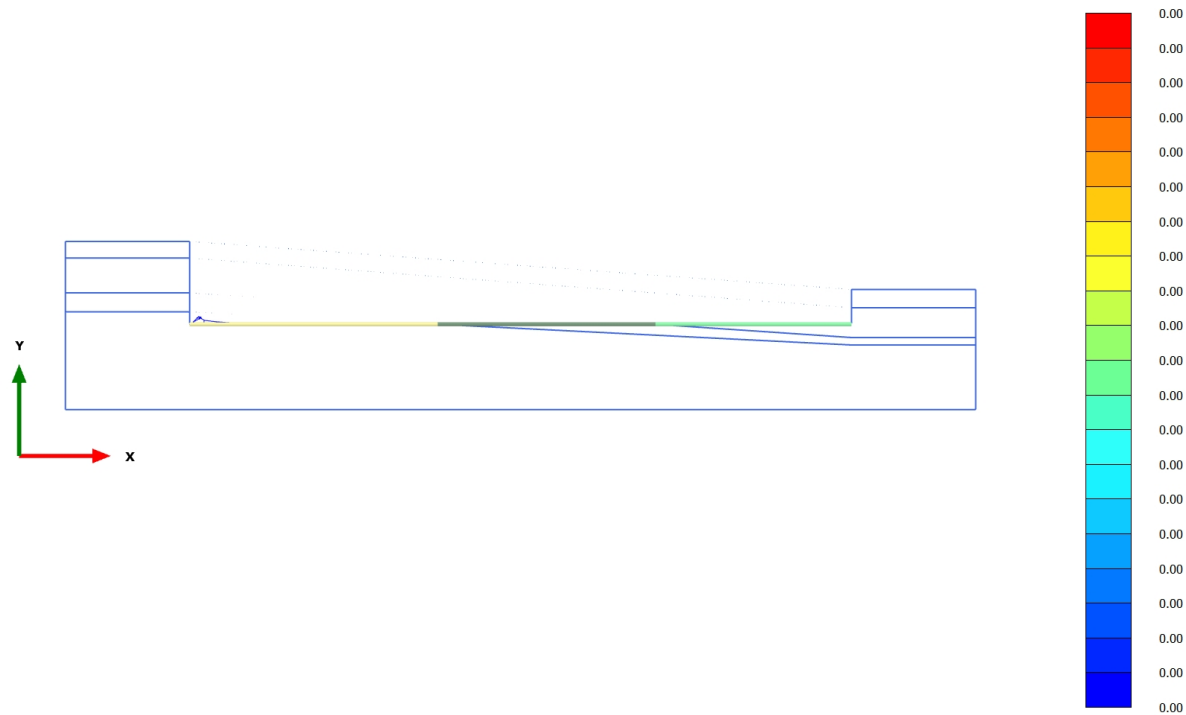
**Groundwater flow |q| (scaled up  $200 \cdot 10^6$  times) (Time 0.000 s)**

