

COPYRIGHT

DIMENSIONS

USE FIGURED DIMENSIONS, DO NOT SCALE. CONTRACTORS MUST VERIFY ALL DIMENSIONS ON THE SITE BEFORE COMMENCING ANY WORK OR MAKING ANY SHOP DRAWING WHICH MUST BE SUBMITTED AND REVIEWED BEFORE MANUFACTURE.

FIXTURES, FITTINGS & EQUIPMENT SPECIFICATIONS

SERVICE POINTS DISCLAIMER

THE SERVICE PORTS DON'TED ON THESE DRAWNING HAVE BEEN OFFER WITH PROVIDE SERVICES FROMEERING MODEL, AND ARE PROVIDED FOR NO FOR NATION AND SET-OUT PROPOSES ON WE WAVE BEEN ANABORD NY THE SERVICES ENONEMED FOR NO FOR NATION AND SET-OUT PROPOSES ON WE WAVE BEEN ANABORD NY THE SERVICES ENONEMED FOR THE THE OF PREJICATION. THIS OFFICE DOES NOT WARRANT THE DRAWNINGS ARE CORRECT AT THE BUILDING SERVICES HOMERE MOD COLUMNING AND FOR THE BUILDING SERVICES HOMERE MOD COLUMNING FOR THE SERVICES FOR THE BUILDING SERVICES HOMERE MOD COLUMNING FOR THE SERVICES FOR THE SERVICES FOR THE WAVE FOR THE SERVICES FOR THE SERVICES



BATESSMART.

ABN: 21 134 476 065 LEVEL 4, 89 YORK STREET SYDNEY NSW 2000, AUSTRALIA PH: (02) 8299 4600 FAX: (03) 9885 2455

SURRY HILLS, NSW 2010
ABN: 68 094 740 986 PH: (02) 8354 5100
E: syd@balessmart.com.au THIS ARCHITECTURAL DESIGN IS THE COPYRIGHT OF SELVER THOMAS HANLEY AND BATES SMART AND SHALL NOT BE REPRODUCED WITHOUT THER WRITTEN PERMISSION





Health NSW Infrastructure | Northern 1907 | Local Health District

TWEED VALLEY HOSPITAL

771 Cudgen Road, Cudgen

STAGE 1 EARLY AND ENABLING WORKS CONSTRUCTION **GENERAL ARRANGEMENT**



AR-SKE-10-110

AB

LANDSCAPE PROPOSAL

TREE REMOVAL AND PRESERVATION PLAN

Hospital planning has focused development high on the site and with an appropriate offset from the northern bush covered wetland to ensure it is undisturbed.

Existing windbreak planting to Cudgen Rd is proposed for retention where possible to mitigate visual impacts of the development and spray drift from adjacent farmland. Removal of vegetation is required in some locations to accommodate road widening, new entry roadways, and easements. Vegetated buffer zones along Cudgen Rd will be designed to supplement the existing buffer planting.

Existing trees will be retained wherever possible, and integrated as landscape features of the development.

LEGEND

Existing trees to be retained in accordance with BMP (Biodiversity Management Plan).

(TPZ in accordance with arborist report and AS 4970-2009)



Existing trees to be removed (due to development footprint and road works)

High retention value tree - to be retained
(TPZ in accordance with arborist report and AS 4970-2009)

Moderate retention value tree - to be retained if possible, pending detail civil roadworks design

Moderate retention value tree - to be removed



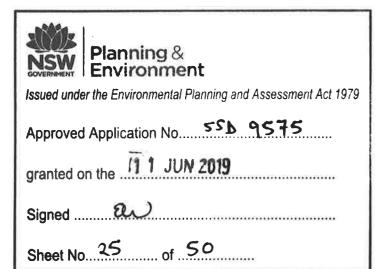
| DWG, L-EIS-1 | Rev F - 03.05.2019





TWEED VALLEY HOSPITAL DEVELOPMENT STAGE 1 - EARLY AND ENABLING WORKS

DRAWING LIST DELETED FOR SSDA
RESUBMISSION - NOT APPLICABLE



GENERAL NOTES

- G1 THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ARCHITECTURAL AND OTHER CONSULTANTS DRAWINGS AND SPECIFICATIONS AND WITH SUCH OTHER WRITTEN INSTRUCTIONS OR SKETCHES AS MAY BE ISSUED DURING THE COURSE OF THE CONTRACT, ANY DISCREPANCY SHALL BE REFERRED TO THE SUPERINTENDENT BEFORE PROCEEDING WITH WORK.
- G2 MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE SPECIFICATION, CURRENT SAA CODES, BUILDING REGULATIONS AND THE REQUIREMENTS OF ANY OTHER RELEVANT STATUTORY AUTHORITIES.
- G3 THESE DRAWINGS MUST NOT BE SCALED, ALL DIMENSIONS ARE IN METERS, ALL SET DUT DIMENSIONS AND LEVELS, INCLUDING THOSE SHOWN ON THESE DRAWINGS SHALL BE IN ACCORDANCE WITH THE ARCHITECT'S DRAWINGS AND VERIFIED ON SITE.
- G4 ALL SETOUT AND DIMENSIONS OF THE STRUCTURE INCLUDING KERBS AND RETAINING WALLS, AND BULK EARTHWORKS MUST BE TAKEN FROM THE ARCHITECT'S DRAWINGS. SETOUT OF THE STORMWATER PITS BY OTHERS, CONTRACTOR TO CONFIRM SETOUT OF SERVICE TRENCHING INCLUDING SUBSOIL ON SITE.
- GS THE CONTRACTOR SHALL COMPLY WITH ALL REGULATIONS OF AUTHORITIES HAVING JURISDICTON OVER THE WORKS, REFER TO GEOTECHNICAL REPORT BY MORRISON GEOTECHNIC PTY LTD, REFERENCE: GE18/144, DATED AUGUST 2018.
- G6 ALL DIMENSIONS AND REDUCED LEVELS MUST BE VERIFIED ON SITE BEFORE THE COMMENCEMENT OF ANY WORK
- G7 THE APPROVAL OF A SUBSTITUTION SHALL BE SOUGHT FROM THE SUPERINTENDENT BUT IS NOT AN AUTHORISATION OF A COST VARIATION. THE SUPERINTENDENT MUST APPROVE ANY COST VARIATION INVOLVED BEFORE ANY WORK STARTS.
- G8 ALL LEVELS SHOWN ARE TO THE AUSTRALIAN HEIGHT DATUM
- G9 SERVICE INFORMATION SHOWN IS APPROXIMATE ONLY, PRIOR TO COMMENCEMENT OF ANY WORKS, THE CONTRACTOR SHALL LOCATE ALL UNDERGROUND SERVICES AND COMPLY WITH ALL REQUIREMENTS OF THOSE AUTHORITIES.
- G10 EXISTING SURFACE CONTOURS, WHERE SHOWN, ARE INTERPOLATED AND MAY NOT BE ACCURATE.
- G11 UNLESS NOTED OTHERWISE, ALL VEGETATION SHALL BE STRIPPED TO A MINIMUM DEPTH OF 150mm UNDER ALL PROPOSED PAVEMENT AND BUILDING AREAS.
- G12 MAKE SMOOTH CONNECTION WITH ALL EXISTING WORKS

SITEWORKS NOTES

- S1 PRIOR TO THE PLACEMENT OF ANY PAVEMENTS, BUILDINGS OR DRAINS THE EXPOSED SUBGRADE SHALL BE COMPACTED TO A MINIMUM OF 98% STAMDARD COMPACTION IN ACCORDANCE WITH TEST "EL" OF A 5. 1289 FOR THE TOP 300m, ANY SOFT SPOTS SHALL BE REMOVED AND REPLACED WITH GRANULAR FILL TO THE ENGINEERS APPROVAL AND COMPACTED IN ACCORDANCE WITH THE LOMPACTION REQUIREMENTS SET OUT BELOW, ON HIGHLY REACTIVE CLAY AREAS SITE EXCAVATED MATERIAL MAY BE USED WITH THE PRIOR AUTHORISATION OF THE ENGINEER.
- 22 ALL FILL AND PAVEMENT MATERIALS SHALL BE COMPACTED IN ACCORDANCE WITH GEOTECHNICAL REPORT BY MORRISON GEOTECHNIC PTY LTD REFERENCE: GE18/144 DATED AUGUST 2019 MOISTURE CONTENT TO BE MAINTAINED AT +/- 2% OMC. MINIMUM COMPACTION REQUIREMENTS ARE DETAILED BELOW FOR TALL REQUIREMENTS ARE TO VERIFIED BY A SUITABLY DUALIFIED GEOTECHNICAL EMGINEET).

LANDSCAPED AREAS
 95% STD.

FILL UNDER ANY FOOTINGS AND FLOOR SLABS FOR ANY STRUCTURE TO SUBGRADE LEVEL

- FINE CRUSHED ROCK
- SELECTED FILL WITHOUT CONSPICUOUS CLAY CONTENT

• BUILDING BASECOURSE

• FILL UNDER ROAD PAVEMENTS;
- TO WITHIN SODOMO OF FINISHED SUBGRADE LEVEL
- UP TO FINISHED SUBGRADE LEVEL
98% STD.

ROAD PAVEMENT MATERIALS;
- SUB BASE
- BASE COURSE

THE MAXIMUM COMPACTION IS TO BE NO GREAT THAN 4% ON TOP OF THE ABOVE MENTION VALUES.

- S3 GRADE EVENLY BETWEEN FINISHED SURFACE SPOT LEVELS. FINISHED SURFACE CONTOURS ARE SHOWN FOR CLARITY, WHERE FINISHED SURFACE LEVELS ARE NOT SHOWN, THE SURFACE SHALL BE GRADED SMOOTHLY SO THAT IT WILL DRAIN AND MACTH ADJACENT SURFACES OR STRUCTURES
- S4 ALL DIMENSIONS GIVEN ARE TO FACE OF KERB, CENTER OF PIPE OR EXTERIOR FACE OF BUILDING UNLESS NOTED OTHERWISE, $\,$
- ANY STRUCTURES, PAVEMENTS OR SURFACES DAMAGED, DIRTIED OR MADE UNSERVICABLE DUE TO CONSTRUCTION WORK SHALL BE REINSTATED TO THE SATISFACTION OF THE ENGINEER
- S6 ANY FILL REQUIRED SHALL BE APPROVED BY THE ENGINEER / GEOTECHNICAL CONSULTANT
- 7 CONTRACTOR IS TO ENSURE THAT ALL EXCAVATIONS ARE MAINTAINED IN A DRY CONDITION WITH NO WATER ALLOWED TO REMAIN IN THE EXCAVATIONS.
- S8 ALL FINISHES AND COLOURS TO BE IN ACCORDANCE WITH ARCHITECTURAL SPECIFICATIONS...
- S9 REFER TO STRUCTURAL DRAWINGS FOR CONCRETE, REINFORCEMENT AND RETAINING WALL DETAILS
- S10 GENERALLY FOR TRENCHING WORKS THE CONTRACTOR MUST:
 A) COMPLY WITH THE GENERAL PROVISIONS OF PART 3.1 "MANAGING RISKS TO HEALTH AND SAFETY" OF NSW WORK AND HEALTH AND SAFETY REGULATION 2011
- B) COMPLY PART 6.3 DIVISION 3 "EXCAVATION WORK" OF NSW WORK HEALTH AND SAFETY REGULATION NSW 2011
- S11 PRIOR TO THE EXCAVATION OF ANY TRENCH DEEPER THAN 1.5 METRES THE CONTRACTOR MUST:

 A) NOTIFY THE OCCUPATIONAL HEALTH AND SAFETY AUTHORITY ON THE APPROPRIATE FORM.

STORMWATER DRAINAGE NOTES

- SW1 UNLESS NOTED OTHERWISE BY HYDRAULIC ENGINEERS DRAWINGS, ALL DOWNPIPES & GRATEO INLETS SHALL BE CONNECTED TO PITS OR MAIN STORMWATER DRAINS WITH 150 DIA. UPVC PIPES LAID AT A MINIMUM GRADE OF 1 IN 100. FOR SYPHONIC ROOF DRAINAGE SYSTEMS ALL DOWNPIPES CONNECTION DRAIN SIZES TO BE CONNECTED INTO MAIN STORMWATER DRAINS SHALL BE IN ACCORDANCE WITH HYDRAULIC ENGINEERS DRAWINGS.
- SW2 ALL MAIN STORMWATER DRAINS SHALL BE CONSTRUCTED USING MATERIALS AS SPECIFIED ON THE DRAWINGS IN ACCORDANCE WITH THE APPROPRIATE A.S. IF NOT SPECIFIED THEN CLASS 2 RRJ RCP SHALL BE USED FOR DIAMETERS > 225mm, SEWER CLASS SEH UPVC IN ACCORDANCE WITH AS1260 SHALL BE USED FOR Ø225mm OR SMALLER.
- SW3 ALL PIPEWORK TO BE INSTALLED IN ACCORDANCE WITH AS3725 FOR RCP AND AS2032 FOR PVC ALL BEDDING TO BE TYPE H2 UNLESS NOTED OTHERWISE.
- SW4 FOR ALL PITS > 1.2m DEEP, STEP IRONS SHALL BE INSTALLED
- SW5 PRECAST PITS MAY BE USED EXTERNAL TO THE BUILDING SUBJECT TO APPROVAL BY BONACCI
- SW6 ENLARGERS, CONNECTIONS AND JUNCTIONS TO BE PREFABRICATED FITTINGS WHERE PIPES ARE LESS THAN 300 DIA.
- SW7 WHERE SUBSOIL DRAINS PASS UNDER FLOOR SLABS AND VEHICULAR PAVEMENTS, UNSLOTTED
- SW8 GRATES AND COVERS SHALL CONFORM WITH AS 3996 AND AS 1428,1 FOR ACCESS REQUIREMENTS,
- SW9 CARE IS TO BE TAKEN WITH LEVELS OF STORMWATER LINES, GRADES ARE NOT TO BE REDUCED WITHOUT APPROVAL.
- SW10 AT ALL TIMES DURING CONSTRUCTION OF STORMWATER PITS, ADEQUATE SAFETY PROCEDURES SHALL BE TAKEN TO ENSURE AGAINST THE POSSIBILITY OF PERSONNEL FALLING DOWN PITS.
- SW11 ALL EXISTING STORMWATER DRAINAGE LINES AND PITS THAT ARE TO REMAIN ARE TO BE INSPECTED AND CLEANED, DURING THIS PROCESS ANY PART OF THE STORMWATER ORAINAGE SYSTEM THAT WARRANTS REPAIR SHALL BE REPORTED TO THE SUPERINTENDENT/ENGINEER FOR ENDRED DIGGTORS.

KERBING NOTES

- K1 ALL CONCRETE TO HAVE A MINIMUM COMPRESSIVE STRENGTH OF 32 MPa U N O
- K2 ALL KERBS, GUTTERS, DISH DRAINS AND CROSSINGS TO BE CONSTRUCTED ON 75mm GRANULAR
 BASECOURSE COMPACTED TO A MINIMUM 98% MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS1289
 5.2.1
- K3 EXPANSION JOINTS (EJ) TO BE FORMED FROM 10mm COMPRESSIBLE CORK FILLER BOARD FOR THE FULL DEPTH OF THE SECTION AND CUT TO PROFILE EXPANSION JOINTS TO BE LOCATED AT DRAINAGE PITS, ON TANGENT POINTS OF CURVES AND ELSEWHERE AT MAX 12m CENTRES EXCEPT FOR INTEGRAL KERBS WHERE THE EXPANSION JOINTS ARE TO MATCH THE JOINT LOCATIONS IN THE SLAB.
- K4 WEAKENED PLANE JOINTS TO BE MIN 3mm WIDE AND LOCATED AT 3m CENTRES EXCEPT FOR INTEGRAL KERBS WHERE THE WEAKENED PLANE JOINTS ARE TO MATCH THE JOINT LOCATIONS IN THE SLAB
- K5 BROOMED FINISH TO ALL RAMPED AND VEHICULAR CROSSINGS. ALL OTHER KERBING OR DISH DRAINS TO BE STEEL FLOAT FINISHED.
- K6 IN THE REPLACEMENT OF KERBS:-
- IN THE REPLACEMENT OF REMOSE.

 EXISTING ROAD PAYEMENT IS TO BE SAWCUT 900mm U.N.O., FROM THE LIP OF GUTTER, UPON COMPLETION OF THE NEW KERB AND GUTTER, NEW BASECOURSE. AND SURFACE TO BE LAID 600mm WIDE U.N.O.
 - EXISTING KERBS ARE TO BE COMPLETELY REMOVED WHERE NEW KERBS ARE SHOWN

JOINTING NOTES

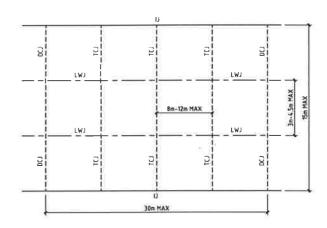
PEDESTRIAN FOOTPATH JOINTS

- JI EXPANSION JOINTS (E.J) ARE TO BE LOCATED WHERE POSSIBLE AT TANGENT POINTS OF CURVES AND ELSEWHERE AT 6m CENTRES.
- 32 SAWCUT JOINTS (SC) ARE TO BE LOCATED AT A MAX 15m x WIDTH OF PAVEMENT, THE TIMING OF THE SAWCUT IS TO BE CONFIRMED BY THE CONTRACTOR ON SITE, SITE CONDITIONS WILL DETERMINE HOW MANY HOURS AFTER THE CONCRETE POUR BEFORE THE SAW CUTS ARE COMMENCED.
- J3 WHERE POSSIBLE JOINTS SHOULD BE LOCATED TO MATCH KERBING AND / OR ADJACENT PAVEMENT
- J4 PROVIDE 10mm WIDE FULL DEPTH EXPANSION JOINTS (EJ) BETWEEN BUILDINGS AND ALL CONCRETE OR UNIT PAVERS
- J5 ALL PEDESTRIAN FOOTPATH JOINTINGS AS FOLLOWS (U.N.O.)



