Appendix F: Revised Traffic Queuing Procedure and Traffic Impact Assessment



# <u>Traffic Stacking and Queuing Procedure Version 2</u> <u>September 2020</u>

This procedure is for the Stacking and Queuing of oncoming Traffic into Bulk Recovery Solutions Pty Ltd (BRS) located at 16 Kerr Road, Ingleburn and it must be adhered to by all truck drivers transporting materials from and/or to the BRS site.

The BRS site is not open for the public in the meaning that no transport from and/or to the site will be accepted on an ad-hoc basis. On the contrary, all jobs are pre-booked through the operations office in coordination with the weighbridge operator prior to arrival to the site for tipping to assist in scheduling and avoid traffic congestion onsite at Kerr Rd.

Despite the pre-booking of all jobs, all drivers must call the weighbridge operator prior to entering the Ingleburn Industrial Estate. This will give the yard staff ample opportunity to provide feedback on whether there is room in the yard for an extra truck.

Based on this practice, there should be no queuing or stacking anywhere within or outside the site boundaries and the traffic flow should be sooth. However, if for unforeseen events such as breakdowns (trucks, machinery or equipment), trucks will be diverted to other lawfully licensed facilities that can accept such wastes.

If the yard is full and arriving trucks cannot be accommodated, truck drivers must wait for further instruction from the Weighbridge operator as to when they can come into the site. Drivers are to be on UHF Channel 10.

Truck drivers will be informed to avoid parking their vehicles in any of the streets in the vicinity of the BRS site and preferable in dedicated trucks parking/stopping bays to assist in smooth traffic flow and avoid traffic congestion within the Ingleburn Industrial Estate.

Note: A copy of this procedure will be sent to all parties as part of booking the jobs with BRS.

Location of BRS Facility within the Ingleburn Industrial Estate





RESOURCE RECOVERY FACILITY EXPANSION

LOT 16 DP 717203 16 KERR ROAD, INGLEBURN

PREPARED FOR: BULK RECOVERY SOLUTIONS

**AMENDED OCTOBER 2020** 



REF: 17/164

TRAFFIC IMPACT ASSESSMENT

RESOURCE RECOVERY FACILITY LOT 16 DP 717203 16 KERR ROAD, INGLEBURN BULK RECOVERY SOLUTIONS

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#### **QUALITY ASSURANCE**

This document has been prepared, checked and released in accordance with the Quality Control Standards established by Intersect Traffic Pty Ltd.

Issue	Date	Description	Ву
Α	03/07/18	Draft	JG
В	04/07/18	Edit	JG
С	16/07/18	Final Proof	JG
D	16/07/18	Approved	JG
E	26/11/18	Batching Plant added / Approved	JG
F	06/11/19	Response to Submissions / Approved	JG
G	05/12/19	Amended Traffic generation / Approved	JG
Н	10/10/20	Council comments / Approved	JG
I	16/10/20	Client amendments / approved	JG

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This document has been authorised by

Date: - 16th October 2020

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Table 6 - Lancaster Street / Aero Road roundabout -

- Sidra Modelling Results Summary 16

Sidra Modelling Results Summary



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## 1. INTRODUCTION

Intersect Traffic Pty Ltd has been engaged by KDC Pty Ltd to undertake a traffic impact assessment for an expansion of an existing resource recovery facility on Lot 16 DP 717203, 16 Kerr Road, Ingleburn. The existing resource recovery facility includes a concrete batching plant and is located within the existing Ingleburn Industrial area. Development Plans are provided in *Attachment A*. Note the area designated SB15 on the plans is a tip and spread area (this was omitted from the plans).

This traffic assessment is required to support a development application to the NSW Department of Planning and Environment and allow the Department, Campbelltown City Council and NSW Roads and Maritime Services (NSW RMS) officers to assess the traffic related impacts associated with the development. It is noted that the development does not propose the parking of heavy vehicles on any public roads in the area prior to or after being processed through the facility.

This report presents the findings of the traffic impact assessment and includes the following:

- An outline of the existing situation near the site.
- An assessment of the traffic impacts of the proposed development including the predicted traffic generation and its impact on existing road and intersection capacities.
- Reviews the on-site parking provided within the proposed development and assesses it against Council and Australian Standards requirements.
- Presentation of conclusions and recommendations.

This assessment has been undertaken with reference to the RTA's Guide to Traffic Generating Developments (2002), Austroads Guide to Road Design – Part 4A Unsignalised and signalised intersections (2010), latest Australian Standards AS2890.1 & 2 – Parking Facilities – Part 1 – Off street car parking and Part 2 – Commercial vehicle facilities and Campbelltown City Council requirements.

In respect of the SEARS issued for this project dated 27<sup>th</sup> September 2017 the following traffic and transport measures are addressed as follows;

- 1. Traffic generated by the development **Section 2.5**;
- 2. Traffic Impacts including Sidra Intersection modelling **Section 3**:
- 3. Road Upgrades / New Infrastructure **Section 3** none required:
- 4. Road Pavement Impacts Visual Inspection / Assessment only Section 2.3; and
- 5. Public Transport Accessibility Section 3.5.



## 2. DEVELOPMENT PROPOSAL

#### 2.1 Site Location

Kerr Road is an industrial standard cul-de-sac within the Ingleburn Industrial area located approximately 1.2 km's east of the Hume Motorway and 9 km's north-east of the Campbelltown CBD area. Access to and from the Hume Highway for origin / destinations to the north is via Brooks Road, Williamson Road, Henderson Road, Lancaster Street and Aero Road to Kerr Road while access for origins to the south would be via Campbelltown Road, Williamson Road, Henderson Road, Lancaster Street and Aero Road to Kerr Road. *Figure 1* below shows the site location while *Photograph 1* shows the existing development on the site.

The surrounding area is made of up industrial standard roads with kerb and gutter and longitudinal drainage constructed to a suitable standard for heavy vehicle use. High standard intersection control in the form of roundabout controls all the existing intersections on the likely haulage routes to the site except at the Aero Road / Kerr Road intersection which is a give way-controlled priority T-intersection. *Photograph 2* below shows the roundabout at the Henderson Road / Lancaster Road intersection which provides the main connection to the local collector road network from the site.



Figure 1 – Site Location

The site is titled Lot 16 in DP 717203 and addressed as 16 Kerr Road, Ingleburn. It has a total area of approximately 12,849 m<sup>2</sup> and is zoned IN1 – General Industrial pursuant to the requirements of the Campbelltown LEP (2015).





Photograph 1 – Existing site development and vehicular access.



Photograph 2 – Henderson Road / Lancaster Road roundabout.



#### 2.2 Development Proposal

The proposed development concept involves the following;

- Expansion of the existing Waste Management Facility / Resource Recovery Facility on the site with a capacity to cater for 225,000 tonnes per annum of waste (100,000 tonnes per year solid waste and 125,000 tonnes per year liquid waste) and potentially provide for 8,000 tonnes (4,500 tonnes solid waste and 3,500 tonnes of liquid waste on the site at any time.
- An upgraded concrete batching plant with an annual capacity of 50,000 tonnes per annum.

It is understood no additional site infrastructure is proposed and the development seeks approval for additional waste to be processed and recycled on site only. It should also be noted that only 20 % of the liquid waste is removed from site with the remaining liquid waste being recycled in on-site operations e.g. wheel washes and landscaping or disposed of through the sewerage system under licence.

The current site operator has advised that the existing production from the Waste Management Facility on the site involves the processing of approximately 30,000 tonnes of solid and 'muddy' waste per year (with storage of 5,000 tonnes being 1,900 tonnes of solid waste and 3,100 tonnes of liquid waste) which generally results in the generation of between a maximum three (3) truck and dog (28 tonne capacity) solid waste loads and eleven (11) liquid vacuum trucks (7 tonne trucks or 16 tonne tankers). Daily processing usually involves a combination of solid and liquid waste vehicles. A detailed breakdown of traffic and trip generation is provided within **Sections 2.5 and 2.6** below.

Advice from the operator is that the peak hours for truck movements from the site are 7 am - 9 am and 4 pm to 6 pm with a total of up to 4 trucks arriving in these peaks which averaged out would be 1 delivery per hour. It is noted however that as part of this proposal the number of vehicles delivering waste during non-peak periods will increase markedly and there will be little difference between the peak hour and off-peak hour deliveries.

#### 2.3 Existing Road Network

#### 2.3.1 Campbelltown Road

Campbelltown Road under a functional road hierarchy is a sub-arterial road that not only connects the Campbelltown area to the Liverpool area but also connects the Ingleburn Industrial area to the arterial road network (Hume Motorway) for traffic with an origin / destination to the south. Near the site it is a high standard two-lane two-way sealed rural road with 3 to 3.5 metre lane widths and variable width sealed shoulders (up to 4.5 metres) wide which are also line marked as on-road cycleways. It is under the care and control of NSW RMS and a 70 km/h speed zone exists through the area. At the time of inspection, Campbelltown Road was found to be in good condition.

#### 2.3.2 Brooks Road

Brooks Road under a functional road hierarchy is a local collector road that connects the Ingleburn Industrial area to the arterial road network (Hume Motorway) for traffic with an origin / destination to the north. Brooks Road operates as the on and off-ramp for the Hume Motorway and near the site it is generally a four-lane two-way sealed urban road with kerb and gutter and additional turning lanes near intersections. Lane widths are in the order of 3.5 metres and on inspection Brooks Road was found to be in good condition as evidenced in **Photograph 3** below. It is under the care and control of Campbelltown City Council and a 60 km/h speed zone exists through the area.



#### 2.3.3 Williamson Road

Williamson Road under a functional road hierarchy performs the function of a local collector road and the main collector road through the Ingleburn Industrial area. It is a dual carriageway sealed urban road with kerb and gutter and a raised and vegetated wide central median and two travel lanes in each direction. Indented parking areas are provided within the central median with no parking evidenced in the outer lanes allowing two travel lanes per direction. Lane widths were found to be in the order of 3.1 to 3.5 metres wide and a 50 km/hr speed zoning would apply to the road. The road would also be under the care and control of Campbelltown City Council and at the time of inspection Williamson Road was found to be in good condition as evidenced in **Photograph 4** below. Williamson Road connects to Brooks Road via a 2-lane roundabout.

#### 2.3.4 Henderson Road

Henderson Road under a functional road hierarchy performs the function of a local collector road in the Ingleburn Industrial area. It is a four lane two way sealed urban road with kerb and gutter and a raised concrete central median. With no parking evidenced in the outer lanes the road contained two travel lanes per direction. Lane widths were found to be in the order of 3.1 to 3.5 metres wide and a 50 km/hr speed zoning would apply to the road. The road would be under the care and control of Campbelltown City Council and at the time of inspection Henderson Road was found to be in good condition as evidenced in *Photograph 5* below. Henderson Road connects to Williamson Road via a two-lane roundabout.



Photograph 3 – Brooks Road near Williamson Road

#### 2.3.5 Lancaster Street

Lancaster Street under a functional road hierarchy is a local industrial road within the Ingleburn Industrial area primarily providing vehicular access to properties along its length. Near the site it is a two-lane two-way sealed urban road (12.5 metre carriageway width) with kerb and gutter. On inspection Lancaster Street was found to be in good condition as evidenced in **Photograph 6** below. It is under the care and control of Campbelltown City Council and a 50 km/h speed zone exists through the area. Lancaster Street connects to Henderson Road via a two-lane roundabout.



#### 2.3.3 Aero Road

Aero Road under a functional road hierarchy is a local industrial road within the Ingleburn Industrial area providing vehicular access to properties along its length. Near the site it is a two-lane two-way sealed urban road (12 metre carriageway width) with kerb and gutter. On inspection Aero Road was found to be in fair condition as evidenced in *Photograph 7* below. It is under the care and control of Campbelltown City Council and a 50 km/h speed zone exists through the area. Aero Road connects to Lancaster Street via a single lane roundabout.



Photograph 4 - Williamson Road.

#### 2.3.4 Kerr Road

Kerr Road under a functional road hierarchy is a local industrial cul-de-sac road within the Ingleburn Industrial area providing vehicular access to properties along its length. Near the site it is a two-lane two-way sealed urban road (11 metre carriageway width) with kerb and gutter with a 25-metre radius turning area which includes a central vegetated island. This turning area is suitably for convenient use by all sizes of heavy vehicles. On inspection Kerr Road was found to be in good condition as evidenced in **Photograph 8** below. It is under the care and control of Campbelltown City Council and a 50 km/h speed zone exists through the area. Kerr Road connects to Aero Road via a give way-controlled T-intersection.





Photograph 5 – Henderson Road.



Photograph 6 – Lancaster Street near site.





Photograph 7 – Aero Road near site.



Photograph 8 – Kerr Road near site.



#### 2.4 Alternative Transport Modes

Public transport (buses) in the area are provided by Interline Bus Services with service route 869 Ingleburn to Liverpool via Edmondson Park running past the site along Henderson Street. This route connects Ingleburn Railway Station to Edmondson Park Railway Station and Liverpool Railway Station. Other bus and rail connections at these locations provides access to all the major residential, commercial, retail, health and educational areas near the site. The nearest bus stops to the site are located on Henderson Road near the Lancaster Road roundabout about 200 to 350 metres north west of the site. A bus route extract for Route 869 is provided below in *Figure 2*.

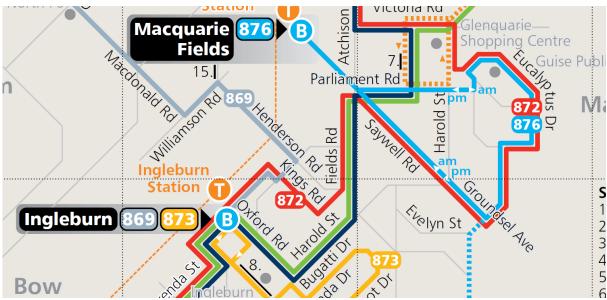


Figure 2 – Interline Bus Services Route 869 – Route Extract

The site is within 900 metres of Ingleburn Railway Station which lies on Sydney Trains Airport and South Line (T8). This is approximately a 20-minute walk from the site and provides access to the Sydney Trains and Regional rail networks.

A suitable concrete pedestrian footpath network exists along the major roads in the area connecting to Ingleburn Railway Station and local bus stops. Near the site a shared pathway exists along Henderson Road while pedestrian footpaths are provided along Lancaster Road to Aero Road and Aero Road from Kerr Road to Ingleburn Railway Station. The only gap in the pedestrian footpath network near the site is along Kerr Road itself where pedestrians are required to use the grass verges or parking lanes for trip making purposes. **Photograph 9** below shows the existing concrete footpath in Henderson Road near the site.

The only observed cycle way near the site is in the form of an off-road shared pathway on Henderson Road from Lancaster Road to Williamson Road as shown below in *Photograph 10*.