Maximum value =  $0.01816 \cdot 10^{-6}$  m/s

Minimum value = 0.000 m/s

Total discharge is  $0.02328 \cdot 10^{-6}$  m<sup>3</sup>/s/m

### 4.1.1 Calculation results, cross section A-A\*, Excavation [Phase\_1] (1/1), Groundwater flow |q|



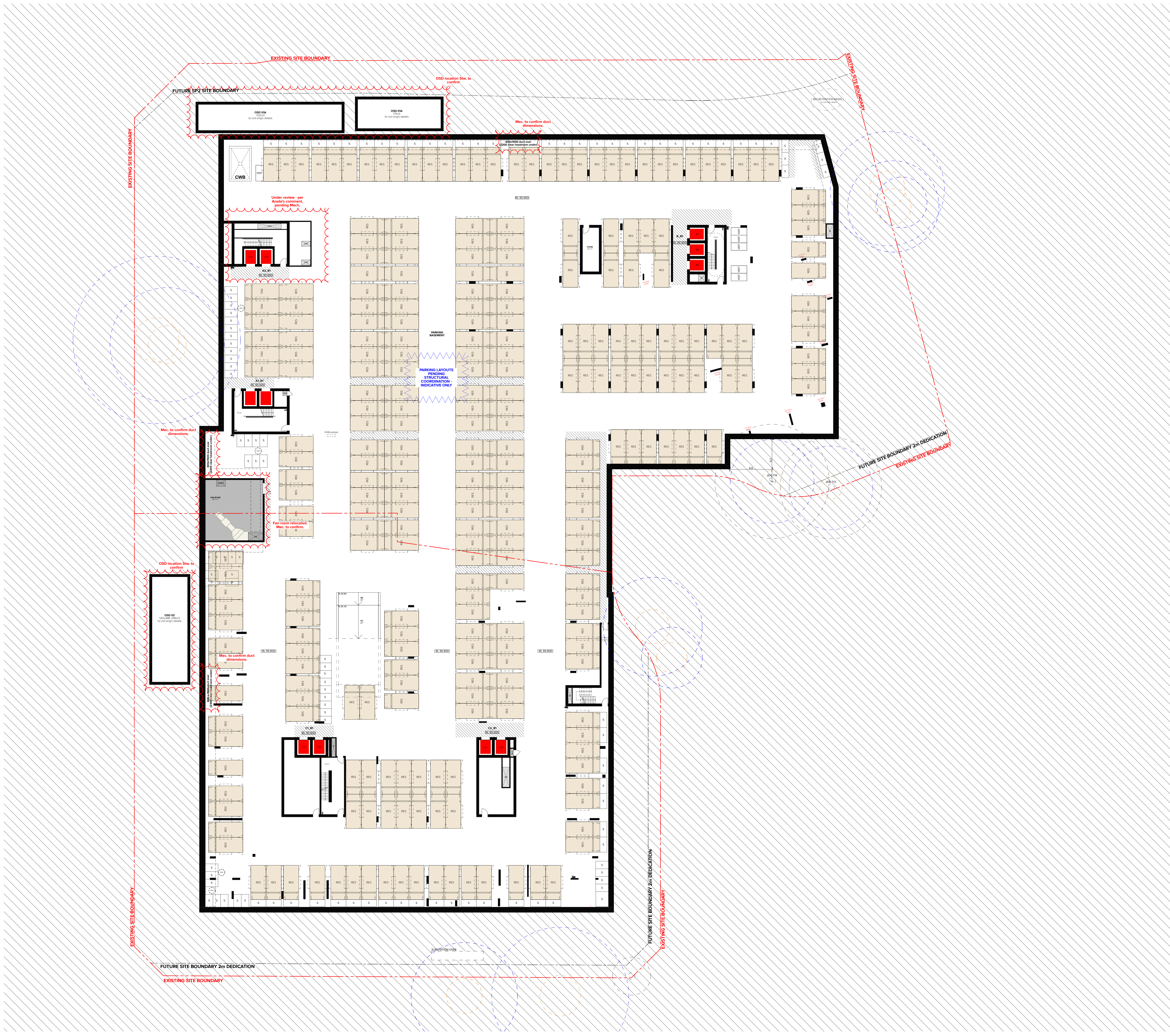
**Groundwater flow |q| (scaled up  $1.00 \times 10^9$  times) (Time 0.000 s)**  
 Maximum value =  $1.710 \times 10^{-9}$  m/s  
 Minimum value =  $2.046 \times 10^{-15}$  m/s  
 Total discharge is  $3.497 \times 10^{-9}$  m<sup>3</sup>/s/m



# Appendix E

# Architectural Drawings

---



**NOTES**

1. All drawings shall be made in accordance with the Australian Standards AS/NZS 1546:2011 and AS/NZS 1546:2012.
2. All drawings shall be made in accordance with the Australian Standards AS/NZS 1546:2011 and AS/NZS 1546:2012.
3. All drawings shall be made in accordance with the Australian Standards AS/NZS 1546:2011 and AS/NZS 1546:2012.

**LEGEND**

AC	Air Conditioning
ACU	Air Conditioning Condenser Unit
ACC	Accessible
ADP	Adaptable
AHD	Australian Height Datum
B	Bedroom 1, Bedroom 2, etc.
BL	Boiler
BKS	Bicycle Rack
BKS	Bicycle Storage
BL	Boiler
BOH	Back of House
BY	Balcony
CLMR	Cleaner Store
COM	Commercial
COMS	Communications Services
CP	Carparking Space
CPD	Cupboard
CPE	Car Park Exhaust
CWB	Car Wash Bay
CWS	Cold Water Supply
CY	Courtyard
D	Dining
E	Entry
E&C	Electrical & Communications
ELEC	Electrical Services
EN	Ensuite
EX	Existing
EXH	Exhaust
F	Fire Services
FCR	Fire Control Room
FEX	Fire Extinguisher
FFL	Finished Floor Level
FGL	Finished Ground Level
FH	Fire Hydrant
FIP	Fire Indicator Panel
FS_01	Fire Stair No.1, 2, etc.
GA_01	Grease Arrestor No.01, 02, etc.
OZ	Garbage Chute
GBE	Garbage Exhaust
GBR	Garbage Room
GHR	Garbage Holding Room
GL	Ground Line
H	Hydraulic Services
HL	High Level
HW	Hot Water
HWU	Hot Water Unit
I	Invert Level
INT	Integrated Assembly
K	Kitchen
KE	Kitchen Exhaust
LO_01	Lift No.1, 2, etc.
L	Living
LA	Landscape
LDE	Loading Dock Exhaust
LG	Lower Ground
LY	Laundry
M	Mechanical Services
MBP	Motor Bike Parking
MBX	Mail Box Assembly
MGB	Mobile Garbage Bin
MRV	Medium Rigid Vehicle
MSB	Main Switch Board Services incl. Main Distribution Board & Frame
MTR	Meter
MV	Mechanical Vent
NGL	Natural Ground Level
NRZ	Notional Root Zone
OSD	On Site Detention Tank
OSR	On Site Retention Tank
P	Pantry
PDR	Powder Room
PENO	Penetration
R	Roof
RES	Residential
RF	Refrigerator
RL	Relative Level to AHD
RTL	Retail
S	Storage
SA	Supply Air
SCN	Screen
SHR	Shower
SKL	Skylight
SP	Stair Pressurisation
SRV	Small Rigid Vehicle
SLL	Structural Slab Level
ST_01,02	Stair No.1, 2, etc.
ST	Styly
SWD	Stormwater Drain
SWP	Stormwater Pit
SRZ	Structural Root Zone
T	Tree
TCE	Terrace
TD	Timber Deck
TOW	Top of Wall
TRA	Tenant Return Air
TSA	Tenant Supply Air
TS	Traffic Signal
TYP	Typical
UG	Upper Ground
UNO	Unless Noted Otherwise
UT	Utility Space
VIS	Visitor
WC	WC
WC_A	WC - Accessible
WC_F	WC - Female
WC_M	WC - Male
WC_P	WC - Parents
WC_U	WC - Urisex
WR	Walk in Robe
WM	Washing Machine
WS	Wheel Stop
END.	