VEHICULAR PAVEMENT JOINTS

- J6 ALL VEHICULAR PAVEMENTS TO BE JOINTED AS SHOWN ON DRAWINGS.
- J7 LONGITUDINAL WARPING JOINTS (LWJ) SHOULD GENERALLY BE LOCATED AT A MAXIMUM OF 3m TO 4,5m MAX CENTERS, ALL LWJ'S SHOULD BE TIED UP TO A MAXIMUM TOTAL WIDTH OF 30m.
- JB TRANSVERSE CONTRACTION JOINTS (TCJ) SHOULD GENERALLY BE LOCATED AT A MAXIMUM OF 8m TO 12m MAX CENTERS. TCJ'S CAN BE SPACED AT SUITABLE INTERVALS UP TO A RECOMMENDED MAXIMUM I FINISTH OF 15m
- 9 TRANSVERSE DOWELLED CONSTRUCTION JOINTS (DC.)! TO BE PROVIDED FOR PLANNED INTERRUPTIONS SUCH AS AT THE END OF EACH DAY'S OPERATIONS (POUR BREAK), AT BLOCK OUTS FOR BRIDGES AND INTERSECTIONS OR FOR UNEXPECTED DELAYS WHEN THE SUSPENSION OF OPERATIONS IS LIKELY TO CREATE A JOINT.
- J10 ISOLATION JOINTS WITH SUB-GRADE BEAM (IJ) TO BE PROVIDED AT INTERSECTIONS OR AT THE
- J11 ALL VEHICULAR PAVEMENTS TO BE JOINTED IN ACCORDANCE WITH AUSTROADS AGPT02-12 GUIDE TO PAVEMENT TECHNOLOGY PART 2 STRUCTURAL PAVEMENT DESIGN AND SUPPLEMENT AP-T36-06 PAVEMENT DESIGN FOR LIGHT TRAFFIC
- J12 VEHICULAR PAVEMENT JOINTING AS FOLLOWS (U.N.



BONACCI

BONACCI GROUP Ply Ltd:
ABN 42 665 333 345
Consuling Engineen, Structural - CN2 - Nikastructur
Level 6, 37 Vin Street, Sydney, NSW 2000 Australia
Tel: 191 2 6247 8400 Fax: 191 2 6247 8444
sydleng (Stonacolgroup zone)

TWEED VALLEY HOSPITAL
DEVELOPMENT, CUDGEN
STAGE 1 FARLY WORKS

DRAWING REGISTER AND CONSTRUCTION NOTES

File: G:Job11074803 Work in Progressi02 CMI01 CADI01 Autocadi000 Series - Ex

Of the Bonnaco Croup

PS DA SSUE

SOLUTION FRANCY AND EMAILANG WORKS

16.10 10 0 =

P4 SSUED FOR EARLY MODERS DA

48.10 10 PH
P5 SSUED FOR EARLY WORKS DA

48.00 10 PH
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

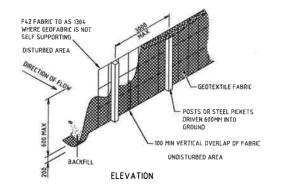
48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

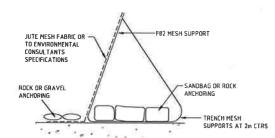
48.00 10 PA
P1 SSUED FOR EARLY WORKS DA

48.00 PA
P1 SSUED FOR EARLY WORKS DA

89.00 PA
P1 SSUED



SEDIMENT FENCE

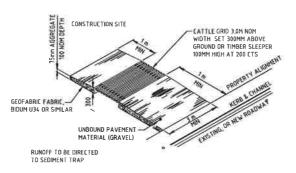


ALTERNATIVE SEDIMENT FENCE

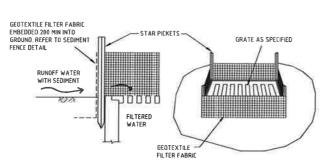
ALTERNATIVE SEDIMENT FENCE NOTES

- INSTALL THIS TYPE OF SEDIMENT FENCE WHEN USE OF SUPPORT POSTS IS NOT DESIRABLE OR NOT POSSIBLE. SUCH CONDITIONS MIGHT APPLY, FOR EXAMPLE, WHERE APPROVAL IS GRANTED FROM THE APPROPRIATE AUTHORITIES TO PLACE THESE FENCES IN HIGHLY SENSITIVE ESTUARINE AREAS.
 USE BENT TRENCH MESH TO SUPPORT THE F82 WELDED MESH FALING AS SHOWN ON THE DRAWING.
- ABOVE, ATTACH THE JUTE MESH TO THE WELDED MESH FACING USING UV-RESISTANT CABLE TIES.

 3. STABILISE THE WHOLE STRUCTURE WITH SANDBAG OR ROCK ANCHORING OVER THE TRENCH MESH AND THE LEADING EDGE OF THE JUTE MESH. THE ANCHORING SHOULD BE SUFFICIENTLY LARGE TO ENSURE STABILITY OF THE STRUCTURE IN THE DESIGN STORM EVENT, USUALLY THE 10 YEAR EVENT...

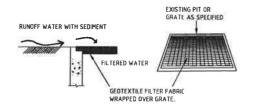


TEMPORARY CONSTRUCTION VEHICLE ENTRY/EXIT SEDIMENT TRAP

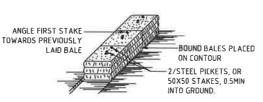


GEOTEXTILE PIT FILTER 1

NOT TO SCALE



GEOTEXTILE PIT FILTER 2



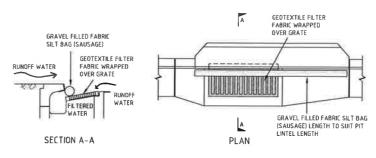


BEDDING DETAIL

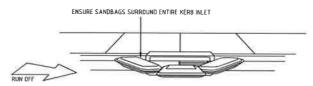
STRAW BALE BANK SEDIMENT CONTROL

NOT TO SCALE

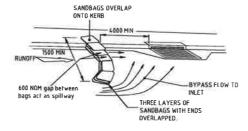
Dalin By App



KERB INLET SEDIMENT TRAP NOT TO SCALE



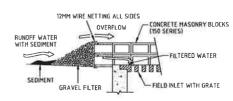
SANDBAG KERB INLET SEDIMENT TRAP



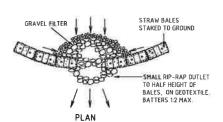
ON GRADE KERB INLET SEDIMENT TRAP

FALL -1:3 MINIMUM 1:6 DESIRABLE

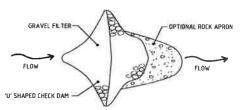


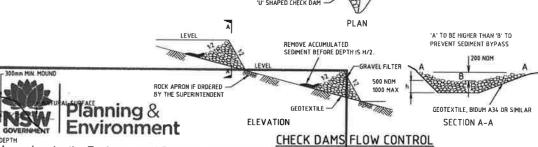


FIELD INLET SEDIMENT TRAP



STRAW BALE AND STONE TRAP SEDIMENT CONTROL (CONCENTRATE FLOW)





Issued under the Environmental Planning and Assessment Act 1979 CATCH DRA NOT TO SCALE

Approved Application No. 558 9575

granted on the 1 1 JUN 2019

Sheet No. 26 of 50

TWEED VALLEY HOSPITAL DEVELOPMENT, CUDGEN STAGE 1 EARLY WORKS

Drawing SOIL AND WATER MANAGEMENT DETAILS

DEVELOPMENT APPLICATION Date 03 09 18 A1 20 10748 01 C006 P2

SWMP Commentary, Detailed Calculations

Note: These "Detailed Calculation" spreadsheets relate only to high erosion hazard lands as identified in figure 4.6 or where the designer chooses to use the RUSLE to size sediment basins. The "Standard Calculation" spreadsheets should be used on low erosion hazard lands as identified by figure 4.6 and where the designer chooses not to run the RUSLE in calculations.

1. Site Data Sheet

Site Name: TWEED VALLEY HOSPITAL Site Location: TWEED VALLEY HOSPITAL CUDGEN ROAD KINGSCLIFF

Precinct: HEALTH

Description of Site:

-10000	ia .
NSW	Planning &
GOVERNMENT	Environment

27

Issued under the Environmental Planning and Assessment Act 1979

Approved Application No.

... of ... 50

Site area			Si	ite			Remark		1		
Site alea	1	2	3	4	5	6a	Keman	granted	on the	1 1 JUN	1 2019
Total catchment area (ha)	1.9	3.12	2.17	0.34	0.21	0.86			1		
Disturbed catchment area (ha)	1.9	3.12	2.17	0.34	0.21	0.86			- 7	.)	
			-					Signed			

Soll analysis

% sand (faction 0.02 to 2.00 mm	Soil texture should be assessed through
% silt (fraction 0.002 to 0.02 mm)	mechanical dispersion only. Dispersing
% clay (fraction finer than 0.002 mm)	agents (e.g. Calgon) should not be used
Dispersion percentage	E.g. enter 10 for dispersion of 10%
% of whole soil dispersible	See Section 6.3.3(e)
Soil Texture Group	See Section 6.3.3(c), (d) and (e)

Rainfall data

D. J	1 -						In a
Design rainfall depth (days)	5	5	5	5	5	5	See Sections 6.3.4 (d) and (e)
Design rainfall depth (percentile)	85	85	85	85	85	85	See Sections 6.3.4 (f) and (g)
x-day, y-percentile rainfall event	62.5	62.5	62.5	62.5	62.5	62.5	See Section 6.3.4 (h)
Rainfall intensity: 2-year, 6-hour storm	15.9	15.9	15.9	15.9	15.9	15.9	See IFD chart for the site

RUSLE Factors

Rainfall erosivity (R -factor)	5750	5750	5750	5750	5750	5750	Automatic calculation from above data		
Soil erodibility (K -factor)	0.015	0.015	0.015	0.015	0.015	0.015			
Slope length (m)	173	103	106	55	52	85	1		
Slope gradlent (%)	6.36	13.9	4.9	25	7.1	18.8	RUSLE data can be obtained from		
Length/gradient (LS -factor)	2.38	5.42	1.36	6.8	1.31	7.29	Appendixes A, B and C		
Erosion control practice (P -factor)	1.3	1.3	1.3	1.3	1.3	1.3			
Ground cover (C -factor)	1	1	1	1	1	1			

Calculations

Soil loss (t/ha/yr)	267	608	152	762	147	817	
Soil Loss Class	3	5	2	6	1	6	See Section 4.4.2(b)
Soil loss (m³/ha/yr)	205	467	117	587	113	629	
Sediment basin storage volume, m3	66	248	43	34	4	92	See Sections 6.3.4(i) and 6.3.5 (e)

Revised Catchment Calc 1.1 CAM 190110.xls

SWMP Commentary, Detailed Calculations

4. Volume of Sediment Basins, Type D and Type F Soils

Basin volume = settling zone volume + sediment storage zone volume

Settling Zone Volume

ettling zone volume for Type F and Type D soils is calculated to provide capacity to contain all runoff expected from up to the y-percentile rainfall event. The volume of the basin's settling zone (V) can be determined as a function of the basin's surface area and depth to allow for particles to settle and can be determined by the following equation:

$$V = 10 \times C_v \times A \times R_{x-day, y-\%ile} (m^3)$$

10 = a unit conversion factor

C_v = the volumetric runoff coefficient defined as that portion of rainfall that runs off as stormwater over the x-day period

= is the x-day total rainfall depth (mm) that is not exceeded in y percent of rainfall events. (See Sections 6.3.4(d), (e), (f), (g) and (h)).

A = total catchment area (ha)

Sediment Storage Zone Volume

In the detailed calculation on Soil Loss Classes 1 to 4 lands, the sediment storage zone can be taken as 50 percent of the settling zone capacity. Alternately designers can design the zone to store the 2-month soil loss as calculated by the RUSLE (Section 6.3.4(i)(ii)). However, on Soil Loss Classes 5, 6 and 7 lands, the zone must contain the 2-month soil loss as calculated by the RUSLE (Section 6.3.4(i)(iii).

Place an "X" in the box below to show the sediment storage zone design parameters used here:

50% of settling zone capacity, 2 months soil loss calculated by RUSLE

Total Basin Volume

Site	C _v	R _{x-day, y-%lie}	Total catchment area (ha)	Settling zone volume (m³)	Sediment storage volume (m³)	Total basin volume (m³)
1	0.70	62.5	1.9	831.25	66	897.25
2	0.70	62.5	3.12	1365	248	1613
3	0.70	62.5	2.17	949.375	43	992.375
4	0.70	62.5	0.34	148.75	34	182.75
5	0.70	62.5	0.21	91.875	4	95.875
6a	0.70	62.5	0.86	376.25	92	468.25

Revised Catchment Calc 1.1 CAM 190110.xls

TWEED VALLEY HOSPITAL DEVELOPMENT, CUDGEN STAGE 1 EARLY WORKS

Drawing SOIL AND WATER MANAGEMENT CALCULATIONS - SHEET 1 OF 2

DEVELOPMENT APPLICATION 1:1000 Date 03.09.18 ¹⁹ 20 10748 01 C007 P3

SWMP Commentary, Detailed Calculations

Note: These "Detailed Calculation" spreadsheets relate only to high erosion hazard lands as identified in figure 4.6 or where the designer chooses to use the RUSLE to size sediment basins. The "Standard Calculation" spreadsheets should be used on low erosion hazard lands as identified by figure 4,6 and where the designer chooses not to run the RUSLE in calculations.

1. Site Data Sheet

Site Name: TWEED VALLEY HOSPITAL

Site Location: TWEED VALLEY HOSPITAL CUDGEN ROAD KINGSCLIFF

Precinct: HEALTH

Description of Site:

Site area

Total catchment area (ha)

-Milita		
NSW	Planning &	
GOVERNMENT	Environmen	t

t No. 28

SWMP Commentary, Detailed Calculations

4. Volume of Sediment Basins, Type D and Type F Soils

Basin volume = settling zone volume + sediment storage zone volume

Settling Zone Volume

The settling zone volume for Type F and Type D soils is calculated to provide capacity to contain all runoff expected from up to the y-percentile rainfall event. The volume of the basin's settling zone (V) can be determined as a function of the basin's surface area and depth to allow for particles to settle and can be determined by the following equation:

 $V = 10 \times C_v \times A \times R_{x-day, y-\%ile} (m^3)$

Issued under the Environmental Planning and Assessment Act 1979

Approved Application No. 558 9575 Remarks 1 1 JUN 2019 granted on the ..

Disturbed catchment area (ha) 0.19 0.34 1.46 0.83 0.88 1.06 Signed Soil analysis

0.19 0.34 1.46 0.83 0.88 1.06

8 9 10 11

% sand (faction 0.02 to 2.00 mm	Soil texture should be assessed through
% silt (fraction 0.002 to 0.02 mm)	mechanical dispersion only. Dispersion agents (e.g. Calgon) should not be used
% clay (fraction finer than 0.002 mm)	agents (e.g. Calgon) should not be used
Dispersion percentage	E.g. enter 10 for dispersion of 10%
% of whole soil dispersible	See Section 6.3.3(e)
Soil Texture Group	See Section 6.3.3(c), (d) and (e)

Rainfall data

Design rainfall depth (days)	5	5	5	5	5	5	See Sections 6.3.4 (d) and (e)
Design rainfall depth (percentile)	85	85	85	85	85	85	See Sections 6.3.4 (f) and (g)
x-day, y-percentile rainfall event	62.5	62.5	62.5	62.5	62.5	62.5	See Section 6.3.4 (h)
Rainfall intensity: 2-year, 6-hour storm	15.9	15.9	15.9	15.9	15.9	15.9	See IFD chart for the site

RUSLE Factors

Rainfall erosivity (R -factor)	5750	5750	5750	5750	5750	5750	Automatic calculation from above data			
Soil erodibility (K-factor)	0.015	0.015	0.015	0.015	0.015	0.15				
Slope length (m)	64	98	76	124	82	188	1			
Slope gradient (%)	1.5	2.9	19.7	5.3	1.95	3.4	RUSLE data can be obtained from			
Length/gradient (LS -factor)	0.25	0.71	7.1	1.6	0.41	1.3	Appendixes A, B and C			
Erosion control practice (P -factor)	1.3	1.3	1.3	1.3	1.3	1.3				
Ground cover (C-factor)	1	1	1	1	1	1				

Calculations

Soil loss (t/ha/yr)	28	80	796	179	46	1458	
Soil Loss Class	1	1	6	2	1	6	See Section 4.4.2(b)
Soil loss (m³/ha/yr)	22	61	612	138	35	1121	
Sediment basin storage volume, m3	1	4	152	19	5	202	See Sections 6.3.4(i) and 6.3.5 (e)

Revised Catchment Calc 1.2 CAM 190110.xlsx

10 = a unit conversion factor

C_v = the volumetric runoff coefficient defined as that portion of rainfall that runs off as stormwater over the x-day period

v-%ile = is the x-day total rainfall depth (mm) that is not exceeded in y percent of rainfall events. (See Sections 6.3.4(d), (e), (f), (g) and (h)).

A = total catchment area (ha)

Sediment Storage Zone Volume

In the detailed calculation on Soil Loss Classes 1 to 4 lands, the sediment storage zone can be taken as 50 percent of the settling zone capacity. Alternately designers can design the zone to store the 2-month soil loss as calculated by the RUSLE (Section 6.3.4(i)(ii)). However, on Soil Loss Classes 5, 6 and 7 lands, the zone must contain the 2-month soil loss as calculated by the RUSLE (Section 6.3.4(i)(iii).