This provides only a short section of benefit for cyclists in the area though does provide safe passage through a difficult and dangerous section of the road network for cyclists. In all other area's cyclists are required to share the outside travel lanes with other vehicles. This situation is only suitable for experienced cyclists.





Photograph 9 – Lancaster Road footpath near site.



Photograph 10 – Henderson Road off-road shared pathway near site



#### 2.5 Traffic Generation

Traffic generation data for this assessment report has been determined from the operational details provided by KDC Pty Ltd and assumptions made in relation to operating hours of the facility and truck sizes as described in **Section 2.2** and is based on the existing operations on the site. The key data used for the traffic generation calculations are;

- Waste delivery is 225,000 tonnes per annum;
- Waste removal based on 8,000 tonnes of storage on site (4,500 tonnes solid waste and 3,500 tonnes liquid waste) i.e. 217,000 tonnes per annum.
- Each vehicle load (delivery and removal) represents an inbound and outbound trip that will occur in the same hour.
- The majority of liquid waste is removed either through the local sewer under licence or recycled on-site through the wheel washes and landscaping (only 20 % is removed off site)
- Operating Hours 10 hours per day weekdays and 5 hours on Saturdays.
- Facility is open 50 weeks of the year (Closed Christmas, New Year & Easter)
- Waste delivery provided in different sized trucks nominated in the calculations below.
- Waste removal undertaken using truck and dog combinations or semi-trailers or B-Doubles with an average haulage load of 28 tonnes operated by contractors.
- Staff numbers assumed to be 15 staff including drivers.
- Concrete Agi-trucks carry 15 tonnes of concrete per load (6 m³ capacity).

Therefore, the traffic generation calculations are;

- 1. Solid Waste delivery 100,000 tonnes per annum / 354 working days / 13.86 average hours per day / 28 tonnes per vehicle x 2 trips per vehicle = approximately 2 (1.40) vehicle trips per hour.
- 2. Liquid Waste and Muddy Waste Delivery 125,000 tonnes per annum / 354 working days per annum / 13.86 average hours per day / average 12 tonne per vehicle x 2 trips per vehicle = approximately 5 (4.2) vehicle trips per hour
- 3. Solid waste removal 95,500 tonnes per annum / 354 working days per annum / 13.86 average hours per day / 28 tonnes per vehicle x 2 trips per vehicle = approximately 2 (1.4) vehicle trips per hour.
- 4. Liquid waste removal 24,300 tonnes per annum / 354 working days per annum / 13.86 average hours per day / 28 tonnes per vehicle x 2 trips per vehicle = approximately 1 (0.2) vehicle trip per hour.
- 5. Staff trips Peak Hour considered to be arrival at work (AM) all inbound trips 15 vtph and departure from work (PM) all outbound trips 15 vtph.
- 6. Concrete trucks Peak hour 50000 tonnes/year/ 354 working days per annum / 19 hrs per day / 15 tonne per load = 1 deliveries per hour maximum i.e. 1 inbound and 1 outbound trip. Assume maximum material delivery of 1 per day maximum in non-peak periods.

Therefore, Peak Hour and Daily Trips can be calculated as follows;

Weekday Daily Vehicle Trips =  $(2 + 5 + 2 + 1) \times 13.86 + (1 + 1) \times 19 + 15 \times 2 = 207$  (206.6) vtpd.

AM Peak hour = 7 inbound trips + 3 outbound trips (waste delivery and removal) + 15 inbound (staff)+ 1 inbound and 1 outbound (concrete batching plant) = 27 vtph (19 inbound and 8 outbound).

PM Peak hour = 7 inbound trips + 3 outbound trips (waste delivery and removal) + 15 outbound trips (staff) + 1 inbound + 1 outbound (concrete batching plant) = 27 vtph (8 inbound and 19 outbound).



#### **Existing Traffic**

The current site has a production output of 30,000 tonnes per annum with the concrete batching plant output remaining the same at 50,000 tonnes per year and storage capacity for only 5,000 tonnes (1,900 tonne solid waste and 3,100 tonnes liquid waste). Staff numbers for the existing operation is 7. Therefore the current traffic generation is as follows;

- Solid Waste delivery 10,000 tonnes per annum / 354 working days / 13.86 average hours per day / 28 tonnes per vehicle x 2 trips per vehicle = approximately 1 (0.14) vehicle trips per hour.
- 2. Liquid Waste and Muddy Waste Delivery 20,000 tonnes per annum / 354 working days per annum / 13.86 average hours per day / average 12 tonne per vehicle x 2 trips per vehicle = approximately 1 (0.7) vehicle trips per hour
- 3. Solid waste removal 8,100 tonnes per annum / 354 working days per annum / 13.86 average hours per day / 28 tonnes per vehicle x 2 trips per vehicle = approximately 1 (0.12) vehicle trips per hour.
- 4. Liquid waste removal 3,380 tonnes per annum / 354 working days per annum / 13.86 average hours per day / 28 tonnes per vehicle x 2 trips per vehicle = approximately 1 (0.01) vehicle trip per hour.
- 5. Staff trips Peak Hour considered to be arrival at work (AM) all inbound trips 7 vtph and departure from work (PM) all outbound trips 7 vtph.
- 6. Concrete trucks Peak hour 50000 tonnes/year/ 354 working days per annum / 19 hrs per day / 15 tonne per load = 1 deliveries per hour maximum i.e. 1 inbound and 1 outbound trip. Assume maximum material delivery of 1 per day maximum in non-peak periods.

Therefore, existing Peak Hour and Daily Trips can be calculated as follows;

Weekday Daily Vehicle Trips =  $(1 + 1 + 1 + 1) \times 13.86 + (1 + 1) \times 19 + 7 \times 2 = 108 (107.44) \text{ vtpd}$ .

AM Peak hour = 2 inbound trips + 2 outbound trips (waste delivery and removal) + 7 inbound (staff)+ 1 inbound and 1 outbound (concrete batching plant) = 13 vtph (10 inbound and 3 outbound).

PM Peak hour = 2 inbound trips + 2 outbound trips (waste delivery and removal) + 7 outbound trips (staff) + 1 inbound + 1 outbound (concrete batching plant) = 13 vtph (3 inbound and 10 outbound).

Therefore the additional traffic generated by this development is calculated as;

Weekday Daily Vehicle Trips = 221 -108 = 113 vtpd

AM Peak hour trips = 27 - 13 = 14 vtph (9 inbound and 5 outbound); and

PM Peak hour trips = 27 - 13 = 14 vtph (5 inbound and 9 outbound).

These values have been adopted in this assessment.



#### 2.6 Trip Distribution

In determining the trip distribution for the site, it has been assumed that during the AM and PM peak traffic periods in terms of origin / destination approximately 50 % of trips will have an origin / destination to the north via the Hume Motorway, 40 % of trips will have an origin / destination to the south via Campbelltown Road and 10% of trips will have an origin destination to the east via Henderson Road. The resultant trip distribution on the local road network is therefore as shown in *Figure 3* below.

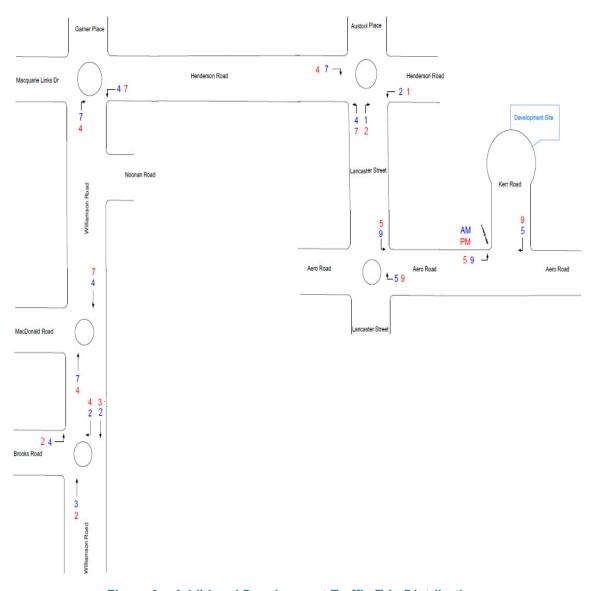


Figure 3 – Additional Development Traffic Trip Distribution



# 3. TRAFFIC IMPACTS AND CONSIDERATIONS

#### 3.1 Mid-block Road Network Capacity

Table 4.3 of the Transport for NSW (TfNSW) publication "RTA's Guide to Traffic Generating Developments" provides some guidance on the capacity of urban roads for a LoS C though the capacity of urban roads is generally determined by intersection capacity. This table is reproduced below.

Table 4.3

Typical mid-block capacities for urban roads with interrupted flow

Type of Road	One-Way Mid-block Lane C	Capacity (pcu/hr)
Median or inner lane:	Divided Road	1,000
Median or inner lane.	Undivided Road	900
	With Adjacent Parking Lane	900
Outer or kerb lane:	Clearway Conditions	900
	Occasional Parked Cars	600
4 Jane undivided:	Occasional Parked Cars	1,500
4 lane undivided:	Clearway Conditions	1,800
4 lane divided:	Clearway Conditions 1,900	

Source: - RTA's Guide to Traffic Generating Developments 2002

In determining the capacity of the road network from this table the following has been considered;

- All roads are two-lane two-way roads except Williamson Road and Henderson Road which effectively operate as four lane two-way roads;
- No marked parking lanes exist on the network.

On this basis the likely mid-block two-way road capacity for Brooks Road, Williamson Road and Henderson Road being 4 lane undivided roads is 3,000 vtph (i.e. 2 x 1,500 vtph) and for Lancaster Street and Aero Road is 1,200 vtph (i.e. 2 x 600 vtph).

Roar Data on behalf of Intersect Traffic undertook traffic counts at the following intersections during the AM and PM peak periods during November 2017. This data is provided in *Attachment B*;

- Brooks Road / Williamson Road roundabout;
- Williamson Road / MacDonald Road roundabout;
- Williamson Road / Henderson Road roundabout;
- Henderson Road / Lancaster Street roundabout; and
- Lancaster Street / Aero Road roundabout.

This data indicates the peak hour traffic volumes on the local road network affected by the development are currently as follows;

- Brooks Road 2,052 vtph in the AM peak and 2,188 vtph in the PM peak.
- ♦ Williamson Road 2,045 vtph in the AM peak and 2,183 vtph in the PM peak.
- ◆ Henderson Road 2,486 vtph in the AM peak and 2,604 vtph in the PM peak
- ◆ Lancaster Street 1,097 vtph in the AM peak and 1,188 vtph in the PM peak; and
- Aero Road 328 vtph in the AM peak and 339 vtph in the PM peak.

The additional traffic from the proposed development would increase these traffic volumes (see *Figure 2*) as follows;



- Brooks Road 6 vtph in both the AM and PM peak hour;
- Williamson Road 11 vtph in both the AM and PM peak hour;
- Henderson Road 11 vtph in both the AM and PM peak hour
- Lancaster Street 14 vtph in both the AM and PM peak hour; and
- Aero Road 14 vtph in both the AM and PM peak hour.

Therefore, in terms of mid-block road network capacity the following assessment as shown in *Table 1* below has been determined by adopting a background traffic growth of 2 % per annum for the next 10 years.

Table 1 – Two-way mid-block capacity assessment

Road	Section	2018		2028		Road	Developme	ent Traffic
		AM (vtph)	PM (vtph)	AM (vtph)	PM (vtph)	Capacity	AM	PM
Brooks Road	West of Williamson Road	2058	2194	2507	2673	3000	6	6
Williamson Road	North of Brooks Road	2056	2194	2504	2672	3000	11	11
Henderson Road	east of Williamson Road	2497	2615	3041	3185	3000	11	11
Lancaster Street	south of Henderson Road	1111	1202	1351	1462	1200	14	14
Aero Road	west of Kerr Road	342	353	414	427	1200	14	14

It is noted from a review of the above table that post development the mid-block traffic volumes on the local road network will remain at a LoS C or D therefore the proposal does not adversely impact on the local road network. However with background traffic growth only both Henderson Road and Lancaster Street will just exceed the capacity threshold of a LoS C and these streets will be operating mid-block with LoS D. In recent times road authorities have accepted that for major sub-arterial, collector and local roads a LoS D is still an acceptable level of service on the road network. Therefore it is concluded that the proposed development does not adversely impact on the mid-block traffic volumes on the local road network. Subject to continued satisfactory intersection performance the development can therefore be supported from a traffic impact perspective.

#### 3.2 Intersection Capacity

To determine the impact of the development on intersection capacity all the roundabout intersections for which traffic volume data was collected have been modelled for the AM and PM peak traffic periods using the Sidra Intersection modelling program. This software package predicts likely delays, queue lengths and thus levels of service that will occur at intersections. Assessment is then based on the level of service requirements of the RMS shown below:

Table 4.2
Level of service criteria for intersections

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way & Stop Signs
Α	< 14	Good operation	Good operation
В	15 to 28	Good with acceptable delays & spare capacity	Acceptable delays & spare capacity
С	29 to 42	Satisfactory	Satisfactory, but accident study required
D	43 to 56	Operating near capacity	Near capacity & accident study required
Е	57 to 70	At capacity; at signals, incidents will cause excessive delays	At capacity, requires other control mode
		Roundabouts require other control mode	

Source: - RTA's Guide to Traffic Generating Developments (2002).



Assumptions made in this modelling are:

- The intersection layout will remain as per current conditions.
- Traffic volumes used in the modelling are as collected by Roar Data on Wednesday 1st November 2017.
- Traffic generated by the development is distributed as per Figure 3.
- Future 2028 traffic growth predicted using a 2.0 % per annum background traffic growth rate.

The results of the modelling are summarised in *Tables 2 - 6* below for 'all vehicles'. The Sidra Movement Summary Tables are provided in *Attachment C.* 

Table 2 – Brooks Road / Williamson Road roundabout - Sidra Modelling Results Summary

Model	Deg. Satn (v/c)	Average Delay (s)	Worst Level of Service	95 % back of queue length (cars)
2018 AM + development	0.820	10.3	Α	10.8
2018 PM + development	0.697	12.1	А	8.9
2028 AM + development	0.939	14.0	Α	20.6
2028 PM + development	0.829	16.6	В	14.8

Table 3 – Williamson Road / MacDonald Road roundabout - Sidra Modelling Results Summary

Model	Deg. Satn (v/c)	Average Delay (s)	Worst Level of Service	95 % back of queue length (cars)
2018 AM + development	0.609	8.0	А	5.9
2018 PM + development	0.560	7.5	А	4.9
2028 AM + development	0.807	9.6	А	8.3
2028 PM + development	0.681	8.5	Α	6.9

Table 4 – Williamson Road / Henderson Road roundabout - Sidra Modelling Results Summary

Model	Deg. Satn (v/c)	Average Delay (s)	Worst Level of Service	95 % back of queue length (cars)
2018 AM + development	0.638	7.6	А	7.5
2018 PM + development	0.764	7.0	А	10.7
2028 AM + development	0.748	8.0	А	9.5
2028 PM + development	0.848	7.2	Α	15.5

Table 5 – Henderson Road / Lancaster Street roundabout - Sidra Modelling Results Summary

Model	Deg. Satn (v/c)	Average Delay (s)	Worst Level of Service	95 % back of queue length (cars)
2018 AM + development	0.562	6.9	А	4.6
2018 PM + development	0.620	8.3	А	5.7
2028 AM + development	0.626	7.4	Α	5.6
2028 PM + development	0.709	9.5	А	8.1

Table 6 – Lancaster Street / Aero Road roundabout - Sidra Modelling Results Summary

Model	Deg. Satn (v/c)	Average Delay (s)	Worst Level of Service	95 % back of queue length (cars)
2018AM + development	0.410	6.1	А	3.0
2018 PM + development	0.432	7.3	Α	3.0
2028 AM + development	0.451	6.2	Α	3.5
2028 PM + development	0.488	7.6	А	3.6



This modelling shows the development has little impact on the operation of the major intersections in the adjoining road network with all intersections continuing to operate satisfactorily post development through to at least 2028. Average delays, LoS and queue lengths remain within the acceptable criteria set by NSW RMS. Therefore, the development does not adversely impact on the efficiency and effectiveness of the local road network.