---	Setback Line
---	Site Boundary
---	Existing Apartment (Silver Levels)
---	Existing Apartment (Gold Levels)
---	Existing Apartment (Platinum Levels)
---	Accessible Parking Space
---	Atterforable Housing Unit
---	Existing Tree to be Retained
---	Existing Tree to be Demolished
---	Proposed Tree (Refer to Landscape Architect's Documentation)
---	1 Bed Apartment
---	2 Bed Apartment
---	2 Bed+ Apartment
---	3 Bed Apartment
---	3 Bed+ Apartment
---	4 Bed Apartment

Rev	Date	Approved By	Revision Notes
A	09/02/20	CM	For Coordination
B	17/02/20	CM	For Coordination
C	27/02/20	CM	For Coordination
D	01/04/20	CM	For Coordination
E	10/02/20	CM	For Coordination

**CLIENT**  
12 Henty Street Pyrmont Sydney NSW 2000

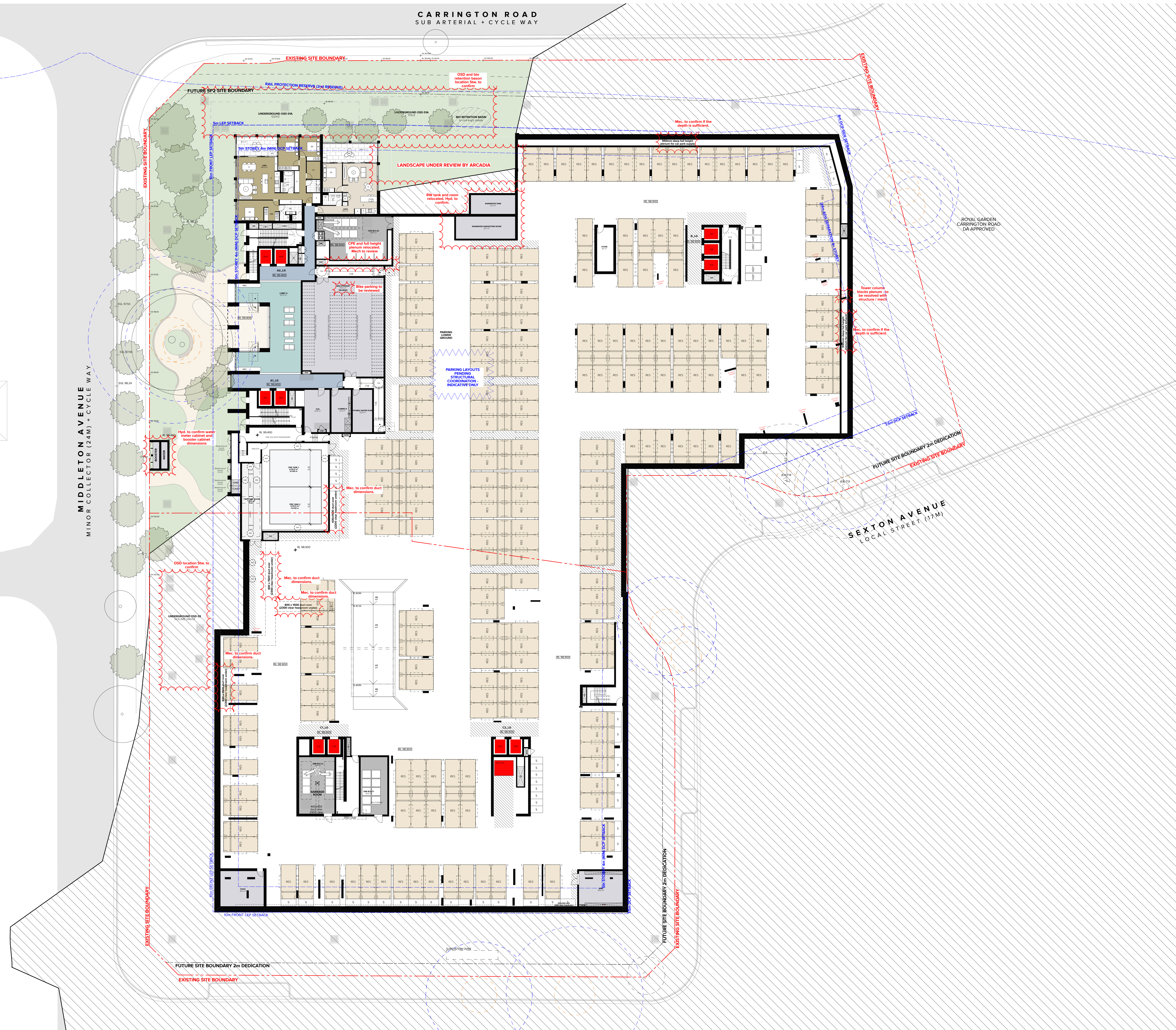
**ARADA**

**Project Title**  
Carrington Road  
2 Fishburn Crescent Castle Hill NSW 2154

**Drawing Title**  
GA Plans  
Basement 01

Scale	Project No.	Drawn By	North
1:200 @ A3	23104	CR, BE	
Status	Draw No.	Rev	
For Coordination	DA-110-006	E	

CARRINGTON ROAD  
SUB ARTERIAL + CYCLE WAY



**NOTES**

1. All drawings to be read in conjunction with the relevant contract documents and specifications.
2. All drawings to be read in conjunction with the relevant contract documents and specifications.
3. All drawings to be read in conjunction with the relevant contract documents and specifications.
4. All drawings to be read in conjunction with the relevant contract documents and specifications.

**LEGEND**

- AC Air Conditioning
- ACU Air Conditioning Condenser Unit
- ACC Accessible
- ADP Adaptable
- AHD Australian Height Datum
- B Bathroom
- BL2 Bedroom 1, Bedroom 2, etc.
- BGS Boom Gate System
- BKR Bicycle Rack
- BKS Bicycle Storage
- BL Balcony
- BOH Back of House
- BY Balcony
- CLMR Cleaner Store
- COM Commercial
- COMS Communications Services
- CP Carparking Space
- CPD Cupboard
- CPE Car Park Exhaust
- CWB Car Wash Bay
- CWS Cold Water Supply
- CY Courtyard
- D Dining
- E Entry
- E&C Electrical & Communications
- ELEC Electrical Services
- EN Ensuite
- EXO Existing
- EXH Exhaust
- F Fire Services
- FCR Fire Control Room
- FEX Fire Extinguisher