Place an "X" in the box below to show the sediment storage zone design parameters used here:

50% of settling zone capacity, 2 months soil loss calculated by RUSLE

Total Basin Volume

Site	C,	R _{x-day, y-%ile}	Total catchment area (ha)	Settling zone volume (m³)	Sediment storage volume (m³)	Total basin volume (m³)
6b	0.70	62.5	0.19	83.125	1	84.125
7	0.70	62.5	0.34	148.75	4	152.75
8	0.70	62.5	1.46	638.75	152	790.75
9	0.70	62.5	0.83	363.64375	19	382,64375
10	0.70	62.5	0.88	385	5	390
11	0.70	62.5	1.06	463.75	202	665.75

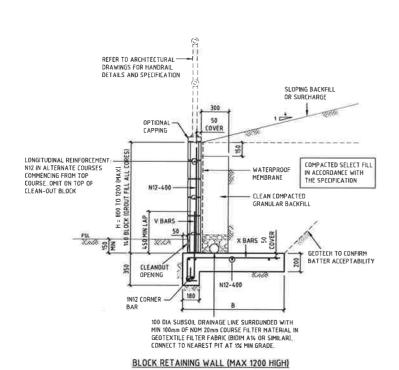
Revised Catchment Calc 1.2 CAM 190110.xisx

STAGE 1 EARLY WORKS SOIL AND WATER MANAGEMENT CALCULATIONS - SHEET 2 OF 2

TWEED VALLEY HOSPITAL

DEVELOPMENT, CUDGEN

DEVELOPMENT APPLICATION 1:1000 20 10748 01 C008



REFER TO ARCHITECTURAL
DRAWINGS FOR HANDRAIL
DETAILS AND SPECIFICATION

OPTIONAL
CAPPING

OPTIONAL
COMPACTED
GRANULAR BACKFILL

OMPACTED SELECT FILL
IN ACCORDANCE WITH
THE SPECIFICATION

GEOTECH TO CONFIRM
BATTER ACCEPTABLITY

NIS CORNER

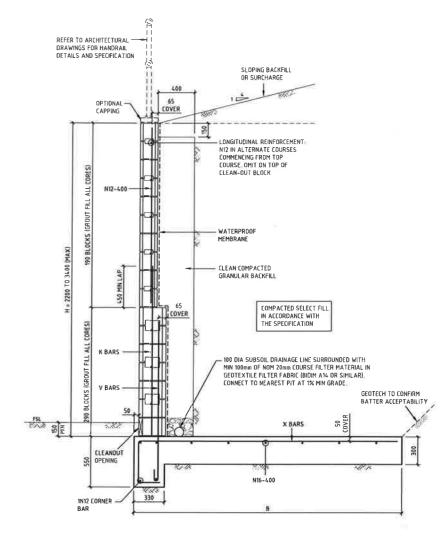
OD DIA SUBSOIL DRAINAGE LINE SURROUNDED WITH
MIN 100mm OF NOM 20mm COURSE FILTER HATERIAL IN
GEOTECTILE FILE FABRIC (BIDINA 14 OR SHIRL)
CONNECT TO NEAREST PIT AT 1% HIN GRADE.

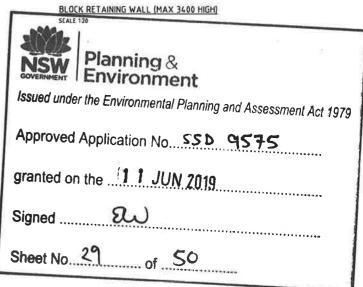
BLOCK RETAINING WALL (MAX 2000 HIGH)
SCALE 120

SCALE 1:20

NOTE: DESIGNER TO CHECK THE NEED FOR SHEAR KEY

						SE TYPE 1	
	WA	LL HEIGHT		REINFO	RCEMENT	BASE D	DIMENSIONS
TOTAL HEIGHT	HEIG	HT OF BLOCKW	ORK	X-BARS AND	K-BARS	WIDTH, B (mm) WITH FOLL	OWING BACKFILL CONDITIONS
(mm) H	150 SERIES	200 SERIES	300 SERIES	V-BARS	N-DAKS	LEVEL	MAX 1:4 SLOPE
800	800	*	- 1	N12-400	- 1	800	1000
1000	1000		-	N12-400	16	1000	1200
1200	1200	ŝ		N12-400	15	1100	1500
1400		1400		N12-400	i K	1300	1700
1600		1600		N16-400	183	1400	2000
1800	-	1800	5.	N16-400	165	1600	2200
2000	20	2000	2	N16-200	72	1700	2500
2200	14	1400	800	N16-400	N16-400	1900	2800
2400		1600	800	N16-400	N16-400	2000	3100
2600	- 14	1600	1000	N20-400	N20-400	2200	3300
2800		1800	1000	N20-400	N20-400	2400	3600
3000		2000	1000	N16-200	N16-200	2600	3900
3200	S .	2000	1200	N20-200	N16-200	2800	4200
3400		2000	1400	N20-200	N16-200	2900	4500



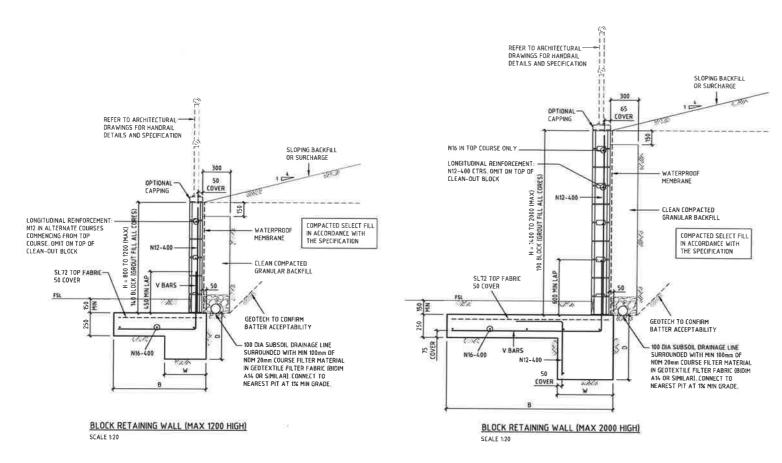


_	maco (Insue		_	_	-		_	_
				=			_	-
_	DA BSUF	N. 01 10	DB	-	-		_	-
2	THE COSE							

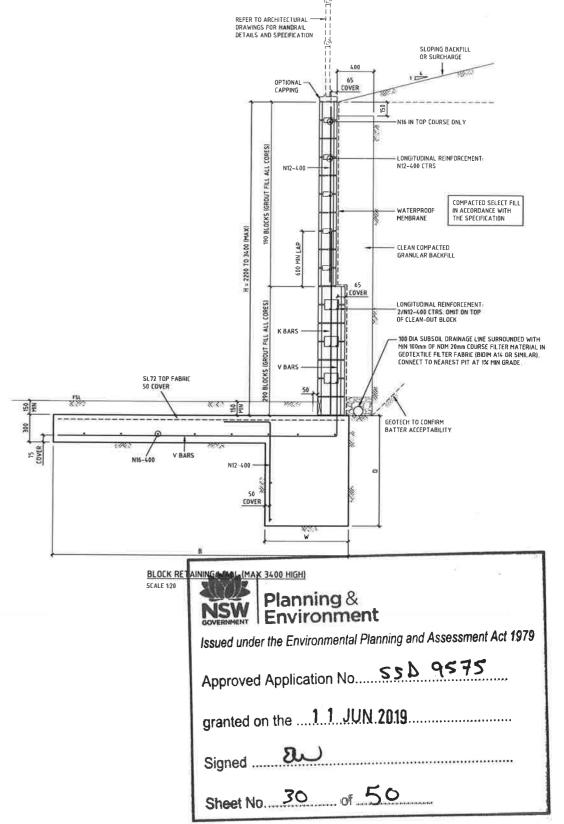


ABN 42 000 332 345 Consulting Empreers. Structural - Chill - Inhustrust Level 6, 37 York Sheet, Sydney, NSW 2000 Australi Tel 98 2 8247 8400 Fee: +61 2 8247 8444	BONACCI GROUP	Pty Ltd
Level 6, 37 York Street, Sydney, NSW 2000 Australi	ABN 42 000 332 34	9
	Consulting Enginee	rs. Structural - Civil - Infrastructu
Tot +81 7 0047 8400 For +81 9 8347 8444	Level 6, 37 York Sh	eet, flydney, NSW 2000 Australia
	Tel. +81 28247 84	00 Fee: +61 2 8247 8444
	sydney@bonsocign	

Project Name	TWEED VALLEY HOSPITAL DEVELOPMENT, CUDGEN	ı	DEVEL	OPMENT APPI	LICATION	٧
	STAGE 1 EARLY WORKS	Des gne	d PA	Project Director Approved	Date	North
Drawing Tale	RETAINING WALL	Scale	121	Project Ref	Drawing No	Rev
	DETAILS SHEET 1	Date Sheet	03 09 18 A1	20 10748 01	C055	P2



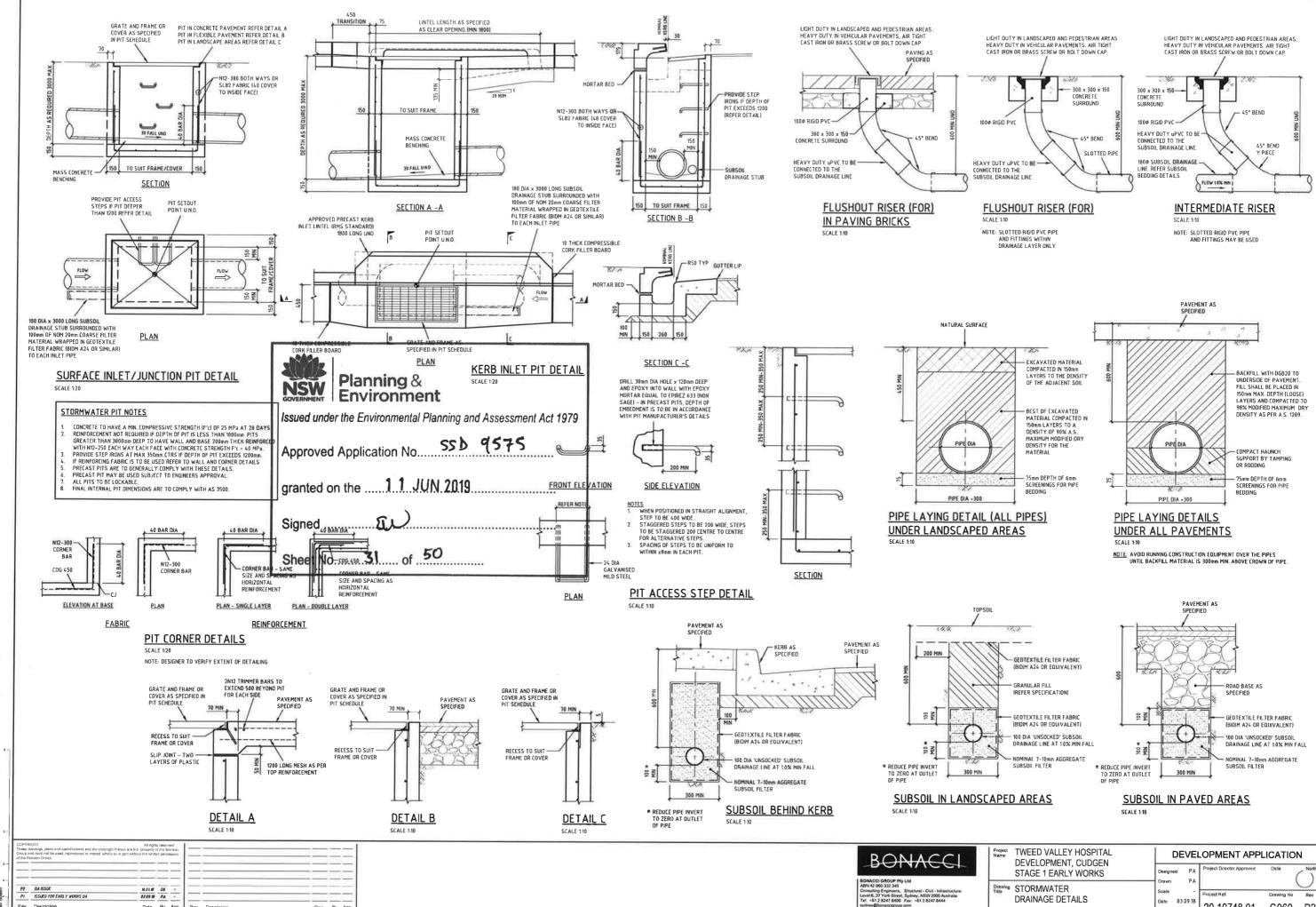
			BLO	OCK RETAI	NING WAL	L BASE TY	'PE 2			
	WA	LL HEIGHT		REINFO	RCEMENT			BASE DIMENSI	ONS	
TOTAL	HEIG	HT OF BLOCKW	ORK	X-BARS			LEVEL B	ACKFILL	MAX 1:4 SLOP	ING BACKFILL
HEIGHT (mm) H	150 SERIES	200 SERIES	300 SERIES	AND V-BARS	K-BARS	HEEL WIDTH (mm) W	BASE WIDTH (mm) B	HEEL DEPTH (mm) D	BASE WIDTH (mm) B	HEEL DEPTH (mm) D
800	800	290		N12-400		450	600	500	800	500
1000	1000	题(速	N12-400	*	450	800	500	1000	500
1200	1200	727		N12-400		450	1000	500	1200	600
1400		1400		N16-400	2	450	1200	500	1400	600
1600	<u>:</u>	1600		N16-400		450	1400	600	1600	700
1800	(A)	1800	3	N16-400	2	450	1600	700	1800	800
2000	96	2000	9	N16-200		600	1800	700	2000	800
2200	200	1400	800	N16-400	N16-400	600	2000	800	2200	900
2400	•	1600	800	N16-400	N16-400	600	2200	900	2400	1000
2600	- 2	1600	1000	N20-400	N20-400	900	2400	900	2600	1000
2800	30	1800	1000	N20-400	N20-400	900	2600	900	2800	1100
3000		2000	1000	N16-200	N16-200	900	2800	1000	3000	1200
3200	- 4	2000	1200	N20-200	N16-200	900	3000	1100	3200	1300
3400	(a) I	2000	1400	N20-200	N16-200	900	3200	1200	3400	1500



				_	1-			-
=				\equiv				
P2	DA ISSUE	16.01 IB	DB		I-			-
PI	ISSUED FOR EARLY WORKS DA	16,10,18	PN	$\overline{}$	_			
Rev	Description	Date	Ву	App	Rev	Description	Dalin By	Ap

	CI GROUP Pt 060 332 345	Ltd	
Consult Level 6,	ng Engineers. 37 York Street	Gra	clurel - GWé - Hifrantructure vey, NSW 2000 Australia
sydney (2 8247 8400 Boonsocigroup maccigroup cor	10001	-61 2 8247 6444

* TWEED VALLEY HOSPITAL DEVELOPMENT, CUDGEN	DEVE	OPMENT APPL	ICATION
STAGE 1 EARLY WORKS	Designed PA	Project Director Approved	Date North
™ RETAINING WALL	Drawn PN		
DETAILS SHEET 2	Scale -	Project Ref	Drawing No Rev
DETAILS SHEET Z	Date 03.09.18 Sheet A1	20 10748 01	C056 P2



DRAINAGE DETAILS

Date 03.09.18

A1 20 10748 01 C060 P2

Date By App Rev Description

Dale By App

TWEED VALLEY HOSPITAL DEVELOPMENT **TURNOCK STREET ROUNDABOUT WORKS**

DRAWING No. **DESCRIPTION** 20 10748 C500 DRAWING REGISTER AND CONSTRUCTION NOTES 20 10748 0505 SEDIMENT AND EROSION CONTROL PLAN 20 10748 C540 EXTERNAL WORKS INTERSECTION PLAN 20 10748 C545 DEMOLITION PLAN 20 10748 Γ560 CIVIL WORKS DETAILS

Planning & Environment
Issued under the Environmental Planning and Assessment Act 1979
Approved Application No. SSD 9575
granted on the1 1 JUN 2019
Signed
Sheet No32 of .50

GENERAL NOTES

- THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ARCHITECTURAL AND OTHER CONSULTANTS DRAWINGS AND SPECIFICATIONS AND WITH SUCH OTHER WRITTEN INSTRUCTIONS OR SKETCHES AS MAY BE ISSUED DURING THE COURSE OF THE CONTRACT, ANY DISCREPANCY SHALL BE REFERRED TO THE SUPERNITENDENT BEFORE PROCEEDING WITH WORK.
- MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE SPECIFICATION, CURRENT SAA CODES, BUILDING REGULATIONS AND THE REQUIREMENTS OF ANY OTHER RELEVANT STATUTORY
- G3 THESE DRAWINGS MUST NOT BE SCALED, ALL DIMENSIONS ARE IN METERS, ALL SET OUT DIMENSIONS AND LEVELS, INCLUDING THOSE SHOWN ON THESE DRAWINGS SHALL BE IN ACCORDANCE WITH THE ARCHITECT'S DRAWINGS AND VERIFIED ON SITE
- ALL SETOUT AND DIMENSIONS OF THE STRUCTURE INCLUDING KERBS AND RETAINING WALLS, AND BULK EARTHWORKS MUST BE TAKEN FROM THE ARCHITECT'S DRAWINGS, SETOUT OF THE STORMWATER PITS BY OTHERS. CONTRACTOR TO CONFIRM SETOUT OF SERVICE TRENCHING
- THE CONTRACTOR SHALL COMPLY WITH ALL REGULATIONS OF AUTHORITIES HAVING JURISDICTON OVER THE WORKS, REFER TO GEOTECHNICAL REPORT BY MORRISON GEOTECHNIC PTY LTD, REFERENCE, GE18/144 REV 2. DATED 26th SEPTEMBER 2018.
- ALL DIMENSIONS AND REDUCED LEVELS MUST BE VERIFIED ON SITE REFORE THE COMMENCEMENT OF
- THE APPROVAL OF A SUBSTITUTION SHALL BE SOUGHT FROM THE SUPERINTENDENT BUT IS NOT AN AUTHORISATION OF A COST VARIATION. THE SUPERINTENDENT MUST APPROVE ANY COST VARIATION INVOLVED BEFORE ANY WORK STARTS.
- G8 ALL LEVELS SHOWN ARE TO THE AUSTRALIAN HEIGHT DATUM.
- SERVICE INFORMATION SHOWN IS APPROXIMATE ONLY, PRIOR TO COMMENCEMENT OF ANY WORKS, THE CONTRACTOR SHALL LOCATE ALL UNDERGROUND SERVICES AND COMPLY WITH ALL
- G10 EXISTING SURFACE CONTOURS, WHERE SHOWN, ARE INTERPOLATED AND MAY NOT BE ACCURATE.
- G12 MAKE SMOOTH CONNECTION WITH ALL EXISTING WORKS.
- G13 THESE DWGS SHOULD BE READ IN CONJUNCTION WITH COUNCIL'S STANDARD DETAILS.