It should also be noted this assessment is likely to be very conservative as it has not allowed for existing traffic generated by the development. It would be appropriate to discount the additional traffic generated by the new development by the existing traffic generated by the site however ignoring existing traffic results in a robust traffic impact assessment.

#### 3.3 Site Access / Road Upgrading

Post development the site access will service more than 25 car spaces but less than 100 car parks. Under Table 3.1 of Australian Standard *AS2890.1-2004 Parking facilities – Part 1 - Off-street car parking* a car park with between 25 to 100 car parking spaces accessed via a local road providing long term employee parking (Class 1) is required to have a Category 2 access facility. A Category 2 access facility is combined entry / exit access 6 m to 9 metres wide. It is noted the existing access is approximately 10 metres wide therefore compliant with AS2890.1-2004. However, for the type of vehicle using the site the access width will be determined by the swept path analysis for entry and exit to and from the site by B-Double vehicles. Having observed the existing site access, it is considered suitable for the proposed development and will not require upgrading.

Part 6.3 of the Campbelltown (Sustainable City) Development Control Plan (2015) which details car parking and access requirements for Industrial development in the Campbelltown LGA identifies that each industrial site can have only one heavy vehicle entry / exit and may have a second light vehicle entry / exit and must be designed in accordance with Australian Standard AS2890.1-2004 Parking facilities — Part 1 - Off-street car parking and Australian Standard AS2890.2-2002 Parking facilities — Part 2 - Off-street commercial vehicle facilities. The proposed development complies with these requirements.

It is noted from TfNSW restricted vehicle access maps that Campbelltown Road, Williamson Road, Brooks Road, Henderson Road and Lancaster Street are already B-Double approved routes while Aero Road and Kerr Road are approved routes with travel conditions while the Lancaster Street / Aero Road roundabout is a restricted intersection with conditional approval for B-Doubles. It is likely that these conditional approvals already apply to the existing site operations and similar conditions are likely to be placed on the expanded development. Therefore, no nexus would exist for any additional road upgrading conditions to be required by Council for the proposed development.

It is noted that all vehicle movements to and from the site will be contained within the site and no queuing out onto Kerr Road will result from the operation of this development.

#### 3.4 On-site parking and driveway

On-site parking requirements for development in the Campbelltown City Council LGA are contained within the Campbelltown (Sustainable City) Development Control Plan (2015). Part 6 deals with Industrial Development and Section 6.3 details the requirements for car parking and access.

Relevant to the site the requirements for car parking are;

For offices / lunch rooms / storage – 1 space per 35 m²;
For other areas - minimum of 2 spaces per unit;
1 space per 100 m² GFA up to 2,000 m²;
1 space per 250 m² GFA above 2,000 m²; plus
1 space per 300 m² outdoor storage area.



It is argued however that the site car parking requirement based on the operational details for the site would be as follows;

#### Car Parking

- Staff car parking 15 spaces
- Visitor car parking 2 spaces

The site plan shows provision of 22 on-site car parks including 14 stacked spaces. Being mainly staff parking stacked parking is supportably as it can managed on site without impeding any motorists. Therefore there is sufficient on-site car parking to meet the DCP requirements of Campbelltown City Council but more importantly the operational requirements of the development. Note this parking will need to be set out in accordance with the requirements of Australian Standard *AS2890.1-2004 – Parking facilities – Part 1 – Off-street car parking* which will need to be conditioned on any consent issued for the development.

#### Heavy vehicle parking.

Based on the operational advice from the client the average number of trucks on site at any one time during the peak operating periods will be 4 vehicles however during infrequent times this may increase to 6 vehicles. The site has the ability to cater for at least 4 vehicles within the recycling and concrete batching areas as well as queuing space from the shed to and including the weighbridge of at least 3 heavy vehicles including truck and trailer combinations. Therefore there is sufficient space on the site to cater for the expected heavy vehicle arrival rate with turnover times less than 1 hour. As such no queuing from the site onto Kerr Road will occur and the operations will not result in operational impediments to the local road network.

It is therefore concluded that sufficient and suitable car and heavy vehicle parking is provided on the site to meet the expected peak parking demand generated by the development and comply with the requirements of Campbelltown City Council's DCP and Australian Standards.

#### 3.5 Alternative Transport Modes

The proposed development will not increase use of the existing public transport service significantly therefore there would be no nexus from this development for the provision of additional infrastructure or changes to the existing service resulting from this development.

Similarly, the development is unlikely to significantly increase pedestrian and cycle traffic on the local road network therefore no nexus exists for the provision of additional external pedestrian or cycle way infrastructure.



## 4. CONCLUSIONS

This traffic impact assessment for the expansion of an existing resource recovery facility on Lot 16 DP 717203, 16 Kerr Road, Ingleburn has concluded;

- The proposed development is likely to generate in the order of an additional 14 vtph during the AM and PM peak hour traffic periods.
- There is sufficient two-way mid-block capacity within the local road network to cater for the additional traffic generated by this development.
- SIDRA INTERSECTION modelling has shown that all the major intersections along the likely haulage routes to the Hume Motorway and local areas have sufficient spare capacity to cater for the proposal noting they will continue to operate satisfactorily post development through to at least 2028. Therefore, the development will not adversely impact on the local road network and no road upgrading is considered warranted.
- The existing vehicular access is satisfactory for the proposed development and would be compliant with Australian Standard AS2890.1-2004 Parking facilities Part 1 Off-street car parking and Australian Standard AS2890.2-2002 Parking facilities Part 2 Off-street commercial vehicle facilities. The access would also comply with the Campbelltown (Sustainable City) Development Control Plan (2015)
- It is noted from the TfNSW restricted vehicle access maps that the haulage routes to the site are already approved for 25/B26 metre B-Double heavy vehicles though Aero Road and Kerr Road are approved routes with travel conditions while the Lancaster Street / Aero Road roundabout is a restricted intersection with conditional approval for B-Doubles. If the site were to generate B-Double vehicle movements, future consultation with Campbelltown City Council's Traffic Committee will be required.
- Overall it is concluded that the local road network has sufficient spare capacity to cater for the development and the proposal will not adversely impact on the local and state road network.
- Sufficient and suitable on-site car parking can be provided on-site to meet the operational requirements of the development as well as Campbelltown (Sustainable City) Development Control Plan (2015) and Australian Standards.
- Suitable loading arrangements and queuing areas for heavy vehicles exist within the site to cater for the expected arrival rate of heavy vehicles during peak operational periods (4 heavy vehicles on site at any one time plus a maximum 2 vehicles queuing in the driveway). Therefore no queuing of heavy vehicles onto the local road network will occur due to the increased production on site.
- The proposed development will not increase use of the existing public transport service significantly therefore there would be no nexus from this development for the provision of additional infrastructure or changes to the existing service resulting from this development.
- The development is unlikely to significantly increase pedestrian and cycle traffic on the local road network therefore no nexus exists for the provision of additional external pedestrian or cycle way infrastructure.



# 5. RECOMMENDATION

Having carried out this traffic impact assessment for the proposed expansion of an existing resource recovery facility on Lot 16 DP 717203, 16 Kerr Road, Ingleburn it is recommended that the proposal can be supported from a traffic perspective as it will not adversely impact on the local and state road network and generally complies with the requirements of Campbelltown City Council, Australian Standards and NSW Roads and Maritime Services.

JR Garry BE (Civil), Masters of Traffic

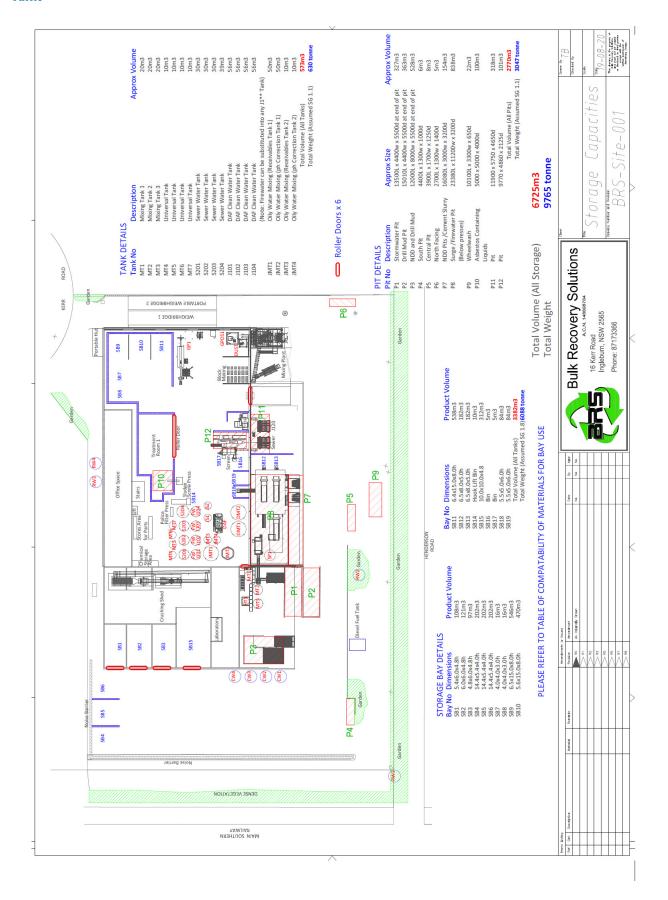
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Intersect Traffic Pty Ltd