- FFL Finished Floor Level
- FGL Finished Ground Level
- FH Fire Hydrant
- FIP Fire Indicator Panel
- FS\_01 Fire Stair No.1, 2, etc.
- OZ Grease Arrestor No.01, 02, etc.
- GA\_01 Grease Arrestor No.01, 02, etc.
- GBC Garbage Chute
- GBE Garbage Exhaust
- GBR Garbage Room
- GHR Garbage Holding Room
- GL Ground Line
- H Hydraulic Services
- HL High Level
- HM Hot Water
- HWU Hot Water Unit
- I Invert Level
- INT Integrated Assembly
- K Kitchen
- KB Kiosk
- KE Kitchen Exhaust
- LO\_01 Lift No.1, 2, etc.
- OZ Living
- LA Landscape
- LDE Loading Dock Exhaust
- LG Lower Ground
- LV Laundry
- M Mechanical Services
- MBP Motor Bike Parking
- MBX Mail Box Assembly
- MGH Mobile Garbage Bin
- MRV Medium Rigid Vehicle
- MSB Main Switch Board Services incl. Main Distribution Board & Frame
- MTR Meter
- MV Mechanical Vent
- NGL Natural Ground Level
- NRZ Notional Root Zone
- OSD On Site Detention Tank
- OSR On Site Retention Tank
- P Pantry
- PDR Powder Room
- PEN0 Penetration
- R Robe
- RES Residential
- RF Refrigerator
- RL Relative Level to AHD
- RTL Retail
- S Storage
- SA Supply Air
- SCN Screen
- SHR Shower
- SKL Skylight
- SP Stair Pressurisation
- SRV Small Rigid Vehicle
- SSL Structural Slab Level
- ST\_01,02 Stair No.1, 2, etc.
- ST Study
- SWD Stormwater Drain
- SWP Stormwater Pit
- SRZ Structural Root Zone
- Tx Tree
- TCE Terrace
- TD Timber Deck
- TOW Top of Wall
- TRA Tenant Return Air
- TSA Tenant Supply Air
- TS Traffic Signal
- TYP Typical
- UG Upper Ground
- UNO Unless Noted Otherwise
- UT Utility Space
- VIS Visitor
- WC WC
- WC\_A WC - Accessible
- WC\_F WC - Female
- WC\_M WC - Male
- WC\_P WC - Parents
- WC\_U WC - Urisex
- WR Walk in Robe
- WM Washing Machine
- WS Wheel Stop
- END

**KEY PLAN**

- Setback Line
- - - Site Boundary
- ▤ Liable Apartment (Silver Level)
- ▥ Liable Apartment (Gold Level)
- ▧ Liable Apartment (Platinum Level)
- ▨ Accessible Parking Space
- ⊙ Affordable Housing Unit
- ⊕ Existing Tree to be Retained
- ⊖ Existing Tree to be Demolished
- ⊗ Proposed Tree (Refer to Landscape Architect's Documentation)
- 1 Bed Apartment
- 2 Bed Apartment
- 3 Bed Apartment
- 4 Bed Apartment