SITEWORKS NOTES

- PRIOR TO THE PLACEMENT OF ANY PAVEMENTS. BUILDINGS OR DRAINS THE EXPOSED SURGRADE SHALL BE COMPACTED TO A MINIMUM OF 98% STANDARD COMPACTION IN ACCORDANCE WITH TEST 'E1,1' OF A.S., 1289 FOR THE TOP 300mm. ANY SOFT SPOTS SHALL BE REMOVED AND REPLACED WITH GRANULAR FILL TO THE ENGINEERS APPROVAL AND COMPACTED IN ACCORDANCE WITH THE COMPACTION REQUIREMENTS SET OUT BELOW. ON HIGHLY REALTY ELAY AREAS SITE EXCAVATED MATERIAL MAY BE USED WITH THE PRIDR AUTHORISATION OF THE ENGINEER.
- ALL FILL AND PAVEMENT MATERIALS SHALL BE COMPACTED IN ACCORDANCE WITH GEOTECHNICAL REPORT BY MORRISON GEOTECHNIC PTY LTD REFERENCE. GE18/14. REV 2 DATED 28th SEPTEMBER 2018 MOISTURE CONTENT TO BE MAINTAINED AT 7-22 MML, NINIMUM COMPACTION REQUIREMENTS ARE DETAILED BELOW FOR TALL REQUIREMENTS ARE TO VERNIFED BY A SUITABLY DUALIFIED

- FILL UNDER ANY FOOTINGS AND FLOOR SLABS FOR ANY STRUCTURE TO SUBGRADE LEVEL;
- FINE CRUSHED ROCK SELECTED FILL WITHOUT CONSPICUOUS CLAY CONTENT
- FILL UNDER ROAD PAVEMENTS;
 TO WITHIN 500mm OF FINISHED SUBGRADE LEVEL
 UP TO FINISHED SUBGRADE LEVEL
- . ROAD PAVEMENT MATERIALS;

- THE MAXIMUM COMPACTION IS TO BE NO GREAT THAN 4% ON TOP OF THE ABOVE MENTION VALUES.
- GRADE EVENLY BETWEEN FINISHED SURFACE SPOT LEVELS. FINISHED SURFACE CONTOURS ARE GRADED SMOOTHLY SO THAT IT WILL DRAIN AND MATCH ADJACENT SURFACES OR STRUCTURES
- ALL DIMENSIONS GIVEN ARE TO FACE OF KERB, CENTER OF PIPE OR EXTERIOR FACE OF BUILDING
- ANY STRUCTURES. PAVEMENTS OR SURFACES DAMAGED, DIRTIED OR MADE UNSERVICABLE DUE TO CONSTRUCTION WORK SHALL BE REINSTATED TO THE SATISFACTION OF THE ENGINEER
- S6 ANY FILL REQUIRED SHALL BE APPROVED BY THE ENGINEER / GEDTECHNICAL CONSULTANT
- CONTRACTOR IS TO ENSURE THAT ALL EXCAVATIONS ARE MAINTAINED IN A DRY CONDITION WITH NO
- S8 ALL FINISHES AND COLOURS TO BE IN ACCORDANCE WITH ARCHITECTURAL SPECIFICATIONS
- REFER TO STRUCTURAL DRAWINGS FOR CONCRETE, REINFORCEMENT AND RETAINING WALL DETAILS
- A) COMPLY WITH THE GENERAL PROVISIONS OF PART 3.1 "MANAGING RISKS TO HEALTH AND SAFETY" OF NSW WORK AND HEALTH AND SAFETY REGULATION 2011
 - B) COMPLY PART 6.3 DIVISION 3 "EXCAVATION WORK" OF NSW WORK HEALTH AND SAFETY REGULATION NSW 2011
- S11 PRIOR TO THE EXCAVATION OF ANY TRENCH DEEPER THAN 15 METRES THE CONTRACTOR MUST:

 A) NOTIFY THE DECUPATIONAL HEALTH AND SAFETY AUTHORITY ON THE APPROPRIATE FORM

STORMWATER DRAINAGE NOTES

- SWI UNLESS NOTED OTHERWISE BY HYDRAULIC ENGINEERS DRAWINGS, ALL DOWNPIPES & GRATED INLETS SHALL BE CONNECTED. TO PITS OR MAIN STORMWATER DRAINS WITH ISO DIA, UPVC PIPES LAID AT A MINIMUM GRADE OF 1 IN 100. FOR SYPHONIC ROOF DRAINAGE SYSTEMS ALL DOWNPIPES CONNECTION DRAIN SIZES TO BE CONNECTED IND MAIN STORMWATER DRAINS SHALL BE IN ACCORDANCE WITH HYDRAULIC ENGINEERS DRAWINGS.
- ALL MAIN STORMWATER DRAINS SHALL BE CONSTRUCTED USING MATERIALS AS SPECIFIED ON THE DRAWINGS IN ACCORDANCE WITH THE APPROPRIATE A.S., IF NOT SPECIFIED THEN CLASS 4 RRJ RCP SHALL BE USED FOR DIAMETERS > 225mm SEWER CLASS SEH UPVC IN ACCORDANCE WITH AS1260 SHALL BE USED FOR #225mm OR SMALLER.
- ALL PIPEWORK TO BE INSTALLED IN ACCORDANCE WITH AS3725 FOR RCP AND AS2032 FOR PVC. ALL BEDDING TO BE TYPE H2 UNLESS NOTED OTHERWISE
- SW4 FOR ALL PITS > 1.2m DEEP, STEP IRONS SHALL BE INSTALLED.
- PRECAST PITS MAY BE USED EXTERNAL TO THE BUILDING SUBJECT TO APPROVAL BY BONACCI
- ENLARGERS, CONNECTIONS AND JUNCTIONS TO BE PREFABRICATED FITTINGS WHERE PIPES ARE LESS THAN 300 DIA.
- WHERE SUBSOIL DRAINS PASS UNDER FLOOR SLABS AND VEHICULAR PAVEMENTS, UNSLOTTED
- SW8 GRATES AND COVERS SHALL CONFORM WITH AS 3996 AND AS 1428.1 FOR ACCESS REQUIREMENTS.
- CARE IS TO BE TAKEN WITH LEVELS OF STORMWATER LINES: GRADES ARE NOT TO BE REDUCED.
- SWIO AT ALL TIMES DURING CONSTRUCTION OF STORMWATER PITS. ADEQUATE SAFETY PROCEDURES SHALL BE TAKEN TO ENSURE AGAINST THE POSSIBILITY OF PERSONNEL FALLING DOWN PI
- SW11 ALL EXISTING STORMWATER DRAINAGE LINES AND PITS THAT ARE TO REMAIN ARE TO BE INSPECTED AND CLEANED, DURING THIS PROCESS ANY PART OF THE STORMWATER DRAINAGE SYSTEM THAT WARRANTS REPAIR SHALL BE REPORTED TO THE SUPERINTENDENT/ENGINEER FOR

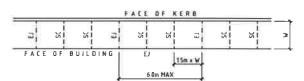
KERBING NOTES

- K1 ALL CONCRETE TO HAVE A MINIMUM COMPRESSIVE STRENGTH OF 32 MPa U.N.O.
- ALL KERBS, GUTTERS, DISH DRAINS AND CROSSINGS TO BE CONSTRUCTED ON 75mm GRANULAR BASECOURSE COMPACTED TO A MINIMUM 98% MAXIMUM DRY DENSITY IN ACCORDANCE WITH AS1289
- EXPANSION JOINTS (E.)! TO BE FORMED FROM 10mm COMPRESSIBLE CORK FILLER BOARD FOR THE FULL DEPTH OF THE SECTION AND CUT TO PROFILE. EXPANSION JOINTS TO BE LOCATED AT DRAINAGE PITS, ON TAMBERN POINTS OF CURVES AND ELSEWHERE AT MAX 12m CENTRES EXCEPT FOR INTEGRAL KERBS WHERE THE EXPANSION JOINTS ARE TO MATCH THE JOINT LOCATIONS IN THE SLAB.
- BROOMED FINISH TO ALL RAMPED AND VEHICULAR CROSSINGS, ALL OTHER KERBING OR DISH DRAINS TO BE STEEL FLOAT FINISHED.
- IN THE REPLACEMENT OF KERBS:
 EXISTING ROAD PAVEMENT IS TO BE SAWCUT 900mm U.N.O., FROM THE LIP OF GUTTER, UPON COMPLETION OF THE NEW KERB AND GUTTER, NEW BASECOURSE AND SURFACE TO BE LAID 600mm WIDE U.N.O.
 - EXISTING KERBS ARE TO BE COMPLETELY REMOVED WHERE NEW KERBS ARE SHOWN

JOINTING NOTES

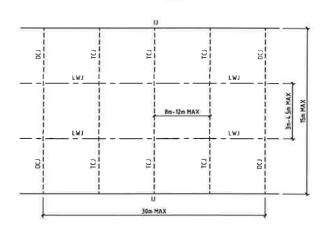
PEDESTRIAN FOOTPATH JOINTS

- EXPANSION JOINTS (EJ) ARE TO BE LOCATED WHERE POSSIBLE AT TANGENT POINTS OF CURVES
- SAWCHT IDINTS (SC) ARE TO BE LOCATED AT A MAY 15m v WIDTH DE PAVENENT. THE TIMING DE THE SAWCUT IS TO BE CONFIRMED BY THE CONTRACTOR ON SITE, SITE CONDITIONS WILL DETERMINE HOW MANY HOURS AFTER THE CONCRETE POUR BEFORE THE SAW CUTS ARE COMMENCED.
- WHERE POSSIBLE JOINTS SHOULD BE LOCATED TO MATCH KERBING AND / OR ADJACENT PAVEMENT
- PROVIDE 10mm WIDE FULL DEPTH EXPANSION JOINTS (EJ) BETWEEN BUILDINGS AND ALL CONCRETE OR UNIT PAVERS
- ALL PEDESTRIAN FOOTPATH JOINTINGS AS FOLLOWS (U.N.D.).



VEHICULAR PAVEMENT JOINTS

- ALL VEHICULAR PAVEMENTS TO BE JOINTED AS SHOWN ON DRAWINGS.
- EDNGITUDINAL WARPING JOINTS (LWJ) SHOULD GENERALLY BE LOCATED AT A MAXIMUM OF 3m TO 4.5m MAX CENTERS. ALL LWJ'S SHOULD BE TIED UP TO A MAXIMUM TOTAL WIDTH OF 30m
- TRANSVERSE CONTRACTION JOINTS (TCJ) SHOULD GENERALLY BE LOCATED AT A MAXIMUM OF BM TO 12m MAX CENTERS, TCJ's CAN BE SPACED AT SUITABLE INTERVALS UP TO A RECOMMENDED MAXIMUM LENGTH OF 15m.
- TRANSVERSE DOWELLED CONSTRUCTION JOINTS (DCJ) TO BE PROVIDED FOR PLANNED INTERRUPTIONS SUCH AS AT THE END OF EACH DAY'S OPERATIONS (POUR BREAK), AT BLOCK OUTS FOR BRIDGES AND INTERSECTIONS OR FOR UNEXPECTED DELAYS WHEN THE SUSPENSION OF OPERATIONS IS LIKELY TO CREATE A JOINT
- ISOLATION JOINTS WITH SUB-GRADE BEAM (IJ) TO BE PROVIDED AT INTERSECTIONS OR AT THE JUNCTION OF A POUR BREAK
- ALL VEHICULAR PAVEMENTS TO BE JOINTED IN ACCORDANCE WITH AUSTROADS AGPT02-12 GUIDE TO PAVEMENT TECHNOLOGY PART 2 STRUCTURAL PAVEMENT DESIGN AND SUPPLEMENT AP-T36-06 PAVEMENT DESIGN FOR LIGHT TRAFFIC

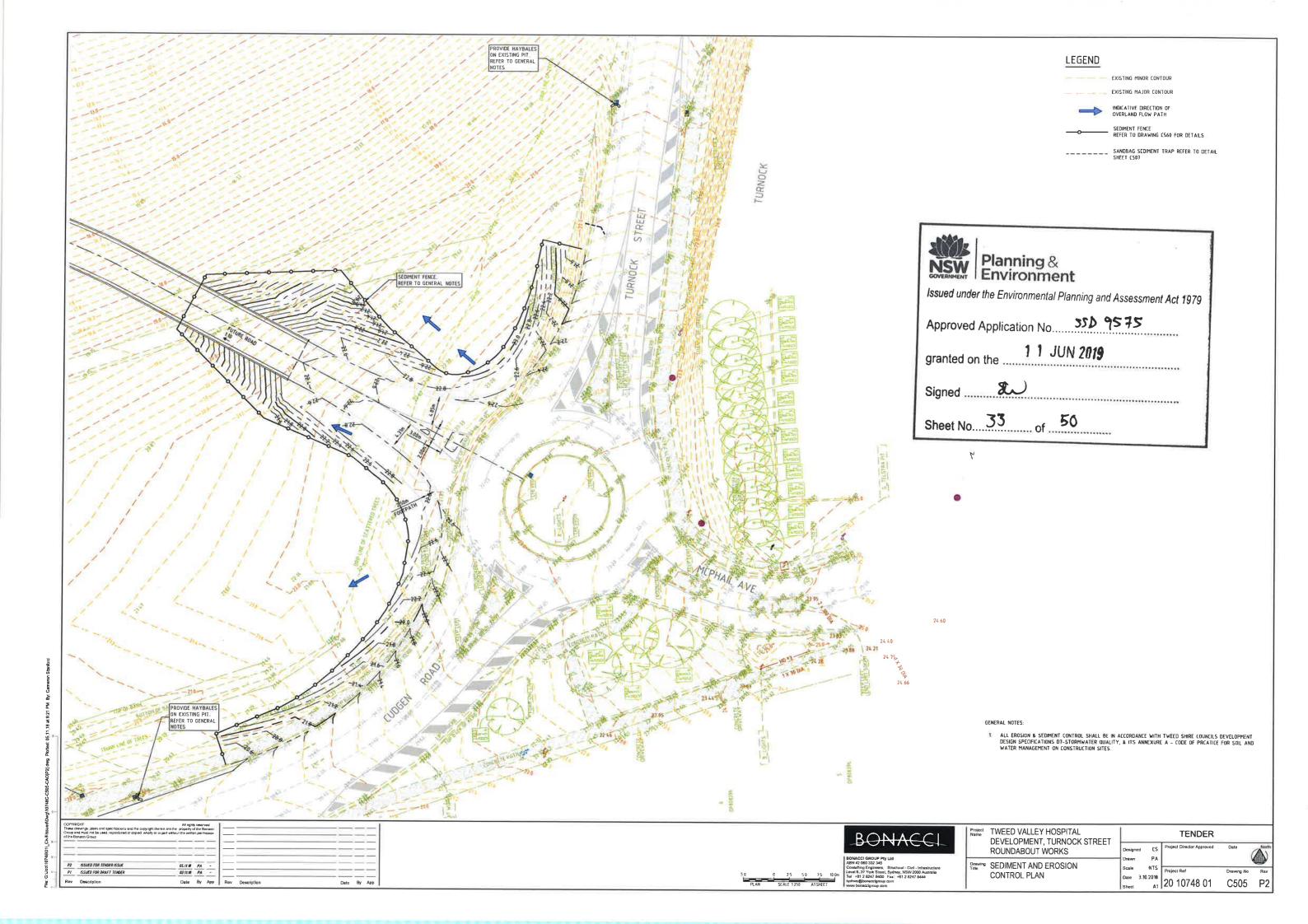


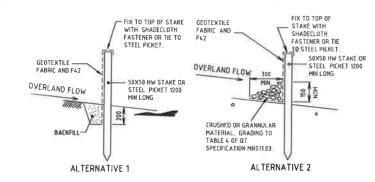
DEVELOPMENT, TURNOCK STREET ROUNDABOUT WORKS DRAWING REGISTER AND CONSTRUCTION NOTES

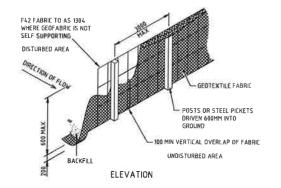
TWEED VALLEY HOSPITAL

TENDER ΡΔ NTS Date 3.10.2018 20 10748 01 C500

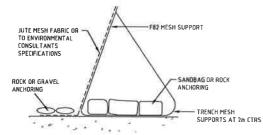
_			_	-			
					-		
P2	ISSUED FOR TENDER ISSUE	05.71 18	PA	-			
-	ISSUED FOR DRAFT TENDER	92.72 財	PA	_	_	 	







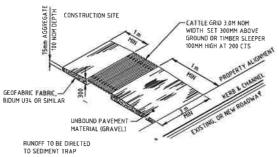
SEDIMENT FENCE



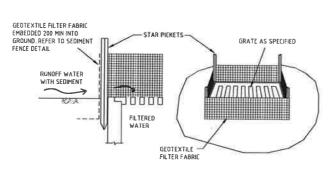
ALTERNATIVE SEDIMENT FENCE

ALTERNATIVE SECIMENT FENCE NOTES

- 1. INSTALL THIS TYPE OF SEDIMENT FENCE WHEN USE OF SUPPORT POSTS IS NOT DESIRABLE OR NOT POSSIBLE. SUCH CONDITIONS MIGHT APPLY, FOR EXAMPLE, WHERE APPROVAL IS GRANTED FROM THE APPROPRIATE AUTHORITIES TO PLACE THESE FENCES IN HIGHLY SENSITIVE ESTUARINE AREAS.
 2. USE BENT TRENCH MESH TO SUPPORT THE F02 WELDED MESH FACING AS SHOWN ON THE DRAWING ABOVE. ATTACH THE JUTE MESH TO THE WELDED MESH FACING USING UV-RESISTANT CABLE TIES.
 3. STABILISE THE WHOLE STRUCTURE WITH SANDBAG OR ROCK ANCHORING OVER THE TRENCH MESH AND THE LONGING FOR THE TRENCH MESH AND THE TR
- THE LEADING EDGE OF THE JUTE MESH. THE ANCHORING SHOULD BE SUFFICIENTLY LARGE TO ENSURE STABILITY OF THE STRUCTURE IN THE DESIGN STORM EVENT, USUALLY THE 10 YEAR EVENT

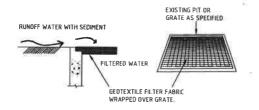


TEMPORARY CONSTRUCTION VEHICLE ENTRY/EXIT SEDIMENT TRAP

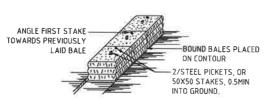


GEOTEXTILE PIT FILTER 1

NOT TO SCALE



GEOTEXTILE PIT FILTER 2 NOT TO SCALE



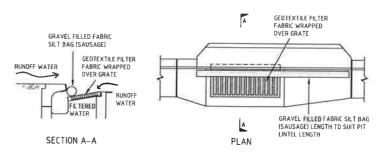
ANCHORING DETAIL



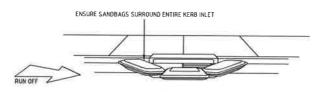
BEDDING DETAIL

STRAW BALE BANK SEDIMENT CONTROL

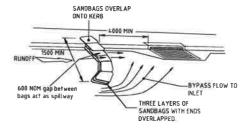
NOT TO SCALE



KERB INLET SEDIMENT TRAP

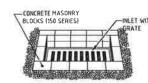


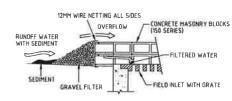
SANDBAG KERB INLET SEDIMENT TRAP



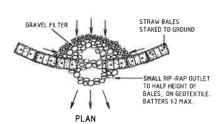
ON GRADE KERB INLET SEDIMENT TRAP

NOT TO SCALE





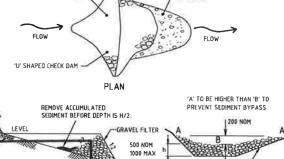
FIELD INLET SEDIMENT TRAP



STRAW BALE AND STONE TRAP SEDIMENT CONTROL (CONCENTRATE FLOW)

NOT TO SCALE

GRAVEL FILTER





CATCH DRAIN sued under the Environmental Planning and Assessment Act 1979

Approved Application No. 553 9575

Signed

granted on the 1 1 JUN 2019 Sheet No. 34 of 50

roup at	newings plans and specifications and the common not must not be used reproduced an economic macci Group	rerangent without the write	of the c	THE CT.		
			Ξ	\equiv		
			=			
4	ISSUED FOR TEMOER ISSUE	05. ft. se	PA	=		
-	Don Frin Hath	Date	ъ.	ānn.	David Brookle	n. h.