# ATTACHMENT A DEVELOPMENT PLANS

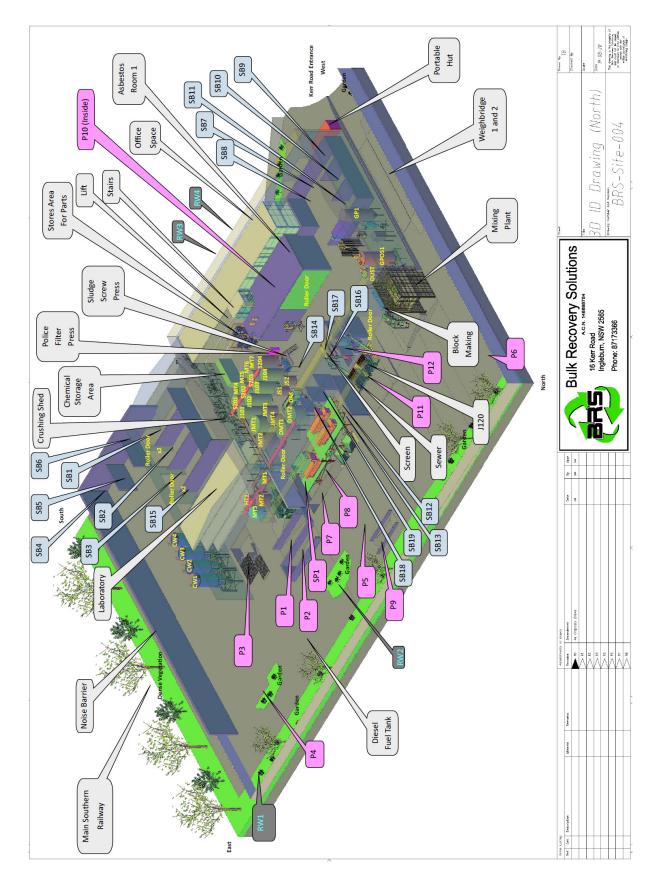




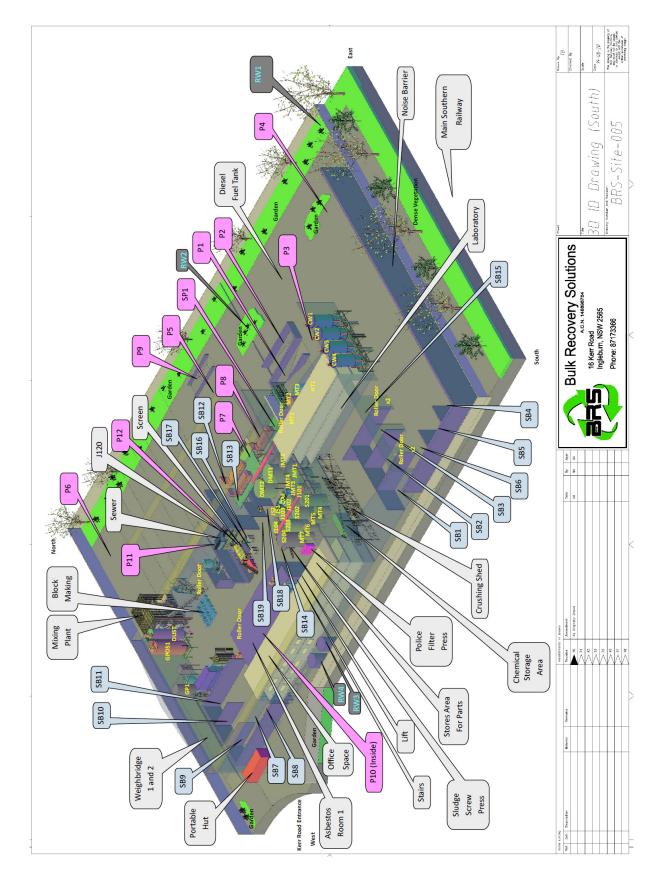




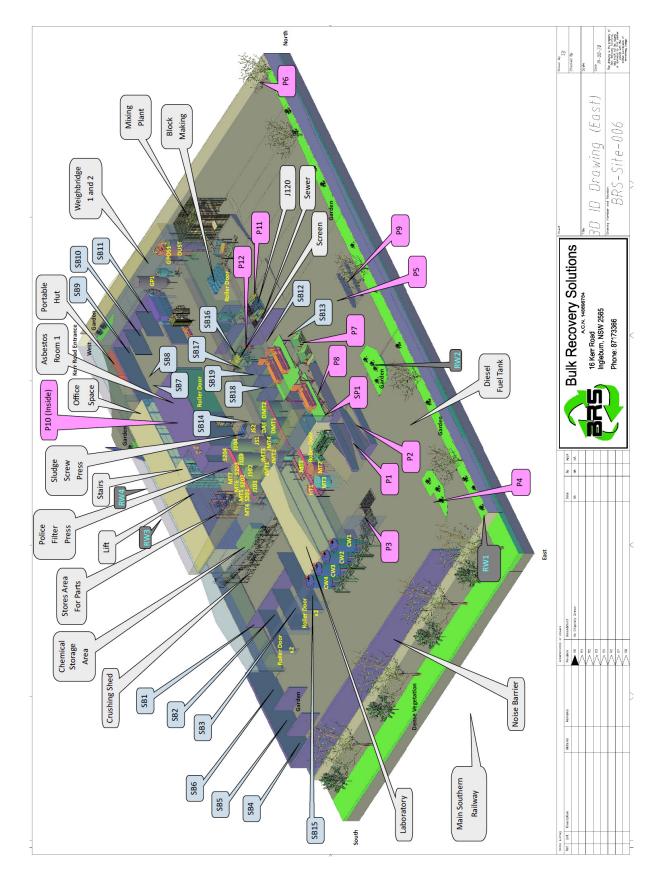




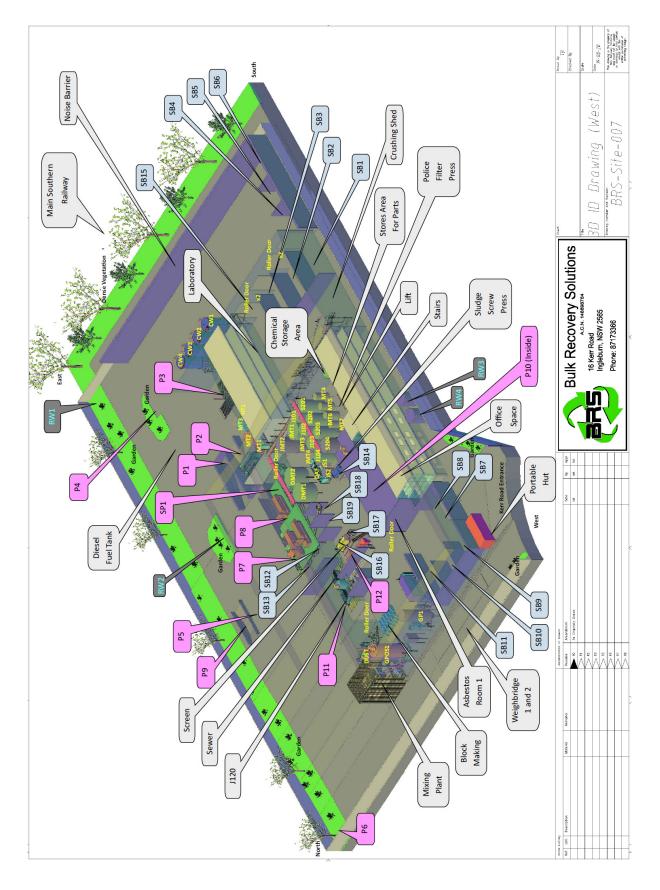




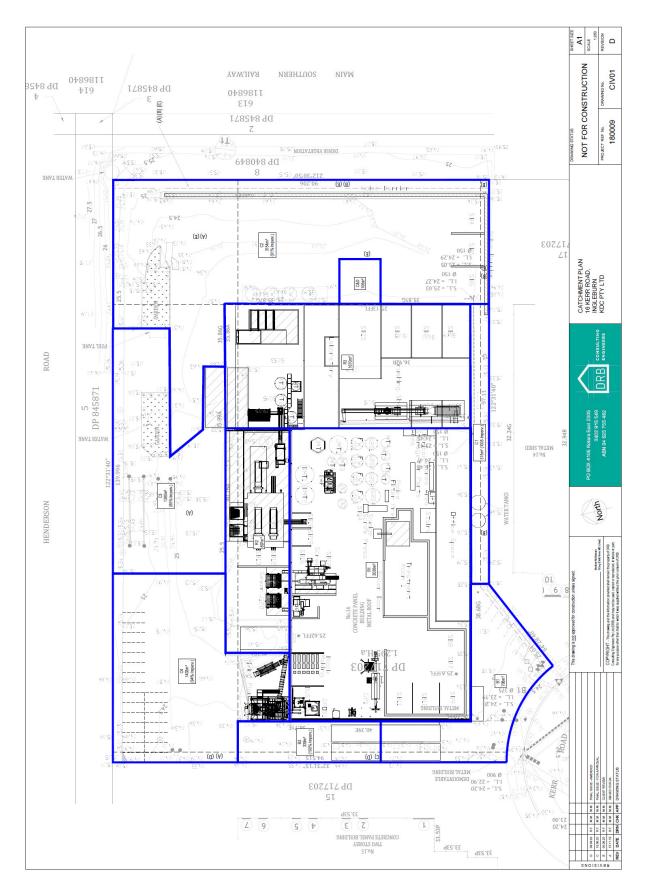














# ATTACHMENT B TRAFFIC COUNT DATA



R.O.A.R. DATA Reliable, Original & Authentic Results

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: InTersed Traffic : 6643 INGLEBURN Traffic Counts : Wednesday 29th November 2017 Client Job No/Name Day/Date

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: InTersect Traffic : 6643 INGLEBURN Traffic Counts : Wednesday 29th November 2017 Client Job No/Name Day/Date

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R.O.A.R. DATA Reliable, Original & Authentic Results Ph. 881 96847 Mob 0418-239019	NORTH WEST SOUTH	Aero Pt Lancaster St		54 8 23 2 7 2 66 3 11 2 35	58 19 15 0 7 3 83 7 6 1 29	19 54 8 21 0 9 2 90 2 17 2 57	55 20 16 2 5 1 83 1 4 2 39	57 11 38 0 4 3 113 6 4 0 58	46 11 32 0 0 3 86 2 7 1 47	15 36 12 24 1 4 2 89 1 5 1 52	10 28 2 5 1 74 1 9 2 32	397 99 197 7 41 17 684 23 63 11 349	WEST SOUTH	Aero H Lancaster St	I B T I B T I B T B	221 55 75 4 28 8 322 13 38 7 160	78 224 58 90 2 25 9 369 16 31 5 183 1	102 50 10 2 18 9 3/2 11 32 5 201	70 176 44 122 3 13 9 362 10 25 4 189	224 58 90 2 25 9 369 16 31 5 183		WEST SOUTH	Aero H Lancaster St	B L I B L I B L I B	58 10 24 2 7 2 72 5 13 2 37	63 19 16 0 9 3 88 7 6 2 30	59 9 21 0 9 2 95 3 18 2 58	60 21 16 2 5 1 88 1 6 2 41	63 12 39 0 4 3 116 8 5 0 62	49 11 32 0 1 3 91 2 8 1 48	



# ATTACHMENT C SIDRA MOVEMENT SUMMARY TABLE



**♥** Site: 101 [2018AM + dev]

Brooks Road / Williamson Road, Ingleburn

Site Category: (None)

Roundabout

Move	ment P	erformanc	e - Veh	icles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	: William	ison										
1	L2	383	20.1	0.466	7.2	LOSA	3.7	29.7	0.77	0.75	0.77	51.5
2	T1	484	6.5	0.466	7.4	LOS A	3.7	29.7	0.78	0.74	0.78	50.7
3u	U	1	0.0	0.466	13.9	LOSA	3.6	26.5	0.78	0.74	0.78	54.1
Appro	ach	868	12.5	0.466	7.3	LOSA	3.7	29.7	0.77	0.74	0.78	51.1
North:	Williams	son										
8	T1	399	5.8	0.471	8.2	LOSA	3.9	29.0	0.82	0.82	0.86	50.8
9	R2	441	8.6	0.471	12.0	LOSA	3.9	29.0	0.81	0.82	0.82	47.5
9u	U	1	0.0	0.471	13.9	LOSA	3.9	29.0	0.81	0.82	0.82	46.0
Appro	ach	841	7.3	0.471	10.2	LOSA	3.9	29.0	0.82	0.82	0.84	49.0
West:	Brooks											
10	L2	819	5.7	0.820	11.3	LOSA	10.8	79.5	0.93	1.07	1.30	46.1
12	R2	508	10.8	0.639	13.9	LOSA	5.3	40.2	0.78	0.98	0.96	49.0
12u	U	1	0.0	0.639	15.5	LOS B	5.3	40.2	0.78	0.98	0.96	49.7
Appro	ach	1328	7.6	0.820	12.3	LOSA	10.8	79.5	0.87	1.03	1.17	47.4
All Vel	hicles	3038	8.9	0.820	10.3	LOSA	10.8	79.5	0.83	0.89	0.96	48.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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₩ Site: 101 [2018PM + dev]

Brooks Road / Williamson Road, Ingleburn

Site Category: (None) Roundabout

Move	ment P	erformanc	e - Veh	icles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	William	son										
1	L2	417	9.1	0.586	10.8	LOS A	6.2	46.8	0.98	0.98	1.16	49.5
2	T1	434	4.1	0.586	12.0	LOSA	6.2	46.8	0.97	1.02	1.17	48.0
3u	U	1	0.0	0.586	18.6	LOS B	5.8	41.9	0.96	1.03	1.18	51.6
Appro	ach	852	6.6	0.586	11.4	LOSA	6.2	46.8	0.97	1.00	1.16	48.8
North:	Williams	son										
8	T1	524	5.4	0.676	12.5	LOSA	7.7	56.4	0.95	1.04	1.24	47.8
9	R2	657	7.2	0.697	16.1	LOS B	8.9	65.9	0.97	1.01	1.25	44.4
9u	U	1	0.0	0.697	17.9	LOS B	8.9	65.9	0.97	1.01	1.25	42.1
Appro	ach	1182	6.4	0.697	14.5	LOSA	8.9	65.9	0.96	1.02	1.24	45.9
West:	Brooks											
10	L2	678	7.5	0.680	8.2	LOSA	6.6	49.2	0.81	0.88	0.95	48.6
12	R2	539	12.7	0.635	13.2	LOSA	5.4	42.2	0.78	0.93	0.92	49.4
12u	U	1	0.0	0.635	14.8	LOS B	5.4	42.2	0.78	0.93	0.92	50.3
Appro	ach	1218	9.8	0.680	10.4	LOSA	6.6	49.2	0.80	0.90	0.94	49.0
All Vel	nicles	3252	7.7	0.697	12.1	LOSA	8.9	65.9	0.90	0.97	1.11	47.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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\Brooks Williamson.sip8



**∀** Site: 101 [2028AM + dev]

Brooks Road / Williamson Road, Ingleburn Site Category: (None) Roundabout

Move	ment P	erformanc	e - Veh	icles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	: William	ison										
1	L2	421	20.0	0.542	8.5	LOS A	5.1	40.7	0.85	0.84	0.93	50.9
2	T1	533	6.3	0.542	8.8	LOSA	5.1	40.7	0.85	0.86	0.95	50.2
3u	U	1	0.0	0.542	15.4	LOS B	4.9	36.3	0.85	0.86	0.95	53.7
Appro	ach	955	12.3	0.542	8.6	LOSA	5.1	40.7	0.85	0.85	0.94	50.5
North:	William	son										
8	T1	439	5.8	0.557	10.1	LOSA	5.5	41.3	0.90	0.94	1.04	49.6
9	R2	484	8.5	0.557	13.7	LOSA	5.5	41.3	0.90	0.91	1.01	46.4
9u	U	1	0.0	0.557	15.5	LOS B	5.5	41.3	0.90	0.91	1.01	44.6
Appro	ach	924	7.2	0.557	12.0	LOSA	5.5	41.3	0.90	0.92	1.03	47.9
West:	Brooks											
10	L2	901	5.5	0.939	20.5	LOS B	20.6	150.9	1.00	1.43	2.06	39.6
12	R2	559	10.7	0.736	16.0	LOS B	7.2	55.4	0.87	1.08	1.18	47.6
12u	U	1	0.0	0.736	17.6	LOS B	7.2	55.4	0.87	1.08	1.18	48.2
Appro	ach	1461	7.5	0.939	18.8	LOS B	20.6	150.9	0.95	1.29	1.72	43.0
All Ve	hicles	3340	8.8	0.939	14.0	LOSA	20.6	150.9	0.91	1.07	1.31	46.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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♥ Site: 101 [2028PM + dev]

Brooks Road / Williamson Road, Ingleburn Site Category: (None)

Roundabout

Move	ment P	erformanc	e - Vehi	icles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	: Williams	son										
1	L2	463	9.1	0.717	15.8	LOS B	9.5	71.7	1.00	1.13	1.41	46.3
2	T1	473	4.0	0.717	17.5	LOS B	9.5	71.7	1.00	1.17	1.44	43.9
3u	U	1	0.0	0.717	24.2	LOS B	8.6	62.5	1.00	1.18	1.45	47.9
Appro	ach	937	6.5	0.717	16.6	LOS B	9.5	71.7	1.00	1.15	1.43	45.2
North:	Williams	on										
8	T1	577	5.3	0.812	19.3	LOS B	12.5	91.3	1.00	1.25	1.65	42.8
9	R2	723	7.1	0.829	22.7	LOS B	14.8	109.8	1.00	1.22	1.66	40.
9u	U	1	0.0	0.829	24.5	LOS B	14.8	109.8	1.00	1.22	1.66	36.9
Appro	ach	1301	6.3	0.829	21.2	LOS B	14.8	109.8	1.00	1.23	1.65	41.3
West:	Brooks											
10	L2	745	7.3	0.771	10.1	LOSA	9.1	67.8	0.90	1.00	1.17	47.0
12	R2	593	12.8	0.725	14.9	LOS B	7.3	57.0	0.87	1.03	1.11	48.2
12u	U	1	0.0	0.725	16.5	LOS B	7.3	57.0	0.87	1.03	1.11	49.0
Appro	ach	1339	9.7	0.771	12.2	LOSA	9.1	67.8	0.89	1.01	1.14	47.6
All Ve	hicles	3577	7.7	0.829	16.6	LOS B	14.8	109.8	0.96	1.13	1.40	44.6

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Organisation: INTERSECT TRAFFIC PTY LTD | Processed: Sunday, 20 September 2020 8:11:34 AM
Project: C:\Work Documents\Projects\17.164 - Kerr Road Inglebum - Resource Recovery Expansion\Sidra\Brooks\_Williamson

\Brooks\_Williamson.sip8



**♥** Site: 101 [2018AM + dev]

Williamson Road / MacDonald Road, Ingleburn Site Category: (None)