**REVISIONS**

Rev	Date	Approved By	Revision Notes
A	02/02/20	CM	For Coordination
B	17/02/20	CM	For Coordination
C	27/02/20	CM	For Coordination
D	01/03/20	CM	For Coordination
E	10/03/20	CM	For Coordination

**CURT**  
12 Hume Street Pyrmont Sydney NSW 2000 ARADA

**Project Title**  
Carrington Road  
2 Fishburn Crescent Castle Hill NSW 2154

**Drawing Title**  
GA Plans  
Lower Ground

**Scale**  
1:200 @ A3

**Project No.**  
23104

**Client**  
CR, BE

**Drawn By**  
E

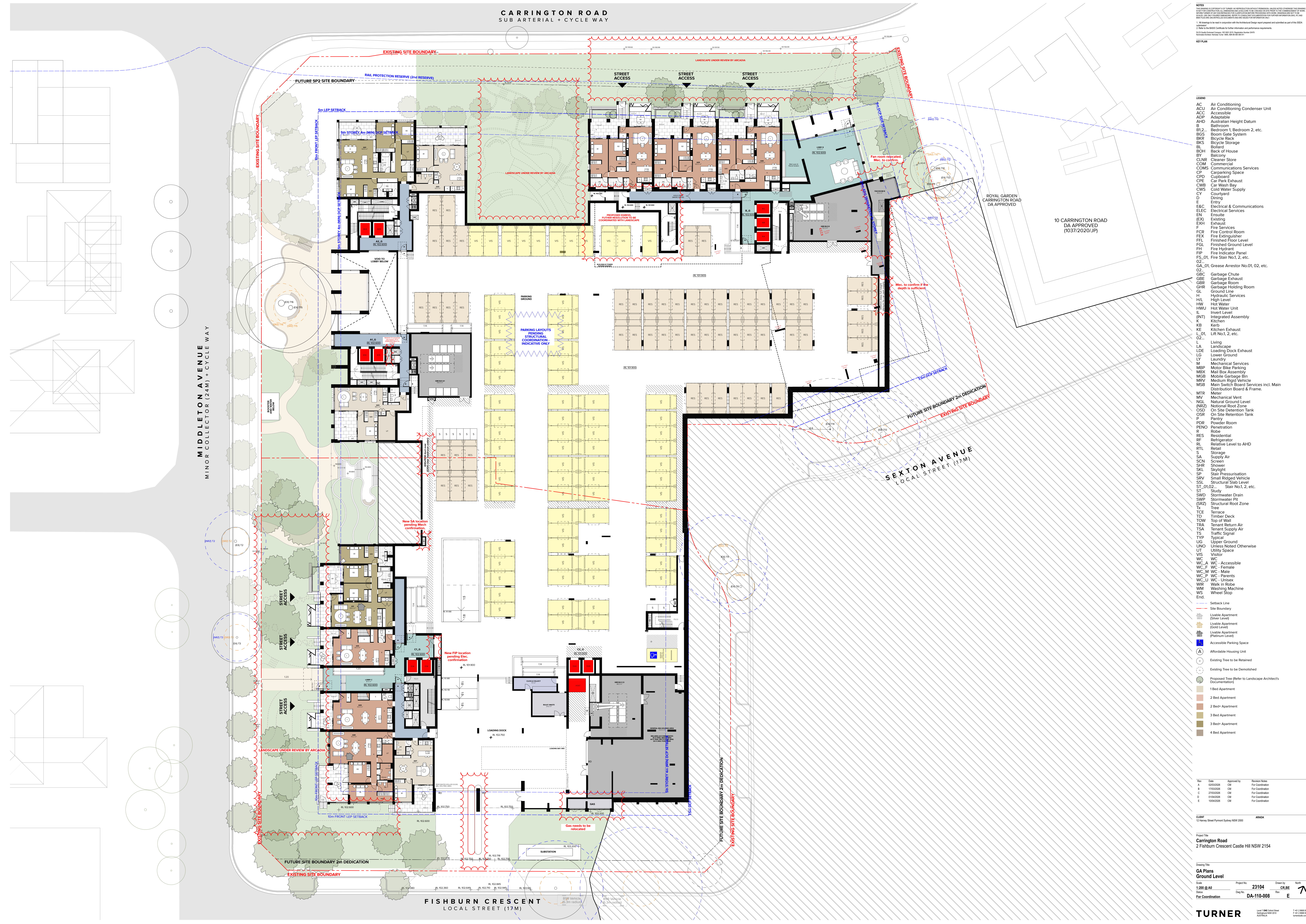
**Checked By**  
E

**Date**  
23/10/20

**For Coordination**  
DA-110-007

**TURNER**

CARRINGTON ROAD  
SUB ARTERIAL + CYCLE WAY



**NOTES**

1. All dimensions are in millimetres unless otherwise stated.
2. All dimensions are to be confirmed with the relevant authority.
3. All dimensions are to be confirmed with the relevant authority.
4. All dimensions are to be confirmed with the relevant authority.