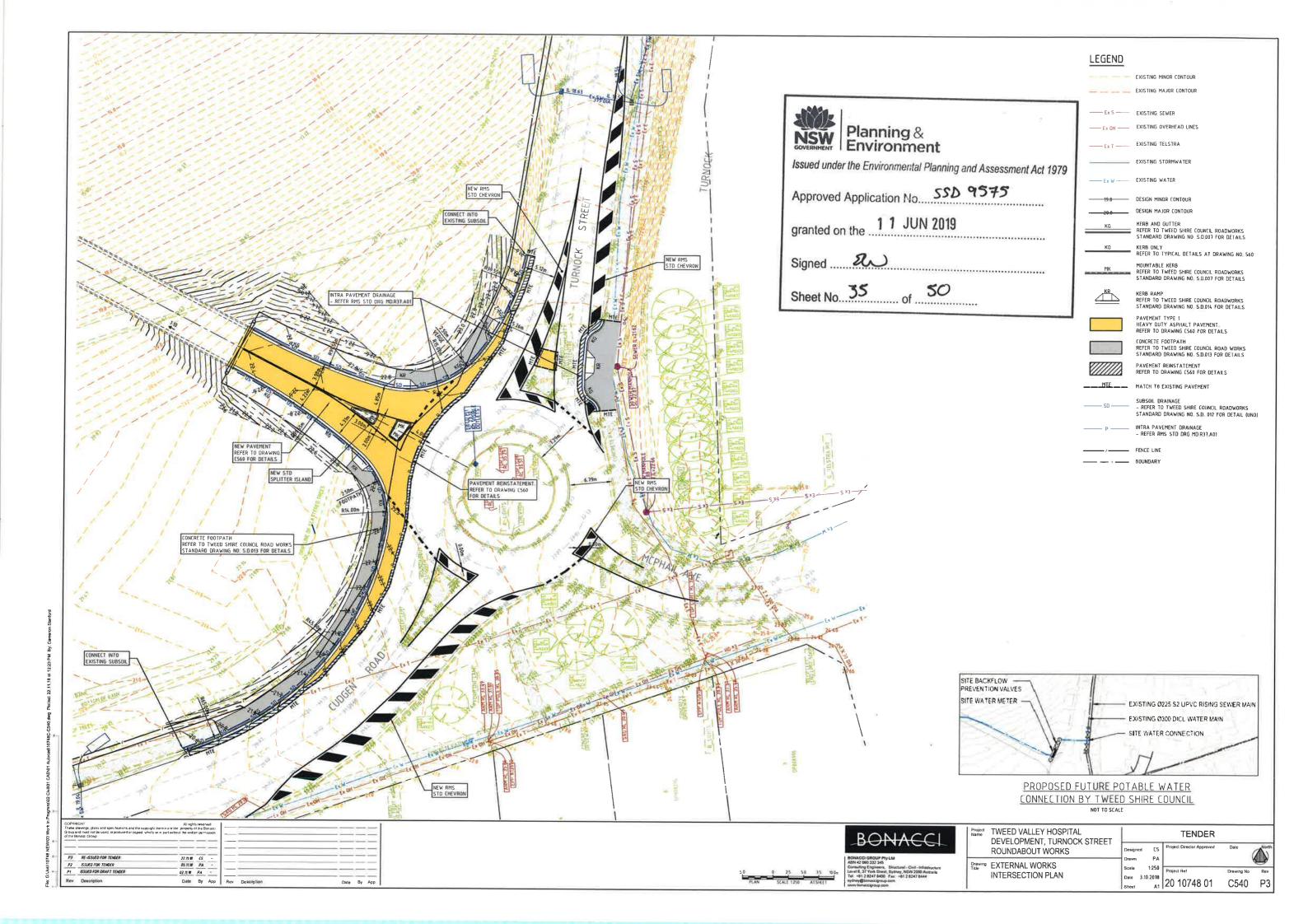
TWEED VALLEY HOSPITAL DEVELOPMENT, TURNOCK STREET ROUNDABOUT WORKS

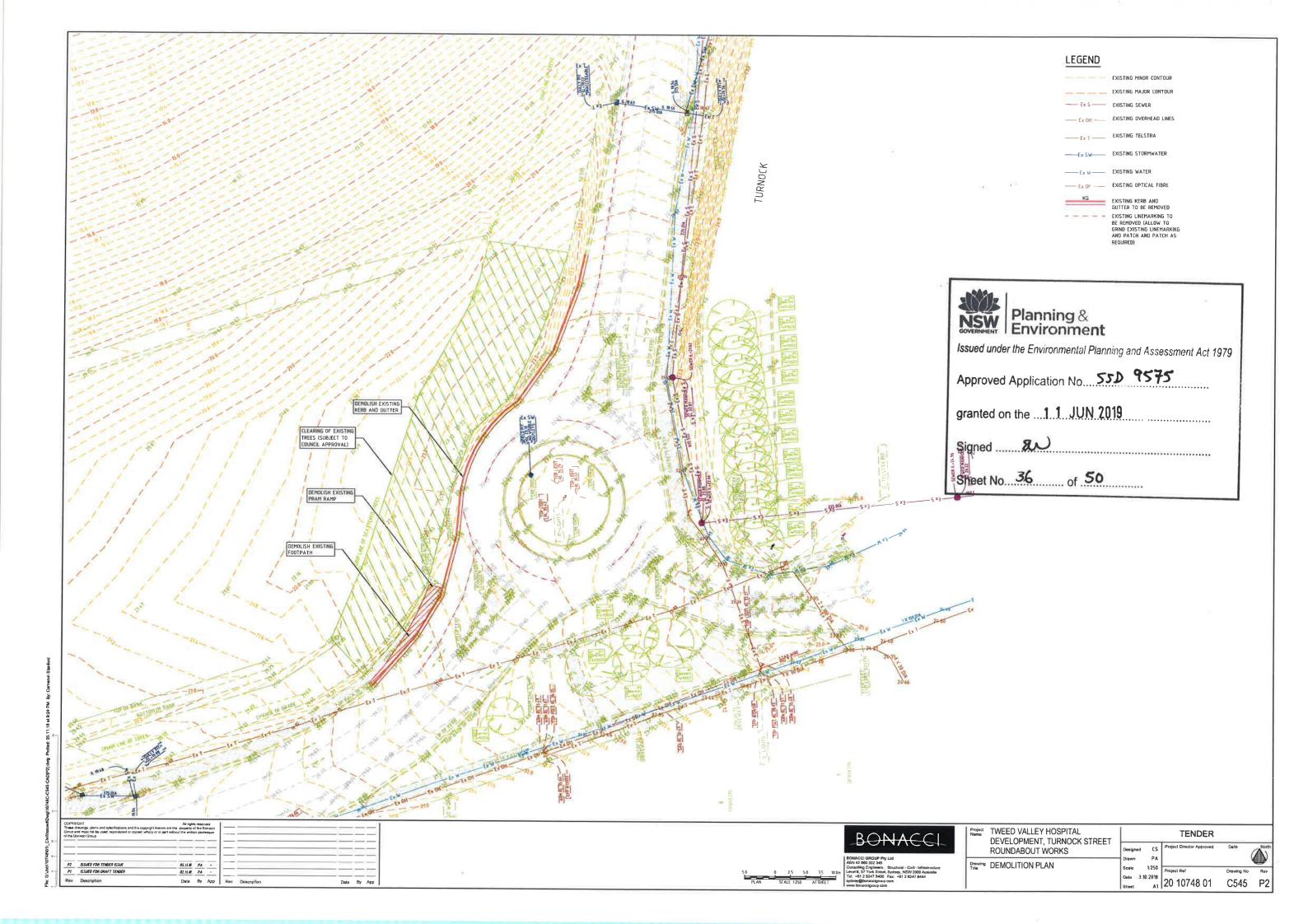
Drawing SOIL AND WATER MANAGEMENT **DETAILS**

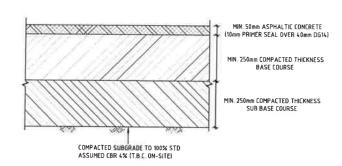
TENDER 02 10 18 A1 20 10748 01 C507 P1

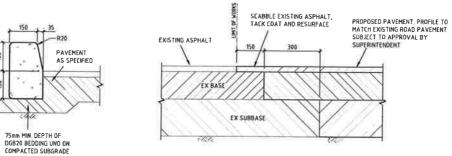
GEOTEXTILE, BIDUM A34 OR SIMILAR

SECTION A-A









AC CONNECTION TO EXISTING PAVEMENT

KERB ONLY (KO)

PAVEMENT TYPE P1 HEAVY DUTY ASPHALT PAVEMENT



- ROAD DESIGN PARAMETERS (FLEXIBLE)

 DESIGN TRAFFIC 15 X 10° (ESA'S OR HVAG)

 ASSUMPTIONS:

 TWEED SHIRE COUNCIL PAVEMENT DESIGN TABLE D2.2 ARTERIAL DISTRIBUTOR, SHOPPING STRIP ACCESS, INDUSTRIAL, DESIGN PERIOD 25 YEAR.

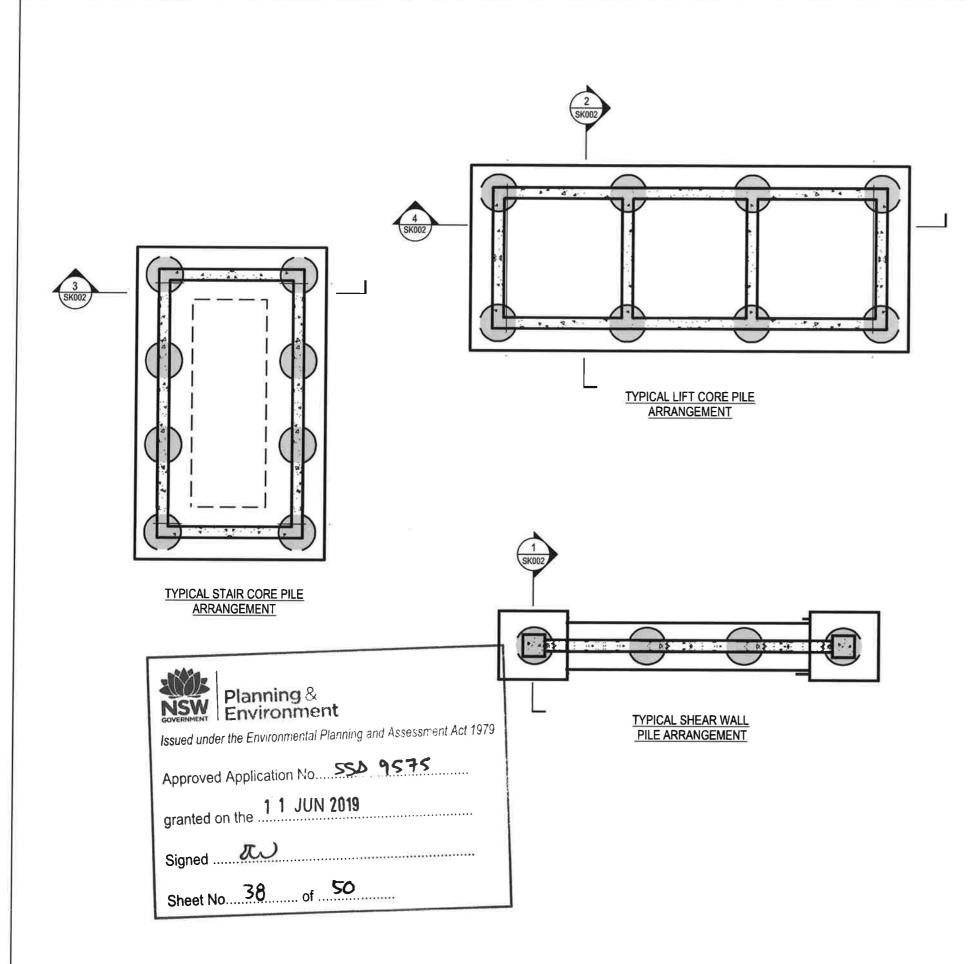
 CBR 4.0%

 PREPARE SUBGRADE AND SELECT FILL IN ACCORDANCE WITH (MORRISON GEOTECH, REFERENCE NO., GE18/144-REV 1)



Dale By App

Project Name TWEED VALLEY HOSPITAL **TENDER** DEVELOPMENT, TURNOCK STREET ROUNDABOUT WORKS Drawing CIVIL WORKS DETAILS



	BORED F	PILE SCHEDULE	
PILE	NO OF SUSPENDED SLABS SUPPORTED	ULTIMATE LIMIT STATE VERTICAL LOAD (kN)	NOMINAL DIAMETER (mm)
P1	3	3600	1000
P2	4	4800	1000
P3	5	6000	1000
P4	6	7200	1000
P5	7	8400	1000
P6	8	9600	1000
P7	9	10800	1000

NOTES

1. PILES TO BE DESIGNED BY D&C PILING SUBCONTRACTOR IN ACCORDANCE WITH AS2159
2. SUBJECT TO FINAL DESIGN, PILE DIAMETERS MAY VARY FROM 600MM TO 1200MM
3. PILES UNDER CORES, LIFT SHAFTS AND SHEAR WALLS TO BE DESIGNED TO RESIST THE LATERAL LOADS NOMINATED ON THE STRUCTURAL DRAWINGS

BORED PILES

- BP1 REFER TO THE GEOTECHNICAL REPORT FOR A DESCRIPTION OF THE ANTICIPATED SITE CONDITIONS. THE PILING CONTRACTOR IS TO STUDY THE REPORT AND MAKE HIS OWN EVALUATION OF THE SITE CONDITIONS. ANY ADDITIONAL COSTS INCURRED SHALL BE BORNE BY THE PILING CONTRACTOR.
- BP2 THE BORED PILES ARE PROPORTIONED FOR THE SCHEDULED LOADS WITH ALLOWABLE SOCKET SKIN FRICTION AND END BEARING CAPACITY AS INDICATED IN THE REPORT. THE DEPTHS AND LENGTHS NOMINATED IN THE SCHEDULE ARE INDICATIVE ONLY. THEY MAY NEED TO BE VARIED DEPENDING ON THE SITE CONDITIONS ENCOUNTERED. THE PILING CONTRACTOR NEEDS TO INCORPORATE ANY DESIGN CHANGES REQUIRED.
- BP3 THE BORED PILES SHALL BE INSTALLED TO A MAXIMUM TOLERANCE OF ±75mm FROM THAT REQUIRED IN PLAN AND INCLINED AT NOT MORE THAN 1 IN 75 FROM THE VERTICAL OR SPECIFIED RAKE.
- BP4 ALL WORKMANSHIP AND MATERIAL SHALL BE IN ACCORDANCE WITH AS 2159.
- BP5 THE BORED PILES SHALL BE LOCATED CONCENTRIC WITH THE COLUMNS AND WALLS UNLESS NOTED OTHERWISE.
- BP6 DRILL AND INSTALL THE BORED PILES IN THE LOCATIONS SHOWN ON THE DRAWINGS AND THE ABOVE REQUIREMENTS.
- BP7 BEFORE ANY CONCRETE IS POURED, ALL ROCK SOCKETS SHALL BE DEWATERED AND INSPECTED BY THE GEOTECHNICAL ENGINEER, WHO SHALL BE EMPLOYED BY THE BUILDER, TO VERIFY THE SOIL PARAMETERS. THE SOCKET BASE AND WALLS MUST BE CLEAN AND FREE FROM CLAY.
- BP8 IF THE CONCRETE NEEDS TO BE TREMIED, SUPER PLASTICIZER MUST BE ADDED TO THE MIX AND THE CONCRETE GRADE INCREASED BY 30%. REFER TO THE SPECIFICATIONS FOR THE INSPECTION OF THE HOLE PRIOR TO CONCRETING.
- BP9 THE PILING CONTRACTOR SHALL ALLOW FOR THE COST INTEGRITY TESTING OF ALL BORED PILES.
- BP10 ANY ALTERNATIVE DESIGN SHALL MEET THE ABOVE REQUIREMENTS AND THE SCHEDULED LOADS. THE PILING CONTRACTOR SHALL OBTAIN CERTIFICATION FOR THE CALCULATIONS OF THE ALTERNATIVE SYSTEM. THE DETAILS AND CALCULATIONS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW. THE CONTRACTOR SHALL BE FULLY RESPONSIBLE FOR THE PERFORMANCE OF THE ALTERNATIVE BORED PILES.