Roundabout

Mov   Turn   Demand Flows   Total   HV   Satn   Vic   Delay   Delay	Move	ement P	erformano	e - Veh	icles						_		
1         L2         116         12.7         0.609         7.7         LOS A         5.9         44.0         0.76         0.74         0.83           2         T1         1175         5.5         0.609         7.9         LOS A         5.9         44.0         0.77         0.77         0.85           3         R2         21         10.0         0.609         15.0         LOS B         5.9         43.0         0.78         0.80         0.88           3u         U         1         0.0         0.609         15.0         LOS B         5.9         43.0         0.78         0.80         0.88           Approach         1313         6.2         0.609         8.0         LOS A         5.9         44.0         0.77         0.77         0.85           East: Private access         ***           4         L2         11         50.0         0.032         7.0         LOS A         0.1         1.1         0.66         0.78         0.66           5         T1         1         0.0         0.032         7.6         LOS A         0.1         1.1         0.66         0.78         0.66           6u	ID		Total veh/h	HV	Satn	Delay		Vehicles	Distance				
2         T1         1175         5.5         0.609         7.9         LOS A         5.9         44.0         0.77         0.77         0.85           3         R2         21         10.0         0.609         13.2         LOS A         5.9         43.0         0.78         0.80         0.88           3u         U         1         0.0         0.609         15.0         LOS B         5.9         43.0         0.78         0.80         0.88           Approach         1313         6.2         0.609         8.0         LOS A         5.9         44.0         0.77         0.77         0.85           East: Private access         **           4         L2         11         50.0         0.032         7.0         LOS A         0.1         1.1         0.66         0.78         0.66           5         T1         1         0.0         0.032         5.7         LOS A         0.1         1.1         0.66         0.78         0.66           6 R2         2         0.0         0.032         7.6         LOS A         0.1         1.1         0.66         0.78         0.66           Approach         15	South		son										
3         R2         21         10.0         0.609         13.2         LOS A         5.9         43.0         0.78         0.80         0.88           3u         U         1         0.0         0.609         15.0         LOS B         5.9         43.0         0.78         0.80         0.88           Approach         1313         6.2         0.609         8.0         LOS A         5.9         44.0         0.77         0.77         0.85           East: Private access         U           4         L2         11         50.0         0.032         7.0         LOS A         0.1         1.1         0.66         0.78         0.66           5         T1         1         0.0         0.032         5.7         LOS A         0.1         1.1         0.66         0.78         0.66           6         R2         2         0.0         0.032         7.6         LOS A         0.1         1.1         0.66         0.78         0.66           Approach         15         35.7         0.032         7.6         LOS A         0.1         1.1         0.66         0.78         0.66           Approach         15	1	L2	116	12.7	0.609	7.7	LOSA	5.9	44.0	0.76	0.74	0.83	49.3
3u         U         1         0.0         0.609         15.0         LOS B         5.9         43.0         0.78         0.80         0.88           Approach         1313         6.2         0.609         8.0         LOS A         5.9         44.0         0.77         0.77         0.85           East: Private access         4         L2         11         50.0         0.032         7.0         LOS A         0.1         1.1         0.66         0.78         0.66           5         T1         1         0.0         0.032         5.7         LOS A         0.1         1.1         0.66         0.78         0.66           6         R2         2         0.0         0.032         9.6         LOS A         0.1         1.1         0.66         0.78         0.66           6u         U         1         0.0         0.032         7.6         LOS A         0.1         1.1         0.66         0.78         0.66           Approach         15         35.7         0.032         7.6         LOS A         0.1         1.1         0.66         0.78         0.66           North: Williamson Road         7         L2 <td< td=""><td></td><td>T1</td><td></td><td>5.5</td><td>0.609</td><td>7.9</td><td>LOSA</td><td>5.9</td><td>44.0</td><td>0.77</td><td>0.77</td><td>0.85</td><td>51.2</td></td<>		T1		5.5	0.609	7.9	LOSA	5.9	44.0	0.77	0.77	0.85	51.2
Approach         1313         6.2         0.609         8.0         LOS A         5.9         44.0         0.77         0.77         0.85           East: Private access         4         L2         11         50.0         0.032         7.0         LOS A         0.1         1.1         0.66         0.78         0.66         5         T1         1         0.0         0.032         5.7         LOS A         0.1         1.1         0.66         0.78         0.66         6         R2         2         0.0         0.032         9.6         LOS A         0.1         1.1         0.66         0.78         0.66         6         R2         2         0.0         0.032         9.6         LOS A         0.1         1.1         0.66         0.78         0.66         6         0.0         0.032         11.7         LOS A         0.1         1.1         0.66         0.78         0.66         6         0.0         0.032         11.7         LOS A         0.1         1.1         0.66         0.78         0.66           Approach         15         35.7         0.032         7.6         LOS A         3.4         25.3         0.46         0.78         0.66	3		21							0.78		0.88	32.3
East: Private access  4	3u	U	1	0.0	0.609	15.0	LOS B	5.9	43.0	0.78	0.80	0.88	49.4
4         L2         11         50.0         0.032         7.0         LOS A         0.1         1.1         0.66         0.78         0.66           5         T1         1         0.0         0.032         5.7         LOS A         0.1         1.1         0.66         0.78         0.66           6         R2         2         0.0         0.032         9.6         LOS A         0.1         1.1         0.66         0.78         0.66           6u         U         1         0.0         0.032         7.6         LOS A         0.1         1.1         0.66         0.78         0.66           Approach         15         35.7         0.032         7.6         LOS A         0.1         1.1         0.66         0.78         0.66           North: Williamson Road         V         V         1         0.63         0.431         4.9         LOS A         3.4         25.3         0.46         0.49         0.46           8         T1         759         6.1         0.431         5.1         LOS A         3.4         25.3         0.46         0.51         0.46           9         R2         377         3.1	Appro	ach	1313	6.2	0.609	8.0	LOSA	5.9	44.0	0.77	0.77	0.85	50.9
5         T1         1         0.0         0.032         5.7         LOS A         0.1         1.1         0.66         0.78         0.66           6         R2         2         0.0         0.032         9.6         LOS A         0.1         1.1         0.66         0.78         0.66           6u         U         1         0.0         0.032         11.7         LOS A         0.1         1.1         0.66         0.78         0.66           Approach         15         35.7         0.032         7.6         LOS A         0.1         1.1         0.66         0.78         0.66           North: Williamson Road         7         L2         6         0.0         0.431         4.9         LOS A         3.4         25.3         0.46         0.49         0.46           8         T1         759         6.1         0.431         5.1         LOS A         3.4         25.3         0.46         0.51         0.46           9         R2         377         3.1         0.431         9.8         LOS A         3.3         24.2         0.48         0.61         0.48           4proach         1143         5.1         0.431 </td <td>East:</td> <td>Private a</td> <td>ccess</td> <td></td>	East:	Private a	ccess										
6 R2 2 0.0 0.032 9.6 LOS A 0.1 1.1 0.66 0.78 0.66 6u U 1 0.0 0.032 11.7 LOS A 0.1 1.1 0.66 0.78 0.66 Approach 15 35.7 0.032 7.6 LOS A 0.1 1.1 0.66 0.78 0.66 North: Williamson Road 7 L2 6 0.0 0.431 4.9 LOS A 3.4 25.3 0.46 0.49 0.46 8 T1 759 6.1 0.431 5.1 LOS A 3.4 25.3 0.46 0.51 0.46 9 R2 377 3.1 0.431 9.8 LOS A 3.3 24.2 0.48 0.61 0.48 9u U 1 0.0 0.431 11.8 LOS A 3.3 24.2 0.48 0.61 0.48 Approach 1143 5.1 0.431 6.6 LOS A 3.4 25.3 0.47 0.55 0.47 West: MacDonald Road 10 L2 415 3.0 0.606 9.6 LOS A 4.1 29.4 0.86 1.00 1.08 11 T1 5 0.0 0.309 9.2 LOS A 1.4 10.4 0.76 0.93 0.79 12 R2 122 10.3 0.309 14.5 LOS A 1.4 10.4 0.76 0.93 0.79 12 U 1 0.0 0.309 15.9 LOS B 1.4 10.4 0.76 0.93 0.79 Approach 543 4.7 0.606 10.7 LOS A 4.1 29.4 0.83 0.98 1.01	4	L2	11	50.0	0.032	7.0	LOSA	0.1	1.1	0.66	0.78	0.66	31.9
6u         U         1         0.0         0.032         11.7         LOS A         0.1         1.1         0.66         0.78         0.66           Approach         15         35.7         0.032         7.6         LOS A         0.1         1.1         0.66         0.78         0.66           North: Williamson Road         **Out of the colspan="6">**Out of t	5	T1	1	0.0	0.032	5.7	LOSA	0.1	1.1	0.66	0.78	0.66	49.0
Approach         15         35.7         0.032         7.6         LOS A         0.1         1.1         0.66         0.78         0.66           North: Williamson Road         7         L2         6         0.0         0.431         4.9         LOS A         3.4         25.3         0.46         0.49         0.46           8         T1         759         6.1         0.431         5.1         LOS A         3.4         25.3         0.46         0.51         0.46           9         R2         377         3.1         0.431         9.8         LOS A         3.3         24.2         0.48         0.61         0.48           9u         U         1         0.0         0.431         11.8         LOS A         3.3         24.2         0.48         0.61         0.48           Approach         1143         5.1         0.431         6.6         LOS A         3.4         25.3         0.47         0.55         0.47           West: MacDonald Road           10         L2         415         3.0         0.606         9.6         LOS A         4.1         29.4         0.86         1.00         1.08 <t< td=""><td>6</td><td>R2</td><td>2</td><td>0.0</td><td>0.032</td><td>9.6</td><td>LOSA</td><td>0.1</td><td>1.1</td><td>0.66</td><td>0.78</td><td>0.66</td><td>49.0</td></t<>	6	R2	2	0.0	0.032	9.6	LOSA	0.1	1.1	0.66	0.78	0.66	49.0
North: Williamson Road  7	6u	U	1	0.0	0.032	11.7	LOSA	0.1	1.1	0.66	0.78	0.66	11.3
7         L2         6         0.0         0.431         4.9         LOS A         3.4         25.3         0.46         0.49         0.46           8         T1         759         6.1         0.431         5.1         LOS A         3.4         25.3         0.46         0.51         0.46           9         R2         377         3.1         0.431         9.8         LOS A         3.3         24.2         0.48         0.61         0.48           9u         U         1         0.0         0.431         11.8         LOS A         3.3         24.2         0.48         0.61         0.48           Approach         1143         5.1         0.431         6.6         LOS A         3.4         25.3         0.47         0.55         0.47           West: MacDonald Road           10         L2         415         3.0         0.606         9.6         LOS A         4.1         29.4         0.86         1.00         1.08           11         T1         5         0.0         0.309         9.2         LOS A         1.4         10.4         0.76         0.93         0.79           12         R2         <	Appro	ach	15	35.7	0.032	7.6	LOS A	0.1	1.1	0.66	0.78	0.66	35.2
8         T1         759         6.1         0.431         5.1         LOS A         3.4         25.3         0.46         0.51         0.46           9         R2         377         3.1         0.431         9.8         LOS A         3.3         24.2         0.48         0.61         0.48           9u         U         1         0.0         0.431         11.8         LOS A         3.3         24.2         0.48         0.61         0.48           Approach         1143         5.1         0.431         6.6         LOS A         3.4         25.3         0.47         0.55         0.47           West: MacDonald Road           10         L2         415         3.0         0.606         9.6         LOS A         4.1         29.4         0.86         1.00         1.08           11         T1         5         0.0         0.309         9.2         LOS A         1.4         10.4         0.76         0.93         0.79           12         R2         122         10.3         0.309         14.5         LOS A         1.4         10.4         0.76         0.93         0.79           12u         U	North:	: Williams	on Road										
9 R2 377 3.1 0.431 9.8 LOS A 3.3 24.2 0.48 0.61 0.48 9u U 1 0.0 0.431 11.8 LOS A 3.3 24.2 0.48 0.61 0.48 Approach 1143 5.1 0.431 6.6 LOS A 3.4 25.3 0.47 0.55 0.47  West: MacDonald Road  10 L2 415 3.0 0.606 9.6 LOS A 4.1 29.4 0.86 1.00 1.08 11 T1 5 0.0 0.309 9.2 LOS A 1.4 10.4 0.76 0.93 0.79 12 R2 122 10.3 0.309 14.5 LOS A 1.4 10.4 0.76 0.93 0.79 12u U 1 0.0 0.309 15.9 LOS B 1.4 10.4 0.76 0.93 0.79 Approach 543 4.7 0.606 10.7 LOS A 4.1 29.4 0.83 0.98 1.01	7	L2	6	0.0	0.431	4.9	LOSA	3.4	25.3	0.46	0.49	0.46	40.2
9u         U         1         0.0         0.431         11.8         LOS A         3.3         24.2         0.48         0.61         0.48           Approach         1143         5.1         0.431         6.6         LOS A         3.4         25.3         0.47         0.55         0.47           West: MacDonald Road           10         L2         415         3.0         0.606         9.6         LOS A         4.1         29.4         0.86         1.00         1.08           11         T1         5         0.0         0.309         9.2         LOS A         1.4         10.4         0.76         0.93         0.79           12         R2         122         10.3         0.309         14.5         LOS A         1.4         10.4         0.76         0.93         0.79           12u         U         1         0.0         0.309         15.9         LOS B         1.4         10.4         0.76         0.93         0.79           Approach         543         4.7         0.606         10.7         LOS A         4.1         29.4         0.83         0.98         1.01	8	T1	759	6.1	0.431	5.1	LOSA	3.4	25.3	0.46	0.51	0.46	51.9
Approach         1143         5.1         0.431         6.6         LOS A         3.4         25.3         0.47         0.55         0.47           West: MacDonald Road           10         L2         415         3.0         0.606         9.6         LOS A         4.1         29.4         0.86         1.00         1.08           11         T1         5         0.0         0.309         9.2         LOS A         1.4         10.4         0.76         0.93         0.79           12         R2         122         10.3         0.309         14.5         LOS A         1.4         10.4         0.76         0.93         0.79           12u         U         1         0.0         0.309         15.9         LOS B         1.4         10.4         0.76         0.93         0.79           Approach         543         4.7         0.606         10.7         LOS A         4.1         29.4         0.83         0.98         1.01	9	R2	377	3.1	0.431	9.8	LOSA	3.3	24.2	0.48	0.61	0.48	52.5
West: MacDonald Road  10	9u	U	1	0.0	0.431	11.8	LOSA	3.3	24.2	0.48	0.61	0.48	53.6
10       L2       415       3.0       0.606       9.6       LOS A       4.1       29.4       0.86       1.00       1.08         11       T1       5       0.0       0.309       9.2       LOS A       1.4       10.4       0.76       0.93       0.79         12       R2       122       10.3       0.309       14.5       LOS A       1.4       10.4       0.76       0.93       0.79         12u       U       1       0.0       0.309       15.9       LOS B       1.4       10.4       0.76       0.93       0.79         Approach       543       4.7       0.606       10.7       LOS A       4.1       29.4       0.83       0.98       1.01	Appro	ach	1143	5.1	0.431	6.6	LOS A	3.4	25.3	0.47	0.55	0.47	52.1
11       T1       5       0.0       0.309       9.2       LOS A       1.4       10.4       0.76       0.93       0.79         12       R2       122       10.3       0.309       14.5       LOS A       1.4       10.4       0.76       0.93       0.79         12u       U       1       0.0       0.309       15.9       LOS B       1.4       10.4       0.76       0.93       0.79         Approach       543       4.7       0.606       10.7       LOS A       4.1       29.4       0.83       0.98       1.01	West:	MacDon	ald Road										
12     R2     122     10.3     0.309     14.5     LOS A     1.4     10.4     0.76     0.93     0.79       12u     U     1     0.0     0.309     15.9     LOS B     1.4     10.4     0.76     0.93     0.79       Approach     543     4.7     0.606     10.7     LOS A     4.1     29.4     0.83     0.98     1.01	10	L2	415	3.0	0.606	9.6	LOSA	4.1	29.4	0.86	1.00	1.08	51.1
12u         U         1         0.0         0.309         15.9         LOS B         1.4         10.4         0.76         0.93         0.79           Approach         543         4.7         0.606         10.7         LOS A         4.1         29.4         0.83         0.98         1.01	11	T1	5	0.0	0.309	9.2	LOSA	1.4	10.4	0.76	0.93	0.79	37.0
Approach 543 4.7 0.606 10.7 LOS A 4.1 29.4 0.83 0.98 1.01	12	R2	122	10.3	0.309	14.5	LOSA	1.4	10.4	0.76	0.93	0.79	45.9
	12u	U	1	0.0	0.309	15.9	LOS B	1.4	10.4	0.76	0.93	0.79	50.4
All Vehicles 3014 5.6 0.609 8.0 LOS A 5.9 44.0 0.67 0.72 0.73	Appro	ach	543	4.7	0.606	10.7	LOSA	4.1	29.4	0.83	0.98	1.01	49.9
	All Ve	hicles	3014	5.6	0.609	8.0	LOSA	5.9	44.0	0.67	0.72	0.73	51.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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Project: C:\Work Documents\Projects\17.164 - Kerr Road Ingleburn - Resource Recovery Expansion\Sidra\Williamson\_MacDonald