**KEY PLAN**

**LEGEND**

- AC Air Conditioning
- ACU Air Conditioning Condenser Unit
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- ADR Adaptable
- AHD Australian Height Datum
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- BL2 Bedroom 1, Bedroom 2, etc.
- BGS Boom Gate System
- BKR Bicycle Rack
- BKS Bicycle Storage
- BL Balcony
- BOH Back of House
- BY Balcony
- CLMR Cleaner Store
- COM Commercial
- COMS Communications Services
- CP Carparking Space
- CPD Cupboard
- CPE Car Park Exhaust
- CWB Car Wash Bay
- CWS Cold Water Supply
- CY Courtyard
- D Dining
- E Entry
- E&C Electrical & Communications
- ELEC Electrical Services
- EN Ensuite
- EX Existing
- EXH Exhaust
- F Fire Services
- FCR Fire Control Room
- FEX Fire Extinguisher
- FFL Finished Floor Level
- FGL Finished Ground Level
- FH Fire Hydrant
- FIP Fire Indicator Panel
- FS\_01 Fire Stair No.1, 2, etc.
- GA\_01 Grease Arrestor No.01, 02, etc.
- OZ
- GBC Garbage Chute
- GBE Garbage Exhaust
- GRR Garbage Room
- GHR Garbage Holding Room
- GL Ground Line
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- HL High Level
- HW Hot Water
- HWU Hot Water Unit
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- INT Integrated Assembly
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- KB Kitchen
- KE Kitchen Exhaust
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- OZ
- L Living
- LA Landscape
- LDE Loading Dock Exhaust
- LG Lower Ground
- LY Laundry
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- MBP Motor Bike Parking
- MBX Mail Box Assembly
- MGB Medium Garbage Bin
- MRV Medium Rigid Vehicle
- MSB Main Switch Board Services incl. Main Distribution Board & Frame
- MTR Meter
- MY Mechanical Vent
- NGL Natural Ground Level
- NRZ Notional Root Zone
- OSD On Site Detention Tank
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- PEN Penetration
- R Robe
- RES Residential
- RF Refrigerator
- RL Relative Level to AHD
- RTL Retail
- S Storage
- SA Supply Air
- SCN Screen
- SHR Shower
- SKL Skylight
- SP Stair Pressurisation
- SRV Small Rigid Vehicle
- SSL Structural Slab Level
- ST\_01,02 Stair No.1, 2, etc.
- ST Study
- SWD Stormwater Drain
- SWP Stormwater Pit
- SRZ Structural Root Zone
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- TX Tree
- TD Timber Deck
- TOW Top of Wall
- TRA Tenant Return Air
- TSA Tenant Supply Air
- TS Traffic Signal
- TYP Typical
- UG Upper Ground
- UNO Unless Noted Otherwise
- UT Utility Space
- VIS Visitor
- WC WC
- WC\_A WC - Accessible
- WC\_F WC - Female
- WC\_M WC - Male
- WC\_P WC - Parents
- WC\_U WC - Urisex
- WR Walk in Robe
- WM Washing Machine
- WS Wheel Stop
- END

**REVISIONS**

Rev	Date	Approved By	Revision Notes
A	02/02/2025	CM	For Coordination
B	17/02/2025	CM	For Coordination
C	27/02/2025	CM	For Coordination
D	01/04/2025	CM	For Coordination
E	10/04/2025	CM	For Coordination

**CURT**  
17 Henry Street Pyrmont Sydney NSW 2000 ARADA

**Project Title**  
Carrington Road  
2 Fishburn Crescent Castle Hill NSW 2154

**Drawing Title**  
GA Plans  
Ground Level

**Scale**  
1:200 @ A3

**Project No.**  
23104

**Drawn By**  
CR,EE

**Checked By**  
E

**For Coordination**  
DA-110-008

**TURNER**

CARRINGTON ROAD  
SUB ARTERIAL + CYCLE WAY



NOTES  
1. All drawings shall be prepared in accordance with the Australian Standards AS/NZS 1546:2011 and AS/NZS 1547:2011.  
2. All drawings shall be prepared in accordance with the Australian Standards AS/NZS 1546:2011 and AS/NZS 1547:2011.  
3. All drawings shall be prepared in accordance with the Australian Standards AS/NZS 1546:2011 and AS/NZS 1547:2011.  
4. All drawings shall be prepared in accordance with the Australian Standards AS/NZS 1546:2011 and AS/NZS 1547:2011.