	\sim	1 4	
- н	1 0	$\Delta \Delta$	
	$\mathbf{\nabla}$		

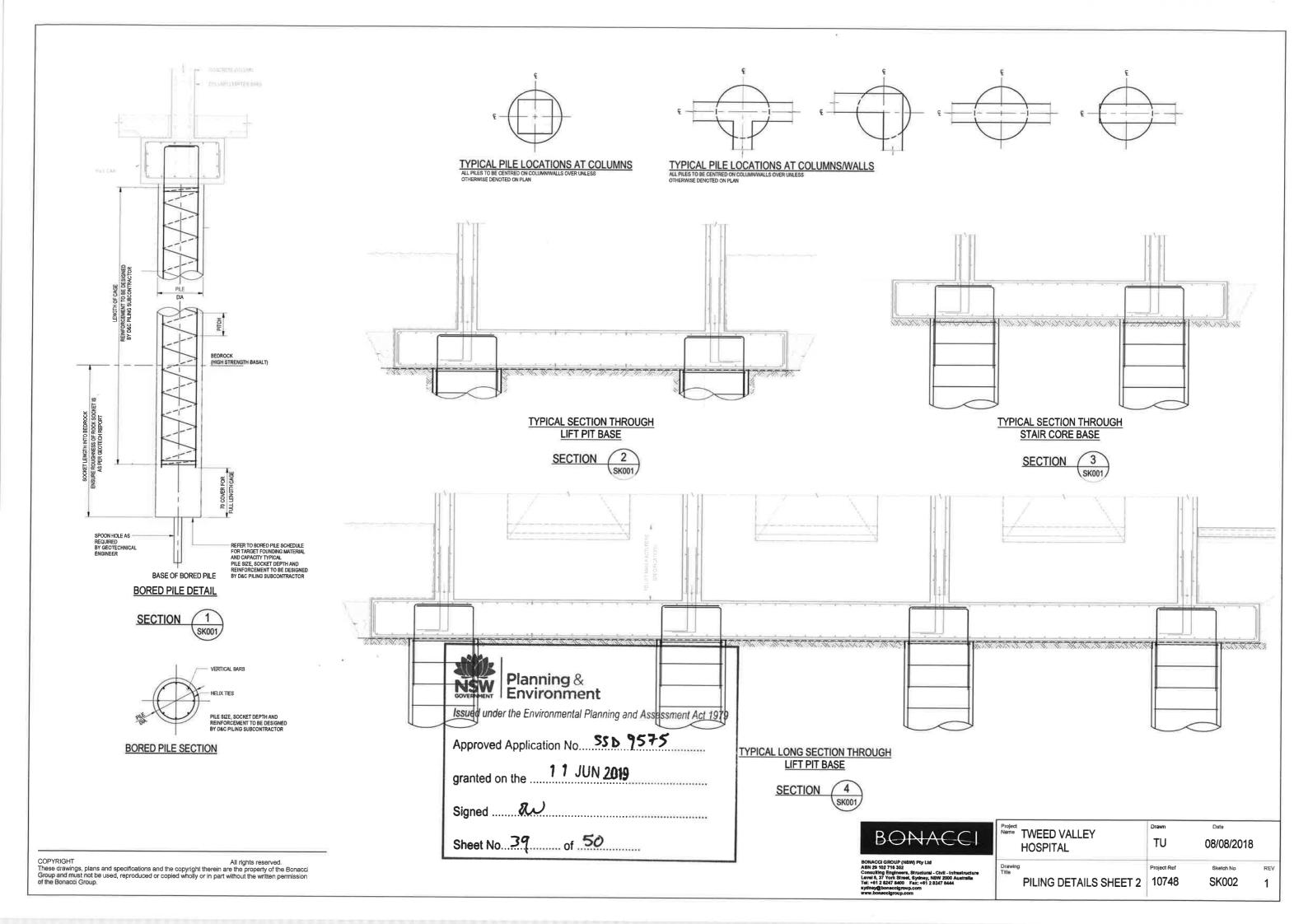
BONACCI GROUP (NSW) Pty Ltd ABN 28 102 716 352 Consulting Engineers, Structural - Civil - Infrastructi Level 6, 37 York Street, Sydney, NSW 2000 Australia Tel: +61 2 8247 8400 Fax: +61 2 8247 8444 sydney@bonaccigroup.com

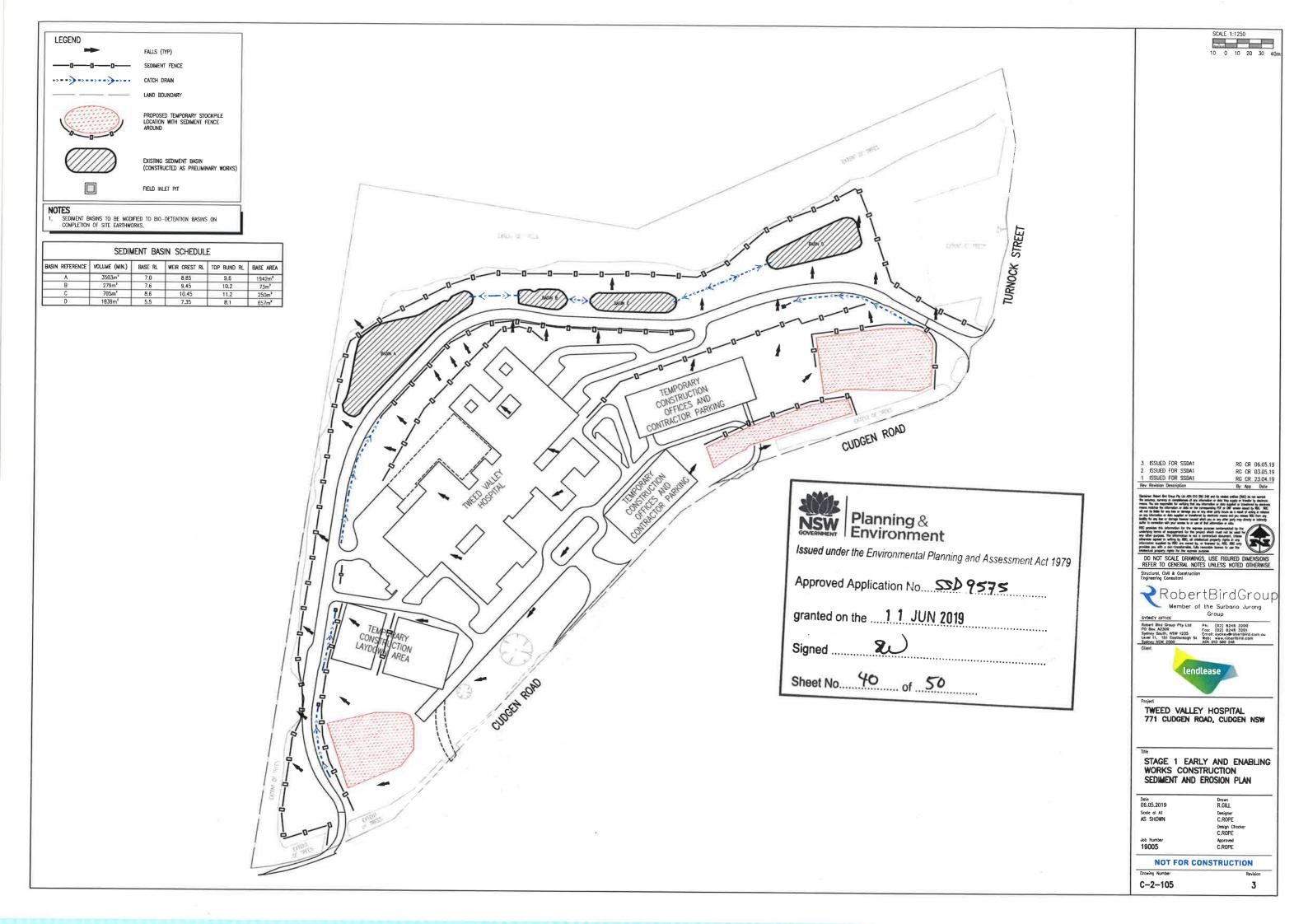
Project TWEED VALLEY HOSPITAL	Drawn TU	08/08/2018	
Drawing Tale PILING DETAILS SHEET 1	Project Ref	SKelch No	REV

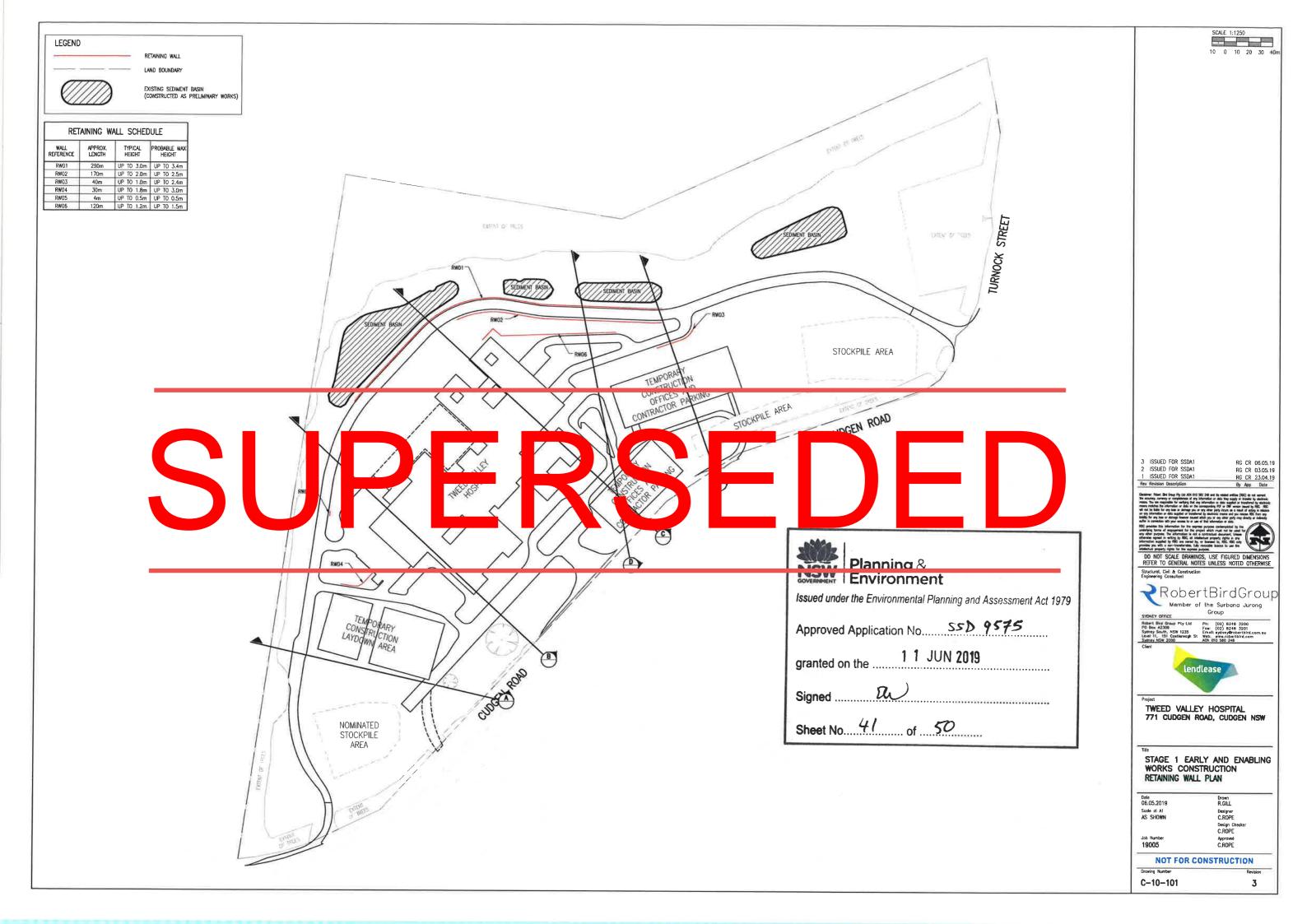
COPYRIGHT

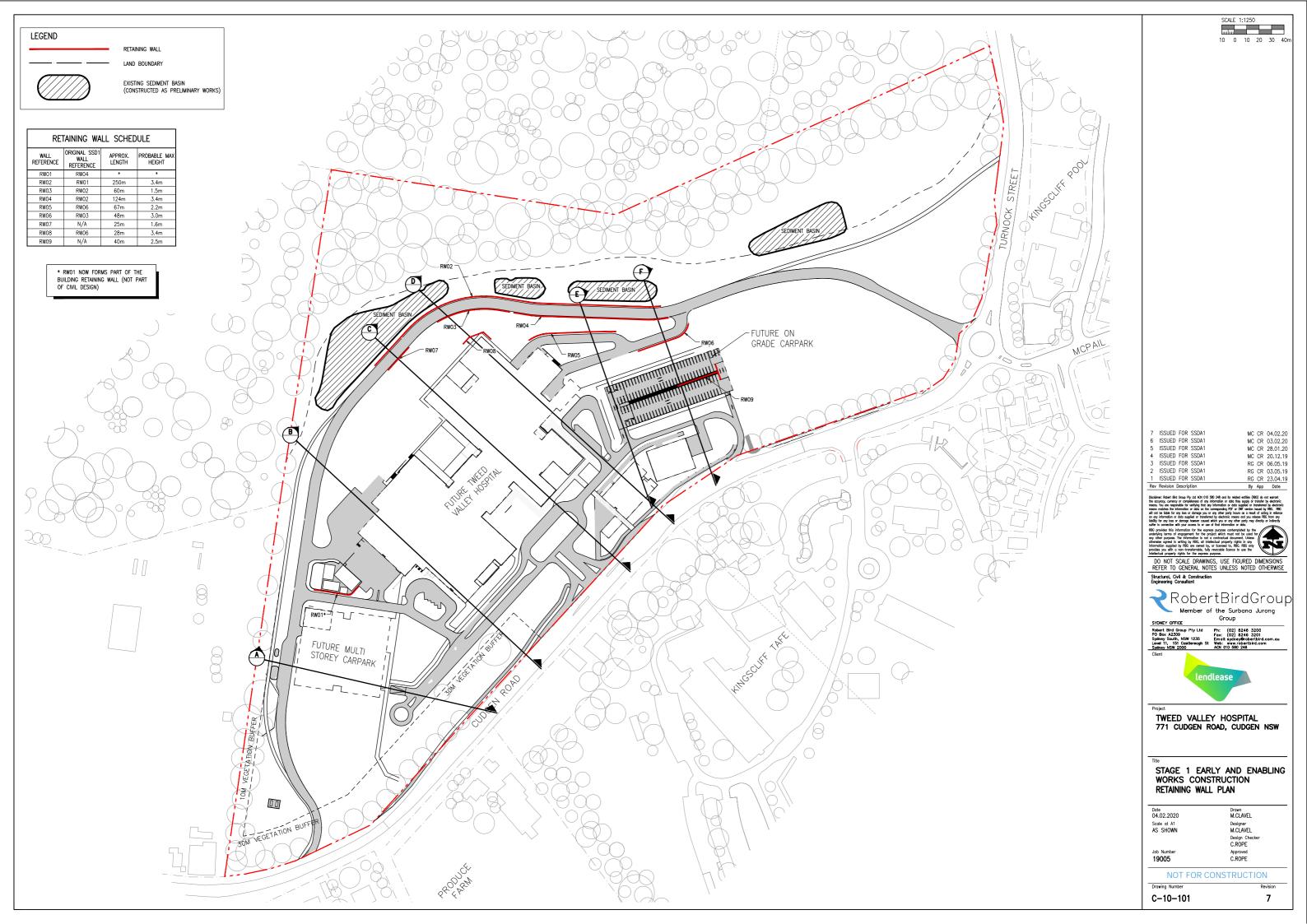
All rights reserved.

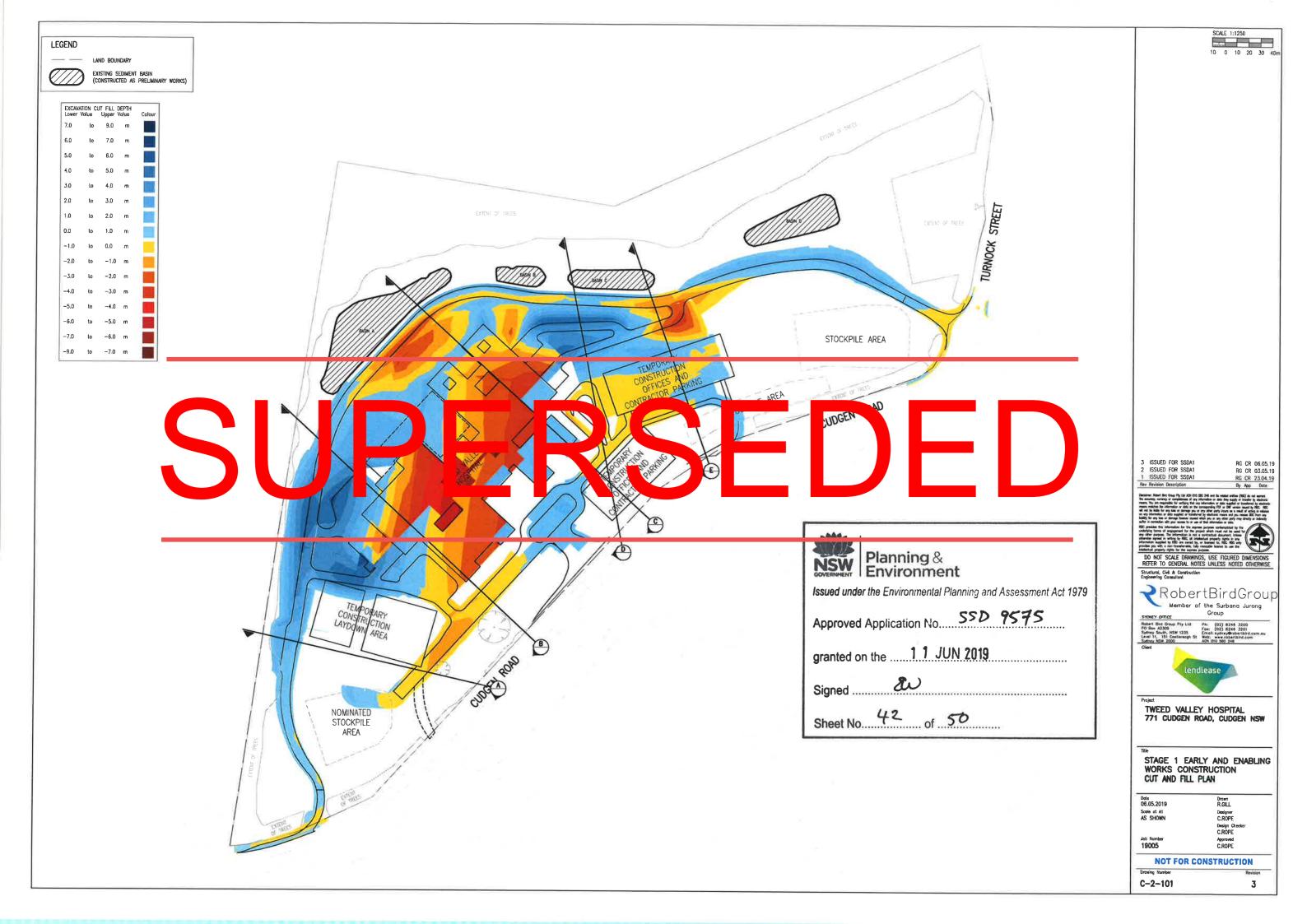
These drawings, plans and specifications and the copyright therein are the property of the Bonacci
Group and must not be used, reproduced or copied wholly or in part without the written permission
of the Bonacci Group.