\Williamson\_MacDonald.sip8



**♥** Site: 101 [2018PM + dev]

Williamson Road / MacDonald Road, Ingleburn

Site Category: (None)

Roundabout

Mov	Turn	Demand		Deg.	Average	Level of	95% Back	of Queu <u>e</u>	Prop.	Effective	Aver. No.	Average
ID		Total veh/h	HV %	Satn v/c	Delay sec	Service	Vehicles veh	Distance m	Queued	Stop Rate	Cycles	Speed km/h
South	: Williams	son										
1	L2	86	2.4	0.547	7.5	LOSA	4.8	35.4	0.77	0.76	0.82	49.6
2	T1	1008	6.2	0.547	8.1	LOSA	4.8	35.4	0.78	0.79	0.85	51.2
3	R2	5	20.0	0.547	13.8	LOSA	4.7	34.8	0.78	0.82	0.87	32.4
3u	U	1	0.0	0.547	15.1	LOS B	4.7	34.8	0.78	0.82	0.87	49.
Appro	ach	1101	5.9	0.547	8.1	LOS A	4.8	35.4	0.78	0.79	0.84	51.0
East:	Private a	ccess										
4	L2	9	0.0	0.024	6.0	LOSA	0.1	0.7	0.70	0.77	0.70	41.8
5	T1	1	0.0	0.024	6.6	LOSA	0.1	0.7	0.70	0.77	0.70	50.
6	R2	1	0.0	0.024	10.5	LOSA	0.1	0.7	0.70	0.77	0.70	50.
6u	U	1	0.0	0.024	12.6	LOSA	0.1	0.7	0.70	0.77	0.70	10.9
Appro	ach	13	0.0	0.024	7.0	LOS A	0.1	0.7	0.70	0.77	0.70	41.9
North:	Williams	on Road										
7	L2	6	0.0	0.528	4.8	LOSA	4.9	36.1	0.44	0.46	0.44	40.3
8	T1	1021	5.3	0.528	4.9	LOSA	4.9	36.1	0.45	0.49	0.45	52.0
9	R2	456	3.0	0.528	9.6	LOSA	4.8	34.9	0.47	0.58	0.47	52.7
9u	U	1	0.0	0.528	11.7	LOSA	4.8	34.9	0.47	0.58	0.47	53.8
Appro	ach	1484	4.5	0.528	6.4	LOS A	4.9	36.1	0.46	0.52	0.46	52.3
West:	MacDona	ald Road										
10	L2	428	2.2	0.560	8.5	LOSA	3.7	26.2	0.81	0.96	0.98	51.9
11	T1	2	0.0	0.239	8.8	LOSA	1.0	7.5	0.71	0.90	0.71	37.2
12	R2	98	9.7	0.239	14.0	LOSA	1.0	7.5	0.71	0.90	0.71	46.2
12u	U	1	0.0	0.239	15.5	LOS B	1.0	7.5	0.71	0.90	0.71	50.6
Appro	ach	529	3.6	0.560	9.5	LOS A	3.7	26.2	0.79	0.95	0.93	50.9
All Ve	hicles	3127	4.8	0.560	7.5	LOSA	4.9	36.1	0.63	0.69	0.67	51.

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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\Williamson\_MacDonald.sip8



**♥** Site: 101 [2028AM + dev]

Williamson Road / MacDonald Road, Ingleburn Site Category: (None) Roundabout

Move	ment P	erformanc	e - Veh	icles								
Mov	Turn	Demand		Deg.	Average	Level of	95% Back		Prop.		Aver. No.	
ID		Total veh/h	HV %	Satn v/c	Delay sec	Service	Vehicles veh	Distance m	Queuea	Stop Rate	Cycles	Speed km/h
South	: William:		70	VIC	300		VOIT					KITI/TI
1	L2	127	12.4	0.689	9.4	LOSA	8.3	61.0	0.86	0.85	1.02	48.7
2	T1	1282	4.5	0.689	9.7	LOSA	8.3	61.0	0.86	0.88	1.05	50.2
3	R2	23	9.1	0.689	15.1	LOS B	8.0	58.5	0.87	0.91	1.08	31.0
3u	U	1	0.0	0.689	16.9	LOS B	8.0	58.5	0.87	0.91	1.08	47.6
Appro	ach	1434	5.3	0.689	9.7	LOSA	8.3	61.0	0.86	0.88	1.05	49.9
East:	Private a	ccess										
4	L2	12	45.5	0.037	7.5	LOSA	0.1	1.3	0.69	0.81	0.69	32.1
5	T1	1	0.0	0.037	6.2	LOSA	0.1	1.3	0.69	0.81	0.69	48.6
6	R2	2	0.0	0.037	10.1	LOSA	0.1	1.3	0.69	0.81	0.69	48.6
6u	U	1	0.0	0.037	12.2	LOSA	0.1	1.3	0.69	0.81	0.69	10.9
Appro	ach	16	33.3	0.037	8.0	LOS A	0.1	1.3	0.69	0.81	0.69	35.1
North:	Williams	on Road										
7	L2	7	0.0	0.488	5.2	LOSA	4.2	30.6	0.53	0.51	0.53	39.8
8	T1	836	5.9	0.488	5.3	LOSA	4.2	30.6	0.54	0.54	0.54	51.5
9	R2	415	3.0	0.488	10.1	LOSA	4.0	29.2	0.55	0.63	0.55	52.3
9u	U	1	0.0	0.488	12.1	LOSA	4.0	29.2	0.55	0.63	0.55	53.4
Appro	ach	1259	4.9	0.488	6.9	LOS A	4.2	30.6	0.54	0.57	0.54	51.8
West:	MacDon	ald Road										
10	L2	498	2.7	0.807	14.4	LOSA	7.1	51.0	0.95	1.16	1.50	47.9
11	T1	6	0.0	0.407	10.5	LOSA	2.0	14.9	0.81	0.97	0.92	36.2
12	R2	146	9.4	0.407	15.7	LOS B	2.0	14.9	0.81	0.97	0.92	45.1
12u	U	1	0.0	0.407	17.2	LOS B	2.0	14.9	0.81	0.97	0.92	49.5
Appro	ach	652	4.2	0.807	14.7	LOS B	7.1	51.0	0.92	1.11	1.36	47.3
All Ve	hicles	3360	5.1	0.807	9.6	LOSA	8.3	61.0	0.75	0.81	0.92	49.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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₩ Site: 101 [2028PM + dev]

Williamson Road / MacDonald Road, Ingleburn Site Category: (None) Roundabout

Move	ment P	erformanc	e - Veh	icles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued		Aver. No. Cycles	Average Speed km/h
South	: Williams											
1	L2	95	2.2	0.634	9.2	LOSA	6.9	50.3	0.86	0.88	1.02	49.0
2	T1	1112	6.0	0.634	10.0	LOSA	6.9	50.3	0.86	0.90	1.04	50.1
3	R2	6	16.7	0.634	15.8	LOS B	6.5	48.2	0.87	0.93	1.07	30.7
3u	U	1	0.0	0.634	17.1	LOS B	6.5	48.2	0.87	0.93	1.07	47.3
Appro		1214	5.7	0.634	9.9	LOSA	6.9	50.3	0.86	0.90	1.04	49.9
East:	Private a											
4	L2	11	0.0	0.029	6.7	LOSA	0.1	0.8	0.73	0.80	0.73	40.9
5	T1	1	0.0	0.029	7.2	LOSA	0.1	0.8	0.73	0.80	0.73	49.8
6	R2	1	0.0	0.029	11.1	LOSA	0.1	0.8	0.73	0.80	0.73	49.8
6u	U	1	0.0	0.029	13.2	LOSA	0.1	0.8	0.73	0.80	0.73	10.5
Appro	ach	14	0.0	0.029	7.6	LOSA	0.1	8.0	0.73	0.80	0.73	41.0
North:	Williams	on Road										
7	L2	7	0.0	0.587	4.9	LOSA	6.0	43.5	0.50	0.48	0.50	40.0
8	T1	1122	5.0	0.587	5.1	LOSA	6.0	43.5	0.51	0.50	0.51	51.8
9	R2	501	2.9	0.587	9.8	LOSA	5.8	42.1	0.53	0.59	0.53	52.5
9u	U	1	0.0	0.587	11.9	LOSA	5.8	42.1	0.53	0.59	0.53	53.6
Appro	ach	1632	4.3	0.587	6.5	LOS A	6.0	43.5	0.52	0.53	0.52	52.0
West:	MacDon	ald Road										
10	L2	472	2.2	0.681	10.3	LOSA	5.1	36.5	0.89	1.04	1.18	50.7
11	T1	2	0.0	0.288	9.3	LOSA	1.2	9.5	0.75	0.92	0.77	36.8
12	R2	107	9.8	0.288	14.6	LOS B	1.2	9.5	0.75	0.92	0.77	45.8
12u	U	1	0.0	0.288	16.0	LOS B	1.2	9.5	0.75	0.92	0.77	50.2
Appro	ach	582	3.6	0.681	11.1	LOSA	5.1	36.5	0.86	1.02	1.10	49.8
All Ve	hicles	3441	4.7	0.681	8.5	LOSA	6.9	50.3	0.70	0.75	0.80	50.9

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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**∀** Site: 101 [2018AM + dev]

Williamson Road / Henderson Road / Macquarie Links Road / Garner Place

Site Category: (None)

Roundabout

Mov	Turn	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID		Total	HV	Satn	Delay	Service	Vehicles	Distance		Stop Rate		Speed
		veh/h	%	v/c	sec		veh	m				km/h
		son Road										
1	L2	43	4.9	0.459	3.7	LOSA	3.7	26.7	0.25	0.58	0.25	51.5
2	T1	6	0.0	0.459	3.7	LOSA	3.7	26.7	0.25	0.58	0.25	52.6
3	R2	1402	4.3	0.459	9.5	LOSA	3.7	26.7	0.26	0.58	0.26	53.2
3u	U	1	0.0	0.459	11.8	LOSA	3.7	26.6	0.27	0.59	0.27	54.5
Appro	ach	1453	4.3	0.459	9.3	LOS A	3.7	26.7	0.26	0.58	0.26	53.1
East:	Henderso	on Road										
4	L2	1031	4.0	0.669	4.4	LOSA	7.5	54.3	0.61	0.53	0.61	54.5
5	T1	40	2.6	0.034	3.5	LOSA	0.2	1.4	0.33	0.43	0.33	56.1
6	R2	11	0.0	0.034	9.6	LOSA	0.2	1.4	0.33	0.43	0.33	56.1
6u	U	1	0.0	0.034	12.0	LOSA	0.2	1.4	0.33	0.43	0.33	57.9
Appro	ach	1082	3.9	0.669	4.4	LOS A	7.5	54.3	0.60	0.52	0.60	54.6
North:	Garner F	Place										
7	L2	8	25.0	0.045	10.1	LOSA	0.2	1.4	0.70	0.79	0.70	50.0
8	T1	13	8.3	0.045	9.0	LOSA	0.2	1.4	0.70	0.79	0.70	52.2
9	R2	1	0.0	0.045	14.4	LOSA	0.2	1.4	0.70	0.79	0.70	52.6
9u	U	1	0.0	0.045	16.7	LOS B	0.2	1.4	0.70	0.79	0.70	53.4
Appro	ach	23	13.6	0.045	10.0	LOSA	0.2	1.4	0.70	0.79	0.70	51.4
West:	Macquar	ie Links Dri	ive									
10	L2	2	0.0	0.420	9.5	LOSA	2.1	15.1	0.75	0.94	0.89	48.5
11	T1	116	1.8	0.420	9.8	LOSA	2.1	15.1	0.75	0.94	0.89	50.8
12	R2	149	0.7	0.420	15.0	LOS B	2.1	15.1	0.75	0.94	0.89	51.2
12u	U	1	0.0	0.420	17.4	LOS B	2.1	15.1	0.75	0.94	0.89	52.3
Appro	ach	268	1.2	0.420	12.7	LOSA	2.1	15.1	0.75	0.94	0.89	51.0
All Ve	hicles	2826	3.9	0.669	7.7	LOSA	7.5	54.3	0.44	0.59	0.45	53.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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\Williamson\_Henderson.sip8



**∀** Site: 101 [2018PM + dev]

Williamson Road / Henderson Road / Macquarie Links Road / Garner Place

Site Category: (None)