KEY PLAN

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BGS	Boom Gate System
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GBC	Garbage Chute
GBE	Garbage Exhaust
GRR	Garbage Room
GHR	Garbage Holding Room
GL	Ground Line
H	Hydraulic Services
HL	High Level
HM	Hot Water
HWU	Hot Water Unit
I	Invent Level
INT	Integrated Assembly
K	Kitchen
KB	Kerb
KE	Kitchen Exhaust
LO_01	Lift No.1, 2, etc.
OZ	Office
L	Living
LA	Landscape
LDE	Loading Dock Exhaust
LG	Lower Ground
LY	Laundry
M	Mechanical Services
MBP	Motor Bike Parking
MBX	Mail Box Assembly
MGB	Mobile Garbage Bin
MRV	Medium Rigged Vehicle
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OSD	On Site Detention Tank
OSR	On Site Retention Tank
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PDR	Powder Room
PENO	Penetration
R	Roof
RES	Residential
RF	Refrigerator
RL	Relative Level to AHD
RTL	Retail
S	Storage
SA	Supply Air
SCN	Screen
SHR	Shower
SKL	Skylight
SP	Stair Pressurisation
SRV	Small Rigged Vehicle
SSL	Structural Slab Level
ST_01,02	Stair No.1, 2, etc.
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SWD	Stormwater Drain
SWP	Stormwater Pit
SRZ	Structural Root Zone
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TE	Terrace
TD	Timber Deck
TOW	Top of Wall
TRA	Tenant Return Air
TSA	Tenant Supply Air
TS	Traffic Signal
TYP	Typical
UG	Upper Ground
UNO	Unless Noted Otherwise
UT	Utility Space
VIS	Visitor
WC	WC
WC_A	WC - Accessible
WC_F	WC - Female
WC_M	WC - Male
WC_P	WC - Parents
WC_U	WC - Urisex
WR	Walk in Robe
WM	Washing Machine
WS	Wheel Stop
END	End

LEGEND

---	Setback Line
---	Site Boundary
---	Existing Apartment (Ground Level)
---	Existing Apartment (First Level)
---	Existing Apartment (Second Level)
---	Existing Apartment (Platinum Level)
---	Accessible Parking Space
---	Attractive Housing Unit
---	Existing Tree to be Retained
---	Existing Tree to be Demolished
---	Proposed Tree (Refer to Landscape Architect's Documentation)
---	1 Bed Apartment
---	2 Bed Apartment
---	2 Bed+ Apartment
---	3 Bed Apartment
---	3 Bed+ Apartment
---	4 Bed Apartment

CLIENT  
University Street Payment Spring NSW 2000 ARADA

Project Title  
Carrington Road  
2 Fishburn Crescent Castle Hill NSW 2154

Drawing Title  
GA Plans  
Upper Ground Level

Scale	1:200 @ A3	Project No.	23104	Drawn By	CR, BE
Status	For Coordination	Drawn No.	DA-110-009	Check By	E