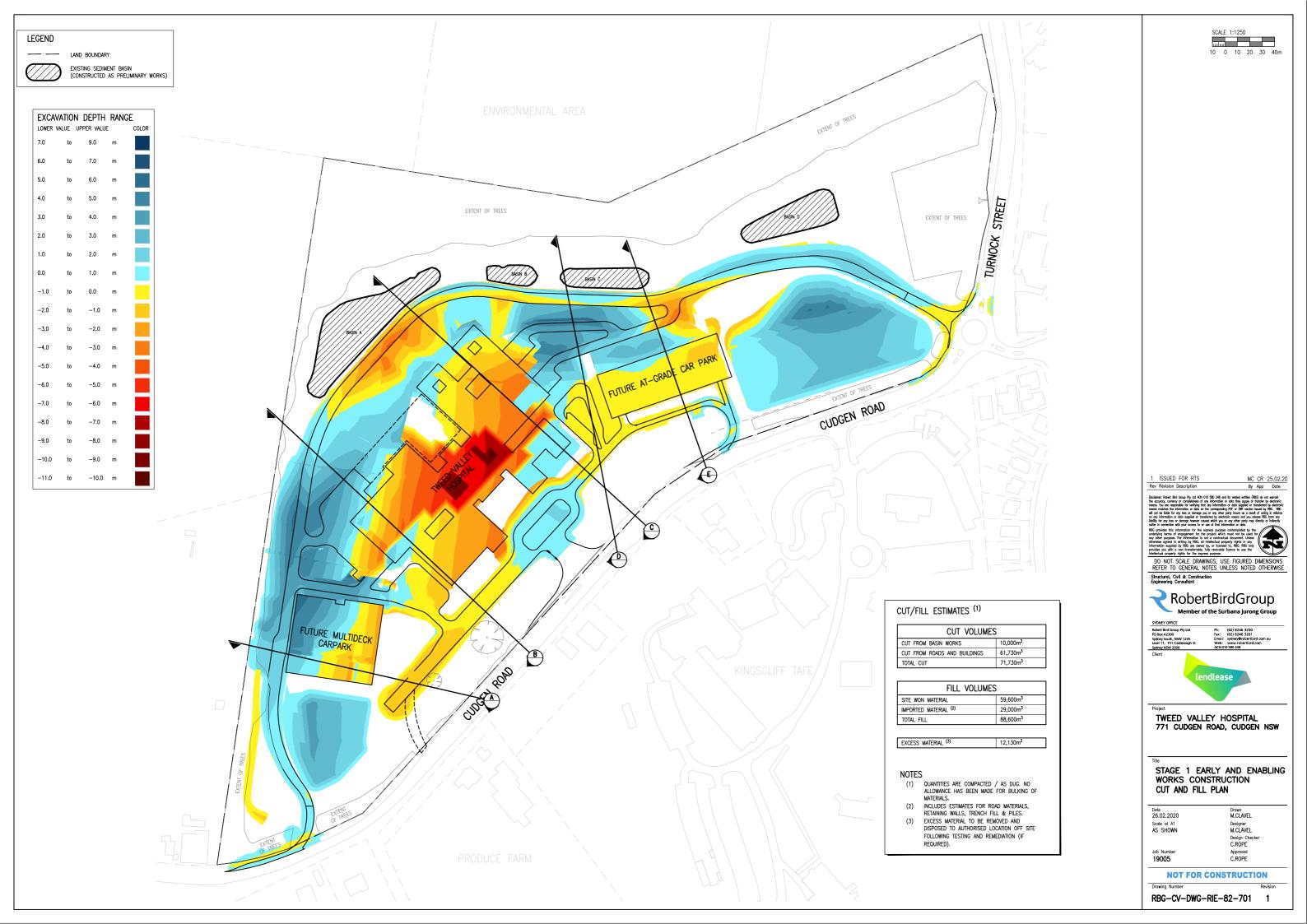


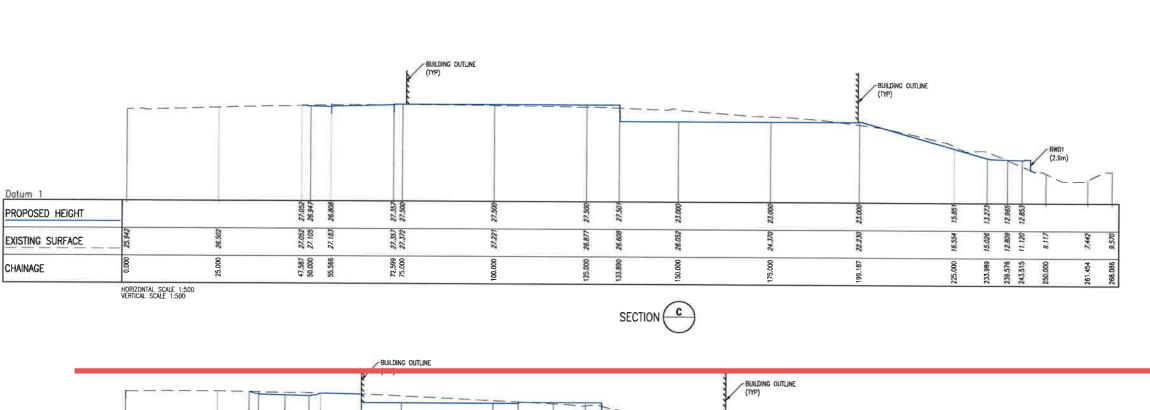


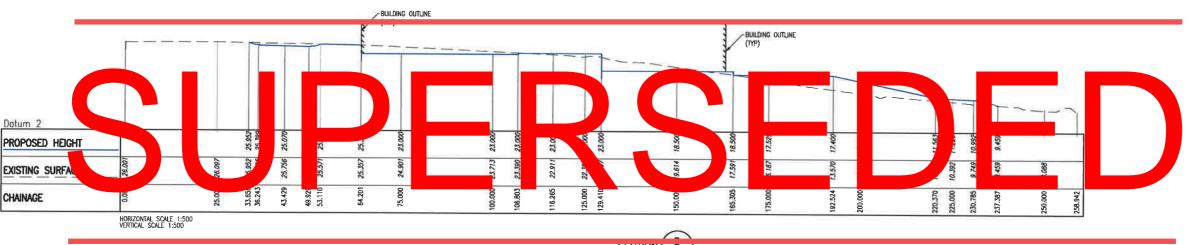












Datum 7 15.842 PROPOSED HEIGHT EXISTING SURFACE 36.839 48,021 53,500 56,544 147.852 CHAINAGE HORIZONTAL SCALE 1:500 VERTICAL SCALE 1:500

SECTION



Issued under the Environmental Planning and Assessment Act 1979

Approved Application No. SSD 9575

granted on the 11 JUN 2019

Signed

Sheet No... 43 of 50

2 ISSUED FOR SSDA1 ISSUED FOR SSDA1

RG CR 03.05.19 RG CR 23.04.19 By App Date

Continue March for the Mr 100 50 50 and for rotate rotation (MC) are at march the country, covering or designation of the plant of the

Structural, Civil & Construction Engineering Consultant

RobertBirdGroup

Member of the Surbana Jurong
Group

SYDNEY OFFICE



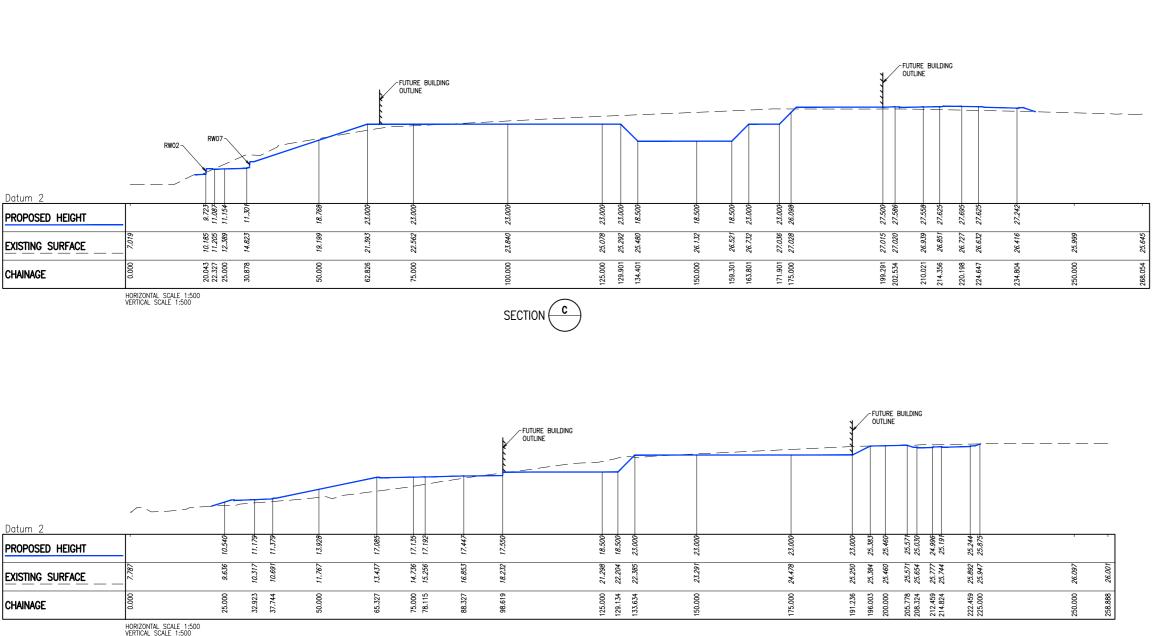
TWEED VALLEY HOSPITAL
771 CUDGEN ROAD, CUDGEN NSW

STAGE 1 EARLY AND ENABLING WORKS CONSTRUCTION LONGITUDINAL SECTION — SHEET 1 OF 2

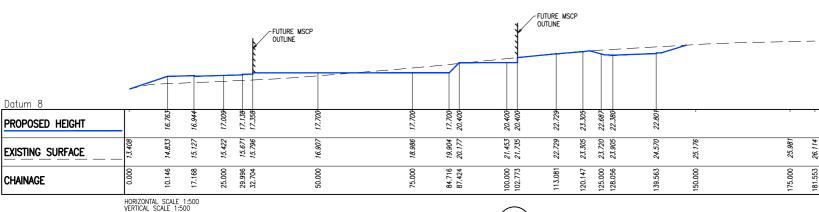
Date 03.05.2019 Scale at A1 AS SHOWN Designer C.ROPE Design Checke C.ROPE Job Number Approved C.ROPE

NOT FOR CONSTRUCTION

C-2-120



SECTION (B)



Iteldemic Ribbert Bird Orac Phy Ltd ANI (10) 590 248 and its related entitles (1901) do not warrout be accuracy, conversely or completeness of any information or data they supply or branker by electrodic more manufacture. It is not to the converse or completeness of any information or data they supply or branker by electrodic more manufacture the information or data on the corresponding PFor DBF various has been been supply or the converse or any or section piles or the converse or any information or data supplies or brancher by electrodic more or supply sections of the converse or any information or data supplies or brancher by electrodic more or supply sections of the first in conversion supply section or supplies of the section of the information or the express purpose contempoted by the conversion or supplies that in conversion self-group without hand not be used for any other purpose. The information is not a controlated document. Unless structured in the conversion of the section of the sec

MC CR 25.02.20 By App Date

Structural, Civil & Construction Engineering Consultant

RobertBirdGroup

Member of the Surbana Jurong

SYDNEY OFFICE

1 ISSUED FOR RTS

Robert Bird Group Pty Ltd P0 Box A2309 Fox: (02) 8246 3200 Fox: (0

lendlease

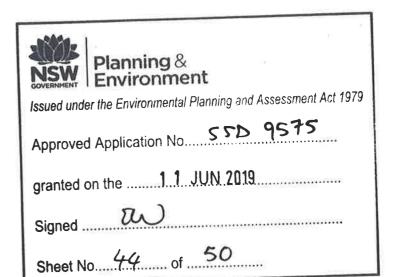
TWEED VALLEY HOSPITAL
771 CUDGEN ROAD, CUDGEN NSW

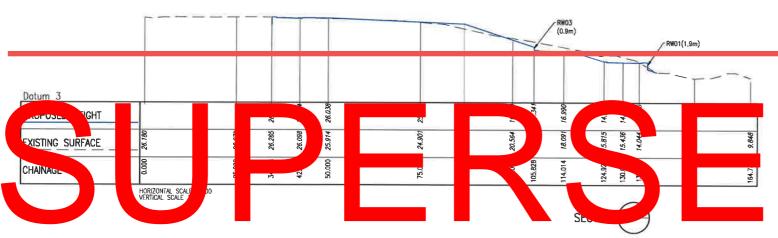
STAGE 1 EARLY AND ENABLING WORKS CONSTRUCTION LONGITUDINAL SECTION -SHEET 1 OF 2

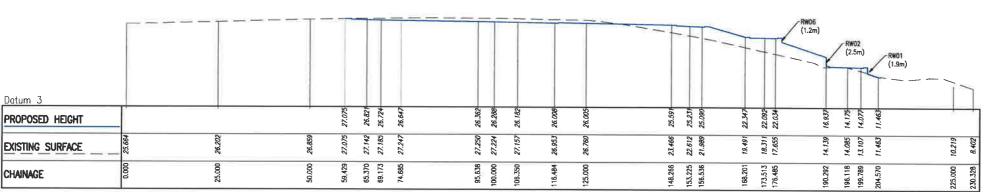
Drawn M.CLAVEL Date 25.02.2020 Scale at A1 AS SHOWN Designer M.CLAVEL Design Checker C.ROPE Job Number 19005

NOT FOR CONSTRUCTION

RBG-CV-DWG-RIE-82-721 1







HORIZONTAL SCALE 1:500 VERTICAL SCALE 1:500

SECTION D

2 ISSUED FOR SSDA1 1 ISSUED FOR SSDA1 REC product the strendship for the segme purpose conventional by the control of t RobertBirdGroup

Member of the Surbana Jurong
Group

SYDNEY OFFICE lendlease

TWEED VALLEY HOSPITAL
771 CUDGEN ROAD, CUDGEN NSW

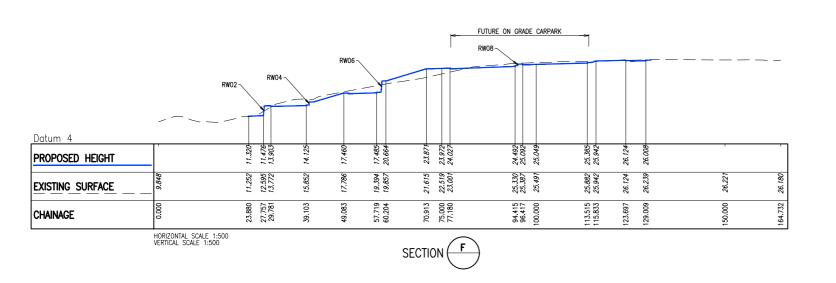
STAGE 1 EARLY AND ENABLING WORKS CONSTRUCTION LONGITUDINAL SECTION -SHEET 2 OF 2

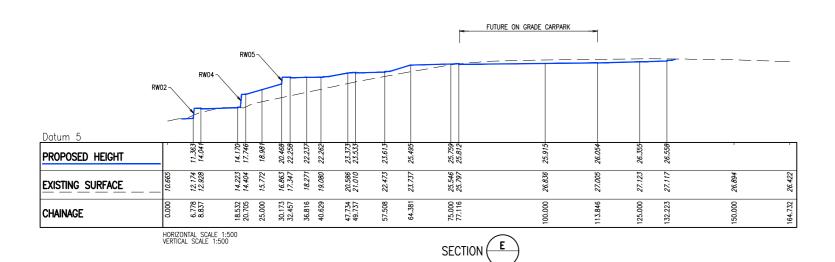
RG CR 23.04.19 By App Date

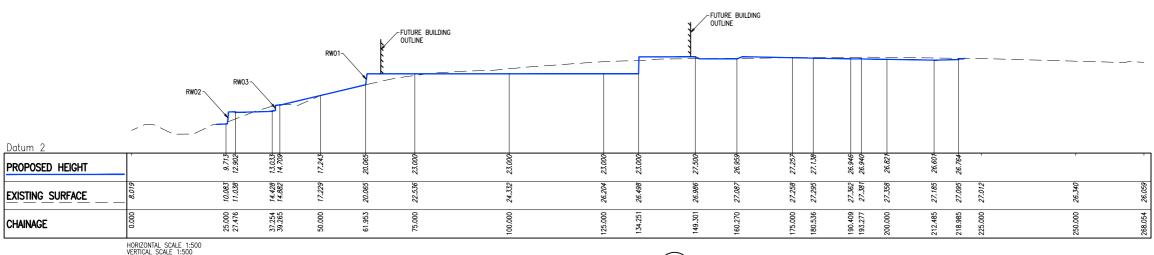
Date 03.05.2019 Drown R.GILL Scale at A1 AS SHOWN Designer C.ROPE Design Checker C.ROPE Job Number 19005

NOT FOR CONSTRUCTION

C-2-121







SECTION

1 ISSUED FOR RTS MC CR 25.02.20

Rev Revision Description By App Date

Dictation: Read that Good by It is AN 001 595 284 and in related efficies (RS) 45 and several the course, namely a completion and primediating of the people of separation revocal. To an energe of the verbing that a primediating of the people of the peop

STAGE 1 EARLY AND ENABLING WORKS CONSTRUCTION LONGITUDINAL SECTION — SHEET 2 OF 2

TWEED VALLEY HOSPITAL
771 CUDGEN ROAD, CUDGEN NSW

| Date | Drawn | 25.02.2020 | M.CLAVEL | Scale at A1 | Designer | AS SHOWN | M.CLAVEL | Design Checker | C.ROPE | Job Number | Approved | 19005 | C.ROPE |

NOT FOR CONSTRUCTION

awing Number Revisio

RBG-CV-DWG-RIE-82-722 1



3 ISSUED FOR SSDA1
2 ISSUED FOR SSDA1
1 ISSUED FOR SSDA1
Rev Revision Description
Descript

DO NOT SCALE DRAWINGS, USE FIGURED DIMENSION REFER TO GENERAL NOTES UNLESS NOTED OTHERWIS

RG CR 06.05.19 RG CR 03.05.19 RG CR 23.04.19 By App Date

RobertBirdGroup

Member of the Surbana Jurong

SYDNEY OFFICE

Robert Bird Group Pty Ltd Pt
PO Box A2309 Sydney South, NSW 1235 E
Level 11, 151 Costlereogh St W

Out Pty Ltd Ph: (02) 8246 3200 Fax: (02) 8246 3201 Fax: (02) 8246 3201 Continency St. Web: www.nhethird.com.