Roundabout

Mov	Turn	Demand F	-lows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Aver. No.	Average
ID		Total veh/h	HV %	Satn v/c	Delay sec	Service	Vehicles veh	Distance m		Stop Rate		Speed km/h
South	: Williams	son Road										
1	L2	96	0.0	0.464	3.9	LOSA	3.6	26.0	0.32	0.58	0.32	51.7
2	T1	12	9.1	0.464	4.0	LOSA	3.6	26.0	0.32	0.58	0.32	52.5
3	R2	1308	4.3	0.464	9.7	LOSA	3.6	26.0	0.33	0.59	0.33	53.1
3u	U	1	0.0	0.464	12.1	LOSA	3.6	25.9	0.35	0.60	0.35	54.2
Appro	ach	1417	4.0	0.464	9.3	LOS A	3.6	26.0	0.33	0.59	0.33	53.0
East: I	Henderso	on Road										
4	L2	1259	4.3	0.764	4.2	LOSA	10.7	78.0	0.56	0.47	0.56	54.7
5	T1	74	1.4	0.055	3.3	LOSA	0.3	2.1	0.24	0.39	0.24	56.9
6	R2	13	8.3	0.055	9.6	LOSA	0.3	2.1	0.24	0.39	0.24	56.6
6u	U	1	0.0	0.055	11.8	LOSA	0.3	2.1	0.24	0.39	0.24	58.7
Appro	ach	1346	4.2	0.764	4.2	LOS A	10.7	78.0	0.54	0.46	0.54	54.8
North:	Garner F	Place										
7	L2	25	4.2	0.114	7.9	LOSA	0.5	3.4	0.67	0.80	0.67	52.0
8	T1	42	5.0	0.114	7.8	LOSA	0.5	3.4	0.67	0.80	0.67	53.8
9	R2	1	0.0	0.114	13.3	LOSA	0.5	3.4	0.67	0.80	0.67	54.2
9u	U	1	0.0	0.114	15.6	LOS B	0.5	3.4	0.67	0.80	0.67	55.3
Appro	ach	69	4.5	0.114	8.0	LOS A	0.5	3.4	0.67	0.80	0.67	53.2
West:	Macquar	ie Links Driv	е									
10	L2	1	0.0	0.162	7.5	LOSA	0.7	4.8	0.67	0.85	0.67	49.8
11	T1	43	2.4	0.162	7.8	LOSA	0.7	4.8	0.67	0.85	0.67	52.0
12	R2	59	1.8	0.162	13.1	LOSA	0.7	4.8	0.67	0.85	0.67	52.4
12u	U	1	0.0	0.162	15.4	LOS B	0.7	4.8	0.67	0.85	0.67	53.7
Appro	ach	104	2.0	0.162	10.9	LOS A	0.7	4.8	0.67	0.85	0.67	52.2
All Vel	hicles	2937	4.1	0.764	7.0	LOSA	10.7	78.0	0.45	0.55	0.45	53.8

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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₩ Site: 101 [2028AM + dev]

Williamson Road / Henderson Road / Macquarie Links Road / Garner Place

Site Category: (None)

Roundabout

Move	ement F	erformanc	e - Veh	icles								
Mov ID	Turn	Demand Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop.	Effective Stop Rate	Aver. No.	Average Speed
טו		veh/h	пv %	V/C	Sec	Service	venicies veh	Distance	Queueu	Stop Rate	Cycles	km/h
South	n: William	son Road										
1	L2	47	4.4	0.507	3.8	LOSA	4.4	31.7	0.28	0.58	0.28	51.4
2	T1	7	0.0	0.507	3.7	LOSA	4.4	31.7	0.28	0.58	0.28	52.5
3	R2	1542	4.1	0.507	9.5	LOSA	4.4	31.7	0.29	0.58	0.29	53.1
3u	U	1	0.0	0.507	11.9	LOSA	4.4	31.5	0.31	0.58	0.31	54.4
Appro	oach	1598	4.1	0.507	9.3	LOSA	4.4	31.7	0.29	0.58	0.29	53.0
East:	Henders	on Road										
4	L2	1135	3.9	0.748	4.7	LOSA	9.5	68.7	0.72	0.56	0.72	54.1
5	T1	44	2.4	0.038	3.5	LOSA	0.2	1.6	0.35	0.44	0.35	56.0
6	R2	12	0.0	0.038	9.7	LOSA	0.2	1.6	0.35	0.44	0.35	56.0
6u	U	1	0.0	0.038	12.0	LOSA	0.2	1.6	0.35	0.44	0.35	57.8
Appro	oach	1192	3.8	0.748	4.7	LOSA	9.5	68.7	0.70	0.56	0.70	54.2
North	: Garner	Place										
7	L2	9	22.2	0.053	11.0	LOSA	0.2	1.7	0.73	0.83	0.73	49.4
8	T1	14	7.7	0.053	10.0	LOSA	0.2	1.7	0.73	0.83	0.73	51.4
9	R2	1	0.0	0.053	15.4	LOS B	0.2	1.7	0.73	0.83	0.73	51.8
9u	U	1	0.0	0.053	17.7	LOS B	0.2	1.7	0.73	0.83	0.73	52.5
Appro	oach	25	12.5	0.053	10.9	LOSA	0.2	1.7	0.73	0.83	0.73	50.7
West	: Macqua	ırie Links Dri	ive									
10	L2	2	0.0	0.499	11.1	LOSA	2.8	19.6	0.79	0.99	1.03	47.4
11	T1	127	1.7	0.499	11.4	LOSA	2.8	19.6	0.79	0.99	1.03	49.7
12	R2	164	0.6	0.499	16.7	LOS B	2.8	19.6	0.79	0.99	1.03	50.0
12u	U	1	0.0	0.499	19.0	LOS B	2.8	19.6	0.79	0.99	1.03	51.1
Appro	oach	295	1.1	0.499	14.4	LOSA	2.8	19.6	0.79	0.99	1.03	49.9
All Ve	hicles	3109	3.8	0.748	8.0	LOSA	9.5	68.7	0.50	0.61	0.52	53.1

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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₩ Site: 101 [2028PM + dev]

Williamson Road / Henderson Road / Macquarie Links Road / Garner Place

Site Category: (None)

Roundabout

Move	ment Pe	erformanc	e - Veh	icles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/l
South	: Williams	on Road										
1	L2	105	0.0	0.514	4.0	LOSA	4.3	30.8	0.35	0.58	0.35	51.
2	T1	13	8.3	0.514	4.0	LOSA	4.3	30.8	0.35	0.58	0.35	52.
3	R2	1440	4.2	0.514	9.8	LOSA	4.3	30.8	0.37	0.59	0.37	53.
3u	U	1	0.0	0.514	12.2	LOSA	4.2	30.7	0.39	0.60	0.39	54.
Appro	ach	1559	3.9	0.514	9.3	LOS A	4.3	30.8	0.37	0.59	0.37	52.
East:	Henderso	n Road										
4	L2	1385	4.2	0.848	4.6	LOSA	15.5	112.4	0.71	0.50	0.71	54.
5	T1	81	1.3	0.061	3.3	LOSA	0.3	2.3	0.26	0.40	0.26	56.
6	R2	14	7.7	0.061	9.6	LOSA	0.3	2.3	0.26	0.40	0.26	56.
6u	U	1	0.0	0.061	11.8	LOSA	0.3	2.3	0.26	0.40	0.26	58.
Appro	ach	1481	4.1	0.848	4.6	LOS A	15.5	112.4	0.68	0.49	0.68	54.
North:	Garner F	Place										
7	L2	27	3.8	0.135	8.5	LOSA	0.6	4.1	0.71	0.82	0.71	51.
8	T1	46	4.5	0.135	8.3	LOSA	0.6	4.1	0.71	0.82	0.71	53.
9	R2	1	0.0	0.135	13.9	LOSA	0.6	4.1	0.71	0.82	0.71	53.
9u	U	1	0.0	0.135	16.2	LOS B	0.6	4.1	0.71	0.82	0.71	54.
Appro	ach	76	4.2	0.135	8.6	LOS A	0.6	4.1	0.71	0.82	0.71	52.
West:	Macquar	ie Links Driv	ve									
10	L2	1	0.0	0.193	8.0	LOSA	0.8	5.9	0.71	0.87	0.71	49.
11	T1	47	2.2	0.193	8.3	LOSA	0.8	5.9	0.71	0.87	0.71	51.
12	R2	65	1.6	0.193	13.6	LOSA	8.0	5.9	0.71	0.87	0.71	52.
12u	U	1	0.0	0.193	15.9	LOS B	0.8	5.9	0.71	0.87	0.71	53.
Appro	ach	115	1.8	0.193	11.4	LOS A	0.8	5.9	0.71	0.87	0.71	51.
All Vel	hicles	3231	3.9	0.848	7.2	LOSA	15.5	112.4	0.53	0.56	0.53	53.

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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\Williamson\_Henderson.sip8



₩ Site: 101 [2018AM + dev]

Henderson Road / Lancaster Street / Austool Place, Ingleburn

Site Category: (None) Roundabout

Move	ement P	erformanc	e - Veh	icles								
Mov ID	Turn	Demand Total	Flows HV	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No.	Average Speed
10		veh/h	%	v/c	sec	COLVICO	veh	m	Quouou	Otop Hato	O y clos	km/h
South	n: Lancast	ter Street										
1	L2	203	15.0	0.263	7.1	LOSA	1.3	10.1	0.67	0.82	0.67	48.0
2	T1	14	15.4	0.198	7.9	LOSA	0.9	6.6	0.66	0.87	0.66	48.9
3	R2	114	6.5	0.198	12.2	LOSA	0.9	6.6	0.66	0.87	0.66	50.2
3u	U	1	0.0	0.198	14.1	LOSA	0.9	6.6	0.66	0.87	0.66	50.6
Appro	oach	332	12.1	0.263	8.9	LOS A	1.3	10.1	0.67	0.84	0.67	48.9
East:	Henders	on Road										
4	L2	236	2.7	0.530	6.7	LOSA	3.7	26.6	0.65	0.69	0.68	51.9
5	T1	857	0.9	0.530	6.8	LOSA	3.7	26.6	0.65	0.70	0.69	52.3
6	R2	9	0.0	0.530	11.6	LOSA	3.7	26.2	0.66	0.71	0.70	53.0
6u	U	1	0.0	0.530	13.7	LOSA	3.7	26.2	0.66	0.71	0.70	54.8
Appro	oach	1103	1.2	0.530	6.8	LOSA	3.7	26.6	0.65	0.70	0.69	52.2
North	: Austool	Place										
7	L2	4	0.0	0.034	8.8	LOSA	0.1	1.1	0.70	0.78	0.70	50.4
8	T1	12	18.2	0.034	9.7	LOSA	0.1	1.1	0.70	0.78	0.70	50.6
9	R2	27	11.5	0.044	12.7	LOSA	0.2	1.5	0.70	0.83	0.70	45.4
9u	U	1	0.0	0.044	14.4	LOSA	0.2	1.5	0.70	0.83	0.70	49.3
Appro	oach	44	11.9	0.044	11.6	LOS A	0.2	1.5	0.70	0.81	0.70	47.4
West	: Henders	on Road										
10	L2	102	3.1	0.562	5.0	LOSA	4.6	32.5	0.46	0.49	0.46	50.2
11	T1	991	1.7	0.562	5.0	LOSA	4.6	32.5	0.47	0.53	0.47	52.9
12	R2	360	10.5	0.562	9.9	LOSA	4.5	33.0	0.48	0.59	0.48	50.3
12u	U	1	0.0	0.562	11.9	LOSA	4.5	33.0	0.48	0.59	0.48	50.7
Appro	oach	1454	4.0	0.562	6.3	LOS A	4.6	33.0	0.47	0.54	0.47	52.1
All Ve	hicles	2933	4.0	0.562	6.9	LOSA	4.6	33.0	0.56	0.64	0.58	51.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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**∀** Site: 101 [2018PM + dev]

Henderson Road / Lancaster Street / Austool Place, Ingleburn Site Category: (None) Roundabout

Move	ment Pe	erformanc	e - Veh	icles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	: Lancast											
1	L2	529	8.0	0.603	8.9	LOSA	4.3	31.9	0.76	0.95	0.96	47.4
2	T1	15	7.1	0.490	8.8	LOSA	2.8	20.1	0.72	0.95	0.84	48.1
3	R2	355	1.2	0.490	13.2	LOS A	2.8	20.1	0.72	0.95	0.84	49.5
3u	U	1	0.0	0.490	15.2	LOS B	2.8	20.1	0.72	0.95	0.84	49.6
Appro	ach	900	5.3	0.603	10.6	LOS A	4.3	31.9	0.74	0.95	0.91	48.4
East:	Henderso	n Road										
4	L2	147	2.1	0.386	5.4	LOSA	2.2	15.7	0.49	0.56	0.49	52.6
5	T1	725	1.3	0.386	5.5	LOSA	2.2	15.7	0.49	0.55	0.49	53.1
6	R2	4	0.0	0.386	10.2	LOSA	2.2	15.4	0.50	0.55	0.50	53.8
6u	U	1	0.0	0.386	12.3	LOSA	2.2	15.4	0.50	0.55	0.50	55.6
Appro	ach	878	1.4	0.386	5.5	LOS A	2.2	15.7	0.49	0.55	0.49	53.0
North:	Austool	Place										
7	L2	19	0.0	0.109	10.2	LOSA	0.5	3.5	0.78	0.87	0.78	49.8
8	T1	27	0.0	0.109	10.2	LOSA	0.5	3.5	0.78	0.87	0.78	50.4
9	R2	84	1.3	0.147	13.4	LOSA	0.8	5.4	0.80	0.93	0.80	46.0
9u	U	1	0.0	0.147	15.5	LOS B	0.8	5.4	0.80	0.93	0.80	48.6
Appro	ach	132	8.0	0.147	12.3	LOS A	8.0	5.4	0.79	0.91	0.79	47.6
West:	Henders	on Road										
10	L2	26	28.0	0.620	8.0	LOSA	5.7	40.7	0.74	0.75	0.81	47.5
11	T1	1066	1.5	0.620	7.4	LOSA	5.7	40.7	0.74	0.77	0.82	51.5
12	R2	168	20.0	0.620	13.0	LOSA	5.6	41.3	0.75	0.81	0.84	48.9
12u	U	1	0.0	0.620	14.5	LOSA	5.6	41.3	0.75	0.81	0.84	49.6
Appro	ach	1262	4.5	0.620	8.2	LOSA	5.7	41.3	0.74	0.78	0.83	51.1
All Ve	hicles	3172	3.7	0.620	8.3	LOSA	5.7	41.3	0.68	0.77	0.76	50.7

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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₩ Site: 101 [2028AM + dev]

Henderson Road / Lancaster Street / Austool Place, Ingleburn

Site Category: (None)