# Appendix F

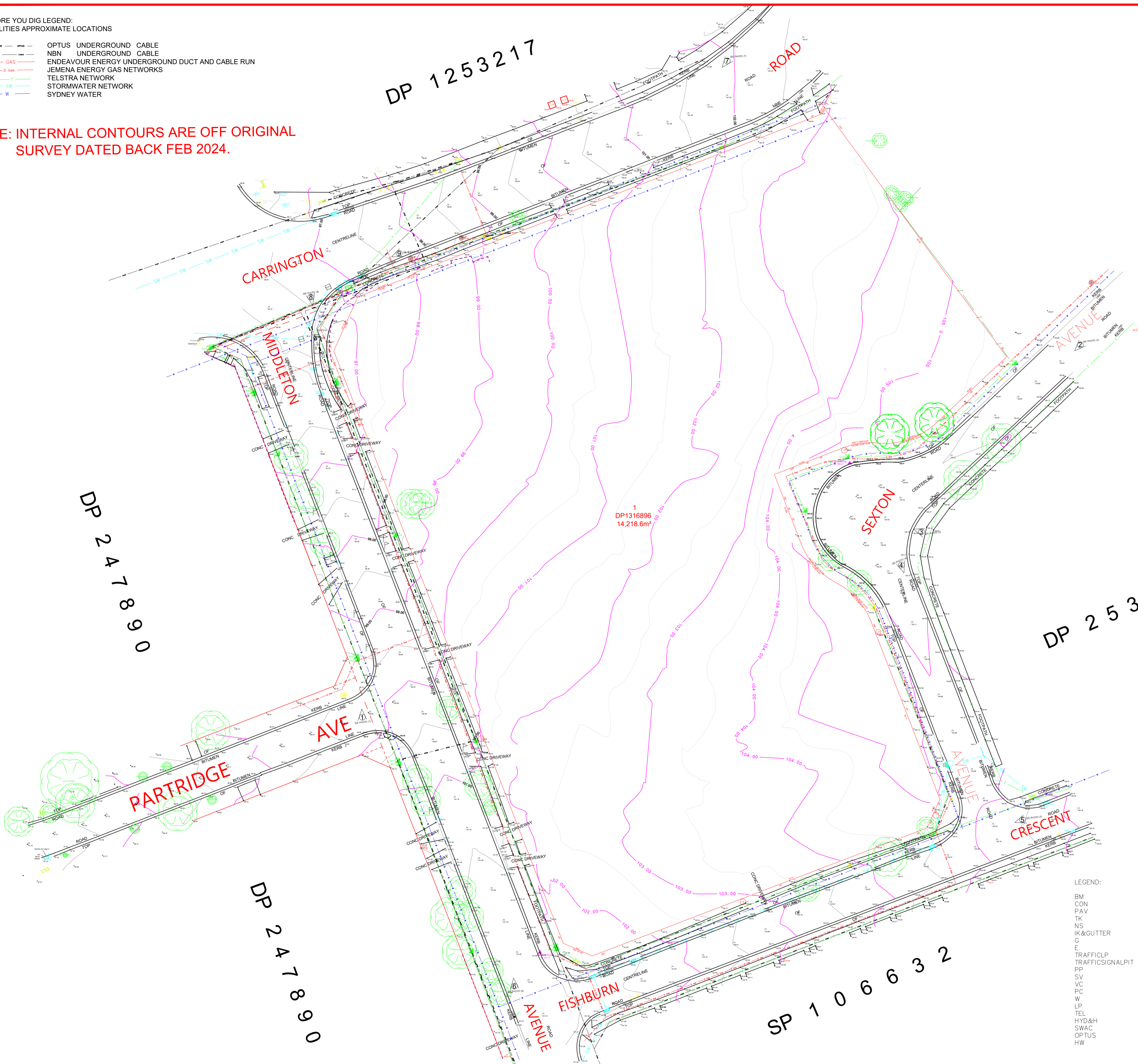
## Survey Plan

---

DIAL BEFORE YOU DIG LEGEND:  
Note: FACILITIES APPROXIMATE LOCATIONS

- OPTUS UNDERGROUND CABLE
- NBN UNDERGROUND CABLE
- GAS ENDEAVOUR ENERGY UNDERGROUND DUCT AND CABLE RUN
- JEMENA ENERGY GAS NETWORKS
- TELSTRA NETWORK
- STORMWATER NETWORK
- SYDNEY WATER

NOTE: INTERNAL CONTOURS ARE OFF ORIGINAL SURVEY DATED BACK FEB 2024.



T.N. (Approx)  
M.G.A.  
1:100



THE TITLE BOUNDARIES AS SHOWN HEREON WERE NOT MARKED AT THE TIME OF SURVEY AND HAVE BEEN DETERMINED BY PLAN DIMENSIONS ONLY AND NOT BY FIELD SURVEY.

SOME SERVICES SHOWN HEREON HAVE BEEN LOCATED BY FIELD SURVEY.

NOTE: LOCATIONS OF DOMESTIC SERVICE CROSSINGS ARE UNKNOWN.

- \* PRIOR TO EXCAVATION THE CONTRACTOR MUST:
- \* NOTIFY A.G.L. ON 0419527919
- \* OBTAIN TELSTRA'S "DUTY OF CARE" DOCUMENT REGARDING WORKING IN THE VICINITY OF TELSTRA PLANT.
- \* VERIFY CO-AXIAL/OPTIC FIBRE CABLE LOCATION

PRIOR TO ANY DEMOLITION, EXCAVATION OR CONSTRUCTION ON THE SITE, THE RELEVANT AUTHORITY SHOULD BE CONTACTED FOR LOCATION OF FURTHER UNDERGROUND SERVICES AND DETAILED LOCATIONS OF ALL SERVICES.

**EAST COAST POSITIONING**

**SURVEYORS**

**NORTHWEST**

P.O. BOX 7388, BAULKHAM HILLS  
N.S.W. 2153, Australia  
Mob: 0408556914 or 0458957000  
Email: info@ecpsurveyors.com.au

**CLIENT/PROJECT**

**ARADA**

16-20 CARRINGTON Rd /  
2-12 MIDDLETON AVE /  
4-6 FISHBURN CRES /  
25-31 SEXTON AVE.  
CASTLE HILL

SURVEYOR: MS, XG, ET, JJ

CHECK: XG

AUTHORISED: MS

**DETAIL SURVEY**

AUTHORITY	THE HILLS SHIRE
PROPERTY	2/DP1257535, 20-32/DP247890, 18/2/DP253774
LOCALITY	CASTLE HILL
ORIGIN OF LEVELS	SS62665
DATUM	A.H.D.
DATE	28/02/2024, 01.04, 06/03/2024
DRAWN BY	XG
SHEET	1 OF 1
DRAWING No.	ECP2896.D.01D
AMENDMENT	D

1:400 @ A1 SCALE 1:800 @ A3

- LEGEND:
- BM BENCHMARK
  - CON CONCRETE SURFACE
  - PAV PAVING SURFACE
  - TK TOP OF KERB
  - NS NATURAL SURFACE
  - IK&GUTTER INVERT OF KERB
  - G GAS
  - E GREEN ELEC-BOX
  - TRAFICLIP TRAFFIC LIGHT POLE
  - TRAFICSPIT TRAFFIC LIGHT SIGNAL CONTROL PIT
  - PP POWER POLE
  - SV STOP VALVE
  - VC VECHICULAR CROSSING
  - PC PEDESTRIAN CROSSING
  - W WATER
  - LP LIGHT POLE
  - TEL TELSTRA PIT
  - HYD&H HYDRANT
  - SWAC STORMWATER ACCESS CHAMBER
  - OPTUS OPTUS TELECOM PIT
  - HW SYDNEY WATER CONNECTION

No.	AMENDMENTS	DRAWN	DATE:
D	FIANLISE PLAN	MS	16/03/2026
B	UPDATED INTERVALS	JJ	06/03/2026
C	PARTRIDGE AVE SURV.	JJ	12/03/2026