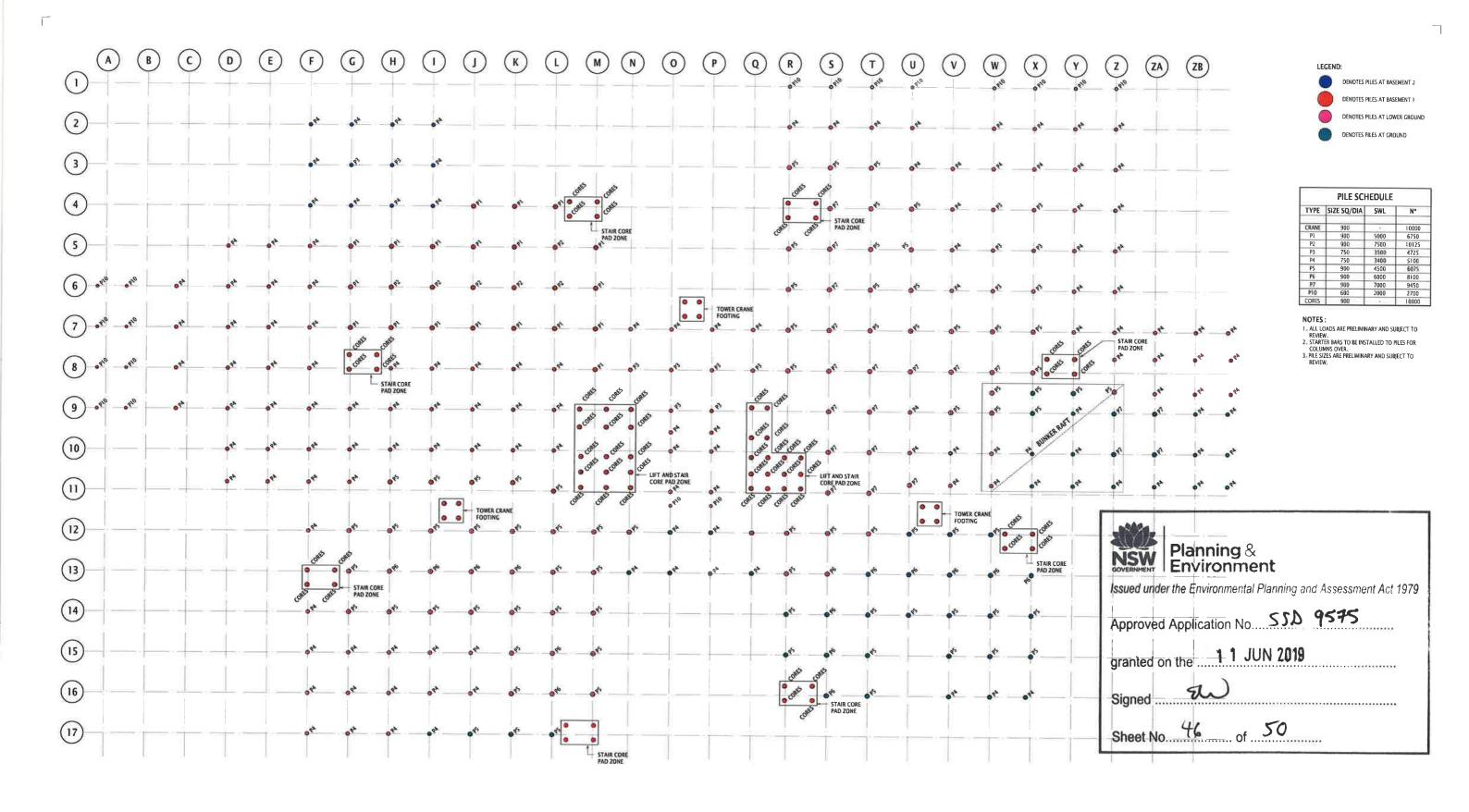
Project

TWEED VALLEY HOSPITAL
771 CUDGEN ROAD, CUDGEN NSW

STAGE 1 EARLY AND ENABLING WORKS CONSTRUCTION STORMWATER MANAGEMENT PLAN

NOT FOR CONSTRUCTION

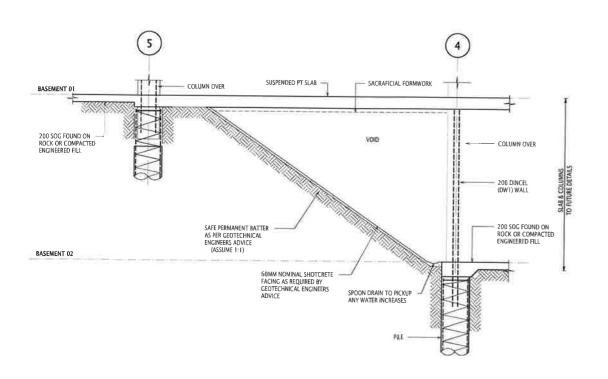
Drawing Number Revision
C-6-150 3



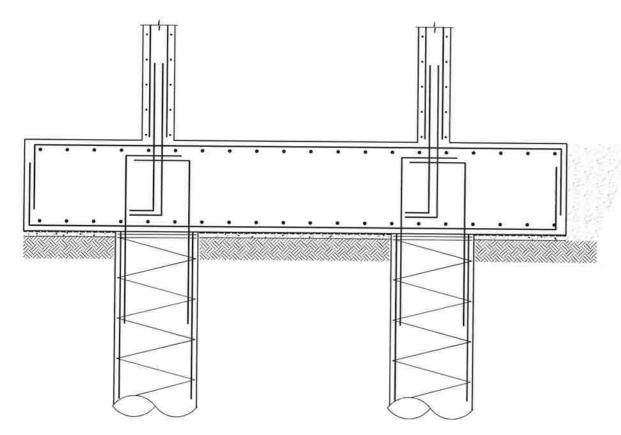


Rev Revision Description	Ry App Date	Rev. Revision Description	By App Date	1883 It Is It It It It It It						
ISSUED FOR SSDA1	KP SH 02.05.19		7.09	Oncloses Asken for dayon by 1014 AD 175 AE 24 and 15 school and 150 (1504) As in second 150 and included a completion. Asked for the residence of any fine for the regions asked from the completion and th	Streturd, Coil & Construction Engineering Consultant Robert Bird Group Member of the Surbana Jurong Group SYDNEY OFFICE	Client	PILING LAYOUT	Scale at A1 1:350 Date Feb 2019	Drawn K.PETROVSKI Designer R.RAMACHANDRA	Design Checker S.HAWDON Approved M.HARDING Job Number 19005
				DO NOT SCALE DRAWINGS, USE FIGURED DIMENSIONS	Robert Bird Group Pty Ltd Ph (02) 8246 3200 PO Box A2309 Fax: (02) 8246 3201 Sydney South, NSW 1235 Emili sydney@robertbird.com.au		TWEED VALLEY HOSPITAL	NOT F	FOR CONSTRUC	TION
				REFER TO GENERAL NOTES UNLESS NOTED OTHERWISE	Sydney NSW 2000 ADV 010 340 248		1	SK02-00		Pl

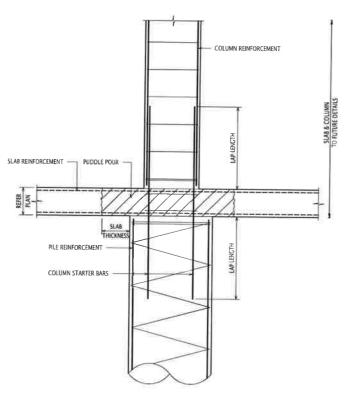




TYPICAL DETAIL FOR RETAINING WALL BUILDING UNDER CROFT (DW1)
SCALE 1:50



TYPICAL STAIR CORE BASE

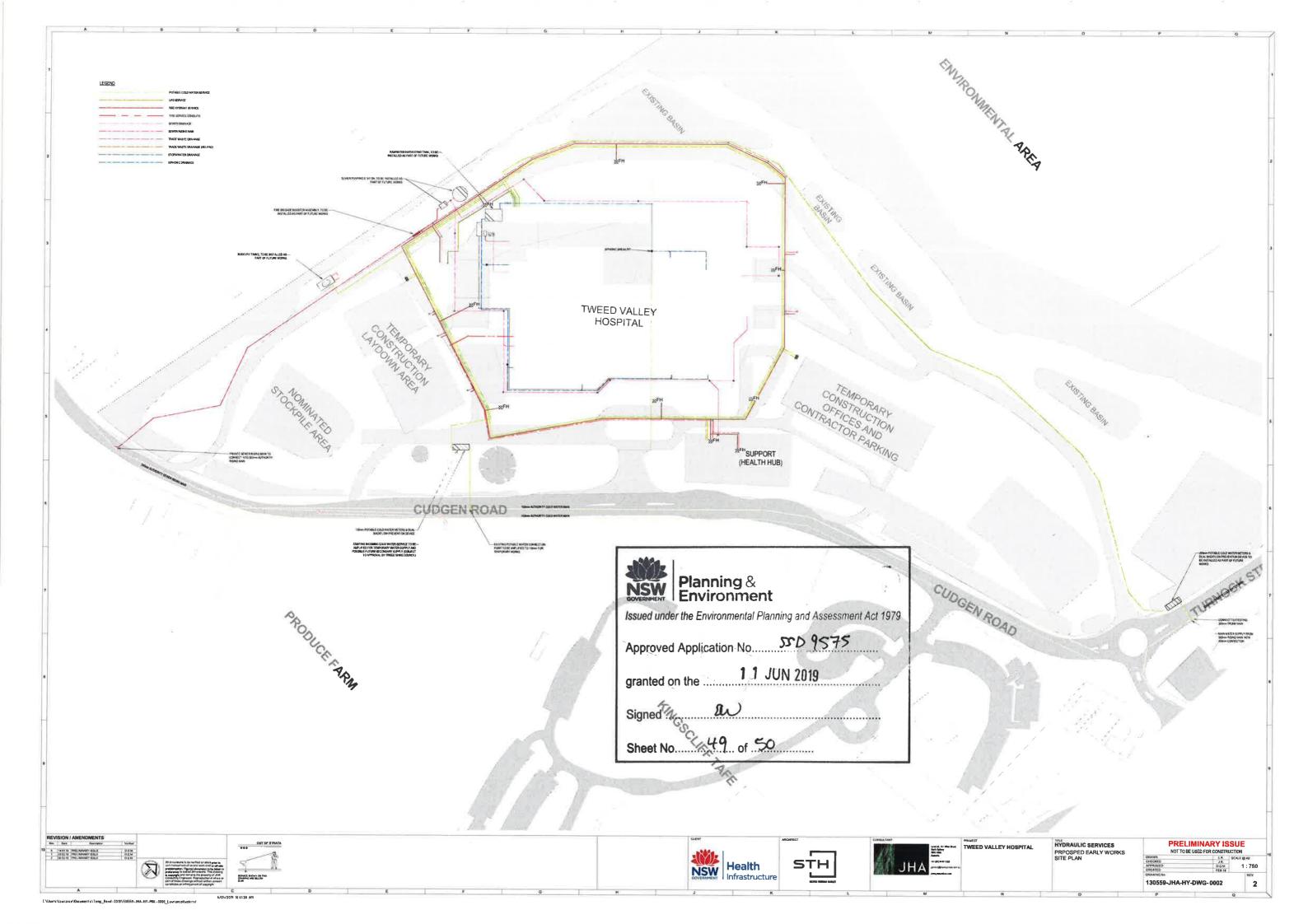


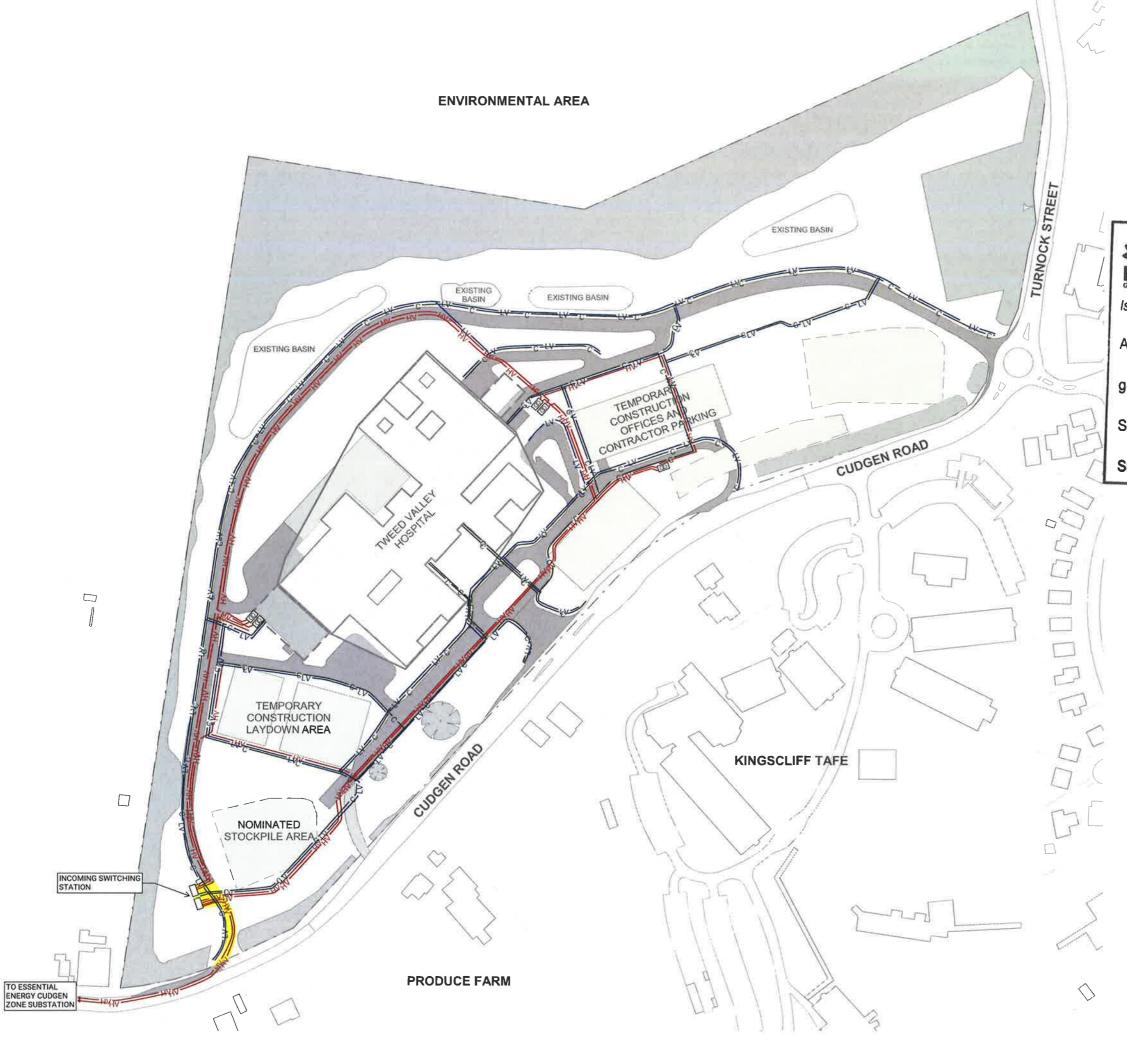
TYPICAL PILE/COLUMN REINFORCEMENT DETAIL SCALE 1:20

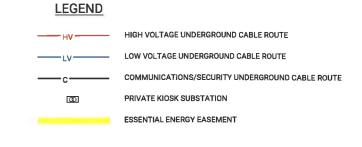
Planning & Environment	
Issued under the Environmental Planning and As	sessment Act 1979
Approved Application No 55D 9	
granted on the1_1_JUN_2019	***************************************
Signed	
Sheet No 48 of 50	

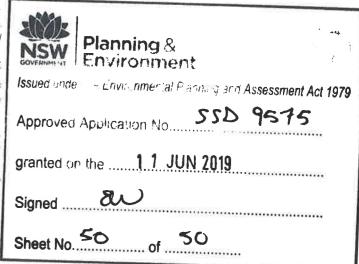
NOTE: ALL FOOTING CONCRETE TO BE 32 MPa MINIMUM U.N.O

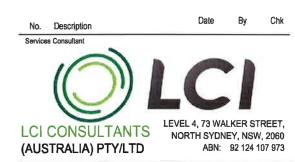
ev Revision Description	By App Date	Rev Revision Description	By App Date	1 30A3 F D D D D D D D						
1 ISSUED FOR SSDA?	KP SH 06.05-19			Activity, coming is explained and along information of data the plugify or tracking information of the set of common facility of the set of proceeding in a common facility of the set of proceeding in a common facility of the set of proceeding in a common facility of the set of proceeding in a common facility of the set of the se	Robert Bird Group Member of the Surbana Jurong Group	Chest	TYPICAL SECTIONS SHEET 1	Scale at A1 1:20, 1:50 Date Feb 2019	Drawn De ligner	Design Checks Approved Job Number 19005
				Interestination togethed by MED or seasoned to, and instantion, MED MED only provided by MED on the Provided by MED of the Provided by MED on the Provided by ME			TWEED VALLEY HOSPITAL	NOT Drawing Number	FOR CONSTRU	
				KEPER TO GENERAL MOTES UNLESS NOTED OTHERWISE	Sydney NSW 2000 ACH HID 200 248		I	SK11-11		Pl











TWEED VALLEY HOSPITAL

Drawing Title
SSD STAGE 1 DA
ELECTRICAL AND COMMUNICATIONS

PRELIMINARY (2019-05-06)	
Scale @	
1:2500@A3	
Project No.	
18202	
Drawing No.	Re
LCI-EL-DWG-SSI-1000001	P