Roundabout

Move	ment P	erformanc	e - Veh	icles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South:		er Street										
1	L2	224	14.6	0.312	7.5	LOSA	1.6	12.5	0.72	0.86	0.72	47.8
2	T1	15	14.3	0.237	8.2	LOSA	1.1	8.2	0.70	0.89	0.70	48.6
3	R2	125	6.7	0.237	12.6	LOSA	1.1	8.2	0.70	0.89	0.70	49.9
3u	U	1	0.0	0.237	14.4	LOSA	1.1	8.2	0.70	0.89	0.70	50.3
Approa	ach	365	11.8	0.312	9.3	LOS A	1.6	12.5	0.72	0.87	0.72	48.7
East: H	Henderso	on Road										
4	L2	259	2.4	0.605	7.7	LOSA	5.1	36.1	0.73	0.80	0.83	51.5
5	T1	942	0.9	0.605	7.9	LOSA	5.1	36.1	0.73	0.81	0.84	51.8
6	R2	11	0.0	0.605	12.7	LOSA	5.0	35.3	0.74	0.82	0.85	52.6
6u	U	1	0.0	0.605	14.8	LOS B	5.0	35.3	0.74	0.82	0.85	54.4
Approa	ach	1213	1.2	0.605	7.9	LOS A	5.1	36.1	0.73	0.81	0.84	51.8
North:	Austool	Place										
7	L2	4	0.0	0.041	9.5	LOSA	0.2	1.3	0.73	0.82	0.73	49.9
8	T1	13	16.7	0.041	10.4	LOSA	0.2	1.3	0.73	0.82	0.73	50.0
9	R2	31	10.3	0.053	13.2	LOSA	0.2	1.9	0.74	0.86	0.74	45.1
9u	U	1	0.0	0.053	14.9	LOS B	0.2	1.9	0.74	0.86	0.74	48.9
Approa	ach	48	10.9	0.053	12.2	LOS A	0.2	1.9	0.74	0.84	0.74	46.9
West:	Henders	on Road										
10	L2	113	2.8	0.626	5.2	LOSA	5.6	40.0	0.53	0.51	0.53	49.8
11	T1	1089	1.7	0.626	5.2	LOSA	5.6	40.0	0.54	0.54	0.54	52.5
12	R2	396	10.1	0.626	10.2	LOSA	5.5	40.5	0.55	0.61	0.55	49.9
12u	U	1	0.0	0.626	12.1	LOSA	5.5	40.5	0.55	0.61	0.55	50.2
Approa	ach	1599	3.9	0.626	6.5	LOSA	5.6	40.5	0.54	0.56	0.54	51.7
All Veh	hicles	3225	3.9	0.626	7.4	LOSA	5.6	40.5	0.64	0.69	0.68	51.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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₩ Site: 101 [2028PM + dev]

Henderson Road / Lancaster Street / Austool Place, Ingleburn

Site Category: (None)

Roundabout

Mov	Turn	Demand Flows		Deg.	Average	Level of		of Queue	Prop.	Effective	Aver. No.	
ID		Total veh/h	HV %	Satn v/c	Delay sec	Service	Vehicles veh	Distance m	Queued	Stop Rate	Cycles	Speed km/
South	: Lancast	er Street										
1	L2	583	7.6	0.697	10.5	LOSA	5.7	42.7	0.83	1.04	1.16	46.
2	T1	16	6.7	0.571	10.0	LOSA	3.7	26.0	0.77	0.99	0.98	47.
3	R2	391	1.1	0.571	14.4	LOSA	3.7	26.0	0.77	0.99	0.98	48.
3u	U	1	0.0	0.571	16.4	LOS B	3.7	26.0	0.77	0.99	0.98	48.
Appro	ach	991	5.0	0.697	12.0	LOSA	5.7	42.7	0.81	1.02	1.08	47.
East:	Henderso	n Road										
4	L2	162	1.9	0.436	5.6	LOSA	2.6	18.8	0.54	0.58	0.54	52.
5	T1	798	1.3	0.436	5.7	LOSA	2.6	18.8	0.55	0.58	0.55	52
6	R2	4	0.0	0.436	10.4	LOSA	2.6	18.4	0.55	0.57	0.55	53
6u	U	1	0.0	0.436	12.5	LOSA	2.6	18.4	0.55	0.57	0.55	55
Approach		965	1.4	0.436	5.7	LOS A	2.6	18.8	0.54	0.58	0.54	52
North:	Austool I	Place										
7	L2	21	0.0	0.142	11.4	LOSA	0.7	4.8	0.82	0.90	0.82	48
8	T1	31	0.0	0.142	11.4	LOSA	0.7	4.8	0.82	0.90	0.82	49
9	R2	93	1.1	0.186	14.4	LOSA	1.0	7.2	0.85	0.95	0.85	45.
9u	U	1	0.0	0.186	16.4	LOS B	1.0	7.2	0.85	0.95	0.85	47
Appro	ach	145	0.7	0.186	13.3	LOSA	1.0	7.2	0.84	0.93	0.84	46.
West:	Henders	on Road										
10	L2	29	28.6	0.709	9.7	LOSA	8.1	57.6	0.84	0.87	1.01	46.
11	T1	1173	1.5	0.709	9.1	LOSA	8.1	57.6	0.85	0.89	1.03	50
12	R2	186	19.2	0.709	14.8	LOS B	7.8	57.7	0.85	0.92	1.05	47
12u	U	1	0.0	0.709	16.2	LOS B	7.8	57.7	0.85	0.92	1.05	48
Appro	ach	1389	4.5	0.709	9.9	LOS A	8.1	57.7	0.85	0.89	1.03	50
All Va	hicles	3491	3.6	0.709	9.5	LOSA	8.1	57.7	0.75	0.84	0.90	49

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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♥ Site: 101 [2018AM + dev]

Lancaster Street / Aero Road, Ingleburn Site Category: (None)

Roundabout

Move	ment Pe	erformano	e - Veh	icles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued		Aver. No. Cycles	Average Speed km/h
South	: Lancast	er Street										
1	L2	34	9.4	0.205	5.5	LOSA	1.1	8.8	0.42	0.55	0.42	49.4
2	T1	168	11.9	0.205	5.8	LOSA	1.1	8.8	0.42	0.55	0.42	53.2
3	R2	12	18.2	0.205	10.0	LOSA	1.1	8.8	0.42	0.55	0.42	52.5
3u	U	1	0.0	0.205	11.4	LOSA	1.1	8.8	0.42	0.55	0.42	54.7
Appro	ach	215	11.8	0.205	6.0	LOS A	1.1	8.8	0.42	0.55	0.42	52.7
East: /	Aero Roa	d										
4	L2	16	13.3	0.132	6.8	LOSA	0.7	5.5	0.55	0.71	0.55	49.3
5	T1	6	0.0	0.132	6.7	LOSA	0.7	5.5	0.55	0.71	0.55	46.9
6	R2	91	18.6	0.132	11.3	LOSA	0.7	5.5	0.55	0.71	0.55	48.9
6u	U	1	0.0	0.132	12.6	LOSA	0.7	5.5	0.55	0.71	0.55	50.5
Appro	ach	114	16.7	0.132	10.4	LOS A	0.7	5.5	0.55	0.71	0.55	48.9
North:	Lancaste	er Street										
7	L2	211	11.5	0.410	4.5	LOSA	3.0	22.3	0.18	0.48	0.18	51.9
8	T1	302	3.5	0.410	4.6	LOSA	3.0	22.3	0.18	0.48	0.18	54.3
9	R2	97	3.3	0.410	8.6	LOSA	3.0	22.3	0.18	0.48	0.18	51.0
9u	U	1	0.0	0.410	10.5	LOSA	3.0	22.3	0.18	0.48	0.18	54.5
Appro	ach	611	6.2	0.410	5.2	LOS A	3.0	22.3	0.18	0.48	0.18	53.1
West:	Aero Roa	ad										
10	L2	40	5.3	0.057	5.7	LOSA	0.3	2.2	0.45	0.59	0.45	48.6
11	T1	2	0.0	0.057	5.8	LOSA	0.3	2.2	0.45	0.59	0.45	50.5
12	R2	13	33.3	0.057	10.6	LOSA	0.3	2.2	0.45	0.59	0.45	49.5
12u	U	1	0.0	0.057	11.7	LOSA	0.3	2.2	0.45	0.59	0.45	47.0
Appro	ach	56	11.3	0.057	6.9	LOS A	0.3	2.2	0.45	0.59	0.45	48.9
All Vel	hicles	995	8.9	0.410	6.1	LOSA	3.0	22.3	0.29	0.53	0.29	52.3

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab). Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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₩ Site: 101 [2018PM + dev]

Lancaster Street / Aero Road, Ingleburn Site Category: (None) Roundabout

Move	ment P	erformano	e - Veh	icles								
Mov ID	Turn	Demand Total veh/h	Flows HV %	Deg. Satn v/c	Average Delay sec	Level of Service	95% Back Vehicles veh	of Queue Distance m	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed km/h
South	: Lancast	er Street										
1	L2	9	0.0	0.432	6.2	LOS A	3.0	22.1	0.61	0.64	0.61	49.2
2	T1	407	4.7	0.432	6.6	LOSA	3.0	22.1	0.61	0.64	0.61	52.5
3	R2	20	15.8	0.432	10.9	LOSA	3.0	22.1	0.61	0.64	0.61	51.7
3u	U	1	0.0	0.432	12.3	LOSA	3.0	22.1	0.61	0.64	0.61	53.8
Appro	ach	438	5.0	0.432	6.8	LOS A	3.0	22.1	0.61	0.64	0.61	52.4
East:	Aero Roa	ıd										
4	L2	37	11.4	0.272	6.7	LOSA	1.6	12.3	0.57	0.73	0.57	49.4
5	T1	6	16.7	0.272	7.1	LOSA	1.6	12.3	0.57	0.73	0.57	45.5
6	R2	201	14.7	0.272	11.1	LOSA	1.6	12.3	0.57	0.73	0.57	49.1
6u	U	1	0.0	0.272	12.5	LOSA	1.6	12.3	0.57	0.73	0.57	50.5
Appro	ach	245	14.2	0.272	10.3	LOS A	1.6	12.3	0.57	0.73	0.57	49.1
North:	Lancast	er Street										
7	L2	82	23.1	0.302	4.8	LOSA	2.0	15.4	0.24	0.48	0.24	51.2
8	T1	258	8.6	0.302	4.8	LOSA	2.0	15.4	0.24	0.48	0.24	53.8
9	R2	64	4.9	0.302	8.7	LOSA	2.0	15.4	0.24	0.48	0.24	50.4
9u	U	1	0.0	0.302	10.6	LOSA	2.0	15.4	0.24	0.48	0.24	54.1
Appro	ach	405	10.9	0.302	5.4	LOS A	2.0	15.4	0.24	0.48	0.24	52.9
West:	Aero Roa	ad										
10	L2	97	2.2	0.174	8.3	LOSA	1.0	7.5	0.71	0.77	0.71	46.0
11	T1	2	0.0	0.174	8.5	LOSA	1.0	7.5	0.71	0.77	0.71	47.6
12	R2	28	7.4	0.174	12.8	LOSA	1.0	7.5	0.71	0.77	0.71	48.4
12u	U	1	0.0	0.174	14.3	LOSA	1.0	7.5	0.71	0.77	0.71	43.0
Appro	ach	128	3.3	0.174	9.4	LOS A	1.0	7.5	0.71	0.77	0.71	46.6
All Ve	hicles	1217	8.7	0.432	7.3	LOSA	3.0	22.1	0.49	0.62	0.49	51.4

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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♥ Site: 101 [2028AM + dev]

Lancaster Street / Aero Road, Ingleburn Site Category: (None)

Roundabout

Move	Turn	Damand	Поли	icles	Augrage	Lovel of	OEO/ Deal	of Ougue	Dron	⊏ffo otio	Avor Na	Augrana
Mov ID	Turn	Demand Total	Flows	Deg. Satn	Average Delay	Level of Service	95% Back Vehicles	of Queue Distance	Prop. Queued	Effective Stop Rate	Aver. No. Cycles	Average Speed
		veh/h	%	v/c	sec		veh	m			-,	km/h
South	: Lancast	er Street										
1	L2	37	8.6	0.229	5.6	LOSA	1.3	10.1	0.45	0.56	0.45	49.3
2	T1	185	11.9	0.229	6.0	LOSA	1.3	10.1	0.45	0.56	0.45	53.1
3	R2	13	16.7	0.229	10.1	LOSA	1.3	10.1	0.45	0.56	0.45	52.4
3u	U	1	0.0	0.229	11.6	LOSA	1.3	10.1	0.45	0.56	0.45	54.6
Appro	ach	236	11.6	0.229	6.2	LOS A	1.3	10.1	0.45	0.56	0.45	52.5
East:	Aero Roa	d										
4	L2	18	11.8	0.152	7.1	LOSA	8.0	6.4	0.58	0.73	0.58	49.2
5	T1	7	0.0	0.152	7.0	LOSA	8.0	6.4	0.58	0.73	0.58	46.7
6	R2	100	17.9	0.152	11.6	LOSA	8.0	6.4	0.58	0.73	0.58	48.7
6u	U	1	0.0	0.152	12.9	LOSA	8.0	6.4	0.58	0.73	0.58	50.3
Approach		126	15.8	0.152	10.7	LOS A	8.0	6.4	0.58	0.73	0.58	48.7
North:	Lancaste	er Street										
7	L2	232	10.5	0.451	4.5	LOSA	3.5	26.1	0.20	0.48	0.20	51.9
8	T1	333	3.5	0.451	4.6	LOSA	3.5	26.1	0.20	0.48	0.20	54.2
9	R2	106	3.0	0.451	8.6	LOSA	3.5	26.1	0.20	0.48	0.20	50.9
9u	U	1	0.0	0.451	10.5	LOSA	3.5	26.1	0.20	0.48	0.20	54.4
Appro	ach	672	5.8	0.451	5.2	LOS A	3.5	26.1	0.20	0.48	0.20	53.0
West:	Aero Roa	ad										
10	L2	44	4.8	0.064	5.8	LOSA	0.3	2.5	0.47	0.60	0.47	48.5
11	T1	2	0.0	0.064	6.0	LOSA	0.3	2.5	0.47	0.60	0.47	50.4
12	R2	14	30.8	0.064	10.8	LOSA	0.3	2.5	0.47	0.60	0.47	49.6
12u	U	1	0.0	0.064	11.8	LOSA	0.3	2.5	0.47	0.60	0.47	46.8
Appro	ach	61	10.3	0.064	7.0	LOS A	0.3	2.5	0.47	0.60	0.47	48.8
All Ve	hicles	1095	8.5	0.451	6.2	LOSA	3.5	26.1	0.32	0.54	0.32	52.2

Site Level of Service (LOS) Method: Delay (RTA NSW). Site LOS Method is specified in the Parameter Settings dialog (Site tab).

Vehicle movement LOS values are based on average delay per movement.

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Roundabout Capacity Model: SIDRA Standard.

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Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

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